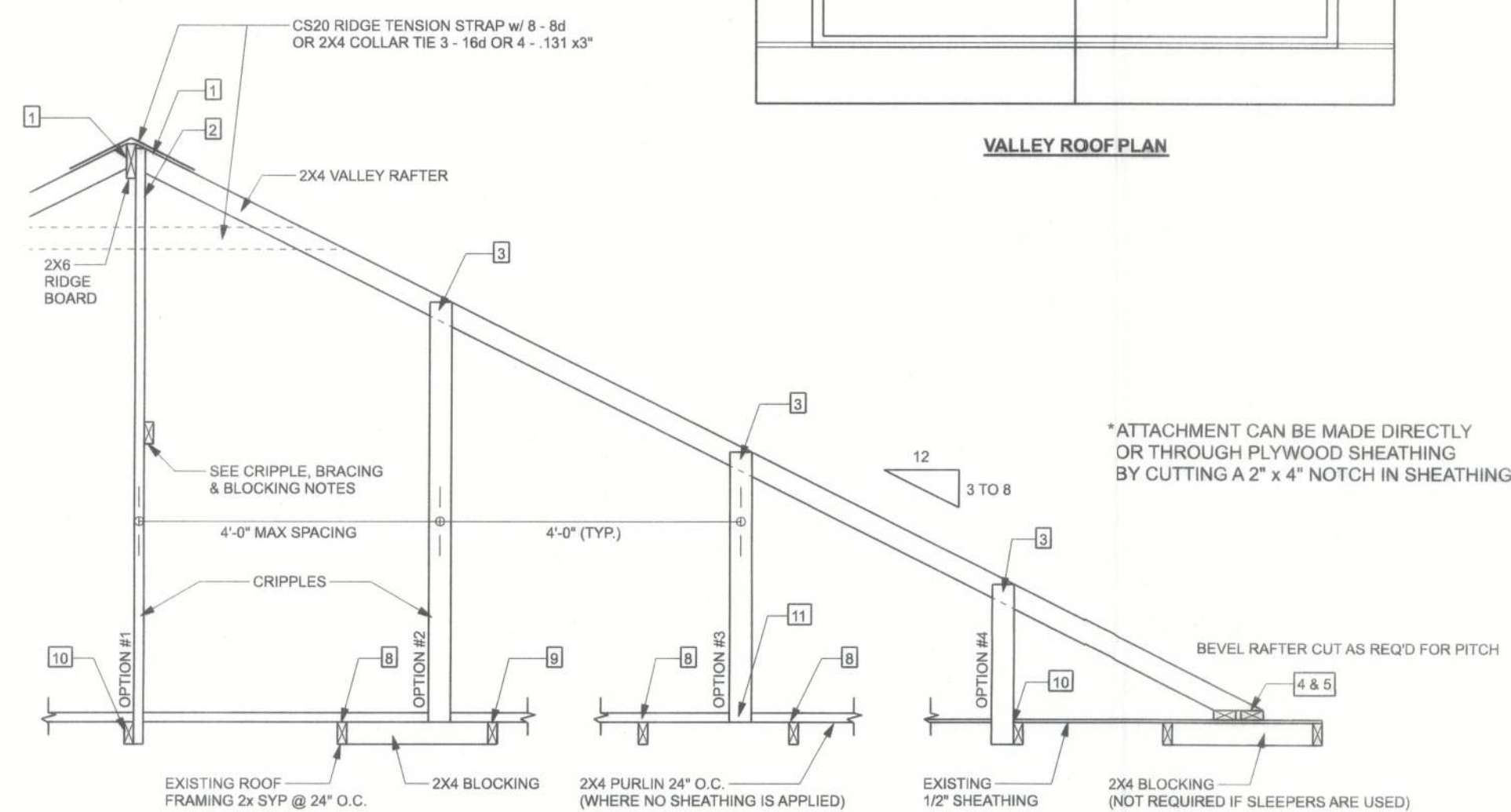


RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



=====	TRUSS
== == ==	TRUSS UNDER VALLEY FRAMING
: : : : : :	VALLEY RAFTER OR RIDGE
⊞	CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

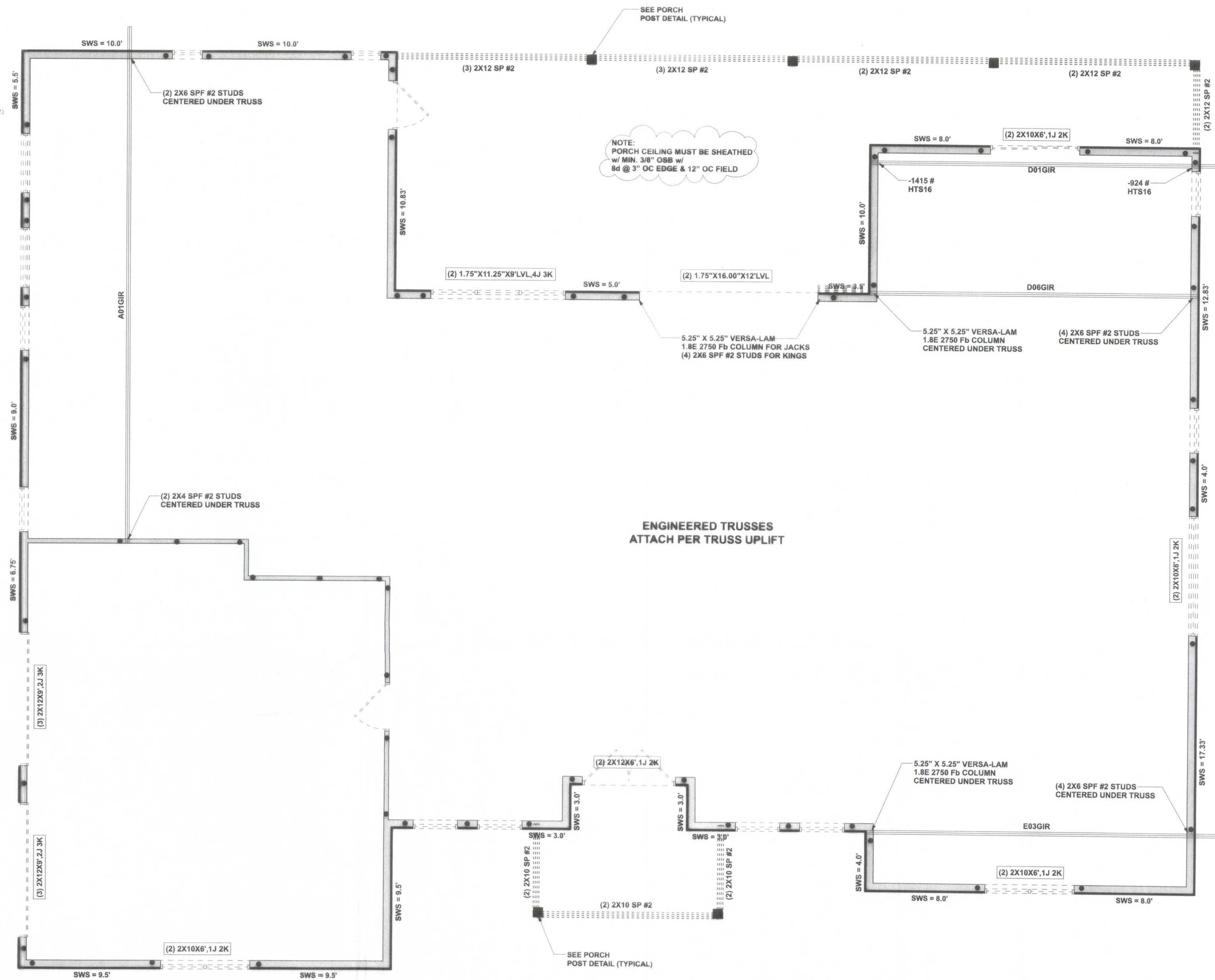
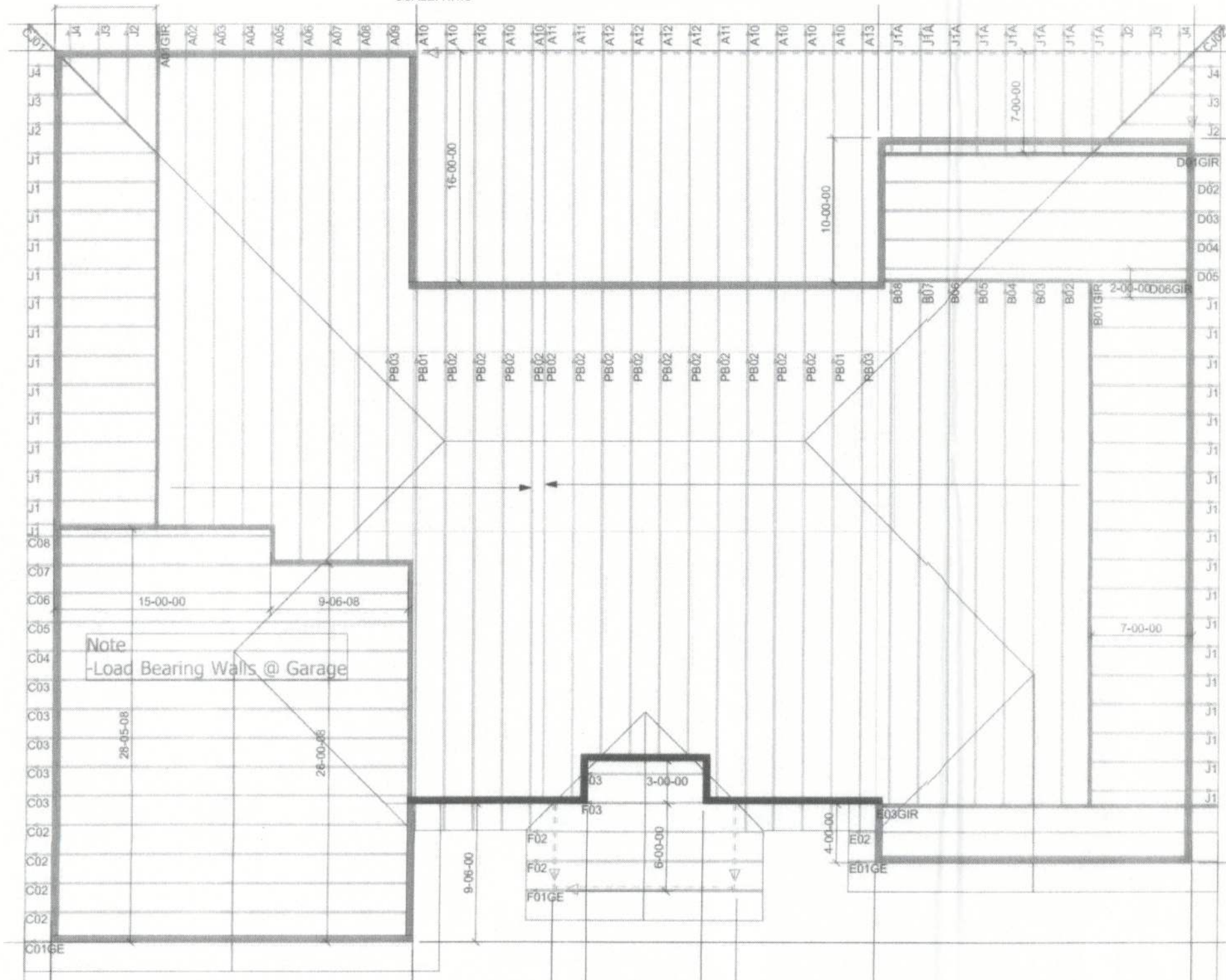
1	2X4 RAFTERS TO RIDGE	3-16d OR 8 - 131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 8 - 131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 8 - 131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12 - 131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8 - 131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3-16d OR 8 - 131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3-16d OR 8 - 131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP)	3-16d OR 8 - 131 x 3" NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8 - 131 x 3" NAILS
10	TRUSS TO BLOCKING	3-16d OR 8 - 131 x 3" END NAILS
11	CRIPPLE TO TRUSS	3-16d OR 8 - 131 x 3" FACE NAILS
12	CRIPPLE TO PURLIN	3-16d OR 8 - 131 x 3" FACE NAILS

MAXIMUM RASTER SPANS
 6"0" FOR 2X4, 9"0" FOR 2X6 SP#2 OR SYP #2.
 MAXIMUM RASTER AREA PER SUPPORT
 1612 IN² ZONES 2, 3 AND 4; 2400 IN² ZONE 1; [EXAMPLE: 4'0" O.C. X 4'0" SPAN
 = 1612 OR 2'0" X 8'0" SPAN = 1612]
 PURLINS REQUIRED 2'0" O.C. IF EXISTING SHEATHING IS REMOVED.
 PURLINS SHOULD BE SPACED TO CARRY ONE TRUSS SPACING MINIMUM.
 IN CASES WHERE THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM
 OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
 MINIMUM OF 8" - 8d COMMON WIRE NAILS.
 PURLINS SHALL BE SPACED TO MEET ALL THE FOLLOWING CONDITIONS
 - SPANS (DISTANCES BETWEEN HEELS) 4'0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 14"0" OR LESS
 - MAXIMUM WIND SPEED: 140 mph
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MAXIMUM TOTAL LOADING: 40 psf
 - MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS
 EXPOSURE CATEGORY: C, K=1, I=1.0, Kd=1
 ENCLOSED BUILDING

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG. NAILS W/ 2- 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE 2X4 OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADUATED LUMBER & BOLT OR COMMON NAILS.

- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FLAT BETWEEN LOWER TRUSS TOP CHORD AND LATERAL BRACE IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

SCALE: N.T.S.



SCALE: 1/4" = 1'-0"

SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
SN-3	USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
SN-4	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
SN-5	PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC511-03, BC511-B1, BC511-B2, & BC511-B3. BC511-B1, BC511-B2, & BC511-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

Diagram illustrating the components of a beam call-out label:

- 2: NUMBER OF KING STUDS (FULL LENGTH)
- 2X10X0': SPAN OF HEADER
- 1J: NUMBER OF JACK STUDS (UNDER HEADER)
- 1K: SIZE OF HEADER MATERIAL
- HEADER/BEAM CALL-OUT (U.N.O.): NUMBER OF PLIES IN HEADER

INDICATES LOCATION OF:
3/8" A307 ALL THREADED ROD

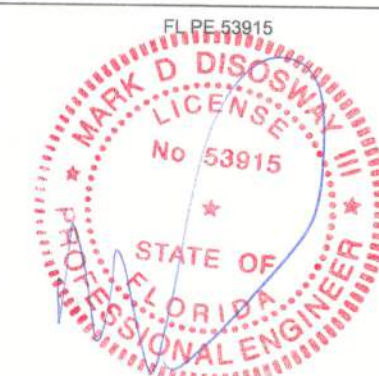
ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	22977 LBF	20150 LBF
REQUIRED	18643 LBF	13319 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. MAYO TRUSS CO
JOB #0423-063

B&B Homes

Austin & Angie Seay Res.

PROJECT ADDRESS:
Birley Road - TBD SW Paces Glen,
Columbia County, FL



Friday, August 25, 2023

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
231045

S-3
OF 3 SHEET

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

NOTE: FOR STEM WALL FOUNDATIONS OVER 5 COURSES IN HEIGHT THE SLAB IS REQUIRED TO BE ATTACHED TO THE STEM WALL @ BOND BEAM w/ 2" X 2" #5 CORNER REBARS

- (1) LEG EXTENDING INTO SLAB &
- (1) LEG LAPPED w/ THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR
- (1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION w/ STEM WALL.
- #5 VERT. REBAR w/ STD. HOOK BOTTOM IN FOOTING & STD. HOOK TOP IN BOND BEAM @ EACH CORNER & 96" OC
- 8X8X16, RUNNING BOND
- CMU STEM WALL, MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES)
- 20" W X 10" D POURED CONCRETE STRIP FOOTING w/ (2) #5 REBAR CONTINUOUS

CONCRETE SLAB

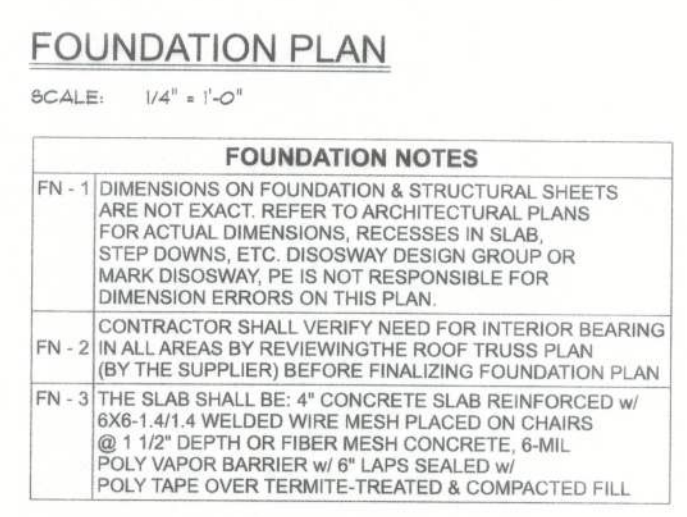
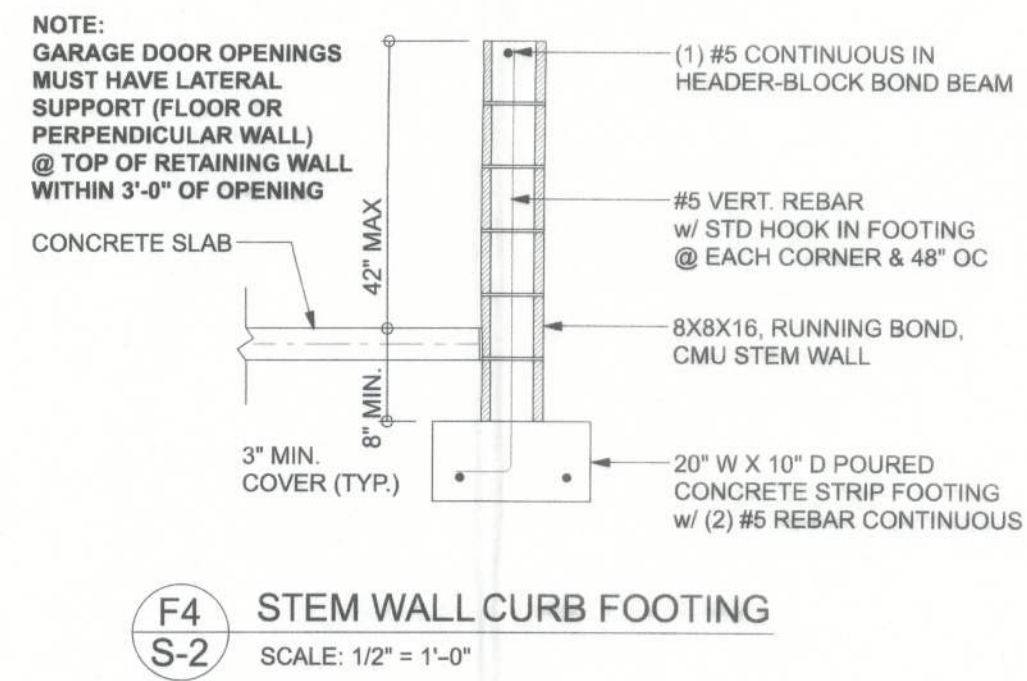
3" MIN COVER (TYP.)

F1

S-2

SCALE: 1/2" = 1'-0"


BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL

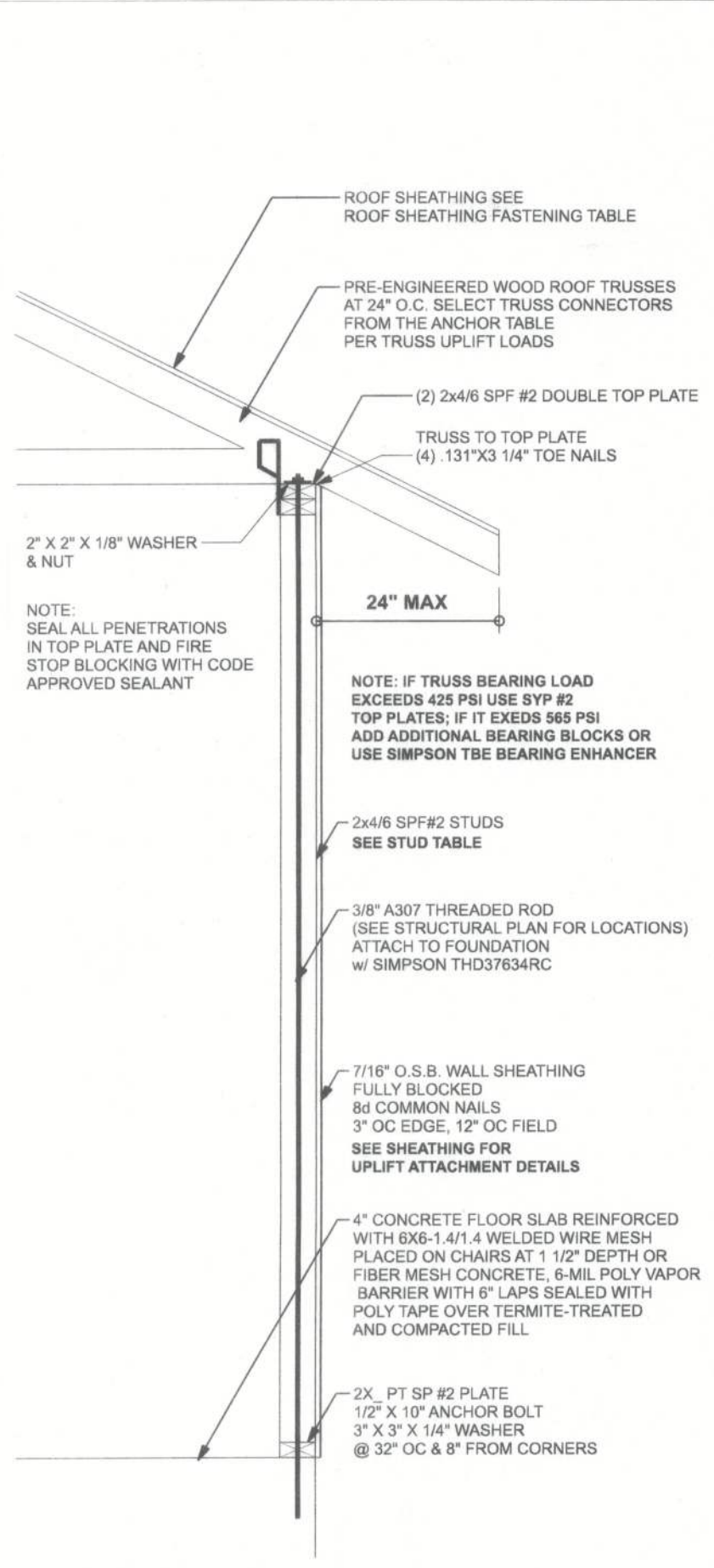


B&B Homes

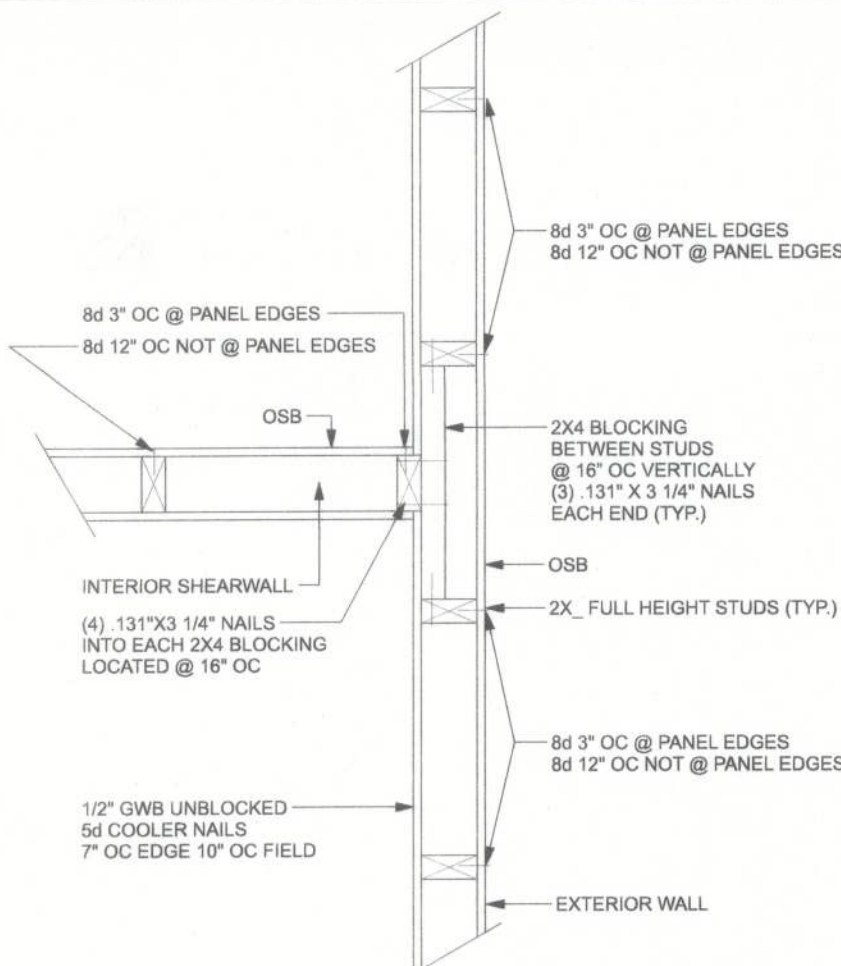
Austin & Angie Seay Res.

PROJECT ADDRESS:
Birley Road - TBD SW Paces Glen,
Columbia County, FL

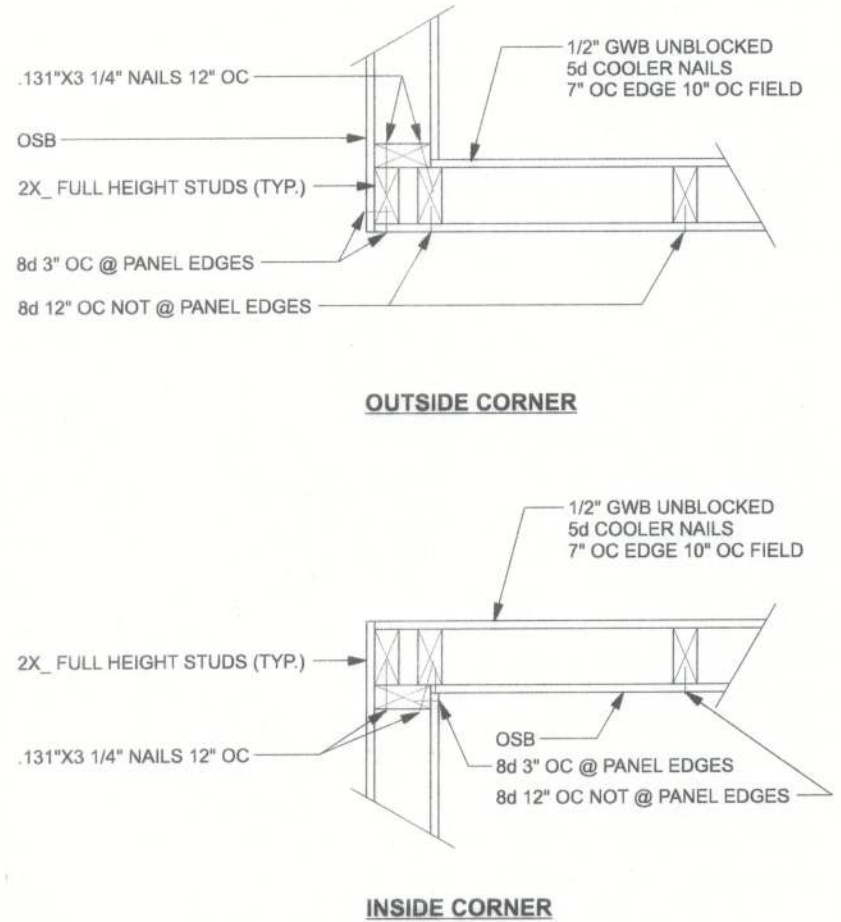
<div style="text-align: center;"><p>FL-PE 53915</p><p>10/14/14</p></div>
<p style="text-align: center;">Friday, August 25, 2023</p>
<p>DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.</p>
<p>COPYRIGHTS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby acknowledges its common law copyrights and property right in these dimensions of service. This document is not to be reproduced, altered or copied in any form or manner without the express written permission and consent of Mark Disoway.</p>
<p>CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition of ASCE 7-16, Building Code Residential (2020) to the best of my knowledge.</p>
<p>LIMITATION: This design is valid for one building, at specified location.</p>
<p style="text-align: center;">Mark Disoway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disowaydesign@gmail.com</p>
<p style="text-align: center;">JOB NUMBER: 231045 S-2 OF 3 SHEETS</p>



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



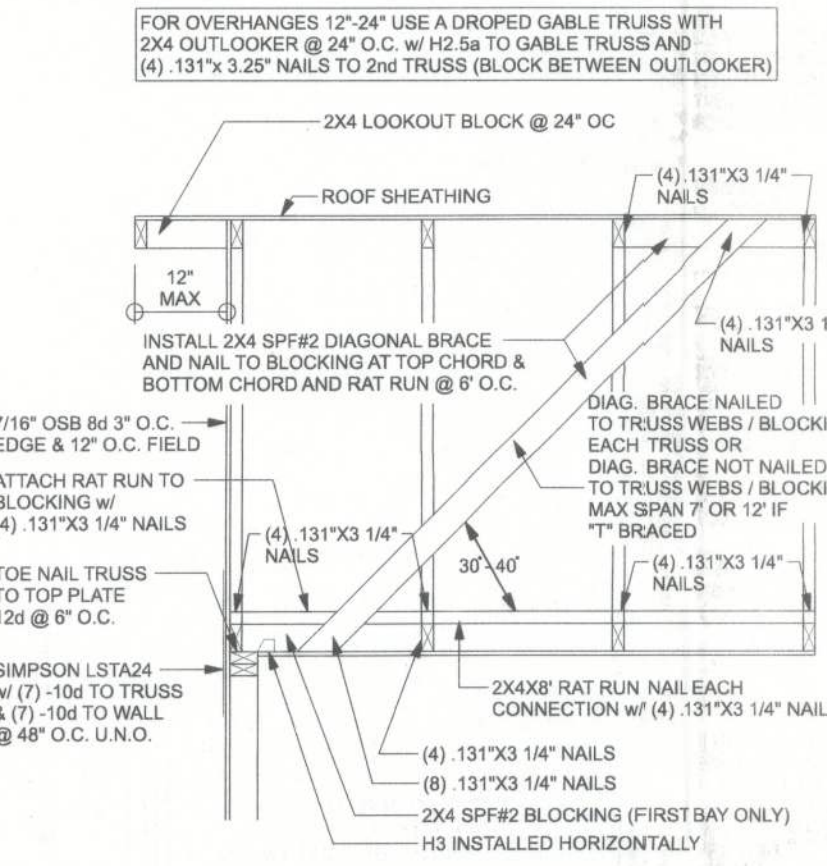
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

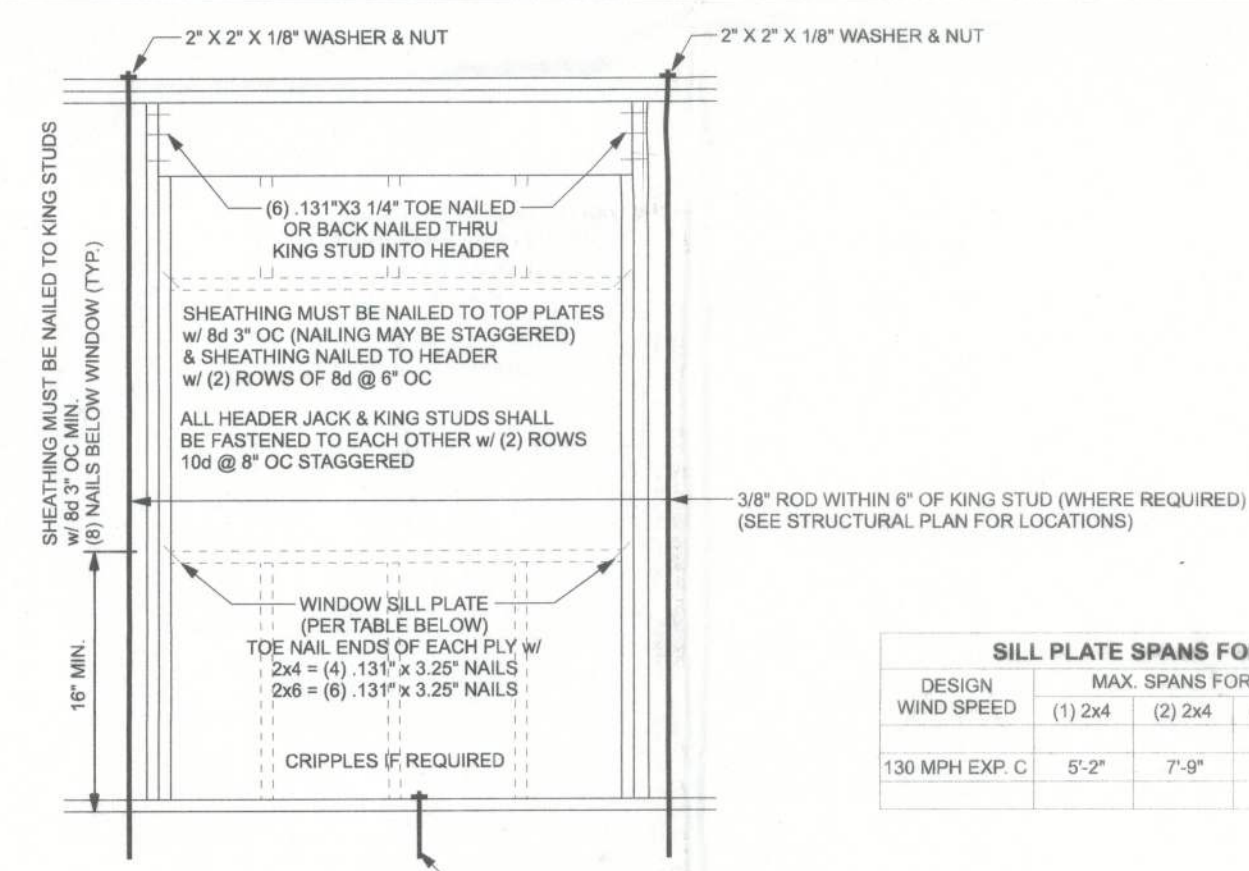
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 8 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

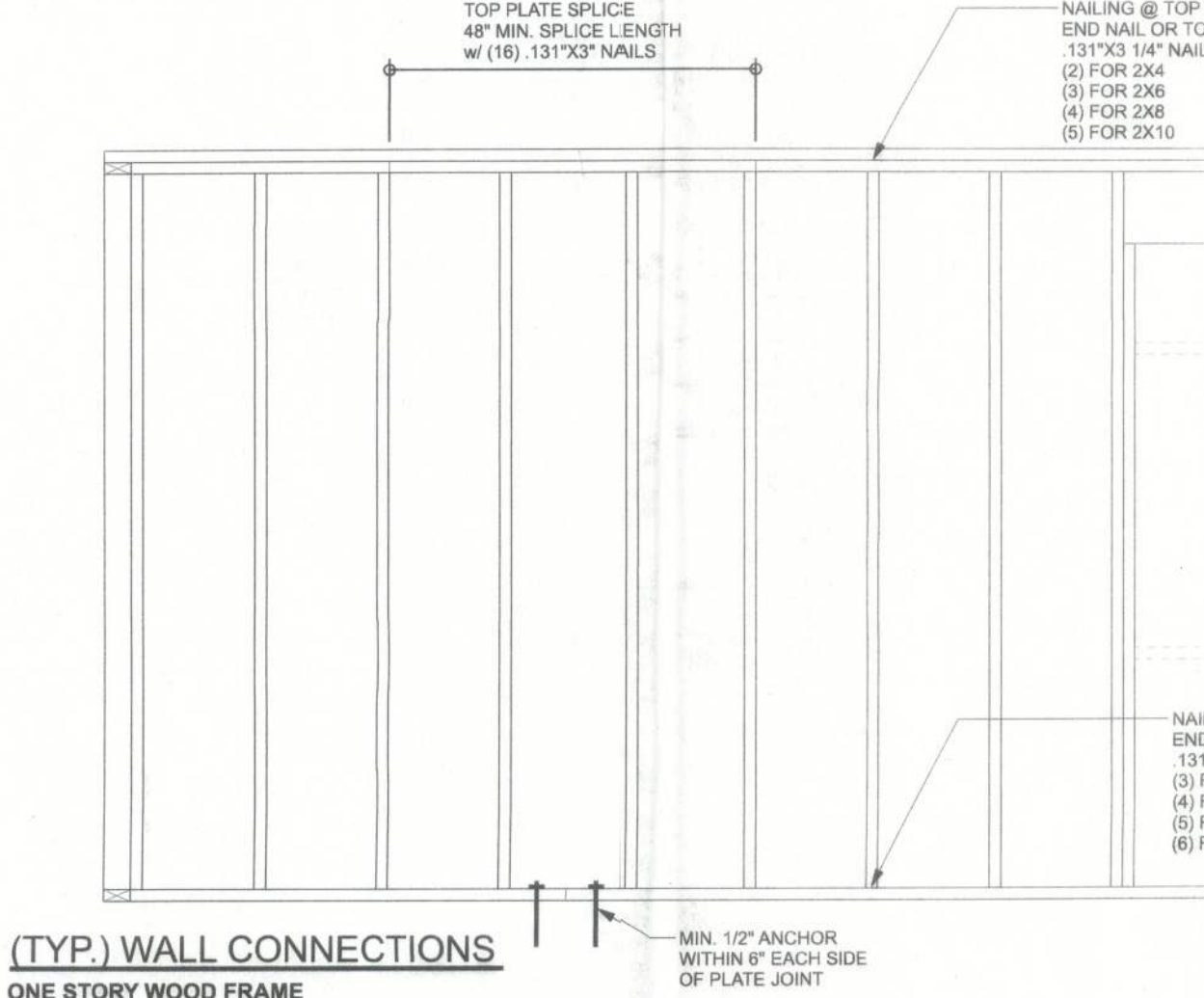


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

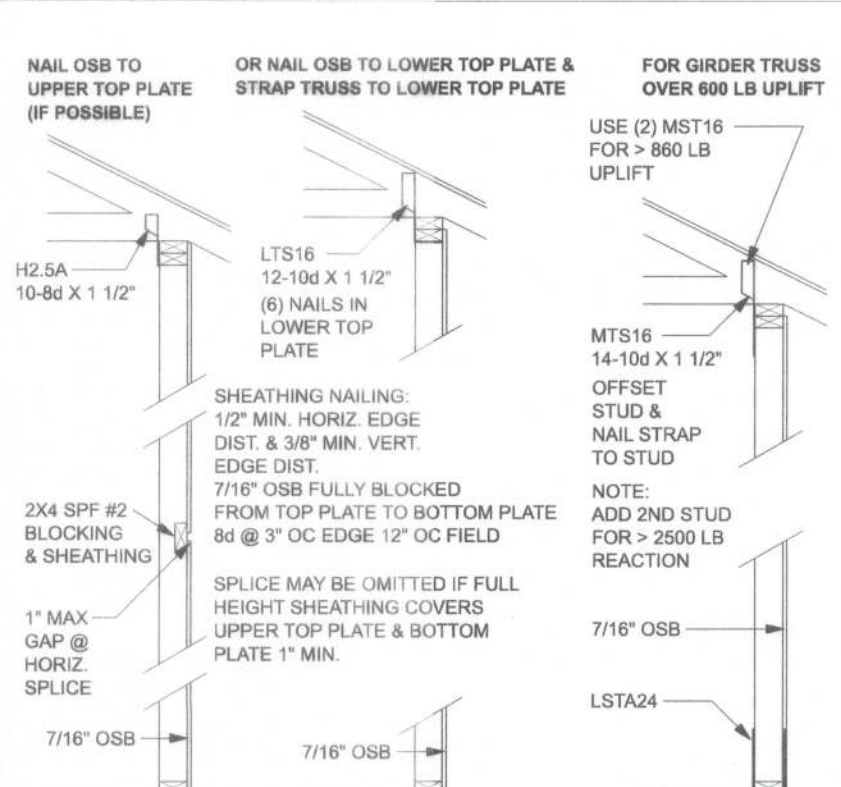
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



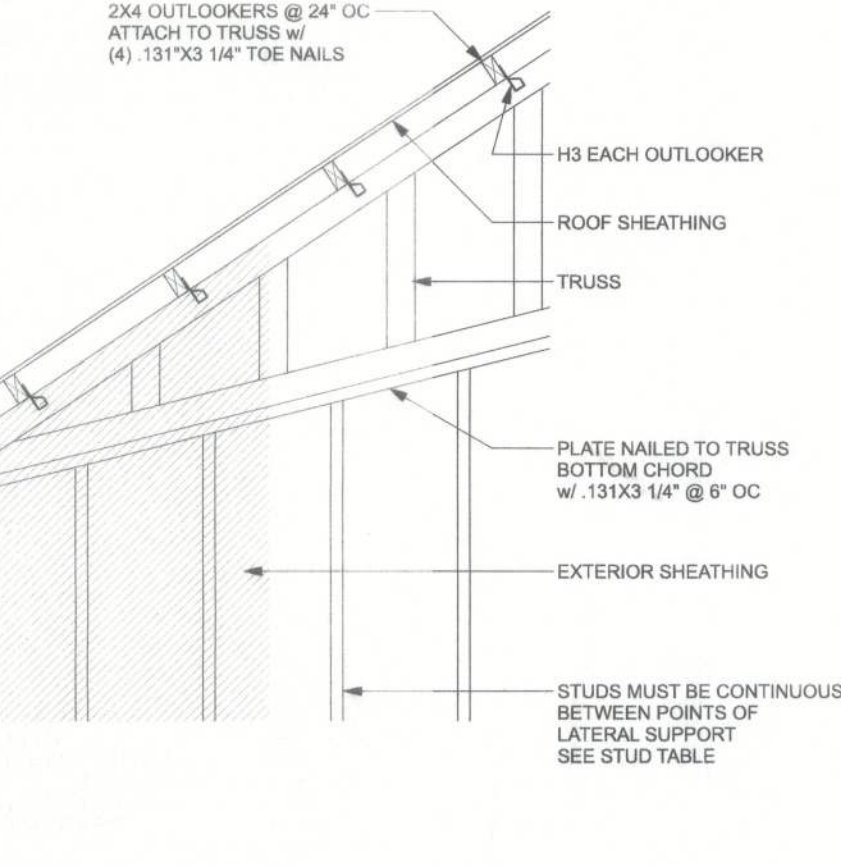
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



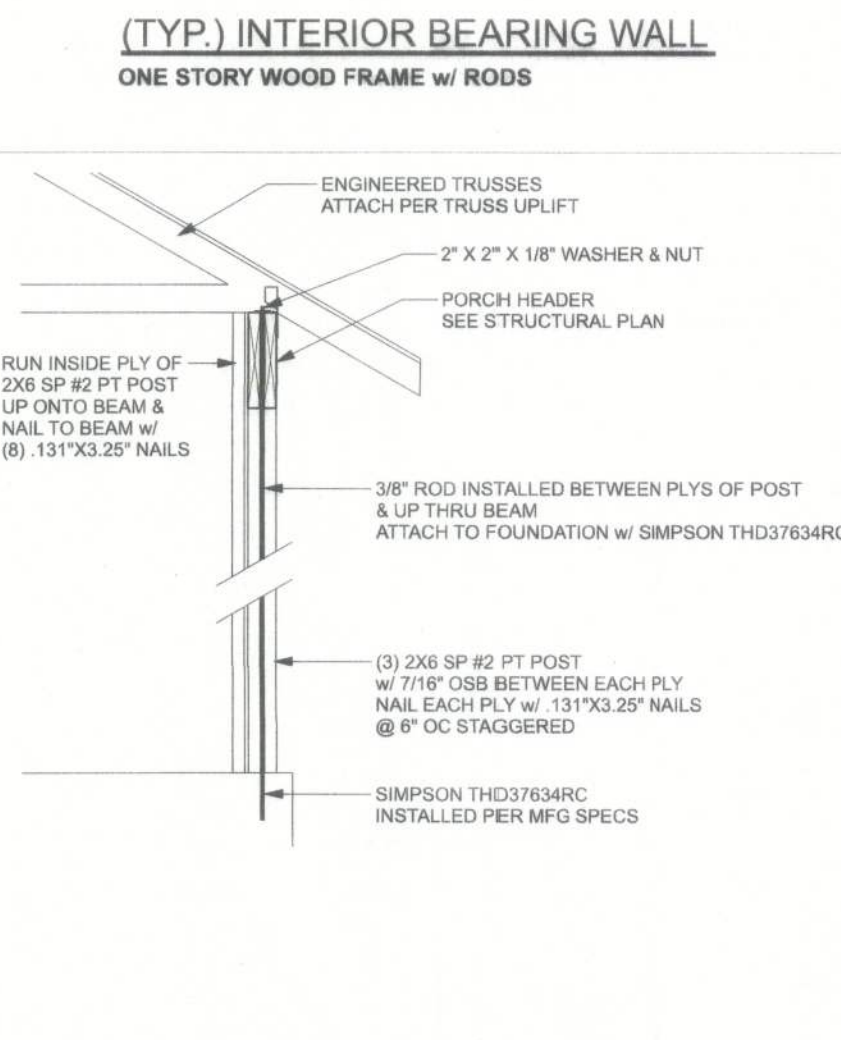
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



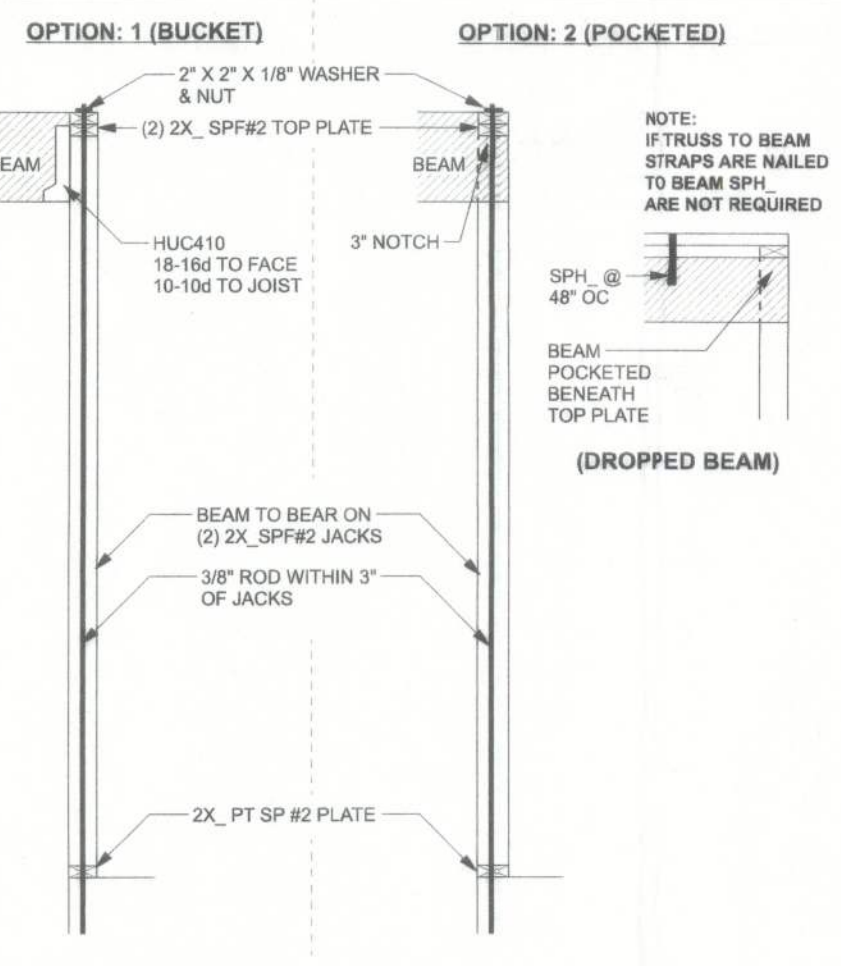
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



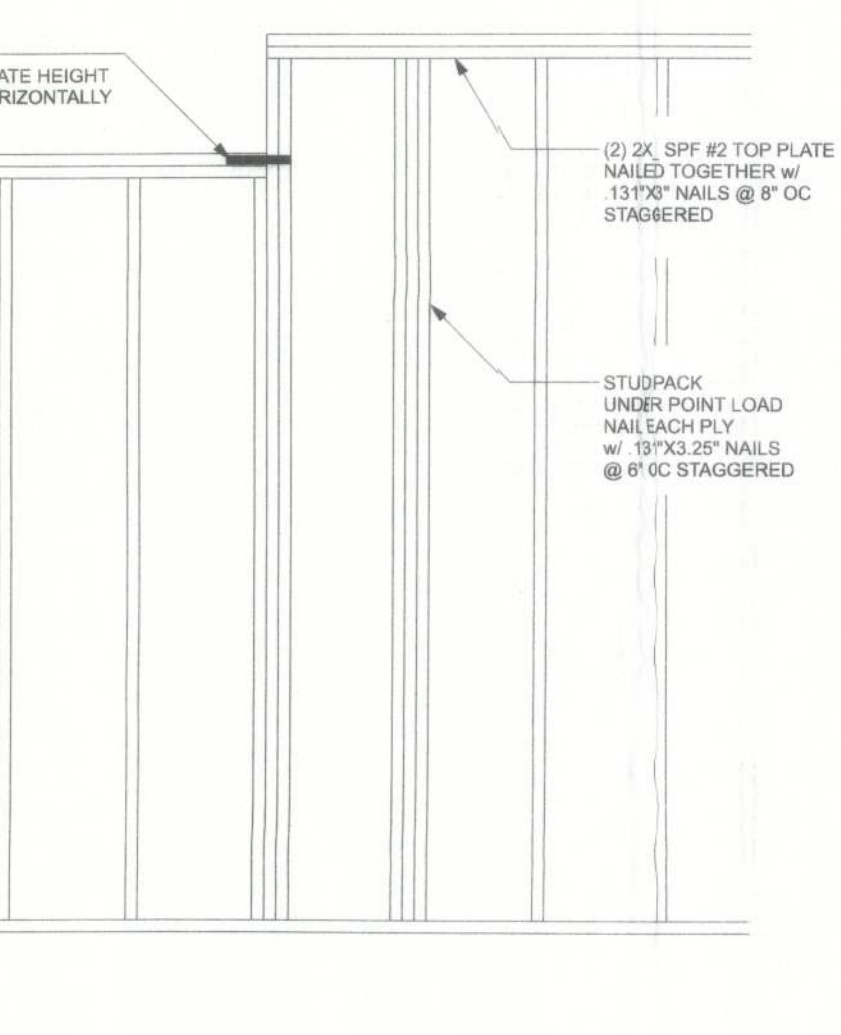
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"
575	495	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"
1540	1015	HT10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12.20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12.30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20.30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x8" Titen HD
4235	3640	HTT4	18-16dX2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x7" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SP#2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTH FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)	
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 86KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORM TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPACES 4' DB (25' FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:	
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 38 GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
FLOOR DESIGN PRESSURES	
SEE TABLE	
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)	
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR
0 - 20	+25.6(Vasd) -27.8(Vasd)
0 - 20	+42.6(Vult) -46.2(Vult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)	
9x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

B&B Homes
Austin & Angie Seay Res.



Friday, August 25, 2023

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
231045
S-1
OF 3 SHEETS