

GRACE COVENANT
LAKE CITY, FLORIDA
INDEX OF DRAWINGS

☐ PROGRESS PRINTS

☒ FOR APPROVAL

☐ FOR PRICING

☐ FOR PERMIT

☐ FOR CONSTRUCTION

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Donald A. Murphy
ARCHITECTURE/PLANNING/INTERIOR DESIGN
CONSTRUCTION MANAGEMENT AIA/ASA

SHEET NUMBER	TITLE	SHEET NUMBER	ARCHITECTURAL	ELECTRICAL	CODE CLASSIFICATION
T-1L	TITLE SHEET			E-1 ELECTRICAL LIGHTING PLAN E-2 ELECTRICAL LIGHTING PLAN E-3 ELECTRICAL POWER PLAN E-4 ELECTRICAL POWER PLAN E-5 MECHANICAL SYSTEMS POWER PLAN E-6 MECHANICAL SYSTEMS POWER PLAN E-7 OVERALL LIGHTING PLAN	NEW WORSHIP CENTER & EDUCATION SPACE 2004 FLORIDA BUILDING CODE ————— "A-3" (CHURCH) NFPA-101 OCCUPANCY GROUP — SMALL ASSEMBLY-I STORY CONSTRUCTION TYPE ————— TYPE V-B / V (000) UNSPRINKLERED
	CIVIL				PORTABLE FIRE EXTINGUISHERS SHALL BE PROVIDED IN ACCORDANCE WITH NFPA 10.
PF01-018	CIVIL COVER AND SITE LOCATION MAP				
1	GENERAL NOTES AND HANDICAP PARKING PLAN				
2	EXISTING CONDITIONS				
3	SITE PLAN				
4	EROSION CONTROL, NOTES AND DETAILS				
5	TOPPGRAPHIC AND BOUNDARY SURVEY	S1.1L	STRUCTURAL NOTES		
		S2.1L	FOUNDATION PLAN AND DETAILS		
		S2.2L	ROOF BRACING AND DETAILS		
	ARCHITECTURAL				
A1.0L	ARCHITECTURAL MASTER PLAN				
A2.0L	WALL DESIGNATION & EXIT CAPACITY PLAN				
A2.1L	FLOOR PLAN				
A2.2L	FIRE-RATED CEILING & MECHANICAL EQUIPMENT FLOOR FRAMING PLAN				
A2.3L	ENLARGED PLANS @ OFFICES, RESTROOMS AND KITCHEN	M-1	OVERALL BUILDING HEATING/AIR-CONDITIONING PLAN & OVERALL BUILDING VENTILATION & HVAC PIPING PLAN		
A2.4L	REFLECTED CEILING PLAN	M-2	HVAC SCHEDULES		
A2.5L	ROOF PLAN	M-3	HVAC DETAILS		
A3.1L	EXTERIOR ELEVATIONS	M-4	HVAC DETAILS		
A4.3L	SMALL SCALE CROSS SECTIONS OF: WORSHIP EDUCATIONAL/WOMENS, FELLOWSHIP, REAR ENTRY PORCH & COVERED DROP-OFF	M-5	HVAC DETAILS		
A5.1L	INTERIOR PARTITIONS SECTIONS	MP-1	MECHANICAL AND PLUMBING SPECIFICATIONS		
A5.3L	EXTERIOR WALL SECTIONS	MP-2	MECHANICAL AND PLUMBING SPECIFICATIONS		
A5.4L	EXTERIOR WALL SECTIONS				
A5.7L	SECTIONS THRU PORCHES & COVERED DROP-OFF				
A6.1L	INTERIOR ELEVATIONS				
A6.3L	INTERIOR ELEVATIONS & DETAILS				
A7.1L	DOOR SCHEDULE, DOOR TYPES & WINDOW TYPES	P-1	OVERALL BUILDING WATER PIPING PLAN AND OVERALL BUILDING SWV PLUMBING PLAN		
A7.2L	WINDOW & DOOR DETAILS				
A7.3L	DOOR DETAILS	P-2	OVERALL BUILDING SWV PLUMBING RISER		
A7.4L	FINISH SCHEDULE & TILE LAYOUT	P-3	PLUMBING DETAILS AND SCHEDULES		
A8.1L	GENERAL NOTES & MISC. DETAILS				

FIRE RESISTANCE OF STRUCTURAL ELEMENTS IN HOURS	
FIRE WALLS & PARTY WALLS	4
INTERIOR BEARING WALLS	0
INTERIOR NONBEARING PARTITIONS	0
COLUMNS	0
BEAMS, GIRDERS, TRUSSES, & ARCHES	0
FLOORS & FLOOR/CEILING CONST.	0
ROOF & ROOF/CEILING CONST.	0
EXTERIOR BEARING WALLS & ROOF END GABLES (ALL OVER 30ft SEPERATION)	0
EXTERIOR NONBEARING WALLS & ROOF END GABLES (ALL OVER 30ft SEPERATION)	0
EXIT SHAFT ENCLOSURES (NONE IN PROJECT)	1
FIRE WALLS (NONE IN THIS PROJECT)	2
EXIT ACCESS CORRIDORS	1

OCCUPANT LOADS	
WORSHIP BLDG.	
WORSHIP	252
PLATFORM	24
CHAIRS	1
JANITOR	1
SOUND BOOTH	2
TOTAL OCCUPANT LOAD	280 (1190 EXIT CAPACITY)

FELLOWSHIP BLDG.	
FELLOWSHIP	142
OFFICE	13
ELECTRICAL	1
CLASSROOMS	34
TOTAL OCCUPANT LOAD	190 (1020 EXIT CAPACITY)

EXIT CAPACITY PROVIDED	
WORSHIP / FUTURE KITCHEN	1190
FELLOWSHIP	510
CORRIDORS	510
TOTAL EXIT CAPACITY	2210

SQUARE FOOTAGE:	
ALLOWABLE PER FLOOR - FBC 506	10,500 SQ FT
6,000 SQ FT + (6,000 x .15) =	
MAX AREA (FELLOWSHIP BLDG.)	6,301 SQ FT
WORSHIP BLDG. AREA	3,939 SQ FT
TOTAL PROJECT AREA	10,240 SQ. FT.

BUILDING HEIGHT:	
ALLOWABLE - FBC 502/504:	15' - 3 STORIES
PROPOSED:	23' 6-1/4" - 1 STORY

"Faith is the substance of things hoped for, the evidence of things not seen." Hebrews 11.1

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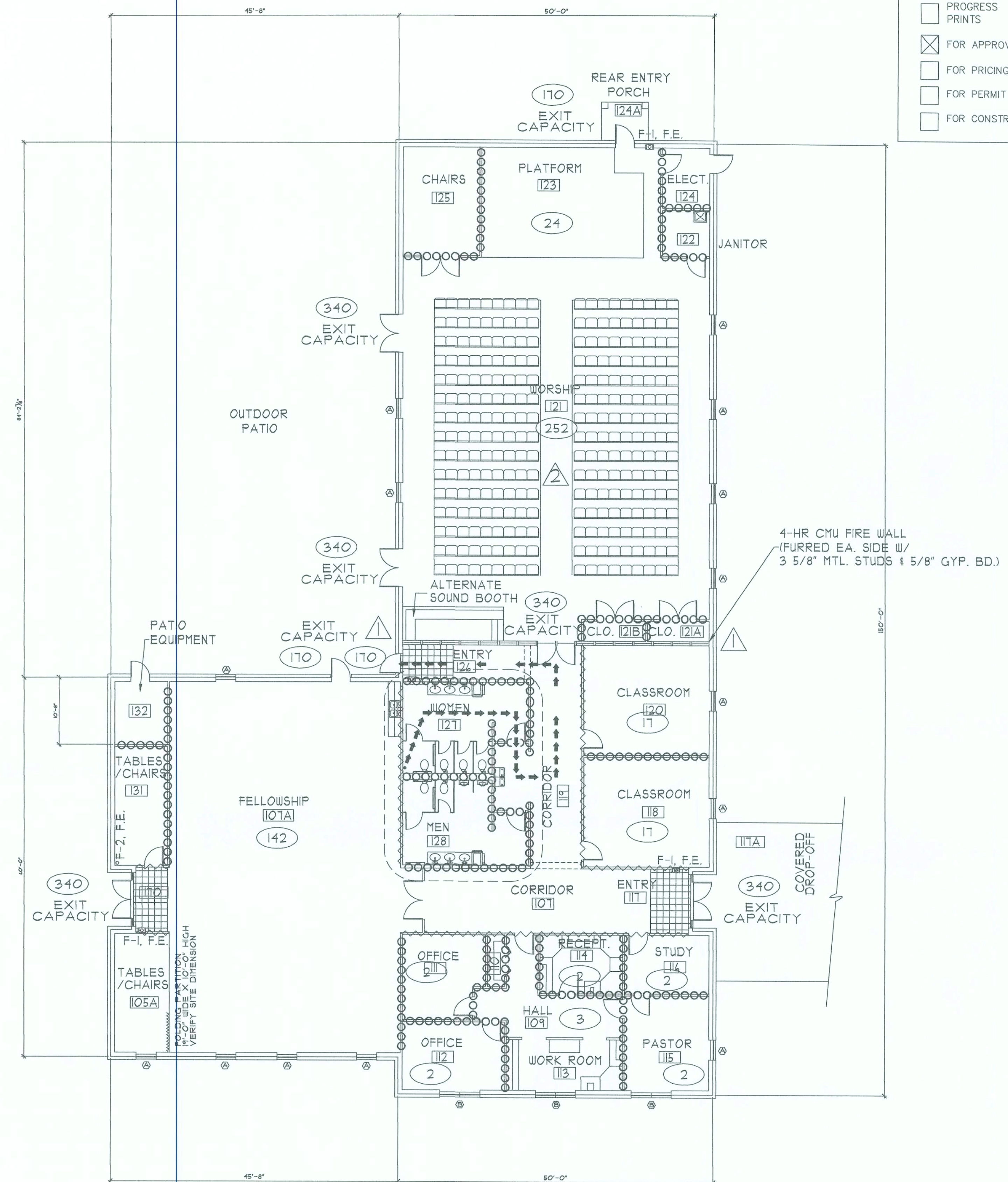
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PLUMBING FIXTURE COUNT FOR 1759 PERSONS		
	REQUIRED	PROVIDED
W/C/UR MEN	2	4
W/C WOMEN	4	4
LAVATORIES	3	1
DRINKING FOUNTAINS	1	2
SERVICE SINKS	1	1

EXIT ACCESS TRAVEL DISTANCE - 1015.1
MAXIMUM TRAVEL DISTANCE ALLOWED - 200'
MAXIMUM TRAVEL DISTANCE- 84'

36" DOOR - 170 PERSONS, 42" DOOR - 200 PERSONS

4- HR CMU FIRE WALL (UL-190IOR EQ.) TO DECK & EXTERIOR WALL FINISH	
1 - HR WOOD STUD SOUND PARTITION TO BOTTOM OF JOIST	
1 - HR WOOD STUD SOUND PARTITION TO RATED CEILING	
NON-RATED WOOD STUD SOUND PARTITION TO 1'-0" MINIMUM AB. CLG (SEE CROSS SECTIONS)-	
1 - HR METAL STUD SOUND PARTITION TO DECK	
E.I.F.S ON METAL STUDS	



1 WALL DESIGNATION & EXIT CAPACITY PLAN
A2.0 SCALE: 3/32"=1'-0"

2 NOTE: CHAIR SEATING TO BE BOLTED TO FLOOR AS NEEDED TO PREVENT REMOVAL

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10/15/07

Thomas Chappell

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LANE CIII, FLORIDA
ALDOSTA, GEORGIA 31602

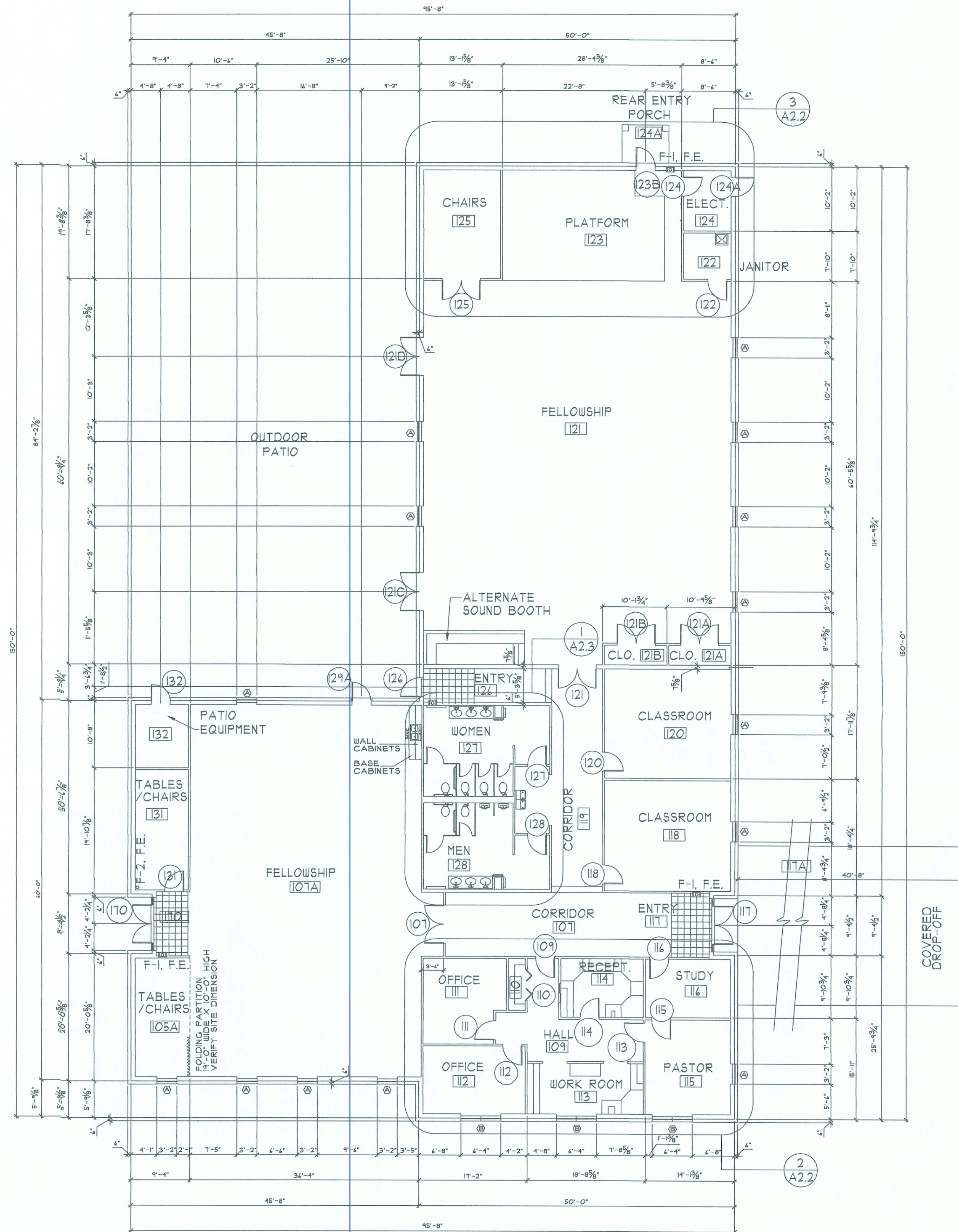
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9

A2.0L



1 FLOOR PLAN
A2.1 SCALE: 3/32"=1'-0"

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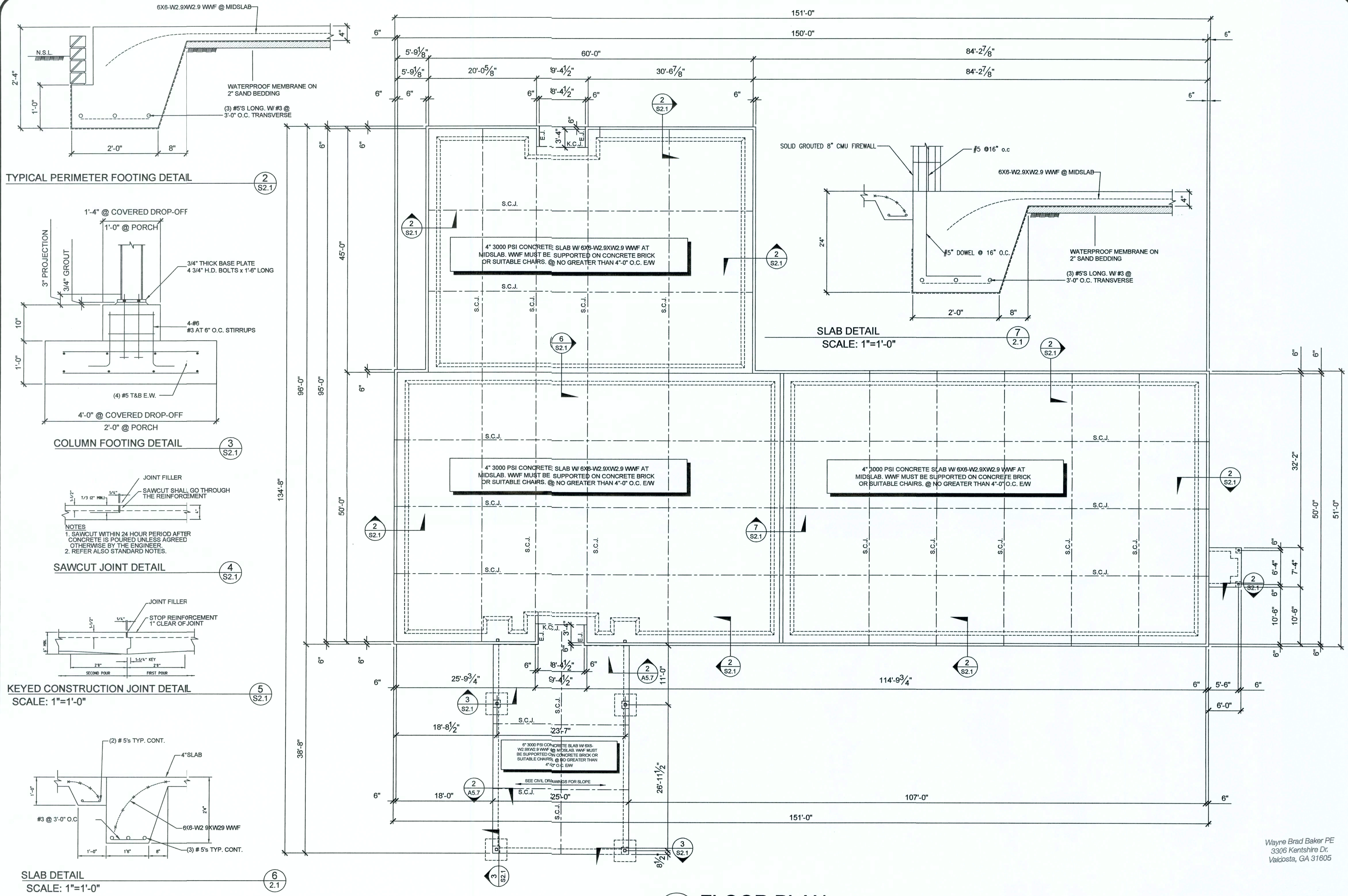
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A2.1L



1 FLOOR PLAN
A2.1 SCALE: 1/8"=1'-0"

Wayne Brad Baker PE
3306 Kentshire Dr.
Valdosta, GA 31605

Wayne Brad Baker
10/15/2007

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ARCHITECTURE/PLANNING/URBAN DESIGN/CONSTRUCTION
MANAGEMENT AIA/AFA

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TYPICAL ABBREVIATIONS

AFF.	ABOVE FINISH FLOOR	FIN.	FINISH FLOOR	P.C.B.	PAINTED CONC. BLOCK
ADJ.	ADJUSTABLE	FLR.	FLOOR	PL.	PLATE
ALT.	ALTERNATE	F.F.	FINISH FLOOR	LB.	POUND
ALUM.	ALUMINUM	F.F.E.	FINISH FLOOR ELEVATION	PCF.	POUNDS PER CUBIC FOOT
AB.	ANCHOR BOLT	F.R.	FIRE RATED	PLF.	POUNDS PER LINEAL FOOT
Z.	ANGLE	FT.	FOOT, FEET	PSF.	POUNDS PER SQUARE FOOT
ANOD.	ANODIZED	FTG.	FOOTING	P.E.J.	PREFORMED EXPANSION JOINT
APPROX.	APPROXIMATELY	FR.	FRAME	PRESS.	PRESSURE
		F.O.	FRAMED OPENING	R.T.	PRESSURE TREATED
		F.S.	FAR SIDE	QTY.	QUANTITY
B.J.	BAR JOIST	GALV.	GALVANIZED	RAD.	RADIUS
BFF.	BELOW FINISHED FLOOR	GA.	GAUGE	REF./RE.	REFERENCE
BS.	BOTH SIDES	GR.	GRADE	REINF.	REINFORCED
BM.	BEAM			REQ'D.	REQUIRED
BLKG.	BLOCK	H.V.A.C.	HEATING, VENTILATION & AIR CONDITIONING	R.D.	ROOF DRAIN
BLK.	BLOCKING	HT.	HEIGHT	R & S	ROD AND SHELF
BOT.	BOTTOM	HOL.	HOLLOW	R.O.	ROUGH OPENING
BMO.	BRICK MASONRY OPENING	H.M.	HOLLOW METAL	S.C.J.	SAWED CONTROL JOINT
BLDG.	BUILDING	HOR.	HORIZONTAL	SECT.	SECTION
BN.	BULLNOSE	I.C.	IN CONTRACT	SHI.	SHEET
BRG.	BEARING	I.D.	INSIDE DIAMETER	SIM.	SIMILAR
CLG.	CEILING	JT.	JOINT	S.R.	SLOPING RIDGE
CMV.	CONCRETE MASONRY UNIT	K.C.J.	KEYED CONTROL JOINT	S.F.	SPREAD FOOTING
CTR.	CENTER	LAM.	LAMINATED	SQ. FT.	SQUARE FEET
CL.	CENTERLINE	L.R.	LEVEL RIDGE	S.S.	STAINLESS STEEL
CLR.	CLEAR	LIN.	LINEAL	S/L.	STEEL
COL.	COLUMN	L.F.	LINEAL FOOT	S	STEEL LINE
CONC.	CONCRETE	MAX.	MAXIMUM	STD.	STANDARD
CSMO.	CONCRETE BLOCK MASONRY OPG.	M.B.M.	METAL BUILDING MFG.	S.D.	STORM DRAIN
CONT.	CONTINUOUS	MFR.	MANUFACTURER	STRUCT.	STRUCTURE OR STRUCTURAL
C.J.	CONTROL JOINT	M.O.	MASONRY OPENING	S.V.	SLOPING VALLEY
CU.	CUBIC	MECH.	MECHANICAL	TK.	THICK
DK.	DECK	MT.	METAL	T.O.B.	TOP OF BEAM
DET/DTL.	DETAIL	MIN.	MINIMUM	T.O.F.	TOP OF FOOTING
DIA.	DIAMETER, ROUND	MISC.	MISCELLANEOUS	T.O.S.	TOP OF STEEL
DIM.	DIMENSION	NOM.	NOMINAL	T.O.M.	TOP OF MASONRY
DN.	DOWN	N.I.C.	NOT IN CONTRACT	T.O.W.	TOP OF WALL
DBL.	DOUBLE	N.T.S.	NOT TO SCALE	TYP.	TYPICAL
DBN.	DOUBLE BULLNOSE	N.S.	NEAR SIDE	T.S.	STRUCTURAL TUBE STEEL
DWG.	DRAWING	O.C.	ON CENTER	U.N.O.	UNLESS NOTED OTHERWISE
EA.	EACH	OPG.	OPENING	VERT.	VERTICAL
EW.	EACH WAY	OPP.	OPPOSITE	W.F.	WALL FOOTING
ELEV.	ELEVATION	O.D.	OUTSIDE DIAMETER	W	WIDE FLANGE SECTION
EQ.	EQUAL			WI	WITH
EXIST.	EXISTING			WD.	WOOD
EXP.	EXPANSION			W.P.	WORK POINT
EJ.	EXPANSION JOINT			W.W.M.	WELDED WIRE MESH
ES.	EXPOSED STRUCTURE			W.W.F.	WELDED WIRE FABRIC
ESP.	EXPOSED STRUCTURE PAINTED				

GENERAL NOTES

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE CONTRACTOR'S PROPERTY AFTER THE COMPLETION OF THE PROJECT.

IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

MECHANICAL FRAMING LOADS, OPENINGS AND STRUCTURE IN ANY WAY RELATED TO MECHANICAL REQUIREMENTS ARE SHOWN FOR BIDDING PURPOSES ONLY. CONTRACTOR SHALL OBTAIN APPROVAL OF MECHANICAL AND OTHER TRADES BEFORE PROCEEDING WITH SUCH PORTION OF THE WORK. EXCESS COST RELATED TO VARIATION IN MECHANICAL REQUIREMENTS TO BE BORNE BY MECHANICAL CONTRACTOR.

SHOULD ANY OF THE DETAILED INSTRUCTIONS SHOWN ON THE PLANS CONFLICT WITH THE GENERAL STRUCTURAL NOTES, THE SPECIFICATIONS OR WITH EACH OTHER, THE STRICTEST PROVISION SHALL GOVERN.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS AND FOR COORDINATING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THOSE SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. IF DISCREPANCIES IN THE DIMENSIONS OCCUR, IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO BRING THE DISCREPANCY TO THE ATTENTION OF THE ARCHITECT BEFORE PROCEEDING WITH THE WORK.

UNLESS NOTED OTHERWISE, REQUIREMENTS GIVEN FOR ONE OR MORE LOCATIONS ALSO APPLY AT OTHER LOCATIONS AT WHICH CONDITIONS ARE SIMILAR. THE REQUIREMENTS GIVEN SHALL BE ADAPTED TO CONDITIONS AT SUCH OTHER LOCATIONS.

THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE MOST CURRENT SET OF DRAWINGS, SPECIFICATIONS, ADDENDA, SUPPLEMENTAL INFORMATION, RFIs, ETC, TO FABRICATORS AND SUPPLIERS.

ALL MEANS AND METHODS OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR AND NOT THE ENGINEER OF RECORD.

STRUCTURAL STEELWORK NOTES

IN ADDITION TO COMPLYING WITH ALL PERTINENT CODES AND REGULATIONS, COMPLY WITH THE FOLLOWING:

AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"

AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", INCLUDING THE "COMMENTARY OF THE AISC SPECIFICATION" (ASD 9TH EDITION WITH ALL SUPPLEMENTS)

AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS" APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS OF THE ENGINEERING FOUNDATION.

PROVIDE A325 BOLTS FOR ALL FRAMING CONNECTIONS. A307 BOLTS MAY BE USED FOR COLUMN ANCHOR BOLTS ONLY. EXPANSION BOLTS SHALL BE "HILTI KWIK BOLTS" OR APPROVED EQUAL.

ALL WELDING SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE" WITH E70XX ELECTRODES BY THE SHIELDED METAL ARC PROCESS. ALL WELDING SHALL BE PERFORMED BY CURRENTLY CERTIFIED WELDERS. SHOP DRAWINGS SHALL SHOW ALL SHOP AND ERECTION DETAILS INCLUDING CUTS, COPE CONNECTIONS, HOLES, THREADED FASTENERS, RIVETS, AND WELDS.

THE APPROVAL OF SHOP DRAWINGS WILL BE FOR SIZE AND ARRANGEMENT OF PRINCIPAL AND AUXILIARY MEMBERS AND STRENGTH OF CONNECTIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DIMENSIONS SHOWN ON THE SHOP DRAWINGS.

IMMEDIATELY AFTER SURFACE PREPARATION, APPLY RED OXIDE STRUCTURAL STEEL PRIMER PAINT IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. USE PAINTING METHODS WHICH WILL RESULT IN FULL COVERAGE OF JOINTS, CORNERS, EDGES AND ALL EXPOSED SURFACES.

PROVIDE TWO COATS OF ASPHALTIC PAINT ON ALL STRUCTURAL STEEL SHAPES EXPOSED TO THE SOIL OR BELOW TOP OF SLAB ON GRADE.

SIMPLE SHEAR CONNECTIONS SHALL BE SELECTED AND DESIGNED BY THE STEEL FABRICATOR BASED ON THE REACTIONS PROVIDED ON THE DRAWINGS. IF REACTIONS ARE NOT PROVIDED ON THE DRAWINGS, THE FABRICATOR SHALL DESIGN THE CONNECTIONS FOR THE MAXIMUM ALLOWABLE UNIFORM LOADING LISTED IN THE AISC ASD MANUAL.

CONCRETE NOTES

ALL CONCRETE WORK SHALL CONFORM TO THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-02) AND SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-02) OF THE AMERICAN CONCRETE INSTITUTE.

PROVIDE AT LEAST ONE COPY OF THE ACI FIELD REFERENCE MANUAL, SP-15, IN THE FIELD OFFICE AT ALL TIMES.

ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60.

C.I.P. CONCRETE CONSTRUCTION OR CONTROL JOINTS IN SLAB ON GRADE TO BE AS SHOWN ON PLANS. WHERE NOT SHOWN, LIMIT CONTROL JOINT SPACING TO NO GREATER THAN 15 FEET ON ANY SIDE FOR 6 INCH SLABS AND 12 FEET FOR 4 INCH SLABS ON GRADE. THE SECTIONS BOUNDED BY CONTROL CONSTRUCTION JOINTS SHALL BE APPROXIMATELY SQUARE, WITH THE LENGTH TO WIDTH RATIO LESS THAN 1.5.

COORDINATE CONCRETE WORK WITH ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ARCHITECTURAL FINISHED CONCRETE, RECESSED AREAS, EMBEDDED ITEMS, AND SPECIAL CONTROL JOINTS.

PROVIDE DOWELS IN FOOTINGS TO MATCH VERTICAL WALL AND PIER REINFORCING. PROVIDE CORNER BARS AT WALL AND FOOTING CORNERS TO MATCH HORIZONTAL REINFORCING. SEE BAR SPLICE TABLE BELOW FOR MINIMUM LENGTH OF EACH LEG.

UNLESS OTHERWISE NOTED OR DETAILED, ALL REINFORCING BAR SPLICES SHALL BE IN ACCORDANCE WITH THE TABLE SHOWN BELOW:

BAR SIZE	CONCRETE SPLICES	
	TYPE A (NON TOP BAR) LENGTH (IN)	TYPE B (TOP BAR) LENGTH (IN)
#3	14	18
#4	19	24
#5	23	30
#6	30	40
#7	41	54
#8	55	71
#9	69	90
#10	88	114
#11	108	140

TOP BARS ARE HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 IN. OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCEMENT.

PROVIDE THE FOLLOWING MINIMUM CLEAR COVER FOR REINFORCING STEEL FROM SURFACE OF CONCRETE:

- FOR CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3" IN GENERAL
- 2" FOR FOUR INCH SLABS ON GRADE
- FOR CONCRETE SURFACE EXPOSED TO THE WEATHER OR AGAINST

1-1/2" FOR #5 BARS AND SMALLER EXCEPT AS NOTED FOR PRECAST

1-1/2" FOR COLUMN SPIRALS OR TIES, OR STIRRUPS (EXCEPT PAN JOISTS)

3/4" FOR SLABS AND FOR STIRRUPS IN PAN JOISTS
TOLERANCES FOR BAR COVER ARE ± 3/8 INCH FOR CONCRETE HAVING A THICKNESS OF 8 INCHES OR LESS AND ± 1/2 INCH FOR CONCRETE HAVING A THICKNESS GREATER THAN 8 INCHES. TOLERANCE FOR LONGITUDINAL LOCATION OF BENDS AND ENDS OF REINFORCEMENT SHALL BE ± 2 INCHES EXCEPT AT DISCONTINUOUS ENDS OF MEMBERS WHERE THE TOLERANCE SHALL BE ± 1/2 INCH. TOLERANCE FOR BAR SPACING IS ± 2 INCHES. TOLERANCE FOR LENGTH OF LAP SPLICE IS -1 INCH, AND TOLERANCE FOR EMBEDDED LENGTH LENGTH IS ± 1 INCH.

DESIGN DATA

BUILDING CODE: FBC (LATEST EDITION)

OCCUPANCY CATEGORY: III (ASSEMBLY)

LIVE LOAD:
ROOF LIVE LOAD: 20 (PSF)

COLLATERAL LOAD: 10 (PSF) (INCLUDES DEAD LOAD OF CEILING)

WIND LOAD:
BASIC WIND SPEED (3-SECOND GUST): 110 (MPH)
WIND IMPORTANCE FACTOR (I): 1.15
WIND EXPOSURE: B
ENCLOSURE CLASSIFICATION: ENCLOSED
INTERNAL PRESSURE COEFFICIENT: +/- 0.18

FOOTING NOTES

SOILS REPORT:

CONTRACTOR SHALL COMPLY WITH THE RECOMMENDATIONS OF THE REPORT, INCLUDING PROVIDING TESTING OF ALL BACKFILLING AND COMPACTION ON SLAB ON GRADE BY AN APPROVED TESTING LABORATORY.

SOIL INVESTIGATION PREFORMED BY:
CAL-TECH TESTING, INC.
P.O. BOX 1625, LAKE CITY, FL 32254 PH: 386-755-3633

THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER AFTER EXCAVATION TO DETERMINE IF THE SOILS ENCOUNTERED ARE CAPABLE OF SUPPORTING THE DESIGN SOIL PRESSURE AND ARE COMPATIBLE WITH THE SOILS REPORT.

ALLOWABLE NET SOIL BEARING PRESSURE ASSUMMED FOR DESIGN.....1500 PSF

MATERIAL SPECIFICATIONS

MINIMUM 28 DAY CONCRETE STRENGTHS:
FOOTINGS.....3000 PSI
SLABS ON GRADE.....3000 PSI

SEPARATE DRAWINGS SHOWING PARTS OF THE DESIGN INVOLVING SPECIALTY ENGINEERING SUCH AS: METAL BUILDING SYSTEMS, BAR JOISTS, TILT-WALL, LIGHT GAUGE STEEL, PRE-FABRICATED WOOD TRUSSES OR ANY OTHER PRE-FABRICATED COMPONENT SHALL BEAR THE DATE, SIGNATURE, AND SEAL OF A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH THE BUILDING IS LOCATED. ALL SUCH DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL (BEFORE FABRICATION PROCEEDS).

AT LEAST ONE COPY OF CALCULATIONS (LONG REPORT OPTION) SHALL ALSO BE SUBMITTED TO THE ARCHITECT FOR FILING PURPOSES ONLY.

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VALDOSTA, GA 31605
PH: 229-834-4693
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PROFESSIONAL ENGINEER'S SEAL

Wayne B. Baker

10/12/07

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MANAGEMENT AIA/AFA

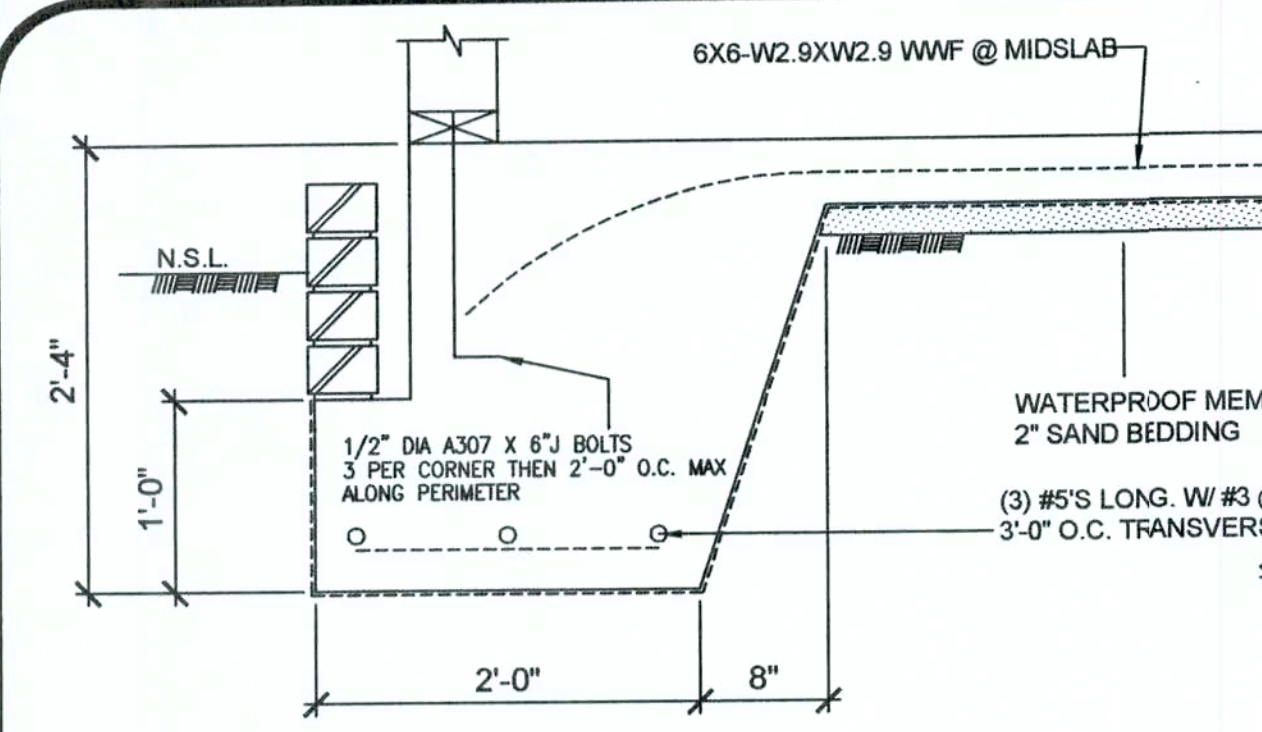
"Faith is the substance of things hoped for, the evidence of things not seen." Hebrews 11:1
GRACE COVENANT BAPTIST CHURCH
LAKE CITY, FL

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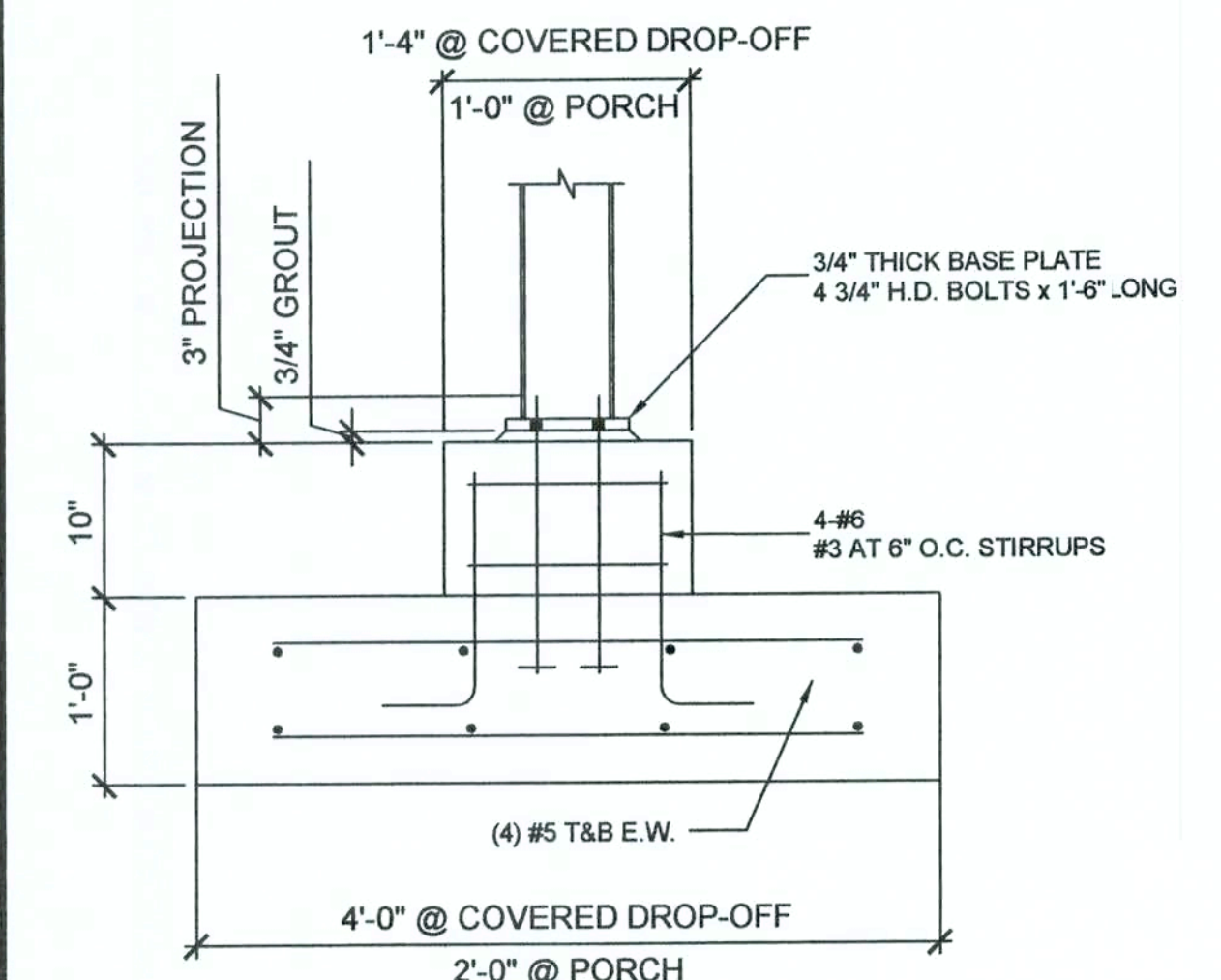
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JOB NO.
04-816
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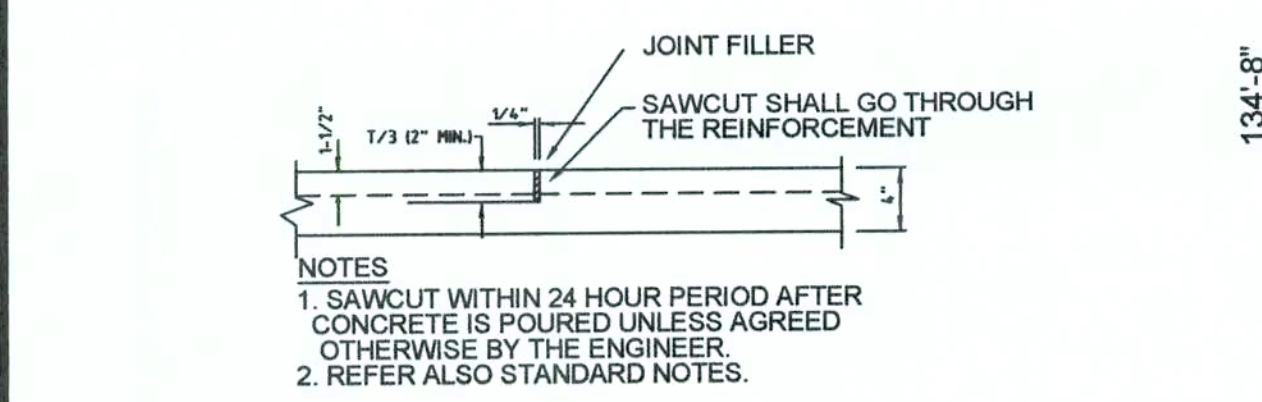
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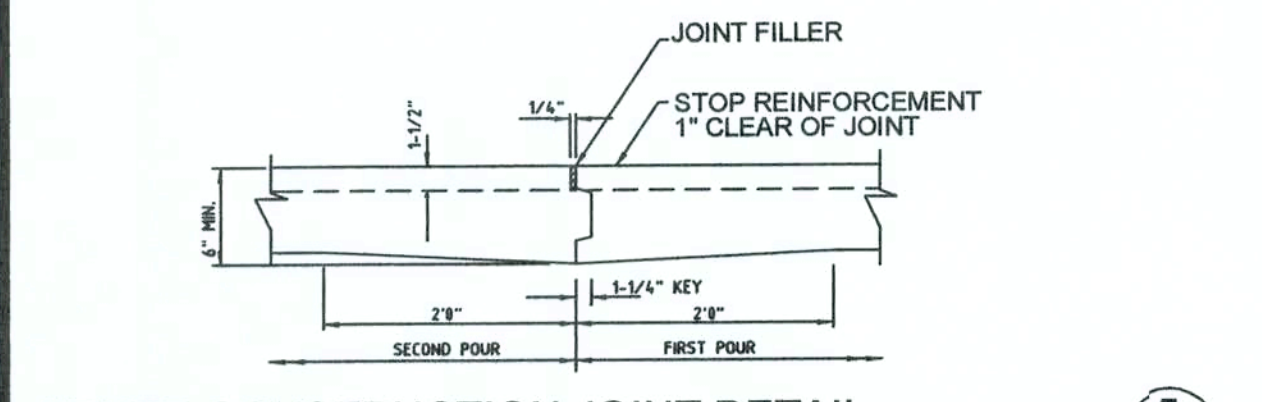
TYPICAL PERIMETER FOOTING DETAIL
2 S2.1



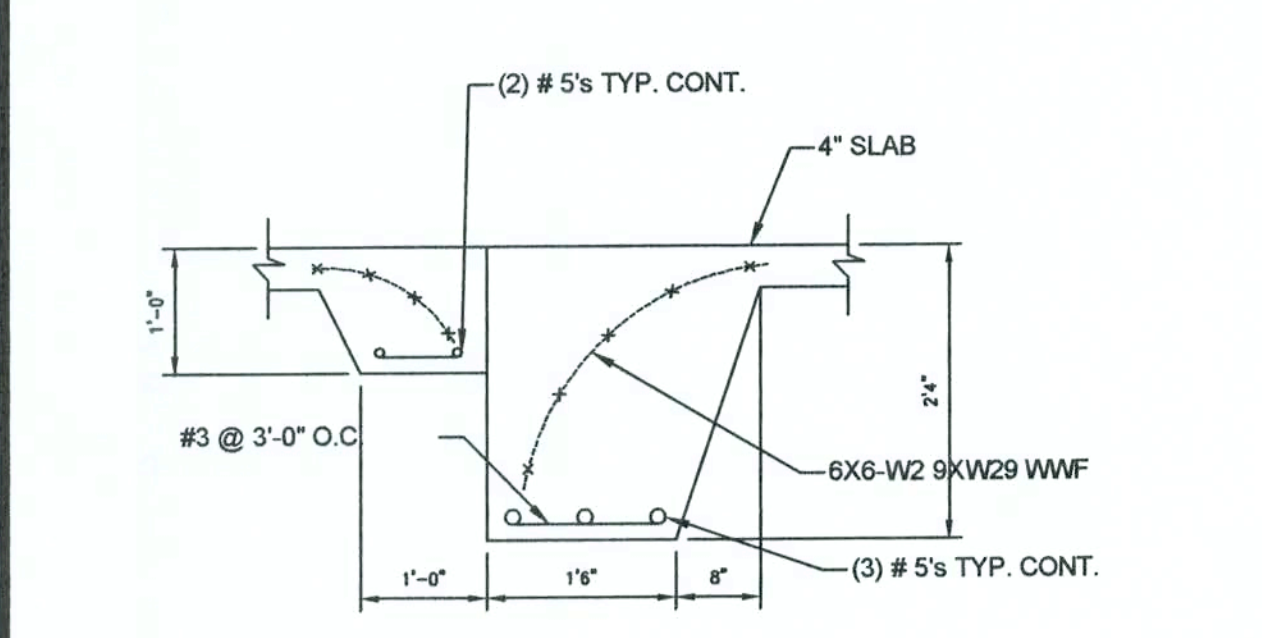
COLUMN FOOTING DETAIL
3 S2.1



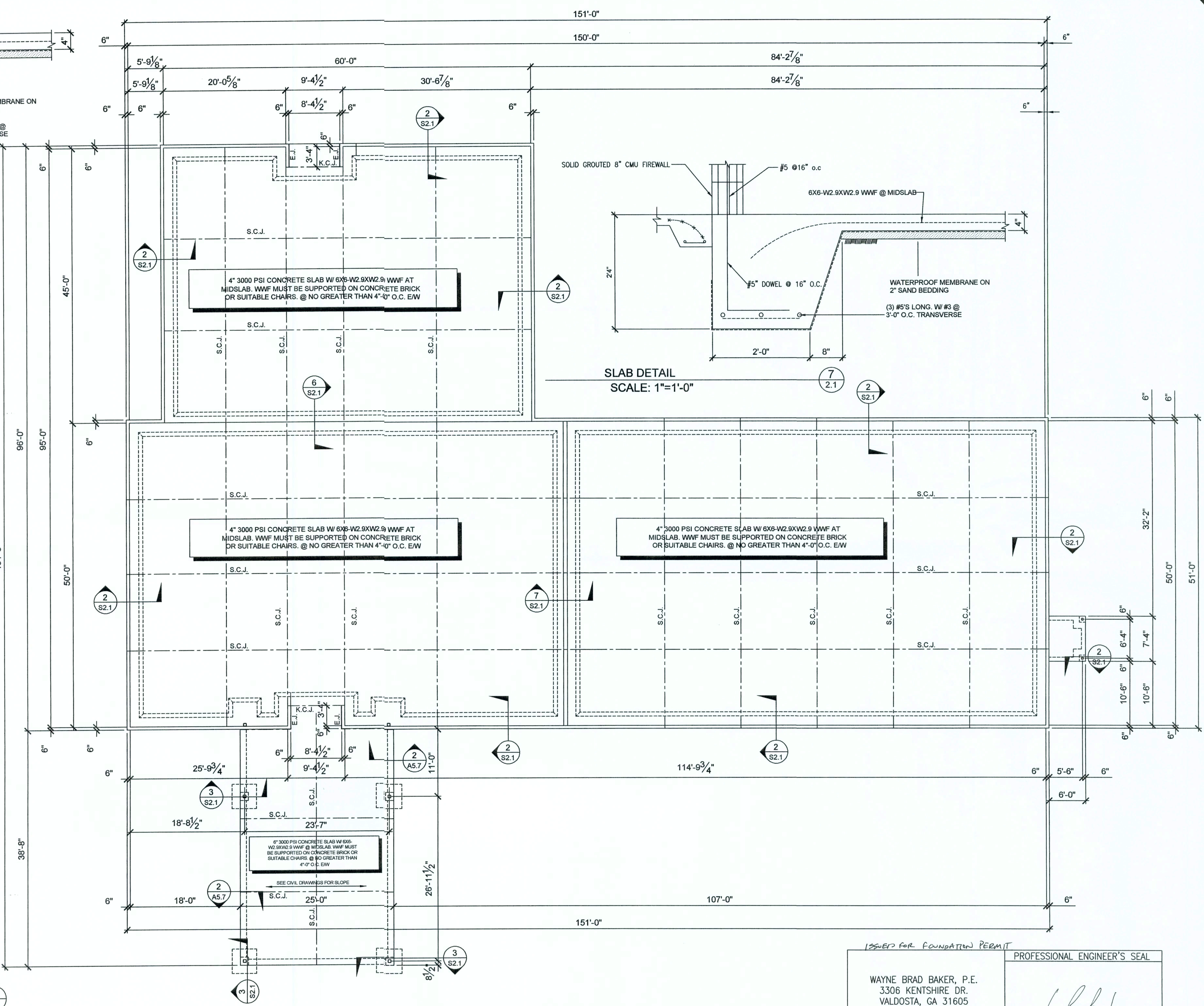
SAWCUT JOINT DETAIL
4 S2.1



KEYED CONSTRUCTION JOINT DETAIL
SCALE: 1"=1'-0"
5 S2.1



SLAB DETAIL
SCALE: 1"=1'-0"
6 S2.1



1 FLOOR PLAN
A2.1 SCALE: 1/8"=1'-0"

ISSUED FOR FOUNDATION PERMIT

PROFESSIONAL ENGINEER'S SEAL

WAYNE BRAD BAKER, P.E.
3306 KENTSHIRE DR.
VALDOSTA, GA 31605
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10/18/07

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ARCHITECTURE/PLANNING/URBAN DESIGN/CONSTRUCTION
MANAGEMENT AIA/APA

"Faith is the substance of things hoped for, the evidence of things not seen." Hebrews 11:1

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Wayne Brad Baker, P.E.

Project : Grace Covenant
Subject : MWFRS
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

Design Wind Pressure, p, Equation 6-19 (ASCE 7-02)

Design wind pressures and forces are determined per equations given in section 6.5.12

System Type	Structure Type	Equation
Main Wind-Force Resisting System	Low-Rise Buildings Gable Roof	$p = qh[(GCp) - (GCp)]$ at mean roof height h given in Figure 6-10 given in Figure 6-5

Velocity Pressure Calculations, qh

Velocity pressure qh is calculated in accordance with section 6.5.10

qh = Velocity pressure @ mean roof height (h) (Eq. 6-10)
qh = Constant: Kh Kzt Kd V² - I
qh = Velocity pressure @ height (h)

Where: Constant = Numerical constant
= $\frac{1}{16} [(Air\ density\ lb/cu\ ft) / (32.2\ ft/sec^2)]$ (Section 6.5.10)
= $\frac{1}{16} [(1.25)(5280\ ft/mi) \cdot (1\ hr/3600\ s)]^2$
= 0.00256

Mean Sea Level = 0.00 ft (Table 6-1)
Air Density @ MSL = 0.0755 lb/cu ft

Category = III (Table 1-1)
Importance Factor = 1.15 (Table 6-1)

Exposure Category = C (Open terrain) (Table 6-2)
Alpha = 9.00 (Table 6-2)
Zg = 900.00 ft

Basic Wind Speed = 110.00 mph (Figure 6-1)
Mean Roof Height = 21.00 ft

Wayne Brad Baker, P.E.

Project : Grace Covenant
Subject : MWFRS
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

Velocity Pressure Calculations, qz (Cont.)

Where: Kh = Velocity pressure coefficient @ height z (Eq. 6-3a)
= $2.01 \cdot (Z/Zg)^{2.67} \cdot (Z/Alpha)$ for $15\ ft \leq Z \leq Zg$ (Eq. 6-3a)
= $2.01 \cdot (15/Zg)^{2.67} \cdot (Z/Alpha)$ for $Z < 15\ ft$
= 0.91

Kzt = Topographic factor obtained from Fig. 6-4
Kzt @ h = $(1 + K1 \cdot K2 \cdot K3)^{-1}$
= 1.00

Topography = None

Kd = Wind directionality factor obtained from Table 6-4
= 0.85

qh = 27.59 (psf)

Internal Pressure Coefficient, GCpi, Figure 6-5

The internal pressure coefficients are given in Figure 6-5

Enclosure Classification	GCpi	GCpi	Ri	GCpi	GCpi
Enclosed Buildings	0.18	-0.18	1.00	0.18	-0.18

Wayne Brad Baker, P.E.

Project : Grace Covenant
Subject : MWFRS
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

External Pressure Coefficient, GCpe, Figure 6-10

The pressure force coefficient are given in Figure 6-10

Roof Angle	1	2	3	4	5	6	7E	8E	9E
22.62 deg	0.54	-0.45	-0.47	-0.41	-0.45	-0.45	0.77	-0.72	-0.69

Design Wind Pressure, p, (cont.) Equation 6-18

Design wind pressures and forces are determined per equations given in section 6.5.12

Building Surface	Cp	qh	GCpi	GCpi	p ⁺	p ⁻
1	0.54	27.59	0.18	-0.18	10.00	19.81
2	-0.45	27.59	0.18	-0.18	-17.59	-10.00
3	-0.47	27.59	0.18	-0.18	-17.85	-10.00
4	-0.41	27.59	0.18	-0.18	-17.38	-10.00
5	-0.45	27.59	0.18	-0.18	-17.59	-10.00
6	-0.45	27.59	0.18	-0.18	-17.38	-10.00
7E	0.77	27.59	0.18	-0.18	10.00	19.81
8E	-0.72	27.59	0.18	-0.18	-17.59	-10.00
9E	-0.69	27.59	0.18	-0.18	-17.38	-10.00
4E	-0.69	27.59	0.18	-0.18	-17.47	-11.83

p⁺ uses GCpi

Design Wind Pressure for Overhangs, p, Equation 6-18

The design equation has been modified to qh (GCpi) - G (Underneath G) for overhang pressures

0.85 is used for Underneath G instead of GCpi (G = 90)

Roof Zone	2E
For roof only	-37.39 -38.59

Wayne Brad Baker, P.E.

Project : Grace Covenant Church
Subject : C & C
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

Design Wind Pressure, p, Equation 6-22 (ASCE 7-02)

Design wind pressures and forces are determined per equations given in section 6.5.12

System Type	Structure Type	Equation
Components and Cladding	Low-Rise Buildings and Buildings with h <= 60 ft Gabled, Hipped and Stepped Roofs	$p = qh[(GCp) - (GCp)]$ at mean roof height given in Figure 6-11, 6-12 given in Figure 6-5

Velocity Pressure Calculations, qh

Velocity pressure qh is calculated in accordance with section 6.5.10

qh = Velocity pressure @ mean roof height (h) (Eq. 6-15)
qh = Constant: Kh Kzt Kd V² - I
qh = Velocity pressure @ mean roof height (h)

Where: Constant = Numerical constant
= $\frac{1}{16} [(Air\ density\ lb/cu\ ft) / (32.2\ ft/sec^2)]$ (Section 6.5.10)
= $\frac{1}{16} [(1.25)(5280\ ft/mi) \cdot (1\ hr/3600\ s)]^2$
= 0.00256

Mean Sea Level = 0.00 ft (Table 6-1)
Air Density @ MSL = 0.0755 lb/cu ft

Category = III (Table 1-1)
Importance Factor = 1.15 (Table 6-1)

Exposure Category = C (Open terrain) (Table 6-2)
Alpha = 9.00 (Table 6-2)
Zg = 900.00 ft

Basic Wind Speed = 110.00 mph (Figure 6-1)
Mean Roof Height = 21.00 ft

Wayne Brad Baker, P.E.

Project : Grace Covenant Church
Subject : C & C
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

Velocity Pressure Calculations, qz (Cont.)

Where: Kh = Velocity pressure coefficient @ height z (Eq. 6-3a)
= $2.01 \cdot (Z/Zg)^{2.67} \cdot (Z/Alpha)$ for $15\ ft \leq Z \leq Zg$ (Eq. 6-3a)
= $2.01 \cdot (15/Zg)^{2.67} \cdot (Z/Alpha)$ for $Z < 15\ ft$
= 0.91

Kzt = Topographic factor obtained from Fig. 6-4
Kzt @ h = $(1 + K1 \cdot K2 \cdot K3)^{-1}$
= 1.00

Topography = None

Kd = Wind directionality factor obtained from Table 6-4
= 0.85

qh = 27.59 (psf)

Internal Pressure Coefficient, GCpi, Figure 6-5

The internal pressure coefficients are given in Figure 6-5

Enclosure Classification	GCpi	GCpi	Ri	GCpi	GCpi
Enclosed Buildings	0.18	-0.18	1.00	0.18	-0.18

Wayne Brad Baker, P.E.

Project : Grace Covenant Church
Subject : C & C
Location : Lake City, FL

File :
Date : 10/17/2007
Eng. : WBB

External Pressure Coefficient, GCpe, Figure 6-11 and Figure 6-12

The pressure force coefficient are given in Figure 6-11 and Figure 6-12

Zone	Area (sq. ft.)	Angle (deg)	GCpi	GCpi	GCpi
1	10.00	22.62	0.50	-0.80	0.00
2	10.00	22.62	0.50	-1.70	-2.30
3	10.00	22.62	0.50	-0.80	-3.70
4	10.00	22.62	1.00	-1.10	-
5	10.00	22.62	1.00	-1.10	-

Design Wind Pressure, p, Equation 6-22

Design wind pressures and forces are determined per equations given in section 6.5.12

Values of external and internal pressures shall be combined algebraically

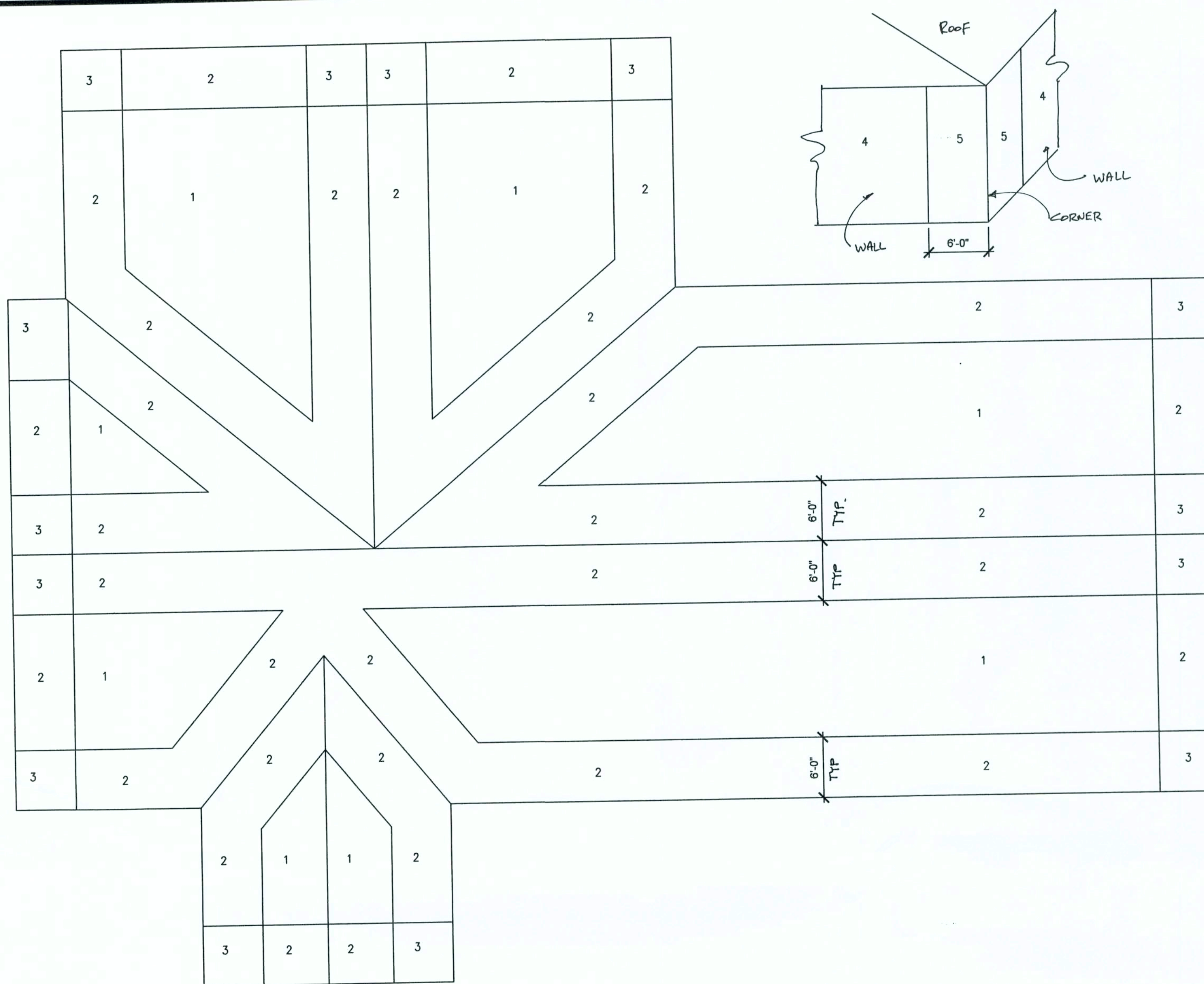
Zone	qh	GCpi	GCpi	GCpi	GCpi	p ⁺	p ⁻	p ⁻
1	27.59	0.50	-0.80	0.18	-0.18	10.00	13.24	-27.04
2	27.59	0.50	-1.70	0.18	-0.18	10.00	18.78	-41.94
3	27.59	0.50	-0.80	0.18	-0.18	10.00	18.78	-41.94
4	27.59	1.00	-1.10	0.18	-0.18	22.62	32.66	-33.69
5	27.59	1.00	-1.10	0.18	-0.18	22.62	32.66	-33.69

p⁺ uses GCpi and GCpip⁻ uses GCpi and GCpi

Roof overhang wind pressures

0.85 is used for Underneath G instead of GCpi (G = 90)

Roof Zone	2E
For roof only	-37.39 -38.59



WIND ZONES

TRUSS
MEG
USE
THESE

MWFS

C+C

ROOF AND WALL WIND LOADS

WAYNE BRAD BAKER, P.E.
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PROFESSIONAL ENGINEER'S SEAL

Wayne Brad Baker
10/18/07

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MANAGEMENT AIA/AFA

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