DATE <u>07/05/200</u>	Columbia County	Building Permit	PERMIT
	This Permit Expires One Ye		000025990
	LIUS HARDEN	PHONE 904.964.7349	<del></del>
ADDRESS 56		STARKE 294 759 7916	FL 32091
	MOTHY & ALLISON SCOTT	PHONE 386.758.7818	_
ADDRESS 19		LAKE CITY	<u> </u>
CONTRACTOR	JULIUS HARDEN	PHONE 904.964.7349	-
LOCATION OF PR	300' ON R.	VATER GLN,TL NEXT TO FIRE DEPT	
TYPE DEVELOPM	MENT SFD/UTILITY ES	TIMATED COST OF CONSTRUCTION	153750.00
HEATED FLOOR	AREA 3076.00 TOTAL ARE	EA <u>4047.00</u> HEIGHT	27.00 STORIES 1
FOUNDATION	CONCRETE WALLS FRAMED F	ROOF PITCH 7'12 F	LOOR CONC
LAND USE & ZON	NING A-3	MAX. HEIGHT	35
Minimum Set Back	Requirments: STREET-FRONT 30.00	REAR 25.00	SIDE 25.00
NO. EX.D.U.	FLOOD ZONE X	DEVELOPMENT PERMIT NO.	
PARCEL ID 34-	-2S-16-01841-011 SUBDIVISIO	N	
LOT BL	OCK PHASE UNIT _	TOTAL ACRES	0.13
	CGC1510843	x(4.1. £	400
Culvert Permit No.	Culvert Waiver Contractor's License Nun	nber Applicant/Owne	Contractor
EXISTING	07-0467-N BLK	JTH	N
Driveway Connection	on Septic Tank Number LU & Zonir	ng checked by Approved for Issuar	nce New Resident
COMMENTS: 1 F	OOT ABOVE ROAD. LEGAL NON-CONFORMIN	G LOT OF RECORD BY PARCELS BE	ING
CREATED AROUN	ND THIS ONE. NOC ON FILE.		
		Check # or 0	Cash 420
	FOR BUILDING & ZONIN	IG DEPARTMENT ONLY	(footer/Slab)
Temporary Power	Foundation	Monolithic	(100101/2142)
	date/app. by	date/app. by	date/app. by
Under slab rough-in	plumbing Slab		
	premong sido _	<del></del>	g/Nailing
Framina	date/app. by	date/app. by	g/Nailingdate/app. by
Framing	date/app. by  Rough-in plumbing ab	<del></del>	date/app. by
	date/app. by  Rough-in plumbing at date/app. by	date/app. by bove slab and below wood floor	date/app. by
	date/app. by  Rough-in plumbing abdate/app. by	date/app. by	date/app. by
	date/app. by  Rough-in plumbing abdate/app. by  Heat & Air Duct date/app. by  C.O. Final	date/app. by  bove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert	date/app. by  date/app. by  tel)  date/app. by
Electrical rough-in	date/app. by  Rough-in plumbing ab date/app. by  Heat & Air Duct  date/app. by  C.O. Final	date/app. by  bove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert	date/app. by  date/app. by
Electrical rough-in	date/app. by  Rough-in plumbing abdate/app. by  Heat & Air Duct date/app. by  C.O. Final	date/app. by  bove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool	date/app. by  date/app. by  tel)  date/app. by  date/app. by
Electrical rough-in	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct date/app. by  C.O. Final date/app. by  king, electricity and plumbing  Pump pole	date/app. by  bove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole	date/app. by  date/app. by  tel)  date/app. by  date/app. by  date/app. by
Electrical rough-in Permanent power M/H tie downs, bloc	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct date/app. by  C.O. Final date/app. by  king, electricity and plumbing  Pump pole	date/app. by  pove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole  date/app. by  date/app.	date/app. by  date/app. by  tel)  date/app. by  date/app. by  date/app. by
Electrical rough-in Permanent power M/H tie downs, bloc Reconnection	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  cking, electricity and plumbing  date/app  Pump pole  date/app. by  Travel Trailer	date/app. by  bove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole	date/app. by  date/app. by  tel)  date/app. by  date/app. by  date/app. by
Electrical rough-in Permanent power M/H tie downs, bloc Reconnection M/H Pole	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  date/app.  Pump pole  date/app. by  Travel Trailer	date/app. by  pove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole  /app. by  Re-roof  late/app. by	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Electrical rough-in Permanent power M/H tie downs, bloc Reconnection M/H Pole date/ap	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct  C.O. Final  date/app. by  cking, electricity and plumbing  date/app  Pump pole  date/app. by  Travel Trailer	date/app. by  pove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole  /app. by  Re-roof  late/app. by  E \$ 20.23 SURCHARG	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Electrical rough-in Permanent power M/H tie downs, bloc Reconnection M/H Pole date/ap BUILDING PERMI	date/app. by  Rough-in plumbing at date/app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  date/app.  Pump pole  date/app. by  Travel Trailer  Tree \$ 770.00 CERTIFICATION FE	date/app. by  pove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do. by  Utility Pole  /app. by  Re-roof  late/app. by  E \$ 20.23 SURCHARG	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  SE FEE \$ 20.23  TE FEE \$
Electrical rough-in Permanent power M/H tie downs, bloc Reconnection M/H Pole date/ap BUILDING PERMI MISC. FEES \$	date/app. by  Rough-in plumbing about date/app. by  Heat & Air Duct date/app. by  C.O. Final date/app. by  Rough-in plumbing about date/app. by  C.O. Final date/app. by  Cate/app. date/app. date/app. date/app. by  Travel Trailer  Tree \$ 770.00 CERTIFICATION FE. date/app. date/app. date/app. date/app. date/app. by  The Sample of the Samp	date/app. by  pove slab and below wood floor  Peri. beam (Lindate/app. by  Culvert  date/app. by  Pool  Do by  Utility Pole  /app. by  Re-roof  late/app. by  E \$	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  SE FEE \$ 20.23  TE FEE \$

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

#### This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

WARRANTY DEED

This Warranty Deed Made the 14TH day of SEPTEMBER A. D. 2006 ISIAH L. PARNELL, WHOSE MAILING ADDRESS IS U.S. EMBASSY MEXICO, P.O. BOX 9000 by BROWNSVILLE, TEXAS 78520

hereinafter called the grantor, to TIMOTHY D. SCOTT AND ALLISON L. SCOTT AS HUSBAND AND WIFE

whose post office address is P.O. BOX 205, WHITE SPRINGS, FL hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

**Witnesseth:** That the grantor, for and in consideration of the sum of \$ 13,500 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, and remises, releases, conveys and confirms unto the grantee, all that certain land situate in COLUMBIA County, Florida, viz:

SEE ATTACHED EXHIBIT "A"

Inst:2007002701 Date:02/02/2007 Time:16:57

Doc Stamp-Deed :

94.50

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our pre	esence:	
Christopher a. Lamber	I feat 2	(2000) (L.S.
Christopher A. lambert	MEXICO CLUB TO	PARNELL .
STATE OF Paderal District Las (National District Las of Marion		BASSY MARICO TX 9000 Brownsv:14, TX 78520
COUNTY OF Embassy of the United States of America	78620	SPACE BELOW FOR RECORDERS USE
I HEREBY CERTIFY that on this day,	before me, an officer duly	
authorized in the State aforesaid and in the County	v aforesaid to take acknowl-	

edgments, personally appeared ISIAH L. PARNELL

to me known to be the person or produced identification , described in and who executed the foregoing instrument and acknowledged before me that HE executed the same, and did/did not

WITNESS my hand and official seal in the County and State last aforesaid this 10 2007 day of A. D. 20

This Instrument prepared by JORGE E. LIZARRALDE U.S. emensy Address VICE CONSUL Mexico

> PO BX 9000 Brownsville, TX 78520

### **Columbia County Building Permit Application**

For Office Use Only Application # 070(2-58) Date Received 6/8 By JW Permit # 25990
Application Approved by - Zoning Official Date 03.07.07 Plans Examiner OK 77#1-2-20 Date 7-2-07
Flood Zone Development Permit NA Zoning A-3 Land Use Plan Map Category A-3
Commense STO PLAN ON PLANS Legal Abancaning Lat of Recard by parcels being weels
□ NOC (F)EH □ Deed or PA □ Site Plan □ State Road Info □ Parent Parcel # □ Development Pern
Fax 904 - 964 - 3353
Name Authorized Person Signing Permit JUIUS Harden Phone 904-964-7349
Address 5609 N.W. 117 St., STACKE, 41 32091 904-173-3662
Owners Name 11mothy and Allison Scott Phone 386-758-7818
911 Address 195 N.W. White water Glenn Lakecity, El 32055
Contractors Name Millennium Mach, Inc. Phone 904-9104-7349
Address PO BOX 280 Starke, FI 32091
Fee Simple Owner Name & Address_ 1897 Same as a bove,
Bonding Co. Name & Address DIA
Architect/Engineer Name & Address $\square A$
Mortgage Lenders Name & Address Monticello Bank 3275. Main Street Fitzgerald.
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progressive Energy ID Number & 34-25-16-01841-011
Property ID Number R-34-25-16-01841-011  Estimated Cost of Condition 12-250, 2000
Subdivision Name 1/A
Driving Directions to 10 HIVIN 41 MAY A 2 Lot Block Unit Phase
turn left on white water blenn next to five department
property is 300 feet on right.
Type of Condensation Melvis (I)
Number of Existing Dwellings on Property NOTA
Actual Distance of Structure from Branch at 115
TATAL KOOT PITCH / /2
Application is hereby made to obtain a normit to the
installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information
and the state of t
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY, IF YOU INTEND TO ORTAIN THE NAME OF THE PROPERTY OF THE PRO
TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT
A. S.
Owner Builder or Authorized Person by Noterized Latter Pontractor State
LAURIE HODSON CONTRACTOR SIGNATURE
COLUNTY OF COLUMN EXPIRES: June 28, 2008 Competency Card Number
Sworn to (or affirmed) and subscribed before me
this day of 20 07.420
Personally known or Produced Identify at
Notary Signature (Revised Sept. 2006)

#### **COLUMBIA COUNTY 9-1-1 ADDRESSING**

P. O. Box 1787, Lake City, FL 32056-1787 PHONE: (386) 758-1125 \* FAX: (386) 758-1365 \* Email: ron\_croft@columbiacountyfla.com

#### **Addressing Maintenance**

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

**DATE REQUESTED:** 

2/26/2007

**DATE ISSUED:** 

3/9/2007

**ENHANCED 9-1-1 ADDRESS:** 

195 NW

WHITEWATER

**GLN** 

LAKE CITY

FL 32055

PROPERTY APPRAISER PARCEL NUMBER:

34-2S-16-01841-011

Remarks:

Address Issued By

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

Approved Address

MAR 0 9 2007

911Addressing/GIS Dept

662

SECTION 34,

DESCRIPTION:

A TRACT OF LAND LYING IN SECTION 34, TOWNSHIP 2 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHEAST CORNER OF COUNTRY LANE ESTATES, A SUBDIVISION AS RECORDED IN PLAT BOOK 5, PAGE 77, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, SAID CORNER ALSO BEING THE INTERSECTION OF THE NORTH LINE OF SAID SECTION 34 AND THE WESTERLY RIGHT OF WAY LINE OF U.S. HIGHWAY 41; THENCE RUN S31'33'56"E ALONG SAID WESTERLY RIGHT OF WAY LINE A DISTANCE OF 391.79 FEET; THENCE S87'48'05"W ALONG THE SOUTH LINE OF LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 744 PAGE 1202 A DISTANCE OF 329.72 FEET TO THE POINT OF BEGINNING; THENCE NOO'50'33"E ALONG THE WEST LINE OF LANDS DESCRIBED IN OFFICIAL MORE OR LESS). FEET; THENCE N80°34'08"E A DISTANCE OF 316.92 FEET TO THE POINT OF BEGINNING. (CONTAINING 1.34 ACRES LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 508 PAGE 811 A DISTANCE OF 318.15 FEET; THENCE S00°52'00"E ALONG THE EAST LINE OF LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 581 PAGE 371 A DISTANCE OF 208.64 RECORDS BOOK 744 PAGE 1202 A DISTANCE OF 161.91 FEET; THENCE S89'03'47"W ALONG THE SOUTH LINE OF

TOGETHER WITH AN EASEMENT FOR INGRESS, EGRESS AND UTILITIES BEING 10 FEET WIDE AND BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE SOUTHEAST CORNER OF COUNTRY LANE ESTATES, A SUBDIVISION AS RECORDED IN PLAT BOOK 5, PAGE 77, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA, SAID CORNER ALSO BEING THE INTERSECTION OF THE NORTH LINE OF SAID SECTION 34 AND THE WESTERLY RIGHT OF WAY LINE OF U.S. HIGHWAY 41; THENCE RUN 414.14 FEET TO THE WESTERLY RIGHT OF WAY LINE OF U.S. HIGHWAY 41; THENCE N31:33'56"W ALONG SAID DISTANCE OF 329.72 FEET; THENCE S80°34'08"W A DISTANCE OF 79.43 FEET; THENÇE N87°48'05"E A DISTANCE OF S31.33'56"E ALONG SAID WESTERLY RIGHT OF WAY LINE A DISTANCE OF 391:79 FEET TO THE POINT OF BEGINNING; WESTERLY RIGHT OF THENCE S87"48"05"W ALONG THE SOUTH LINE OF LANDS DESCRIBED IN OFFICIAL RECORDS BOOK 744 PAGE 1202 A WAY LINE A DISTANCE OF 11.47 FEET TO THE POINT OF BEGINNING.

. . . . Itt



## STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 070 461- N

Scale: Each block represents	feet and 1 inch = 50	- PART II - SITE PLAN- feet.			
		318	<b>S</b> 1		
5.77. GO			30 Tr		161.91
		314.9			WI WhatI
					Llun (
Notes: Tim 5(0T)		·			
	2	0		1	
Site Plan submitted by:  Plan Approved	Cobst w.)	Signature Not Approved		Title  Date 6/12/67	)
ву	04		Columbia	County Health Depa	

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Year T Property	CamaUSA Appraisal Legal Description 01841-011	Maintenance Sel	Co. <b>11792</b>	lumbia Land AG Bldg Xfea	County 001 000 000
SCOTT TIMO	OTHY D & ALLISON L		11792	TOTAL	В*
3 LINE OF SEC, 5 W 329.72 FT 7 208.64 FT, E 9 11 13 15 17 19 21 23 25		ORB 1108-2063	91.79 FT, FT, SOUTH	6 8 10 12 14 16 18 20 22 24 26 28	

STATE OF FLORIDA AC# 3 L32 3 72
DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION

CGC1510843 03/12/07 000000000
CERTIFIED GENERAL CONTRACTOR
HARDEN, JULIUS EDWARD JR
MILLERNIUM MECHANICAL INC

IS CERTIFIED under the provisions of Ch. 489 Fs. majoration data. AUG 31, 2008 [AUG031200358]

Project Name:

Address:

City, State:

**Timothy Scott** 

195 NW Whitewater Glen

Lake City, FL 32055-

#### FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Permit Number:

Permitting Office: COLUMS

Climate Zone: Central	cott	Jurisdiction Number: 2	21000
	New Single family  1 4 No 3076 ft²  Single Pane Double	12. Cooling systems a. Central Unit b. N/A c. N/A  13. Heating systems a. Electric Heat Pump b. N/A c. N/A  14. Hot water systems a. Electric Resistance b. N/A  c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)  15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	Cap: 60000.0 kBtu/hr SEER: 13.00
Glass/Floor Areas	Total as-built po	pints: 29789	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

Glass/Floor Area: 0.07

PREPARED BY:

DATE: 6-18-07

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: DATE:

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



<b>BUILDING OFFICIAL:</b>	
DATE:	

**PASS** 

Total base points: 42351

#### **SUMMER CALCULATIONS**

## Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055-

PERMIT #:

	BAS	E E				AS-	BIII	I T				
GLASS TYPE	-5					7.0	00			-:-	_	
.18 X Condi Floor	tioned X E	BSPM =	Points	Type/SC		erhang Len	Hgt	Area X	SPM	ΙX	SOF	= Points
.18 30	76.0	25.78	14273.9	Double, Tint	N	2.0	10.0	45.0	20.63	3	0.96	893.0
				Double, Tint	N	2.0	10.0	9.0	20.63	3	0.96	178.6
į				Double, Tint	E		10.0	20.0	45.16	3	0.95	858.3
1				Double, Tint	E		10.0	60.0	45.16	5	0.95	2575.0
1				Double, Tint Double, Tint	S		10.0	12.0	33.69		0.92	370.3
1				Double, Tint	w		10.0 10.0	30.0	40.60		0.95	1156.7
				Double, Tint	w		10.0	20.0 9.0	40.60		0.50	405.3
				Double, Tint	w		10.0	9.0 15.0	40.60 40.60		0.95 0.95	347.0 578.3
					• •			10.0	70.00		0.95	5/6.3
				As-Built Total:				220.0				7362.6
WALL TYPES			= Points	Туре		R-\	∕aiue	Area	х :	SPM	=	Points
Adjacent Exterior	430.0	0.70	301.0	Frame, Wood, Exterior		1	3.0	570.0	1	1.70		969.0
Exterior	2140.0	1.90	4066.0	Frame, Wood, Exterior		1	3.0	320.0	. 1	.70		544.0
				Frame, Wood, Adjacent			3.0	180.0	C	0.60		108.0
				Frame, Wood, Adjacent			3.0	250.0		0.60		150.0
				Frame, Wood, Exterior Frame, Wood, Exterior			3.0	350.0		.70		595.0
				Frame, Wood, Exterior			3.0	710.0		.70		1207.0
				Frame, Wood, Exterior			3.0 3.0	140.0		.70		238.0
Base Total:	2570.0		4367.0	As-Built Total:		1.		50.0 <b>2570.0</b>	1	.70		85.0 <b>3896.0</b>
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	x s	PM	=	Points
Adjacent	21.0	1.60	33.6	Exterior Insulated				144.0	4	.80		691.2
Exterior	168.0	4.80	806.4	Exterior Insulated				24.0		.80		115.2
				Adjacent Insulated				21.0		.60		33.6
Base Total:	189.0		840.0	As-Built Total:				189.0				840.0
CEILING TYPE			= Points	Туре	R	-Value	Ar	ea X SI	PM X	SCI	<b>VI</b> =	Points
Under Attic	3076.0	2.13	6551.9	Under Attic		30	0.0 3	076.0 2.1	3 X 1.	00		6551.9
Base Total:	3076.0		6551.9	As-Built Total:			3	076.0				6551.9
FLOOR TYPES	Area X	BSPM =	Points	Туре		R-Va	alue	Area	X S	PM	=	Points
Slab Raised	290.0(p) 0.0	-31.8 0.00	-9222.0 0.0	Slab-On-Grade Edge Insulation	1	9	.0 29	0.0(p	-31.0	30		-9164.0
Base Total:			-9222.0	As-Built Total:			:	290.0				-9164.0

#### **SUMMER CALCULATIONS**

## Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055- PERMIT #:

	BASE		AS-BUILT									
INFILTRATION	Area X BS	PM = Points		Area X SPM = Points								
Summer Bas		44017.6 60828.3	Summer As-Built Points:	3076.0 14.31 44017.6 <b>53504.0</b>								
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X Sys	stem X Credit = Cooling ciplier Multiplier Points								
60828.3	0.4266	25949.4	53504.0 1.000 (1.087 x 1.150 x 1.00) (53504.0 1.00 1.250 0	0.262 0.902 15833.3 .262 0.902 15833.3								

#### **WINTER CALCULATIONS**

## Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055-

PERMIT #:

	BAS	E			AS	-BUI	LT	-	_				
GLASS TYPE	_										-		
.18 X Condit		BWPM =	Points		Ove	erhang	3						
Floor				Type/SC	Ornt	Len	Hgt	Area 2	X W	/PM	X	WOF	= Points
.18 307	76.0	5.86	3244.6	Double, Tint	N	2.0	10.0	45.0	1	1.29		1.00	506.5
				Double, Tint	N	2.0	10.0	9.0	11	1.29		1.00	101.3
ŀ				Double, Tint	Ε	2.0	10.0	20.0	9	9.46		1.01	191.5
				Double, Tint	Ε	2.0	10.0	60.0	9	9.46		1.01	574.4
1				Double, Tint	S	2.0	10.0	12.0	7	7.70	•	1.02	94.5
				Double, Tint	W	2.0	10.0	30.0	10	0.07	1	1.01	303.6
				Double, Tint Double, Tint	W	12.0	10.0	20.0		0.07		1.09	220.4
				Double, Tint	W	2.0	10.0	9.0		0.07		.01	91.1
				Double, Till	W	2.0	10.0	15.0	10	.07	1	.01	151.8
				As-Built Total:				220.0					2235.2
WALL TYPES	Area >	BWPM	= Points	Туре		R-	Value	Are	а Х	WP	M	=	Points
Adjacent	430.0	1.80	774.0	Frame, Wood, Exterior			13.0	570.0		1.8	0		1026.0
Exterior	2140.0	2.00	4280.0	Frame, Wood, Exterior			13.0	320.0		1.8			576.0
				Frame, Wood, Adjacent			13.0	180.0		1.6			288.0
				Frame, Wood, Adjacent			13.0	250.0		1.60			400.0
				Frame, Wood, Exterior			13.0	350.0		1.80	0		630.0
				Frame, Wood, Exterior			13.0	710.0		1.80	0		1278.0
				Frame, Wood, Exterior			13.0	140.0		1.80	0		252.0
Base Total:	2570.0			Frame, Wood, Exterior			13.0	50.0		1.80	)		90.0
Dase Total.	2570.0		5054.0	As-Built Total:				2570.0					4540.0
DOOR TYPES	Area X	BWPM	= Points	Туре				Area	Х	WP	М	=	Points
Adjacent	21.0	4.00	84.0	Exterior Insulated				144.0		5.10	,		734.4
Exterior	168.0	5.10	856.8	Exterior Insulated				24.0		5.10			122.4
				Adjacent Insulated				21.0		4.00	)		84.0
Base Total:	189.0		940.8	As-Built Total:				189.0					940.8
CEILING TYPE	S Area X	BWPM :	= Points	Туре	R-	Value	Are	a X W	/PM	x w	СМ	=	Points
Under Attic	3076.0	0.64	1968.6	Under Attic		3	30.0 3	076.0	0.64	K 1.00			1968.6
Base Total:	3076.0		1968.6	As-Built Total:			3	076.0					1968.6
FLOOR TYPES	Area X	BWPM =	= Points	Туре		R-V	/alue	Area	Х	WPN	VI	= 1	Points
Slab :	290.0(p) 0.0	-1.9 0.00	-551.0 0.0	Slab-On-Grade Edge Insulation	1		9.0 29	0.0(p		-2.70		-	-783.0
Base Total:			- <b>551.0</b>	As-Built Total:			;	290.0					-783.0

#### **WINTER CALCULATIONS**

## Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055- PERMIT #:

	BASE		AS-BUILT								
INFILTRATION	Area X BWF	PM = Points	Area X WP	M = Points							
Winter Base	3076.0 -0.: Points:	9 <b>795.7</b>	3076.0 -0.2 Winter As-Built Points:	8 -861.3 <b>8040.3</b>							
Total Winter ) Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credi Component Ratio Multiplier Multiplier Multipli (DM x DSM x AHU)	t = Heating							
9795.7	0.6274	6145.8	8040.3 1.000 (1.078 x 1.160 x 1.00) 0.411 0.950 <b>8040.3 1.00 1.250 0.411 0.95</b>								

## **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055- PERMIT #:

	BASE				A	S-BUII	LT		· · · · · · · · · · · · · · · · · · ·			
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier	X Credit :	= Total
4		2564.00		10256.0	50.0	0.90	4		1.00	2507.02	1.00	10028.1
					As-Built To	tal:						10028.1

CODE COMPLIANCE STATUS										
BASE							AS	-BUILT		
Cooling + Points	Heating + Points	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
25949	6146	10256	42351	15833		3928		10028		29789

**PASS** 



#### **Code Compliance Checklist**

#### Residential Whole Building Performance Method A - Details

ADDRESS: 195 NW Whitewater Glen, Lake City, FL, 32055- PERMIT #:

#### 6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration regts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal	
		efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

#### ESTIMATED ENERGY PERFORMANCE SCORE\* = 88.7

The higher the score, the more efficient the home.

Timothy Scott, 195 NW Whitewater Glen, Lake City, FL, 32055-

	-						
1.	New construction or existing		New	12.	Cooling systems		
2.	Single family or multi-family		Single family _	_ a	. Central Unit	Cap: 60000.0 kBtu/hr	
3.	Number of units, if multi-family		1 _	_		SEER: 13.00	_
4.	Number of Bedrooms		4	_ b	. N/A		
5.	Is this a worst case?		No	-			
6.	Conditioned floor area (ft²)		3076 ft²	С	. N/A		(200
7.	Glass area & type	Single Pane	Double Pane				700
	Clear - single pane	0.0 ft <sup>2</sup>	0.0 ft <sup>2</sup>	13.	Heating systems		
	. Clear - double pane	0.0 ft <sup>2</sup>	220.0 ft <sup>2</sup>	_ a.	Electric Heat Pump	Cap: 60000.0 kBtu/hr	
	Tint/other SHGC - single pane	0.0 ft <sup>2</sup>	0.0 ft <sup>2</sup>		-	HSPF: 8.30	en en
d.	Tint/other SHGC - double pane			b	N/A		
8.	Floor types						
a.	Slab-On-Grade Edge Insulation	R=9	.0, 290.0(p) ft	c.	N/A		
b.	N/A						
c.	N/A		_	14.	Hot water systems		_
9.	Wall types				Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Exterior	R=1	13.0, 710.0 ft <sup>2</sup>			EF: 0.90	_
b.	Frame, Wood, Exterior		13.0, 570.0 ft²	b.	N/A	DI . 0.90	
c.	Frame, Wood, Exterior		13.0, 350.0 ft <sup>2</sup>				
d.	Frame, Wood, Exterior		13.0, 320.0 ft²	C.	Conservation credits		
e.	4 Others		620.0 ft²		(HR-Heat recovery, Solar		_
10.	Ceiling types				DHP-Dedicated heat pump)		
a.	Under Attic	R=30	0.0, 3076.0 ft <sup>2</sup>	15.	HVAC credits	PT, CF,	
b.	N/A				(CF-Ceiling fan, CV-Cross ventilation,	11, 61,	_
c.	N/A		_		HF-Whole house fan.		
11.	Ducts				PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. F	R=6.0, 40.0 ft		MZ-C-Multizone cooling,		
	N/A				MZ-H-Multizone heating)		
					M2-11-Waldzone heating)		
_							
I cer	tify that this home has complie	d with the Flo	orida Energy Eff	ficiency	Code For Building		
Con	struction through the above ene	ergy saving fe	atures which wi	II be ins	talled (or exceeded)	THE STAN	
in th	is home before final inspection	. Otherwise, a	a new EPL Disp	lay Car	d will be completed		λ.
base	d on installed Code compliant	features.	•	-	•		B
n	1 01					3	鬒
Buil	der Signature:		Dat	te:		5	31

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar <sup>M</sup>designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction,

City/FL Zip:

contact the Department of Community Affair (Nat \$30) (Wersion: FLRCPB v3.30)

Address of New Home:

### NOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

## THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and inaccordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

RECORDING YOUR NOTICE OF COMMENCEMENT.
Tax Parcel ID Number <u>234-25-16-0/841-0//</u> Permit Number
1. Description of property: (legal description of the property and street address or 911 address)  See a Hached for legal description  911 address - 195 N.W. Whitewater Glenn Lake City, Fl 32055
2. General description of improvement: NEW HOME  inst:200712013384 Date:6/18/2007 Time:3:32 PM  \( \textstyle
3. Owner Name & Address TIMOthy & Allison SCOH PO BOX 205 While Springs  32096 Interest in Property ()
4. Name & Address of Fee Simple Owner (if other than,owner):
5. Contractor Name Millannum Mach, The Phone Number 964-964-7348  Address P. D. Box 280 - Stanke, Fl 3269/  6. Surety Holders Name None Phone Number Af /A
riiole Nulliber 70 / 4
Address
Amount of Bond /// //
7. Lender Name <u>Monticello Bank</u> Phone Number <u>229-83/-9993</u>
Address 327 S. Main Street Fitzgerald GA 31750
8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be
Served as provided by section 718.13 (1)(a) 7; Florida Statutes:  Name
9. In addition to himself/herself the owner designates Julius E, Harda
Millonnian Much Lau to receive a copy of the Lien Notice as provided in Section 713.13 (1) -
(a) 7. Phone Number of the designee 904-964-9349
10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)
THE OWNER MUST SIGN THE NOTICE OF COMMENCEMENT AND NO ONE ELSE MAY BE PERMITTED TO SIGN IN HIS/HER STEAD.  LAURIE HOOSON MY COMMISSION # DD 333503 EXPIRES: June 28, 2008 Bonded Thru Notary Public Underwinters  Sworn to (or affirmed) and subscribed before day of
NOTARY STAMP/SEAL
Signature of Notary

5300.804.61.3340.

	•									
N	otice of Treatmen	t 12703								
Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)										
Address: 536 SE BAYA AVE										
City LAKE CMY	City Lake Cry Phone 752-1703									
Site Location: Subdivision Trus of by Scott ) Set										
Lot # Block#	Permit #	15990 / "								
Address 195 NW Whi	ite hater film									
Product used	Active Ingredient	% Concentration								
Premise	Imidacloprid	0.1%								
☐ <u>Termidor</u>	Fipronil	0.12%								
□ Bora-Care Disc	odium Octaborate Tetrah	ydrate 23.0%								
	0.									
Type treatment:	Soil Wood	d								
Area Treated S	Square feet Linear fe	et Gallons Applied								
3450	3450 338	270cals								
- Duelling -	( (									
	<del>)</del>									
As per Florida Building Co										
termite prevention is used, to final building approval.	iinai exterior treatment si	nall be completed prior								
to man comming approvati										
If this notice is for the fina	l exterior treatment, initia	l this line								
9-6-07	1:45 F	299								
Date	Time Print	Technician's Name								
Remarks:										
romano.										
Applicator - White F	Permit File - Canary	Permit Holder - Pink								

2000 · 中国

#### ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1T7J8228Z0121145300

Truss Fabricator: Anderson Truss Company

Job Identification: 7-138--OWNER BUILDER Timothy Scott -- , \*\*

Truss Count: 43

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

#### Notes:

- Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
- 2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

Drawing

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-A11015EE-GBLLETIN-

# Dof Decemintion

ı	#	Ref Description	Drawing#	Date
	1	39240 H7A	07141017	05/21/07
ı	2	39241 H9B	07141004	05/21/07
ı	3	39242H11B	07141005	05/21/07
ı	4	39243H13A	07141018	05/21/07
ı	5	39244 H11A	07141019	05/21/07
ı	6	39245 H9A	07141021	05/21/07
	7	39246H17AI	07141022	05/21/07
1	8	39247 H17AG	07141023	05/21/07
ı	9	39248H17AE	07141025	05/21/07
1	10	39249 H15A	07141026	05/21/07
	11	39250H17A	07141027	05/21/07
	12	39251 H17AA	07141028	05/21/07
1	13	39252H17AB	07141029	05/21/07
ı	14	39253 H17AC	07141030	05/21/07
ı	15	39254 H17AD	07141031	05/21/07
	16	39255H7B	07141032	05/21/07
ı	17	39256H13B	07141006	05/21/07
ı	18	39257 H15B	07141007	05/21/07
	19	39258H17B	07141008	05/21/07
	20	39259C1	07141033	05/21/07
۱	21	39260 C2	07141009	05/21/07
ı	22	39261 C3	07141010	05/21/07
	23	39262 C4	07141011	05/21/07
	24	3926301	07141034	05/21/07
ı	25	39264 D2	07141012	05/21/07
ı	26	39265 D3	07141045	05/21/07
	27	39266E1	07141013	05/21/07
	28	39267 EGE	07141046	05/21/07
	29	39268FGE	07141047	05/21/07
	30	39269 GGE	07141048	05/21/07
	31	39270CJ1	07141035	05/21/07
	32	39271 HJ7	07141036	05/21/07
	33	39272CJ3	07141014	05/21/07
	34	39273 CJ5	07141015	05/21/07
	35	39274 EJ7	07141016	
	36	39275H17-PB4	07141037	05/21/07



Seal Date: 05/21/2007

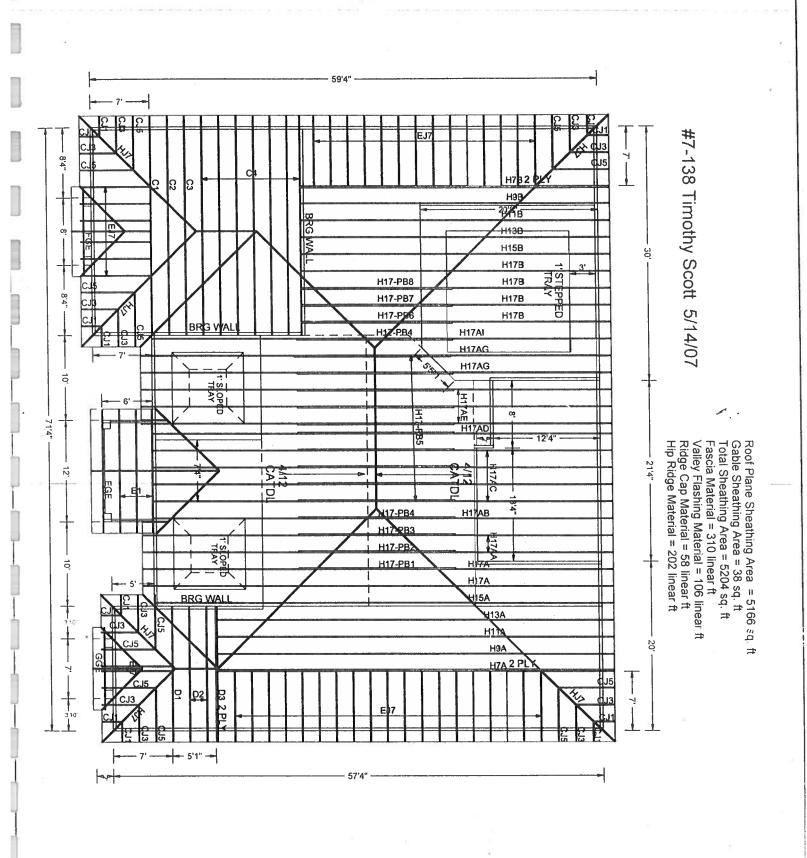
-Truss Design Engineer-Arthur R. Fisher Florida License Number: 59687 1950 Marley Drive Haines City, FL 33844

Drawing#

	- 111	INCI OCSCI IPCIOII	DIGNING	Ducc
	37	39276H17-PB5	07141038	05/21/07
	38	39277H17-PB3	07141039	05/21/07
	39	39278H17-PB2	07141040	05/21/07
8	40	39279H17-PB1	07141041	05/21/07
	41	39280H17-PB8	07141042	05/21/07
	42	39281H17-PB7	07141043	05/21/07
	43	39282H17-PB6	07141044	05/21/07

Description





JOB NO: 7-138 PAGE NO 1 OF 1 JOB DESCRIPTION:: OWNER BUILDER
/: Timothy Scott

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense: Bot chord 2x6 SP #2 Webs 2x4 SP #3 :W12 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure

#1 hip supports 7-0-0 jacks with no webs

COMPLETE TRUSSES REQUIRED

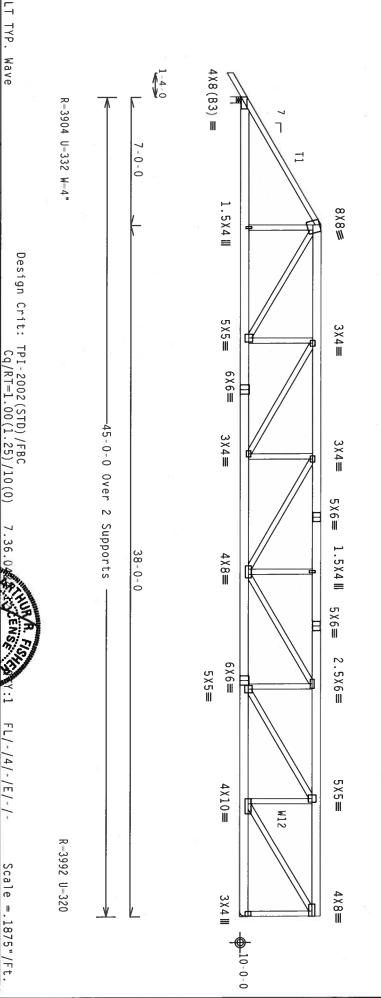
Nailing Schedule: (10d\_Box\_or\_Gun\_(0.128"x3",\_min.)\_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails

in each row to avoid splitting.

In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



PLT TYP.

Wave

Haines City, FL 33844

"Ifficate of Anthonization of Company of Co

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

RESPONSIBILITY OF

SPACING DUR.FAC. TOT.LD.

24.0" 1.25

JREF-

1T7J8228Z01

40.0

SEQN-

25448

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI; OR FARELGATING, MANDING, SHIPPING, INSTALLING A BRACLING OF TRUSSES, BY AFRA, AND TPI. ITM BCG DESIGN CONFIDENS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 20/18/160A (M.H.SYS,N), ASTH AGES GRADE 40/60 (M. K/H.SS) GALV, STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, DIMESS OTHERNISE, LOCATED ON HIS DESIGN, POSITION PER DRAWINGS 160A-2, ANY INSPECTION OF PLATES TOLOWED BY (1) SHALL BE PER NAWE AND TPI1-2002 SEC 3.

ANY INSPECTION OF PLATES TOLOWED BY (1) SHALL BE PER NAWE AND TPI1-2002 SEC 3.

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ANY INSPECTION OF PLATES TOLOWED BY (1) SHALL BE PER NAWE AND TPI1-2002 SEC 3.

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDISHO, SHIPPING, INSTALLING AND BRACING, REFER TO BESSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ERRIERPASSE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

7.36.0

FL/-/4/-/E/-/-

Scale = .1875"/Ft. R8228-

lo. 59687

BC DL TC DL TC LL

10.0 PSF

DRW HCUSR8228 07141017

0.0 PSF PSF

HC-ENG DAB/AF

10.0 PSF 20.0 PSF

DATE REF

05/21/07

39240

(7-138--OWNER BUILDER Timothy Scott --

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

(A) Continuous lateral bracing equally spaced on member.

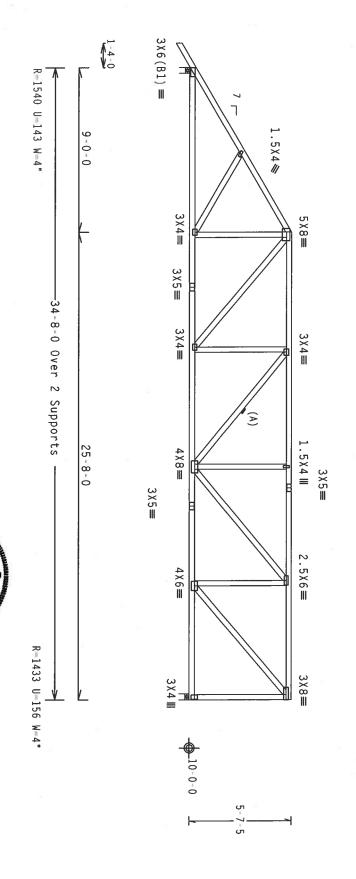
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18 bldg, not located TC DL-5.0 psf, wind

a atticuational constitute of these

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 0C.



\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION. MANDLING. SHIPPING. INSTALLING AND BRACING. REFER TO SECSI. (BUILDING COMPONENT SAFETY IMPORATION). PUBLISHED BY IPT (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 317, ALEXANDRIA, VA, 22314) AND NTCA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMHIG THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO FORDS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SMALL HAVE

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT

TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MAY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FABRICATING, HANDLING, SHIPPIHG, INSTALLING A BRACHEG OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SEC. B. 47 AREA), AND TPI. IT BCG CONNECTOR PLATES ARE HADE OF 20/18/186A (M.H/SS/N) ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF FPI1-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF FIHIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL EDESIGN SHOWN. THE SUITABILLITY AND USE OF THI BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844
Ft Corificate of Authorization # 647

ALPINE

59687 BC DL BC LL TC DL SPACING TC LL DUR.FAC. TOT.LD. FL/-/4/-/E/-/-

24.0" 1.25 JREF -1T7J8228Z01

40.0

PSF

SEQN-

22585

10.0 PSF 20.0 PSF

DATE REF

05/21/07

Scale = .1875"/Ft.

R8228- 39241

10.0 PSF

0.0 PSF

HC-ENG DAB/AF DRW HCUSR8228 07141004

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

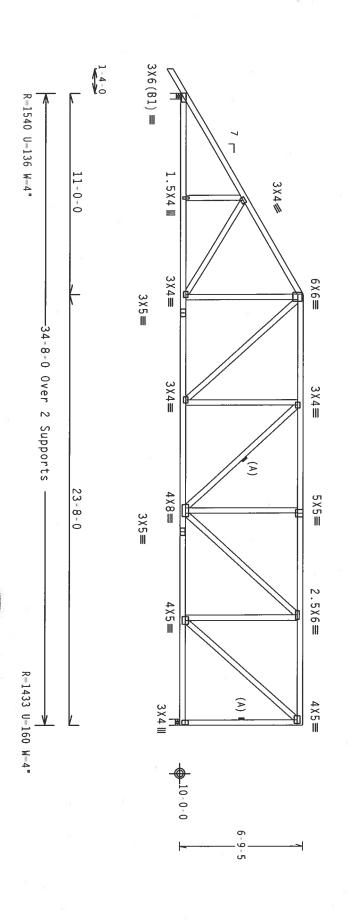
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wind

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 0C. 24"



\*\*#ARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND HTCA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERORISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP COMODO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave

\*\*IMPORTANT\*\*FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. JIM BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FARBLECKING. HANDLING, SHEPPIG, INSTALLING & BRACING OF TRUSSES,

DESIGN COMPORMS HITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

JIM BCG
CONNECTOR PLATES ARE MADE OF ZO/IB/16GA (M.H/SS/K) ASTH A653 GRANDE 40/60 (M. K/H.SS) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND. MURESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF FPII-2002 SEC.3.

A SEAL ON THIS

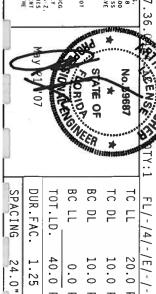
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL B DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL EN DESIGN SHOWN. THE SUITABILITY AND USE OF THIS BUILDING DESIGNER PER ANSI/TPI 1 SEC.

1

ITW Building Components Group, Inc.
Haines City, FL 33844

'ifficate 'orizati "''

ALPINE



40.0

PSF

SEQN-

22581

24.0" 1.25

JREF-

1T7J8228Z01

10.0 PSF 10.0 PSF 20.0 PSF

> DATE REF

05/21/07

Scale = . 1875"/Ft.

R8228- 39242

0.0 PSF

HC-ENG DAB/AF DRW HCUSR8228 07141005

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. (B) 1x4~#3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC. Wind reactions based on MWFRS pressures In lieu of structural panels use purlins to brace all flat TC @ 24" (7-138--OWNER BUILDER Timothy Scott --1-4-0 4X6(B4) =Haines City, FL 33844 R-1969 U-186 W-4" ALPINE Wave 1.5X4 ■ 3-0-0 3X4# \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI: OR FABRICATING, MANDIUMG, SHIPPING, HISTALLING & BRACING OF TRUSSES, BY, 488A) AND TPI. IT BCG CONNECTOR PLATES ARE HADE OF 20/18/166A (N.H/SS/K) ASTM A653 GRADE 40/60 (N. K/H.SS) FOALV-STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. DULCES OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SAA. ON THIS DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. BOACH SHEEL BLOSS COMPONENT DRAWING INDICATES ACCEPTANCE OF TRUSS AND UNESSO THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLAIE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANGRIA, VA. 22314) AND NICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, HI 35719) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. DULCESS OTHERNISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER 3X5/ 3 X 4 ≡ 5 X 8≡ Design Crit: H13A) 3 X 6 ≡ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 45-0-0 Over 2 Supports 3 X 4 ≡  $3 \times 4 =$ 1.5X4 Ⅲ 26-4-0 4×8≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Right end vertical not exposed to wind pressure. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$  $2x4\ \#3$  or better "T" brace. 80% length of web member. Attach with Box or Gun (0.135"x3.5",min.)nails @ 6" 0C. 7.36 3X6≡ 3 X 6≡ (B) &ENS 3×4≡ 4 X 5 ≡ BC DL BC LL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/E/-/-5 X 8≡ 4 X 5 ≡ R=1863 U=185 5-8-0 24.0" 1.25 40.0 20.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF 3 X 4 Ⅲ JREF -SEQN-DATE REF HC-ENG DAB/AF DRW HCUSR8228 07141018 5 X 4 (R) 🐠 Scale = .1875"/Ft. R8228-1T7J8228Z01 25427 05/21/07 39243 \_10-0-0

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

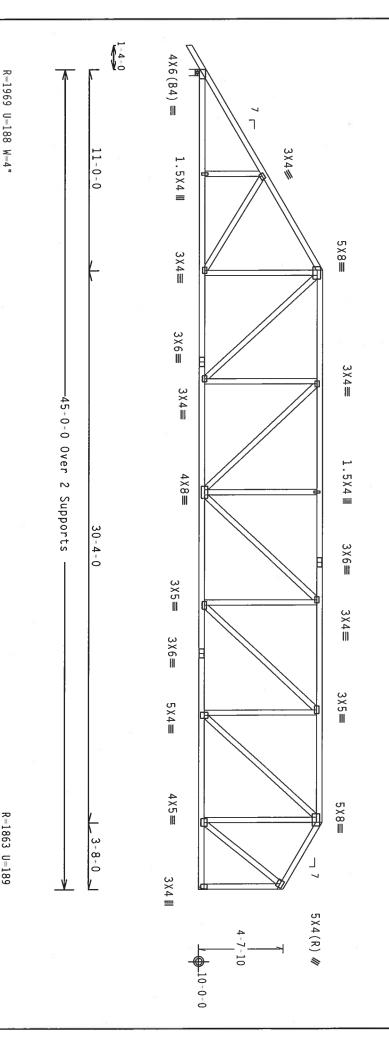
In lieu of structural panels use purlins to brace all flat TC @ 24" (

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP.

Wave

Haines City, FL 33844

BUILDING DESIGNER PER

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. ANY FAILURE TO DUILD THE TRUSS IN COMPORMANCE WITH TP: OR FABRICATING. HANGLUNG, SHAPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS, WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SEC. BY ATERA) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/1666 (M.H/SS/K) ASTH A653 GRADE 40/60 (M.K/M.SS) GALV. STEEL. APPLY LATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE (COATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPI1-2002 SEC. 3. AS SAL ON THIS DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGLYEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN.

THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

0.0 PSF

10.0 PSF 10.0 PSF

DRW HCUSR8228 07141019
HC-ENG DAB/AF

TOT.LD.

PSF

SEQN-

25435

SPACING.

1.25 24.0"

JRFF -

1T7J8228Z01

\*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, INAND.1ME, SHIPPINE, INSTALLING AND BRACING. REFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PRULISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312. ALEXANDRIA, VA. ZZ314) AND WICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDSCAFED OF PROBE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

7.36.

LENS

FL/=/4/=/E/=/=

20.0 PSF

REF

R8228- 39244

Scale =.1875"/Ft.

DATE

05/21/07

(7-138-OWNER BUILDER Timothy Scott -- , \*\* - H9A)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

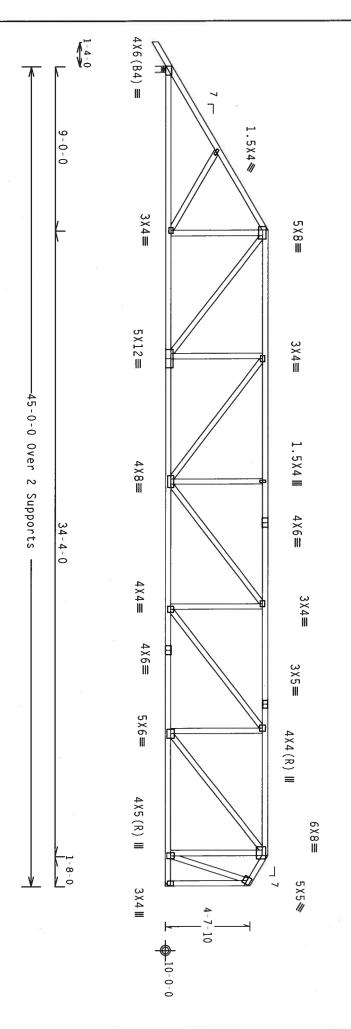
In lieu of structural panels use purlins to brace all flat TC @ 24"

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Right end vertical not exposed to wind pressure

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1969 U=191 W=4"

PLT TYP.

Wave

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

\*\*WARNING\*\* TRUSSES REDURE EXTREME CARE IN FABRICATION, HANDING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCSI (GUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREE, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, HADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE HOUGHAND TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group, Inc. Haines City, FL 33844 or Conficate of Authorization # 547

BUILDING DESIGNER PER

ALPINE

7.36.0 TY:1 FL/-/4/-/E/-/
7.36.0 TY:1 FL/-/4/-/E/-/
7.36.0 TC LL 20.0 I

8.0 LL 0.0 I

9.0 LL 0.

SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF - 1T7J8228Z01		SEQN- 25442	HC-ENG DAB/AF	DRW HCUSR8228 07141021	DATE 05/21/07	REF R8228- 39245

R-1863 U-192

Scale =.1875"/Ft.

(7-138--OWNER BUILDER Timothy Scott

Top chord 2x6 : T2 2x8 SP #1 [
Bot chord 2x6 : Webs 2x4 : SP #2 :T1 2x8 SP SS: Dense: :T6 2x4 SP #2 Dense: SP #2 SP #3

Wind reactions based on MWFRS pressures

 $\Xi$ Continuous lateral bracing equally spaced on member.

> 110 mph wind, 15.32 ft mean hgt, ASCE 7-02, CLOSED within 6.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18 | bldg, not | TC DL-5.0 | located psf, wind

אודה משה ושרושטרה ושמו המוח מורט דווו הו לרהשמת פ מדוורשידתות! המחודוורה מו ושחים וה אי

Calculated horizontal deflection is 0.23\* due to dead load. due to live load and

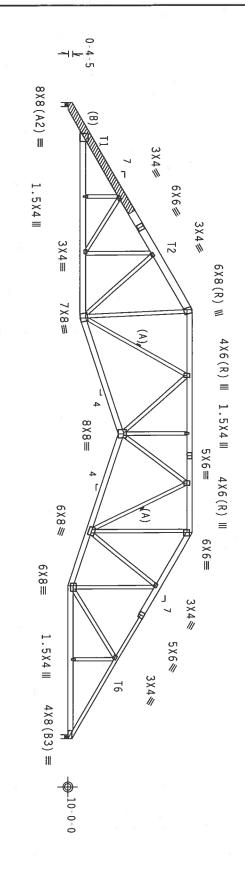
0.38

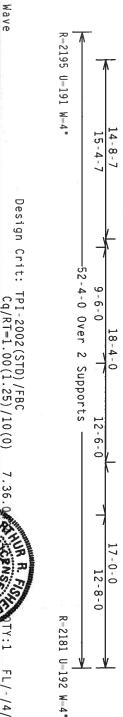
In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(B) point: (3). (2) 2x8x10-5-4 SP SS scabs at left end. Attach one scab to each outer face of chord with: 10d\_Box\_or\_Gun\_(0.128"x3",\_min.)\_nails @ 8" OC, Plus additional nail clusters at : BRG.: (6), heel: (8), 1st panel





\*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, PHARCIAGE, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILCING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NTCA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTEREPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PERFORM THE THESE FUNCTIONS. UNLESS A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

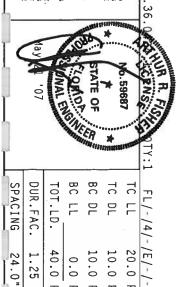
PLT TYP.

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. VAY FAILURE TO BUILD THE TRUSS IN COMPORMANCE MITH FPI. OR FAREIGN. THOULING. SHAPPING. INSTALLING A BRACING OF TRUSSES. PERSONSIBLE FOR ANY DESIGN. CONTRONS WITH APPLICABLE PROVISIONS OF NOS. (MATIONAL DESIGN. SEC. B. \*AFRA) AND TPI. THE RCC. CONNECTOR PLATES ARE HADE OF 20/18/16/36 (M.H/SS/K). ASIM A653 GRADE 40/60 (M. K/M.SS) AGALY. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERSISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANKEX A3 OF TPI1-2002 SEC. 3. A SEAL ON THIS DESIGN SHOWN. THE SUITABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

Haines City, FL 33844
FL Certificate of Amborization # <<7

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

ALPINE



10.0 PSF 20.0 PSF

DATE REF

05/21/07 39246 Scale = .125"/Ft.

R8228-

10.0 PSF

DRW HCUSR8228 07141022

1.25 24.0" JREF-1T7J8228Z01

40.0

PSF PSF

SEQN-

0.0

HC-ENG

DAB/AF 25662

Bot t chord 2x6 t chord 2x6 Webs 2x4 SP #2 SP #3 :T1, T6 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures

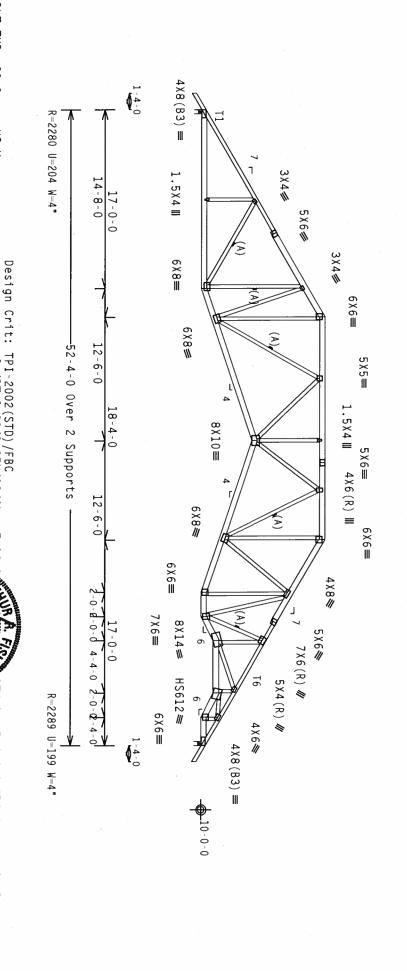
(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 6.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18 bldg, not located TC DL=5.0 psf, wind

Calculated horizontal deflection is 0.20" due to dead load. due to live load and 0.32

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 



PLT TYP.

20 Gauge HS, Wave

ITW Building Components Group, Inc.
Haines City, FL 33844

"" Consistent of Anninging the Components Group, Inc.

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

ALPINE

\*\*IMPORTANT\*\*GURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FARRICATING. HANDLING. SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPORTS WITH APPLICABLE PROYISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (M.H/SS/K) ASTA A653 GRADE 40/60 (M.K.H.SS) GAVE TREEL, APPLY
CONNECTOR PLATES ARE MADE OF 20/18/16GA (M.H/SS/K) ASTA A653 GRADE 40/60 (M.K.H.SS) GAVE TREEL APPLY

PILATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON HIS DESIGN POSITION PER DRAWINGS 16DA-Z. ANY INSPECTION OF FLATES FOLLOHED BY (1) SHALL BE PER MANEX A3 OF IPIT-2002 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

\*\*\*MANNIMG\*\* RRISSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPJ (TRUSS PLATE INSTITUTE, 2)B NORTH LEE SIREET, SUITE 312, ALEXANDRIA, VA, ZE314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DUEESS OTHERWISE INDICATED TO PUROPE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING.

Cq/RT=1.00(1.25)/10(0)

ENS

FL/-/4/-/E/-/-

REF

Scale =.125"/Ft. R8228- 39247

DATE

05/21/07

. 5968

TC DL TC LL

BC LL BC DL

0.0

10.0 PSF 10.0 PSF 20.0 PSF

DRW HCUSR8228 07141023

DAB/AF

40.0

PSF PSF

SEQN-HC-ENG

25673

1.25 24.0"

SPACING DUR.FAC. TOT.LD.

JREF -

1T7J8228Z01

(7-138--OWNER BUILDER Timothy Scott -- , \*\* - H17AE)

וחדים משם נטרו טטרם נטחו פחוו פורט זווו פו (רהטמים פ מזוורטיזמנים) יסמוודוורם מי ואמקים ווווי

Top chord 2x6 SP #2 :T1, T6 2x4 SP #2 Dense: Bot chord 2x6 SP #2 Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

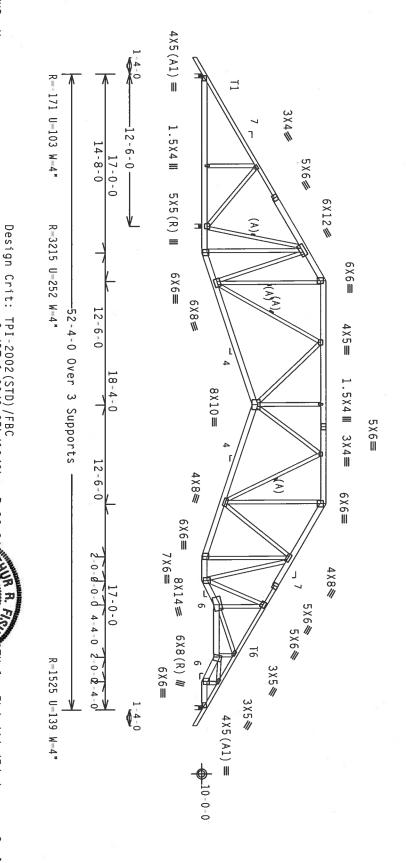
In lieu of structural panels use purlins to brace all flat TC @ 24" O

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING.
REFER TO BCS.I. GBUILDING COMPONENT SAFETY INFORMATION). PUBLICHEDED BY IPI (TRUSS PLATE INSTITUTE. 210
NORTH LEE STREET, SUITE 312, ALEXANDRIA, WA, 22314) AND WICA (MODO TRUSS COUNCIL OF MARENIA. 6300
ENTERRRISE LANE, HADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNIESS
OTHERWISE INDICATED TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

Cq/RT=1.00(1.25)/10(0)

PLT TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, 1TW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FPI; OR FABRICATING, HANDLING, SHAPPING, HYSALLING & BRACING OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY AFEPA), AND TPI;

FLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION ERE DRAMINGS 160A-Z.

ANY INSPECTION OF FLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPI1-2002 SEC.3.

A SEAL ON THIS

DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY OR THE RESPONSIBILITY OF THE

ITW Building Components Group, Inc.
Haines City, FL 33844
Et Conficate of Amborization # 547

BUILDING DESIGNER PER ANSI/TPI 1 SEC.

ALPINE

7.36.04

CENS

TY:1 FL/-/4/-/E/-/
TC LL 20.0 I

TO DL 10.0 I

BC DL 10.0 I

BC DL 0.0 I

BC DL 0.0 I

TOT.LD. 40.0 I

TOT.LD. 40.0 I

BC DUR.FAC. 1.25

BACING 24.0"

L						
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF - 1T7J8228Z01		SEQN- 25753	HC-ENG DAB/AF	DRW HCUSR8228 0714102	DATE 05/21/07	REF R8228- 39248

Scale =.125"/Ft.

H15A)

Bot chord 2x6 SP #2 :T1, T6 2x4 SP #2 Dense: chord 2x6 SP #2 Webs 2x4 SP #3 \*

Wind reactions based on MWFRS pressures.

(A) 1x4~#3 or better "T" brace. 80% length of web member. Attach with Box or Gun (0.113"x2.5",min.)nails @ 6" 0C.

8d

In lieu of structural panels use purlins to brace all flat TC @ 24" 0C.

Calculated vertical deflection is 0.39" due to live load and 0.63" due to dead load at  $X\,=\,27\,\text{-}2\,\text{-}0$  .

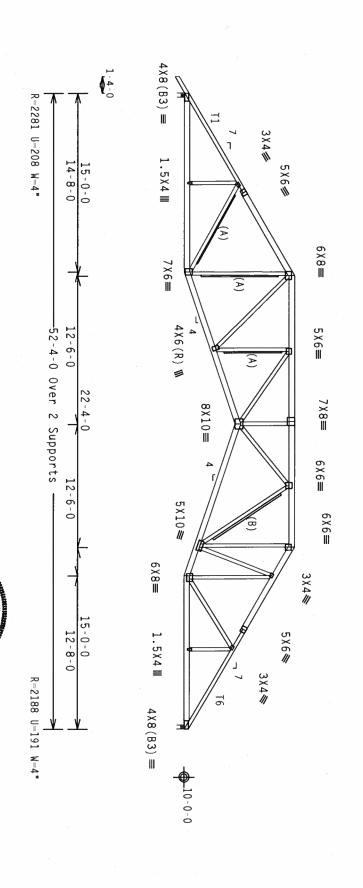
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18 psf, wind

Calculated horizontal deflection is 0.18  $\mbox{\tt m}$  due to dead load. due to live load and 0.29'

16d  $2x6\ \#3$  or better "I" brace. 80% length of web member. Attach with Box or Gun (0.135"x3.5",min.)nails @ 6" 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below. installation



\*\*#ARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDING, SHIPPING, HSKALLING AND BRACING. REFER TO BCS1
REFER TO BCS1
ROBIN LEE SIREE, SUITE 31Z, ALEXANDRIA, VA, ZZ31A) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPAISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORM HG THESE FUNCTIONS. UNLESS OTHERWISE HONGLAGE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

Design Crit:

PLT TYP.

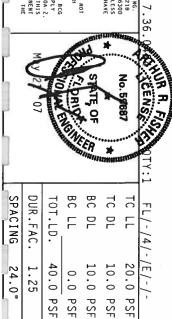
Wave

ALPINE

ITW Building Components Group. Inc. Haines City, FL 33844 FI Codificate of Amborization # 547 \*\*IMPORTANT\*\*FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAW DEVIATION FROM THIS DESIGN, FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FABRICATING, HANDLING, SHPAPING, INSTALLING A BRAFILE OF TRUSSES.

DESIGN COMFORMS HITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFREY) AND TPI. ITM BCG COMMECTOR PLAITES ARE HADE OF 20/18/16/166 (H.H/SS/K), ASIM A653 GRADE 40/60 (M.K/H.SS) GALV. STEEL APPLY PLAIES TO EACH FACE OF TRUSS AND. UNICES OTHERNISE (CCALED ON THIS DESIGN, POSITION PER DRAWHINGS 160A.Z. ANY THIS PECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPIL-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY OND NEED OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER



PSF PSF

SEQN-

25708

HC-ENG DAB/AF DRW HCUSR8228 07141026

JREF -

1T7J8228Z01

PSF

REF

R8228-

Scale =.125"/Ft.

DATE

05/21/07 39249

(7-138--OWNER BUILDER Timothy Scott

Bot chord 2x6 SP #2 :T1, chord 2x6 SP #2 Webs 2x4 SP #3 T6 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures

(A) Continuous lateral bracing equally spaced on member

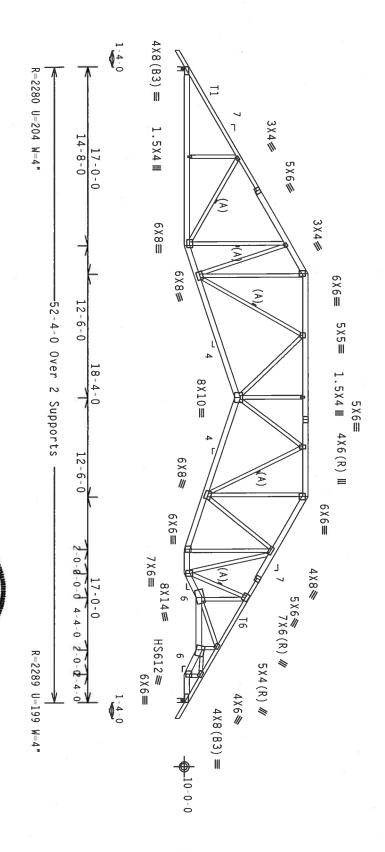
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 6.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wi psf, wind

due to dead load. Calculated horizontal deflection is 0.20" due to live load and 0.32'

In lieu of structural panels use purlins to brace all flat TC @ 24"

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



20 Gauge HS, Wave \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BCSI (BUILDING COMPONENT SAFETY IMPORATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 21B MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA. 6300 EMPERCAPAISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PROBOB SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

Design Crit:

PLT TYP.

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, B. EXPLICABLE PROVISIONS OF INDS. (MATIONAL DESIGN SEC. B. AFREY) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/10/16GA (M.H/SS/K) ASIM A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE (COATED ON THIS DESIGN, POSITION PER DRAWNISS 16GA-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPIL-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN.

DRAWNING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLIFERING RESPONSIBILITY SOLEY FOR THE TRUSS CORPORANT DESIGN SHOWN.

THE SULTABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER

ITW Building Components Group, Inc.
Haines City, FL 33844
Et Codificate of Ambridge # 567

ALPINE

7.36.0 CENS BC DL BC LL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/E/-/-

10.0 PSF 20.0 PSF

DATE REF

05/21/07

Scale = .125"/Ft.

R8228- 39250

10.0 PSF 0.0

DRW HCUSR8228 07141027

PSF PSF

HC-ENG

DAB/AF 25712

40.0

SEQN-

24.0" 1.25

JREF-

117J8228Z01

(7-138--OWNER BUILDER Timothy Scott --

Top chord 2x6 SP #2 :T1, T6 2x4 SP #2 Dense: Bot chord 2x6 SP #2 Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

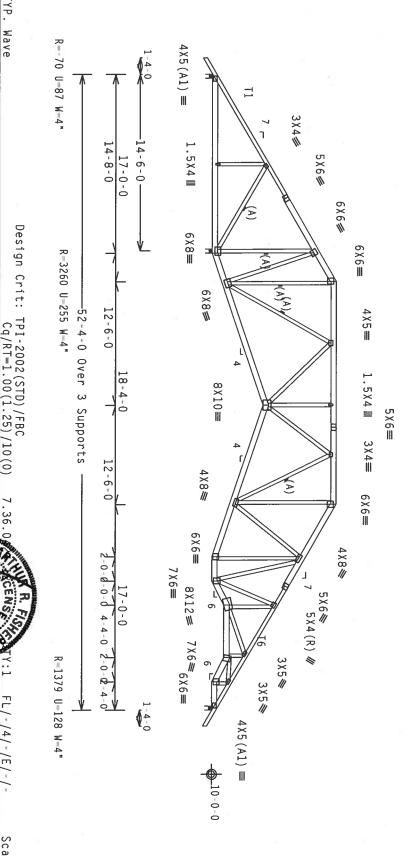
(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 6.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wind

In lieu of structural panels use purlins to brace all OC. @ 0T

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI. QUILOING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND MICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR COMBO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

7.36.

TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY AFRAYAMD IP!. IT BCG CONNECTOR PARTES ARE MADE OF 20/18/1604 (N. H/SSY, M. ASTM A653 GRADE 40/60 (N. K/HSS) 6ALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IPI1-2002 SEC.3. A SEAL ON THIS DRAWINGS INCLUDED BY (1) SHALL BE PER ANNEX A3 OF IPI1-2002 SEC.3. A SEAL ON THIS DRAWINGS INCLUDED BY (1) SHALL BE PER ANNEX A3 OF IPI1-2002 SEC.3. A SEAL ON THIS DRAWINGS INCLUDED BY (1) SHALL BE PER ANNEX A3 OF IPI1-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONEN BUILDING DESIGNER PER ANSI/FP I SEC. 2.

Haines City, FL 33844
FI Contificate of amborization # 547

ALPINE

	May 21 '07	NAME OF THE PARTY	S S S S S S S S S S S S S S S S S S S	TATE OF LEMM	* * * *	No.59687	CENS COLY:1	
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	דט רר	FL/-/4/-/E/-/	
24.0	1.25	40.0	0.0	10.0	10.0	20.0	-/E/-	

DATE

05/21/07

Scale = .125"/Ft. R8228- 39251

JREF -117J8228Z01

PSF PSF PSF PSF PSF

SEQN-

25719

HC-ENG DAB/AF

DRW HCUSR8228 07141028

(7-138--OWNER BUILDER Timothy Scott --H17AB)

Top chord 2x6 SP #2 :T1, T6 2x4 SP #2 Dense: Bot chord 2x6 SP #2 Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat TC @ 24"

OC.

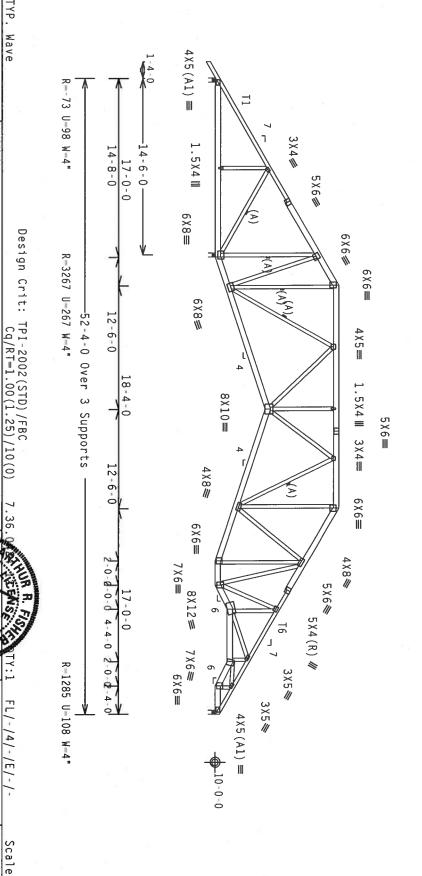
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

נודה מנות וטרוטטרת וטמון ממון מורט דעוומן (רמטמת פ מדוורנותומות) תמחודוורת מו קוומתה וו וויי

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY IMPORATION), PUBLISHED BY FFT (IRUSS PLATE LASTIDUTE, ZIB NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, Z2314) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO GROOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

TATE OF

BC LL BC DL TC DL

0.0 PSF PSF

10.0 PSF 10.0 PSF

DRW HCUSR8228 07141029

05/21/07 39252

DAB/AF 25726

TOT.LD.

40.0

SEQN-HC-ENG . 59687

TC LL

20.0 PSF

REF DATE

Scale =.125"/Ft. R8228-

FL/-/4/-/E/-/-

\*

PLT TYP.

Wave

ITW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Authorization # 567 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IPI: OR FARRICATHING, HANDLIGN, SUPPPING, INSTALLING A BRACKING OF TRUSSES.

OR SABRICATING, HANDLIGN, SUPPPING, INSTALLING A BRACKING OF TRUSSES. BY AGEN) AND TPI.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AGEN) AND TPI.

CONNECTION FAIRES ARE MADE OF ZO/JES JOEGA, WILMESS, STANDAM, BOSTON BOSTON BOSTON PORTON POSITION FOR DRAWHINGS 160A, Z.

PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON HIS DESIGN, POSITION FOR DRAWHINGS 160A, Z.

ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX AS OF FYDI-ZOOZ SEC. 3.

AS SEAL ON THIS SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

ALPINE

DUR.FAC. SPACING 1.25 24.0" JREF -

1T7J8228Z01

Bot chord 2x6 SP #2 :T1. T6 2x4 SP #2 Dense: chord 2x6 SP #2 Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

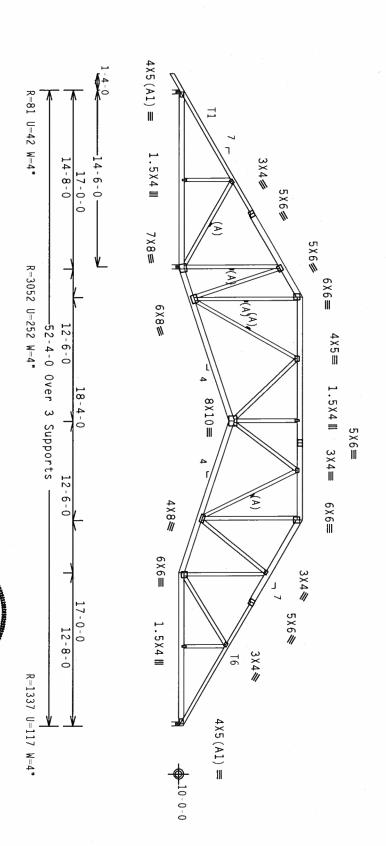
(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 6.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wind

In lieu of structural panels use purlins to brace all flat  $0\ensuremath{\text{C}}\xspace.$ 0 0 J

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ 

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



\*\*HARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIP! (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUIT EIZ, ALEXANDRIA, VA, 22214) AND WITCA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HADISON, NI 53718) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING.

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

PLT TYP.

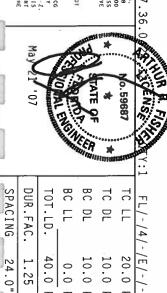
Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BGG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FAREICATING, MANDING. SUPPING, INSTALLING & BRACLING OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC. BY ATAPA) AND TPI. ITH BGG. CONNECTION PARTS ARE MADE OF ZOLDES INGO. ALV. MATIONAL DESIGN SPEC. BY ATAPA) AND TPI. BILL APPLY DELICES TO EACH FACE OF TRUSS. AND. UNLESS OTHERWISE LOCATED ON HIS DESIGN, POSITION FER DRAMHINGS 160A. Z. ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNER X.) OF FPII ZOOZ SEC. 3. AS SEA. ON THIS DRAMHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLNEEDING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group, Inc. Haines City, FL 33844 Et Cortificate of Authorization # 547

ALPINE



			SER WITHIN		IN PROPERTY.	¥ Y : 1
DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL	FL/-/4/-/E/-/-
1.25	40.0 PSF	0.0 PSF	10.0 PSF	.10.0 PSF	20.0 PSF	/E/-/-
	SEQN- 25733	HC-ENG DAB/AF	DRW HCUSR8228 07141030	DATE 05/21/07	REF R8228- 39253	Scale =.125"/Ft.

24.0"

JREF -

1T7J8228Z01

Top chord 2x6 SP #2 :T1, Bot chord 2x6 SP #2 Webs 2x4 SP #3 T6 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures

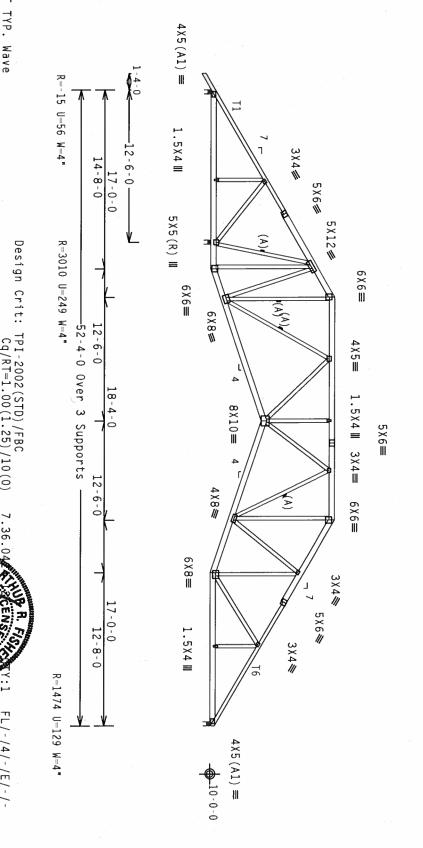
(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

In lieu of structural panels use purlins to brace all flat TC @ 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FARRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSI. (BUILDING COMPONENT SAFETY IMPORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUITE, ZIB MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND NICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPORTSE LANE, MADISON, NI 33719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

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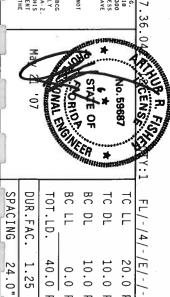
Wave

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DESIGN COMPORES WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI. ITM BCG. CONNECTION PARTES ARE MADE OF POOLEY BLOGA (M. HASSE) GANDE ADJOGO (M. K.H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWHINGS 150A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AIMER AS OF FPII-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI I SEC. 2.

ITW Building Components Group. Inc. Haines City, FL 33844 Ft Cartificate of Authorization # <<7

ALPINE



				"All	12771117	diam.
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF - 1T7J8228Z01		SEQN- 25741	HC-ENG DAB/AF	DRW HCUSR8228 07141031	DATE 05/21/07	REF R8228- 39254

Scale =.125"/Ft

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense: Bot chord 2x6 SP #2 Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 00

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-4-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

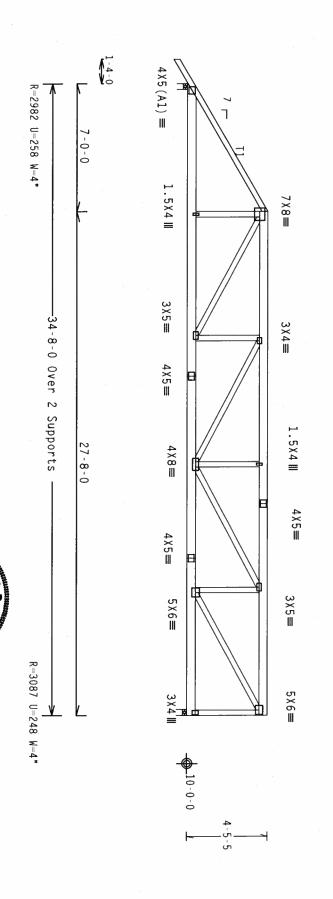
COMPLETE TRUSSES REQUIRED

Nailing Schedule: (10d\_Box\_or\_Gun\_(0.128"x3",\_min.)\_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs: 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure

#1 hip supports 7-0-0 jacks with no webs

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (FRUSS PLATE INSTITUTE, 219 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND MICA (MODO TRUSS COUNCIL OF AMERICA. 6300 ENTERPORTS (LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

Design Crit:

7.36.

SCENSE

PLT TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH 8GG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGNS ANY TAILURE FOR BUILD THE TRUSS IN COMPORMANCE WITH FPI: ON FABRICATING, AMEDICA. SHIPPING, INSTALLING & BRACHING OF TRUSSES. DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY ATAPA) AND TPI. DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY ATAPA) AND TPI. COMPORTS PROVISIONS OF 20/18/16GA (M. H/S/S/X) ASTIN ADDRESS OF HOS (MATIONAL DESIGN SPEC, BY ATAPA) AND TPI. PROVISION FOR DANHINGS SPEC, BY ATAPA AND TRUSSES OF TRUSS AND. UNLESS OTHERWISE (OCATED ON THIS DESIGN, POSITION PER DRAHINGS SPEC, BY ATAPA AND THE SUITABLITY AND SPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMER AND SPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMER AND SPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMER AND SPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMER AND SPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMER AND SPECTION OF PLATES FOR SPECTION OF PROFESSIONAL ENGLINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THE DESIGN PROFESSIONAL ENGLINEERING RESPONSIBILITY SOLELY FOR THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSI/TPI I SEC. 2.

ITW Building Components Group, Inc. Haines City, FL 33844 FI Cortificate of Authorization # 567

ATE OF

SPACING DUR. FAC.

24.0"

JREF -

1T7J8228Z01

	ВС	BC	TC DL	10	1 FL
TOT.LD.		DL	10	רר	FL/-/4/-/E/-/-
40.0 PSF	0.0 PSF 40.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF	/E/-/
PSF	PSF	PSF	PSF	PSF	'
SEQN-	HC-ENG DAB/AF SEQN- 25463	DRW HCUSR8228 07141032	DATE	REF	Scal
	<u> </u>	5		ᅍ	ര
25463	DAB,	ISR822	05/21/07	R8228-	Scale =.1875"/Ft.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W4, W5 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures.

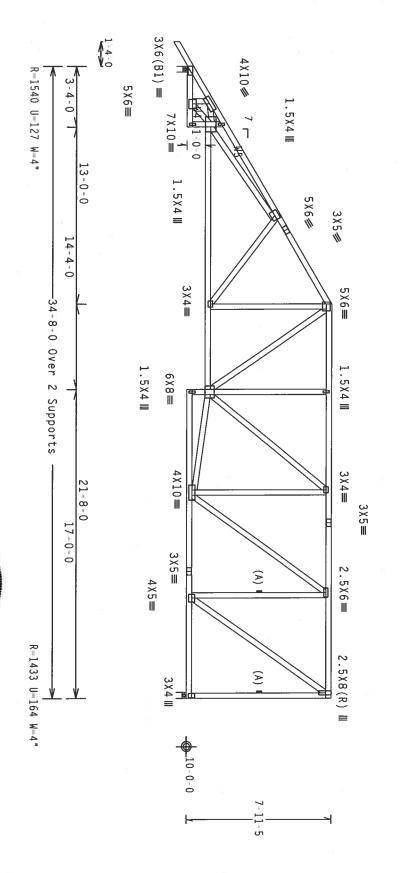
Calculated horizontal deflection is 0.12" due to live load and 0.20" due to dead load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ 

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24"  $\,$  0C. (A) Continuous lateral bracing equally spaced on member.



PLT TYP. Wave Haines City, FL 33844

"ficate" prizatig ALPINE \*\*IMPORTANT\*\*GURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL HOT BERESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; MAY FAILURE TO BUILD THE TRUSS IN COMPONANCE WITH IP: OR FARBLOATHING, MANDLUIG, SHIPPING, INSTALLING & BRACHING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SPEC, BY AFAPA) AND IPI. BITM BCG CONNECTOR PLAIRS ARE MADE TO FO1/1916AG (M.H./SEY), ASTH AGS GANCE SPEC, BY AFAPA) AND FOIL OCCUPANCETOR PLAIRS ARE MADE TO FO1/1916AG (M.H./SEY), ASTH AGS GANCE SPEC, BY AFAPA) AND FOIL THIS CONNECTOR PLAIRS ARE MADE TO FO1/1916AG (M.H./SEY), ASTH AGS GANCE SPEC, BY AFAPA) AND FOIL THIS CONNECTOR PLAIRS ARE MADE TO FO1/1916AG (M.H./SEY), ASTH AGS GANCE SPEC, BY AFAPA) AND FOIL THIS CONNECTOR PLAIRS ARE MADE TO THE PROVINCE OF THE MADE TO THE ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3 DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS TH BUILDING IS SECONDAL PER ANSI/TP1 1 SEC. 2. \*\*\*MARNING\*\*\* TRUSSES REDUIRE ETTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACHNG. REFER TO BOSS! QUILLING COMPONENT SAFETY INFORMATION, DUBLISHED BY TPT (TRUSS PLATE HESTITUTE, 218 MORTH LEE STREE, SUITE 312. ALEXANDRIA, VA. 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ERREPRISE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DHLESS OTHERWISE HOLDCANED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) RADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY THIS DESIGN. POSITION PER DRAMINGS 160A.7 THIS DESIGN. POSITION FER DRAMINGS 10THIS COPPONENT SOLELY FOR THE TRUSS COMPONENT ANY BUILDING IS THE RESPONSIBILITY OF THE 7.36.0 TATE OF 6.5968 BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/E/-/-40.0 10.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF SEQN-DATE REF JREF -DRW HCUSR8228 07141006 HC-ENG DAB/AF Scale =.1875"/Ft. R8228- 39256 1T7J8228Z01

25517

05/21/07

(7 138 OWNER BUILDER Timothy Scott , \*\* H15B)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W2, W4 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures.

Calculated herizontal deflection is 0.12" due to live load and 0.20" due to dead load.

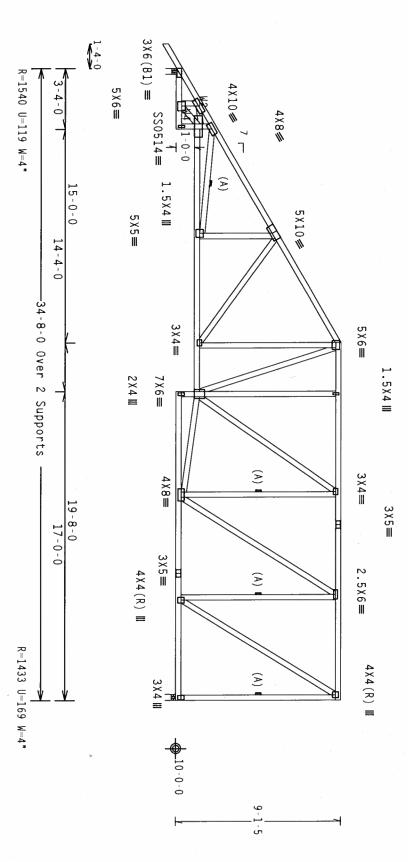
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi (+/-)=0.18

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24° 0C.



PLT TYP. Haines City, FL 33844

"Ificate of a morization of the first of the fi ALPINE 18 Gauge HS, Wave \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE MITH FOIL OR FAREIGNATING, ANDICHIG, SHEPPING, INSTALLING A BRACHEN OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROPISIONS OF DIDS (MATIONAL DESIGN SPEC, BY AFRA) AND TRI. IT BCG CONNECTION FOILTS ARE AND OF 20/18/160A (W H/SSY) ASTM ASS JEADE 40/60 (W, K/H-SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PRE DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF FPII-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BESSON SHOWN. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. \*\*#ARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTAILING AND BRACING. REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE HESTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRESE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PROBOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/ /10(0) ω BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/E/-/-10.0 PSF 20.0 PSF 40.0 PSF 1.25 10.0 PSF 24.0" 0.0 PSF REF JREF -DATE SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141007 Scale = .1875"/Ft. R8228-1T7J8228Z01 25524 05/21/07 39257

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W2, W4 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures

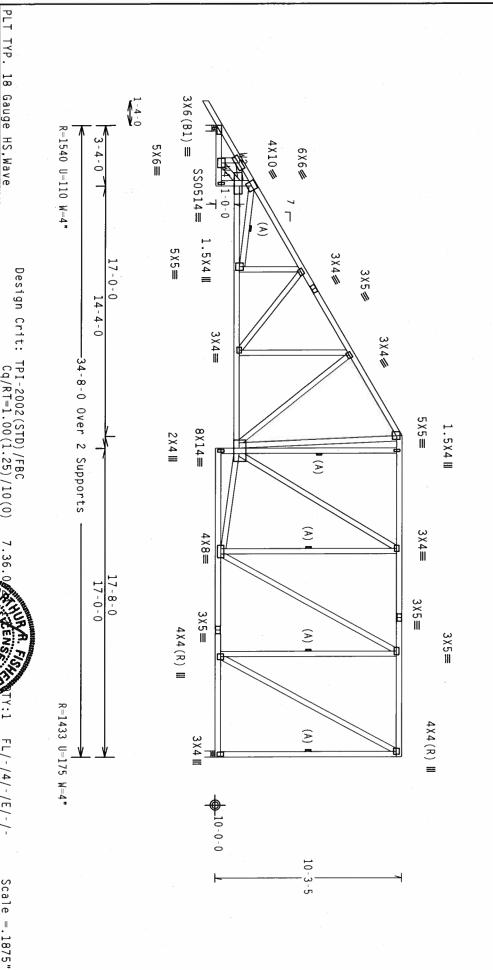
Calculated horizontal deflection is 0.11" due to to dead load. live load and 0.17" due

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wind

Right end vertical not exposed to wind pressure. (A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat IC @ 0C. 24"



PLT TYP.

18 Gauge HS Wave

BUILDING DESIGNER PER ANSI/TPI I SEC.

ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ARY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FDI. OR FAREIGATING, HANDLING. SHEPPING, INSTALLING & BRACHEM OF TRUSSES, DESIGN CONTROLING, SHEPPING, INSTALLING & BRACHEM OF TRUSSES, DESIGN COMPORES, WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AREA), AND TPI. ITM BCG CONNECTOR PLATES ARE ANDE OF 20/18/166A (M.H/SS/K) ASH A653 GRADE 40/60 (M. K/MSS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A. OF TPI1-2002 SEC.3. A STAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

\*\*WARNING\*\* TRUSEES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, HSIAILING AND BRACING, REFER TO BCSI (BUILDING COMPONENT SAFEIY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE HSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE HOLDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

ZENS o. 5968.

FL/-/4/-/E/-/-

Scale =.1875"/Ft.

R8228- 39258

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TC DL TC LL

DATE REF

05/21/07

BC LL BC DL

0.0

HC-ENG DAB/AF

DRW HCUSR8228 07141008

10.0 PSF 10.0 PSF 20.0 PSF

DUR.FAC. SPACING

24.0"

JREF-

1T7J8228Z01

TOT.LD.

40.0 1.25

PSF PSF

SEQN-

25655

Top chord 2x4 SP #2 Dense :T2 2x6 SP #2: Bot chord 2x6 SP #2 Webs 2x4 SP #3 PLT TYP. #1 hip supports 7-0-0 jacks with no webs Wind reactions based on MWFRS pressures. (7 138 OWNER BUILDER Timothy Scott Haines City, FL 33844
FT Conficate of high prization with a second secon ALPINE Wave 1-4-0 3X9(B3) == R=2123 U=190 W=4" \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH IP: OR FARRICATING, HANDLING, SHIPPING, INSTALLING & BRACHING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ANDS. (MATIONAL DESIGN SEC. B. 46 ASA) AND IPI. ITM BCG CONNECTOR PLATES ARE HADE OF 20/18/166A (M.H/SS/K) ASIM A653 GRADE 40/60 (M. K/M.SS) GALV. SIEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERRISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF IPI1 2002 SEC.3. A SEAL ON THIS DESIGN. POSITION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF IPI1 2002 SEC.3. A SEAL ON THIS DESIGN AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF IPI1 2002 SEC.3. A SEAL ON THIS DESIGN AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF IPI1 2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL EN DESIGN SHOWN. THE SUITABILITY AND USE OF THIS BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 HORTH LEE STREET, SUITE 317, ALEXANDRIK, VA. 22314) AND NTCA (MOOD TRUSS COUNCILS OF AMERICA. 6300 ENTERPRISE LANE, MADISON, NI 33719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE 7-0-0 Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 3×4 **Ⅲ** 7X8≡ (1) -24-8-0 Over 2 Supports 12 4X8≡ 1.5X4 Ⅲ 10-8-0 4×4≡ In lieu of structural panels use purlins to brace all flat TC @ 0C. 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf.  $I_W=1.00$  GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ 7.36 7×8≝ 3×4 Ⅲ . 59687 7-0-0 וווז משם בטרבשטרם ושמנו להטבמורט ושבמו לרמשה? פ מזהרשפומש? פספהדוורה מו וצפפים וחשי R-2028 U-166 W-4" 5X5 (A1) ≡ BC LL BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/E/-/-7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF DATE REF SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141033 Scale =.25"/Ft. R8228- 39259 25418 05/21/07

SPACING

24.0"

JREF -

1T7J8228Z01

Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3

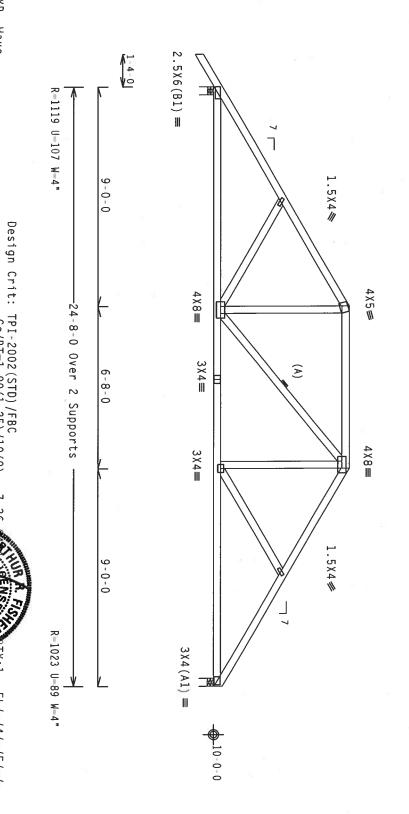
Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat TC @ 24" 0C.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50.\,$ 



\*\*MARN.MG\*\* IRUSES REQUIRE EXTREME CARE IN FARRICATION, HANDLINED BY THE TRUSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CIRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312. ALEXANDRIA, VA. 22314) AND NTCA (MODO TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DUESS OTHERNISE INDICATED TO FORDE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.

Cq/RT=1.00(1.25)/10(0)

PLT TYP. Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, VY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI; OR FABRICATING, HANDLING, SHEPPING, HISTALLING & BRACING OF TRUSSES, X 4784A) AND IPS. IT HE COSIGN COMPORNS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY 4784A) AND IPS. ITH BCG COMMECTION PARTES ARE HADE OF 20/18/1606 (H. H/SS)K) ASTM A653 GRADE 40/60 (H. K/H.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER NAMEX A3 OF 1911-2003 ECC.3. A SAAL ON THIS DRAWING 1001ACTIES ACCEPTANCE OF PROFESSIONAL REGISTER NAMEX A3 OF 1911-2003 ECC.3. A SAAL ON THIS DRAWING 1001ACTIES ACCEPTANCE OF PROFESSIONAL REGISTER NAMEX A3 OF 1911-2003 ECC.3.

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BRANIBG 1001CATIES ACCEPTANCE OF PROFESSIONAL REGISTER NAMEX A3 OF 1911-2003 ECC.3.

BRANIBG 1001CATIES ACCEPTANCE OF PROFESSIONAL REGISTER NAMEX A3 OF 1911-2003 ECC.3.

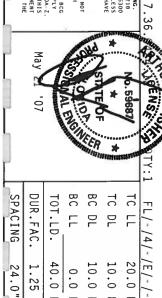
BRANIBG 1001CATIES ACCEPTANCE OF PROFESSIONAL REGISTER NAMEX A3 OF 1911-2003 ECC.3.

CONNECTOR PLATES ARE THE TRUES AND. UNLESS THE PER ANDED ANY LASECTION OF PLATES TO EACH FACE FROUS AND. UNLESS THE PER ANDED ANY LASECTION OF PROFESSIONAL ENGINEERING THE PER AND USE OF THIS COMPONENT

ITW Building Components Group, Inc.
Haines City, FL 33844

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ALPINE



40.0 PSF

SEQN-

22505

0.0 PSF

HC-ENG DAB/AF DRW HCUSR8228 07141009

24.0" 1.25

JREF-

1T7J8228Z01

10.0 PSF 10.0 PSF 20.0 PSF

> DATE REF

05/21/07

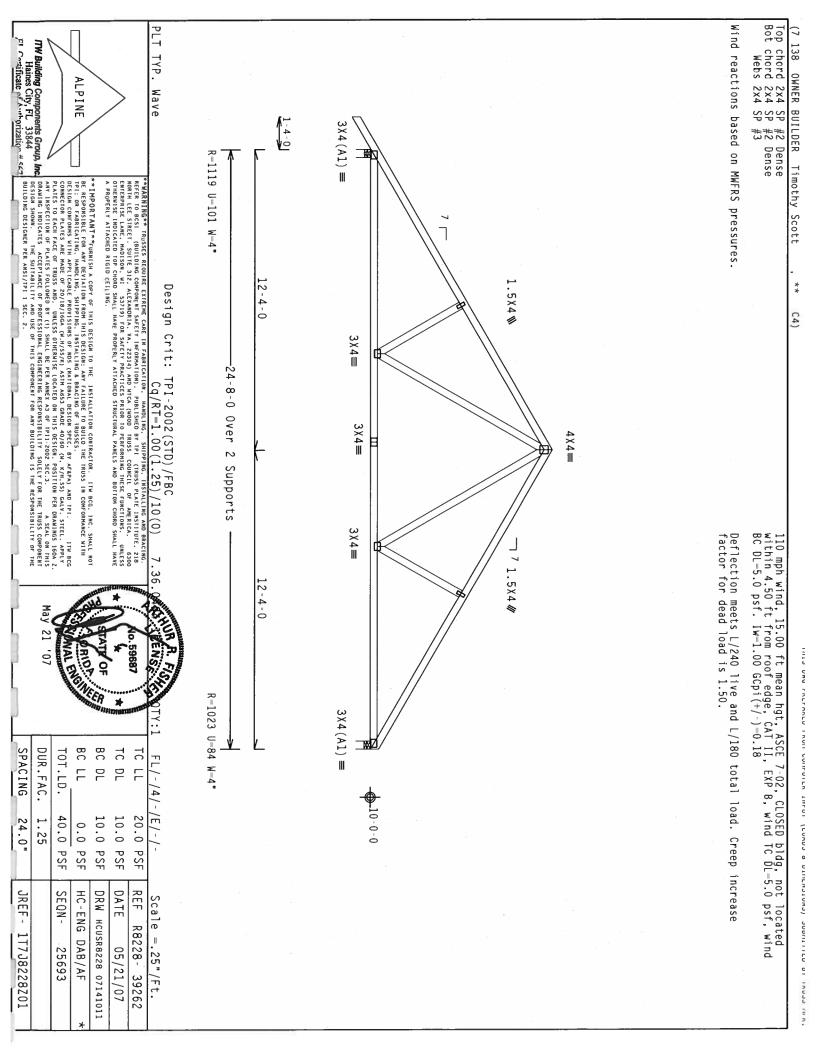
Scale = .25"/Ft. R8228- 39260

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Deflection meets  $L/240\,$  live and  $L/180\,$  total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures (7 138 OWNER BUILDER Timothy Scott TYP. ALPINE Wave 1-4-01 3X4(A1) =R=1119 U=103 W=4" \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. THE TRUSS IN COMPORMANCE WITH TPI: OR FABRICATING, HANDLING, SHPPING, INSTALLING & BRACING OF TRUSSES, DESIGN. CONFORMS MITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. B. 47.82A) AND TPI. ITM BCG CONNECTOR PLATES ARE HADE OF 20/18/16/36GA (H.H/SS/K). ASIM A653 GRADE 40/60 (M. K/M.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNICES OTHERNISE LOCATED ON THIS DESIGN. POSITION PER DRAWING 180A.Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF TPI1: 200E YEG. 3. ASIAL ON THIS DESIGN SHOWN. THE SUITABILLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILLITY OF THE REFER TO BOSS. (BUILDING COMPONENT SAFETY INFORMATION). HANDLING. SHIPPING, INSTALLING AND BRACING.

NORTH LEE STREET, SUITE 137. ALEXANDRIA, VA. 22:214) AND MICH CA (MODO TRUSS COUNCIL OF AMERICA. 6300

ENTERPRISE LANE, MODISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DUECES

OTHERRISE LONGLATED NOP CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SMALL HAVE
A PROPERLY ATTACHED RIGID CEILING. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 11-0-0 1.5X4 III 3×4/ Design Crit: C3) -24-8-0 Over 2 Supports TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 4×4≡ 4 X 8≡ 3 X 4≡ 2-8-0 4X5≡ In lieu of structural panels use purlins to brace all flat TC @ 0C. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18 7.36 1.5X4 III 3×4≡ 11-0-0 CENSE יוודק משה בטרבשטרה וצמנו המובמורט זעבמו (רמטסק פ מזנורעקומוס) קממנדוורס מו וצמקק אוצי R=1023 U=86 W=4" 3 X 4 (A1) = BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. SPACING FL/-/4/-/E/-/-10-0-0 40.0 24.0" 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF bldg, not located TC DL=5.0 psf, wind PSF JREF-DATE REF SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141010 Scale =.25"/Ft. R8228- 39261 1T7J8228Z01 22501 05/21/07



Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Wind reactions based on MWFRS pressures. (7-138--OWNER BUILDER Timothy Scott ITW Building Components Group, Inc.
Haines City, FL 33844
Et Carificate of Amborization # 547 ALPINE Wave K1-4-0 V 2.5X6(B1) =R-1210 U-117 W-4\* \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARRICATING. HANDLING. SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN COMPORES, WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. BY ATERA) AND IPI. IT BCG COMMECTOR PLATES ARE HADE OF 20/18/1666 (M.H/SS/K) ASIM A653 GRADE 40/60 (M.K/H.SS) GALV. STEELS. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OHERSISE LOCATED ON THIS DESIGN. POSITION PER DEAAHGES 160.A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FER AHMEX AS OF TPI11-2002 SEC.3. A SCAL ON THIS DESIGN SEC. SHALL SHALL BE FOR REPORTSIBILITY OR THE TRUSS COMPONENT OF THE DESIGN SHALL BE FOR AMERICA SO THE TRUSS COMPONENT OF THE SULTABLICITY OF THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEER DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPOUNDED DESIGNEER PER ANSI/TPI 1 SEC. 2. \*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS! (BULLDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB MORIH LEE STREE, SUITE 312. ALEENANDEIA, VA, ZEZIA) AND MICA (MORDO TRUSS COUNCIL OF AMERICA, 6300 EXTREMPLISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PERFORMING THESE FUNCTIONS. UNLESS A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL MAYE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL MAYE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL MAYE Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -4-0 01) -14-8-0 Over 2 Supports 3 X 4 Ⅲ 5×6≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. #1 hip supports 7–0–0 jacks with no webs -4-0 .36 R=1110 U=92 W-4"  $3X4(\lambda1) \equiv$ BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/E/-/-המווו מודני דווו מו לרמשמה מי מדוורנומדמנוס! המתוודוו רת חו זעחחת ונונוי \_10-0-0 24.0" 1.25 40.0 PSF 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF DATE REF JREF-SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141034 Scale = .375"/Ft. R8228- 39263 1T7J8228Z01 25413 05/21/07

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Wind reactions based on MWFRS pressures (7-138--OWNER BUILDER Timothy Scott ITW Building Components Group, Inc. Haines City, FL 33844 F1 Certificate of Amborization # 567 ALPINE Wave K1-4-0 2X4(A1) =R-705 U=67 W=4" \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. AT FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH IPI: OR FABRICATING, HANDLING, SHIPPING, HISTALLING & BRACHES OF TRUSSES, DOSIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFEA), AND IPI. ITW BCG CONNECTOR PLAITES ARE HADE OF 20/18/166A (M.H./SS/K), ASTH A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL, APPLICABLE OF TRUSS AND, UNILESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWHENS 150A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FER ARMEX AS OF FPIL 2002 SEC. 3. A SEAL ON THIS DESIGN MIGHT BUILDING IS THE RESPONSIBILITY OF THE DESIGN MIGHT BUILDING IS THE RESPONSIBILITY OF THE \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, "NAMOLING, SHIPPING, INSTALLING AND BRACING, REFER TO BCS1 (GUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREE, SUITE 312, ALEXANDRA, VA, 22314) AND MICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPENSE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OFFICENISE HOLDS, ALD TO MEDISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OFFICENISE HOLDS, ALD TO MEDISON SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. Design Crit: -4-0 02) -14-8-0 Over 2 Supports TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /10(0) 1.5X4 Ⅲ 4 X 5 = 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. -4-0 R=605 U=48 W=4"  $2X4(\lambda1) =$ BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/E/-/-10-0-0 1.25 40.0 20.0 PSF 24.0" 10.0 PSF 10.0 PSF 0.0 PSF bldg, not located TC DL=5.0 psf, wind PSF JREF -DATE REF SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141012 Scale = .375"/Ft. R8228- 39264 1T7J8228Z01 22523 05/21/07

Wind Top chord 2x4 SP #2 Dense Bot chord 2x8 SP #1 Dense Webs 2x4 SP #3 :W3 2x4 SP SPECIAL LOADS From From From 13992 reactions based on MWFRS pressures. UMBER DUR.FAC.=1.25 / PLATE
n 63 PLF at -1.33 to
63 PLF at 7.33 to
63 PLF at 7.33 to
7 5 PLF at -0.00 to
7 20 PLF at -0.00 to
92 LB Conc. Load at 7.12
63 LB-Conc. Load at 9.06, 1 PLATE #2 Dense: E DUR.FAC.=1.25)
63 PLF at 7.33
63 PLF at 14.67
5 PLF at -0.00
20 PLF at 14.67 11.06, 13.06

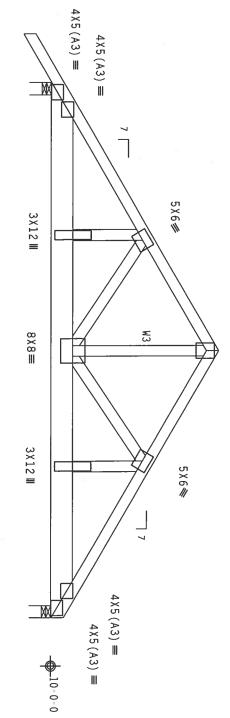
> COMPLETE TRUSSES REQUIRED

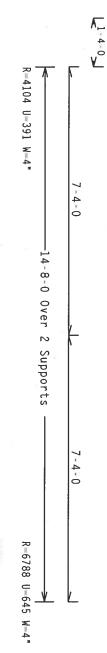
Nailing Schedule: (10d\_Box\_or\_Gun\_(0.128"x3",\_min.)\_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, within 6.50 ft from roof edge, CAT II, EXP BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 B, wind bldg, not located TC DL-5.0 psf, wind

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ 

# 5 X 6 (R) Ⅲ





Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0)

PLI

TYP.

Wave

\*\*MARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLINE, SHIPPING, INSTALLING AND BRACING. REFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PROLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 MORTH LEE SIREEI, SUITE 312, ALEXANDRIA, VA. ZZ314) AND MTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDCARED OF PROROS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

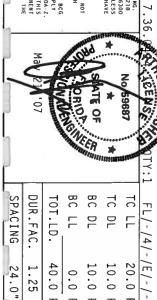
\*\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FARBLICATION. HANGLUNG, SHAPPING, INSTALLING & BRACING OF TRUSSES.

BESIGN CONFORMS HITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. B. ATERA) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF ZO/JB/JGGA, (M.H.55/K), ASTH A653 GRADE 40/60 (M. K/H.55) GALLY. STEEL. APPLY LALES TO EACH FACE OF TRUSS AND. UNLESS OTHERSISE LOCATED BY HIS DESIGN, POSITION PER DRAWNESS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF TPI1-ZOOZ SEC. 3. A SEAL ON THIS DRAWNING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEER HIG RESPONSIBILITY SOLLEY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER

ITW Building Components Group, Inc. Haines City, FL 33844 Fr Carificate of Archanization 4 4471

ALPINE



SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 1T7J8228Z01		SEQN- 25458	HC-ENG DAB/AF	DRW HCUSR8228 07141045	DATE 05/21/07	REF R8228- 39265

Scale

=.375"/Ft

Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP :Stack Chord SC1 :Stack Chord SC2 P #2 Dense P #2 Dense P #3 1 2x4 SP #2 Dense: 2 2x4 SP #2 Dense:

notched. Truss spaced at 24.0" OC designed to support 1–0–0 top chord outlookers. Cladding load shall not exceed 4.00 PSF. Top chord must not be cut or

In lieu of structural panels use purlins to brace TC @ 24" OC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

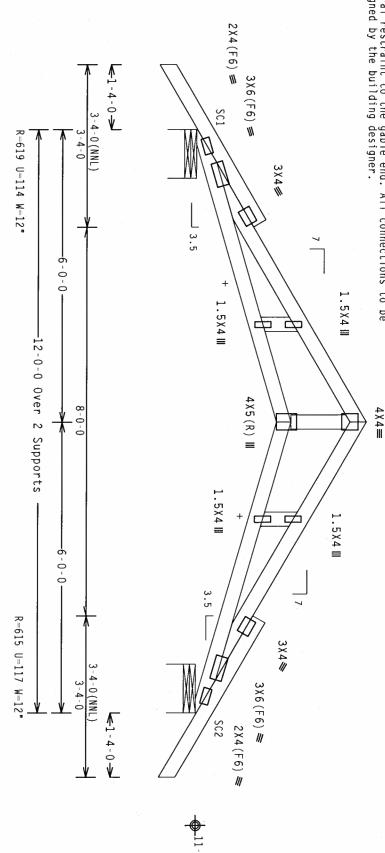
> 110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.18 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Wind reactions based on MWFRS pressures.

See DWGS Al1015EE0207 & GBLLETIN0207 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area

+ Member to be laterally braced for horizontal wind loads. Bracing system to be designed and furnished by others.





Design Crit:

TPI-2002 (STD) /FBC

PLT TYP.

Wave

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, YE ALLURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI; OR FABRICATING, MANDLUNG, SHEPPING, INSTALLING & BRACKING OF TRUSSES, DESIGN COMPORES WITH APPLICABLE PROVISIONS OF MOS (MATIONAL DESIGN SECC. 8, YAREA), AND TPI. 1TM BCG COMMECTOR PLATES ARE MADE OF 20/18/160A, (M.H/SS/K), ASTM A653 GRADE 40/60 (M. K/M.SS), GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERRISE LOCATED ON THIS DESIGN, POSITION PRO BRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC. 3. STAL ON THIS BRAING INDICATES ACCEPTANCE OF ADDRESSIONAL TRUSHERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FOR ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC. 3. STAL ON THIS BRAING INDICATES ACCEPTANCE OF ADDRESSIONAL TRUSHERING RESPONSIBILITY FOR THE TRUSS COMPONENT FOR ANY INSPECTION OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ITW Building Components Group. Inc. Haines City, FL 33844 FI Conficate of Ambarization 41 5471

ALPINE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2

7.36. CENS 0.59687 30 FL/-/4/-/E/-/-

SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	וכ רר
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 1T7J8228Z01	0.000	SEQN- 25409	HC-ENG DAB/AF	DRW HCUSR8228 07141046	DATE 05/21/07	REF R8228- 39267

Scale =

.5"/Ft

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 D
:Stack Chord SC2 2x4 SP #2 D Dense: Dense

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 4.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace TC @ 24"

Deflection meets L/240 live and L/180 total load. Creep increase for dead load is 1.50. factor

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

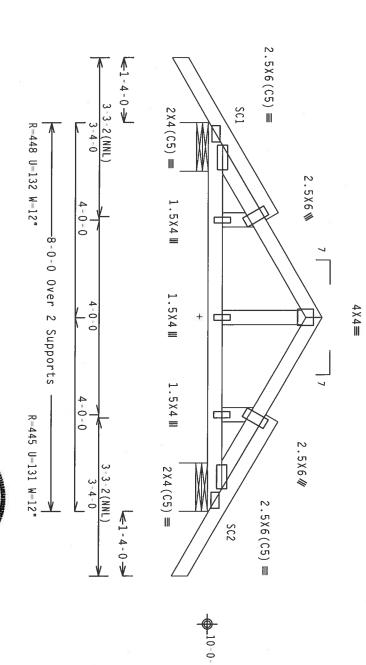
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6

+ Member to be Bracing system laterally braced for horizontal wind loads to be designed and furnished by others.



\*\*WARNING\*\* TRUSSES REDUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (RULLING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (RUSS PLATE INSTITUTE, 219 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE Cq/RT=1.00(1.25)/10(0) 7.36.

Design Crit:

TPI-2002 (STD) /FBC

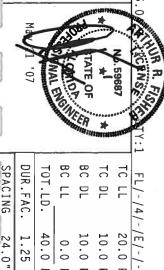
TYP.

Wave

Haines City, FL 33844

FI Consificate of Authorization 4 567 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH PP: OR FARRICATING, ANDULIG. SHPPING, INSTALLING & BRACLING OF TRUSSES. BCG. BY AFAPA) AND TPI. CRIEF PROVISIONS OF HIS SECTIONAL DESIGNS SPECE. BY AFAPA) AND TPI. IT IN BCC. CONNECTION PLATES ARE ALSO OF 20/19/16CA (M.H.5X)K) ASTH AFAS GRADE 40/60 (M. X.H.SS) GALV. SIEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS OFSIGN, POSITION PER DRAWHINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PPI1-2002 SEC. 3. A SEAL ON THIS DRAWHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ALPINE



SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 1T7J8228Z01		SEQN- 25401	HC-ENG DAB/AF	DRW HCUSR8228 07141047	DATE 05/21/07	REF R8228- 39268

.5"/Ft

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be Bot Truss spaced at 24.0" OC designed to support 1–0–0 top chord outlookers. Cladding load shall not exceed 4.00 PSF. Top chord must not be cut or PLT TYP. designed by the building designer Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to brace TC @ 24" notched :Stack Chord SC1 2x4 SP #2 :Stack Chord SC2 2x4 SP #2 (7-138--OWNER BUILDER ITW Building Components Group, Inc.
Haines City, FL 33844

Et Confidente of Authorization # 447 chord 2x4 Webs 2x4 SP ALPINE S S #2 Dense #2 Dense #3 Timothy Scott \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FABRICATING, MANDLING, SHIPPING, INSTALLING & BRACHING OF TRUSSES, A 7848A) AND TPI. ITM BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY A784A) AND TPI. URGC CONNECTOR PLATES ARE MADE OF 20/18/166A (N.H/SS/K) ASTM A653 GRADE 40/60 (N. K/H.SS) GALV STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNICESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX A3 OF TPII-2002 SEC. 3. SEAL ON THIS DRAWINGS 100A-Z. BRAWINGS 100A-Z \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BOST (BUILDING COMPONENT SAFETY IMPORANTON), PUBLISHED BY TPI (TRUSS PLATE HESTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, NA, 22314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDSLAFE OF ORDER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE BUILDING DESIGNER PER ANSI/TPI 1 SEC. Dense: **★**1-4-0-**>**  $2.5 \times 6 (C5) =$ SC1 2X4(C5) = 4-0 (NNL 3-4-0 Design Crit: GGE) U=128 W=12\* 3×4/ 3-6-0 -7-0-0 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ 1.5X4 III Over 3-0-0 4 X 4 == ~ + Ф Supports /10(0)3 - 6 - 0R-400 U-129 W-12\* 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 + Member to be Bracing system Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6. See DWGS A11015EE0207 & GBLLETIN0207 for more requirements Wind reactions based on MWFRS pressures 3×4// . ა 2X4(C5) =4-0 (NNL 3-4-0 SC2  $2.5 \times 6 (C5) =$ **★**1-4-0→ JE FNS laterally braced for horizontal wind loads to be designed and furnished by others. (59687 10-0-0 BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/E/-/-1.25 40.0 20.0 10.0 PSF 10.0 PSF 24.0" 0.0 PSF PSF PSF JREF-SEQN-DATE REF HC-ENG DAB/AF DRW HCUSR8228 07141048 Scale =.5"/Ft. R8228- 39269 1T7J8228Z01 25405 05/21/07

(7-138--OWNER BUILDER Timothy Scott --

ווועם בשק בארוטטרה ואמו ממונים ובעים (במשמים מינונים מונים מחנים במחנים ווכר מו ושמפים ווואים

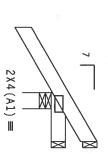
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

R--42 U-34





R--10 U-10



**★**1-4-0-**★** 1-0-0 Over 3 Supports

Design Crit:

R-225 U-35 W-4"

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

7.36.

PLT TYP.

Wave

\*\*WARNING\*\* TRUSSES REDUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST: (BUILDING COMPONENT SAFEIT IMPORATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 21B MORTH LEE STREE, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFEITY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR FORDO SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE

\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. I'M BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN GOMPORMANCE WITH PI: OR FABRICATING, HANDLIGS, SUPPING, INSTALLING & BRACLING OF TRUSSES, DESIGN CONFIDENCY HAS PLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY ATRA) AND TPI. DESIGN CONFIDENCY HAS ARE HAD OF 20/18/18/16GA (M. H/SS/K) ASTH AGE 40/60 (M. K/H. SS) GALV. SIEEL. APPLY DLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER BOAMINGS 160A-Z. ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANKEX AS OF FPI1-2002 SEC. 3. A SEAL ON THIS DESIGN ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT THE SUITABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ALPINE

ITW Building Components Group, Inc. Haines City, FL 33844 FT Cortificate of Amborization # 567.

		May 21 '07	4		STATE OF SERVICE	*	Ng/59687	TY:1	SUR R. F.
]	SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL	FL/-/4/-/E/-/-	
	24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF	/E/-/-	
	JREF- 1T7J8228Z01		SEQN- 22448	HC-ENG DAB/AF	DRW HCUSR8228 0714103	DATE 05/21/07	REF R8228- 39270	Scale =.5"/Ft.	

228 07141035

Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3

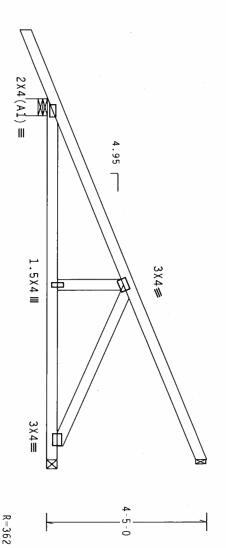
Wind reactions based on MWFRS pressures

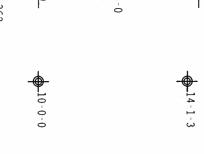
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi (+/-)=0.187-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Hipjack supports 7-0-0 setback jacks with no webs.

R=269 U=71







\*\*MARNIG\*\* TRUSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PRICE ISSUED BY TPI (TRUSS PLATE INSTITUTE, ZIB NORTH LEE SIRRET, SUITE ZIZ, ALEXANDRIA, VA, ZZZJA) AND VIZA (MODD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LAME, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED TO POWNOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00 (1.25) /10 (0)

PLT TYP.

Wave

ITW Building Components Group, Inc.
Haines City, FL 33844
Ft Conficate of Authorization 44 567

ALPINE

7.36. THE R. CENS SPACING

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	SPACING	DUR.FAC.	TOT.LD.	86 רר	BC DL	TC, DL	TC LL
	24 0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
97.000000	.1RFF- 1T7.18228701		SEQN- 25393	HC-ENG DAB/AF	DRW HCUSR8228 07141036	DATE 05/21/07	REF R8228- 39271

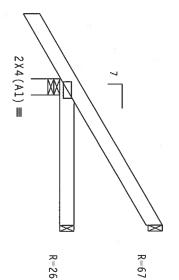
Scale =.375"/Ft.

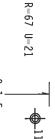
Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense

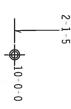
Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE anywhere in roof, CAT II, EXP B, wind psf. Iw=1.00 GCpi(+/-)=0.18 7-02, CLOSED bldg, Located TC DL=5.0 psf, wind BC DL=5.0

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.







**1-4-0**→

3-0-0 -0-0 Over 3 Supports

R-247 U-18 W-4"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0)

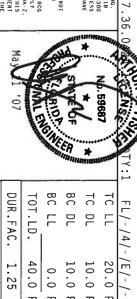
PLT TYP. Wave

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, MAND ING. SHIPPING, INSTALLING AND BRACING.
REFER TO BOSSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2198
MORTH LEE SIRREI, SUITE 312. ALEXANDRIA, VA. 22314) AND WICA (MODD TRUSS COUNCIL OR AMERICA, 6300
ENTERPRISE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. UNLESS
OTHERWISE INDICATED FOR FORDS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

ITW Building Components Group, Inc.
Haines City, FL 33844
Ft Constitution # 547 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSSE IN COMPORMANCE WITH TP1: OR FARBICATING. HANDLUNG. SHIPPING, HISTALLING & BRACING OF TRUSSES. A CASA) AND TP1. IT NEG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SECC. BY ACASA) AND TP1. ITN RCG CONNECTOR PLATES ARE MADE OF ZO/ID/166A (M.H/SS/K) ASIM A653 GRADE 40/60 (M. Y/M.SS) GALV. SIELL APPLY LAIES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A 30 F (F1): 2002 SEC. 3. A SAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DRAWING INDICATES ACCEPTANCE OF THE TRUSS COMPONENT OF THE SUMMAN SHOWN. THE SULLABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

ALPINE

BUILDING DESIGNER PER ANSI/TPI 1 SEC.



			Transit.	ER Vann	* !!!!!!!!	antities.
SPACING	DUR.FAC.	TOT.LD.	BC LL	BC DL	TC DL	TC LL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF	20.0 PSF
JREF- 1T7J8228Z01		SEQN- 22453	HC-ENG DAB/AF	DRW HCUSR8228 0714101	DATE 05/21/07	REF R8228- 39272

014

Scale =.5"/Ft.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Wind reactions based on MWFRS pressures. PLT TYP. (7-138-OWNER BUILDER Timothy Scott TW Building Components Group, Inc. Haines City, FL 33844 FI Certificate of Amborization # 567 ALPINE Wave \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG. INC. SHALL NOT BE RESPONSIBLE FOR MAY DEPAITION FROM HIS DESIGN. ANY FALLURE TO BUILD THE TRUSS IN COMPORNANCE WITH PI; OR FABRICATING, HADDLIGS. SUPPPINC, INSTALLING & BRACHING OF TRUSSES. DESIGN COMPORES WITH APPLICABLE PROVISIONS OF HIS SKATIONAL DESIGNES SPEC, BY AFAPA) AND TP].

BUSION COMPORES WITH APPLICABLE PROVISIONS OF HIS SKATIONAL DESIGNES SPEC, BY AFAPA) AND TP].

FLATES TO EACH FACE OF TRUSS, AND. JUNESS OTHERNISE LOCATED ON THIS DESIGN. POSITION PER BRAHINGS 160A. Z. ANY HISPECTION OF PLATES POLICHORED BY (1) SHALL BE PER AMBER AS OF FP11-2002 SEC. 3. AS ALO IN THIS DESIGN OF PLATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITIABLITY MOUSE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST (BUILDING COMPONENT SAFETY IMPORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB
MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. Z2314) AND NICA (MODO TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, HADISON, NI 33719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INJOICATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING. **★**1-4-0-▶ 2X4(A1) = R-320 U-15 W-4" MX Design Crit: CJ5) -5-0-0 Over 3 Supports TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 5-0-0 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ R=55 R-131 U-40 7.36. w Ś 12-11-13 BC DL BC LL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/E/-/-20.0 PSF 24.0" 1.25 10.0 PSF 40.0 PSF 10.0 PSF 0.0 PSF DATE REF JREF -SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141015 Scale = .5"/Ft. R8228- 39273 177J8228Z01 05/21/07 22457

מורט זעומי לרמטמת מ מזורוותומות! תממוודוורת מו ושחתת ווו ווי

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense PLT TYP. Wave Wind reactions based on MWFRS pressures. (7-138--OWNER BUILDER Timothy Scott Haines City, FL 33844

Fit Contificate of Authorization 4 647 ALPINE \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI: OR FAREICATING, HANDLING, SHIPPING, INSTALLING & BRACHING OF TRUSSES; OR FAREA, NAM PPI. CABLE PROVISIONS OF NIDS (MATIONAL DESIGN SPEC, BY AFREA) AND TPI. DESIGN COMPORTS WITH APPLICABLE PROVISIONS OF NIDS (MATIONAL DESIGN SPEC, BY AFREA) AND TPI. ITH BCC COMPORTS OF TRUSS AND. DIRECTOR PARTES ARE ARROWDED OF 20/18/16GA (M.H.SYS), ASTH AGE 40.04 EVALUATION OF PARTES OF TRUSS AND. DIRECTOR PARTES OF TRUSS OF TRU \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST! (BUILCING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREET, SUITE 312, ALEXANDRAI, VA, 22314) AND HICA (HOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGION CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE K1-4-0 V  $2X4(A1) \equiv$ M R=398 U=13 W=4" Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) -7-0-0 Over 3 Supports EJ7) 7-0-0 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. R-83 R=191 U=57 7.36 May 21 '07 4-5-5 COLUNS **⊕**14-1-13 10-0-0 BC LL BC DL DUR.FAC. TC DL TC LL SPACING TOT.LD. FL/-/4/-/E/-/-24.0" 1.25 40.0 PSF 10.0 PSF 20.0 PSF 10.0 PSF 0.0 PSF DATE REF JREF -SEQN-HC-ENG DAB/AF DRW HCUSR8228 07141016 Scale = .375"/Ft. R8228- 39274 1T7J8228Z01 05/21/07 25452

tire care turnitures tures consisted tures (course a stire action of constitute of the same in the

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C, UNLESS OTHERWISE SPECIFIED. Wind reactions based on MWFRS pressures. PLT TYP. Wave Note: All Plates Are 1.5X4 Except As Shown. (7-138--OWNER BUILDER Timothy Scott Haines City, FL 33844

Fit Conficate Controlization 4 647 ALPINE 55 U=83 W=6.45" 2X4(A1) =\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR AWY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FPI; OR FAREIGATING, HANDLING, SHEPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. BY AREA), AND TPI.

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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SEC. BY AREA), AND TPI.

APPLICABLE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ARREX AS OF FPI1-2002 SEC. 3.

BRAHNE INDICATES ACCEPTANCE OF PROFESSIONAL FRIGINGERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL DESIGN SHOWN. THE SUITABILITY AND USE OF TH BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. \*\*HARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONITY SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDIA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CORDOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE 7-1-10 -8-3-10 \* Design Crit: H17-PB4) R-78 PLF U-29 PLF W-16-7-4 18-3-0 Over 3 Supports TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/10(0) 4 X 4 = 2-4-0 4×4≡ 3 X 4 ≡ In lieu flat TC 110 mph wind, 22.62 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50.\,$ of structural panels or rigid ceiling use @ 24° OC, all BC @ 24° OC. 7-1-10 #ENS o. 59687 ARIOP. TATE OF '07 × 2X4(A1) =55 U=31 W=6.45" BC DL BC LL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/E/-/-40.0 10.0 PSF 10.0 PSF 20.0 PSF 1.25 0.0 purlins TC DL=5.0 PSF PSF to brace all JREF-DATE REF SEQN-DRW HCUSR8228 07141037 HC-ENG DAB/AF Scale =.375"/Ft. located psf, wind R8228- 39275 117J8228Z01 25488 05/21/07

SPACING

24.0"

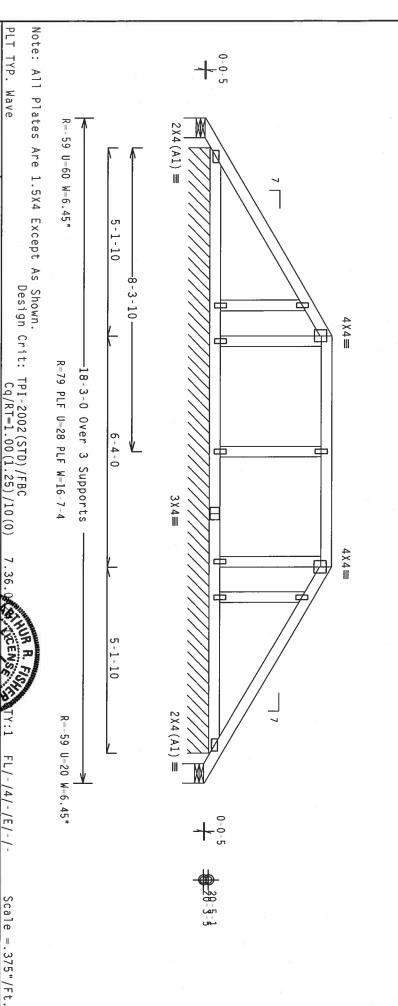
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED
@ 24" O.C, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 22.96 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Haines City, FL 33844

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ALPINE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN COMPORANCE WITH FPI: OR FARELACTING, HANDLIGS, SHPPING, INSTALLING & BRACING OF TRUSSES. DAY AFAPA, AND TPI DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA, AND TPI COCCOMECTOR PAICES ARE AGREED OF 20/19/16/EGA, (M.H.55X) ASTENDANDE ADJOES (M. K./H.5S) GALV. STEEL APPLY DATES TO EACH FACE OF TRUSS AND. JUNESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAHINGS 160A-Z. ANY HISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANKEX AS OF FPI1-2002 SEC. 3. ASEA, ON THIS DESIGN ADJOES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

May

21 '07

DUR.FAC SPACING

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DRW HCUSR8228 07141039

DAB/AF 25622 REF DATE

R8228- 39277

05/21/07

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\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IM FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONEM) SAFETY IMPORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREEI, SUITE 312, ALEXANDRIA, VA, Z2314) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDINGTON TO THOSE FUNCTIONS. THE SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLDINGTON TO THOSE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

Top chord 2x4 Bot chord 2x4 Webs 2x4 4 SP #2 Dense 4 SP #2 Dense 4 SP #3

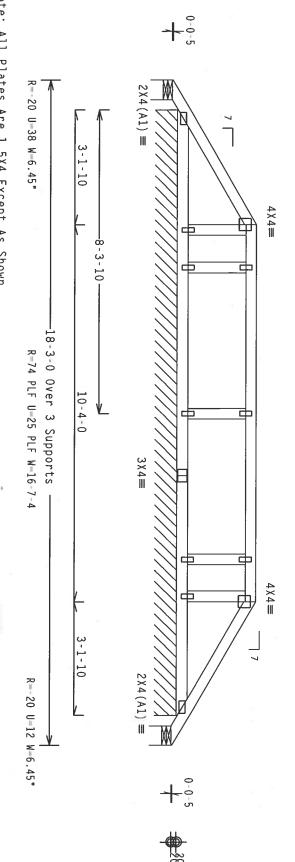
Wind reactions based on MWFRS pressures

REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C, UNLESS OTHERWISE SPECIFIED.

110 mph wind, 21.45 ft mean hgt, ASCE 7-02, CLOSED within 4.50 ft from roof edge, CAT II, EXP B, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 bldg, not located TC DL=5.0 psf, wind

In lieu of structural panels or rigid flat TC @ 24" OC, all BC @ 24" OC. ceiling use purlins to brace all

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

PLT TYP.

Wave

\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUS PLATE INSTITUTE, 219
MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND MICA (MODO TRUSS COUNCILS TO AMERICA. 6300
ENTERPRISE LANE, HADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP COMODO SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGIO CELLING. Cq/RT=1.00(1.25)/10(0)

TPI-2002 (STD) /FBC

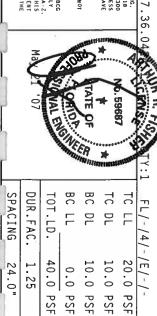
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DRAHING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/PP I 1 SEC. 2.

Haines City, FL 33844

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ALPINE



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SEQN-

25627

HC-ENG DAB/AF DRW HCUSR8228 07141040 DATE REF

05/21/07

Scale = .375"/Ft. R8228- 39278

JREF-

1T7J8228Z01

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 PLT TYP. Note: All Plates Are 1.5X4 Except As Shown. REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C, UNLESS OTHERWISE SPECIFIED. (7-138--OWNER BUILDER Haines City, FL 33844

"Tificate "Torizati" " " Tificate " Tificat ALPINE Wave 22 U-60 W-6 -1-10 2X4(A1) =Timothy Scott \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN ARE ACKNOWN FOR THUSSES.

BERESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN ABACHURG THUSSES.

DOSIGN CORFORTATION, HANDLUGH, SHPPPHG, INSTALLING A BRACHURG OF TRUSSES.

DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI.

DESIGN COMPORNS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI.

PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHINGS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPI1-2002 SEC, 3.

A SEAL ON THIS

DRAWHING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONER IN RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN.

THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. \*\*WARNING\*\* RUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BESS (QUILLING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORH LEE STREET, SUITE 312, ALEXANDRIA, VA, Z2314) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, NI S3719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP GROOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE . 45" 8-3-10 Design Crit: 中中 H17-PB1) 18-3-0 Over 3 Supports R-79 PLF U-28 PLF W-16-7-4 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 14-4-0 뮵 3 X 4≡ In lieu flat TC 110 mph wind, 22.96 ft mean hgt, ASCE 7 within 4.50 ft from roof edge, CAT II, BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ of structural panels or rigid @ 24" OC, all BC @ 24" OC. 中中 May 21 '07 ZENS 4 X 4 ==  $2X4(A1) \equiv$ R-22 U-50 W-6.45 BC LL BC DL TC DL DUR.FAC. SPACING TC LL TOT.LD. FL/-/4/-/E/-/ceiling use purlins to brace all 7-02, CLOSED, EXP B, wind 1.25 40.0 24.0" 10.0 PSF 10.0 PSF 20.0 PSF 0.0 bldg, not located TC DL=5.0 psf, wind PSF PSF JREF -DATE REF SEQN-HC-ENG DRW HCUSR8228 07141041 Scale = .375"/Ft. R8228- 39279 1T7J8228Z01 DAB/AF 05/21/07 25632

ווונים עחש ויצרוחארם וצמון כמודיטורע ועדכו (רמשטים מינורתייבושים) שמפונוונט פו ומשטים וודד.

SPACING

24.0"

JREF -

1T7J8228Z01

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED. PLT TYP. Note: All Plates Are 1.5X4 Except As Shown. (7 138 OWNER BUILDER Timothy Scott ITW Building Components Group, Inc.
Haines City, FL 33844

FT Comifficate of Authorization # 567 ALPINE Wave R=-19 2X4(A1) =0 = 40\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITN BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILION FROM HIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI: OR FAREICATING, HANDLIGG. SUPPING, INSTALLING & BRACHING OF TRUSSES.

DESIGN COMPORMS WITH APPLICABLE PROVISIONS OF HIS (MATIONAL DESIGN SPEC, BY ATAPA) AND TPI. ON THE PROVISIONS OF HIS SEX, STATE AND THE PROVISIONS OF HIS GANTONAL DESIGN SPEC, BY ATAPA) AND TPI. CONNECTION FARES ARE AND COPY.

PLATES TO EACH FACE OF TRUSS AND. DIMESS OTHERWISE LOCATED ON HIS DESIGN, POSITION FER BRAHINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY CI) SHALL BE FER ANNEX A OF FPII-2002 SEC. 3. A SEAL ON THIS DESIGN AND THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING SECRETARCE OF PROFESSIONAL ENGLINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THE SUITABLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE \*\*WARNING\*\* TRUSSES REDUIRE EXTRÊME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO SEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB MORTH LEE STREEE, SUITE 312, ALEXANDRÍA, VA, Z2314) AND MICA (MODRO TRUSS COUNCIL O'AMERICA, 6300 CHTERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE HOLOCATED TOP CORDOS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE 3 - 1 - 10W=6.45" 4×4≡ Design Crit: H17 PB7) 17-5-2 Over 2 Supports TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/ R=79 PLF U=28 PLF W=16-7-4 10-4-0 3 X 4≡ /10(0) 110 mph wind, 22.96 ft mean hgt, ASCE 7-02, CLOSED bldg, not within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 BC DL=2.0 psf. Iw=1.00 GCpi (+/-)=0.18Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels or rigid ceiling use flat TC @ 24" OC, all BC @ 24" OC. 7.36.04 KINUR R.  $4 \times 4 =$ SCEPS MONOR. TATE OF 5. 59687 WEINER, 3 - 1 - 10\*  $2X4(A1) \equiv$ BC DL TC DL SPACING ВС TC LL DUR FAC. TOT.LD. FL/-/4/-40.0 /E/-/-24.0" 1.25 20.0 10.0 PSF 10.0 PSF 0.0 purlins PSF PSF PSF to brace all JREF -SEQN-DATE REF HC-ENG DAB/AF DRW HCUSR8228 07141043 Scale =.375"/Ft. located psf, wind R8228- 39281 1T7J8228Z01 25646 05/21/07

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 REFER TO PIGBACKA0405 OR PIGBACKB0405 FOR PIGYBACK DETAILS. TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED. Wind reactions based on MWFRS pressures. Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. (7-138--OWNER BUILDER Timothy Scott Haines City, FL 33844

"" (Figure 1) Transcription | 10 cm | 1 ALPINE Wave  $2X4(A1) \equiv$ 57 \*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BER RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMPROMANCE WITH FDI; OR FABRICATING, HANDLUNG, SHAPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN COMPROMES WITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SEC. BY AFRA) AND PPI.

DESIGN COMPROMES ARE HADE OF ZO/IB/16GA (M.H/SS/M) ASIM A653 GRADE 40/60 (M. K/M.SS) GALV. SIEGL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNICSS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.

ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2003 ESC. 3.

ASEALOW THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY OR THE TRUSS COMPONENT THE SUITABILITY OF THE \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFITY IMPORATION), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 2718 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE HOLDSCARED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE DRAWING INDICATES ACCEPTANCE OF PROFESSI DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. 89=D W-6.45" 5-1-10 Design Crit: H17-PB6 4 X 4 ≡ 17-5-2 TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /10(0) R-73 PLF U-26 PLF W-16-7-4 Over 2 Supports 6-4-0 3 X 4≡ 110 mph wind, 22.04 ft mean hgt, ASCE 7–02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ In lieu of structural panels or rigid flat TC @ 24" OC, all BC @ 24" OC. 4 X 4≡ 7.36 EENS 5-1-10 . 59687 2X4(A1) =BC LL BC DL SPACING DUR.FAC. TC DL C TOT.LD. FL/-/4/-/E/-/ceiling use 40.0 20.0 10.0 24.0" 1.25 10.0 PSF 0.0 purlins to PSF PSF PSF PSF DATE REF SEQN-DRW HCUSR8228 07141044 JREF -HC-ENG DAB/AF Scale =.375"/Ft. brace all R8228-117J8228Z01 25651 05/21/07 39282

# BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

### NOTES:

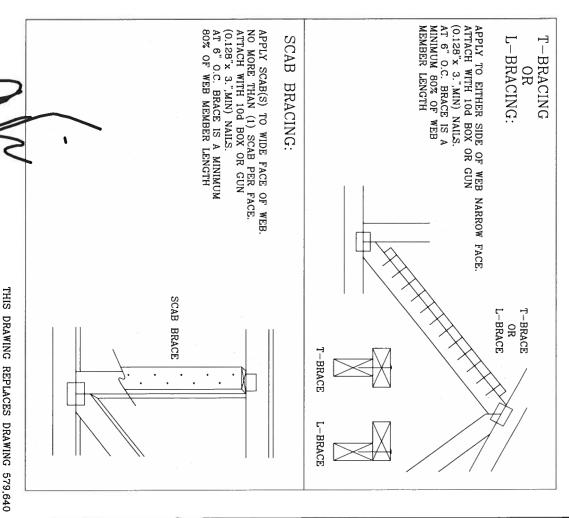
BRACING THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB

BRACING. ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE

2-2X6(*)	2X6	2 ROWS	2X8
1-2X8	2X6	1 ROW	2X8
2-2X4(*)	2X6	2 ROWS	2X6
1-2X6	2X4	1 ROW	2X6
2-2X4	2X6	2 ROWS	2X3 OR 2X4
1-2X4	2X4	1 ROW	
SCAB BRACE	T OR L-BRACE	BRACING	SIZE
E BRACING	ALTERNATIVE BRACING	SPECIFIED CLB	WEB MEMBER

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

\* CENTER SCAB ON WIDE FACE OF WEB. FACE OF WEB APPLY (1) SCAB TO EACH





ITWBUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

\*\*AVANNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, FANDLING, SHPPING, INSTALLING AND BRACING. REFER TO BEST GRULDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CTRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VITGA CYDDD TRUSS COUNCIL ARERICA, 6300 ENTERPRISE LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNICESS OTHERWISE NOTICE, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION CELLING.

GALV. STEEL. APPLY PL GALV. STEEL. APPLY PL DESIGN, POSITION PER DI ANNEX A3 OF TPI 1-2006 ENGINEERING RESPONSIBI USE OF THIS COMPONENT ANSI/TPI I SEC. 2. MATHODRIANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC., SHALL

OUT BE RESDONSBLE FUR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN
CONTORNACIE WITH THIS DEFENDANCE WITH THE TRUSS IN THE TRUSS OF THE TRUS

SIONAL ENGINEE

SPACING DUR. FAC. TOT. LD. BC DL

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DATE REF

2/23/07 CLB SUBST

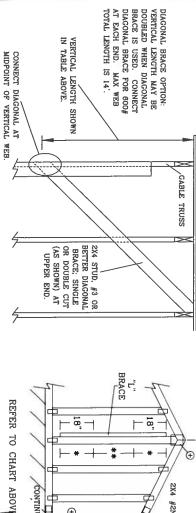
BRCLBSUB0207 MLH/KAR

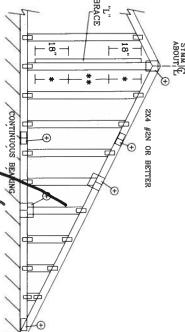
TC

H

## ASCE 7-02: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, $\overline{\phantom{a}}$ 11 1.00, EXPOSURE a

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	THU	1	<u>(/</u>	)	TTT	口 口	CLI	2	1	<u> </u>	1	<u>V.</u>	)	TIT	ΠŢ	O'T'	Ξ Ξ Σ			1		) j	TTT	L L	בו בו	Į. Σ	SPECIES	2X4
STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
4 11	5	1 3	5. 3.	1 "	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4' 6"	4, 6,	4' 9"	4 10	4' 4"	4' 4"	4' 4"	4' 5"	3' 10"	4' O"	4' 0"	4 2"	4' 3"	3′ 9″	3' 9"	3' 9"	3' 10"	BRACES	N O
7 5	η .	8, 5,	8, 5,	۵. ن	7' 3"	8' 5"	8 5	8, 5,	6' 5"	7' 6"	7' 7"	7' 8"	7' 8"	6' 4"	7' 4"	7' 4"	7' 8"	١.	6' 1"	6' 2"	6' 8"	6' 8"	"	6' 0"	6' 0"	6' 8"	GROUP A	(1) 1X4 "L"
7 5	١.	8' 5'	9' 1"	9' 1"	7' 3"	8, 5,	8' 5"	8' 8"	6° 5°	7' 6"	7' 7"	8 <sup>1</sup> 3 <sup>1</sup>	8' 3"	6' 4"	7' 4"	7' 4"	7' 10"	5 <sup>1</sup> , α <sup>1</sup>	6'1"	6' 2"	7' 2"	7' 2"	<sub>ວ</sub> ຸ ຄູ	6' 0"	6' 0"	6' 10"	GROUP B	" BRACE .
9 10	19	10′0″	10' 0"	10' 0"	9' 7"	10' 0"	10' 0"	10' 0"	8' 6"	9' 1"	9' 1"	9' 1"	9' 1"	8, 4,"	9'1"	9' 1"	9'1"	6'11"	7' 11"	7' 11"	7' 11"	7' 11"	6' 9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
9 10	٦٧	10' 6"	10' 9"	10′ 9″	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9' 9"	9' 9"	8' 4"	9' 1"	9' 1"	9' 4"	6' 11"	8' 0"	8' 1"	8' 6"	8' 6"	6' 9"	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *
11 11	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"		10' 10"		10' 10"	9' 4"	9' 5"	9' 5"	9' 5"	9′ 5″	9' 1"	9 5	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"
12 3	1 "	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"		10' 10"		11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10' 2"	9' 1"	9' 5"	9. 5.	9' 8"	GROUP B	" BRACE **
14 0	14. 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14' 0"	13' 3"	14' 0"		14' 0"	14' 0"	12' 11"	14' 0"	-	14' 0"	10' 10"	12' 5"	12' 5"	12' 5"	12' 5"	10' 7"	12' 3"	12' 4"	12' 5"	GROUP A	(1) 2X6 "L"
14 0	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	-	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 6"	12' 8"	13′ 5″	13′ 5″	10' 7"	12' 3"	12′ 4″	12' 9"	GROUP B	" BRACE *
14 0	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L" BRACE
14 0	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14'0"	14' 0"	14′0″	14′0″	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	GROUP B	BRACE **
GABLE END SUPPORTS LOAD FROM 4' 0"	CONTINUOUS BEARING (5 PSF TC DEAD LOAD).	PROVIDE LIBILIET CONNECTIONS FOR BO PLE OVER	LIVE LOAD DEFLECTION CRITERIA IS L/240.	GADEE INCOC DEIMIE NOIDS.	CARIE TRIESS DETAIL NOTES:			#2 #2		SOUTHERN PINE DOUGLAS FIR-LARCH	H	8 0 2	HEM-TIK	GROOF D.	CBOILD B:		ا <u>ا</u> ا	STANDARD STANDARD	#3 #3	-LARCH SOUTH		S	#1 / #2 STANDARD #2 STUD	, O O I	CBOILD V:	BRACING GROUP SPECIES AND GRADES:		





OUTLOOKERS WITH 2 0 OVERHANG, OR 12" PLYWOOD OVERHANG. OVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LE END SUPPORTS LOAD FROM 4' 0" LOAD DEFLECTION CRITERIA IS L/240.

- ATTACH EACH "L" BRACE WITH IOD NAILS.

  \* FOR (I) "L" BRACE: SPACE NAILS AT 2" O.C.

  \* NI 18" END ZONES AND 4" O.C. BETWEEN ZONES.

  \*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

  IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.
- GABLE VERTICAL PLATE SIZES BUT 1X4 OR 2X3 NO SPLICE 2.5X4 2X4
- VERTICAL LENGTH
  LESS THAN 4' 0"
  GREATER THAN 4' 0", E
  LESS THAN 11' 6" GREATER THAN 11' 6" REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. REF DRWG DATE A11015EE0207 2/23/07 ASCE7-02-GAB11015

AX CABLE

VERTICAL LENGTH.

\*\*AVANNICS\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDING, SUPPING, INSTALLING AND BRACING. REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTTUTE, 218 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA. 22314) AND "VICA CVUIDD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, HANDSON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS DIMENSIES INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

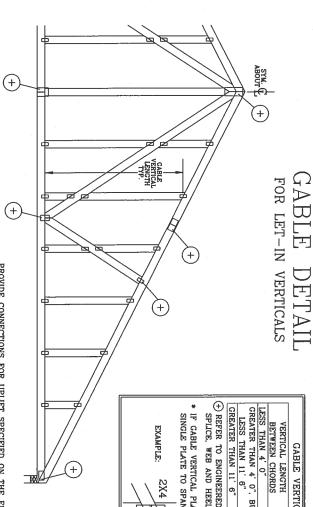
MATHORIFANIMA FURNISH CORY OR THIS DESIGN TO INSTALLATION COMPRACTOR. ITY BCG, INC., SHALL
NOT BE RESPONSIBLE FOR ANY DEVLATION FORM THIS DESIGN, ANY FAILURE OF BUILD THE TRUSS IN
COMPONENCE WITH A PEN ANY DEVLATION FROM THIS DESIGN, ANY FAILURE OF BUILD THE TRUSS IN
DESIGN CONNECTOR PLATES ARE HADE OF 2018 VISON OF DISCNATIONAL DESIGN SECOND OF VAIX-ASS
DESIGN VITEL APPOLABLE PROPUSIONS OF DISCNATIONAL DESIGN SECOND OF VAIX-ASS
DESIGN, POSITION PER DRAWINGS ISON-7. ANY INSPECTION OF PLATES OF ILOUED BY OF SHALL BE PER
DESIGN, POSITION PER DRAWINGS ISON-7. ANY INSPECTION OF PLATES OFFICIALS DEVELOY SHALL BE PER
ANNEX AS OF THE 1-2002 SECOND AS SEAL ON THIS DRAWING DISCOND SHAME OF THE SUITABILITY AND THE SUITABILITY AND THIS COMPONENT FOR ANY BUILDING DESIGNER, PER
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ITW BUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

ALPINE

LORIO TIE STATE OF No. 59687 CENS MAX. MAX. TOT. SPACING LD.

60 PSF 24 Ö



Ω.	Ω	<u> </u>	_	_	
GREATER THAN 11' 6"	GREATER THAN 4' 0". BUT LESS THAN 11' 6"	LESS THAN 4' 0"	BETWEEN CHORDS	VERTICAL LENGTH	CADES ABILITAD LINES OFFICE
2.5X4	2X4	1X4 OR 2X3	SIZE	PLATE	o rustin or
2.5X8	2X8	2X8	OVERLAP*	IF PLATES	250

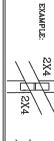
+ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK SPLICE, WEB AND HEEL PLATES.

\* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.









ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

HAND DRIVEN NAILS:

GUN DRIVEN NAILS: 10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS (4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD

8d COMMON (0.131"X 2.5", MIN) TOENAILS AT 4" O.C. (4) TOENAILS IN TOP AND BOTTOM CHORD. PLUS

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD. ASCE 7-93 GABLE DETAIL DRAWINGS

REINFORCING MEMBER

4 TOENAILS

RIGID SHEATHING

TRUSS

TOENAILS SPACED AT 4" O.C.

SCE 7-98 GABLE DETAIL DRAWINGS A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A10030EC0207, A10030EC0207, A08530EC0207

ASCE 7-02 GABLE DETAIL DRAWINGS A08530EC0207

SCE 7-05 GABLE DETAIL DRAWINGS A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A13030EE0207, A11030EE0207, A11030EE0207, A10030EE0207 A13030E50207, A12030E50207, A11030E50207, A10030E503 A13015E50207, A12015E50207, A11015E50207, A10015E5020 07 A08515EE0207, A08530EE0207 A08515E50207 A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

4 TOENAILS

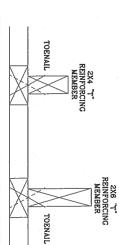
CEILING

ALPINE

ITW BUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

MEMVARMINGEM TRUSSES REQUIRE EXTREME CARE IN FARRICATING, HAUGILNG, SHIPPING, INSTALLING AND BRACING. REFER TO BEST GUILDING CORPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA CYODD TIXUS COUNCIL GRANERICA, 6300 EMTERRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRICE TO PERFORMING THESE FUNCTIONS. UNIESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL

WHIPDREVANTER FLEWISH COPY OF THIS DESIGN TO INSTALLATION CONFACTOR ITY BGG, INC., SMALL
NOT BE RESPONSUSBLE CTOR ANY DEVIATION CONFORMANCE ALLURE IN BUILD THE TRUSS IN
COMPONENCE WITH A PRI OR FABRICATING, HANDLING, SHEPPING, INSTALLING, & BACKING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE POPULSTONS OF MISS CHANTISHALING, SHEARCH OF OR VIX.ALSO,
DESIGN CONFORMS WITH APPLICABLE POPULSTONS OF MISS CHANTISHALING, SHEARCH AND AND TRI.
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SBCCI WIND LOAD. APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

# WEB LENGTH INCREASE W/ "T" BRACE

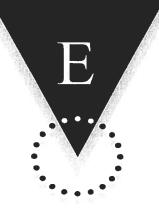
30 FT	70 MPH	15 FT	70 MPH	30 FT	80 MPH	15 FT	80 MPH	30 FT	90 MPH	15 FT	90 MPH	30 FT	100 MPH	15 FT	100 MPH	30 FT	110 MPH	15 FT	110 MPH	AND MRH	WIND SPEED
2x6	2x4	2x6	2x4	2x6	2x4	2x6	2x4	MBR. SIZE	T KEINE.												
10 %	10 %	2 0	0 %	20 %	20 %	10 %	2 01	30 %	10 %	20 %	20 %	40 %	10 %	30 %	10 %	50 %	10 %	40 %	10 %	25001	SBCCI
30 %	20 %	20 %	20 %	40 %	10 %	30 %	20 %	50 %	10 %	40 %	20 01	40 %	10 %	50 %	10 %	50 %	10 %	50 %	2 01	2000	ACCE.
_	_		_		_	_		_		_			_					_	_		

#### EXAMPLE:

GABLE VERTICAL = 24" O.C. SP #3 "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7" MEAN ROOF HEIGHT = 30 FT "T" MUMIXAM ASCE WIND SPEED = 100 MPH REINFORCING MEMBER SIZE = 2X4 REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 ጵ HC26294035

STATE OF DUR. FAC. ANY  WAY SPACING 240"
--



Prepared for:

#### THE SCOTT RESIDENCE

By:

#### Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED

#### SCHAFER ENGINEERING LLC

Trusses: Pre-engineered with manufacturer's required bracing system installed.  Roof sheathing: TypeSize// Fastener type nails 8d/.113 Ring Shank	
Interior zone spacing: Interior6 in. Periphery in.  Edge and end zone spacing: Interior6 in. Periphery in.	
Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 10 in.	
Studs: Type Spruce Grade #1 #2 Size 2 x 4 Interior stud spacing16 in. Composite (yes or no)Y End stud spacing16 in. Composite (yes or no)Y	
Shearwall siding: Type OSS Thickness 7/16 in.  1997— Trans: Fastener 13/1 Spacing: Int 1 in. Edge 2 in.  7997— Long: Fastener 13/1 Spacing: Int 1 in. Edge 2 in.	
Allowable unit shear on shearwalls: 4/8 pounds per linear foot Unit shear transferred from diaphragm: Trans: Long: 10 4	
Wall tension transferred by: Siding nails 24/3/@O.C. edges	
Foundation anchor bolts: Concrete strength 3000 psi Size 1/2 in. Shape L Washer 2" Embedment 7 in. Location of first anchor bolt from corner 8 in.	
Anchor Bolts @ 48" O.C. Model A307 Loc. from corner8 in.	
Type of foundation: 1 #5 rebar continuous required in bond beam.  Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. #5 at 72 in.  Monolithic footing: Depth 20 in. Bottom width 16 in. Reinf. 3 # 5 bars	
Footing: Width in. Depth/o in. Reinforcing#bars Interior Footings: 16" W X 10" D	
Porch Columns: 4x4x105x 10 26 20 MX Column Fasteners: Supro - CB44/cc 4400	eg 1 +
Special Comments: Intall ceiling deaph man On open porches using same Grade	
LILSIZES, + hack patterns as root she athing.	
NOTE:  1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.  2. All trusses must bear on exterior walls & porch beams.	

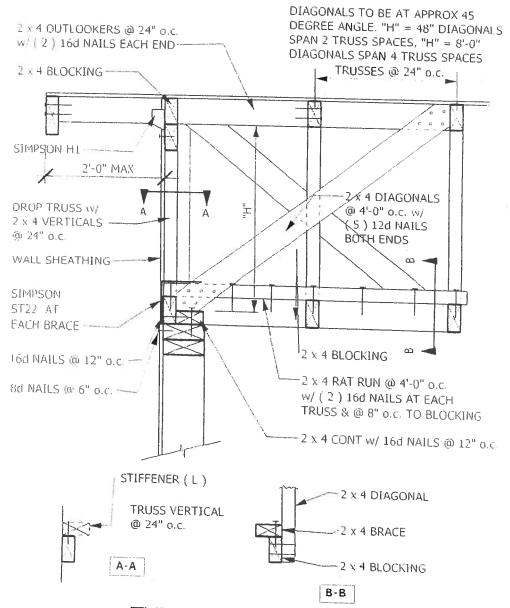
- 3. All walls to be nailed with same nailing pattern as shear walls.
- 4. This is a wind load only, NOT a structural analysis.
- 5. This wind load is not valid without a raised, embossed seal.
- 6. It is assumed that ideal soil conditions and pad preparations are provided.
- 7. Fiber mesh or WWM may be used in concrete slab.
- 8. Trusses must be anchored and supported in accordance to the truss engineering.
- 9. Wind design and analysis valid for one use only, no copies permitted.
- 10. The foundation is for minimum design use and may be increased.
- 11. All headers over 12 feet to be pre-engineered.

48984

7104 NW 42nd Ln Gainesville, FI

# SCHAFER ENGINEERING, LLC

#### 7104 N. W. 42<sup>ND</sup> LANE GAINESVILLE, FLORIDA 32606



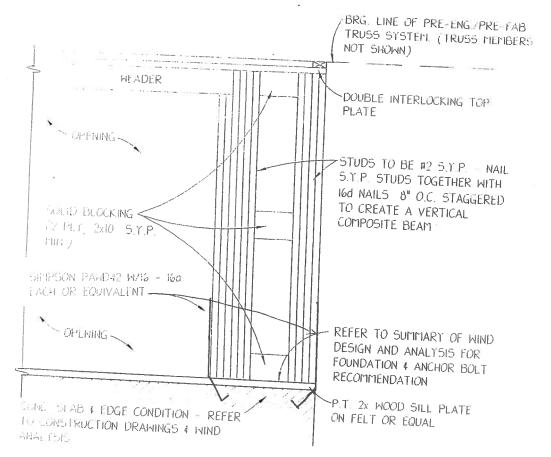
TYPICAL GABLE END BRACING

48984

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# SCHAFER ENGINEERING, LLC

7104 N. W. 42<sup>ND</sup> LANE GAINESVILLE, FLORIDA 32606



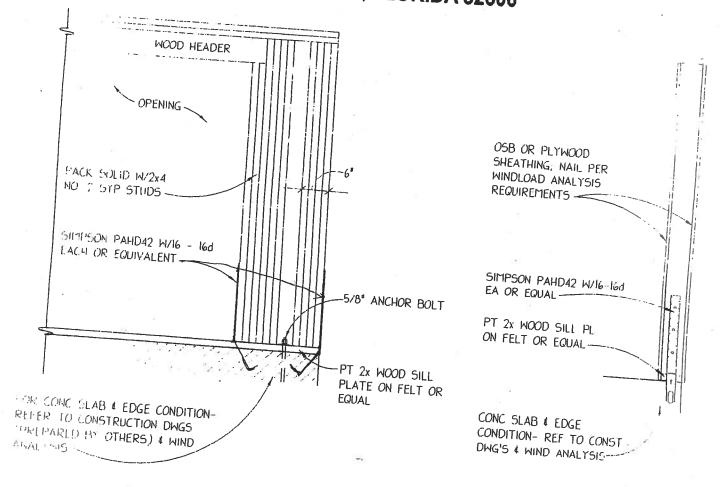
## MINIMUM 2'-0" SHEAR WALL SEGMENT

ALLOMABLE EQUIVALENT SHEAR WALL EQUAL TO 1.5 TIMES THE ACTUAL MALL SEGMENT LENGTH, MINIMUM WALL SEGMENT LENGTH OF 2'-0" MINIMUM WALL SEGMENT, SEE MINDLOAD ANALTSIS FOR NAIL SIZES AND SPACING ON SHEATHING.

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# SCHAFER ENGINEERING, LLC

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FRONT VIEW

SIDE VIEW

EQUIVALENT 3'-0" SHEAR WALL SEGMENT

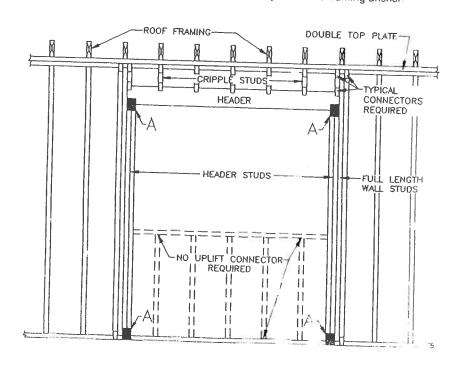
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		Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number	of Header	Studs 9	Supportin	ng End of	Heade
		11	1	2	2	2	2
Unsupported Wall Height	Stud Spacing		Numbe at E	r of Full	Length !	Studs er	*********
10' or less	12 in. 16 in. 24 in.	2 2 1	2 2 2	3 3 2	3 3	3	3 3
greater	12 in. 16 in.	2 2	2 2	3 3	4	<u>2</u> 5	2 5

1. The header stud shall not be required if the header is supported by a suitable framing anchor.

Uplift connection requirement at points A ttop and bottom of header studs): Uplift load per framing member above the header from Table 307F1 or 307A, as appropriate, multiplied by the number of framing members displaced divided by two

NOTE. Uplift connection is required at each end of header and at bottom of header studs in addition to connectors at wall studs and at top and bottom of cropples.



#### **TIE-DOWN TABLES**

HEADERS'		0		1
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors.

Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.

Strap ridge beam at each end with minimum uplift resistance of 1000#.

It is the contractors' responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

Top Connector **	Rating Lbs	Bottom Connector **	Rating
LSTA18*	1200	LTT19*	1250
2-LSTA18	2400	ABU44	2300
	LSTA18*	LSTA18* 1200	LSTA18* 1200 LTT19*

<sup>\*</sup>or per truss engineering

Use proper bolt anchors

All beams to be sheathed or strapped to Double Top Plate when applicable.

#### CRIPPLES | Sheathing nailing alone adequate w/8d nails @ 3" O.C.

# Wall sheathing nailing Adequate exterior walls bottom w/8d nails. Use SP1 & SP2 @32" O.C. on all interior non-sheathed bearing walls. Interior anchor bolts to be ½" x 8" A307 or ½" x 7" wedge anchor or equivalent.

- \*\* Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as NOTE:
- 1. For nailing into SPF members, multiply table values by .86
- 2. See truss engineering for anchor tie-down values.

#### Wind Load Design per ASCE 7-02

User Input Data					
Structure Type	Building	4. 11 =			
Basic Wind Speed (V)	110	mph			
Structural Category	H				
Exposure	В				
Struc Nat Frequency (n1)	1	Hz			
Slope of Roof (Theta)	30.3	Deg			
Type of Roof	Hipped				
Eave Height (Eht)	10.00	ft			
Ridge Height (RHt)	24.42	ft			
Mean Roof Height (Ht)	17.38	ft			
Width Perp. to Wind (B)	59.00	ft			
Width Parallel to Wind (L)	71.33	ft			
Damping Ratio (beta)	0.01	100			

Red values should	be changed only	through "Main	Menu"
-------------------	-----------------	---------------	-------

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.29
Flexible Structure	No

Calculated Parameters					
Importance Factor	1				
Hurricane Prone F	Region (V>100 m	ph)			
Table C6	6-4 Values				
Alpha =	7.000				
zg =	1200.000				
At =	0.143	9 = -			
Bt =	0.840				
Am =	0.250				
Bm =	0.450				
Cc =	0.300				
I =	320.00	ft			
Epsilon =	0.333				
Zmin =	30.00	ft			

	Gust Factor Category I: Rigid Structures - Simplified Meti	nod
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
	Gust Factor Category II: Rigid Structures - Complete Anal	ysis
Zm	Zmin	30.00 ft
lzm	Cc * (33/z)^0.167	0.3048
Lzm	I*(zm/33)^Epsilon	309.99 ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8906
Gust2	0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	0.8605
	Gust Factor Category III: Flexible or Dynamically Sensitive Str	ructures
Vhref	V*(5280/3600)	161.33 ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89 ft/s
NF1	NatFreq*Lzm/Vzm	4.37 Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552
Nh	4.6*NatFreq*Ht/Vzm	1.13
Nb	4.6*NatFreq*B/Vzm	3.83
Nd	15.4*NatFreq*Depth/Vzm	15.50
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5348
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2271
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0625
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.6122
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19
Gust3	0.925*((1+1.7*lzm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*lzm))	1.02

Gust Factor Summary						
Main Wind-force re	esisting system:	Components and Cla	dding:			
Gust Factor Category:	I	Gust Factor Category:	Ī			
Gust Factor (G)	0.86	Gust Factor (G)	0.86			

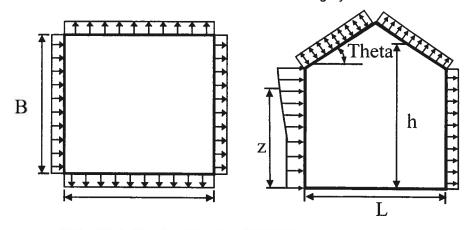
#### Wind Load Design per ASCE 7-02

#### 6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (ib/ft/	
					Windwa	rd Wall*
ft			1.00	lb/ft^2	+GCpi	-GCpi
24.42	0.70	1.00	1.00	21.70	11.60	18.28
20	0.70	1.00	1.00	21.70	11.60	18.28
17.38	0.70	1.00	1.00	21.70	11.60	18.28
15	0.70	1.00	1.00	21.70	11.60	18.28

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units	
Kh	2.01*(Ht/zg)^(2/Alpha)	0.60		
Kht	Topographic factor (Fig 6-2)	1.00		
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	18.57	psf	

Wall Pressure Coefficients, Cp	
Surface	Ср
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Ср	Pressure	(psf)
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 59 ft wall)	-0.46	-10.66	-3.98
Leeward Walls (Wind Dir Parallel to 71.33 ft wall)	-0.50	-11.33	-4.65
Side Walls	-0.70	-14.53	-7.84
Roof - Normal to Ridge (Ti	neta>=10)		
Windward - Max Negative	-0.19	-6.35	0.34
Windward - Max Positive	0.31	1.55	8.23
Leeward Normal to Ridge	-0.60	-12.93	-6.24
Overhang Top	-0.19	-3.00	-3.00
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (A	All Theta)		•••
Dist from Windward Edge: 0 ft to 8.69 ft	-0.90	-17.72	-11.04

#### Wind Load Design per ASCE 7-02

Dist from Windward Edge: 8.69 ft to 17.38 ft	-0.90	-17.72	-11.04
Dist from Windward Edge: 17.38 ft to 34.76 ft	-0.50	-11.33	-4.65
Dist from Windward Edge: > 34.76 ft	-0.30	-8.14	-1.45

<sup>\*</sup> Horizontal distance from windward edge

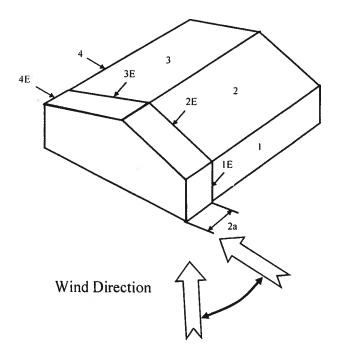
#### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.60
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.57

			Case /	1		
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

<sup>\*</sup> p = qh \* (GCpf - GCpi)



#### Wind Load Design per ASCE 7-02

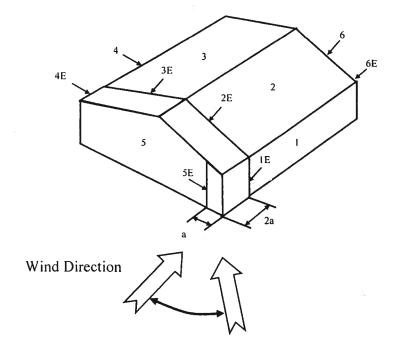
#### Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.60
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.57

			Case I	3	a a	
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

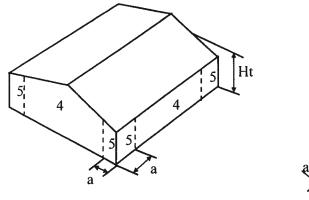
<sup>\*</sup> p = qh \* (GCpf - GCpi)

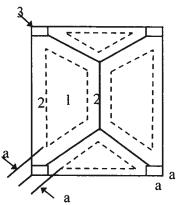


Wind Load Design per ASCE 7-02

#### Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Hipped Roof 10 < Theta <= 30

a = 5.9

==>

5.90 ft

Component	Width	Length	Area	Zone	G	Ср	Wind Pre	ss (lb/ft^2)
	(ft)	(ft)	(ft^2)		Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	18.47	-22.46
	0	0	0.00					
	0	0	0.00					
	0	0	0.00				-	
	0	0	0.00					
	0	0	0.00					
	0	0	0.00				managed of	
	0	0	0.00					
	0	0	0.00					
	0	0	0.00			,		
	0	0	0.00					
11	0	0	0.00				10 (000 000 000 000 000 000 000	1-01-00
	0	0	0.00					
	0	0	0.00					
7= = 2	0	0	0.00					
	0	0	0.00				in the second	
	0	0	0.00					
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	0	0	0.00				77 27 27 2	
	0	0	0.00				ten e m	
	0	0	0.00			THE CHILD	(**************************************	

Note: \* Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Wind Load Design per ASCE 7-02

Condition	Go	pi
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

#### Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

91			Ср	
Condition	Variable	Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Ср	0.13	-1	-0.5
	P (+GCpi) - psf	-1.35	-19.32	-11.33
	P (-GCpi) -psf	5.34	-12.63	-4.65
Roof Springing from Ground	Ср	0.42	-1	-0.5
	P (+GCpi) - psf	3.37	-19.32	-11.33
	P (-GCpi) -psf	3.37	-19.32	-11.33

#### Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	T
L	Roof dimension normal to wind direction	71.33	ft
В	Roof dimension parallel to wind direction	59.00	ft
L/B	Ratio of L to B	1.209	
Theta	Slope of Roof	30.3	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft

#### PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

moet for any of the applicace	Manufacturer	Product Description	Approval Number(s)
		Florida Description	
	not yet purchased		
SWNGING	$u \cdot u \cdot u$		
SLIDING	11		
SECTIONAL/ROLL UP	MOAY yet purchosed		
OTHER			
		70	
WINDOWS	not yet purchased	/	
SINGLE/DOUBLE HUNG	, ,		
HORIZONTAL SLIDER			
CASEMENT			
. FIXED			
. MULLION			
. SKYLIGHTS			
3. OTHER	V		
		,	
, PANEL WALL	not yet purchase		
A. SIDING			
3. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER	V		
		A	
4. ROOFING PRODUCTS	not yet ournose	<i>g</i>	
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL	,		
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			<del> </del>
5. STRUCT COMPONENTS	see attached		
A WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
A			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.				
	APPLICANT SIGNATURE	DATE		

3867553885

ASPEN PEST CONTROL

PAGE 01/01



Lake City (386) 755-3611 Gainesville (352) 494-5751 Fax (388) 755-3885 Toll Free 1-900-616-4707

WAYNE 26997)

Notice of Intent for Preventative Treatment for Termites

(as required by Florida Building Code (PBC) 104.2.6)

Aspen Pest Control, Inc. (386) 755-3611 State License # - JB109476 State Certification # - JF104376

(Ragan) 149 NW Pinellas Ct. Lake City, Fl (Isaac Construction)

Address of Treatment or Lot/Black of Treatment

Bora-Care Wood Treatment - 23% Disodium Octaborate Tetrahydrate

Method of Termite Prevention - Soil Barrier, Wood Treatment, Bait System, Other

Application onto Structural Wood

Description of Treatment

The above named structure will receive a complete treatment for the prevention of subterranean termites at the dried-in stage of construction. Treatment is done in accordance with the rules and laws established by the Florida Department of Agriculture and Consumer Services and according to EPA registered label directions as stated in Florida Building Code Section 1861.1.8.

Muchelle HECher
Authorized Signature

9-26-07







# COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-2S-16-01841-011

Building permit No. 000025990

100.50

Fire:

Use Classification SFD/UTILITY

Permit Holder JULIUS HARDEN

Owner of Building TIMOTHY & ALLISON SCOTT

139.02

Total:

Waste: 38.52

Location: 195 NW WHITEWATER GLN

Date: 04/15/2008

**Building Inspector** 

**POST IN A CONSPICUOUS PLACE** Business Places Only)