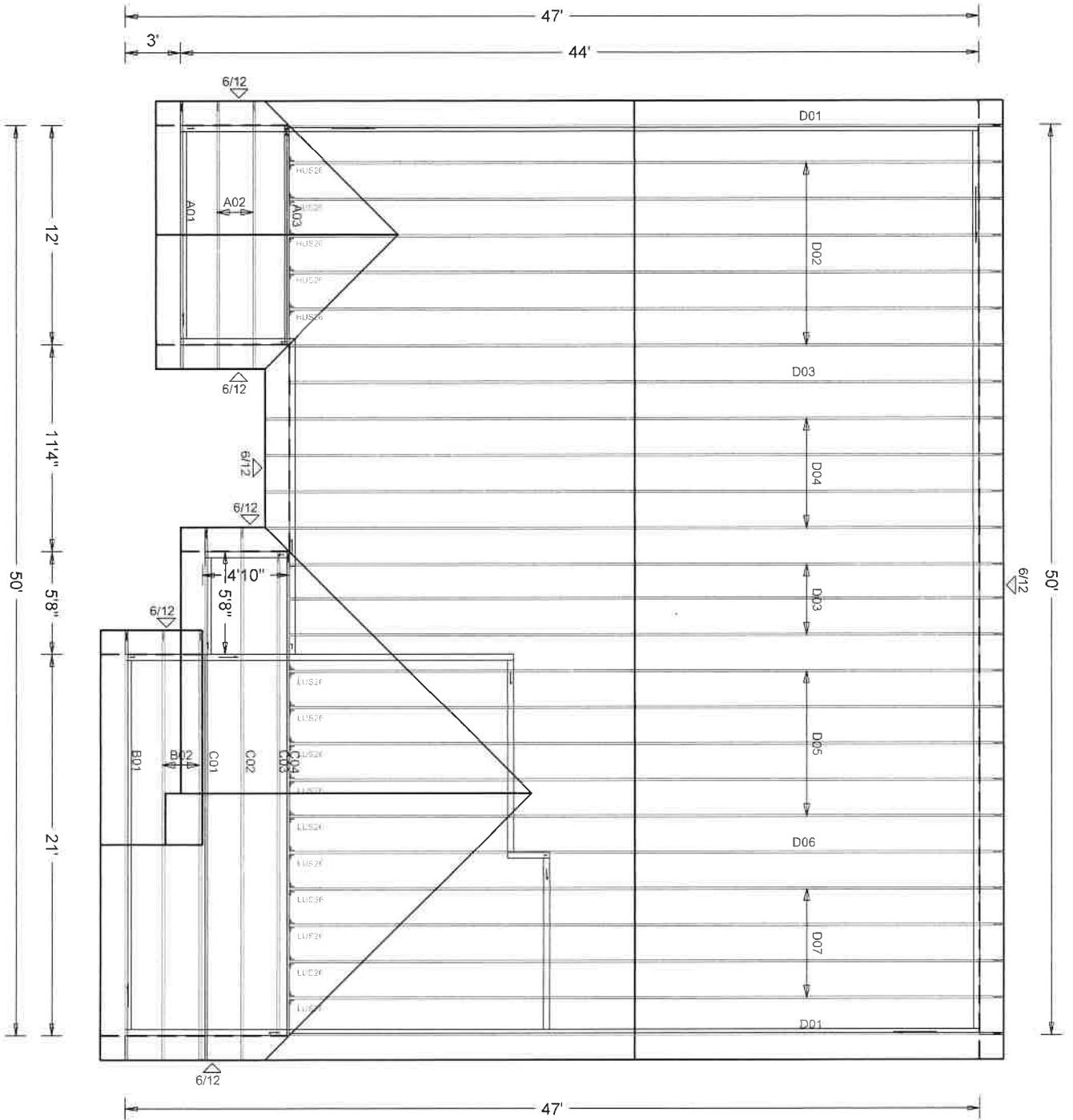




W.B. Howland Truss Co.
610 11th St. SW
Live Oak, FL 32064
(386) 362-1235
(386) 362-7124 (Fax)
howlandtruss@gmail.com

ROOF PITCH: 6/12
OVERHANG: 16"
CEILING: 8"
EXT. WALLS: 4"
LOADING: 40PSF
WIND LOAD: 130MPH
EXPOSURE: B

DATE: 10/3/22

Truss to Truss Connectors:
(6) HUS26
(10) LUS26

JOB #: 22-8293

Job Name: Mayer - Litchfield Classi
Customer: Red Door Homes
Designer: Kelly Caudill
ADDRESS:
SALESMAN: Fill in later
: <Not Found>

JOB NO:

22-8293

PAGE NO:

1 OF 1



This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.



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10/03/2022

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8293
Job Description: Mayer - Litchfield Classic	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.01 JRef #: 1XJH2150006
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 16 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	276.22.1459.51703	A01
3	276.22.1459.46913	A03
5	276.22.1459.31930	B02
7	276.22.1459.05740	C02
9	276.22.1458.55810	C04
11	276.22.1458.37823	D02
13	276.22.1458.33690	D04
15	276.22.1458.28140	D06
17	BRCLBSUB0119	
19	GBLLETIN0118	

Item	Drawing Number	Truss
2	276.22.1459.48543	A02
4	276.22.1459.35510	B01
6	276.22.1459.23413	C01
8	276.22.1459.00603	C03
10	276.22.1458.39353	D01
12	276.22.1458.35497	D03
14	276.22.1458.31173	D05
16	276.22.1458.21617	D07
18	A14015ENC160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

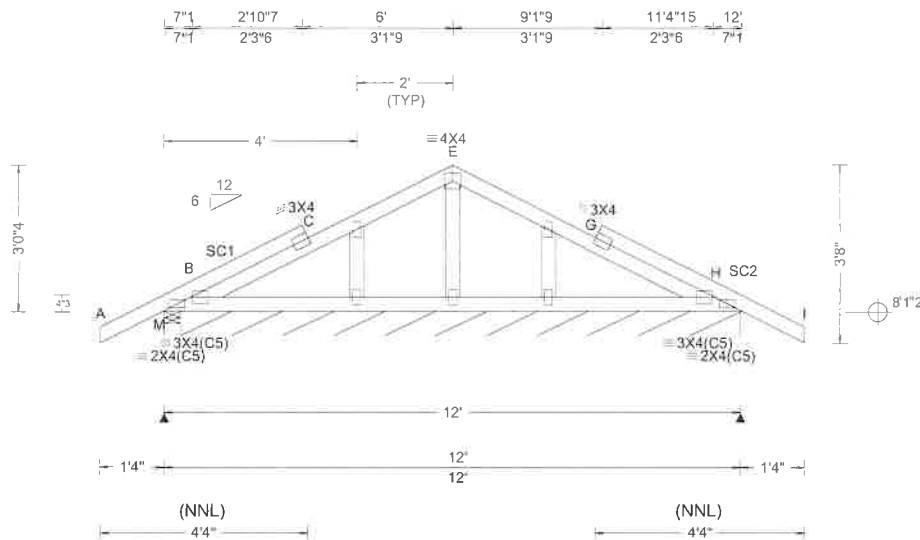
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 446322 FROM: CDM	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 22-8293 Mayer - Litchfield Classic Truss Label: A01	Cust: R 215 JRef: 1XJH2150006 T1 DrwNo: 276.22.1459.51703 KD / YK 10/03/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.006 C 999 180 HORZ(LL): -0.001 G - - HORZ(TL): 0.003 G - - Creep Factor: 2.0 Max TC CSI: 0.192 Max BC CSI: 0.068 Max Web CSI: 0.043 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity M 258 /- /- /150 /18 /73 H* 78 /- /- /42 /2 /- Wind reactions based on MWFRS M Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = 140 Min Req = - Bearings M & B are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

The overall height of this truss excluding overhang is 3'-0-4.



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****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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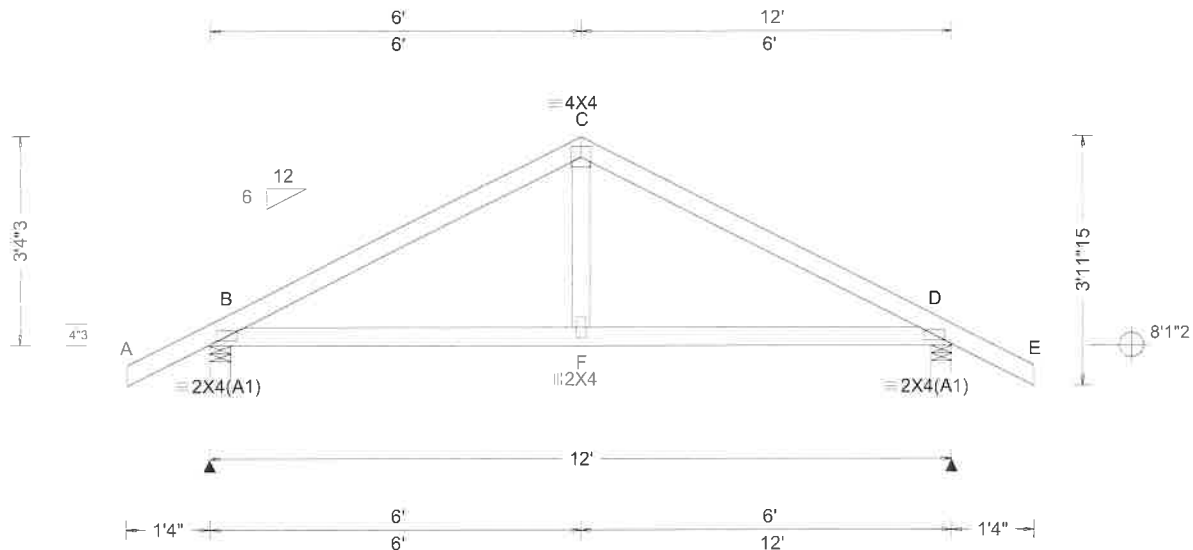
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg.Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpt: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.009 F 999 240 VERT(CL): 0.019 F 999 180 HORZ(LL): 0.004 D - - HORZ(TL): 0.009 D - - Creep Factor: 2.0 Max TC CSI: 0.322 Max BC CSI: 0.353 Max Web CSI: 0.100 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 583 /- /- /337 /26 /75 D 583 /- /- /337 /26 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 222 -654 C - D 223 -654

Lumber

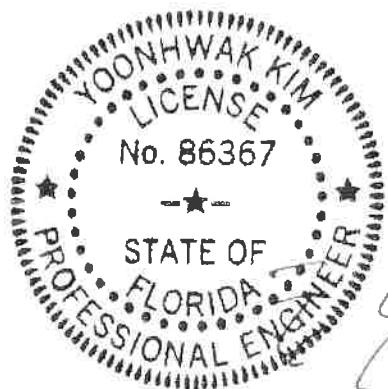
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-4-3.



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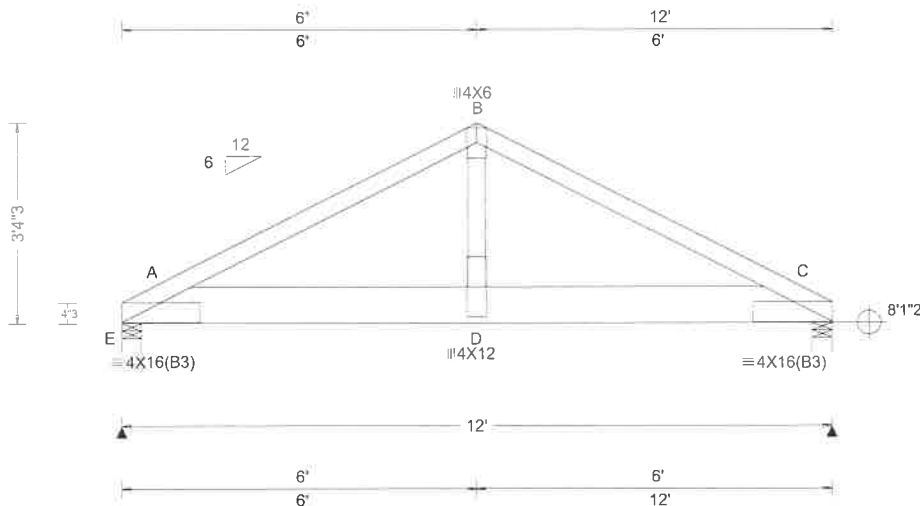
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbaccomponents.com; ICC: iccsafe.org; AWC: awc.org



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Glenview, IL 60025

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.066 D 999 240 VERT(CL): 0.132 D 999 180 HORZ(LL): 0.015 A - - HORZ(TL): 0.029 A - - Creep Factor: 2.0 Max TC CSI: 0.680 Max BC CSI: 0.529 Max Web CSI: 0.684 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 4789 -/- /- /173 -/ C 4883 -/- /- /176 -/ Wind reactions based on MWFRS E Brg Wid = 4.0 Min Req = 2.0 (Truss) C Brg Wid = 4.0 Min Req = 2.0 (Truss) Bearings E & C are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 134 -3665 B - C 134 -3664

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x8 SP 2400f-2.0E;
Webs: 2x4 SP #2;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at 0.00 to 62 plf at 12.00
BC: From 10 plf at 0.00 to 10 plf at 12.00
BC: 1761 lb Conc. Load at 2.06, 4.06, 6.06, 8.06
10.06

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 34'-3".

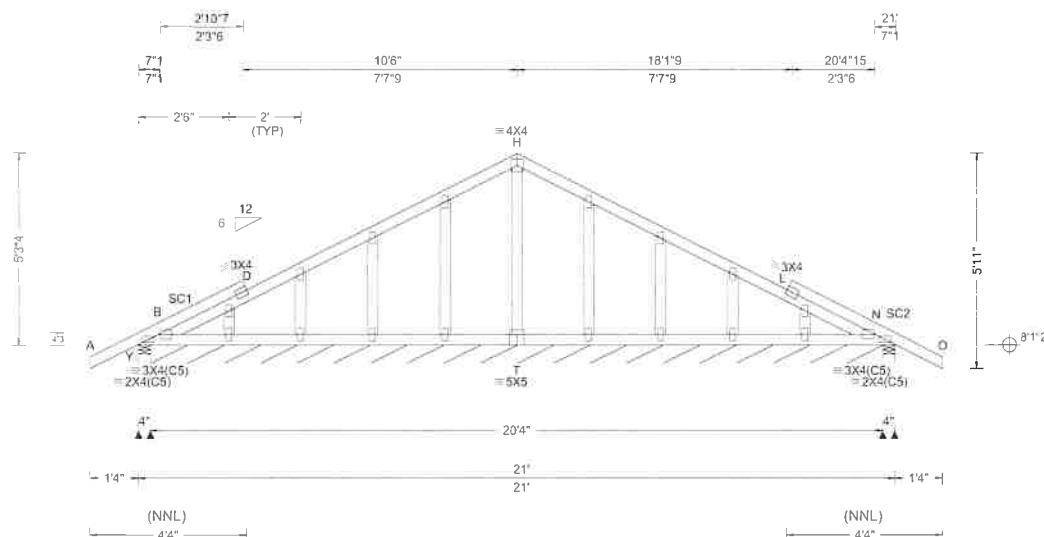


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or * = PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 L 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 L 999 180	Y 247 /- /- /132 /11 /113
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.001 L - -	B* 70 /- /- /137 /3 /-
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.002 L - -	N 247 /- /- /147 /10 /-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.196	Y Brg Wid = 4.0 Min Req = 1.5
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.028	B Brg Wid = 244 Min Req = -
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.052	N Brg Wid = 4.0 Min Req = 1.5
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Bearings Y, B, & N are a rigid surface.
	Loc. from endwall: Any	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 21.02.01.1214.12	

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

All plates are 2X4 except as noted

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

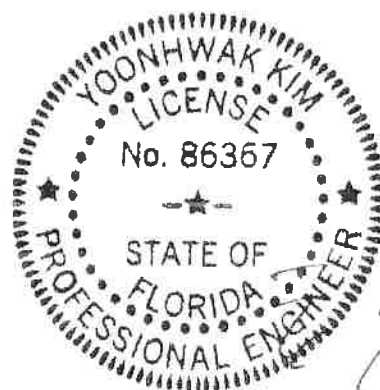
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

See DWGS A14015ENC160118 & GBLLETIN0118 for
gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 5-3-4



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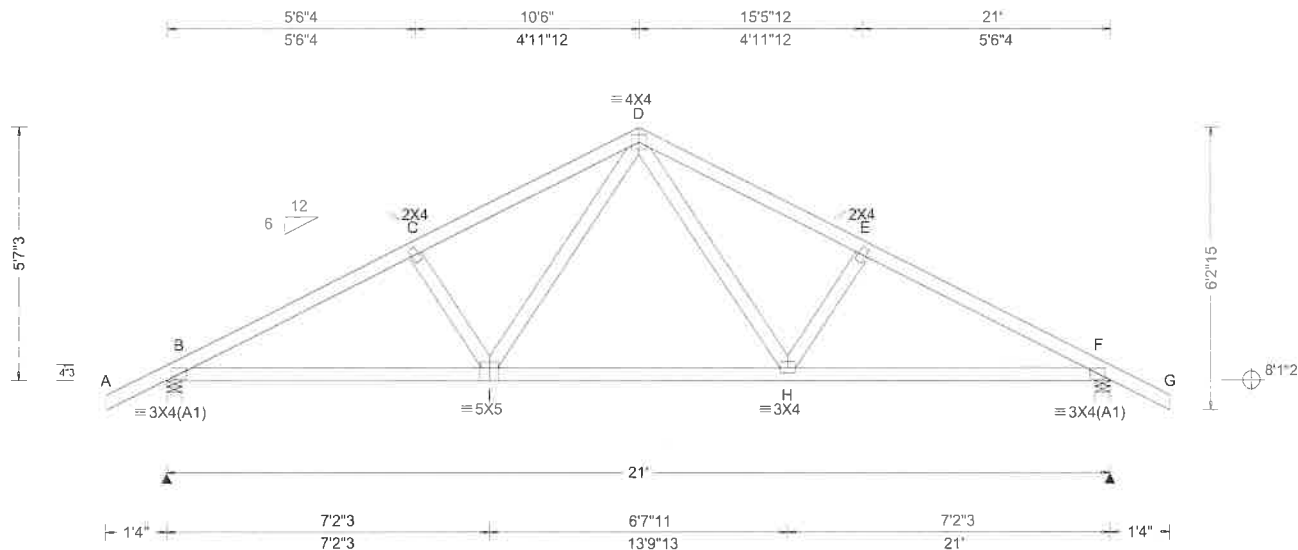
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)										
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc	L/defl	L/#	Gravity			Non-Gravity				
TCDL:	10.00	Speed:	130 mph	Pf: NA	Ce: NA		VERT(LL):	0.045	H	999	240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.090	H	999	180	B	954	/-	/-	/542	/39	/114
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.018	F	-	-	F	954	/-	/-	/542	/39	/-
Des Ld:	40.00	EXP:	B Kzt: NA				HORZ(TL):	0.036	F	-	-	Wind reactions based on MWFRS						
NCBCLL:	10.00	Mean Height:	15.00 ft	Building Code:			Creep Factor:	2.0				B	Brg Wid	= 4.0	Min Req	= 1.5		
Soffit:	2.00	TCDL:	5.0 psf	FBC 7th Ed.	2020 Res.		Max TC CSI:	0.260				F	Brg Wid	= 4.0	Min Req	= 1.5		
Load Duration:	1.25	BCDL:	5.0 psf	TPI Std:	2014		Max BC CSI:	0.509				Bearings B & F are a rigid surface.						
Spacing:	24.0 "	MWFRS Parallel Dist:	0 to h/2	Rep Fac:	Yes		Max Web CSI:	0.173				Members not listed have forces less than 375#						
		C&C Dist a:	3.00 ft	FT/RT:	20(0)/10(0)							Maximum Top Chord Forces Per Ply (lbs)						
		Loc. from endwall:	Any	Plate Type(s):								Chords	Tens.Comp.		Chords	Tens. Comp.		
		GCpi:	0.18	WAVE								B - C	315 - 1440		D - E	326 - 1275		
		Wind Duration:	1.60															

Lumber

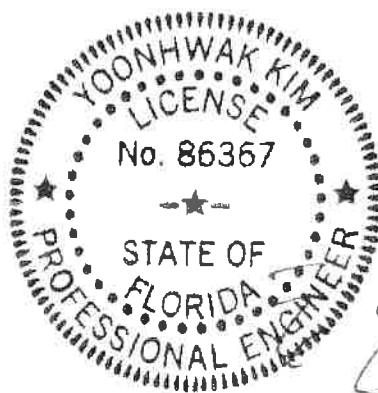
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 5-7-3.

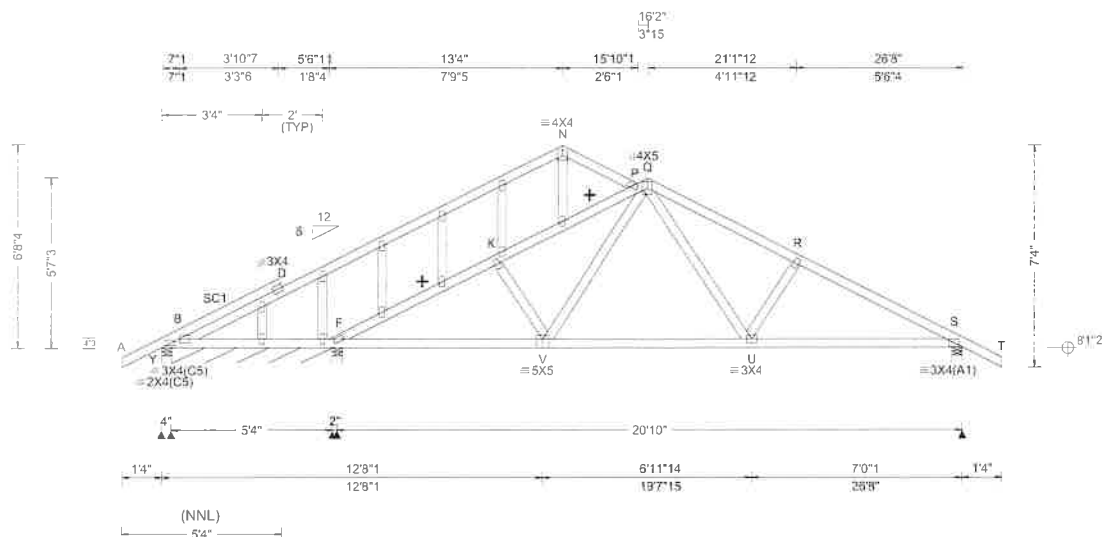


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Florida State Seal of Product Approval #FL 1999

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *=PLF
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.043 U 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.088 U 999 180	Y 361 -/- /- /192 -/- /133
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.019 S - -	B* 44 -/- /- /17 /9 -/-
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.038 S - -	F 812 -/- /- /502 -/- -/-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	S 971 -/- /- /565 /29 -/-
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.268	Wind reactions based on MWFRS
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.518	Y Brg Wid = 4.0 Min Req = 1.5 (Truss)
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.181	B Brg Wid = 64.0 Min Req = -
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)		F Brg Wid = 4.0 Min Req = 1.5 (Truss)
	Loc. from endwall: not in 4.50 ft	Plate Type(s):		S Brg Wid = 4.0 Min Req = 1.5 (Truss)
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1214.12	Bearings Y, B, F, & S are a rigid surface.
	Wind Duration: 1.60			Members not listed have forces less than 375#

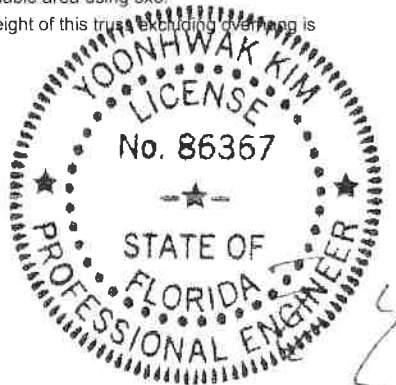
Lumber	Additional Notes	Maximum Top Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2;	See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.	Chords Tens.Comp. Chords Tens. Comp.
Bot chord: 2x4 SP #2;	Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.	F - K 195 - 1147 Q - R 159 - 1331
Webs: 2x4 SP #3;	The overall height of this truss excluding overhang is 6-8-4.	K - P 169 - 1067 R - S 139 - 1484
Stack Chord: SC1 2x4 SP #2;		P - Q 143 - 1088

Plating Notes	Maximum Bot Chord Forces Per Ply (lbs)
All plates are 2X4 except as noted.	Chords Tens.Comp. Chords Tens. Comp.
	F - V 1154 -24 U - S 1268 -58
	V - U 862 0

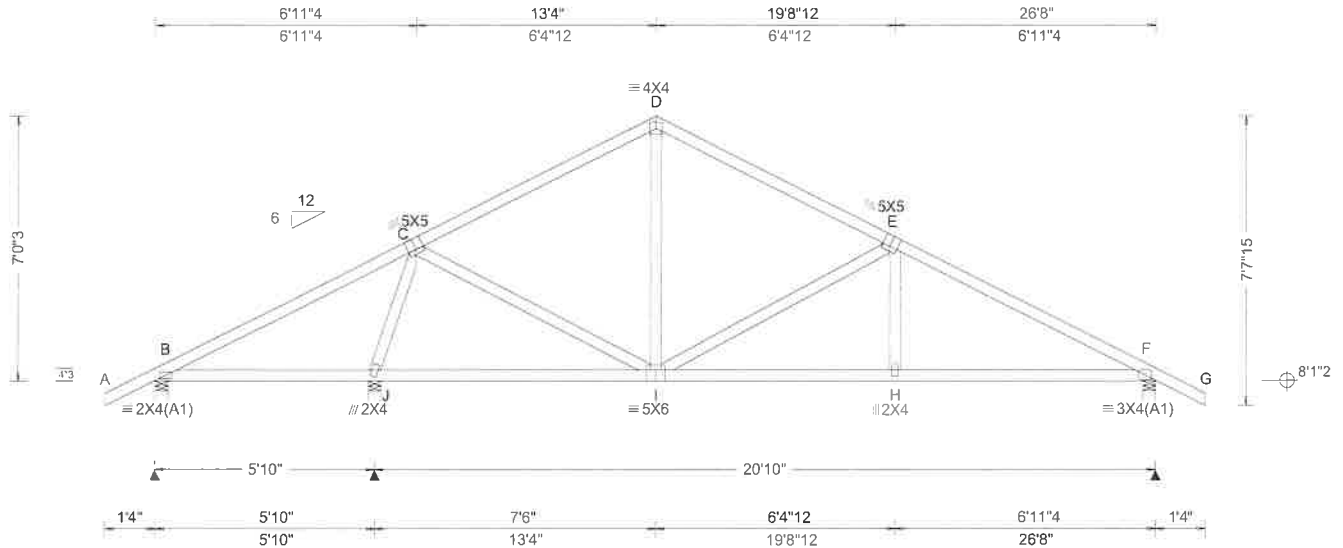
Loading	Maximum Web Forces Per Ply (lbs)
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.	Webs Tens.Comp.
	Q - U 476 -32

Wind	Blocking
Wind loads based on MWFRS with additional C&C member design.	Blocking reinforcement required to prevent buckling of members over the bearings:
Wind loading based on both gable and hip roof types.	Bearing 3 located at 5.7' (blocking >= 3.50" if used)

Blocking	Member to be laterally braced for horizontal wind loads.
Blocking reinforcement required to prevent buckling of members over the bearings:	Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lur: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed, 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.034 H 999 240 VERT(CL): 0.068 H 999 180 HORZ(LL): 0.011 F - - HORZ(TL): 0.022 F - - Creep Factor: 2.0 Max TC CSI: 0.543 Max BC CSI: 0.483 Max Web CSI: 0.616 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 320 - / - / - /134 /35 /139 J 1202 - / - / - /685 /14 /- F 925 - / - / - /545 /43 /- Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) J Brg Wid = 4.0 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings B, J, & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

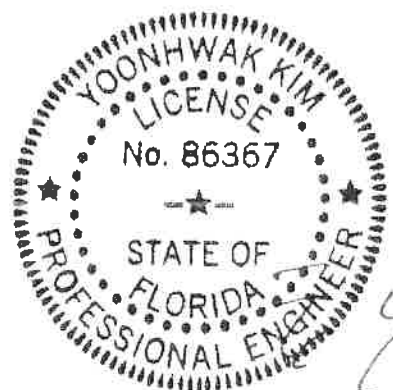
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 7'-0-3".

THIS TRUSS MUST BE INSTALLED AS SHOWN AND NOT END FOR END.

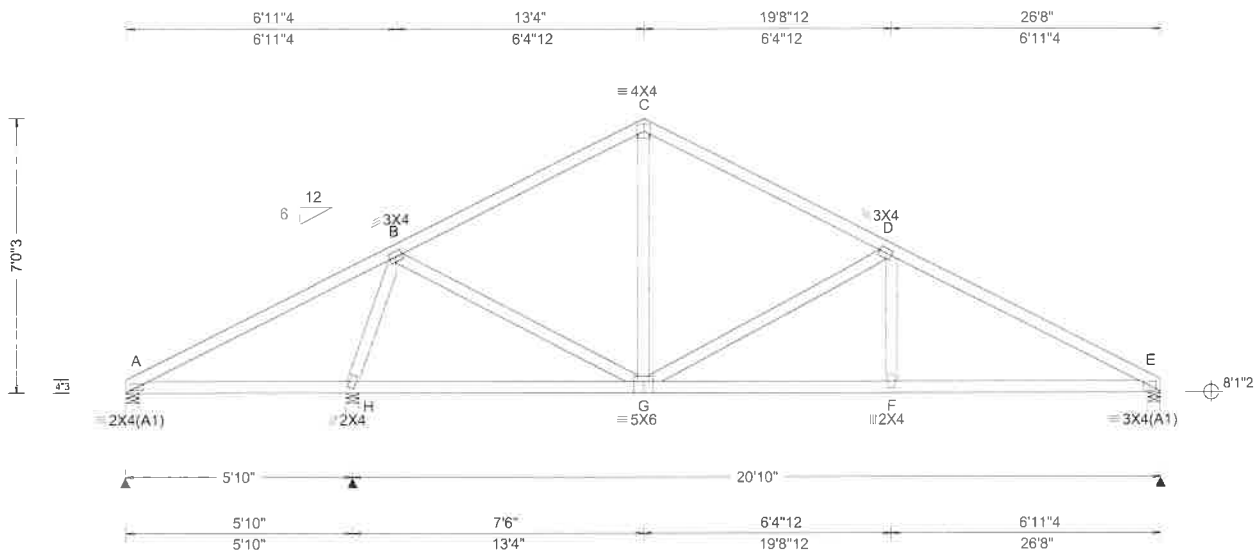


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Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
I - H	1113 -40	H - F	1116 -38

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
J - C	137 -1071	I - E	103 -610
C - I	547 0		

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpt: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.034 F 999 240 VERT(CL): 0.067 F 999 180 HORZ(LL): 0.011 E - - HORZ(TL): 0.024 E - - Creep Factor: 2.0 Max TC CSI: 0.570 Max BC CSI: 0.505 Max Web CSI: 0.642 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 235 /-6 /- /80 /21 /116 H 1199 /- /- /678 /18 /- E 840 /- /- /484 /32 /- Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = 4.0 Min Req = 1.5 (Truss) E Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A, H, & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

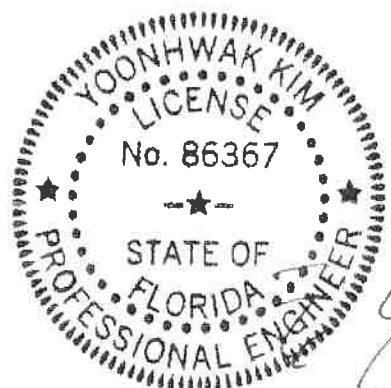
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 7'-0-3".

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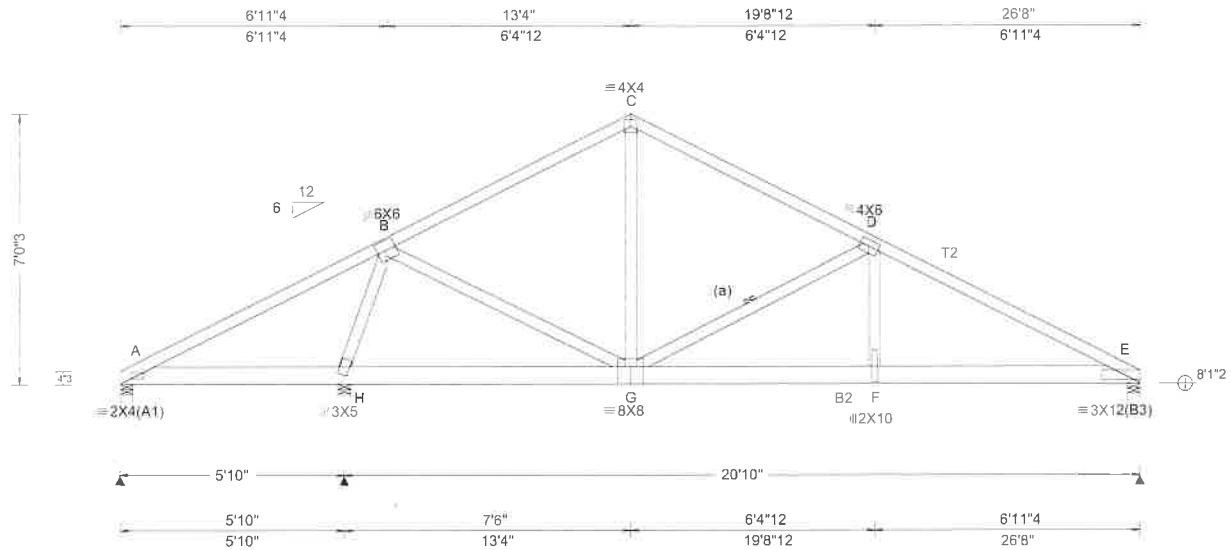
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.091 F 999 240 VERT(CL): 0.178 F 999 180 HORZ(LL): 0.020 C - - HORZ(TL): 0.038 C - - Creep Factor: 2.0 Max TC CSI: 0.695 Max BC CSI: 0.881 Max Web CSI: 0.768 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 77 /-336 /- /3 /- /- H 3593 /- /- /- /140 /- E 2494 /- /- /- /112 /- Wind reactions based on MWFRS A Brg Wid = 4.0 Min Req = 1.5 (Truss) H Brg Wid = 4.0 Min Req = 3.9 (Truss) E Brg Wid = 4.0 Min Req = 2.1 (Truss) Bearings A, H, & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x6 SP #2; B2 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 62 plf at 0.00 to 62 plf at 26.67
BC: From 20 plf at 0.00 to 20 plf at 6.60
BC: From 10 plf at 6.60 to 10 plf at 26.67
BC: 372 lb Conc. Load at 6.60, 8.60, 10.60, 12.60
14.60
BC: 429 lb Conc. Load at 16.60
BC: 397 lb Conc. Load at 18.60, 20.60, 22.60, 24.60

Wind

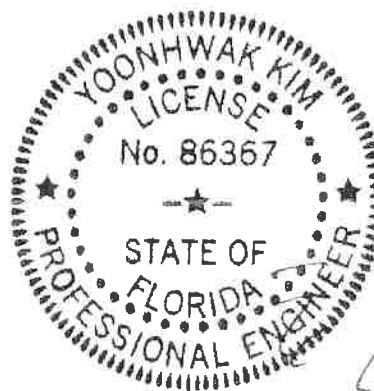
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

Negative reaction(s) of -336# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

The overall height of this truss excluding overhang is 7'-0-3.

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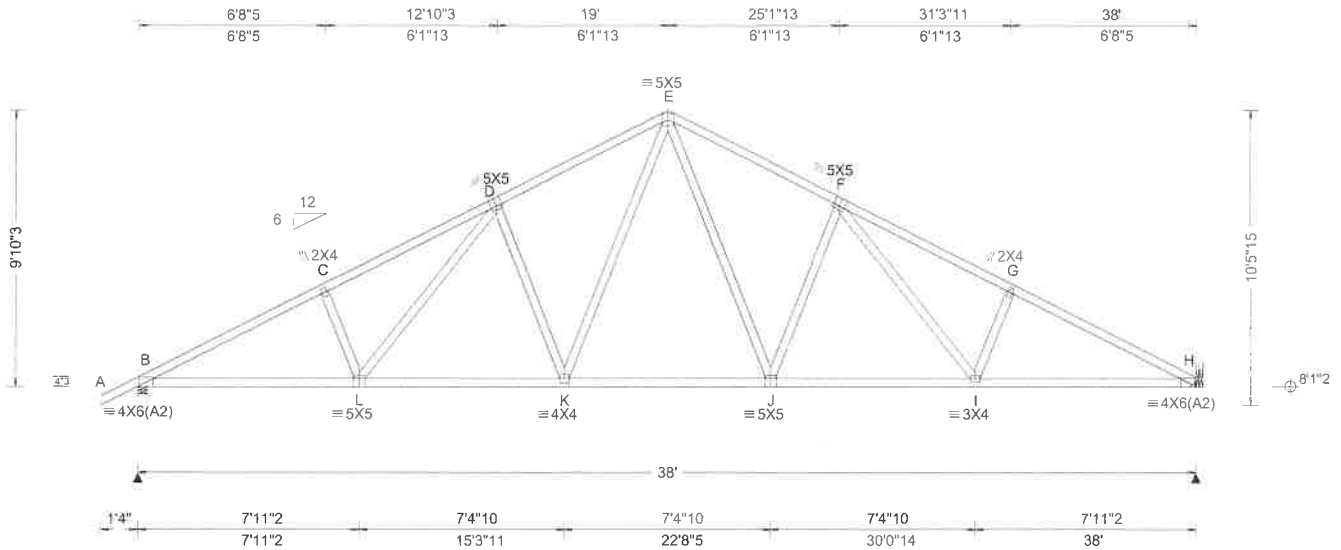
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Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.212 K 999 240 VERT(CL): 0.390 K 999 180 HORZ(LL): 0.083 H - - HORZ(TL): 0.152 H - - Creep Factor: 2.0 Max TC CSI: 0.589 Max BC CSI: 0.964 Max Web CSI: 0.590 VIEW Ver: 21.02.01.1214.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1856 - / - / - / 944 / 63 / 187 H 1761 - / - / - / 882 / 51 / - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 2.2 (Truss) H Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 576 - 3338 E - F 575 - 2538 C - D 624 - 3198 F - G 635 - 3226 D - E 574 - 2538 G - H 586 - 3365

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=37'9" uses the following support conditions: 37'9"

Bearing H (37'9", 8'1"2) HUS26
Supporting Member: (2)2x8 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Loading

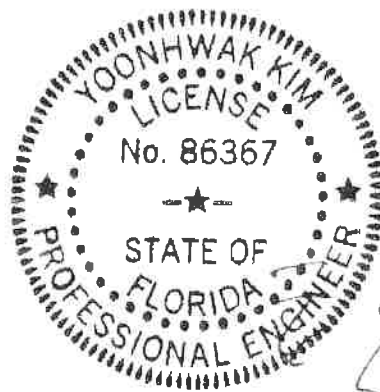
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-10-3/4".



FL REG# 278, Yoonhwak Kim, FL PE #86367
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Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp.

B - L 2906 - 455 J - I 2424 - 310
L - K 2420 - 326 I - H 2935 - 452
K - J 1842 - 170

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp. Webs Tens. Comp.

L - D 608 - 114 E - J 970 - 177
D - K 254 - 624 J - F 256 - 635
K - E 968 - 175 F - I 639 - 124

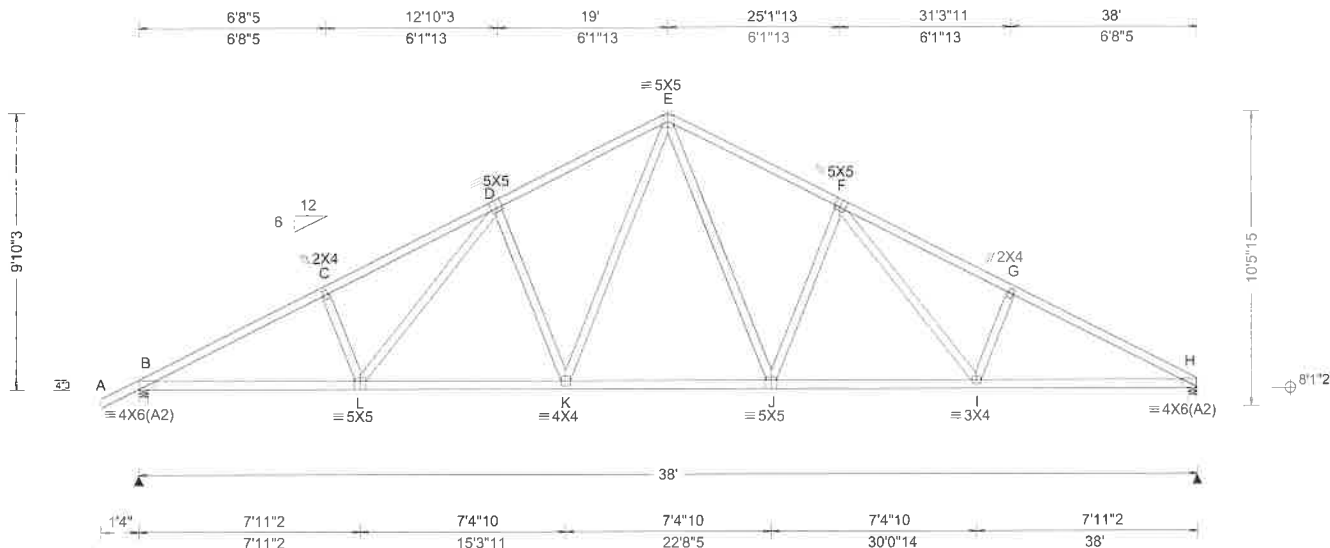
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)			Defl/CSI Criteria			▲ Maximum Reactions (lbs)								
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in	loc	L/defl	L/#	Gravity			Non-Gravity				
TCDL:	10.00	Speed:	130 mph	Pf: NA		Ce: NA	VERT(LL):	0.212	K	999	240	Loc	R+	R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL):	0.389	K	999	180	B	1855	/-	/-	/944	/-	/187
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL):	0.083	H	-	-	H	1762	/-	/-	/883	/-	/-
		EXP:	B Kzt: NA				HORZ(TL):	0.152	H	-	-	Wind reactions based on MWFRS						
Des Ld:	40.00	Mean Height:	15.00 ft				Creep Factor:	2.0				B	Brg Wid = 4.0 Min Req = 2.2 (Truss)					
NCBCLL:	10.00	TCDL:	5.0 psf			Building Code:						H	Brg Wid = 4.0 Min Req = 2.1 (Truss)					
Soffit:	2.00	BCDL:	5.0 psf			FBC 7th Ed. 2020 Res.						Bearings B & H are a rigid surface.						
Load Duration:	1.25	MWFRS Parallel Dist:	h to 2h			TPI Std:	2014					Members not listed have forces less than 375#						
Spacing:	24.0 "	C&C Dist a:	3.80 ft			Rep Fac:	Yes					Maximum Top Chord Forces Per Ply (lbs)						
		Loc. from endwall:	not in 9.00 ft			FT/RT:	20(0)/10(0)					Chords	Tens.	Comp.	Chords	Tens.	Comp.	
		GCpi:	0.18			Plate Type(s):						B - C	196	-3336	E - F	256	-2536	
		Wind Duration:	1.60			WAVE						C - D	237	-3196	F - G	248	-3215	
												D - E	254	-2536	G - H	206	-3354	
												E - F	237	-3196	H - I	248	-3215	
												F - G	254	-2536	I - J	206	-3354	
												G - H	237	-3196	J - K	248	-3215	
												H - I	254	-2536	K - L	206	-3354	
												I - J	237	-3196	L - M	248	-3215	
												J - K	254	-2536	M - N	206	-3354	
												K - L	237	-3196	N - O	248	-3215	
												L - M	254	-2536	O - P	206	-3354	
												M - N	237	-3196	P - Q	248	-3215	
												N - O	254	-2536	Q - R	206	-3354	
												O - P	237	-3196	R - S	248	-3215	
												P - Q	254	-2536	S - T	206	-3354	
												Q - R	237	-3196	T - U	248	-3215	
												R - S	254	-2536	U - V	206	-3354	
												S - T	237	-3196	V - W	248	-3215	
												T - U	254	-2536	W - X	206	-3354	
												U - V	237	-3196	X - Y	248	-3215	
												V - W	254	-2536	Y - Z	206	-3354	
												W - X	237	-3196	Z - AA	248	-3215	
												X - Y	254	-2536	AA - AB	206	-3354	
												Y - Z	237	-3196	AB - AC	248	-3215	
												Z - AA	254	-2536	AC - AD	206	-3354	
												AA - AB	237	-3196	AD - AE	248	-3215	
												AB - AC	254	-2536	AE - AF	206	-3354	
												AC - AD	237	-3196	AF - AG	248	-3215	
												AD - AE	254	-2536	AG - AH	206	-3354	
												AE - AF	237	-3196	AH - AI	248	-3215	
												AF - AG	254	-2536	AI - AJ	206	-3354	
												AG - AH	237	-3196	AJ - AK	248	-3215	
												AH - AI	254	-2536	AK - AL	206	-3354	
												AI - AJ	237	-3196	AL - AM	248	-3215	
												AJ - AK	254	-2536	AM - AN	206	-3354	
												AK - AL	237	-3196	AN - AO	248	-3215	
												AL - AM	254	-2536	AO - AP	206	-3354	
												AM - AN	237	-3196	AP - AQ	248	-3215	
												AN - AO	254	-2536	AQ - AR	206	-3354	
												AO - AP	237	-3196	AR - AS	248	-3215	
												AP - AQ	254	-2536	AS - AT	206	-3354	
												AQ - AR	237	-3196	AT - AU	248	-3215	
												AR - AS	254	-2536	AU - AV	206	-3354	
												AS - AT	237	-3196	AV - AW	248	-3215	
												AT - AU	254	-2536	AW - AX	206	-3354	
												AU - AV	237	-3196	AX - AY	248	-3215	
												AV - AW	254	-2536	AY - AZ	206	-3354	
												AW - AX	237	-3196	AZ - BA	248	-3215	
												AX - AY	254	-2536	BA - BB	206	-3354	
												AY - AZ	237	-3196	BB - BC	248	-3215	
												AZ - BA	254	-2536	BC - BD	206	-3354	
												BA - BB	237	-3196	BD - BE	248	-3215	
												BB - BC	254	-2536	BE - BF	206	-3354	
												BC - BD	237	-3196	BF - BG	248	-3215	
												BD - BE	254	-2536	BG - BH	206	-3354	
												BE - BF	237	-3196	BH - BI	248	-3215	
												BF - BG	254	-2536	BI - BJ	206	-3354	
												BG - BH	237	-3196	BJ - BK	248	-3215	
												BH - BI	254	-2536	BK - BL	206	-3354	
												BI - BJ	237	-3196	BL - BM	248	-3215	
												BJ - BK	254	-2536	BM - BN	206	-3354	
												BK - BL	237	-3196	BN - BO	248	-3215	
												BL - BM	254	-2536	BO - BP	206	-3354	
												BM - BN	237	-3196	BP - BQ	248	-3215	
												BN - BO	254	-2536	BQ - BR	206	-3354	
												BO - BP	237	-3196	BR - BS	248	-3215	
												BP - BQ	254	-2536	BS - BT	206	-3354	
												BQ - BR	237	-3196	BT - BU	248	-3215	
												BR - BS	254	-2536	BU - BV	206	-3354	
												BS - BT	237	-3196	BV - BW	248	-3215	
												BT - BU	254	-2536	BW - BX	206	-3354	
												BU - BV	237	-3196	BX - BY	248	-3215	
												BV - BW	254	-2536	BY - BZ	206	-3354	
												BW - BX	237	-3196	BZ - CA	248	-3215	
												BX - BY	254	-2536	CA - CB	206	-3354	
												BY - BZ	237	-3196	CB - CC	248	-3215	
												BZ - CA	254	-2536	CC - CD	206	-3354	
												CA - CB	237	-3196	CD - CE	248	-3215	
												CB - CC	254	-2536	CE - CF	206	-3354	
												CC - CD	237	-3196	CF - CG	248	-3215	
												CD - CE	254	-2536	CG - CH	206	-3354	
												CE - CF	237	-3196	CH - CI	248	-3215	
												CF - CG	254	-2536	CI - CJ	206	-3354	
												CG - CH	237	-3196	CJ - CK	248	-3215	
												CH - CI	254	-2536	CK - CL	206	-3354	
												CI - CJ	237	-3196	CL - CM	248	-3215	
												CJ - CK	254	-2536	CM - CN	206	-3354	
												CK - CL	237	-3196	CN - CO	248	-3215	
												CL - CM	254	-2536	CO - CP	206	-3354	
												CM - CN	237	-3196	CP - CQ	248	-3215	
												CN - CO	254	-2536	CQ - CR	206	-3354	
												CO - CP	237	-3196	CR - CS	248	-3215	
												CP - CQ	254	-2536	CS - CT	206	-3354	
												CQ - CR	237	-3196	CT - CU	248	-3215	
												CR - CS	254	-2536	CU - CV	206	-3354	
												CS - CT	237	-3196	CV - CW	248	-3215	
												CT - CU	254	-2536	CW - CX	206	-3354	
												CU - CV	237	-3196	CX - CY	248	-3215	
												CV - CW	254	-2536	CY - CZ	206	-3354	
												CW - CX	237	-3196	CZ - DA	248	-3215	
												CX - CY	254	-2536	DA - DB	206	-3354	
												CY - CZ	237	-3196	DB - DC	248	-3215	
												CZ - DA	254	-2536	DC - DD	206	-3354	
												DA - DB	237	-3196	DD - DE	248	-3215	
												DB - DC	254	-2536	DE - DF	206	-3354	
												DC - DD	237	-3196	DF - DG	248	-3215	
												DD - DE	254	-2536	DG - DH	206	-3354	
												DE - DF	237	-3196	DH - DI	248	-3215	
												DF - DG	254	-2536</				

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

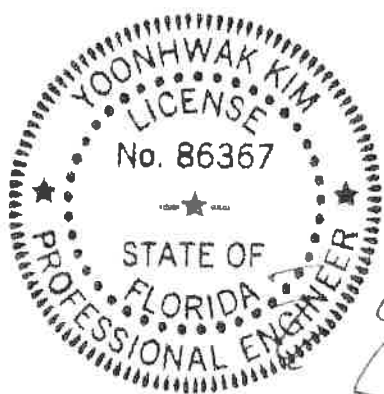
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

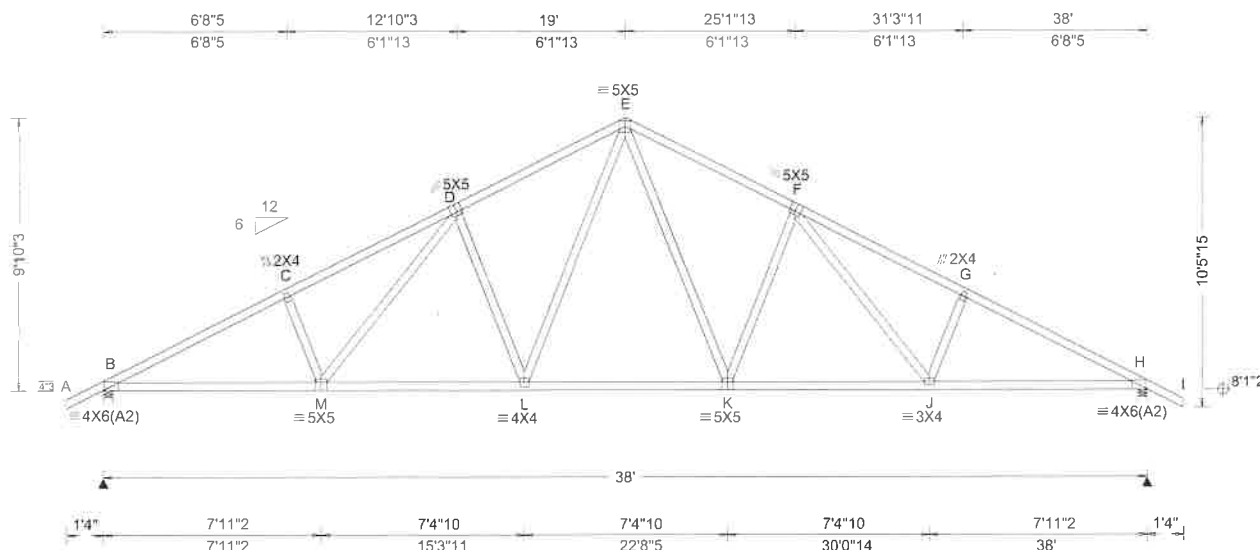
The overall height of this truss excluding overhang is 9'-10-3/4".



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Florida State Seal of Product Approval #FL 1999

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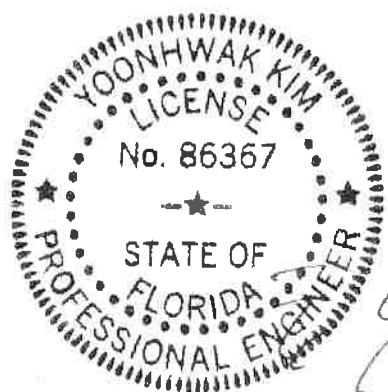
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▲ Maximum Reactions (lbs)						
Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1853	/-	/-	/944	/-	/195
H	1853	/-	/-	/944	/-	/-
Wind reactions based on MWFRS						
B	Brg Wid = 4.0		Min Req = 2.2 (Truss)			
H	Brg Wid = 4.0		Min Req = 2.2 (Truss)			
Bearings B & H are a rigid surface.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Chords		Tens.Comp.		Chords		Tens. Comp.
B - C		195 - 3332		E - F		253 - 2530
C - D		236 - 3192		F - G		236 - 3194
D - E		253 - 2531		G - H		195 - 3334

The overall height of this truss excluding overhang is 9-10-3.

Maximum Web Forces Per Ply (lbs)					
Webs			Tens. Comp.		
M - D	608	-33	E - K	964	-61
D - L	140	-624	K - F	139	-626
L - E	968	-60	F - J	611	-32



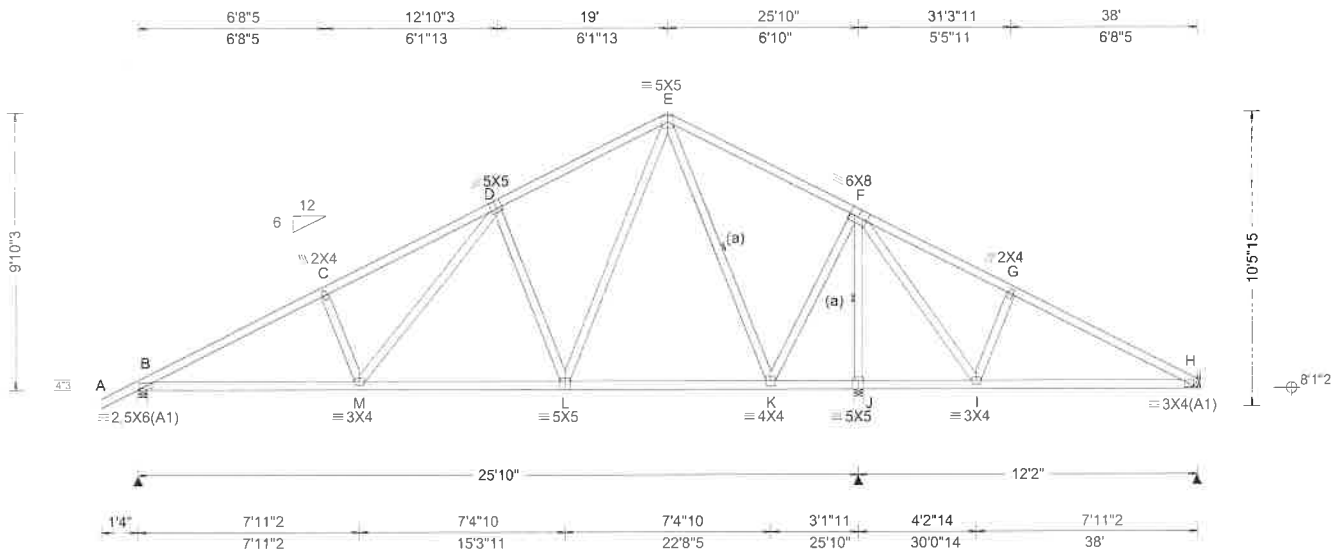
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Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.074 M 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.140 M 999 180	B 1185 /- /- /645 /41 /187
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.024 C - -	J 2021 /- /- /946 /69 /-
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.045 C - -	H 372 /- /- /238 /8 /-
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	Wind reactions based on MWFRS
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed, 2020 Res.	Max TC CSI: 0.488	B Brg Wid = 4.0 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.691	J Brg Wid = 4.0 Min Req = 2.4
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.588	H Brg Wid = - Min Req = -
	C&C Dist a: 3.80 ft	FT/RT:20(0)/10(0)		Bearings B & J are a rigid surface.
	Loc. from endwall: not in 9.00 ft	Plate Type(s):		Members not listed have forces less than 375#
	GCpi: 0.18	WAVE	VIEW Ver: 21.02.01.1214.12	Maximum Top Chord Forces Per Ply (lbs)
	Wind Duration: 1.60			Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

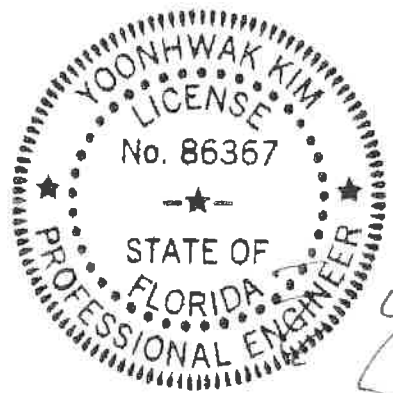
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9'-10.3\"/>



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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=37'9" uses the following support conditions: 37'9"

Bearing H (37'9", 8'1"2) LUS26

Supporting Member: (1)2x6 SP #2

(4) 0.148"x3" nails into supporting

member,

(3) 0.148"x3" nails into supported

member.

Bearing H (37'9", 8'1"2) LUS26

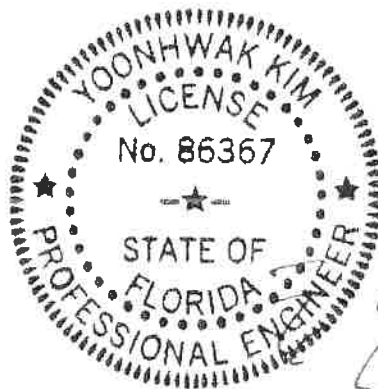
Supporting Member: (1)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting

member,

(3) 0.148"x3" nails into supported

member.



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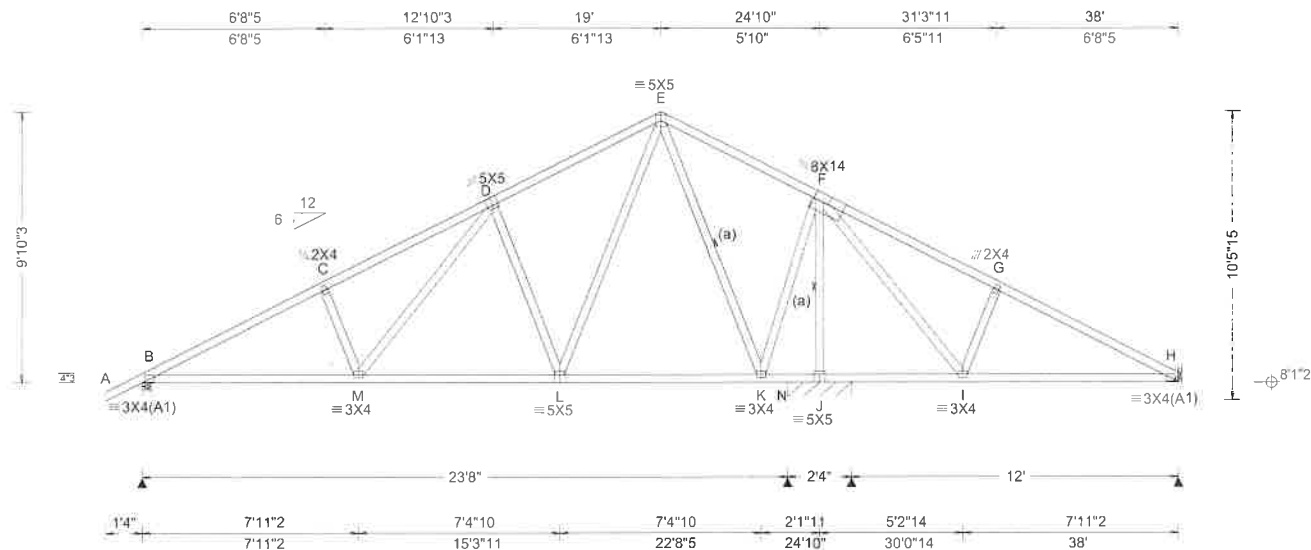
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Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg. Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs), or *PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.80 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.054 M 999 240 VERT(CL): 0.112 M 999 180 HORZ(LL): 0.018 C - - HORZ(TL): 0.037 C - - Creep Factor: 2.0 Max TC CSI: 0.523 Max BC CSI: 0.618 Max Web CSI: 0.581 VIEW Ver: 21.02.01.1214.12	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1037 - / - / - / 615 / 35 / 187 N* 781 - / - / - / 408 / 32 / - H 429 - / - / - / 262 / 3 / - Wind reactions based on MWFRS B Brg Wid = 4.0 Min Req = 1.5 (Truss) N Brg Wid = 28.0 Min Req = - H Brg Wid = - Min Req = - Bearings B & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=37'9" uses the following support conditions: 37'9"

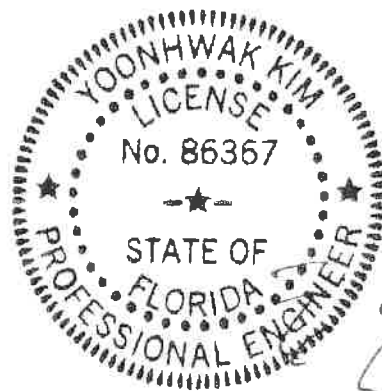
Bearing H (37'9", 8'1"2) LUS26
Supporting Member: (1)2x6 SP 2400f-2.0E
(4) 0.148"x3" nails into supporting member,
(3) 0.148"x3" nails into supported member.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 9-10-3.



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Maximum Bot Chord Forces Per Ply (lbs)	Maximum Web Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.	Webs Tens.Comp. Webs Tens. Comp.
B - C 99 - 1570 C - D 141 - 1433	M - D 618 - 34 D - L 141 - 625 L - E 792 - 62 E - K 70 - 906
D - E 156 - 763 G - H 59 - 485	K - F 838 0 J - F 119 - 1627 F - I 692 - 71 I - G 126 - 411

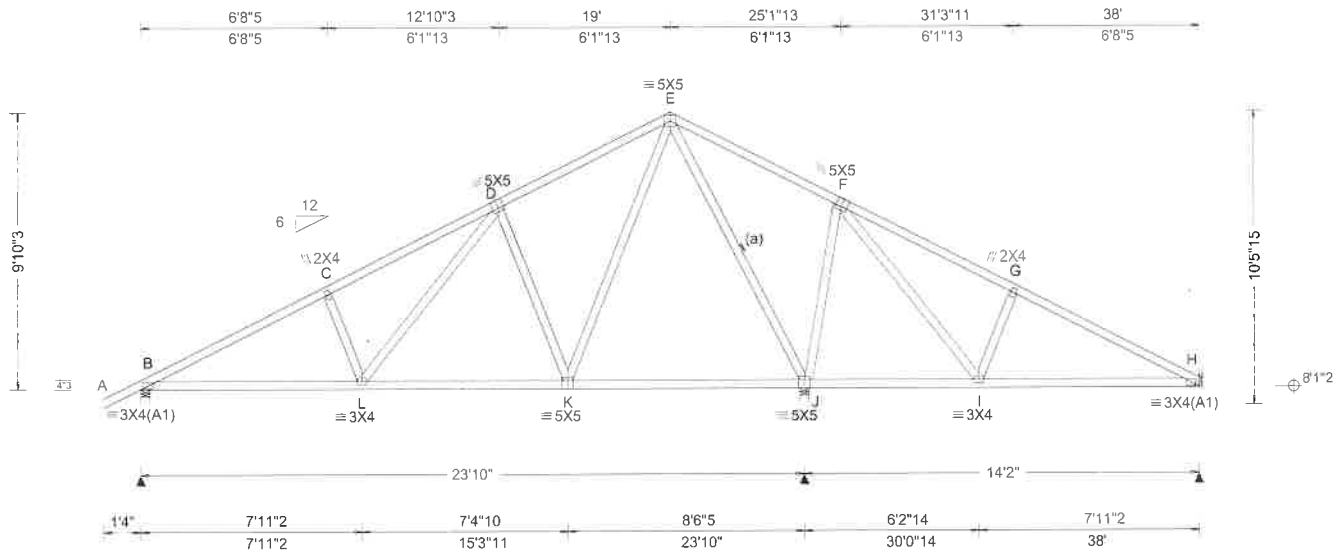
WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity			Non-Gravity			
TCDL:	10.00	Speed:	130 mph	Pf: NA	Ce: NA		VERT(LL): 0.059 L 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL): 0.111 L 999 180	B	1038	/-	/-	/569	/34	/187
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL): 0.019 C - -	J	2291	/-	/-	/1008	/78	/-
Des Ld:	40.00	EXP: B	Kzt: NA				HORZ(TL): 0.036 C - -	H	397	/-	/-	/249	/7	/-
NCBCLL:	10.00	Mean Height:	15.00 ft	Building Code:			Creep Factor: 2.0	Wind reactions based on MWFRS						
Soffit:	2.00	TCDL:	5.0 psf	FBC 7th Ed. 2020 Res.			Max TC CSI: 0.602	B	Brg Wid = 4.0	Min Req = 1.5 (Truss)				
Load Duration:	1.25	BCDL:	5.0 psf	TPI Std: 2014			Max BC CSI: 0.712	J	Brg Wid = 4.0	Min Req = 2.7				
Spacing:	24.0 "	MWFRS Parallel Dist:	0 to h/2	Rep Fac: Yes			Max Web CSI: 0.861	H	Brg Wid = -	Min Req = -				
		C&C Dist a:	3.80 ft	FT/RT:20(0)/10(0)				Bearings B & J are a rigid surface.						
		Loc. from endwall:	Any	Plate Type(s):				Members not listed have forces less than 375#						
		GCpi:	0.18					Maximum Top Chord Forces Per Ply (lbs)						
		Wind Duration:	1.60	WAVE			VIEW Ver: 21.02.01.1214.12	Chords	Tens.Comp.	Chords	Tens. Comp.			

Lumber		Loading		Maximum Bot Chord Forces Per Ply (lbs)	
Top chord:	2x4 SP #2;	Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.		Chords	Tens.Comp.
Bot chord:	2x4 SP #2;			Chords	Tens. Comp.
Webs:	2x4 SP #3;			B - L	1337 -173

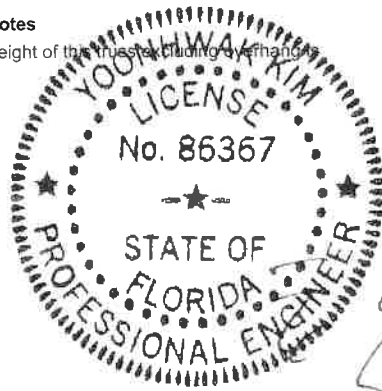
Bracing		Wind		Maximum Web Forces Per Ply (lbs)	
(a) Continuous lateral restraint equally spaced on member.		Wind loads based on MWFRS with additional C&C member design.		Chords	Tens. Comp.
		Wind loading based on both gable and hip roof types.		Chords	Tens. Comp.
				L - D	643 -118

Hangers / Ties		Additional Notes		Maximum Bot Chord Forces Per Ply (lbs)	
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.		The overall height of the truss including overhang is 9'-10".		Chords	Tens. Comp.
Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.				Chords	Tens. Comp.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.				J - F	298 -694

Bearing at location x=37'9"		Bearing H (37'9", 8'1"2) LUS26		Maximum Bot Chord Forces Per Ply (lbs)	
uses the following support conditions: 37'9"		Supporting Member: (1)2x6 SP 2400f-2.0E		Chords	Tens. Comp.
		(4) 0.148"x3" nails into supporting member,		Chords	Tens. Comp.
		(3) 0.148"x3" nails into supported member.		L - K	825 -41

Bearing at location x=37'9"		Bearing H (37'9", 8'1"2) LUS26		Maximum Bot Chord Forces Per Ply (lbs)	
uses the following support conditions: 37'9"		Supporting Member: (1)2x6 SP 2400f-2.0E		Chords	Tens. Comp.
		(4) 0.148"x3" nails into supporting member,		Chords	Tens. Comp.
		(3) 0.148"x3" nails into supported member.		L - K	825 -41

Bearing at location x=37'9"		Bearing H (37'9", 8'1"2) LUS26		Maximum Bot Chord Forces Per Ply (lbs)	
uses the following support conditions: 37'9"		Supporting Member: (1)2x6 SP 2400f-2.0E		Chords	Tens. Comp.
		(4) 0.148"x3" nails into supporting member,		Chords	Tens. Comp.
		(3) 0.148"x3" nails into supported member.		L - K	825 -41



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Florida State Seal of Professional Engineer

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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

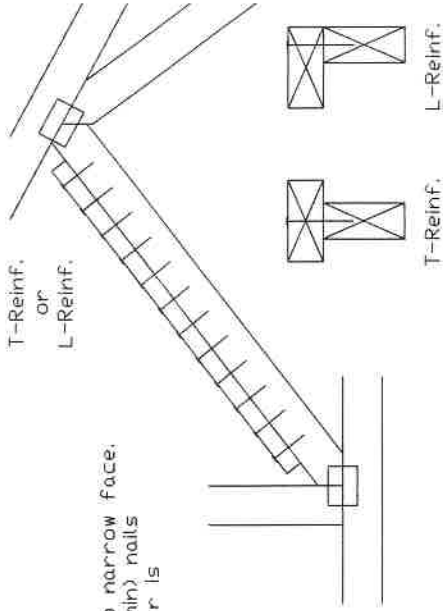
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

T-Reinforcement

or

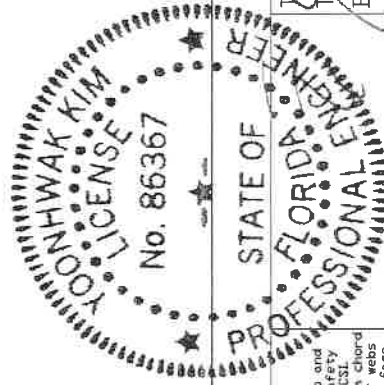
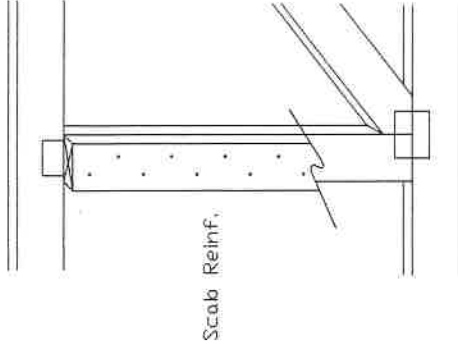
L-Reinforcement:



Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.

Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



DISCLAIMER: READ AND FOLLOW ALL NOTES ON THIS DRAWING THE INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information), by TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and positions as shown above and on the joint details, unless noted otherwise. Refer to drawings 10012 for standard plate positions.
Alpine, a division of TPI Building Components Group Inc., shall not be responsible for any deviation from this detail. Trusses shall be installed in accordance with ANSI/TPI 1, or for handling, shipping, installation, bracing of trusses.
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For more information see this job's general notes page and these web site: www.alpineinc.com, www.tpinstrg.org, www.sbcacomponents.com, ICC www.iccsafe.org
ALPINE: www.alpineinc.com TPI: www.tpinstrg.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

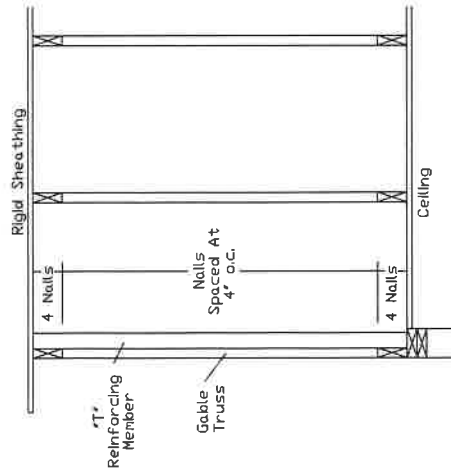
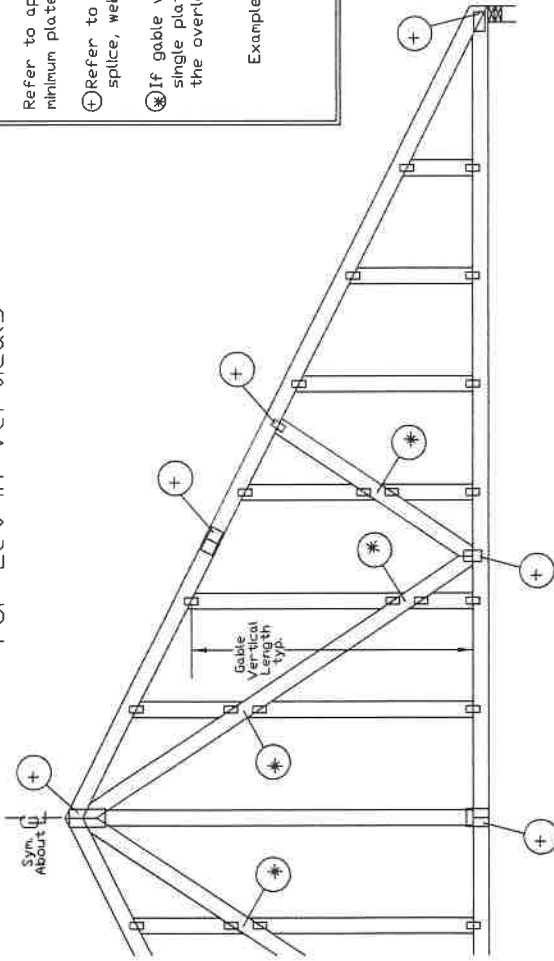
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TL DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

Yoonhwak Kim, FL PE #86367



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Detail For Let-in Verticals

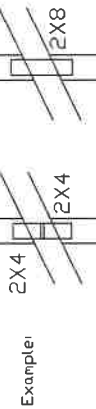


Gable Truss Plate Sizes

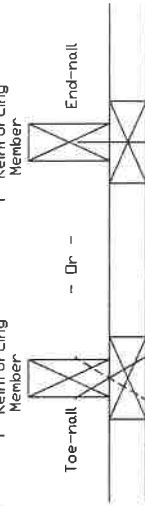
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf.	'T' Mbr. Size	Increase
2x4	2x6	30 %
2x6	2x8	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 100

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length

130 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3", min) Nails at 4' o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3", min) Toenails at 4' o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A10015051014, A14015051014,

A13030051014, A12030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A10015ENC100118,

A18015ENC100118, A20015ENC100118, A20015ENC100118, A20015ENC100118,

A11530ENC100118, A12030ENC100118, A14030ENC100118, A10030ENC100118,

A18030ENC100118, A20030ENC100118, A20030ENC100118, A20030ENC100118,

S11515ENC100118, S12015ENC100118, S14015ENC100118, S20015ENC100118,

S11530ENC100118, S12030ENC100118, S14030ENC100118, S20030ENC100118,

S18030ENC100118, S20030ENC100118, S20030ENC100118, S20030ENC100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

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For more information, see this job's general notes page and these web sites: ALPINE: www.alpineinc.com TPI: www.tpinet.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

REF LET-IN VERT
DATE 01/02/2018
DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0'



155 Harlem Ave
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Glenview, IL 60025

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