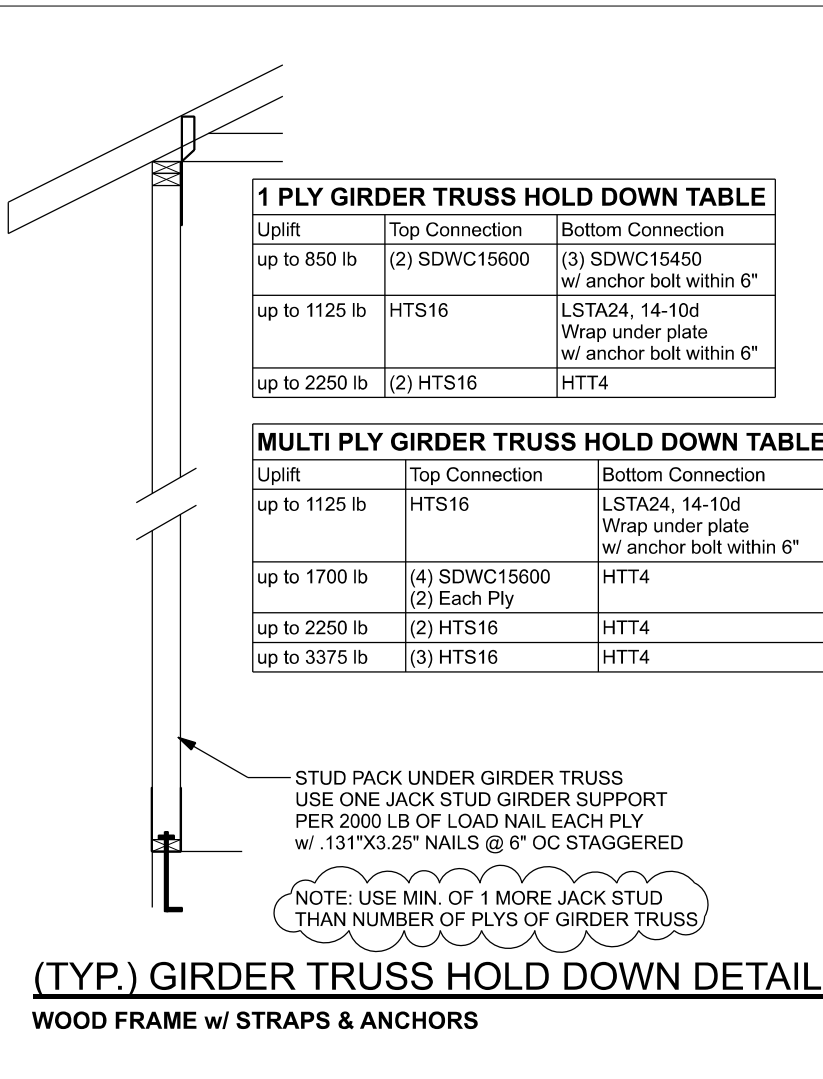
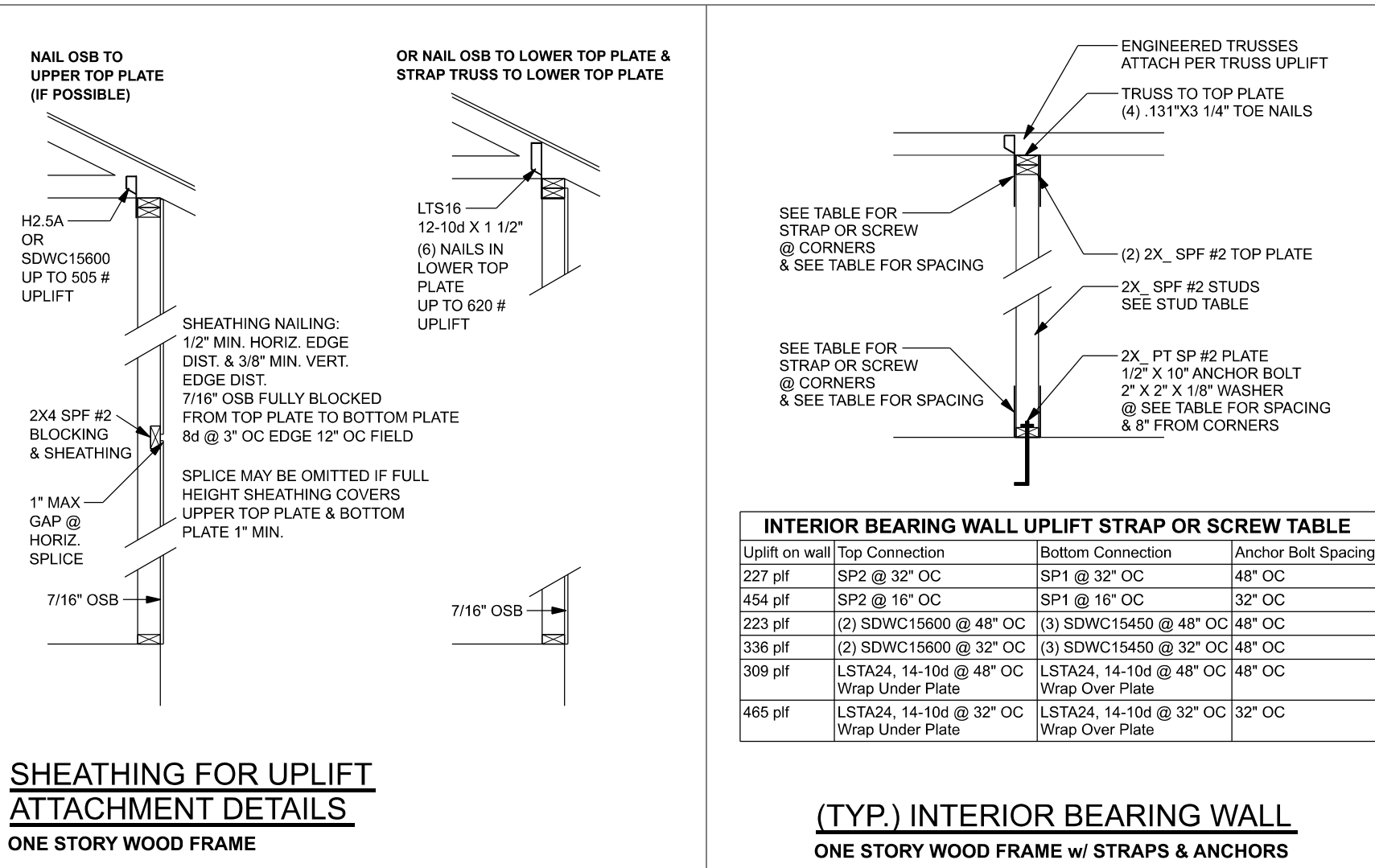


ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)				
Wind Speed	Sheathing Thickness Plywood OR OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
120 mph Exp. D	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	1/8"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. D	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. B	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	1/8"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet from each edge of ridges and hips, the nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased for certain types of roofing materials. See manufacturer Florida product approval.



CONNECTOR TABLE						
UL919 SPF			Truss Connector	To Plate	To Truss/Rafter	
805	505	SDWC15600		-	-	
415	290	H3		4-8x4 1/2"	4-8x4 1/2"	
615	540	H2.5A		5-8x4 1/2"	5-8x4 1/2"	
1340	1015	H5		9-8x4 1/2"	9-8x4 1/2"	
620	620	TS12-30		6-10x1 1/2"	6-10x1 1/2"	
1000	860	MTS12-30		6-10x1 1/2"	6-10x1 1/2"	
1450	1245	HTS20-30		12-10x1 1/2"	12-10x1 1/2"	
UL919 SPF			Strap Ties	To One Member	To Other Member	
1235	1235	LSA721		8-10d	8-10d	
1540	1435	HS720A		9-10d	9-10d	
1030	1030	CS20		7-10d	7-10d	
UL919 SPF			Stud Plate Ties	To Stud	To Plate	
585	535	SP1		6-10d	4-10d	
1065	605	SP2		6-10d	6-10d	
771	771	LS730A		10-10d	10-10d	
1235	1235	LSA724		14-10d	14-10d	
UL919 SPF			Holdowns @ Stewrnall	To Stud / Post	12"x12" Tien HD	
1825	1800	DT722		8-SDS 1/4"x1"	12"x12" Tien HD	
4235	3840	HTT4		18-16x2 1/2"	12"x12" Tien HD	
UL919 SPF			Holdowns @ Mono	To Stud / Post	12"x12" Tien HD	
1825	1800	DT722		8-SDS 1/4"x1"	12"x12" Tien HD	
4235	3840	HTT4		18-16x2 1/2"	12"x12" Tien HD	
UL919 SPF			Post Bases @ Stewrnall	To Post	12"x12" Drill & Epoxy	
1000		ABU442		12-16d	5/8"x12" Drill & Epoxy	
2475		ABU602		12-16d	5/8"x12" Drill & Epoxy	
UL919 SPF			Post Bases @ Mono	To Post	12"x12" Drill & Epoxy	
1000		ABU442		12-16d	5/8"x12" Drill & Epoxy	
2475		ABU602		12-16d	5/8"x12" Drill & Epoxy	

**EXTERIOR WALL STUD TABLE FOR SPF 22 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5.  
EXTERIOR WALL BEARING & NON WALL BEARING STUD LENGTH  
FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR  
RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C,  
STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH).  
STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING  
LOCATED OUTSIDE 4 FEET OF THE EXTERIOR ZONE LOADING.  
(END ZONE EXAMPLE 16" O.C. @ 8" & 12" O.C.)

(1) 2x4 @ 16" O.C.	TO 10'-1" STUD HEIGHT
(2) 2x4 @ 12" O.C.	TO 11'-2" STUD HEIGHT
(3) 2x6 @ 16" O.C.	TO 15'-7" STUD HEIGHT
(4) 2x6 @ 12" O.C.	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TRUSS CONNECTIONS. THE ENGINEER SHALL REVIEW THE TRUSS DESIGN FOR THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE ENGINEER SHALL REVIEW THE TRUSS DESIGN FOR THE TRUSS MANUFACTURER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 70 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 2500$  PSI

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F'_c = 2500$  PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.), CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI. ALL LAP SPLICES  $40 \times DB$  ( $25 \times$  FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE THE COMPLETE PART OF THE WIND ENGINEERING DESIGN OF WORK. THE COMPLETE SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DIRECTION, ZONE, AND FLOOD ZONE.

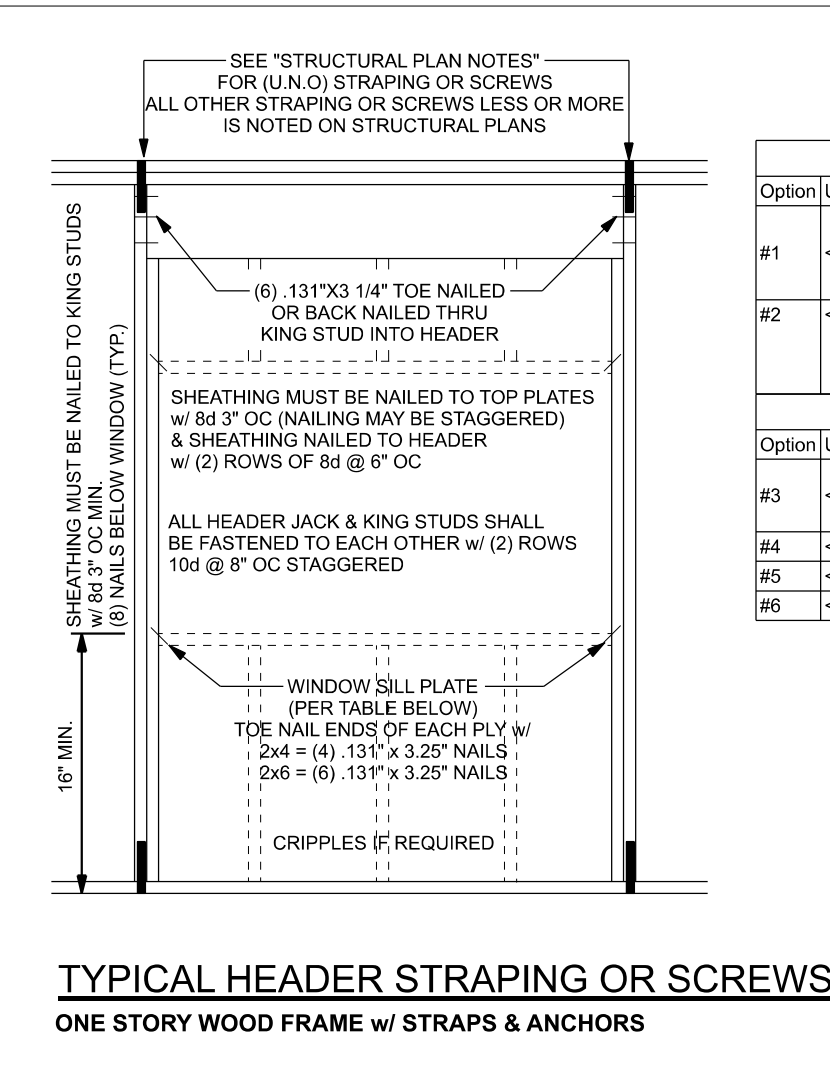
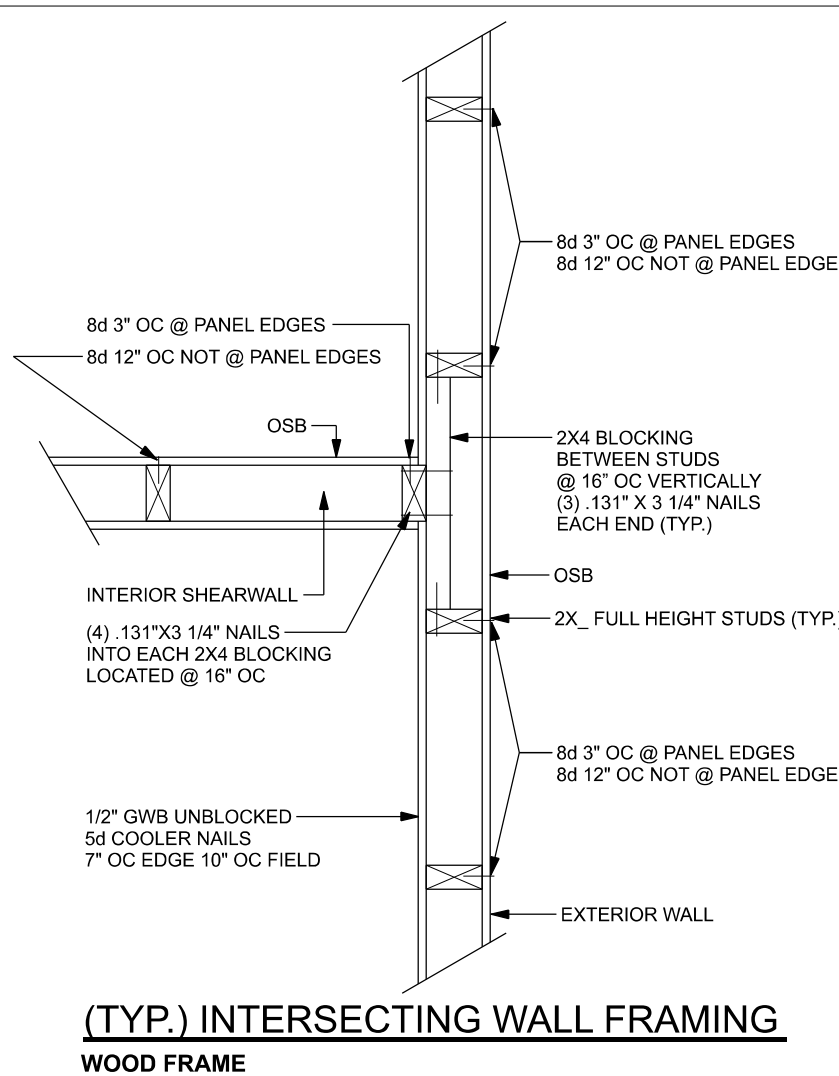
PROVIDE MANUFACTURER'S DESIGN TECHNIQUES, WHICH COMPLY WITH FBKR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN MOTIS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER FOR VERIFICATION.

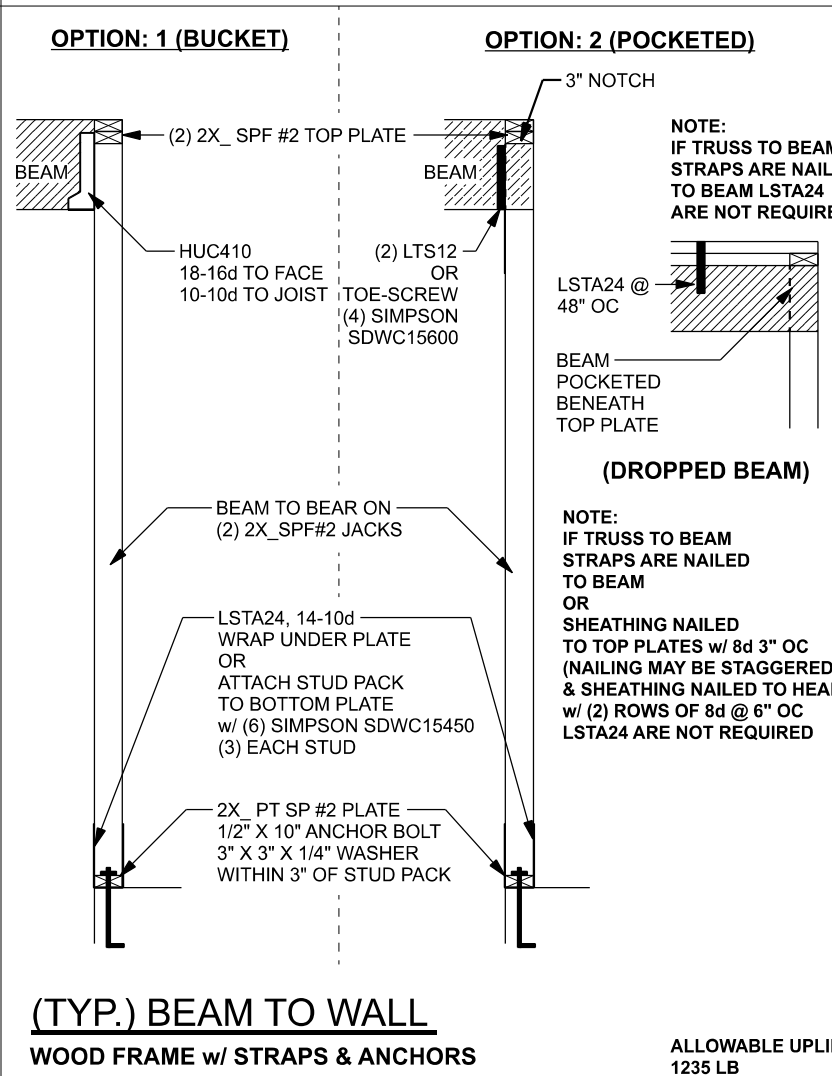
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, LAYOUT, PLACEMENT, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS AT ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

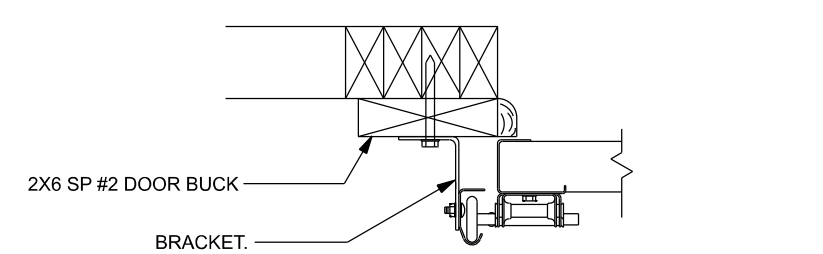
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBKR, IS BASED ON REACTIONS, UPLIFTS AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND ENGINEER. HOWEVER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN, INCLUDING THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL. THE BUILDER IS RESPONSIBLE FOR THE TRUSS DESIGN LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH WIND ENGINEERING DESIGN AND TRUSS MANUFACTURER SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE TRUSS MANUFACTURER'S DESIGN IS ALSO DEPENDENT ON THE DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER. THE TRUSS MANUFACTURER IS ALSO RESPONSIBLE FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEET.



<h2 style="text-align: center;">HEADER SCREWS TABLE</h2>					
Top Connection		Bottom Connection			
Attach king stud to top plate w/ (1) Simpson SDWC15600		Attach king stud to bottom plate w/ (2) Simpson SDWC15450 $1/2" \times 10"$ Anchor bolt w/ $3" \times 3" \times 14"$ washer must be located within 6" of king stud @ all door locations			
Attach king stud to top plate w/ (2) Simpson SDWC15600		Attach king stud to bottom plate w/ (3) Simpson SDWC15450 $1/2" \times 10"$ Anchor bolt w/ $3" \times 3" \times 14"$ washer must be located within 6" of king stud @ all door locations			
<h2 style="text-align: center;">HEADER STRAP TABLE</h2>					
Top Connection		Bottom Connection			
LSTA24, 14-10d wrap over plate		LSTA24, 14-10d wrap under plate $1/2" \times 10"$ Anchor bolt w/ $3" \times 3" \times 14"$ washer must be located within 6" of king stud @ all door locations			
MSTA24, 18-10d header to jacks		DT12Z			
(2) MSTA24, 18-10d header to jacks		DT12Z			
(2) MSTA24, 18-10d header to jacks		HTT4			
<h2 style="text-align: center;">SILL PLATE SPANS FOR 10'-0" WALL HEIGHT</h2>					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-2.9B
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
100 MPH EXP C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS IN SILL SPAN SHALL BE DIVIDED BY (p/10)



DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION  
WOOD FRAME

DESIGN CRITERIA & LOADS:	
BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2003)
CODE FOR DESIGN LOADS	ASCE 7-22
<b>WIND LOADS</b>	
BASIC WIND SPEED (ASCE 7-22, 16 GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
<b>SOIL BEARING CAPACITY</b>	1500 PSF
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)				
EFFECTIVE WIND AREA (FT <sup>2</sup> )	ZONE 4 INTERIOR		ZONE 5 END 4 FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd)	-34.2(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult)	-57(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(Vasd)	-25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd)	-24.1(Vasd)

Bryan Zecher Homes, Inc

Terry Wayne Res.

**PROJECT ADDRESS:**  
SW Kimberly Lane  
Lake City, FL

Mark Disosway FL PE 53915  
This item has been digitally signed and sealed by  
Mark Disosway PE on digital signature date.  
Printed copies of this document are not considered  
signed and sealed and the signature must be  
verified on any electronic copies.

C=US, O=Florida  
dnQualifier=A01  
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0, CN=Mark d  
Disosway  
2024-10-17 18:  
21:10

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

**LIMITATION:** This design is valid for one building, at specified location.

**Mark Disosway P.E.**  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
241103

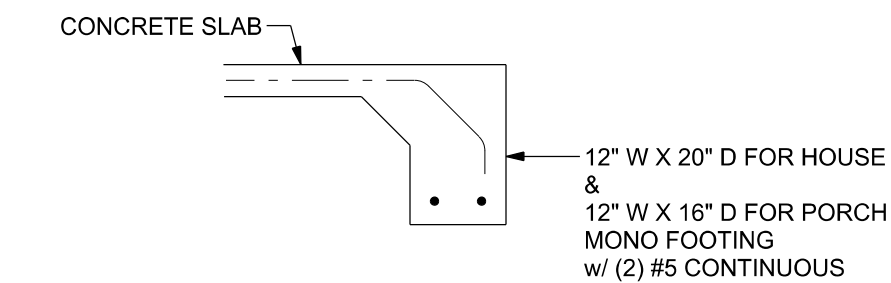
**S-1**  
OF 3 SHEETS



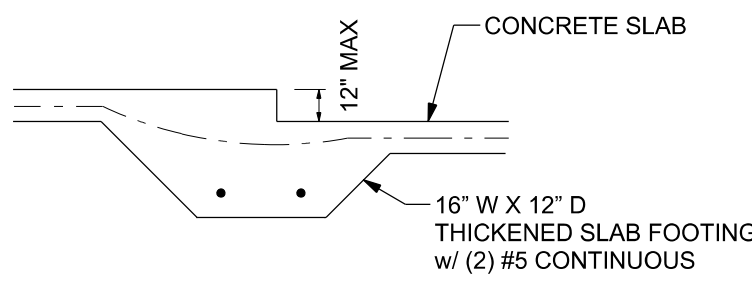
TALL STEM WALL TABLE:						
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.						
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7
3.3	3.0	96	96	96	96	96
4.0	3.7	96	96	96	96	96
4.7	4.3	88	96	96	96	96
5.3	5.0	56	96	96	96	96
6.0	5.7	40	80	96	80	96
6.7	6.3	32	56	80	56	96
7.3	7.0	24	40	56	40	80
8.0	7.7	16	32	48	32	64
8.7	8.3	8	24	32	24	48
9.3	9.0	8	16	24	16	40

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wine ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

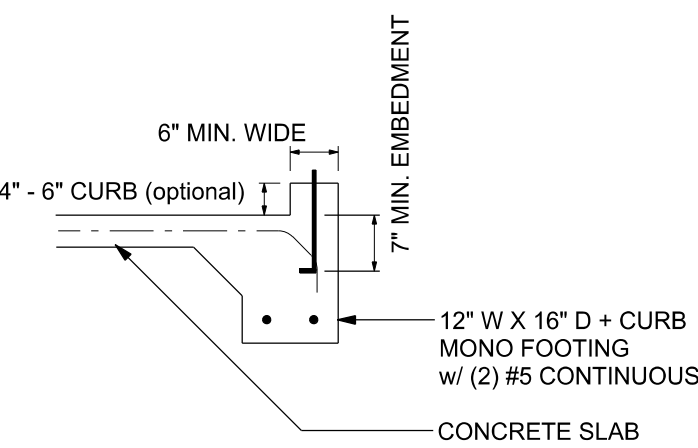
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



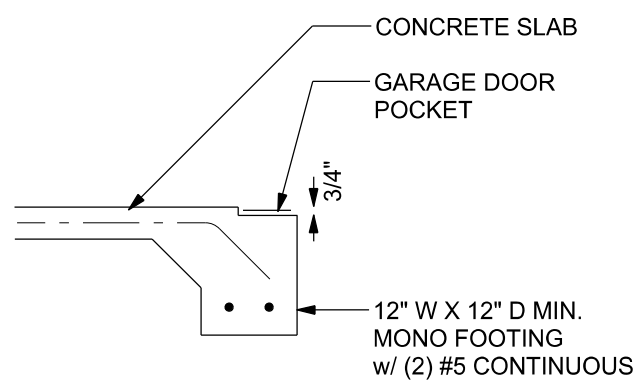
F1 S-2 OPTIONAL MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



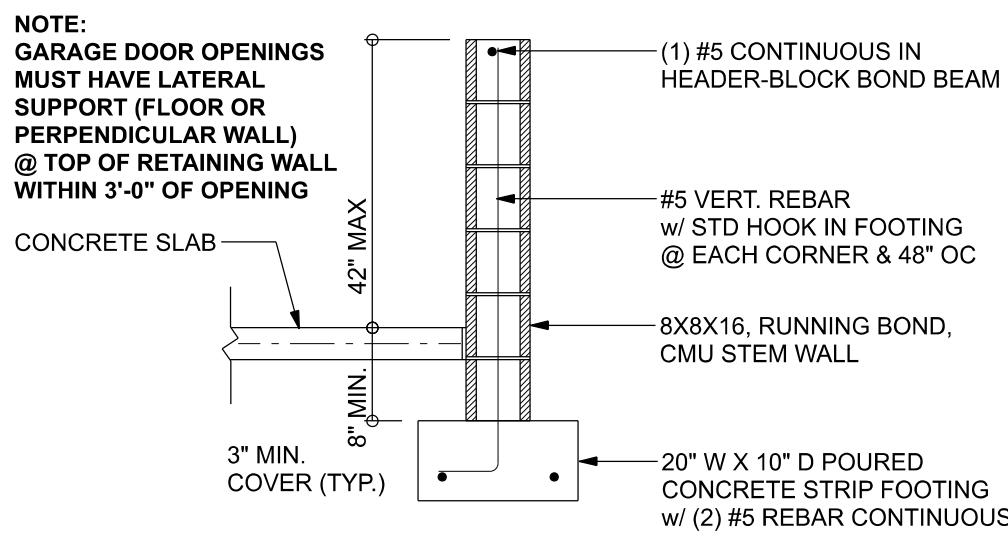
F3 S-2 OPTIONAL INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



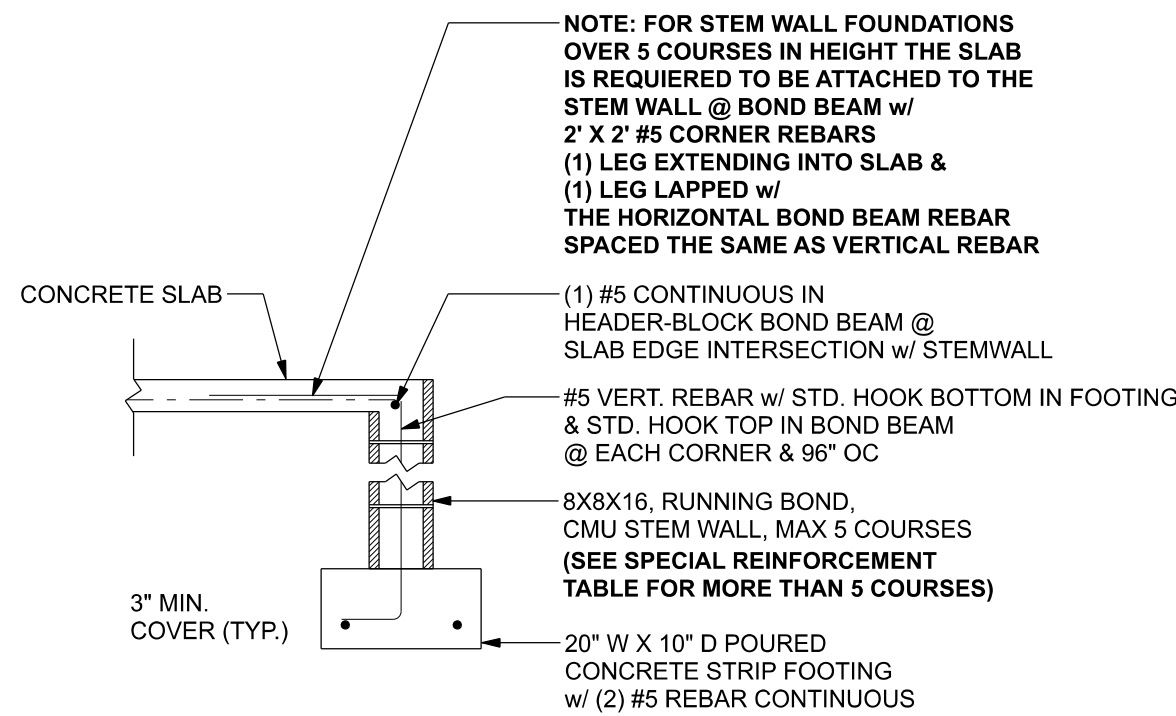
F4 S-2 OPTIONAL MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING  
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"



F1 S-2 STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"

#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

#### FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 2 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED W/ 6X6-1/4" WELDED WIRE MESH PLACED ON CHAIRS @ 1' 1/2" DEPTH OR FIBER MESH CONCRETE, 8-MIL POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITE-TREATMENT METHOD CAN BE USED INSTEAD).

NOTE: GARAGE DOOR OPENINGS MUST HAVE LATERAL SUPPORT (FLOOR OR PERPENDICULAR WALL) @ TOP OF RETAINING WALL WITHIN 3'-0" OF OPENING

- (1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM
- #5 VERT. REBAR w/ STD HOOK IN FOOTING @ EACH CORNER & 48" OC
- 8X8X16, RUNNING BOND, CMU STEM WALL
- 20" W X 10" D POURED CONCRETE STRIP FOOTING w/ (2) #5 REBAR CONTINUOUS

NOTE: FOR STEM WALL FOUNDATIONS OVER 5 COURSES IN HEIGHT THE SLAB IS REQUIRED TO BE ATTACHED TO THE STEM WALL @ BOND BEAM w/ 2" X 2" #5 CORNER REBARS (1) LEG EXTENDING INTO SLAB & (1) LEG LAPPED w/ THE HORIZONTAL BOND BEAM REBAR SPACED THE SAME AS VERTICAL REBAR

- (1) #5 CONTINUOUS IN HEADER-BLOCK BOND BEAM @ SLAB EDGE INTERSECTION w/ STEMWALL
- #5 VERT. REBAR w/ STD. HOOK BOTTOM IN FOOTING & STD. HOOK TOP IN BOND BEAM @ EACH CORNER & 96" OC
- 8X8X16, RUNNING BOND, CMU STEM WALL, MAX 5 COURSES (SEE SPECIAL REINFORCEMENT TABLE FOR MORE THAN 5 COURSES)
- 20" W X 10" D POURED CONCRETE STRIP FOOTING w/ (2) #5 REBAR CONTINUOUS

Bryan Zecher Homes, Inc

Terry Wayne Res.

PROJECT ADDRESS:  
SW Kimberly Lane  
Lake City, FL

Mark Disosway FL PE 53915  
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DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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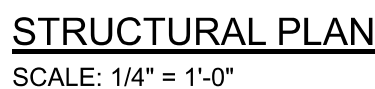
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.  
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386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
241103

S-2  
OF 3 SHEETS

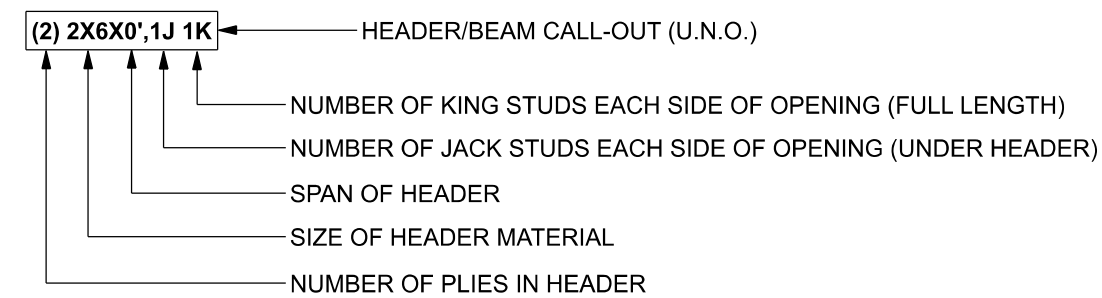


SN-1 DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. SN-2 LATERAL BRACING IS TO BE RESTRAINED PER BCSI-103, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

CONNECTIONS, WALL, & HEADER DESIGN IS BASED  
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING  
FURNISHED BY BUILDER. W. B. HOWLAND COMPANY INC.  
JOB #24-1378

### HEADER LEGEND




Bryan Zecher Homes, Inc.

Terry Wayne Res.

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Lake City, FL

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**S-3**  
OF 3 SHEETS