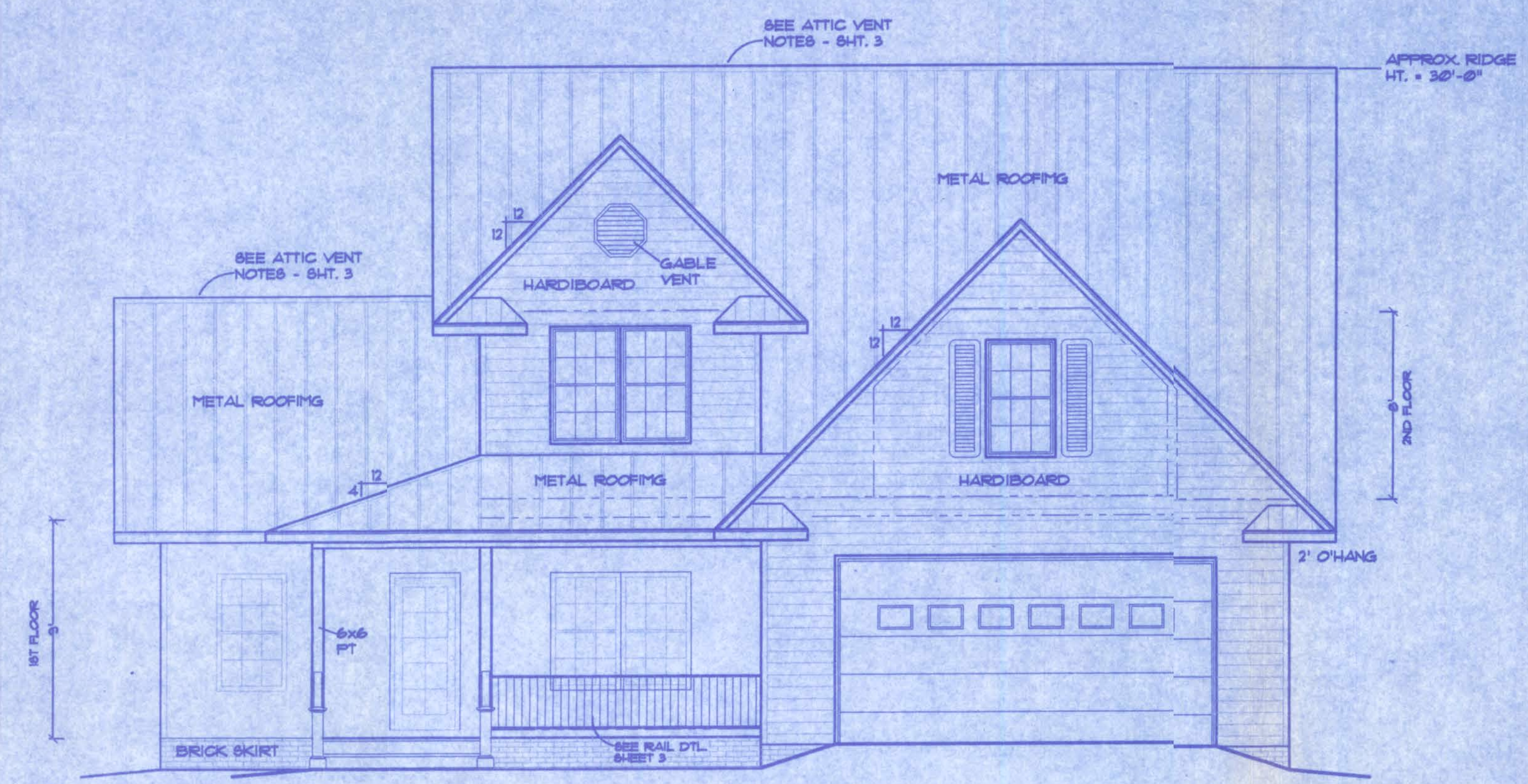
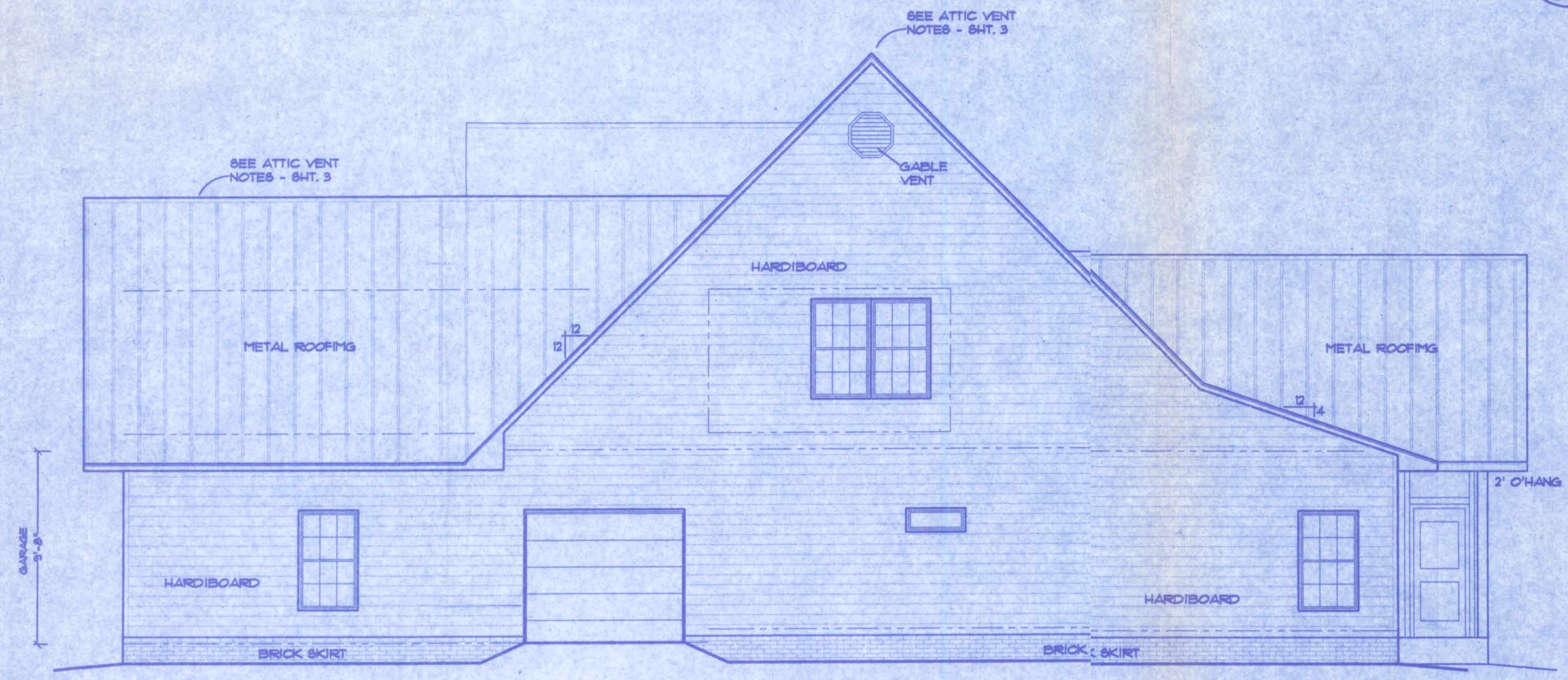


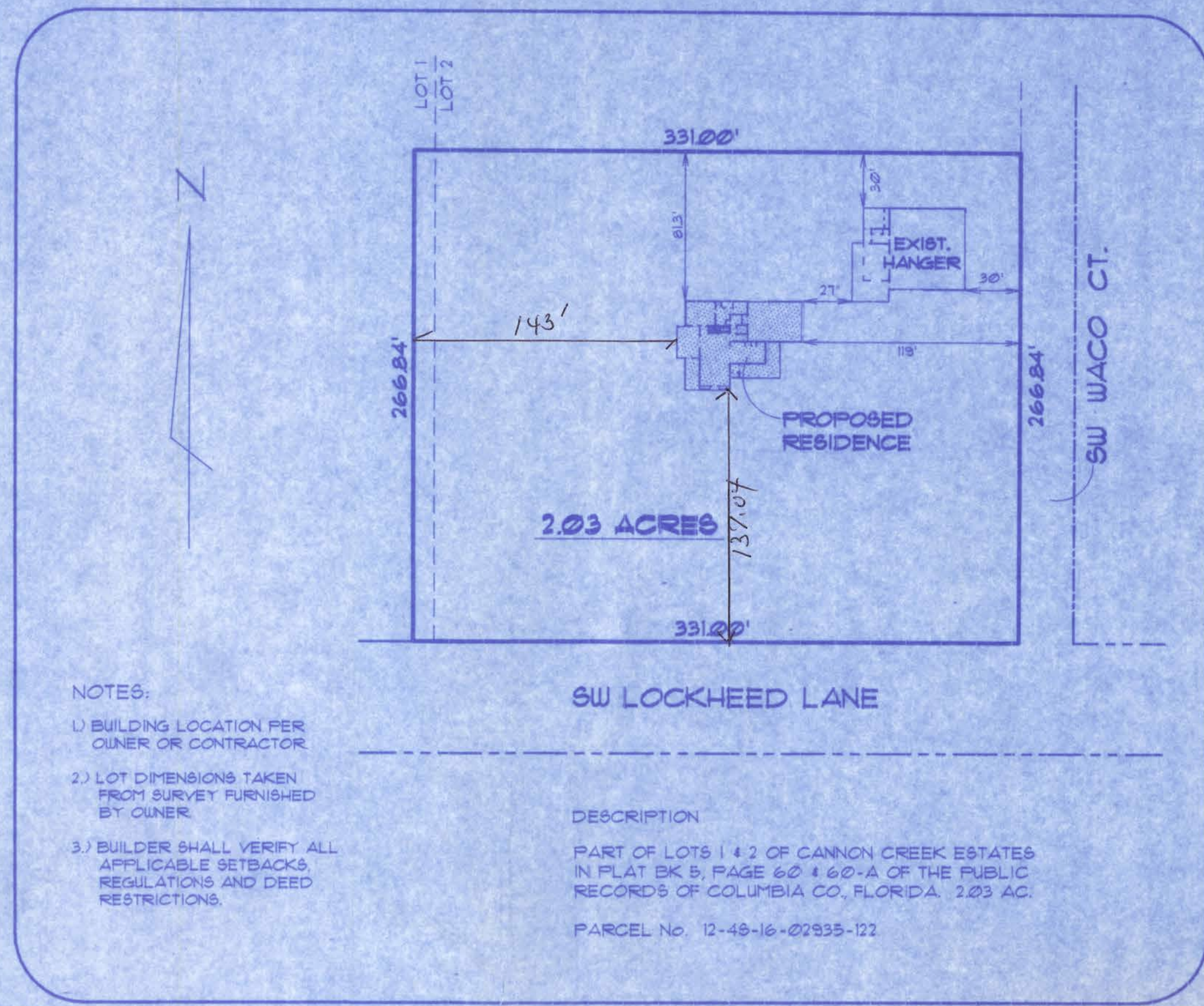
Ryan Residence



FRONT ELEVATION
SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION
SCALE: 1/4 IN. = 1 FT.



Index to Sheets

SHEET A-1	ELEVATIONS + FLOOR PLAN
SHEET A-2	ELEVATIONS + GEN. NOTES
SHEET A-3	FLOOR PLAN
SHEET A-4	FOUNDATION + SECTIONS
SHEET A-5	ELECTRICAL
SHEET S-1	WIND ENGINEERING

A-1

WINDLOAD ENGINEER: Mark Dicosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: _____ Job No.: 808257

FILE: 08-008	RYAN RESIDENCE	SHEET: 1 OF 5
DATE: 8/8/08		CAD. FILE: 08008
DRAWN: T A D	PREPARED BY: TIM DELBENE	REV:
CHECK: T A D	Drafting + Technical Services	REV:
	192 SW Sagewood Gln., Lake City, FL 32024	
	Phone (386) 755-5891	



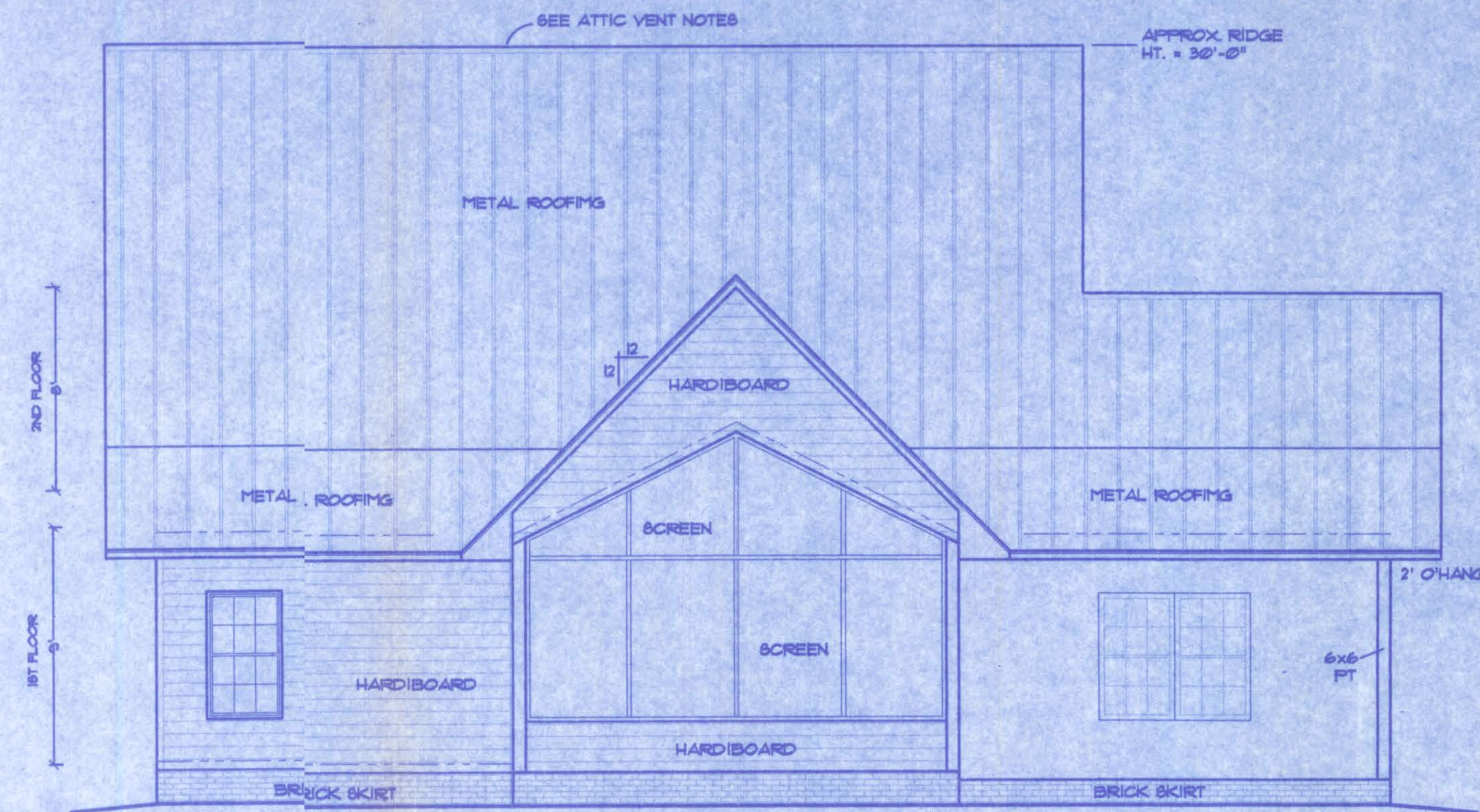
GENERAL NOTES

- 1.) See Wind Load Detail Sheet S-1 and WindEngineer's Notes for data pertaining to Wind Design and comply w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for modification.
- 7.) Site analysis or preparation information is part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

ATTIC VENTILATION

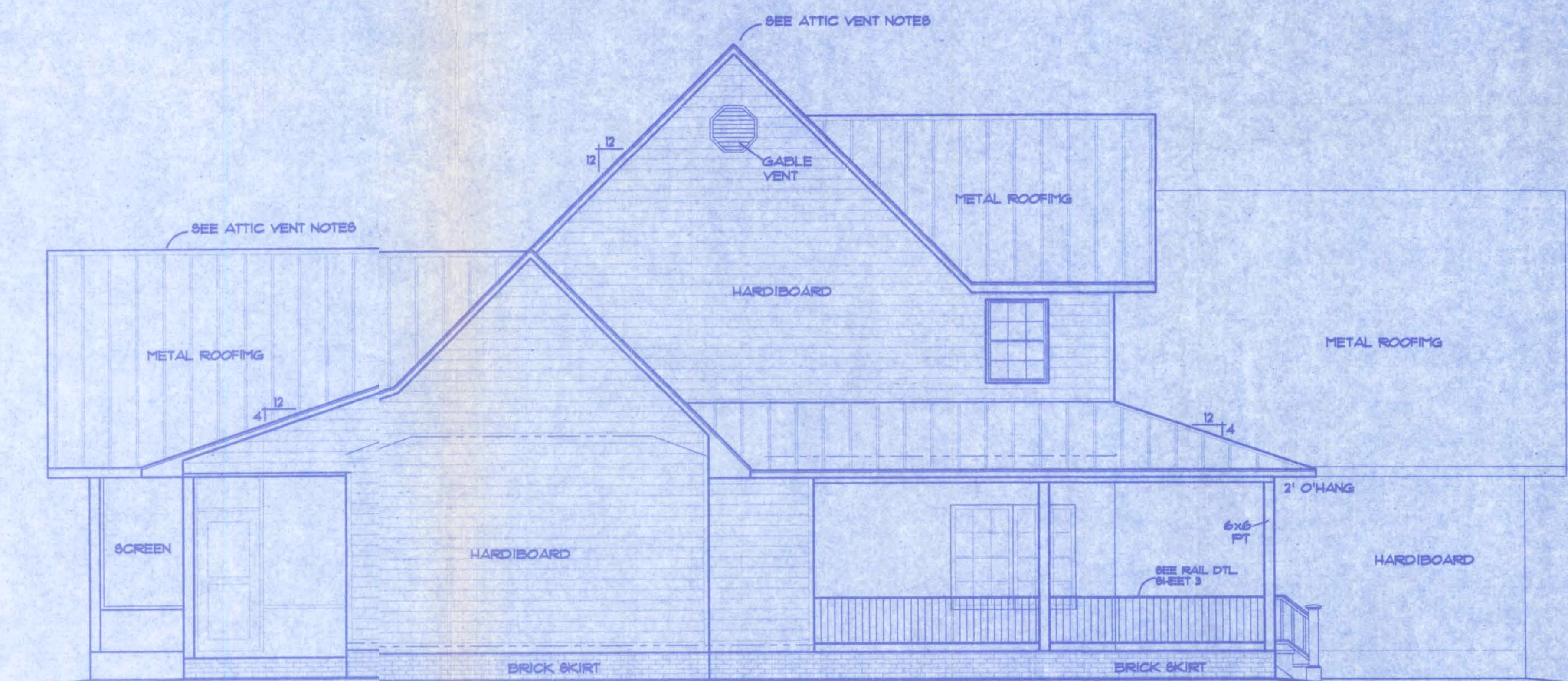
Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.



REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



LEFT ELEVATION

SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location:

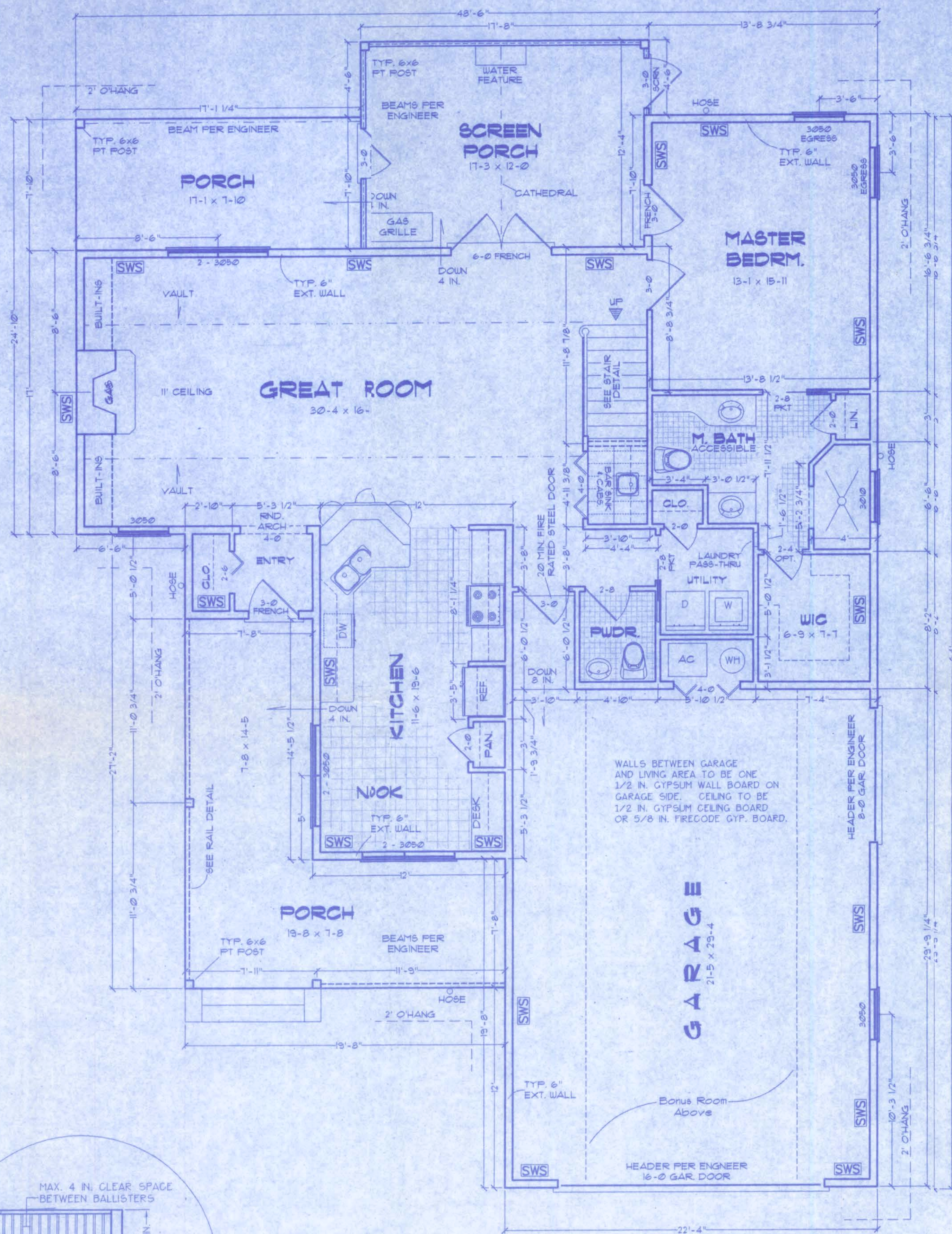
Job No.:

808292
7/9/08

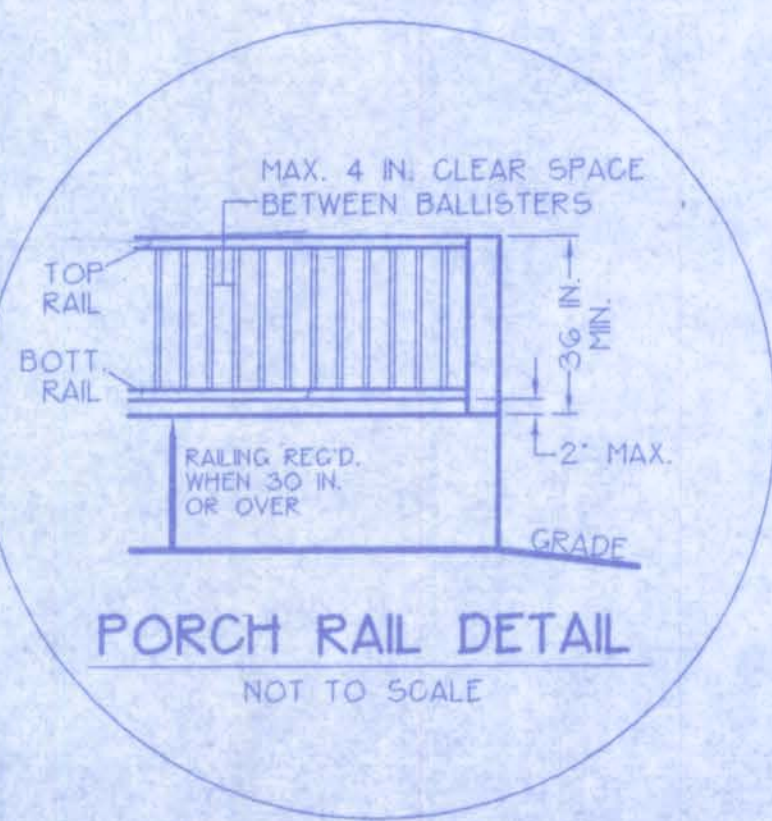
A-2



FILE: 08-008	RYAN RESIDENCE	SHEET: 2 OF 5
DATE: 8/8/08		CAD FILE: 08008
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW. Sagewood Cir. Lake City, FL 32024 Phone: (386) 755-5891	REV:



1st FLOOR PLAN
SCALE: 1/4 IN. = 1 FT.



SWS indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

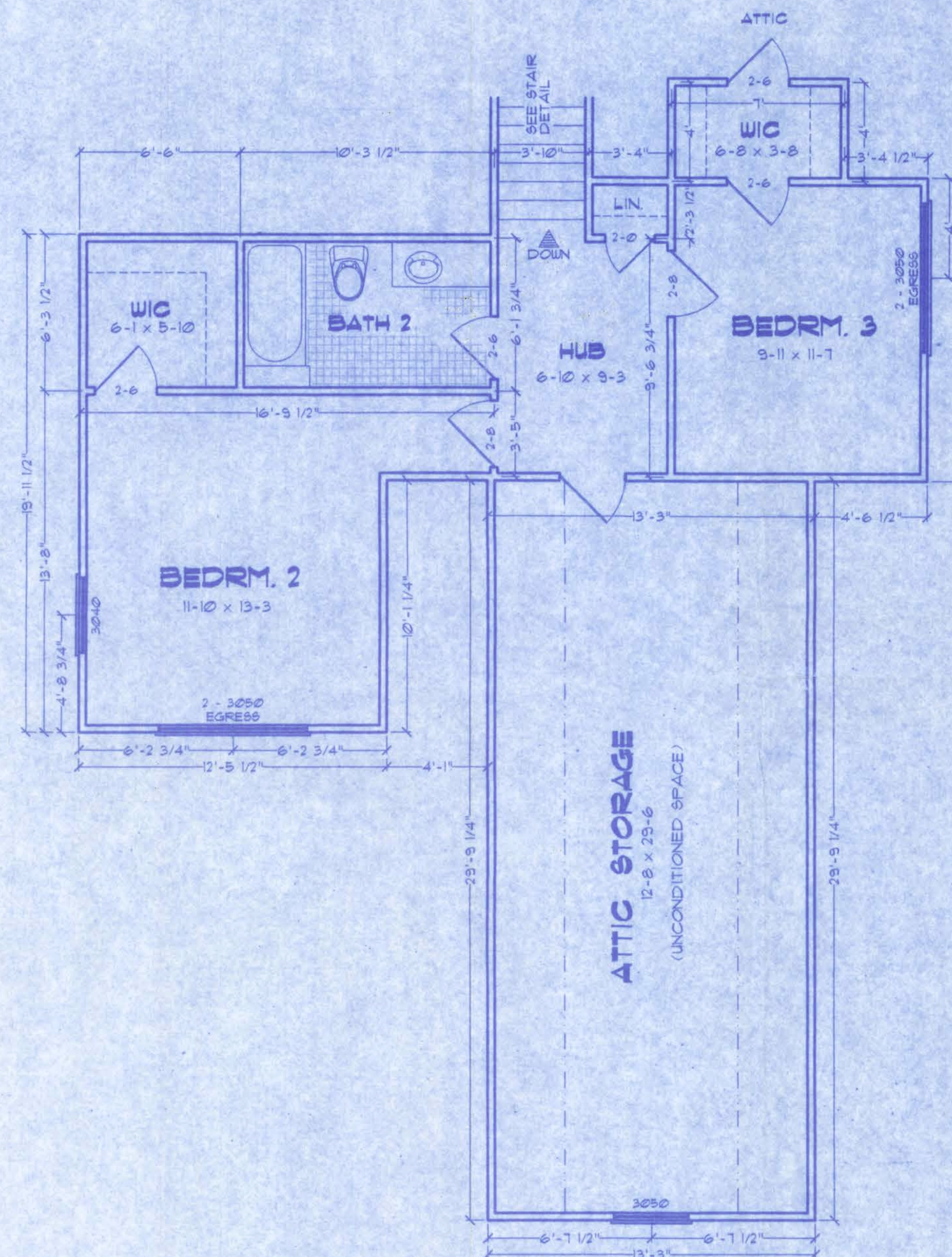
AREA SUMMARY

1st FLOOR CONDITIONED - - - - - 1400 SF
2nd FLOOR CONDITIONED - - - - - 522 SF

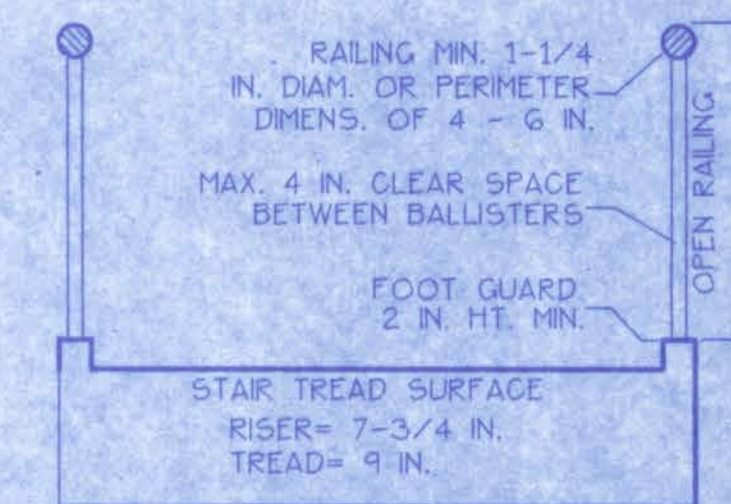
TOTAL CONDITIONED - - - - - 1922 SF

BONUS ROOM - - - - - 327 SF
GARAGE - - - - - 603 SF
FRONT PORCH - - - - - 262 SF
REAR PORCH - - - - - 134 SF
SCREEN PORCH - - - - - 216 SF

TOTAL ROOF - - - - - 3550 SF



2nd FLOOR PLAN
SCALE: 1/4 IN. = 1 FT.



STAIR DETAIL
NOT TO SCALE



A-3

WINDLOAD ENGINEER: Mark Disceway, PE No.53915, POB 866, Lake City, FL 32056, 386-754-5419

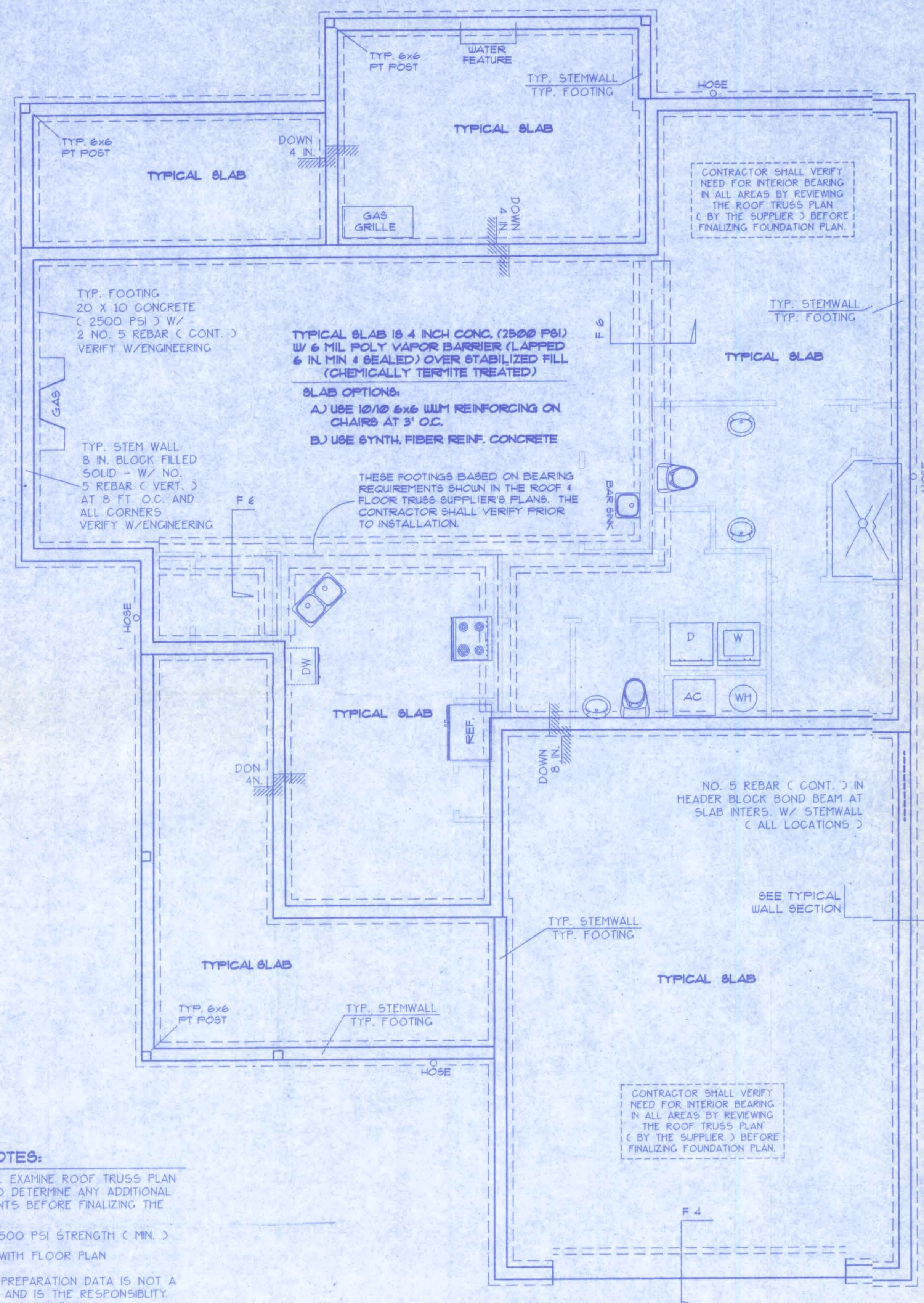
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: _____ Job No.: _____

FILE: 08-008	RYAN RESIDENCE	SHEET: 3 OF 3
DATE: 8/8/08		CAD FILE: 08008
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services 192 SW Sagewood Cir. Lake City, FL 32024 Phone: (813) 755-5891	REV:
CHECK: T A D		REV:

808292
7/9/08

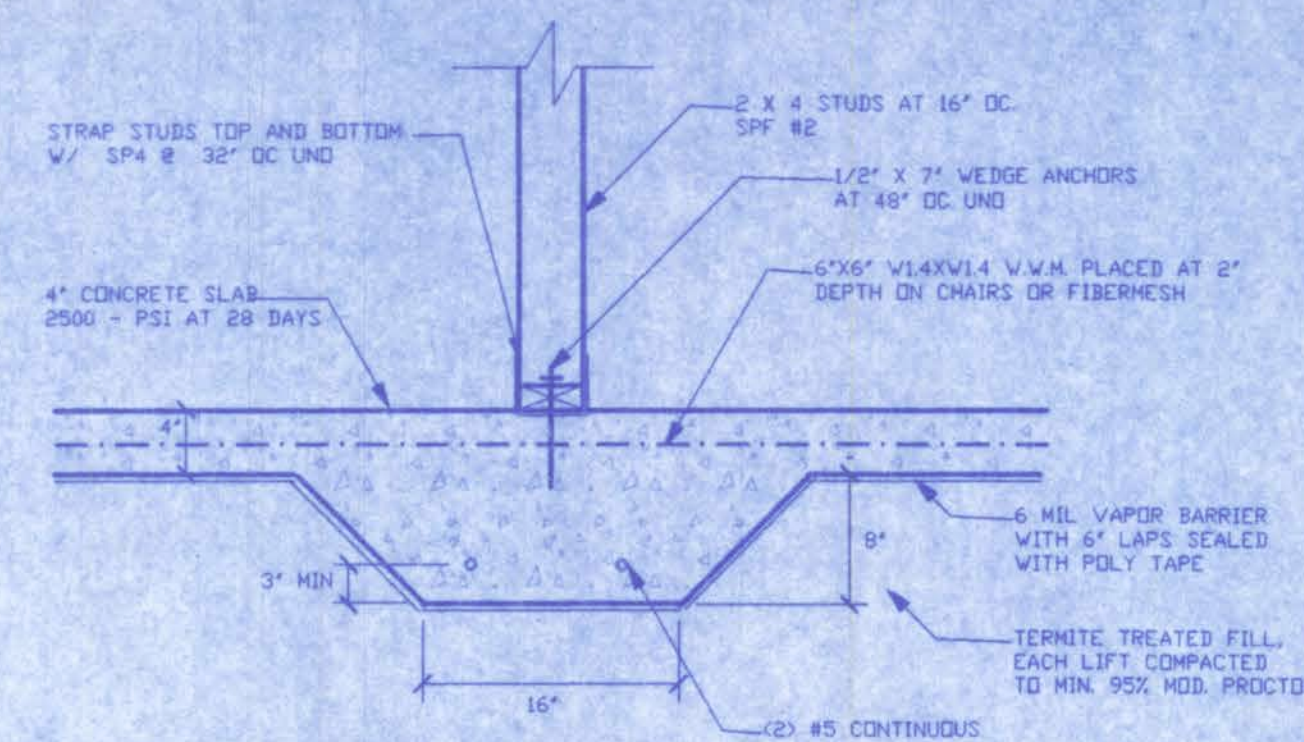


FOUNDATION NOTES:

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.

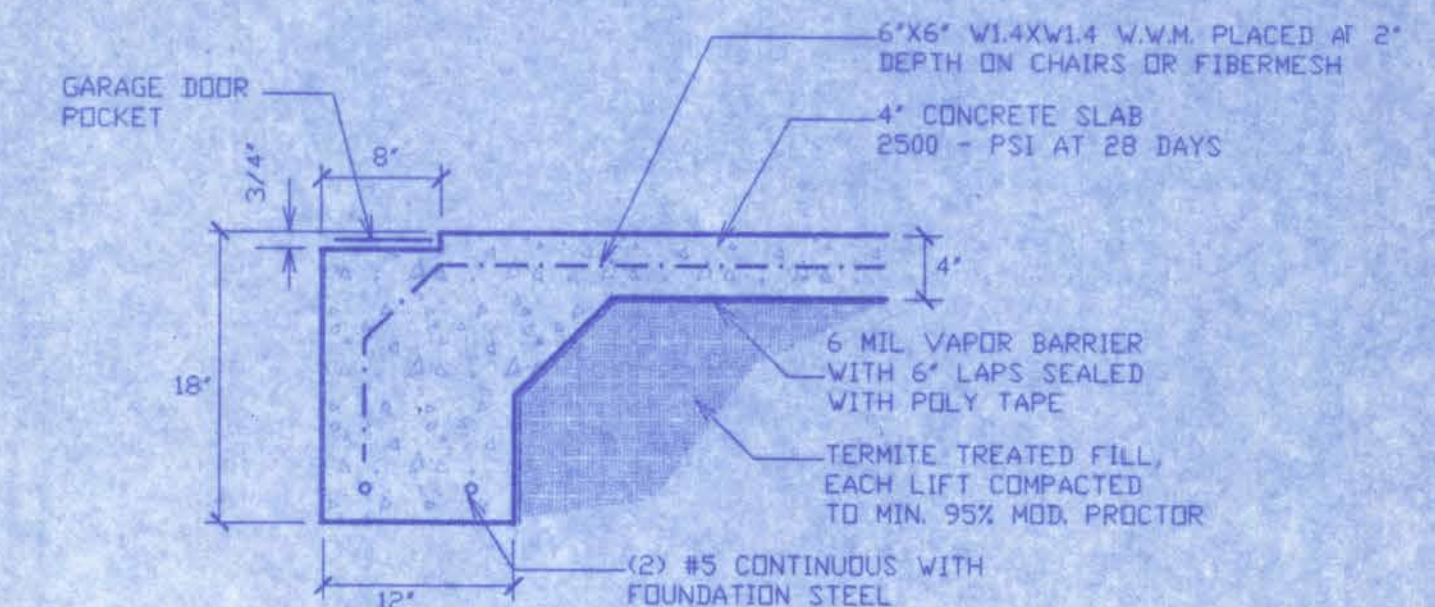
FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.



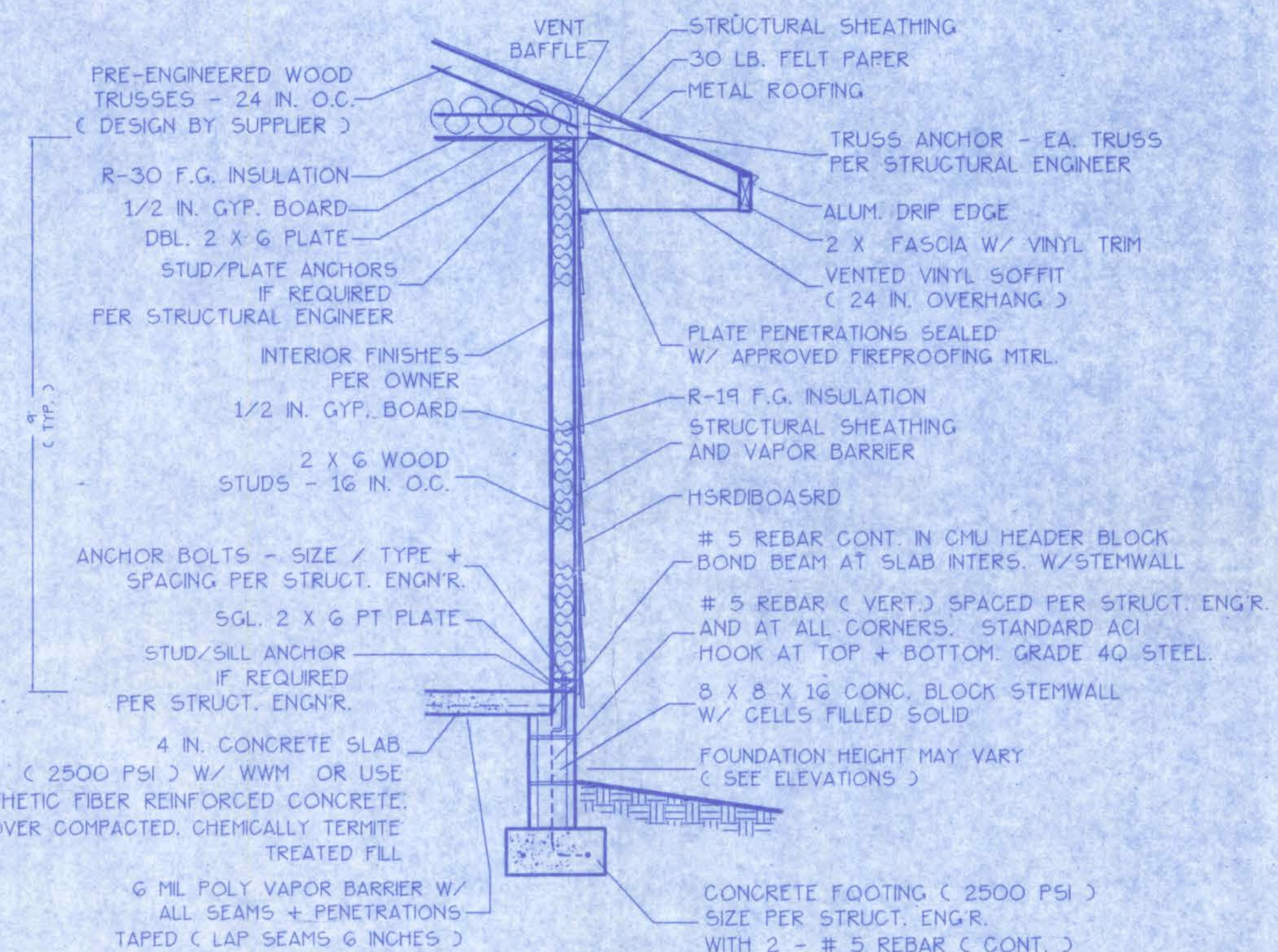
F6 - INTERIOR BEARING FOOTING

SCALE: 1" = 1'-0"



F4 - GARAGE DOOR POCKET

SCALE: 1" = 1'-0"



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

DESIGN WALL SECTION

NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.



A-4

WINDLOAD ENGINEER: Mark Diasoway, PE No.53915, POB 888, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: _____ Job No.: 808292

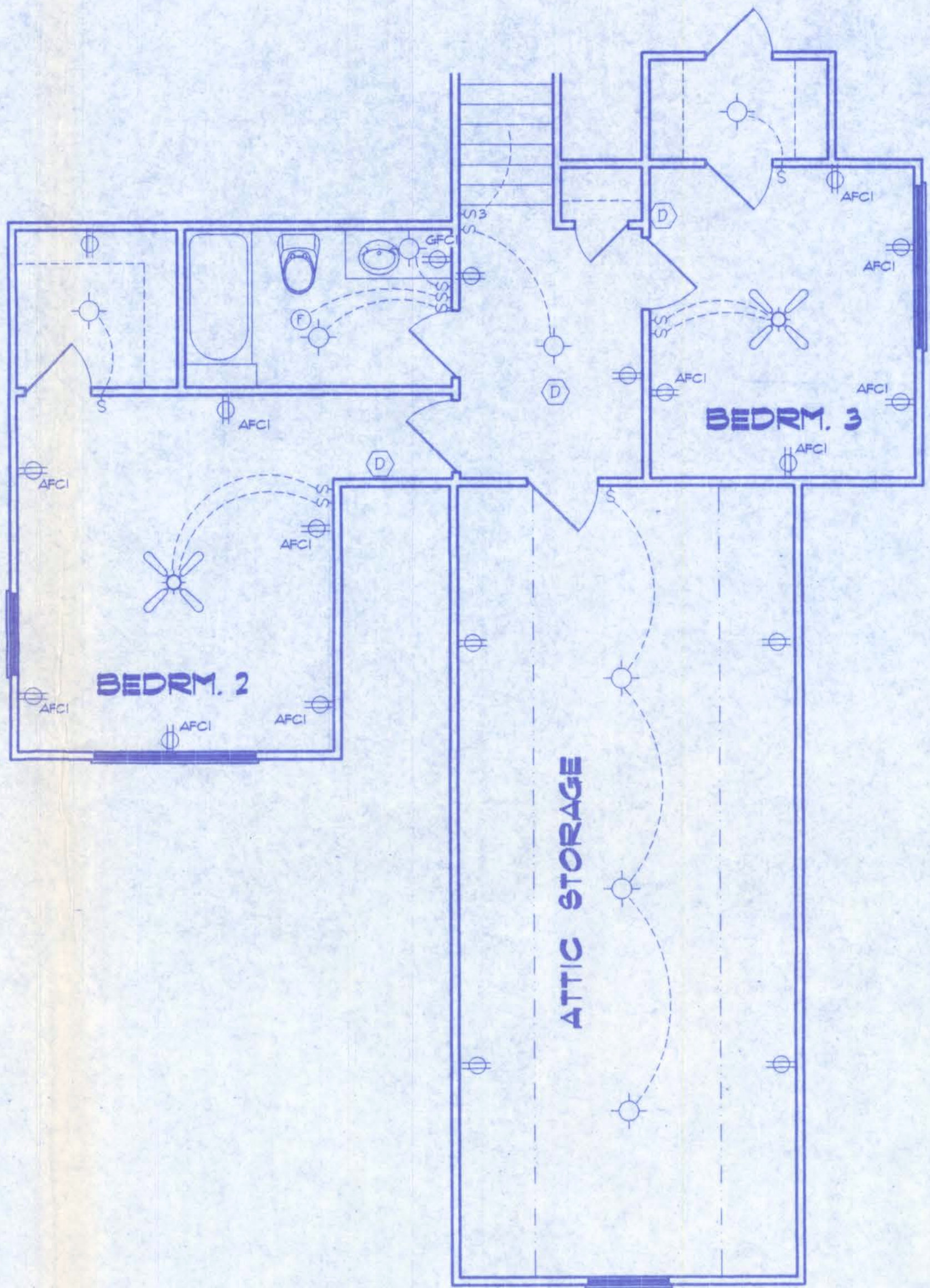
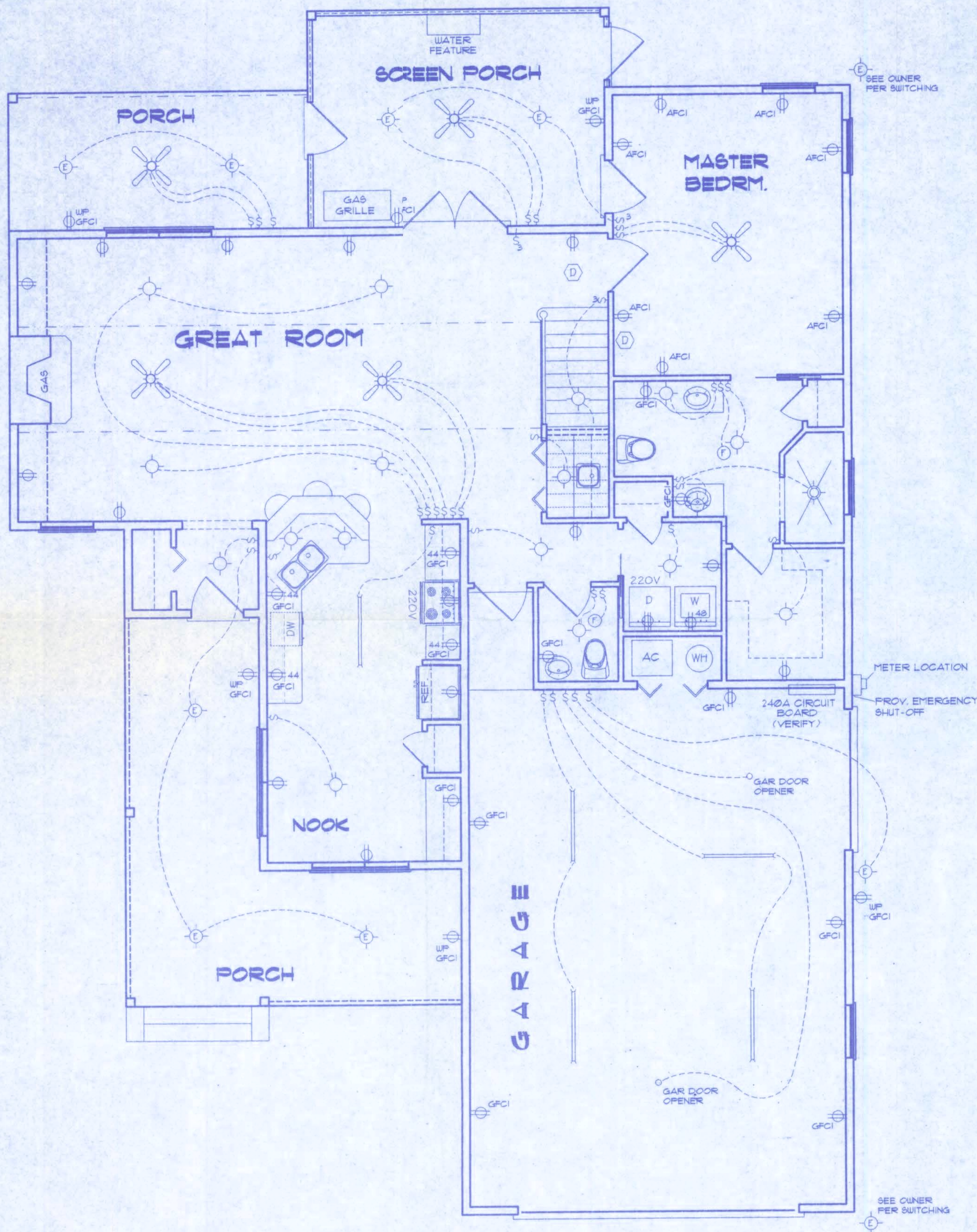
FILE: 08-008	RYAN RESIDENCE	SHEET: 4 OF 5
DATE: 8/8/08		CAD FILE: 08008
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	192 SW Sagewood Ct., Lake City, FL 32024 Phone: (386) 755-5891	REV:

ELECTRICAL PLAN NOTES

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

ELECTRICAL SYMBOL LEGEND

- FLUORESCENT LIGHTING FIXTURE.
- CEILING LIGHT FIXTURE
- EXTERIOR LIGHTING FIXTURE
- LIGHT SWITCH.
- THREE-WAY SWITCH.
- 110 V. DUPLEX OUTLET.
- SPECIAL HEIGHT 110 V. DUPLEX OUTLET
- GROUND FAULT CIRC. OUTLET
- ARC FAULT CIRC. OUTLET
- 110 V. SINGLE RECEPTACLE OUTLET.
- 220 VOLT OUTLET (4 WIRE)
- FAN LOCATION (CEILING)
- FAN LOCATION (EXHAUST)
- SMOKE + CARBON MONOXIDE DETECTOR



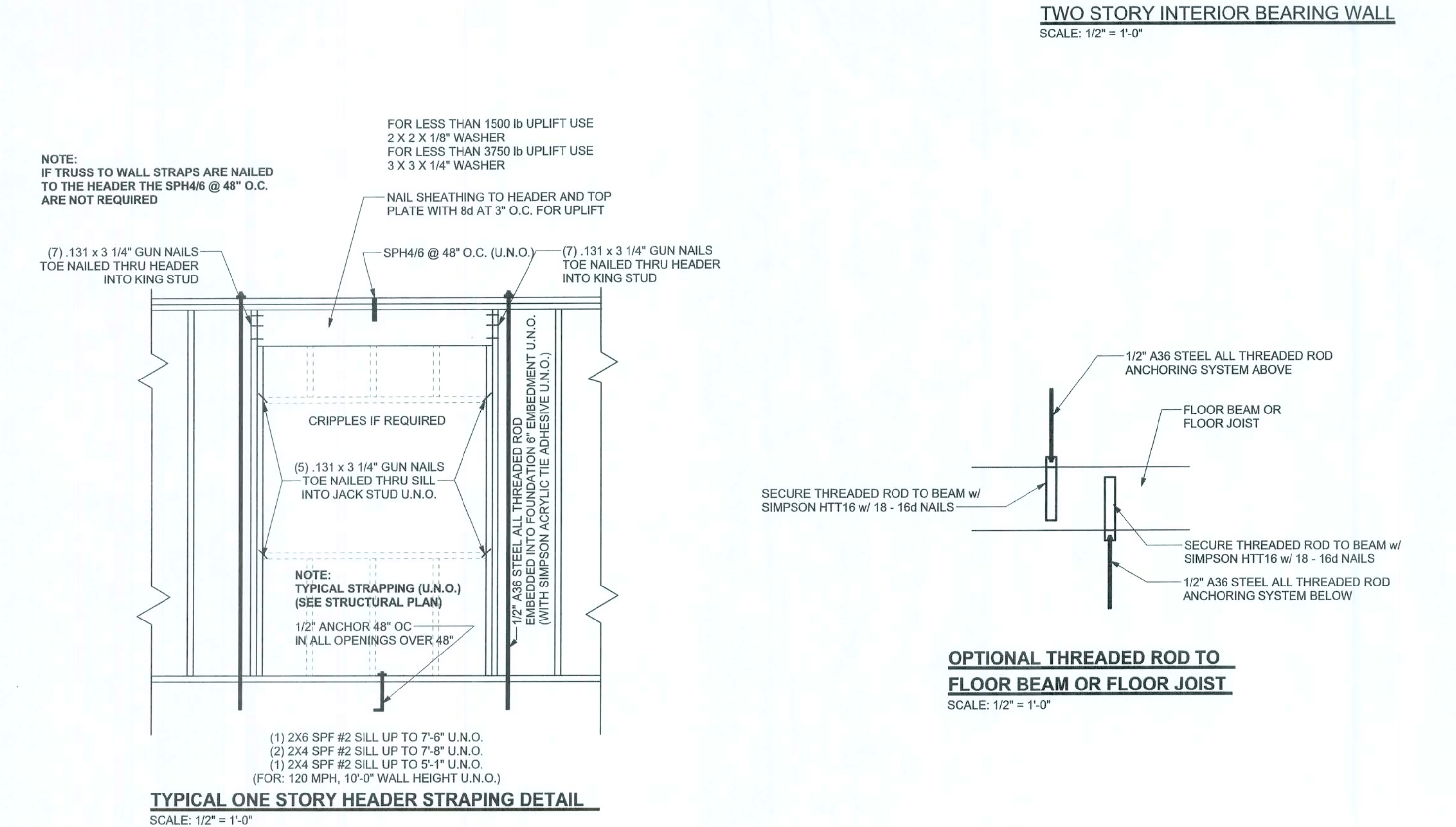
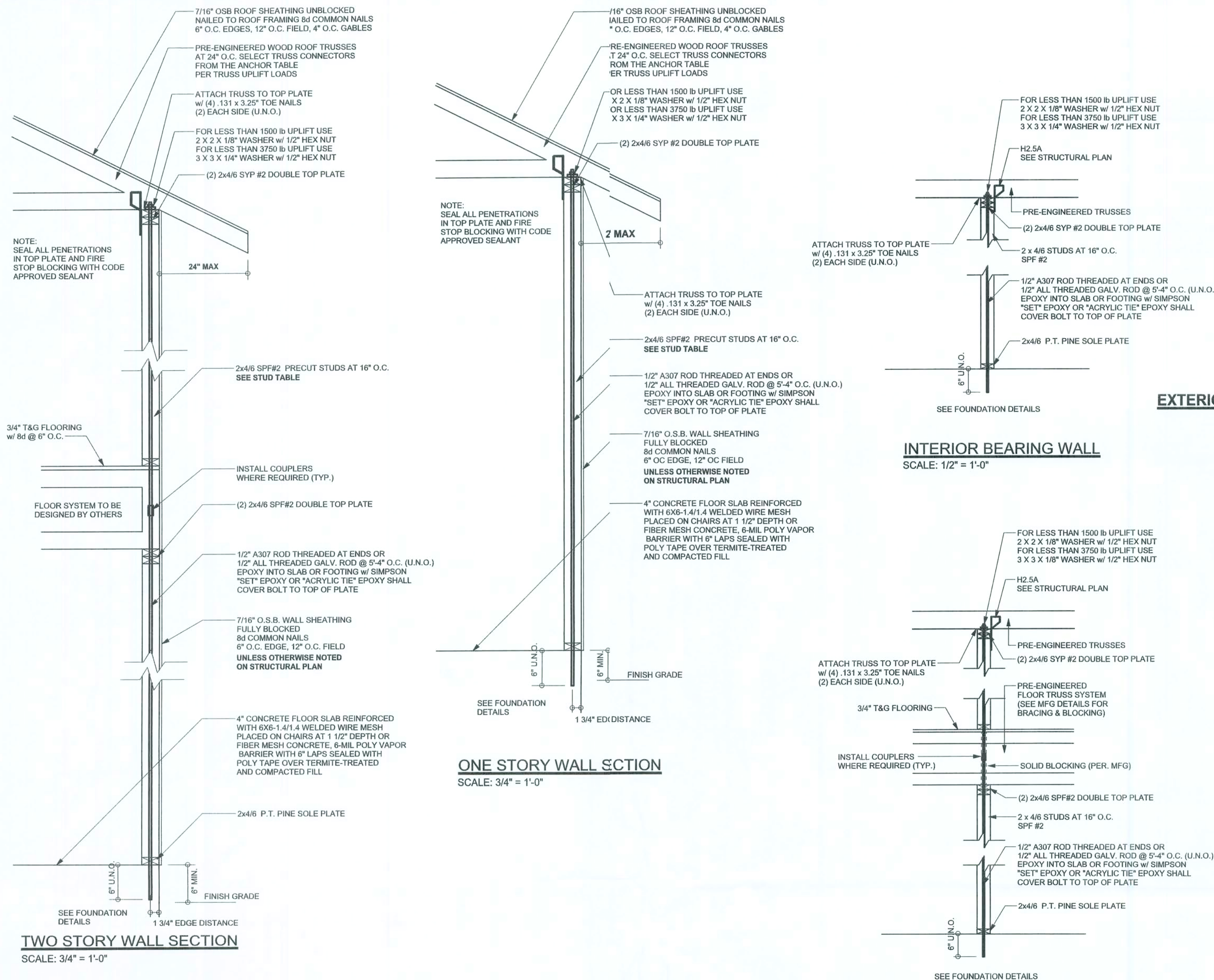
ELECTRICAL PLAN

NOT TO SCALE

A-5

FILE: 08-008	RYAN RESIDENCE	SHEET: 5 OF 5
DATE: 8/8/08		CAD FILE: 08008
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:
CHECK: T A D	142 SW. Greenwood Cir., Lake City, FL 32024 Phone (386) 755-5891	REV:





GRADE & SPECIES TABLE

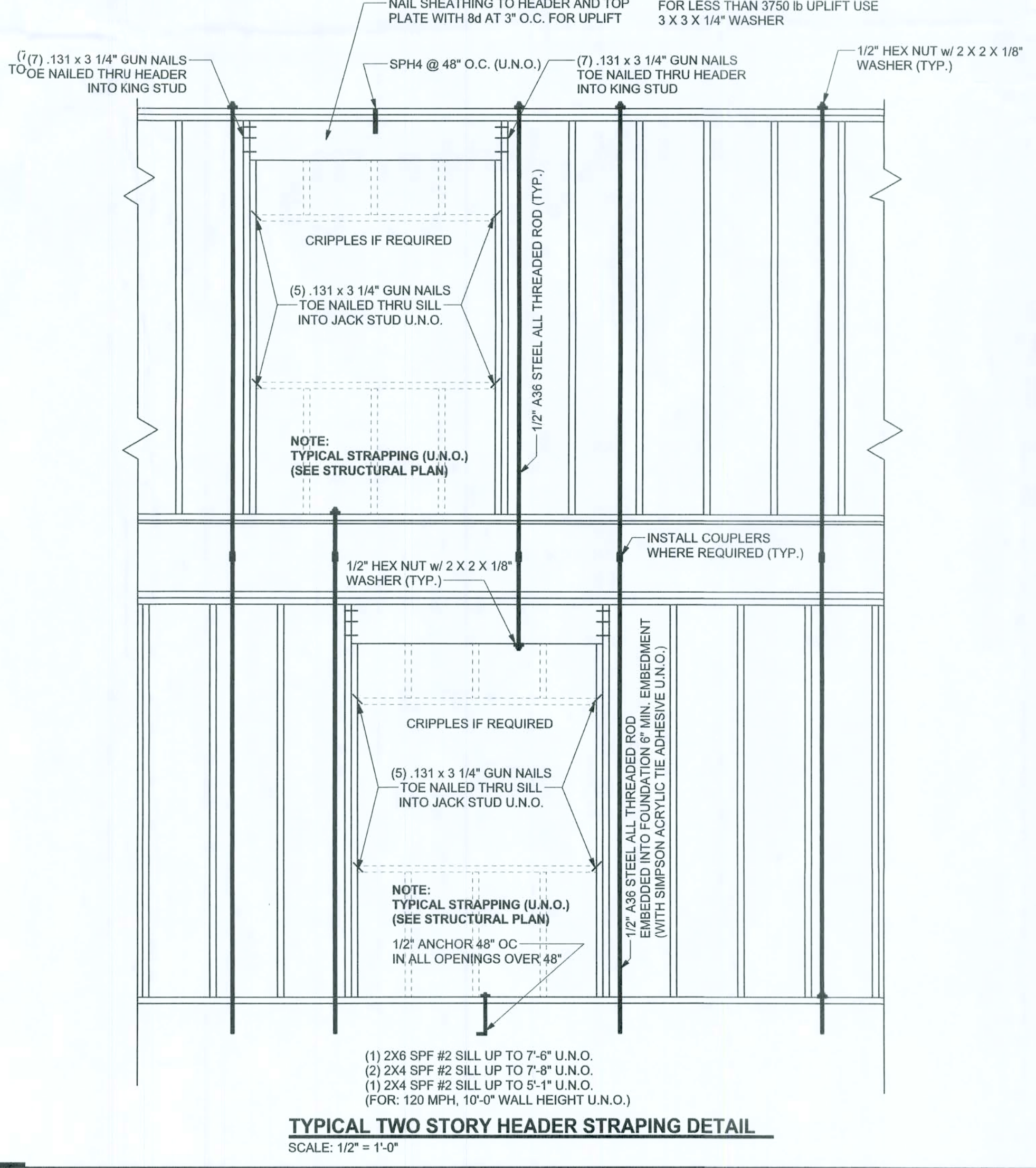
		Fb (psi)	E (10 ⁶ psi)
2"x8	SYP #2	1200	1.6
2"x10	SYP #2	1050	1.6
2"x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT	
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT	
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT	
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT	

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS: 110 MPH EXPOSURE & STUD SPACING SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" 3" O.C. x 0.85 = 13.6" O.C.

NOTE:
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 800	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2-HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d	4-10d	
< 455	< 420	SSP SINGLE SILL PLATE	1-10d	4-10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d	8-10d	
< 825	< 600	DSP SINGLE SILL PLATE	2-10d	8-10d	
< 885	< 760	SP4		6-10d, 1 1/2"	
< 1240	< 1065	SPH4		10-10d, 1 1/2"	
< 885	< 760	SP6		6-10d, 1 1/2"	
< 1240	< 1065	SPH6		10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LT119	8-16d		1/2" AB
< 2310	< 2310	LT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO VERIFY UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 41/8" EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET FOUNDATION LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT WWW OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 315-96, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS @ 131d, 1/2" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS HAVING EVIDENCE STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 Other	-	-40.6	-40.6
3	19.9 - 25.5	18.1	-21.8
3 Other	-	-68.3	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6

Doors & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft R2)		
8x7 Garage Door	19.5	-22.9
16x7 Garage Door	18.5	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

NO.	DESCRIPTION

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Dsosway, P.E. No. 53812, Woburn, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Shaded dimensions supersede scaled dimensions. Refer all questions to Mark Dsosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specific location.

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P.E. 53812
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Joi Ryan
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PRINTED DATE:
September 08, 2008

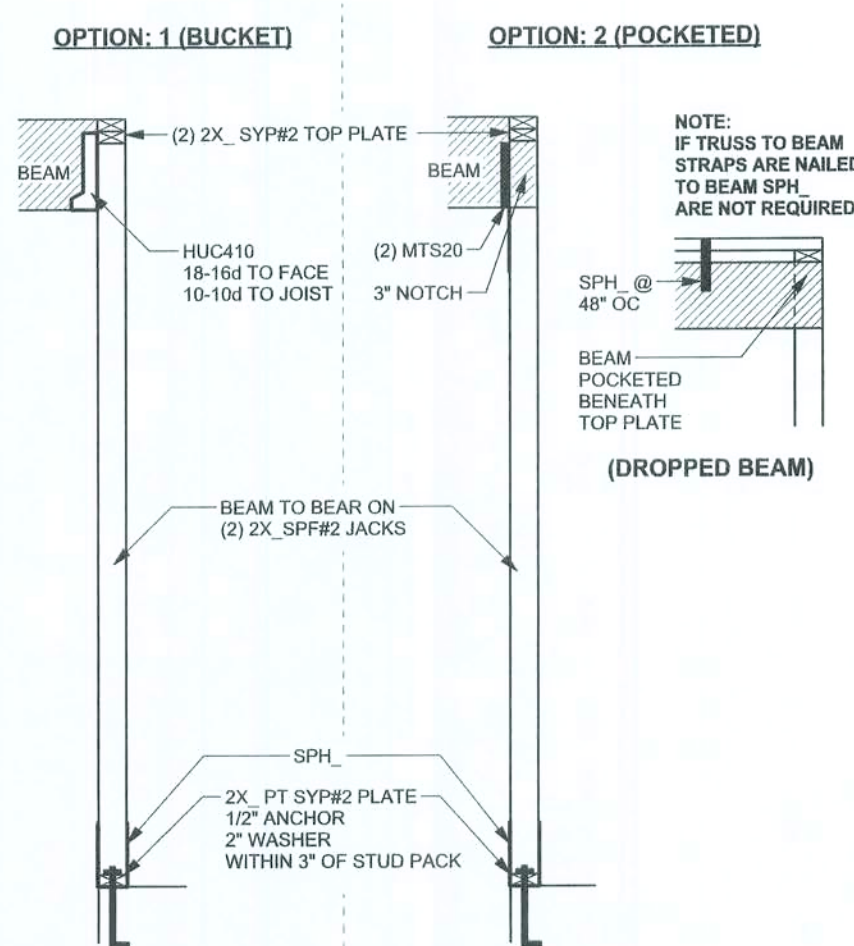
DRAWN BY:
David Dsosway

FINALS DATE:
08Sep08

JOB NUMBER:
838292

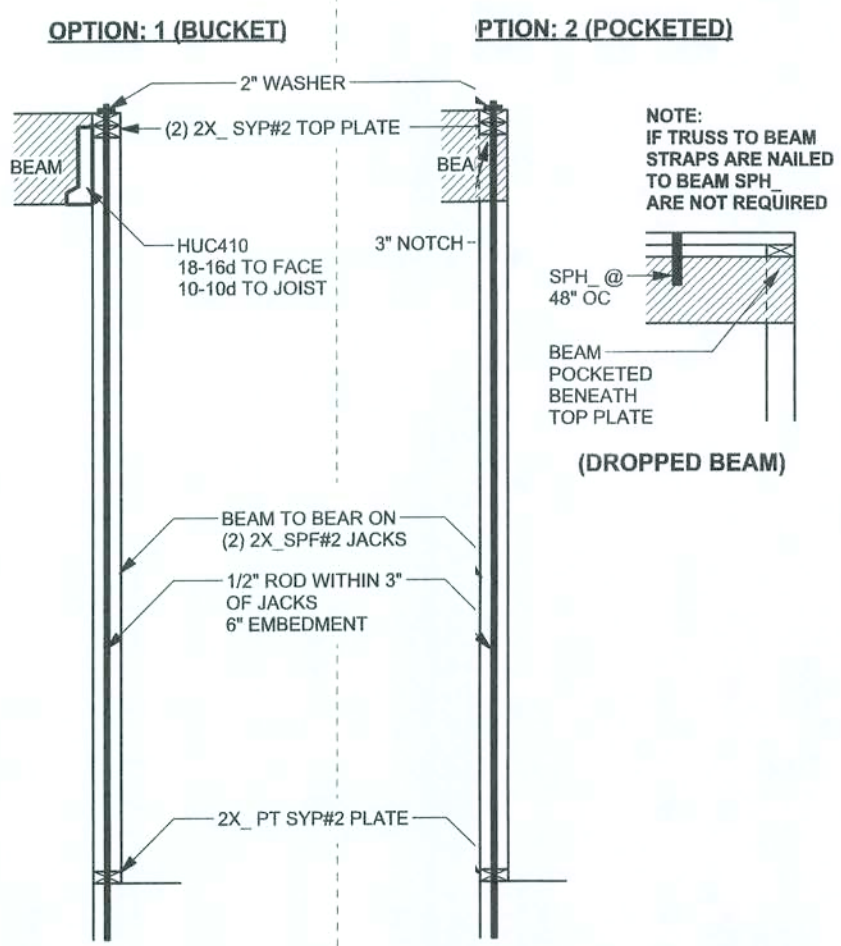
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COLUMBIA COUNTY BUILDING DEPARTMENT
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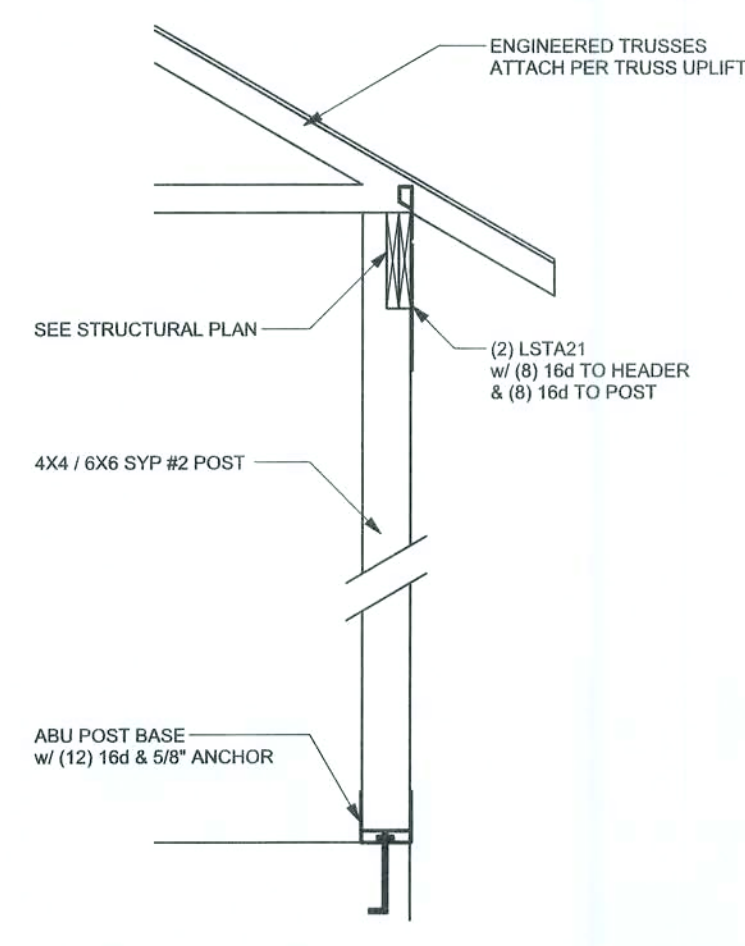
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

ALLOWABLE UPLIFT:
1285 LB



(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS

ALLOWABLE UPLIFT:
1776 LB



(TYP.) PORCH POST
ONE STORY WOOD

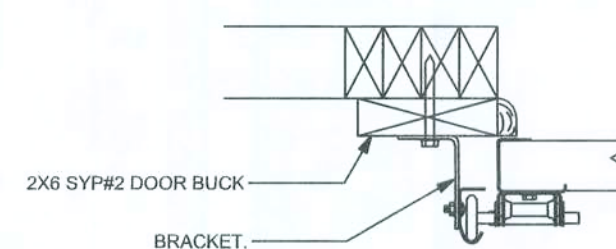
ALTERNATE CONNECTION WHERE
ROD CANNOT BE PLACED IN WALL
ONE STORY WOOD FRAME w/ RODS

ALLOWABLE UPLIFT:
1900 LB

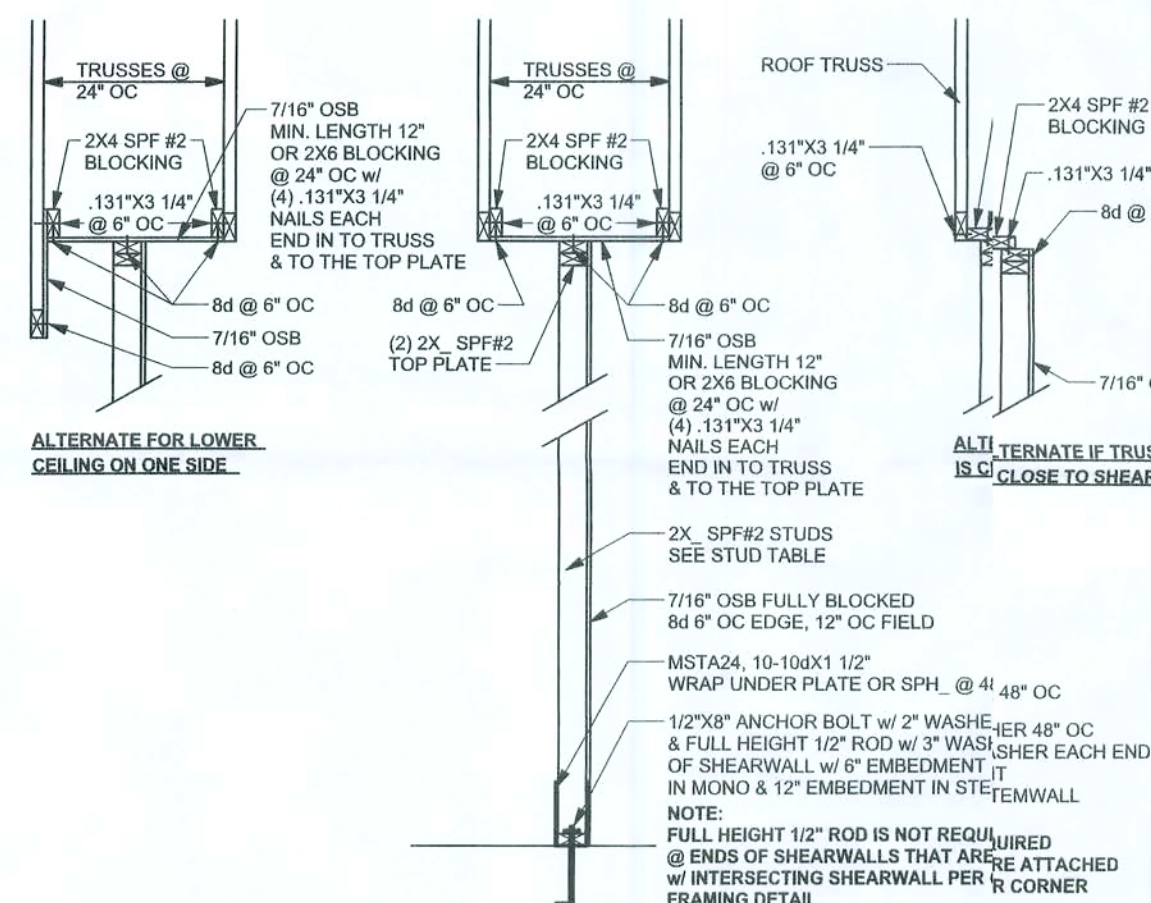
2X6 SYP#2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16" NAILS OR (2) ROWS OF .131"x3 1/4" ON PER TABLE BELOW:

DOOR WIDTH	3/8"x4" LAG	16" STAGGER	(2) ROWS OF .131"x3 1/4" NAILS
8' - 10'	24" OC	6" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	18" OC	3" OC	3" OC

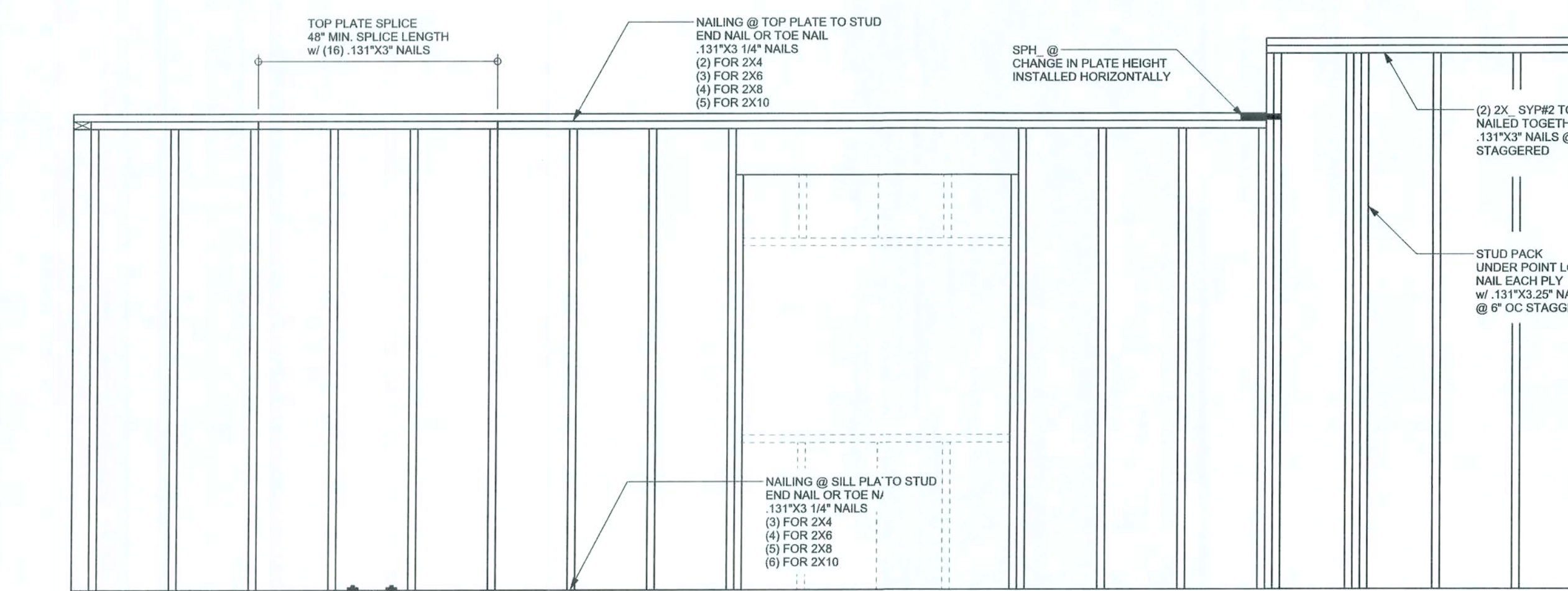


(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

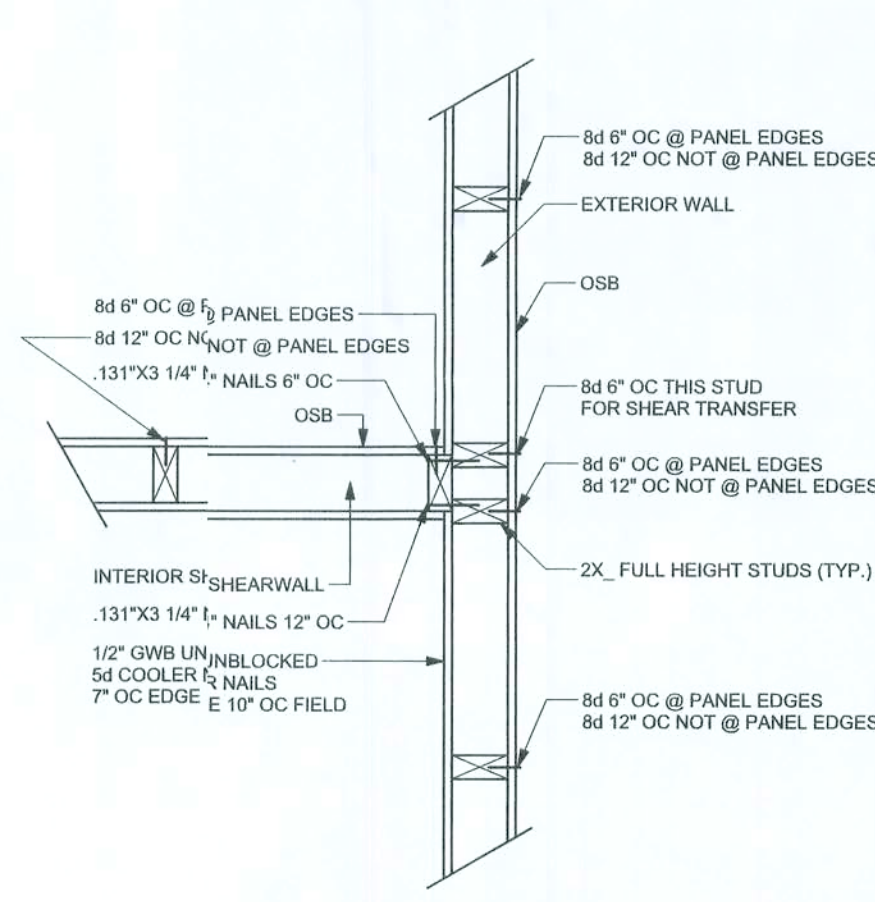


(NOTE: HAVE TRUSS DESIGNER LOAD TRUSS FOR 400 PLF DRAG LOAD)

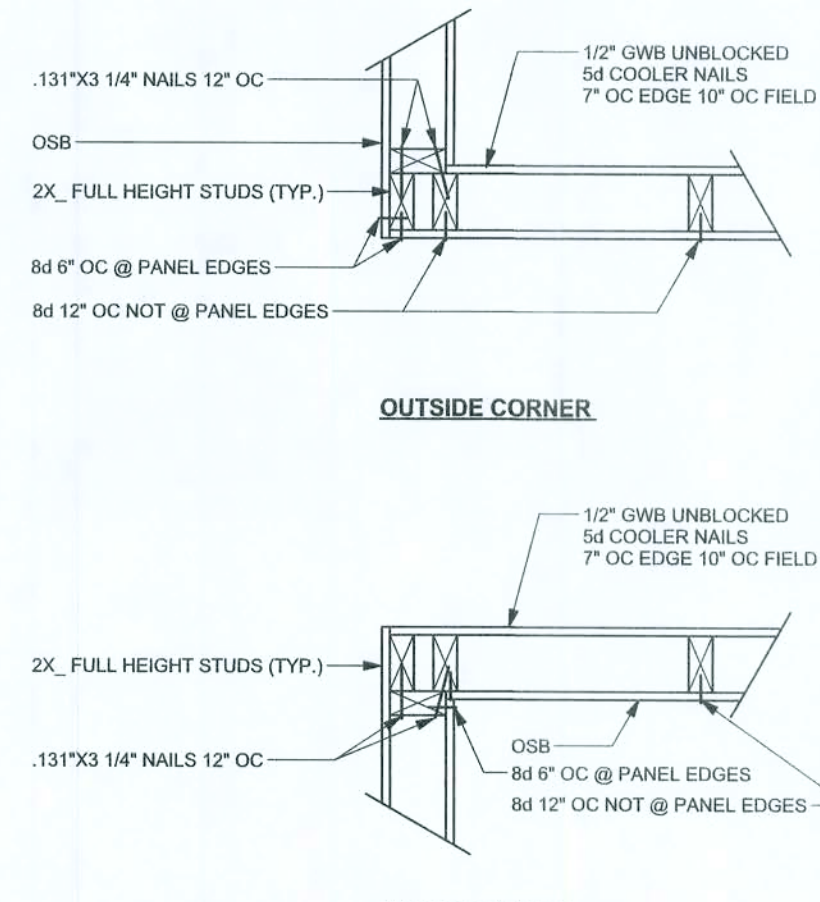
INTERIOR SHEAR WALL
ONE STORY WOOD FRAME w/ STRAPS & AB



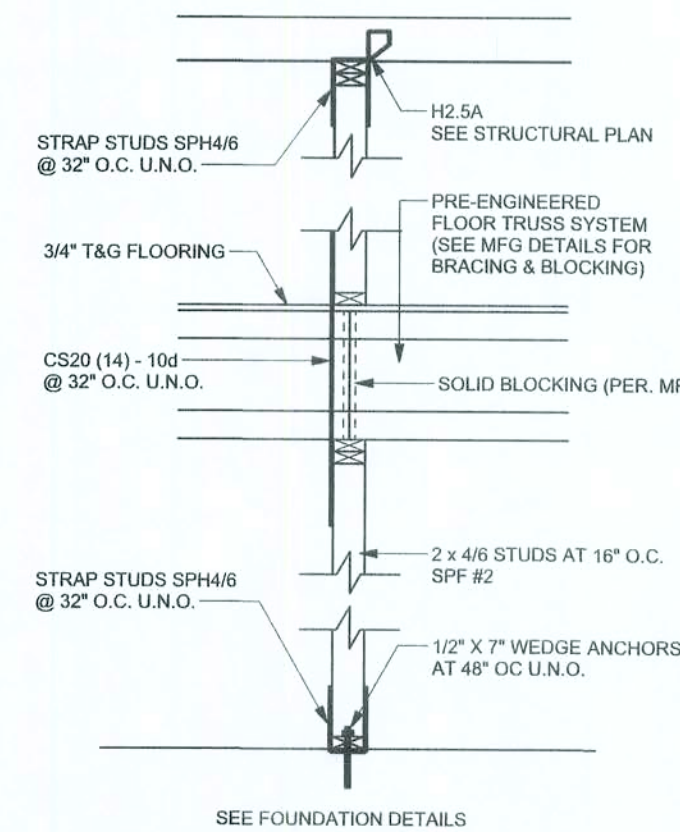
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



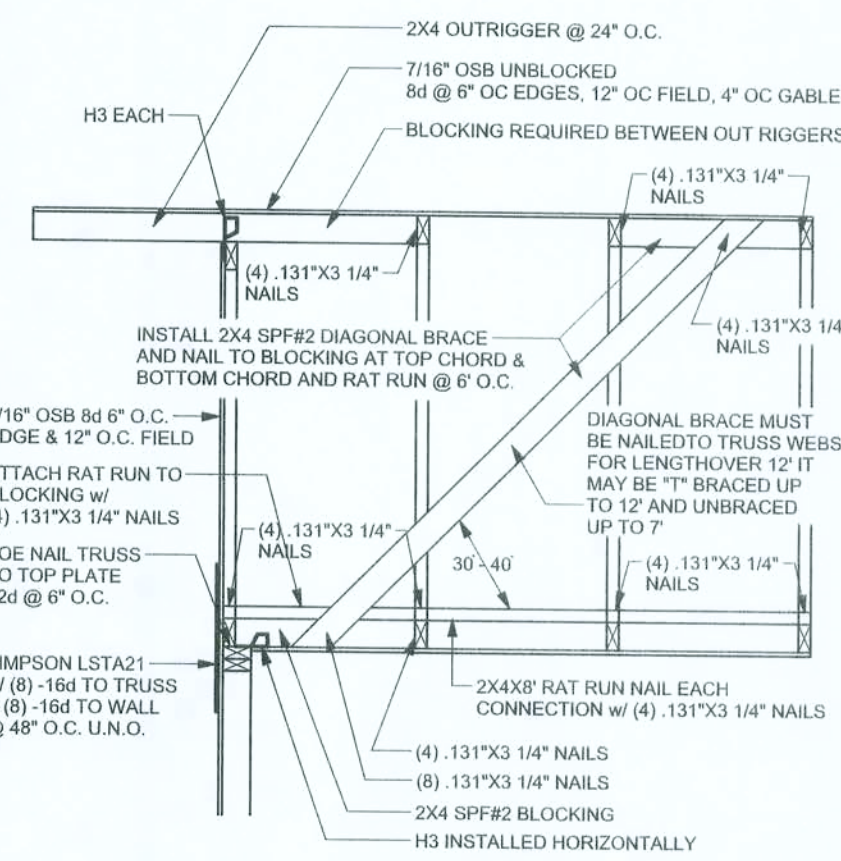
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



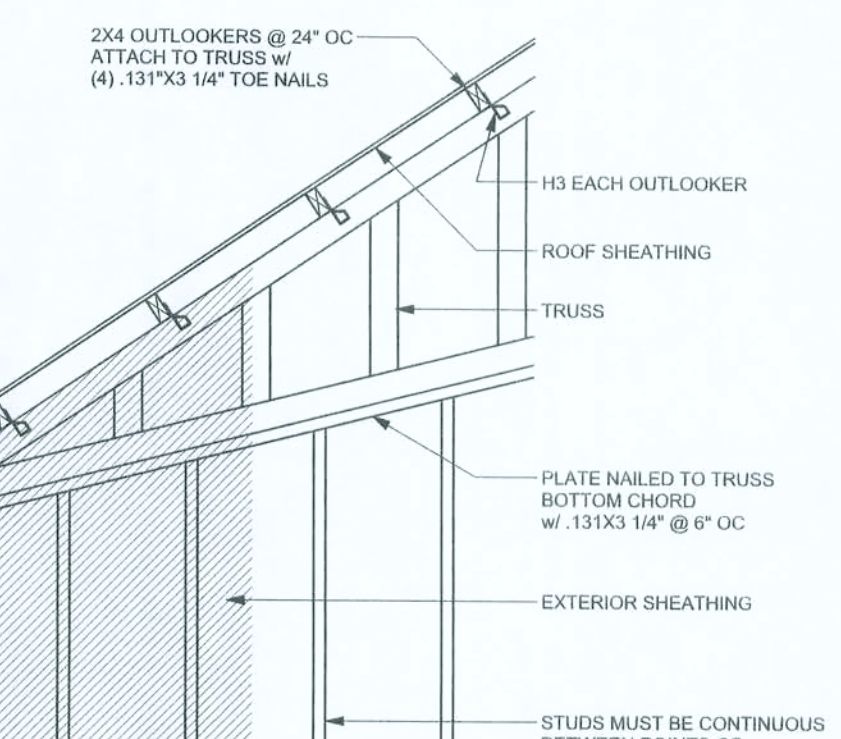
(TYP.) CORNER FRAMING
WOOD FRAME



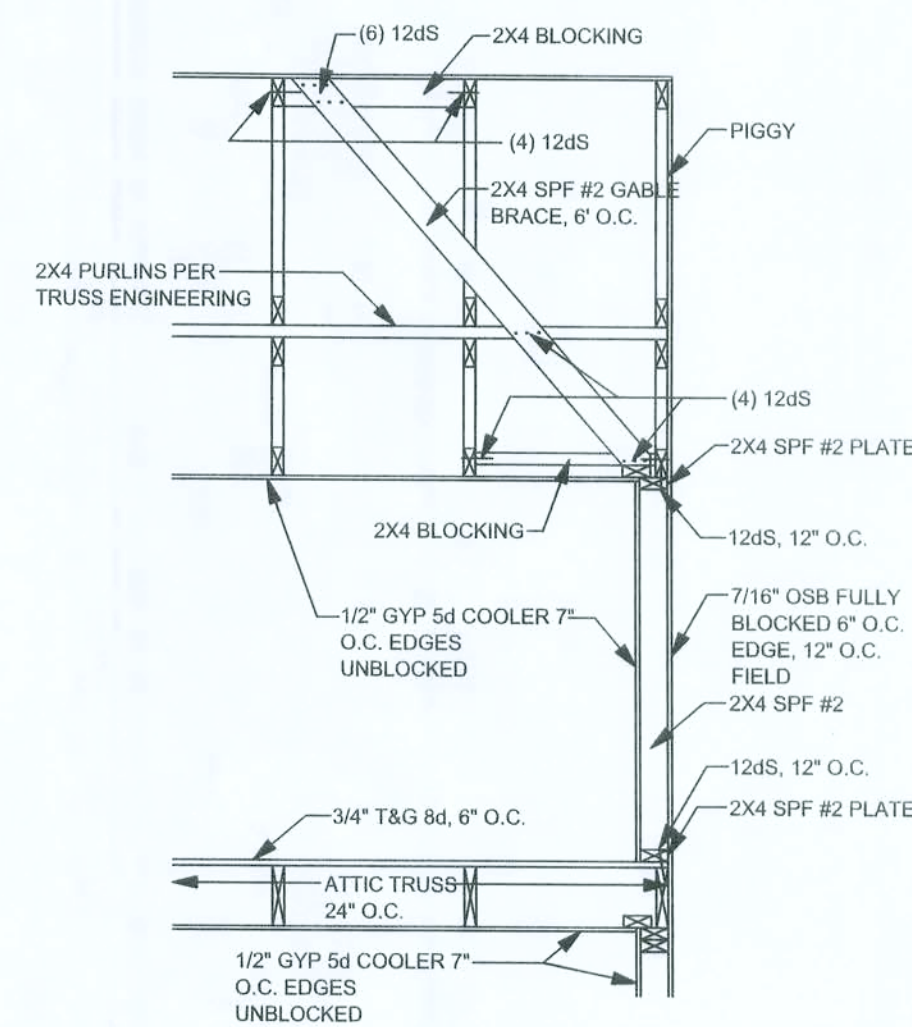
(ALT.) 2 STORY INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



(TYP.) GABLE BRACING DETAIL
WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



W67 - BONUS ROOM / GABLE END BRACING
SCALE: 1/2" = 1'-0"

REVISIONS

SOFTPLAN
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER: Mark Disisway,
PE No. 53915, POB 86, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Slated dimensions supersede scaled dimensions. Refer all questions to Mark Disisway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R01.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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PRINTED DATE:
September 08, 2008

DRAWN BY: STRUCTURAL BY:
David Disisway

FINALS DATE:
08Sep08

JOB NUMBER:
80292

DRAWING NUMBER

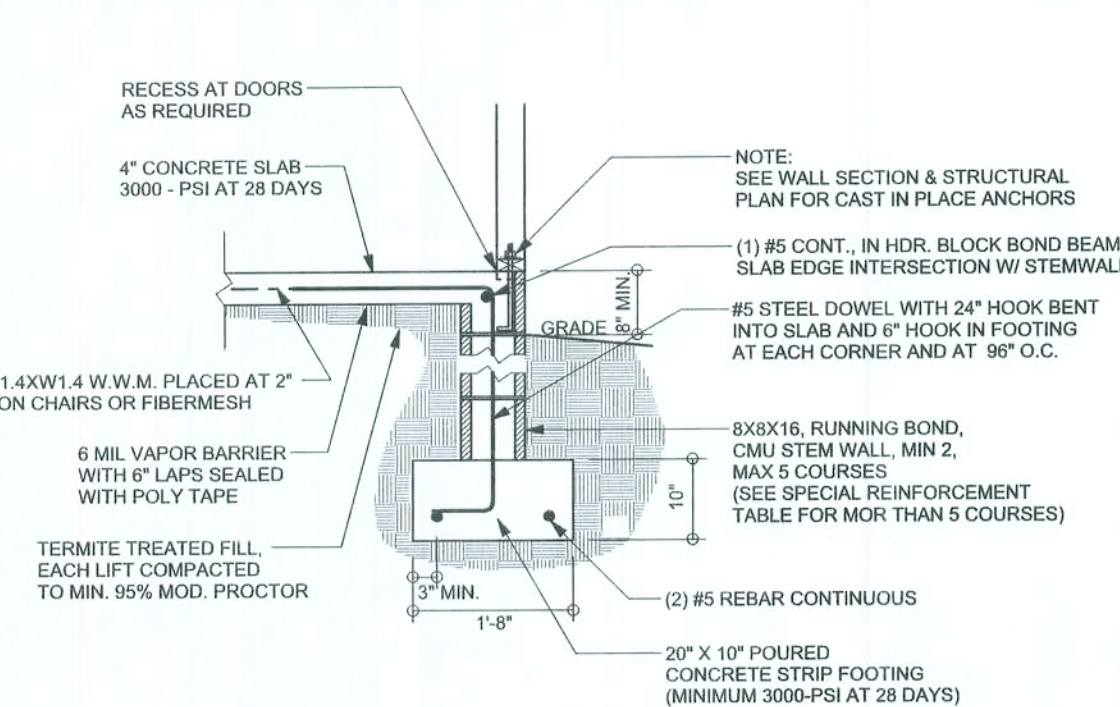
S-1.1

OF 4 SHEETS

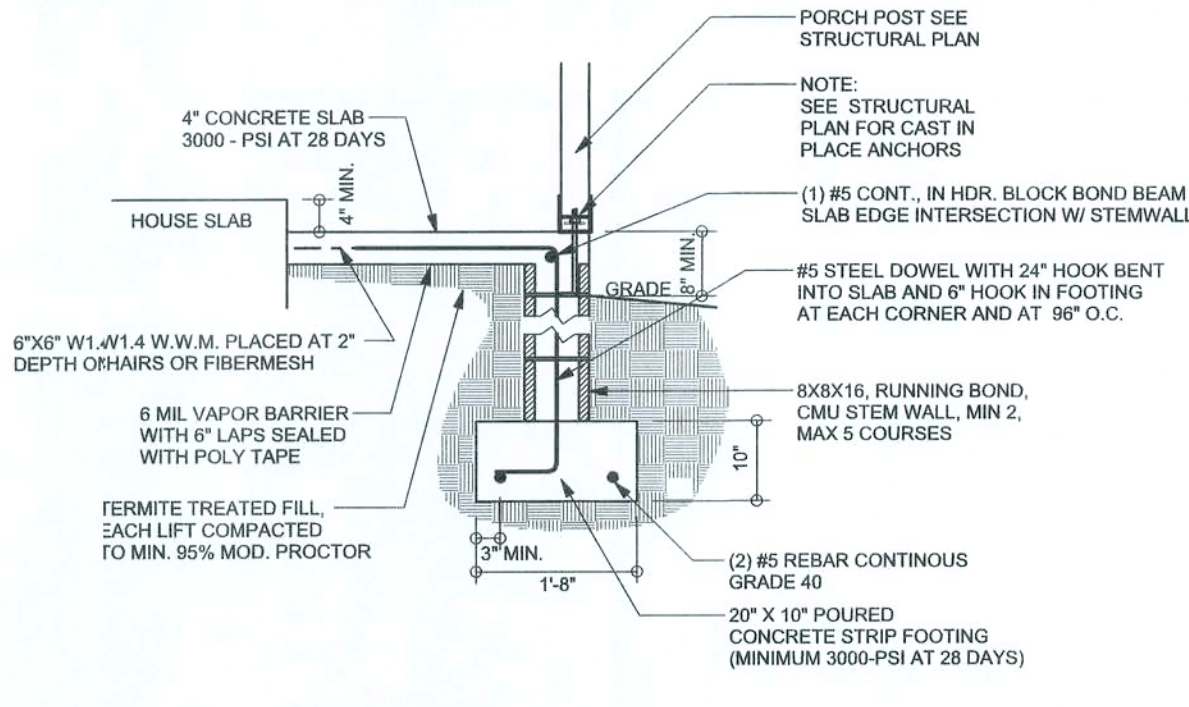
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REVISIONS

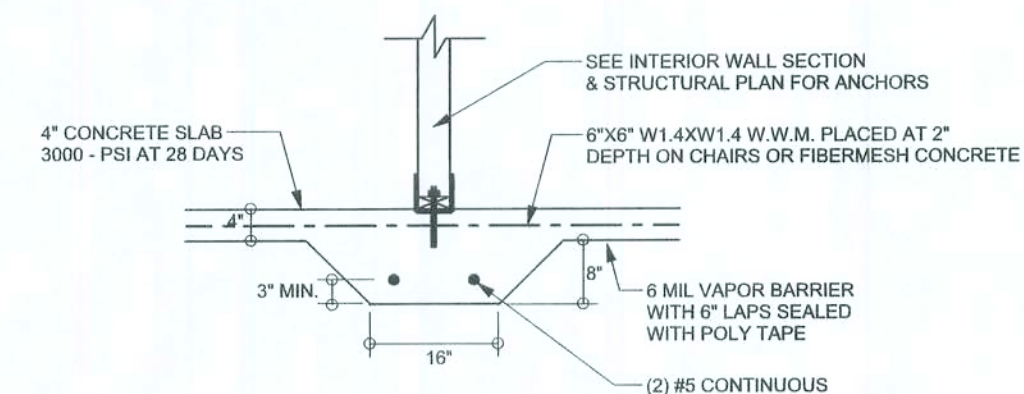
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



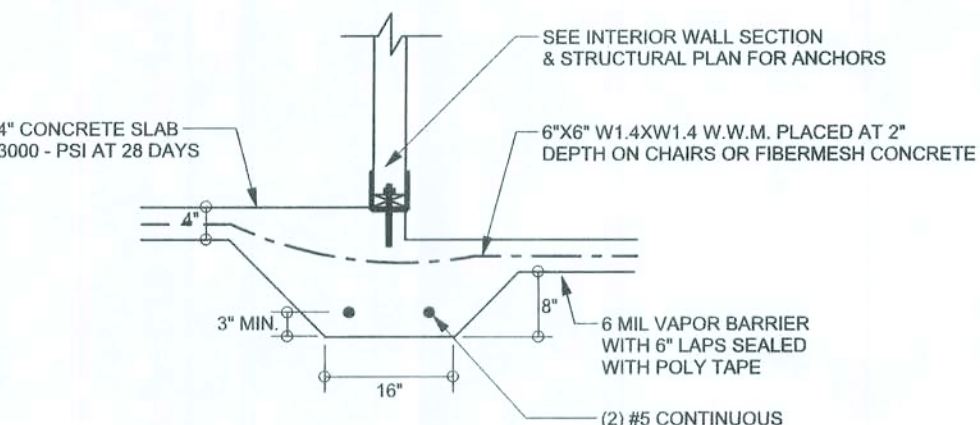
F9
S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



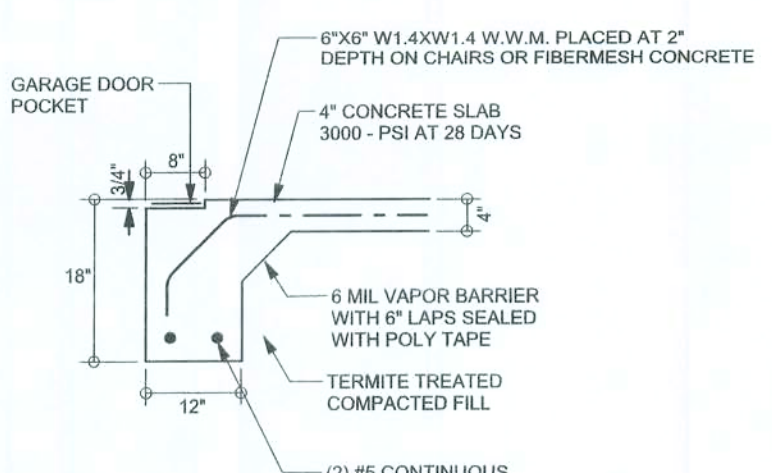
F12
S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



F2
S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3
S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

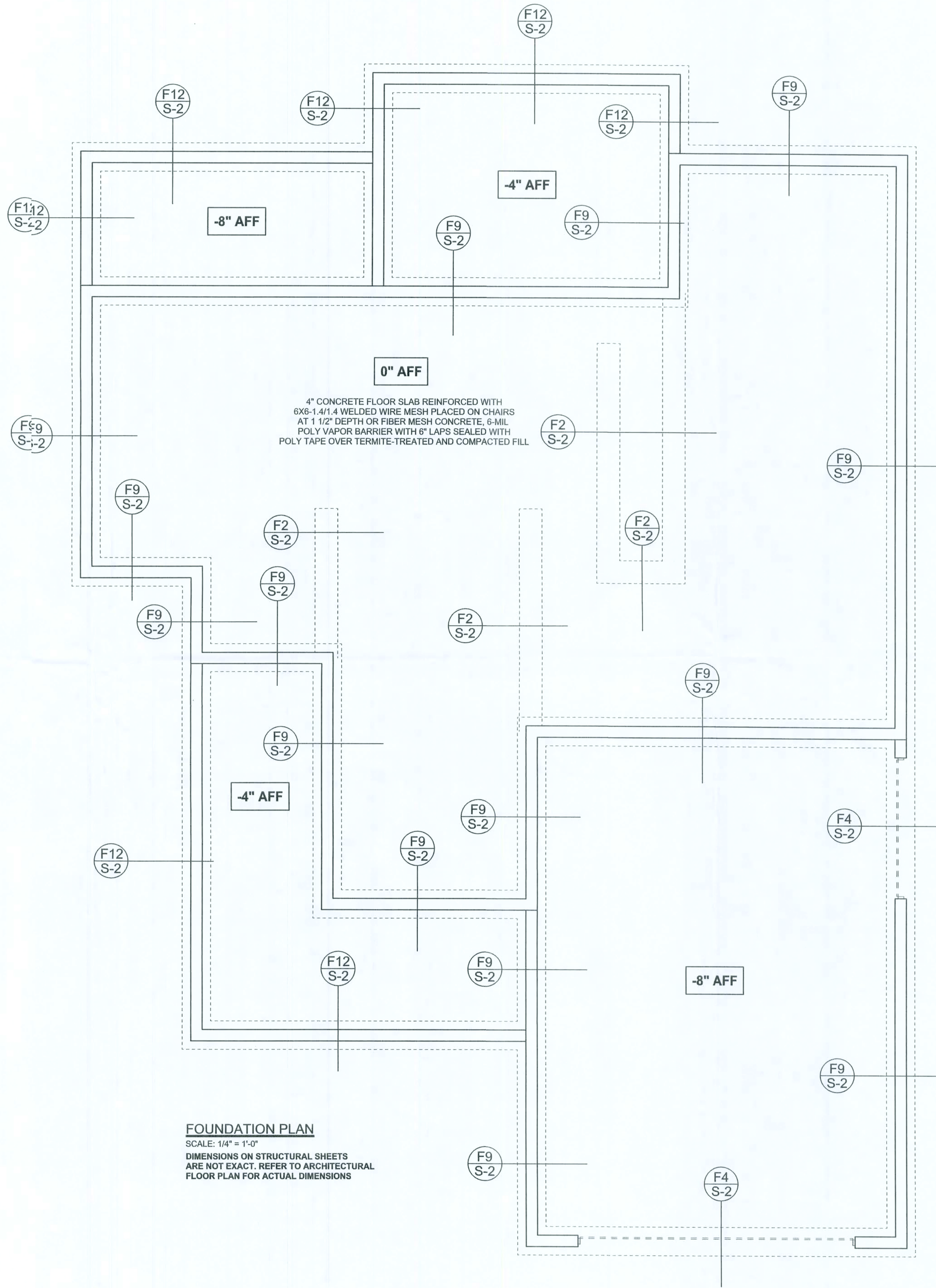


F4
S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 9' high, add Durowall ladder reinforcement at 16" O.C. vertically or a horizontal band beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Dsoisway
PE No. 53915, POB88, Lake City, FL
32056, 386-754-549

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
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permission and consent of Mark Dsoisway.

CERTIFICATION: hereby certify that I have
examined this plan and that the applicable
portions of the plan relating to wind engineering
comply with section R301.2.1, Florida building
code residential 204, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

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PRINTED DATE:
September 08, 2008

DRAWN BY: David Dsoisway

FINAL DATE:
08Sep08

JOB NUMBER:
808292

DRAWING NUMBER
3-2

OF4 SHEETS





SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
SN-3	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
SN-4	PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWING. LATERAL BRACING IS TO BE RESTRAINED PER BC311-0, BC311-B1, BC311-B2, & BC311-B3. BC311-B1, BC311-B2, & BC311-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE TRUSS PACKAGE

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS
IBW	2ND FLOOR INTERIOR BEARING WALLS

CONTINUOUS FRAMING FLOOR
TO CEILING & "X" BRACE ABOVE

	TRANSVERSE	LONGITUDINAL
ACTUAL	18061 LBF	17865 LBF

Main Wind Force Resisting System Wind Loads (FBC2004 Table 1609.6A)										
Transverse (Perpendicular to ridge)						Longitudinal (Parallel to ridge)				
	Area	Exd	Intsr			Area	Exd	Intsr		
Roof Area	925 ft2	14.8 psf	11.8 psf	5 psf	11470	1026 ft2	14.8 psf	11.8 psf	5 psf	12722
Wall Area	227 ft2	21.6 psf	17.2 psf	5 psf	4104	1bf	290 ft2	19.2 psf	12.7 psf	4060
Lateral Load					15574	16782				
Truss F1	Drag Strut	137 ft			227	1bf	Drag Strut	100 ft		336
Truss F2	Wall Ht	9 ft			97	1bf	Wall Ht	9 ft		86

● INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL TH_H I_H READED ROD

⊗ INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL TH_H I_H READED ROD

Diagram illustrating the components of a header/beam call-out:

- HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER



CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. ANDERSON TRUSS CO.
(JOB #8-207)

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ia County, Florida

PRINTED DATE:
September 08, 2008

DRAWN BY	STRUCTURAL BY: David Disosway
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FINALS DATE:	08Sep08
--------------	---------

JOE NUMBER:
808292

WING NUMBER

S-3

OF 4 SHEETS