

RE: 6243111 - 1755-D-14x10 Lanai

Address: 742 SW Rosemary Dr, .

# Site Information:

Lot/Block: 094

City: Lake City

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Customer Info: Adams Homes-Gainesville Project Name: The Preserve at Laurel Lase4094.1200del: 1755-D-14x10 Lanai Subdivision: The Preserve at Laurel Lake

State: fl

#### Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Wind Code: ASCE 7-22 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 44 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T35970793	A01	1/7/25	23	T35970815	C3C	1/7/25
2	<u>T35970794</u>	A02	1/7/25	24	<u>T35970816</u>	C3L	1/7/25
3	135970795	A03	1/7/25	25	135970817	C5	1/7/25
4	135970796	A04	1/7/25	26	135970818	CSB	1/7/25
C C	T35970797	A05 A06	1/7/25	21	T35970019	E51	1/7/25
7	T35970790	A00 A07	1/7/25	20	T35970820	F7	1/7/25
8	T35970800	A08	1/7/25	30	T35970822	<u>G</u> 01	1/7/25
9	T35970801	A09	1/7/25	31	T35970823	Ğ02	1/7/25
10	T35970802	A10	1/7/25	32	T35970824	G03	1/7/25
11	T35970803	A11	1/7/25	33	<u>T</u> 35970825	G04	1/7/25
12	<u>T35970804</u>	A12	1/7/25	34	<u>T35970826</u>	H5L	1/7/25
13	135970805	A13	1/7/25	35	135970827		1/1/25
14	T25070807	A14 A15	1/7/25	30	T25070920		1/7/25
16	T35970807	A16	1/7/25	38	T35970829	102	1/7/20
17	T35970809	A17	1/7/25	39	T35970831	LV1	1/7/25
18	T35970810	A18	1/7/25	40	T35970832	ĒV2	1/7/25
19	T35970811	C1	1/7/25	41	T35970833	LV3	1/7/25
20	T35970812	C1L	1/7/25	42	T35970834	M1	1/7/25
21	T35970813	C3	1/7/25	43	T35970835	M2	1/7/25
22	135970814	C3B	1/7/25	44	135970836	M3	1/7/25

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Tibbetts Lumber Co., LLC.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2025.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.







I	7-0-0	11-6-14	16-0-0	20-5-2	I.	25-0-0	I.	32-0-0	I
	7-0-0	4-6-14	4-5-2	4-5-2	1	4-6-14		7-0-0	
Plate Offsets (X,Y)	[3:0-5-4,0-2-0], [6:0-2-8,0-3-0	)], [7:0-5-4,0-2-0], [11:0	0-4-0,0-3-0]						
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/TPI20	2-0-0 <b>CSI</b> 1.25 TC 1.25 BC NO WB 014 Mat	0.88 0.72 0.22 rix-S	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) 0 Wind(LL) 0	in (loc) .19 11 .39 10-11 .12 8 .12 11	l/defl L >999 36 >984 24 n/a n >999 24	/d 60 40 /a	PLATES MT20 Weight: 314 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	P No.2 P No.2 P No.2			BRACING- TOP CHORD BOT CHORD	Structu Rigid c	iral wood shea eiling directly a	thing directly applied or 10-	applied. ·0-0 oc bracing.	
REACTIONS. (size Max H Max U Max G	e) 8=0-3-8, 2=0-3-8 orz 2=71(LC 7) plift 8=-103(LC 8), 2=-153(LC rav 8=2399(LC 1), 2=2491(L	C 8) C 1)							
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           7-8=-         80T CHORD           80T CHORD         2-13=           8-9=-         8-9=-           WEBS         3-13=	Comp./Max. Ten All forces 4655/164, 3-4=-5516/297, 4- 4779/214 88/4047, 12-13=-80/4065, 1 137/4165 -0/634, 3-12=-167/1907, 4-12	250 (lb) or less excep 5=-5998/338, 5-6=-599 1-12=-212/5516, 10-1 2=-981/215, 4-11=-53/6	t when shown. 98/338, 6-7=-5568/3 1=-233/5568, 9-10= 646, 5-11=-533/170.	118, -129/4183,					
<ul> <li>NOTES-</li> <li>1) 2-ply truss to be con Top chords connect Bottom chords connect Bottom chords connect Bottom chords connect Bottom chords connect Bottom chords connect Bottom chords connect Building Designer / F to the use of this trus Building Designer / F to the use of this trus Provide adequate dr This truss has been will fit between the b Provide mechanical joint 2.</li> </ul>	=-26/572, 6-10=-933/194, 7-1 =-26/572, 6-10=-933/194, 7-1 anected together with 10d (0.1 ected as follows: 2x4 - 1 row at 0 ected as follows: 2x4 - 1 row at 0-9-0 ered equally applied to all plie e been provided to distribute e been provided to distribute e been provided to distribute a loads have been considered (ult=130mph (3-second gust)) bi=0.18; MWFRS (directional) Project engineer responsible iss component. rainage to prevent water pond designed for a 10.0 psf botton n designed for a live load of 2 bottom chord and any other m connection (by others) of trus	(=-981/215, 4-11=-53/6 0=-132/1827, 7-9=0/64 131"x3") nails as follow 0-9-0 oc. at 0-9-0 oc. oc. is, except if noted as fr only loads noted as (F for this design. Vasd=101mph; TCDL: ; cantilever left and rig for verifying applied root ting. m chord live load nonce 0.0psf on the bottom of embers. is to bearing plate cap.	s: ont (F) or back (B) f ) or (B), unless othe =4.2psf; BCDL=6.0p ht exposed ; Lumbe of live load shown c oncurrent with any chord in all areas wf able of withstanding	ace in the LOAD rwise indicated. psf; h=15ft; B=45 r DOL=1.60 plat overs rain loadin other live loads. here a rectangle 103 lb uplift at ju	CASE(S) ft; L=24ft; e grip DOL g requiren 3-6-0 tall b pint 8 and	section. Ply to eave=4ft; Cat. =1.60 tents specific y 2-0-0 wide 153 lb uplift at	Joaquin MiTek D	NO 68 ST ST Velez PE No.68182 nc. DBA MITek USA	VELE SE 182 OF UNCLENCE

MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
					Т	Г35970793
6243111	A01	HIP GIRDER	1	2		
				<b>_</b>	Job Reference (optional)	
Tibbetts Lumber Co., LLC (C	Dcala, FL), Ocala, FL - 34	472,		3.730 s De	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:06 2025	Page 2

ID:nV5ZFUJaGJLKOI1jrAiSDcyHyrn-5fK5pNNae4mOI?tFfosi07fmvCCLrwHLosQfKMzy5Jh

#### NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 142 lb down and 86 lb up at 7-0-0, 123 lb down and 83 lb up at 9-0-12, 123 lb down and 83 lb up at 11-0-12, 123 lb down and 83 lb up at 13-0-12, 123 lb down and 83 lb up at 15-0-12, 123 lb down and 83 lb up at 18-11-4, 123 lb down and 83 lb up at 20-11-4, and 123 lb down and 83 lb up at 22-11-4, and 251 lb down and 170 lb up at 25-0-0 on top chord, and 311 lb down at 7-0-0, 96 lb down at 9-0-12, 96 lb down at 13-0-12, 96 lb down at 24-11-4, and 96 lb down at 22-11-4, and 96 lb down at 22-11-4, and 311 lb down at 24-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

# LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-60, 3-7=-60, 7-8=-60, 2-8=-20

Concentrated Loads (lb)

Vert: 3=-123(F) 7=-204(F) 13=-264(F) 9=-264(F) 14=-123(F) 15=-123(F) 16=-123(F) 17=-123(F) 18=-123(F) 19=-123(F) 20=-123(F) 21=-123(F) 23=-48(F) 24=-48(F) 25=-48(F) 2





Scale = 1:57.0



L	6-4-12	9-0-0	16-0-0	23-0-0	25-7-3	32-0-0	
Dioto Offecto (V.V)	6-4-12	2-7-3 '	7-0-0	7-0-0	2-7-3	6-4-13	
	[4.0-5-4,0-2-0], [0.0-5-4,0	)-2-0], [11.0-4-0	,0-3-0]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	<b>CSI.</b> TC 0.81 BC 0.67 WB 0.18	DEFL.         in         (loc)           Vert(LL)         -0.14         11           Vert(CT)         -0.30         10-11           Horz(CT)         0.10         8	l/defl L/d >999 360 >999 240 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 10.0	Code FBC2023/T	PI2014	Matrix-S	Wind(LL) 0.08 11	>999 240	Weight: 165 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI REACTIONS. (siz Max H Max U Max C	P No.2 P No.2 P No.2 Horz 2=87(LC 11) Jplift 8=-53(LC 12), 2=-11 Grav 8=1264(LC 1), 2=14	8(LC 12) 01(LC 1)		BRACING- TOP CHORD Structu BOT CHORD Rigid c	ural wood sheathing æiling directly applied	directly applied or 2-2-0 or d or 10-0-0 oc bracing.	c purlins.
FORCES.         (lb) - Max.           TOP CHORD         2-3=           7-8=         7-8=           BOT CHORD         2-13           8-9=         8-9=           WEBS         3-12           7-10         7-10	. Comp./Max. Ten All foi -2324/185, 3-4=-2035/203 -2342/193 =-115/1986, 12-13=-115/1 -117/2027 =-324/71, 4-12=-1/370, 4- )=-369/83	rces 250 (lb) or 3, 4-5=-2278/23 1986, 11-12=-69 11=-53/667, 5- <sup>-</sup>	less except when shown. 1, 5-6=-2278/231, 6-7=-20 9/1789, 10-11=-72/1802, 9 11=-481/141, 6-11=-45/65	154/211, -10=-117/2027, 7, 6-10=-10/378,			
NOTES- 1) Unbalanced roof liv 2) Wind: ASCE 7-22; \ II; Exp B; Encl., GC Zone1 13-2-15 to 2 members and force 3) Building Designer / to the use of this tru 4) Provide adequate d 5) This truss has been will fit between the l	e loads have been consid Vult=130mph (3-second g pi=0.18; MWFRS (directio 3-0-0, Zone2 23-0-0 to 27 s & MWFRS for reactions Project engineer responsi iss component. Irainage to prevent water p o designed for a 10.0 psf b en designed for a live load bottom chord and any othe	ered for this des ust) Vasd=101n inal) and C-C Zi -2-15, Zone1 27 shown; Lumbel ble for verifying bonding. ottom chord live of 20.0psf on th er members.	sign. hph; TCDL=4.2psf; BCDL= bne3 -2-0-0 to 1-0-0, Zone -2-15 to 31-10-4 zone; ca r DOL=1.60 plate grip DOI applied roof live load sho bload nonconcurrent with he bottom chord in all area	=6.0psf; h=15ft; B=45ft; L=24ft; 1 1-0-0 to 9-0-0, Zone2 9-0-0 to ntilever left and right exposed ; L=1.60 wn covers rain loading requirem any other live loads. Is where a rectangle 3-6-0 tall b	eave=4ft; Cat. o 13-2-15, C-C for nents specific y 2-0-0 wide	P. STADA	VELEN 182 OF

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 8 and 118 lb uplift at joint 2.

# SIONAL Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

Date:

January 7,2025



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. WARNING - verify design parameters and READ NOTES ON THIS AND INCLOSED INTERNETING TO THE REFERENCE FOR UNITY TO THE INSTANCE OF THE ADDRESS OF THE ADDRESS





	6-4-12 6-4-12	11-0-0 4-7-4		21-0-0 10-0-0		25-7 4-7-	-4 4	32-0-0 6-4-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/1	2-0-0 1.25 1.25 YES TPI2014	<b>CSI.</b> TC 0.75 BC 0.49 WB 0.31 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.29 10-12 -0.54 10-12 0.08 8 0.06 10-12	l/defl L >999 3 >706 2 n/a r >999 2	./d 60 40 1/a 40	PLATES MT20 MT20HS Weight: 164 lb	<b>GRIP</b> 244/190 187/143 FT = 20%

# LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP M 31 or 2x4 SP SS

 WEBS
 2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=104(LC 11) Max Uplift 8=-53(LC 12), 2=-118(LC 12)

Max Grav 8=1405(LC 12), 2=118(LC 12)Max Grav 8=1405(LC 18), 2=1525(LC 17)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2550/191, 3-4=-2108/188, 4-5=-1853/190, 5-6=-1861/188, 6-7=-2119/192,

7-8=-2569/198

- BOT CHORD 2-13=-122/2263, 12-13=-122/2263, 10-12=-88/1987, 9-10=-123/2230, 8-9=-123/2230
- WEBS 3-12=-476/93, 4-12=-5/704, 5-12=-327/75, 5-10=-320/70, 6-10=-8/712, 7-10=-511/104

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 11-0-0, Zone2 11-0-0 to 15-2-15, Zone1 15-2-15 to 21-0-0, Zone2 21-0-0 to 25-2-15, Zone1 25-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for

- members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 8 and 118 lb uplift at joint 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025







I	7-2-5	<u>13-0-0</u> <u>14-0-0</u> 5-9-11 <u>1-0-0</u>	<u>18-3-0</u> 4-3-0	<u>19-0-0</u> 0-9-0	25-7-5 6-7-4	+ <u>32-0-0</u> 6-4-12
Plate Offsets (X,Y)	[4:0-2-8,0-3-0], [5:0-5-8,0-2-0], [6:0-5-4,	)-2-0], [9:0-3-8,0-1-8], [12	::0-5-8,0-2-12]		•••	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	CSI. TC 0.90 BC 0.70 WB 0.49 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.29 14-15 -0.60 14-15 0.37 8 0.17 14-15	l/defl L/d >999 360 >631 240 n/a n/a >999 240	PLATES         GRIP           MT20         244/190           Weight: 170 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF 2-13: 2 WEBS 2x4 SF	P No.2 P No.2 *Except* x4 SP M 31 or 2x4 SP SS P No.2		BRACING- TOP CHOR BOT CHOR	D Structur D Rigid ce	ral wood sheathing dire iling directly applied o	ectly applied. r 10-0-0 oc bracing.
REACTIONS. (siz Max H Max U Max G	e) 8=0-3-8, 2=0-3-8 lorz 2=120(LC 11) lplift 8=-45(LC 12), 2=-113(LC 12) irav 8=1277(LC 1), 2=1411(LC 1)					
FORCES.         (lb) - Max.           TOP CHORD         2-3=:           7-8=:         7-8=:           BOT CHORD         2-15:           WEBS         4-15:           6-12         6-12	Comp./Max. Ten All forces 250 (lb) or -4301/297, 3-4=-4072/254, 4-5=-3311/22 -2404/176 =-237/3860, 14-15=-168/3569, 13-14=-58 =0/379, 4-14=-566/107, 5-14=-15/416, 5- =0/312, 7-12=-1/334, 7-9=-521/125	less except when shown. 0, 5-6=-3168/215, 6-7=-2 5/3006, 12-13=-38/2361, 8 13=0/788, 6-13=-15/1043	632/198, 8-9=-106/2088 8, 9-12=-111/2179	,		
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GCr Zone1 17-2-15 to 15 members and forces 3) Building Designer /1 to the use of this tru 4) Provide adequate di 5) * This truss has been 6) * This truss has been	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=101r pi=0.18; MWFRS (directional) and C-C Z 9-0-0, Zone2 19-0-0 to 23-2-15, Zone1 2: s & MWFRS for reactions shown; Lumbe Project engineer responsible for verifying ss component. rainage to prevent water ponding. designed for a 10.0 ps foottom chord live n designed for a live load of 20 cost on the	sign. nph; TCDL=4.2psf; BCDL one3 -2-0-0 to 1-0-0, Zon; 3-2-15 to 31-10-4 zone; cr r DOL=1.60 plate grip DC applied roof live load sho e load nonconcurrent with	=6.0psf; h=15ft; E e1 1-0-0 to 13-0-0 antilever left and r vL=1.60 wwn covers rain lo any other live loa	s=45ft; L=24ft; e , Zone2 13-0-0 ight exposed ;C ading requirement ads.	eave=4ft; Cat. to 17-2-15, -C for ents specific	No 68182

- will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 8 and 113 lb uplift at joint 2.



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F		7-2-5		14-0-0	15-0-0	17-0-0 18-3-0	21-7-4	25-7-	-4	32-0-0	
		7-2-5	6	5-9-11	'1-0-0	2-0-0 1-3-0	3-4-4	4-0-	0 '	6-4-12	I
Plate Offsets	(X,Y)	[3:0-2-8,0-3-0], [5:0-5-8,0-	2-4], [9:0-2-8,0	0-3-0], [15:0-	5-12,0-3-12]						
LOADING (p TCLL 20 TCDL 10 BCLL 0 BCDL 10	osf) 0.0 0.0 0.0 * 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/TF	2-0-0 1.25 1.25 YES 12014	<b>CSI.</b> TC BC WB Matrix	0.91 0.67 0.48 <-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.30 18-19 -0.64 18-19 0.37 10 0.18 18-19	l/defl >999 >590 n/a >999	L/d 360 240 n/a 240	<b>PLATES</b> MT20 Weight: 195 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS REACTIONS	<ul> <li>2x4 SF</li> <li>2x4 SF</li> <li>1-18: 2</li> <li>2x4 SF</li> <li>(siz)</li> <li>Max H</li> <li>Max U</li> <li>Max G</li> </ul>	<ul> <li>No.2</li> <li>No.2 *Except*</li> <li>X4 SP M 31 or 2x4 SP SS</li> <li>No.2</li> <li>e) 1=0-3-8, 10=0-3-8</li> <li>lorz 1=125(LC 11)</li> <li>lplift 1=-52(LC 12), 10=-51</li> <li>irav 1=1274(LC 1), 10=12</li> </ul>	(LC 12) 76(LC 1)			BRACING- TOP CHOR BOT CHOR	D Structur D Rigid ca 10-0-0	ral wood sh eiling directl oc bracing:	eathing dire y applied or 13-15	ectly applied. r 10-0-0 oc bracing. E	kcept:
FORCES. () TOP CHORD BOT CHORD WEBS	lb) - Max. 1-2=- 6-7=- 1-19= 7-15= 3-19= 12-1	Comp./Max. Ten All forc 4376/307, 2-3=-4153/254, -2335/208, 7-8=-2624/206, =-245/3957, 18-19=-171/36 =-25/591, 11-12=-103/2050 =0/429, 3-18=-731/120, 4-1 5=-67/2126, 8-15=0/788, 8	es 250 (lb) or 3-4=-3189/20 8-9=-1973/19 512, 17-18=0/2 0, 10-11=-102/ 8=-286/144, 5 5-12=-985/38, 5	less except v 8, 4-5=-3179 3, 9-10=-237 2389, 16-17= 2054 5-16=-611/18 9-12=-462/75	when shown. 1/277, 5-6=-2 '2/176 -6/2389, 15- 5, 6-16=-62/9 5, 5-18=-148	1110/192, 16=-26/2324, 103, 7-16=-634/92, /2005					
NOTES- 1) Unbalance	ed roof live	e loads have been conside	red for this des	sign.	1 2nsf: BCDI	-6 0nsf: h-15ft: B	-45ft: I -24ft: c	ave-4ft: Ca	at	IN AQUIN	VELE

II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 15-0-0, Zone3 15-0-0 to 17-0-0, Zone2 17-0-0 to 21-2-15, Zone1 21-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1 and 51 lb uplift at joint 10.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

January 7,2025







3x4 ||

7-2-5	1	4-0-0 16-0	0-0 18-3-0	25-7-5		32-0-0	
7-2-5	6	-9-11 2-0	)-0 2-3-0	7-4-5		6-4-11	,
LOADING (psf)SPACITCLL20.0Plate GTCDL10.0LumbeBCLL0.0 *Rep StBCDL10.0Code	NG- 2-0-0 srip DOL 1.25 r DOL 1.25 ress Incr YES FBC2023/TPI2014	<b>CSI.</b> TC 0.91 BC 0.69 WB 0.64 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) l/defl -0.32 16-17 >999 -0.67 16-17 >564 0.38 10 n/a 0.18 16-17 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 178 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

10-0-0 oc bracing: 12-14

Rigid ceiling directly applied or 10-0-0 oc bracing. Except:

LUMBER-

- TOP CHORD 2x4 SP No.2 2x4 SP No.2 \*Except\* BOT CHORD 1-16: 2x4 SP M 31 or 2x4 SP SS
- WFBS
- 2x4 SP No.2
- REACTIONS. (size) 1=0-3-8, 10=0-3-8 Max Horz 1=133(LC 11) Max Uplift 1=-52(LC 12), 10=-51(LC 12) Max Grav 1=1274(LC 1), 10=1276(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-4378/367, 2-4=-4152/303, 4-5=-3185/231, 5-6=-2273/231, 6-7=-2290/229,

- 7-9=-2649/227. 9-10=-2402/209
- BOT CHORD 1-17=-288/3959. 16-17=-182/3601. 15-16=-52/2870. 14-15=-59/2333. 7-14=0/310. 10-11=-122/2087
- WEBS 4-17=0/439, 4-16=-725/123, 5-16=-24/1775, 5-15=-1796/152, 6-15=-196/1942, 7-15=-546/131, 11-14=-145/2138, 9-14=-12/266, 9-11=-531/133

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II: Exp B: Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15 Zone1 20-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1 and 51 lb uplift at ioint 10.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/TPI20	-0-0 <b>CSI.</b> 1.25 TC 0.91 1.25 BC 0.68 YES WB 0.64 114 Matrix-S	DEFL.         in           Vert(LL)         -0.32           Vert(CT)         -0.68           Horz(CT)         0.38           Wind(LL)         0.18	n (loc) l/defl 2 14-15 >999 3 14-15 >563 9 8 n/a 3 14-15 >999	L/d 360 240 n/a 240	PLATES MT20 2 Weight: 176 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 1-1	SP No.2 SP No.2 *Except* 4: 2x4 SP M 31 or 2x4 SP SS	I	BRACING- TOP CHORD BOT CHORD	Structural wood s Rigid ceiling dire 10-0-0 oc bracing	sheathing dire ctly applied o g: 10-12	ectly applied. r 10-0-0 oc bracing. Exc	ept:

REACTIONS. (size) 1=0-3-8, 8=0-3-8 Max Horz 1=133(LC 11) Max Uplift 1=-51(LC 12), 8=-49(LC 12) Max Grav 1=1275(LC 1), 8=1278(LC 1)

2x4 SP No.2

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-4383/363, 2-3=-4160/300, 3-4=-3193/228, 4-5=-2278/228, 5-6=-2294/216,

6-7=-2701/224, 7-8=-2394/204 BOT CHORD 1-15=-284/3963, 14-15=-180/3616, 13-14=-50/2877, 12-13=-66/2401, 6-12=0/303, 8-9=-116/2077 WEBS 3-15=0/432, 3-14=-733/124, 4-14=-25/1778, 4-13=-1798/151, 5-13=-171/1896, 6-13=-522/114, 9-12=-136/2153, 7-12=0/290, 7-9=-560/125

#### NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15, Zone1 20-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 1 and 49 lb uplift at joint 8.

# AQUIN VEL No 68182 PD STACH OF NO 68182 ACCOR I D ACINAL SORIDACINAL SORIDACINAL SORIDACINAL

Joaquin Velez PE No.68182 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





<b> </b>	7-2-5 7-2-5		<u>14-0-0</u> 6-9-11	16-0-0 2-0-0	+ <u>21-0</u> 5-0	0-2	25-7 4-7	7-5 -3	<u>32-0-0</u> 6-4-11	
Plate Offsets (X,Y)	[3:0-2-4,0-3-0], [7:0-2-8,0-	-3-0]							-	
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.91		Vert(LL)	-0.32 14-15	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.71		Vert(CT)	-0.68 14-15	>563	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.63		Horz(CT)	0.39 8	n/a	n/a		
BCDL 10.0	Code FBC2023/TF	PI2014	Matrix-S		Wind(LL)	0.18 14-15	>999	240	Weight: 170 lb	FT = 20%
LUMBER-					BRACING-					

TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied.	
BOT CHORD	2x4 SP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	Except:
	1-14: 2x4 SP M 31 or 2x4 SP SS		10-0-0 oc bracing: 10-12	-
WEBS	2x4 SP No.2			
REACTIONS.	(size) 1=0-3-8, 8=0-3-8			
	Max Horz 1=133(LC 11)			
	Max Uplift 1=-52(LC 12), 8=-49(LC 12)			
	Max Grav 1=1274(LC 1), 8=1279(LC 1)			
FORCES. (Ib	) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.			
TOP CHORD	1-2=-4379/366, 2-3=-4154/302, 3-4=-3185/230, 4-5=-2275/225, 5-6=-23	23/206,		

 
 TOP CHORD
 1-2=-4379/366, 2-3=-4154/302, 3-4=-3185/230, 4-5=-2275/225, 5-6=-2323/206, 6-7=-2788/232, 7-8=-2379/203

 BOT CHORD
 1-15=-286/3960, 14-15=-182/3603, 13-14=-54/2868, 12-13=-100/2541, 8-9=-113/2060

 WEBS
 3-15=0/439, 3-14=-726/125, 4-14=-36/1770, 4-13=-1765/149, 5-13=-132/1802, 6-13=-545/114, 9-12=-115/2160, 7-12=0/376, 7-9=-647/109

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15, Zone1 20-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1 and 49 lb uplift at joint 8.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.25	TC 0.93	Vert(LL) -0	0.24 11-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.25	BC 0.97	Vert(CT) -0	0.53 11-12	>627	240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.51	Horz(CT) (	0.32 8	n/a	n/a		
BCDL	10.0	Code FBC2023/TPI2014	Matrix-S	Wind(LL) (	0.15 11-12	>999	240	Weight: 157 lb	FT = 20%
LUMBE	र-			BRACING-					

TOP CHORD

BOT CHORD

LUMBER-

- TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 WFBS 2x4 SP No.2
- REACTIONS. (size) 1=0-3-8, 8=0-3-8 Max Horz 1=115(LC 11) Max Uplift 1=-46(LC 12), 8=-50(LC 12) Max Grav 1=1108(LC 1), 8=1108(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-3720/376, 2-3=-3481/313, 3-4=-2474/248, 4-5=-1706/219, 5-6=-1806/201,

- 6-7=-1523/170.7-8=-1081/144
- BOT CHORD 1-12=-349/3361, 11-12=-255/2966, 10-11=-129/2210, 9-10=-128/1407
- 3-12=0/462, 3-11=-736/127, 4-11=-84/1447, 4-10=-1445/178, 5-10=-98/1208, WEBS 6-10=-13/279, 6-9=-549/142, 7-9=-118/1382

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15 , Zone1 20-2-15 to 27-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 50 lb uplift at joint 8.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

2-2-0 oc bracing: 1-12.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	1	7-2-5	1	14-0-	0	16-0-0		23-7-4		25-0-2	28-0-0	1
		7-2-5	1	6-9-1	1	2-0-0		7-7-4		1-4-14	2-11-14	1
Plate Offsets	(X,Y)	[1:0-4-0,Edge], [3:0-2-8,0	-3-0]									
LOADING (p	osf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PL	ATES	GRIP
TCLL 2	0.0	Plate Grip DOL	1.25	TC	0.92	Vert(LL)	-0.24 11-12	>999	360	MT	20	244/190
TCDL 1	0.0	Lumber DOL	1.25	BC	0.97	Vert(CT)	-0.53 11-12	>627	240			
BCLI	0.0 *	Rep Stress Incr	YES	WB	0.51	Horz(CT)	0.32 8	n/a	n/a			

Wind(LL)

0.15 11-12 >999

240

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied, except end ver
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
WEBS	2x4 SP No.2		2-2-0 oc bracing: 1-12.
WEBS	2x4 SF N0.2		2-2-0 oc blacing. 1-12.
DEACTIONS	(0, -0, -0, -0, -0, -0, -0, -0, -0, -0, -		

Matrix-S

REACTIONS. (size) 1=0-3-8, 8=0-3-8 Max Horz 1=115(LC 11) Max Uplift 1=-46(LC 12), 8=-50(LC 12) Max Grav 1=1108(LC 1), 8=1108(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 1-2=-3719/376, 2-3=-3482/313, 3-4=-2476/248, 4-5=-1707/220, 5-6=-1805/201,

6-7=-1525/170.7-8=-1081/145 BOT CHORD 1-12=-349/3360, 11-12=-256/2977, 10-11=-130/2212, 9-10=-128/1409

Code FBC2023/TPI2014

3-12=0/455, 3-11=-745/127, 4-11=-83/1445, 4-10=-1448/179, 5-10=-99/1210, WEBS 6-10=-14/277, 6-9=-547/141, 7-9=-118/1383

#### NOTES-

BCDL

10.0

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15 , Zone1 20-2-15 to 27-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 50 lb uplift at joint 8.



Weight: 153 lb

FT = 20%

except end verticals.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	1	7-2-5	1	14-0-	·0	16-0-0	2	3-7-6		28-0-0	1
		7-2-5	1	6-9-1	1	2-0-0	-	7-7-6		4-4-11	7
Plate Offs	sets (X,Y)	[1:0-4-0,Edge], [3:0-2-8,0-3-0	]								
LOADING	G (psf)	SPACING- 2	-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	.25	тс	0.93	Vert(LL)	-0.24 11-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	.25	BC	0.97	Vert(CT)	-0.53 11-12	>627	240		
BCLL	0.0 *	Rep Stress Incr	/ES	WB	0.51	Horz(CT)	0.32 8	n/a	n/a		
BCDL	10.0	Code FBC2023/TPI20	14	Matr	ix-S	Wind(LL)	0.15 11-12	>999	240	Weight: 148 lb	FT = 20%

					-	
LUMBER-			BRACING-			
TOP CHORD	2x4 SP	No.2	TOP CHORD	Structural wood sheathing dire	ectly applied, except	t end verticals.
BOT CHORD	2x4 SP	No.2	BOT CHORD	Rigid ceiling directly applied o	r 10-0-0 oc bracing,	Except:
WEBS	2x4 SP	No.2		2-2-0 oc bracing: 1-12.		

REACTIONS. (size) 1=0-3-8, 8=0-3-8 Max Horz 1=115(LC 11) Max Uplift 1=-46(LC 12), 8=-50(LC 12) Max Grav 1=1108(LC 1), 8=1108(LC 1)

**FORCES.** (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-3719/376, 2-3=-3482/313, 3-4=-2476/248, 4-5=-1707/220, 5-6=-1806/201,

- 6-7=-1524/170, 7-8=-1081/145
- BOT CHORD 1-12=-349/3360, 11-12=-256/2977, 10-11=-130/2212, 9-10=-128/1408
- WEBS 3-12=0/455, 3-11=-745/127, 4-11=-84/1445, 4-10=-1448/179, 5-10=-99/1210, 6-10=-13/278, 6-9=-548/141, 7-9=-118/1382

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 16-0-0, Zone2 16-0-0 to 20-2-15 , Zone1 20-2-15 to 27-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1 and 50 lb uplift at joint 8.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 16-0-0, Zone2 16-0-0 to 20-2-15, Zone1 20-2-15 to 27-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 2, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 2 and 47 lb uplift at joint 9.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





1	7-2-5	14-0-0	16-0-0 <sub> </sub> 18-3-0 <sub> </sub>	25-7-5	32-0-0
I	7-2-5	6-9-11	2-0-0 2-3-0	7-4-5	6-4-11
Plate Offsets (X,Y)	[4:0-2-8,0-3-0], [8:0-2-8,0-3-4]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCodeFBC2023/TPI2014	CSI. TC 0.65 BC 0.67 WB 0.63 Matrix-S	DEFL.         in           Vert(LL)         -0.31           Vert(CT)         -0.67           Horz(CT)         0.38           Wind(LL)         0.18	(loc) I/defl L/d 15-16 >999 360 15-16 >569 240 9 n/a n/a 15-16 >999 240	PLATES         GRIP           MT20         244/190           Weight: 181 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP 2-15: 2 WEBS 2x4 SP REACTIONS. (size Max H Max U Max G	No.2 No.2 *Except* x4 SP M 31 or 2x4 SP SS No.2 e) 2=0-3-8, 9=0-3-8 orz 2=145(LC 11) plift 2=-115(LC 12), 9=-48(LC 12) rav 2=1407(LC 1), 9=1271(LC 1)		BRACING- TOP CHORD S BOT CHORD F	Structural wood sheathing dire Rigid ceiling directly applied o 0-0-0 oc bracing: 11-13	ectly applied or 2-2-6 oc purlins. r 10-0-0 oc bracing. Except:
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           7-8=-           BOT CHORD         2-16=           9-10=           WEBS         4-16=           7-14=	Comp./Max. Ten All forces 250 (lb) or 4292/292, 3-4=-4066/252, 4-5=-3163/22 2631/223, 8-9=-2389/205 227/3855, 15-16=-172/3567, 14-15=-47 118/2075 -0/418, 4-15=-711/113, 5-15=-19/1756, 5 544/131, 10-13=-144/2139, 8-13=-16/2	less except when shown. 3, 5-6=-2258/223, 6-7=-2 7/2850, 13-14=-38/2318, 5-14=-1781/148, 6-14=-1 55, 8-10=-530/131	2273/224, 7-13=0/311, 92/1927,		
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GCp Zone1 20-2-15 to 31 Lumber DOL=1.60 p 3) Building Designer / F to the use of this trus 4) This truss has been will fit between the b 6) Bearing at joint(s) 2 capacity of bearing s	loads have been considered for this det ult=130mph (3-second gust) Vasd=101n i=0.18; MWFRS (directional) and C-C Z -10-4 zone; cantilever left and right expo late grip DOL=1.60 Project engineer responsible for verifying ss component. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on th ottom chord and any other members. considers parallel to grain value using A surface.	sign. nph; TCDL=4.2psf; BCDL one3 -2-0-0 to 1-0-0, Zon osed ;C-C for members ar applied roof live load sho load nonconcurrent with ne bottom chord in all are NSI/TPI 1 angle to grain f	=6.0psf; h=15ft; B=45ft; L= e1 1-0-0 to 16-0-0, Zone2 nd forces & MWFRS for re- own covers rain loading re- own covers rain loading re- own other live loads. was where a rectangle 3-6-0 formula. Building designer	=24ft; eave=4ft; Cat. 16-0-0 to 20-2-15, actions shown; quirements specific 0 tall by 2-0-0 wide should verify	No 68182

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 2 and 48 lb uplift at joint 9.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	7-2-5	14-0-0	15-0-0 17-0-0	22-6-0	25-7-4	32-0-0
Plate Offsets (X,Y)	[4:0-2-8,0-3-0], [5:0-3-0,0-2-0], [6:0-5-4,	0-2-0], [8:0-2-8,0-3-0],	[10:0-3-8,0-1-8], [13:0-4	5-12,0-3-8]	5-1-4	0-4-12
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.91 BC 0.69 WB 0.50 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) l/de 0.30 16-17 >99 0.64 16-17 >59 0.37 9 n/ 0.17 16-17 >99	fl L/d 9 360 2 240 a n/a 9 240	PLATES         GRIP           MT20         244/190           Weight: 175 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF 2-16: 2 WEBS 2x4 SF	P No.2 P No.2 *Except* 2x4 SP M 31 or 2x4 SP SS P No.2	1	BRACING- TOP CHORD BOT CHORD	Structural wo Rigid ceiling o 10-0-0 oc bra	od sheathing dired lirectly applied or cing: 11-13	otly applied. 10-0-0 oc bracing. Except:
REACTIONS. (siz Max H Max U Max C	e) 2=0-3-8, 9=0-3-8 lorz 2=136(LC 11) Jplift 2=-116(LC 12), 9=-47(LC 12) Grav 2=1405(LC 1), 9=1273(LC 1)					
FORCES.         (lb) - Max.           TOP CHORD         2-3=           7-8=         2-17           BOT CHORD         2-17           9-10         WEBS           4-17.         7-14	Comp./Max. Ten All forces 250 (lb) or -4278/275, 3-4=-4063/241, 4-5=-3127/19 -2907/217, 8-9=-2363/170 =-216/3841, 16-17=-177/3576, 15-16=-7 =-96/2045 =0/417, 4-16=-769/151, 5-16=-9/1706, 5 =-585/110, 10-13=-86/2228, 8-13=0/571	less except when show 96, 5-6=-2297/205, 6-7= /2384, 14-15=-1/2151, -15=-584/13, 6-15=-45/ , 8-10=-862/104	vn. =-2402/184, 13-14=-98/2671, /585, 6-14=0/388,			
NOTES- 1) Unbalanced roof liv. 2) Wind: ASCE 7-22; V II; Exp B; Encl., GC, Zone2 17-0-0 to 21- for reactions shown 3) Building Designer / to the use of this tru 4) Provide adequate d 5) This truss has been will fit between the f 7) Bearing at joint(s) 2 capacity of bearing 8) Provide mechanical joint 9.	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=101 pi=0.18; MWFRS (directional) and C-C 2 :2-15, Zone1 21-2-15 to 31-10-4 zone; c ; Lumber DOL=1.60 plate grip DOL=1.60 Project engineer responsible for verifying ss component. rainage to prevent water ponding. designed for a 10.0 psf bottom chord liv in designed for a live load of 20.0psf on to sottom chord and any other members. considers parallel to grain value using A surface. connection (by others) of truss to bearing	sign. mph; TCDL=4.2psf; BC cone3 -2-0-0 to 1-0-0, Z antilever left and right e ) g applied roof live load s e load nonconcurrent w he bottom chord in all a .NSI/TPI 1 angle to grai	DL=6.0psf; h=15ft; B=4 one1 1-0-0 to 15-0-0, z xposed ;C-C for memb shown covers rain load with any other live loads areas where a rectangle in formula. Building de standing 116 lb uplift at	15ft; L=24ft; eave= cone3 15-0-0 to 17 ers and forces & M ing requirements s	Ift; Cat. o-0, WFRS pecific ) wide plift at	No 68182 No 68182

SIONALEN Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025







<b> </b>	7-2-5	14-0-0	19-0-0	22-6-0	25-7-4	32-0-0
Plate Offsets (X,Y)	[4:0-2-8,0-3-0], [6:0-5-4,0-2-0], [10:0-3-8	,0-1-8], [13:0-6-0,0-3-4]	3-0-0	3-0-0	3-1-4	0-4-12
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	<b>CSI.</b> TC 0.91 BC 0.67 WB 0.50 Matrix-S	<b>DEFL.</b> Vert(LL) -0.3 Vert(CT) -0.6 Horz(CT) 0.3 Wind(LL) 0.1	in (loc) l/defl 31 15-16 >999 55 15-16 >585 38 9 n/a 17 15-16 >999	L/d 360 240 n/a 240	PLATES         GRIP           MT20         244/190           Weight: 165 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF 2-15: 2 WEBS 2x4 SF	P No.2 P No.2 *Except* x4 SP M 31 or 2x4 SP SS No.2		BRACING- TOP CHORD BOT CHORD	Structural wood : Rigid ceiling dire 10-0-0 oc bracin	sheathing direct ctly applied or 1 g: 11-13	tly applied. I0-0-0 oc bracing. Except:
REACTIONS. (sizu Max H Max U Max G	e) 9=0-3-8, 2=0-3-8 orz 2=120(LC 11) plift 9=-47(LC 12), 2=-116(LC 12) rav 9=1273(LC 1), 2=1405(LC 1)					
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           7-8=-         7-8=-           BOT CHORD         2-16=           WEBS         4-16=           10-13         10-13	Comp./Max. Ten All forces 250 (lb) or 4291/306, 3-4=-4061/263, 4-5=-3190/23 2893/229, 8-9=-2368/182 =-245/3855, 15-16=-183/3543, 14-15=-44 =0/422, 4-15=-639/108, 5-15=0/1151, 6-' 3=-107/2208, 8-13=0/553, 8-10=-854/114	less except when shown. 1, 5-6=-3156/230, 6-7=-2 3/2338, 13-14=-101/2612, 5=-24/1059, 6-14=0/320,	545/214, 9-10=-107/2051 7-14=-378/72,			
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-22; V II; Exp B; Encl., GCf Zone1 17-2-15 to Cf members and forces</li> <li>3) Building Designer / I to the use of this true</li> <li>4) Provide adequate di</li> <li>5) This truss has been</li> <li>6) * This truss has been</li> <li>6) * This truss has been</li> <li>7) Bearing at joint(s) 2 capacity of bearing s</li> <li>8) Provide mechanical joint 2.</li> </ul>	e loads have been considered for this de /ult=130mph (3-second gust) Vasd=101r pi=0.18; MWFRS (directional) and C-C Z 0-0-0, Zone2 19-0-0 to 23-2-15, Zone1 23 & MWFRS for reactions shown; Lumbe Project engineer responsible for verifying ss component. ainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on th ottom chord and any other members. considers parallel to grain value using A surface. connection (by others) of truss to bearin	sign. nph; TCDL=4.2psf; BCDL: one3 -2-0-0 to 1-0-0, Zone 3-2-15 to 31-10-4 zone; ca r DOL=1.60 plate grip DO applied roof live load sho e load nonconcurrent with ne bottom chord in all area NSI/TPI 1 angle to grain for g plate capable of withstan	=6.0psf; h=15ft; B=45ft 1 1-0-0 to 13-0-0, Zon intilever left and right e L=1.60 wn covers rain loading any other live loads. as where a rectangle 3 prmula. Building desig hding 47 lb uplift at joir	t; L=24ft; eave=4ft; le2 13-0-0 to 17-2-1 exposed ;C-C for g requirements spec -6-0 tall by 2-0-0 wi ner should verify nt 9 and 116 lb uplift	Cat. 5, iffic de	No 68182 STATE OF

Jonquin Velez PE No.68182 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025







	6-4-12 6-4-12	11-0-0 4-7-4		21-0-0 10-0-0		25-7 4-7-	-4 4	32-0-0 6-4-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/1	2-0-0 1.25 1.25 YES TPI2014	<b>CSI.</b> TC 0.75 BC 0.49 WB 0.31 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.29 10-12 -0.54 10-12 0.08 8 0.06 10-12	l/defl L >999 3 >706 2 n/a r >999 2	./d 60 40 1/a 40	PLATES MT20 MT20HS Weight: 164 lb	<b>GRIP</b> 244/190 187/143 FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP M 31 or 2x4 SP SS

BOT CHORD2x4 SP M 31 or 2x4 SP SSWEBS2x4 SP No.2

REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=104(LC 11) Max Uplift 8=-53(LC 12), 2=-118(LC 12)

Max Grav 8=1405(LC 18), 2=1525(LC 17)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-2550/191, 3-4=-2108/188, 4-5=-1853/190, 5-6=-1861/188, 6-7=-2119/192,

7-8=-2569/198

- BOT CHORD 2-13=-122/2263, 12-13=-122/2263, 10-12=-88/1987, 9-10=-123/2230, 8-9=-123/2230
- WEBS 3-12=-476/93, 4-12=-5/704, 5-12=-327/75, 5-10=-320/70, 6-10=-8/712, 7-10=-511/104

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 11-0-0, Zone2 11-0-0 to 15-2-15, Zone1 15-2-15 to 21-0-0, Zone2 21-0-0 to 25-2-15, Zone1 25-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 8 and 118 lb uplift at joint 2.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





Scale = 1:57.2



<b> </b>	6-4-13 9-0-0 6-4-13 2-7-4	<u>16-8-9</u> 7-8-9		24-3-7	32-0-0
Plate Offsets (X,Y)	[4:0-5-4,0-2-0], [6:0-4-0,0-3-0]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCodeFBC2023/TPI2014	CSI. TC 0.99 BC 0.71 WB 0.69 Matrix-S	DEFL.         i           Vert(LL)         -0.1:           Vert(CT)         -0.3           Horz(CT)         0.0:           Wind(LL)         0.0:	in (loc) I/defl L/d 3 10 >999 360 1 10-12 >999 240 9 8 n/a n/a 8 10 >999 240	PLATES         GRIP           MT20         244/190           Weight: 173 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 P No.2 P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing di Rigid ceiling directly applied 1 Row at midpt	rectly applied, except end verticals. or 10-0-0 oc bracing. 6-8
REACTIONS. (siz Max H Max U Max G	e) 8=0-3-8, 2=0-3-8 orz 2=144(LC 12) plift 8=-61(LC 12), 2=-110(LC 12) rav 8=1264(LC 1), 2=1401(LC 1)				
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-13=-	Comp./Max. Ten All forces 250 (lb) o 2321/150, 3-4=-2041/166, 4-5=-2277/1 205/1984, 12-13205/1984, 10-121	r less except when shown. 71, 5-6=-2277/171	.0118/1687		

- WFBS
- 3-12=-304/71, 4-12=0/389, 4-10=-25/553, 5-10=-482/139, 6-10=-67/688, 6-9=0/324, 6-8=-1929/134

#### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 9-0-0, Zone2 9-0-0 to 13-2-15, Zone1 13-2-15 to 31-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 8 and 110 lb uplift at joint 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





L	7-0-0	13-3-14	19-6-0	25-8-2	32-0-0
1	7-0-0	6-3-14	6-2-2	6-2-2	6-3-14
Plate Offsets (X,Y)	[3:0-2-0,0-2-12], [5:0-4-0,0-4-8]				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2023/TPI2014	CSI. TC 0.48 BC 0.57 WB 0.51 Matrix-S	DEFL. Vert(LL) -0. Vert(CT) -0. Horz(CT) 0. Wind(LL) 0.	in (loc) l/defl L 15 10-12 >999 36 31 10-12 >999 24 07 8 n/a n 10 10-12 >999 24	d PLATES GRIP 0 MT20 244/190 0 Weight: 419 lb FT = 20%
LUMBER- TOP CHORD 2x6 SF 1-3: 2x BOT CHORD 2x6 SF WEBS 2x4 SF	2 No.2 *Except* 4 SP No.2 2 No.2 2 No.2 2 No.2		BRACING- TOP CHORD BOT CHORD	Structural wood shea except end verticals. Rigid ceiling directly a	hing directly applied or 5-3-2 oc purlins,
REACTIONS. (siz Max H Max U Max G	e) 8=0-3-8, 2=0-3-8 lorz 2=119(LC 8) iplift 8=-183(LC 8), 2=-161(LC 8) irav 8=2649(LC 1), 2=2468(LC 1)				
FORCES.         (lb) - Max.           TOP CHORD         2-3=:           7-8=:         7-8=:           BOT CHORD         2-13:           WEBS         3-13:           7-9=:         7-9=:	Comp./Max. Ten All forces 250 (lb) o -4736/182, 3-4=-6067/369, 4-5=-6065/3 -2504/251 =-188/4150, 12-13=-179/4170, 10-12=-3 =0/698, 3-12=-214/2238, 4-12=-850/263 -301/4477	less except when shown. 68, 5-6=-3984/268, 6-7=-3 85/5930, 9-10=-385/5930 , 5-10=0/529, 5-9=-2220/1	984/268, 33, 6-9=-815/272,		
<ul> <li>NOTES-</li> <li>1) 2-ply truss to be corr Top chords connect Bottom chords connect Webs connected as</li> <li>2) All loads are consid ply connections hav</li> <li>3) Unbalanced roof live</li> <li>4) Wind: ASCE 7-22; \lambda li; Exp B; Encl., GC;</li> <li>5) Building Designer // to the use of this tru</li> <li>6) Provide adequate d</li> <li>7) This truss has been</li> <li>8) * This truss has been will fit between the b</li> <li>9) Provide mechanical joint 2.</li> </ul>	anected together with 10d (0.131"x3") na ed as follows: 2x4 - 1 row at 0-9-0 oc, 2 ected as follows: 2x6 - 2 rows staggere follows: 2x4 - 1 row at 0-9-0 oc. ered equally applied to all plies, except i e been provided to distribute only loads a loads have been considered for this de /ult=130mph (3-second gust) Vasd=101 bi=0.18; MWFRS (directional); cantileve Project engineer responsible for verifyin ss component. rainage to prevent water ponding. designed for a 10.0 psf bottom chord lin n designed for a live load of 20.0psf on pottom chord and any other members. connection (by others) of truss to bearin	tils as follows: k6 - 2 rows staggered at 0- d at 0-9-0 oc. f noted as front (F) or back noted as (F) or (B), unless usign. mph; TCDL=4.2psf; BCDL r left and right exposed ; Lu g applied roof live load sho re load nonconcurrent with the bottom chord in all area ng plate capable of withstan	9-0 oc. (B) face in the LOAD otherwise indicated. =6.0psf; h=15ft; B=45 umber DOL=1.60 plate wm covers rain loading any other live loads. as where a rectangle 3 nding 183 lb uplift at jo	CASE(S) section. Ply to ft; L=24ft; eave=4ft; Cat. e grip DOL=1.60 g requirements specific 3-6-0 tall by 2-0-0 wide bint 8 and 161 lb uplift at	PD STATE OF NO 68182

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Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
						T35970810
6243111	A18	HALF HIP GIRDER	1	2		
				<b>∠</b>	Job Reference (optional)	
Tibbetts Lumber Co., LLC (C	cala, FL), Ocala, FL - 34	472,		8.730 s De	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:17 202	25 Page 2
		ID:nV	5ZFUJaGJL	<ol1jraisd< td=""><td>cyHyrn-GmUF78VU2T8q6hDMobZHyRckPe?BwrTzK4bll</td><td>DDzy5JW</td></ol1jraisd<>	cyHyrn-GmUF78VU2T8q6hDMobZHyRckPe?BwrTzK4bll	DDzy5JW

#### NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 142 lb down and 86 lb up at 7-0-0, 123 lb down and 83 lb up at 9-0-12, 123 lb down and 83 lb up at 11-0-12, 123 lb down and 83 lb up at 13-0-12, 123 lb down and 83 lb up at 15-0-12, 123 lb down and 83 lb up at 19-0-12, 123 lb down and 83 lb up at 21-0-12, 123 lb down and 83 lb up at 22-0-12, and 123 lb down and 83 lb up at 21-0-12, and 130 lb down and 83 lb up at 31-0-12 on top chord, and 311 lb down at 7-0-0, 96 lb down at 9-0-12, 96 lb down at 15-0-12, 96 lb down at 15-0-12, 96 lb down at 23-0-12, 96 lb down at 23-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 23-0-12, 96 lb down at 25-0-12, 96 lb down at 23-0-12, 96 lb down at 31-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-3=-60, 3-7=-60, 2-8=-20

Concentrated Loads (lb)

Vert: 3=-123(B) 13=-264(B) 12=-48(B) 14=-123(B) 15=-123(B) 16=-123(B) 17=-123(B) 18=-123(B) 19=-123(B) 20=-123(B) 21=-123(B) 23=-123(B) 24=-123(B) 26=-123(B) 27=-130(B) 28=-48(B) 30=-48(B) 31=-48(B) 32=-48(B) 32=-48(





				1-0-0	
LOADIN	G (psf)	<b>SPACING-</b> 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.25	TC 0.28	Vert(LL) -0.00 2 >999 360 MT20 244/190	
TCDL	10.0	Lumber DOL 1.25	BC 0.01	Vert(CT) -0.00 2 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00 3 n/a n/a	
BCDL	10.0	Code FBC2023/TPI2014	Matrix-P	Wind(LL)         0.00         2         ****         240         Weight: 7 lb         FT = 20%	

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=48(LC 12)

Max Uplift 3=-100(LC 1), 2=-133(LC 12)

Max Grav 3=67(LC 12), 2=289(LC 1), 4=19(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and
- forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 3 and 133 lb uplift at joint 2.

# January 7,2025

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TFI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org)
and **BCSI Building Component Safety Information**available from the Structural Building Component Association (www.sbcscomponents.com)

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			0-10-12	
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.28 BC 0.01 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         2         >999         360           Vert(CT)         -0.00         2         >999         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.00         2         >999         240	PLATES         GRIP           MT20         244/190           Weight: 7 lb         FT = 20%

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

**REACTIONS.** (size) 3=Mechanical, 4=Mechanical, 2=0-3-8

Max Horz 2=48(LC 12)

Max Uplift 3=-100(LC 1), 4=-2(LC 8), 2=-142(LC 12)

Max Grav 3=67(LC 12), 4=19(LC 3), 2=289(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 3, 2 lb uplift at joint 4 and 142 lb uplift at joint 2.

# January 7,2025

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPH1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)

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				3-0-0				
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.33 BC 0.09 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (lo -0.00 2 -0.01 2 -0.00 0.00	2-4 >99 2-4 >99 2-4 >99 3 n 2 **	fl L/d 9 360 9 240 /a n/a ** 240	PLATES MT20 Weight: 13 lb	<b>GRIP</b> 244/190 FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=71(LC 12)

Max Uplift 3=-14(LC 9), 2=-85(LC 12)

Max Grav 3=37(LC 17), 2=290(LC 1), 4=56(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 2-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 3 and 85 lb uplift at joint 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





					3-0-0			1			
	G (psf)	SPACING- 2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.25	TC	0.13	Vert(LL)	-0.00	1-3	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.25	BC	0.09	Vert(CT)	-0.01	1-3	>999	240		
BCLL	0.0 *	Rep Stress Incr YES	WB	0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2023/TPI2014	Matri	x-P	Wind(LL)	0.00	1	****	240	Weight: 10 lb	FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 2=Mechanical, 3=Mechanical

Max Horz 1=36(LC 12)

Max Uplift 2=-31(LC 12)

Max Grav 1=112(LC 1), 2=84(LC 1), 3=56(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and
- forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





		0-4-4	3-0-0 2-7-12	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.37 BC 0.09 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         2-4         >999         360           Vert(CT)         -0.01         2-4         >999         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.00         2-4         >999         240	PLATES         GRIP           MT20         244/190           Weight: 12 lb         FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 4=Mechanical, 2=0-3-8

Max Horz 2=35(LC 12)

Max Uplift 3=-9(LC 9), 4=-7(LC 8), 2=-111(LC 12)

Max Grav 3=34(LC 1), 4=56(LC 3), 2=290(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 2-11-4 zone; cantilever left
- exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 3, 7 lb uplift at joint 4 and 111 lb uplift at joint 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





			:	3-0-0 2-10-12		—
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCodeFBC2023/TPI2014	CSI. TC 0.33 BC 0.09 WB 0.00 Matrix-P	DEFL. Vert(LL) -0 Vert(CT) -0 Horz(CT) -0 Wind(LL) 0	in (loc) 0.00 2-4 0.01 2-4 0.00 3 0.00 2-4	l/defl L/d >999 360 >999 240 n/a n/a >999 240	PLATES         GRIP           MT20         244/190           Weight: 13 lb         FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 4=Mechanical, 2=0-3-8

Max Horz 2=71(LC 12)

Max Uplift 3=-14(LC 9), 4=-7(LC 8), 2=-109(LC 12)

Max Grav 3=37(LC 17), 4=56(LC 3), 2=290(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 2-11-4 zone; cantilever left
- exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 3, 7 lb uplift at joint 4 and 109 lb uplift at joint 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





5-0-0					
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.31 BC 0.28 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.03         2-4         >999         360           Vert(CT)         -0.06         2-4         >909         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.00         2         ****         240	PLATES         GRIP           MT20         244/190           Weight: 19 lb         FT = 20%	

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=95(LC 12)

Max Uplift 3=-36(LC 12), 2=-70(LC 12)

Max Grav 3=115(LC 1), 2=349(LC 1), 4=96(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3 and 70 lb uplift at joint 2.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	1		5-0-0	1	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in	(loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.42	Vert(LL) -0.03	1-3 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.28	Vert(CT) -0.06	1-3 >909 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) -0.00	2 n/a n/a	
BCDL 10.0	Code FBC2023/TPI2014	Matrix-P	Wind(LL) 0.00	1 **** 240	Weight: 16 lb FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

**REACTIONS.** (size) 1=0-3-8, 2=Mechanical, 3=Mechanical

Max Horz 1=60(LC 12)

Max Uplift 2=-53(LC 12)

Max Grav 1=192(LC 1), 2=144(LC 1), 3=96(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 4-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 2.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





		-4-4	<u>5-0-0</u> 4-7-12		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	CSI. TC 0.50 BC 0.28 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/de           Vert(LL)         -0.03         2-4         >99'           Vert(CT)         -0.06         2-4         >90'           Horz(CT)         -0.00         3         n/           Wind(LL)         0.03         2-4         >90'	1 L/d 360 240 a n/a 240 240	PLATES         GRIP           MT20         244/190           Weight: 18 lb         FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 4=Mechanical, 2=0-3-8 Max Horz 2=47(LC 12) Max Uplift 3=-27(LC 12), 4=-12(LC 8), 2=-120(LC 12)

Max Grav 3=115(LC 1), 4=96(LC 3), 2=349(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-11-4 zone; cantilever left
- exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 12 lb uplift at joint 4 and 120 lb uplift at joint 2.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





			<u>5-0-0</u> 4-10-12	—
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.31 BC 0.28 WB 0.00 Matrix-P	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.03         2-4         >999         360           Vert(CT)         -0.06         2-4         >909         240           Horz(CT)         -0.00         3         n/a         n/a           Wind(LL)         0.03         2-4         >999         240	PLATES         GRIP           MT20         244/190           Weight: 19 lb         FT = 20%

BRACING-TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

**REACTIONS.** (size) 3=Mechanical, 4=Mechanical, 2=0-3-8

Max Horz 2=95(LC 12)

Max Uplift 3=-36(LC 12), 4=-12(LC 8), 2=-111(LC 12)

Max Grav 3=115(LC 1), 4=96(LC 3), 2=349(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-11-4 zone; cantilever left and right exposed ; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 12 lb uplift at joint 4 and 111 lb uplift at joint 2.

# January 7,2025

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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			7-0-0 7-0-0		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.72 BC 0.60 WB 0.00 Matrix-P	DEFL.         in         (loc)           Vert(LL)         -0.13         2-4           Vert(CT)         -0.26         2-4           Horz(CT)         -0.00         3           Wind(LL)         0.00         2	l/defl L/d >639 360 >319 240 n/a n/a ***** 240	PLATES         GRIP           MT20         244/190           Weight: 26 lb         FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

**REACTIONS.** (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=119(LC 12)

Max Uplift 3=-62(LC 12), 2=-63(LC 12)

Max Grav 3=183(LC 1), 2=421(LC 1), 4=136(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 3 and 63 lb uplift at joint 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





<b>⊢</b>	7-0-0	10	-0-0 13-0	-0		20-0-0
Plate Offsets (X,Y)	[3:0-5-4,0-2-0], [5:0-5-4,0-2-0], [9:0-4-0,0-	3-0]	00 00	0		
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2023/TPI2014	<b>CSI.</b> TC 0.64 BC 0.82 WB 0.14 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc -0.09 6-4 -0.20 6-4 0.07 0.04	) l/defl L/d 8 >999 360 8 >999 240 6 n/a n/a 9 >999 240	PLATES         GRIP           MT20         244/190           Weight: 96 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP 3-5: 2x BOT CHORD 2x4 SP WEBS 2x4 SP	M 31 or 2x4 SP SS *Except* 4 SP No.2 No.2 No.2		BRACING TOP CHOI BOT CHOI	RD Struc RD Rigid	tural wood sheathir ceiling directly app	ng directly applied or 3-5-8 oc purlins. lied or 10-0-0 oc bracing.
REACTIONS. (size Max H Max U Max G	<ul> <li>2=0-3-8, 6=0-3-8</li> <li>z 2=-74(LC 6)</li> <li>plift 2=-79(LC 8), 6=-88(LC 8)</li> <li>rav 2=1504(LC 1), 6=1528(LC 1)</li> </ul>					
FORCES.         (lb) - Max.           TOP CHORD         2-3=-           BOT CHORD         2-10=           WEBS         3-10=	Comp./Max. Ten All forces 250 (lb) or l 2507/5, 3-4=-2333/57, 4-5=-2333/57, 5-6 -0/2139, 9-10=0/2157, 8-9=0/2204, 6-8=0 -0/611, 3-9=-128/362, 4-9=-337/120, 5-9=	ss except when s 2559/24 2186 -59/277, 5-8=0/61	shown. 11			
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live</li> <li>2) Wind: ASCE 7-22; V II; Exp B; Encl., GCp</li> <li>3) Building Designer / F to the use of this trus</li> <li>4) Provide adequate dr</li> <li>5) This truss has been</li> <li>6) * This truss has been will fit between the b</li> <li>7) Provide mechanical joint 6.</li> <li>8) Hanger(s) or other c 7-0-0, 123 lb down a top chord, and 311 ll chord. The design/s</li> <li>9) In the LOAD CASE(s) Stand</li> <li>4) Doad + Boof live (b)</li> </ul>	e loads have been considered for this desi ult=130mph (3-second gust) Vasd=101m i=0.18; MWFRS (directional); cantilever la Project engineer responsible for verifying a ss component. ainage to prevent water ponding. designed for a 10.0 psf bottom chord live in designed for a live load of 20.0psf on th ottom chord and any other members. connection (by others) of truss to bearing onnection device(s) shall be provided suff ind 83 lb up at 9-0-12, and 123 lb down at b down at 7-0-0, 96 lb down at 9-0-12, a election of such connection device(s) is tt S) section, loads applied to the face of the dard	gn. bh; TCDL=4.2psf; ft and right expos- pplied roof live lo oad nonconcurre bottom chord in plate capable of v cient to support c nd 83 lb up at 10 vd 96 lb down at e responsibility o truss are noted a	; BCDL=6.0psf; h=15ft; sed ; Lumber DOL=1.60 pad shown covers rain to ent with any other live lo all areas where a recta withstanding 79 lb uplift concentrated load(s) 14: )-11-4, and 251 lb down 10-11-4, and 311 lb dow of others. as front (F) or back (B).	B=45ft; L=24ft plate grip DC ading require ads. ngle 3-6-0 tall at joint 2 and 2 lb down and and 170 lb up vn at 12-11-4	; eave=4ft; Cat. IL=1.60 ments specific by 2-0-0 wide 88 lb uplift at 86 lb up at 9 at 13-0-0 on on bottom	No 68182 PRO STAFFOF
1) Dead + Roof Live (b Uniform Loads (plf) Vert: 1-3=-6 Concentrated Loads Vert: 3=-12	alanced): Lumber Increase=1.25, Plate In i0, 3-5=-60, 5-7=-60, 2-6=-20 (lb) 3(F) 5=-204(F) 10=-264(F) 8=-264(F) 11=	crease=1.25 123(F) 12=-123(F	F) 13=-48(F) 14=-48(F)			Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

January 7,2025





6-3-11		13-8-5		20-0-0
<u> </u>		7-4-10	1	6-3-11 '
Plate Offsets (X,Y) [4:0-2-8,0-2-4], [5:0-2-8,0-2	4], [10:0-4-0,0-3-4]	1		1
LOADING (psf)SPACING-TCLL20.0Plate Grip DOLTCDL10.0Lumber DOLBCLL0.0 *Rep Stress IncrBCDL10.0Code FBC2023/TPf	2-0-0         CSI.           1.25         TC         0.58           1.25         BC         0.62           NO         WB         0.17           2014         Matrix-S	DEFL.         in           Vert(LL)         -0.10         9           Vert(CT)         -0.41         9           Horz(CT)         0.04         Wind(LL)         -0.03	(loc) l/defl L/d 9-10 >999 360 9-10 >578 240 7 n/a n/a 10 >999 240	PLATES         GRIP           MT20         244/190           Weight: 94 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP M 31 or 2x4 SP SS WEBS 2x4 SP No.2 REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=-91(LC 10) Max Grav 2=1217(LC 17), 7=121	7(LC 18)	BRACING- TOP CHORD St BOT CHORD Ri	tructural wood sheathing o igid ceiling directly applied	Jirectly applied or 3-10-14 oc purlins. I or 10-0-0 oc bracing.
FORCES.         (lb) - Max. Comp./Max. Ten All force           TOP CHORD         2-3=-2052/0, 3-4=-1875/0, 4-5=-           BOT CHORD         2-10=0/1834, 9-10=0/1277, 7-9=           WEBS         4-10=0/759, 5-9=0/759	is 250 (lb) or less except when shown. 1246/0, 5-6≕1875/0, 6-7≕2053/0 :0/1767			
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been consider</li> <li>2) Wind: ASCE 7-22; Vult=130mph (3-second gus II; Exp B; Encl., GCpi=0.18; MWFRS (directiona Zone2 11-0-0 to 15-2-15, Zone1 15-2-15 to 22-1 for reactions shown; Lumber DOL=1.60 plate gg</li> <li>3) Building Designer / Project engineer responsible to the use of this truss component.</li> <li>4) Provide adequate drainage to prevent water po 5) This truss has been designed for a 10.0 psf bot 6) * This truss has been designed for a 10.0 psf bot 6) * This truss has been designed for a live load of will fit between the bottom chord and any other</li> <li>7) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, Building designer must review loads to verify th</li> <li>LOAD CASE(S) Standard</li> <li>1) Dead + Roof Live (balanced): Lumber Increase Uniform Loads (plf) Vert: 1-4=-60, 4-5=-60, 5-8=-60, 2-10=</li> <li>2) Dead + 0.75 Roof Live (balanced) + 0.75 Uninh Uniform Loads (plf) Vert: 1-4=-50, 4-5=-50, 5-8=-50, 2-10=</li> <li>3) Dead + Uninhabitable Attic Without Storage: Lu Uniform Loads (plf) Vert: 1-4=-20, 4-5=-20, 5-8=-20, 2-10=</li> </ul>	ed for this design. t) Vasd=101mph; TCDL=4.2psf; BCDL II) and C-C Zone3 -2-0-0 to 1-0-0, Zon D-0 zone; cantilever left and right expo ip DCL=1.60 e for verifying applied roof live load sho nding. tom chord live load nonconcurrent with <sup>1</sup> 20.0psf on the bottom chord in all are members, with BCDL = 10.0psf. 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 1 at they are correct for the intended use =1.25, Plate Increase=1.25 -20, 9-10=-80, 7-9=-20 ab. Attic Storage; Lumber Increase=1. -35, 10-15=-95, 15-16=-110, 9-16=-95 mber Increase=1.25, Plate Increase=1 -40, 9-10=-100, 7-9=-40	L=6.0psf; h=15ft; B=45ft; L=2 le1 1-0-0 to 9-0-0, Zone3 9-0 sed ;C-C for members and f own covers rain loading requ n any other live loads. eas where a rectangle 3-6-0 23, 24, 25, 26 has/have bee e of this truss. 25, Plate Increase=1.25 1,7-9=-35 1.25	24ft; eave=4ft; Cat. 0-0 to 11-0-0, forces & MWFRS uirements specific tall by 2-0-0 wide en modified.	AQUIN VS CENS No 68182 STOP OF ORIDAGE ONALEN ONALEN MITEK Inc. DBA MITEK USA FL Cert 6634 1002 Soungley Ridge Rd. Chesterfield, MO 63012 Danuary 7,2025

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MiTek



	1755-D-14x10 Lanai	Ply	Qty	Truss Type	-	Job
T35970823						
		1	1	Hip	1	6243111
1	Job Reference (optional)					
s, Inc. Mon Jan 6 11:17:23 2025 Page 2	c 5 2024 MiTek Industries, Inc. I	.730 s De	6	1472,	L), Ocala, FL - 344	Tibbetts Lumber Co., LLC (C

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LOAD CASE(S) Standard 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf) Vert: 1-2=37, 2-11=21, 4-11=16, 4-5=21, 5-13=21, 7-13=16, 7-8=12, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-2=-46, 2-11=-30, 4-11=-25, 5-13=30, 7-13=25, 7-8=20 Uniform Loads (plf) Uniform Loads (plf) Horz: 1-2=-13, 2-4=12, 5-7=-12, 7-8=-8

5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60

Vert: 1-2=12, 2-12=16, 4-12=21, 4-5=21, 5-14=16, 7-14=21, 7-8=37, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-2=-20, 2-12=-25, 4-12=-30, 5-14=25, 7-14=30, 7-8=46

6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60

Vert: 1-2=-7, 2-4=-32, 4-5=-32, 5-7=-32, 7-8=-28, 2-10=-20, 9-10=-80, 7-9=-20

7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-28, 2-4=-32, 4-5=-32, 5-7=-32, 7-8=-7, 2-10=-20, 9-10=-80, 7-9=-20 Horz: 1-2=8, 2-4=12, 5-7=-12, 7-8=13

8) Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf) Vert: 1-2=15, 2-4=3, 4-5=14, 5-7=9, 7-8=4, 2-10=-12, 9-10=-72, 7-9=-12

Horz: 1-2=-24, 2-4=-11, 5-7=17, 7-8=13

9) Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=4, 2-4=9, 4-5=14, 5-7=3, 7-8=15, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-2=-13, 2-4=-17, 5-7=11, 7-8=24

10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-24, 2-4=-28, 4-5=-21, 5-7=-12, 7-8=-7, 2-10=-20, 9-10=-80, 7-9=-20

- Horz: 1-2=4, 2-4=8, 5-7=8, 7-8=13 11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
- Uniform Loads (plf)

Vert: 1-2=-7, 2-4=-12, 4-5=-21, 5-7=-28, 7-8=-24, 2-10=-20, 9-10=-80, 7-9=-20 Horz: 1-2=-13, 2-4=-8, 5-7=-8, 7-8=-4

12) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=28, 2-4=15, 4-5=15, 5-7=15, 7-8=28, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-2=-37, 2-4=-24, 5-7=24, 7-8=37

13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=15, 2-4=3, 4-5=3, 5-7=3, 7-8=15, 2-10=-12, 9-10=-72, 7-9=-12

Horz: 1-2=-24, 2-4=-11, 5-7=11, 7-8=24 14) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-16, 2-4=-21, 4-5=-21, 5-7=-21, 7-8=-16, 2-10=-20, 9-10=-80, 7-9=-20

- Horz: 1-2=-4, 2-4=1, 5-7=-1, 7-8=4 15) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
  - Uniform Loads (plf) Vert: 1-2=-16, 2-4=-21, 4-5=-21, 5-7=-21, 7-8=-16, 2-10=-20, 9-10=-80, 7-9=-20
    - Horz: 1-2=-4, 2-4=1, 5-7=-1, 7-8=4
- 16) Dead + Uninhabitable Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-4=-20, 4-5=-20, 5-8=-20, 2-10=-40, 10-15=-100, 15-16=-120, 9-16=-100, 7-9=-40
- 17) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-53, 2-4=-56, 4-5=-51, 5-7=-44, 7-8=-40, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35 Horz: 1-2=3, 2-4=6, 5-7=6, 7-8=10

18) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-40, 2-4=-44, 4-5=-51, 5-7=-56, 7-8=-53, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35 Horz: 1-2=-10, 2-4=-6, 5-7=-6, 7-8=-3

19) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60 Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-47, 2-4=-51, 4-5=-51, 5-7=-51, 7-8=-47, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35

Horz: 1-2=-3, 2-4=1, 5-7=-1, 7-8=3

20) Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60. Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=-47, 2-4=-51, 4-5=-51, 5-7=-51, 7-8=-47, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35 Horz: 1-2=-3, 2-4=1, 5-7=-1, 7-8=3

21) Dead + 0.6 C-C Wind Min. Down: Lumber Increase=1.60, Plate Increase=1.60

Uniform Loads (plf)

Vert: 1-2=8, 2-4=-25, 4-5=-25, 5-8=-25, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-2=-16, 2-4=16, 5-8=-16



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai
					T35970823
6243111	G02	Hip	1	1	
					Job Reference (optional)
Tibbetts Lumber Co., LLC (C	cala, FL), Ocala, FL - 34	472,	8	.730 s Deo	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:23 2025 Page 3

8.730 s Dec 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:23 2025 Page 3 ID:nV5ZFUJaGJLKOI1jrAiSDcyHym-5wrWOBaFeJvzqcgW9sghCiskL21pKdGsi023Rtzy5JQ

LOAD CASE(S) Standard

- 22) Dead + 0.6 C-C Wind Min. Upward: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
  - Vert: 1-4=8, 4-5=8, 5-8=8, 2-10=-12, 9-10=-72, 7-9=-12 Horz: 1-4=-16, 5-8=16
- 23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-4=-60, 4-5=-60, 5-8=-20, 2-10=-20, 9-10=-80, 7-9=-20
- 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-4=-20, 4-5=-60, 5-8=-60, 2-10=-20, 9-10=-80, 7-9=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-4=-50, 4-5=-50, 5-8=-20, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35
- 26) 4th Dead + 0.75 Roof Live (unbalanced) + 0.75 Uninhab. Attic Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-4=-20, 4-5=-50, 5-8=-50, 2-10=-35, 10-15=-95, 15-16=-110, 9-16=-95, 7-9=-35





	6-3-11		13-8-5	1	20-0-0
	6-3-11	1	7-4-10	1	6-3-11 '
Plate Offsets (X,Y)-	[9:0-4-0,0-3-0]				
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr NO Code FBC2023/TPI2014	CSI. TC 0.72 BC 0.64 WB 0.17 Matrix-S	DEFL.         in         (loc)           Vert(LL)         -0.05         8-9           Vert(CT)         -0.36         8-9           Horz(CT)         0.04         6           Wind(LL)         0.03         8-9	l/defl L/d >999 360 >648 240 n/a n/a >999 240	PLATES         GRIP           MT20         244/190           Weight: 97 lb         FT = 20%

TOP CHORD

BOT CHORD

- LUMBER-
- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP M 31 or 2x4 SP SS
- WEBS 2x4 SP No.2
- REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=99(LC 11)
  - Max Grav 2=1139(LC 1), 6=1139(LC 1)
- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-1927/0, 3-4=-1718/0, 4-5=-1718/0, 5-6=-1927/0
- BOT CHORD 2-9=0/1659, 8-9=0/1062, 6-8=0/1659
- WEBS 4-8=0/719, 5-8=-268/183, 4-9=0/719, 3-9=-268/183

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 10-0-0, Zone2 10-0-0 to 14-2-15, Zone1 14-2-15 to 22-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

#### LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
  - Uniform Loads (plf)
- Vert: 1-4=-60, 4-7=-60, 2-9=-20, 8-9=-80, 6-8=-20
- 2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-4=-50, 4-7=-50, 2-9=-20, 8-9=-80, 6-8=-20
- Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-4=-20, 4-7=-20, 2-9=-40, 8-9=-100, 6-8=-40
- 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60





Structural wood sheathing directly applied or 3-5-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
				-		T35970824
6243111	G03	Common	2	1		
					Job Reference (optional)	
Tibbetts Lumber Co., LLC (C	ocala, FL), Ocala, FL - 34	472,	8	.730 s Dec	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:23 2025	Page 2

ID:nV5ZFUJaGJLKOI1jrAiSDcyHyrn-5wrWOBaFeJvzqcgW9sghCisi521aKdCsi023Rtzy5JQ

LC	DAD CASE(S) Standard
	Uniform Loads (plf)
	Vert: 1-2=47, 2-10=32, 4-10=19, 4-12=26, 6-12=19, 6-7=14, 2-9=-12, 8-9=-72, 6-8=-12
5)	$\Pi 012$ , $1-2=-50$ , $2-10=-40$ , $4+10=-21$ , $4+12=50$ , $6+12=27$ , $6+12=50$
0)	Uniform Loads (olf)
	Vert: 1-2=14, 2-11=19, 4-11=26, 4-13=19, 6-13=32, 6-7=47, 2-9=-12, 8-9=-72, 6-8=-12
	Horz: 1-2=-23, 2-11=-27, 4-11=-35, 4-13=27, 6-13=40, 6-7=56
6)	Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-8, 2-4=-33, 4-6=-33, 6-7=-29, 2-9=-20, 8-9=-80, 6-8=-20
7)	HOTZ: $1-2=-12$ , $2-4=13$ , $4-5=-13$ , $5-7=-9$
')	Dead + 0.0 C-C Wind (Neg. Internal) Case 2. Lumber increase= 1.00, Flate increase= 1.00
	Vert: 1-2=-29, 2-4=-33, 4-6=-33, 6-7=-8, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=9, 2-4=13, 4-6=-13, 6-7=12
8)	Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=15, 2-4=3, 4-6=9, 6-7=4, 2-9=-12, 8-9=-72, 6-8=-12
~	Horz: 1-2=-24, 2-4=-11, 4-6=17, 6-7=13
9)	Dead + 0.6 MWFRS Wind (Pos. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Vinioni Loads (pii) Vort 1.2-4 2.4-9 4.6-3 6.7-15 2.9-12 8.9-72 6.8-12
	Horz 1-2-13 2-4-17 4-6-11 6-7-24
10	) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-24, 2-4=-28, 4-6=-12, 6-7=-7, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=4, 2-4=8, 4-6=8, 6-7=13
11	) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (pii) Vort: $42 = 7, 24 = 12, 45 = 28, 57 = 24, 20 = 20, 80 = 80, 58 = 20$
	Volt. $1^{-2}-1^{-2}, 2^{-4}-1^{-2}, 4^{-2}-2^{-2}, 0^{-2}-2^{-2}, 0^{-2}-0^{-2}, 0^{-2}-2^{-2}, 0^{-2}-2^{-2}$
12	) Dead + 0.6 MWFRS Wind (Pos. Internal) 1st Parallel: Lumber Increase=1.60. Plate Increase=1.60
	, Uniform Loads (plf)
	Vert: 1-2=28, 2-4=15, 4-6=15, 6-7=28, 2-9=-12, 8-9=-72, 6-8=-12
	Horz: 1-2=-37, 2-4=-24, 4-6=24, 6-7=37
13	) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=15, 2-4=3, 4-6=3, 6-7=15, 2-9=-12, 8-9=-72, 6-8=-12
1.1	HOTZ: 1-2=-24, 2-4=-11, 4-6=11, 6-7=24
14	Dead + 0.0 MWYRS WIND (Neg. Internal) ist Paraller. Lumber increase=1.00, Plate increase=1.00
	Vert: 1-2=-16, 2-4=-21, 4-6=-21, 6-7=-16, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=-4, 2-4=1, 4-6=-1, 6-7=4
15	) Dead + 0.6 MWFRS Wind (Neg. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-16, 2-4=-21, 4-6=-21, 6-7=-16, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=-4, 2-4=1, 4-6=-1, 6-7=4
16	) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
	Unit Loads (pi) Vart: 1.420 4-720 2-920 8-980 6-820
17	) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWERS Wind (Neg. Int) Left): Lumber Increase=1.60. Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-53, 2-4=-56, 4-6=-44, 6-7=-40, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=3, 2-4=6, 4-6=6, 6-7=10
18	) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
	Vert: 1-2=-40, 2-4=-44, 4-0=-50, 0-7=-53, 2-9=-20, 8-9=-80, 0-8=-20
19	1012. 172-10, 274-0, 470-0, 071-3
10	Uniform Loads (of)
	Vert: 1-2-47, 2-4=-51, 4-6=-51, 6-7=-47, 2-9=-20, 8-9=-80, 6-8=-20
	Horz: 1-2=-3, 2-4=1, 4-6=-1, 6-7=3
20	) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-2=-47, 2-4=-51, 4-6=-51, 6-7=-47, 2-9=-20, 8-9=-80, 6-8=-20
~	Horz: 1-2=-3, 2-4=1, 4-6=-1, 6-7=3
21	) Dead + 0.6 C-C Wind Win. Down: Lumber increase=1.60, Plate increase=1.60
	Vert 1-2=8 2-4=-25 4-7=-25 2-9=-12 8-9=-72 6-8=-12
	Horz: 1-2=-16, 2-4=-16, 4-7=-16
22	) Dead + 0.6 C-C Wind Min. Upward: Lumber Increase=1.60, Plate Increase=1.60
	Uniform Loads (plf)
	Vert: 1-4=8, 4-7=8, 2-9=-12, 8-9=-72, 6-8=-12
00	Horz: 1-4=-16, 4-7=16

23) 1st Dead + Root Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
					T	35970824
6243111	G03	Common	2	1		
					Job Reference (optional)	
6243111     G03     Common       Tibbetts Lumber Co., LLC (Ocala, FL),     Ocala, FL - 34472,		472,	8	3.730 s Deo	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:23 2025 I	Page 3

#### 8.730 s Dec 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:23 2025 Page 3 ID:nV5ZFUJaGJLKOI1jrAiSDcyHyrn-5wrWOBaFeJvzqcgW9sghCisi521aKdCsi023Rtzy5JQ

# LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-20, 2-9=-20, 8-9=-80, 6-8=-20 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-20, 4-7=-60, 2-9=-20, 8-9=-80, 6-8=-20

25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-50, 4-7=-20, 2-9=-20, 8-9=-80, 6-8=-20

26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-20, 4-7=-50, 2-9=-20, 8-9=-80, 6-8=-20





- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. 1-2=-1982/0, 2-3=-1759/0, 3-4=-1732/0, 4-5=-1942/0
- TOP CHORD BOT CHORD 1-8=0/1721 7-8=0/1075 5-7=0/1672
- WEBS 3-7=0/718, 4-7=-268/183, 3-8=0/732, 2-8=-301/191

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-1-12 to 3-1-12, Zone1 3-1-12 to 10-0-0, Zone2 10-0-0 to 14-2-15 Zone1 14-2-15 to 22-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Load case(s) 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

# LOAD CASE(S) Standard

- 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
  - Uniform Loads (plf)
- Vert: 1-3=-60, 3-6=-60, 1-8=-20, 7-8=-80, 5-7=-20
- 2) Dead + 0.75 Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-3=-50, 3-6=-50, 1-8=-20, 7-8=-80, 5-7=-20 3) Dead + Uninhabitable Attic Without Storage: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-3=-20, 3-6=-20, 1-8=-40, 7-8=-100, 5-7=-40
- 4) Dead + 0.6 C-C Wind (Pos. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
6243111	G04	Common	3	1		T35970825
0210111			•		Job Reference (optional)	

Tibbetts Lumber Co., LLC (Ocala, FL), Ocala, FL - 34472,

Horz: 1-3=-16, 3-6=16

23) 1st Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25

8.730 s Dec 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:24 2025 Page 2 ID:nV5ZFUJaGJLKOI1jrAiSDcyHyrn-Z6PubXbtPc1qSmFiiaBwlwOtlSNq34O?xgnczJzy5JP

LOAD CASE(S) Standard
Uniform Loads (pir) Vert: 1-9=32, 3-9=19, 3-11=26, 5-11=19, 5-6=14, 1-8=-12, 7-8=-72, 5-7=-12
Horz: 1-9=-40, 3-9=-27, 3-11=35, 5-11=27, 5-6=23
5) Dead + 0.6 C-C Wind (Pos. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-10=19, 3-10=26, 3-12=19, 5-12=32, 5-6=47, 1-8=-12, 7-8=-72, 5-7=-12
Horz: 1-10=-27, 3-10=-35, 3-12=27, 5-12=40, 5-6=56 6) Dead + 0.6 C-C Wind (Neg. Internal) Case 1: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-3=-33, 3-5=-33, 5-6=-29, 1-8=-20, 7-8=-80, 5-7=-20 Horz: 1-3=13, 3-5=-13, 5-6=-9
7) Dead + 0.6 C-C Wind (Neg. Internal) Case 2: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf) Vert: 1-3=-33. 3-5=-33. 5-6=-8. 1-8=-20. 7-8=-80. 5-7=-20
Horz: 1-3=13, 3-5=-13, 5-6=12
<ol> <li>Dead + 0.6 MWFRS Wind (Pos. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)</li> </ol>
Vert: 1-3=3, 3-5=9, 5-6=4, 1-8=-12, 7-8=-72, 5-7=-12
Horz: 1-3=-11, 3-5=17, 5-6=13 9) Dead + 0.6 MWERS Wind (Pos. Internal) Right: Lumber Increase=1.60. Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-3=9, 3-5=3, 5-6=15, 1-8=-12, 7-8=-72, 5-7=-12 Horz: 1-3=-17, 3-5=11, 5-6=24
10) Dead + 0.6 MWFRS Wind (Neg. Internal) Left: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-3=8, 3-5=8, 5-6=13
11) Dead + 0.6 MWFRS Wind (Neg. Internal) Right: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-3=-12, 3-5=-28, 5-6=-24, 1-8=-20, 7-8=-80, 5-7=-20
Horz: 1-3=-8, 3-5=-8, 5-6=-4 12) Dead + 0.6 MWERS Wind (Pos. Internal) 1st Parallel: Lumber Increase-1.60. Plate Increase-1.60
Uniform Loads (plf)
Vert: 1-3=15, 3-5=15, 5-6=28, 1-8=-12, 7-8=-72, 5-7=-12
13) Dead + 0.6 MWFRS Wind (Pos. Internal) 2nd Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Horz: 1-3=-11, 3-5=11, 5-6=24
14) Dead + 0.6 MWFRS Wind (Neg. Internal) 1st Parallel: Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-3=-21, 3-5=-21, 5-6=-16, 1-8=-20, 7-8=-80, 5-7=-20
Horz: 1-3=1, 3-5=-1, 5-6=4 15) Dead + 0.6 MWERS Wird (Neg. Internet) 2nd Percellel: Lymber Increase, 1.60, Piete Increase, 1.60
Uniform Loads (plf)
Vert: 1-3=-21, 3-5=-21, 5-6=-16, 1-8=-20, 7-8=-80, 5-7=-20
16) Dead: Lumber Increase=0.90, Plate Increase=0.90 Plt. metal=0.90
Uniform Loads (plf)
vert: 1-3=-20, 3-6=-20, 7-6=-20, 7-6=-80, 5-7=-20 17) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Left): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
vert: 1-3=-56, 3-5=-44, 5-6=-40, 1-8=-20, 7-8=-80, 5-7=-20 Horz: 1-3=6, 3-5=6, 5-6=10
18) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (pir) Vert: 1-3=-44, 3-5=-56, 5-6=-53, 1-8=-20, 7-8=-80, 5-7=-20
Horz: 1-3=-6, 3-5=-6, 5-6=-3
19) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-3=-51, 3-5=-51, 5-6=-47, 1-8=-20, 7-8=-80, 5-7=-20
20) Dead + 0.75 Roof Live (bal.) + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60
Vert: 1-3=-51, 3-5=-51, 5-6=-47, 1-8=-20, 7-8=-80, 5-7=-20 Horz: 1-3=1, 3-5=-1, 5-6=3
21) Dead + 0.6 C-C Wind Min. Down: Lumber Increase=1.60, Plate Increase=1.60
Unirorm Loads (pit) Vert: 1-3=-25, 3-6=-25, 1-8=-12, 7-8=-72, 5-7=-12
Horz: 1-3=16, 3-6=-16
22) Dead + 0.5 C-C Wind Min. Upward: Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)
Vert: 1-3_8, 3-6=8, 1-8=-12, 7-8=-72, 5-7=-12



Job	Truss	Truss Type	Qty	Ply	1755-D-14x10 Lanai	
					T	35970825
6243111	G04	Common	3	1		
					Job Reference (optional)	
Tibbetts Lumber Co., LLC (C	cala, FL), Ocala, FL - 34	472,		.730 s Deo	c 5 2024 MiTek Industries, Inc. Mon Jan 6 11:17:24 2025 F	Page 3

ID:nV5ZFUJaGJLKOI1jrAiSDcyHyrn-Z6PubXbtPc1qSmFiiaBwlwOtlSNq34O?xgnczJzy5JP

#### LOAD CASE(S) Standard

Uniform Loads (plf)

- Vert: 1-3=-60, 3-6=-20, 1-8=-20, 7-8=-80, 5-7=-20 24) 2nd Dead + Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
  - Vert: 1-3=-20, 3-6=-60, 1-8=-20, 7-8=-80, 5-7=-20
- 25) 3rd Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)
- Vert: 1-3=-50, 3-6=-20, 1-8=-20, 7-8=-80, 5-7=-20
- 26) 4th Dead + 0.75 Roof Live (unbalanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-20, 3-6=-50, 1-8=-20, 7-8=-80, 5-7=-20





		0-0 <u>-1</u> 0-0-1	5	3-4-14 3-3-15					7-0-2 3-7-4		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/Tf	2-0-0 1.25 1.25 NO Pl2014	<b>CSI.</b> TC 0.9 BC 0.2 WB 0.0 Matrix-P	1 1 5 1 6 1	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.02 0.00 0.01	(loc) 6-7 6-7 5 6-7	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 33 lb	<b>GRIP</b> 244/190 FT = 20%

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.2 WFBS

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 5=Mechanical, 2=0-5-6 Max Horz 2=95(LC 27)

Max Uplift 4=-31(LC 8), 5=-53(LC 5), 2=-229(LC 8)

Max Grav 4=127(LC 19), 5=122(LC 3), 2=417(LC 31)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-346/120 BOT CHORD 2-7=-129/256 6-7=-129/256

WEBS 3-6=-274/138

#### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4, 53 lb uplift at joint 5 and 229 lb uplift at joint 2.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 87 lb down and 185 lb up at 1-4-15, 87 lb down and 185 lb up at 1-4-15, and 54 lb down and 23 lb up at 4-2-15, and 54 lb down and 23 lb up at 4-2-15 on top chord, and 8 lb up at 1-4-15, 8 lb up at 1-4-15, and 11 lb down and 24 lb up at 4-2-15, and 11 lb down and 24 lb up at 4-2-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-60, 2-5=-20 Concentrated Loads (lb) Vert: 8=123(F=62, B=62)



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





Scale = 1:19.5



	0 <u>r2-13</u> 0-2-13		4-11-4 4-8-7					7-8-9 2-9-5	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrNOCodeFBC2023/TPI2014	<b>CSI.</b> TC 0.77 BC 0.44 WB 0.08 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.02 -0.05 0.01 0.02	(loc) 2-6 2-6 5 2-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 34 lb	<b>GRIP</b> 244/190 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (size) 5=0-4-3, 2=0-9-14 Max Horz 2=54(LC 4) Max Uplift 5=-97(LC 4), 2=-199(LC 4) Max Grav 5=409(LC 1), 2=532(LC 1)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-3=-558/89

 BOT CHORD
 2-6=-102/510, 5-6=-102/510

WEBS 3-5=-539/107

#### NOTES-

 Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 5 and 199 lb uplift at joint 2.

6) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20 Concentrated Loads (lb) Vert: 8=-115(F=-58, B=-58) 10=-58(F=-29, B=-29)



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





Vert: 8=123(F=62, B=62) 9=-60(F=-30, B=-30) 11=-39(F=-20, B=-20)



January 7,2025





	0-1	4 ؛	5-0-0	1		9-0-0	1			13-10-12	14 <sub>1</sub> Q-0	
	0-1	-4 4-	-10-12	T		4-0-0	1			4-10-12	0-1-4	
Plate Offsets (2	X,Y) [3:	0-5-4,0-2-0]										
LOADING (ps	if)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.	.Ó	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	-0.03	7-8	>999	360	MT20	244/190
TCDL 10.	.0	Lumber DOL	1.25	BC	0.45	Vert(CT)	-0.06	7-8	>999	240		
BCLL 0.	.0 *	Rep Stress Incr	NO	WB	0.08	Horz(CT)	0.02	5	n/a	n/a		
BCDL 10.	.0	Code FBC2023/T	PI2014	Matr	ix-S	Wind(LL)	0.03	2-8	>999	240	Weight: 63 lb	FT = 20%
LUMBER-	I					BRACING-						

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 5=0-3-8 Max Horz 2=-58(LC 25) Max Uplift 2=-216(LC 8), 5=-216(LC 8) Max Grav 2=913(LC 1), 5=913(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1320/213, 3-4=-1116/205, 4-5=-1321/212

BOT CHORD 2-8=-146/1104, 7-8=-146/1116, 5-7=-141/1104

WEBS 3-8=0/333, 4-7=0/334

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 2 and 216 lb uplift at joint 5.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 151 lb down and 120 lb up at 5-0-0, and 76 lb down and 53 lb up at 7-0-0, and 151 lb down and 120 lb up at 9-0-0 on top chord, and 208 lb down at 5-0-0, and 56 lb down at 7-0-0, and 208 lb down at 8-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 4-6=-60, 2-5=-20 Concentrated Loads (lb) Vert: 3=-95(B) 4=-95(B) 8=-99(B) 7=-99(B) 10=-55(B) 12=-28(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria **and DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



Structural wood sheathing directly applied or 4-10-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	0- <u>1-4</u> 0-1-4	7-0-0 6-10-12	2				<u>13-10</u> 6-10-	<u>-12</u> 12	14 <sub>T</sub> ( 0-1	0-0 -4
Plate Offsets (X,Y)	- [2:0-2-0,Edge], [4:0-2-0,	Edge]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.60	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
CDL 10.0	Lumber DOL	1.25	BC 0.52	Vert(CT)	-0.12	2-6	>999	240		
CLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.01	4	n/a	n/a		
3CDL 10.0	Code FBC2023/1	PI2014	Matrix-S	Wind(LL)	0.07	4-6	>999	240	Weight: 56 lb	FT = 20%
UMBER-				BRACING-					1	
TOP CHORD 2x4	SP No.2			TOP CHOR	D	Structu	al wood s	sheathing dire	ectly applied or 5-7-6	oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=74(LC 11) Max Uplift 2=-202(LC 12), 4=-202(LC 12) Max Grav 2=677(LC 1), 4=677(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-737/313, 3-4=-737/313

BOT CHORD 2-6=-161/570, 4-6=-161/570

WEBS 3-6=-88/328

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 7-0-0, Zone2 7-0-0 to 11-2-15, Zone1 11-2-15 to 16-0-0 zone; cantilever left and right exposed ; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 2 and 202 lb uplift at joint 4.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 18 lb uplift at joint 3.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





**REACTIONS.** (size) 1=7-8-0, 3=7-8-0, 4=7-8-0

Max Horz 1=-27(LC 10)

Max Uplift 1=-17(LC 12), 3=-17(LC 12)

Max Grav 1=131(LC 1), 3=131(LC 1), 4=257(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 1 and 17 lb uplift at joint 3.



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January 7,2025





2x4 ⋍

2x4 📚

Rigid ceiling directly applied or 10-0-0 oc bracing.

-0 -0 	0 <u>78</u> 0-8		3-9-0 3-8-8	
late Offsets (X, Y)	[2:0-2-0,Edge]		1	1
OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.25	TC 0.04	Vert(LL) n/a - n/a 999	MT20 244/190
CDL 10.0	Lumber DOL 1.25	BC 0.07	Vert(CT) n/a - n/a 999	
CLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
CDL 10.0	Code FBC2023/TPI2014	Matrix-P		Weight: 10 lb $FT = 20\%$

BOT CHORD

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=3-8-0, 3=3-8-0

Max Horz 1=-10(LC 10) Max Uplift 1=-4(LC 12), 3=-4(LC 12)

Max Grav 1=100(LC 1), 3=100(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 1 and 4 lb uplift at joint 3.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





Scale = 1:14.2



		<mark>0-4-4   5-6-8                                      </mark>									
Plate Offsets (X,Y)	[2:0-4-11,0-2-8]										
LOADING         (psf)           TCLL         20.0           TCDL         10.0           BCLL         0.0         *           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	<b>CSI.</b> TC 0.42 BC 0.35 WB 0.00 Matrix-P	DEFL.         in         (loc)         //de           Vert(LL)         -0.05         2-5         >99           Vert(CT)         -0.09         2-5         >68           Horz(CT)         0.00         n         N           Wind(LL)         0.04         2-5         >98	efi L/d PLA 99 360 MT2 11 240 /a n/a 99 240 Weig	TES         GRIP           0         244/190           ght: 25 lb         FT = 20%						
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI SLIDER Left 22	P No.2 P No.2 P No.2 k4 SP No.2 2-1-12		BRACING- TOP CHORD Structural wo except end v BOT CHORD Rigid ceiling	ood sheathing directly applied erticals. directly applied or 10-0-0 oc	d or 5-6-8 oc purlins, bracing.						
REACTIONS. (siz	ze) 5=0-3-8, 2=0-3-8										

Max Horz 2=50(LC 12) Max Uplift 5=-45(LC 12), 2=-123(LC 12) Max Grav 5=184(LC 1), 2=365(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 5-4-12 zone; cantilever left
- exposed ; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 5 and 123 lb uplift at joint 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	0-4-4	4-10-8		5-6-8 <sub>I</sub>		9-6-8		
	0-4-4	4-6-4		0-8-0		4-0-0		
Plate Offsets (X,Y)	[2:0-5-3,0-1-8]							
LOADING(psf)TCLL20.0TCDL10.0BCLL0.0*	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	<b>CSI.</b> TC 0.33 BC 0.24 WB 0.07	DEFL.         in           Vert(LL)         -0.02           Vert(CT)         -0.04           Horz(CT)         0.00	n (loc) l/defl 2 2-8 >999 4 2-8 >999 0 6 n/a	L/d 360 240 n/a	<b>PLATES</b> MT20	<b>GRIP</b> 244/190	
BCDL 10.0	Code FBC2023/TPI2014	Matrix-S	Wind(LL) 0.03	3 2-8 >999	240	Weight: 46 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP	P No.2 P No.2		BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.					
WEBS 2x4 SP SLIDER Left 2x	9 No.2 4 SP No.2 1-10-14		BOT CHORD	Rigid ceiling dir	ectly applied or	10-0-0 oc bracing.		
REACTIONS. (size	e) 6=4-3-8. 2=0-3-8. 7=0-3-8							

Max Horz 2=74(LC 12) Max Uplift 6=-28(LC 12), 2=-131(LC 12), 7=-25(LC 12) Max Grav 6=178(LC 1), 2=379(LC 1), 7=312(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 9-4-12 zone; cantilever left
- exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 6, 131 lb uplift at joint 2 and 25 lb uplift at joint 7.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





	0-4-4				4-10-8				9-6-8				
Plate Offsets ()	X,Y)	[2:0-4-3,0-3-0]			4-0-4					4-8-	0		
LOADING (psi	f)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0 TCDL 10.0 BCLL 01	0 0 0 *	Plate Grip DOL Lumber DOL Rep Stress Incr	1.25 1.25 YES	TC BC WB	0.33 0.27 0.26	Vert(LL) Vert(CT) Horz(CT)	-0.02 -0.04 0.01	7 2-7 6	>999 >999 n/a	360 240 p/a	MT20	244/190	
BCDL 10.	0	Code FBC2023/T	PI2014	Matri	x-S	Wind(LL)	0.03	2-7	>999	240	Weight: 46 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2						BRACING- TOP CHOR	D	Structur except e	ral wood end vertio	sheathing dir cals.	ectly applied or 6-0-0	oc purlins,	
SLIDER	Left 2x4	N0.2 4 SP No.2 1-10-14				BUTCHUR	D	Rigia ce	aling are	cuy applied c	or 8-2-11 oc bracing.		
REACTIONS.	(size Max Ho	e) 6=0-3-8, 2=0-3-8 orz 2=74(LC 12)											

Max Uplift 6=-95(LC 12), 2=-156(LC 12) Max Grav 6=355(LC 1), 2=514(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 2-4=-772/433

 BOT CHORD
 2-7=-512/708, 6-7=-512/708

WEBS 4-6=-707/504

# NOTES-

 Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 9-4-12 zone; cantilever left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 6 and 156 lb uplift at joint 2.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

January 7,2025





General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor1 bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



# \*\*\* Approved By: \_

Delivery Date:

Name

			BE		<b>7</b> <b>0</b> .,u				
	61 Phone	00 SE 68th e (352) 347	Stree -7661	et, Ocala Fax: (3	FL 344 47) 347	72 7797			
CLIENT APPROVAL	<ul> <li>*** Signature of this docu diagram in its entiret</li> <li>a) The client is responsil and scheduling. Any if from the client, will be Tibbetts Lumber Co.</li> <li>b) Design Criteria: The meets or exceeds the building requirement</li> <li>c) Fabrication and Deliv before fabrication and with the truss manufilevel and clear of mail driver's discretion. C:</li> <li>d) Installation &amp; Bracin; when handling, instal this truss package. The the requirements speed drawings. The overall</li> <li>e) Field Framing: 1.) Tranad valleys not show WOverhangs are 2x4 or beveled</li> <li>f) Repairs: Truss related Do Not Cut Any Truss modifications made wi or crane charges of an management.</li> <li>g) This Truss Placement purely to be used as a Design Drawings, wh</li> </ul>	ument acknowledges that the clien ty as in agreement with the followic ble to verify the accuracy of infor- labor, material or time delay incu- te or the client's expense. Any field- , LLC, are performed as a courter client acknowledges that the truss design criteria specified by the bu- s ery: One approved truss placement d delivery will be scheduled. It is acturer. The client shall provide a terials and debris. In lieu of this, 1 are and handling of the trusses for g: BCSI 2008 (Building Componen- ling & bracing trusses. Temporar russes shall be braced to prevent 1 clificd in the construction document 1 stability of the truss system is th ay ceilings and other ceiling trans n are to be field framed by others; r 2x6 - no blocking is applied. Con- l problems are to be reported to t ses before contacting the truss sys- thout an engineered repair drawi ny kind will be accepted unless spi- tion may be sealed by the Truss D	has reviewed thing terms, includir nation submitted rred from inadequ l measurements, by y to the client and design criteria no ilding designer, er the client's respon marked location russ will be delive lowing delivery is at Safety Informat y and/or permane otation and provi ts for the building e responsibility of tions my require 3.) Overhangs m ner jacks will be : the truss manufact unfacturer with sy ag will be the resp cifically approved ngineer, rather by require a seal. Tr esign Engineer.	s truss placement ig , but not limited to: for use in design, fabricatio rate or incorrect informatio y an associate of d shall be verified by the cl ted on this truss placement igneer of record, and local we returned to the truss ma- sibility to co-ordinate deliv for delivery, which must be red in the best available le the responsibility of the cl tion) WTCA/TPI guidelinese id lateral stability in accor g and on the individual tru the building designer. field framing by others. 2.) ay be over-length - cut to f square cut and hip jacks we urer ASAP, preferably in v possibility of the client. No to in writing by the truss ma- y Tibbetts Lumber Co, LLQ uss design analysis are on the state of the problem.	n supplied ient. diagram and state nufacturer er dates accessible, cation at our ient. shall be followed not included in dance with is design Ceiling drops it in the field. ill be double vriting. y field back charges nufacturer's C staff and is the Truss				
CRITERIA	Floor: Load Des Roof: Load Des Mitek Engineerir Building Code	I: 55# psf; 40 T0 sign checked for : 40# psf; 20 TC sign checked for ng : FBC 2023 : ASCE 7-2 : TPI 1-201	CLL, 10 7 10 psf no LL, 10 T 10 psf no 2 4	CDL, 00 BC on-concurrent CDL, 00 BCL on-concurrent Exposure Mean Heig Bldg. Cate Importanc	LL, 05 BCD LL on BC. L, 10 BCDL LL on BC LL on BC : ght : gory : e Factor :	L; Dur.: 1.0 .; Dur.: 1.25 B ≤ 15' II 1.00	0		
U       Truss Design       :       Comp. & Cladding       Enclosure       :       Enclosure         U       Uplift Calculations       :       MWFRS       Entry       :       Expo         U       Wind Speed       :       130 mph US       Lanai       :       Expo         N       ROOF CRITERIA       FLOOR CRITERIA       FLOOR CRITERIA       FLOOR CRITERIA							o Wind o Wind		
TYPICAL	T.C. Pitch B.C. Pitch T.C. Size Heel Height	: 6/12 : 3/12 : 2x4 : 4 3/16"	T.C Dep Spa Bea	S. Size oth acing aring		PC42 16" 16" O.C. 8"			
	Cantilever Overhang O.H. Cut Spacing Lumber	: 4 : 0 : 24" : Plumb : 24" O.C : SP	Vap Floo bloc verti floor	or barrier bet r trusses held k and fill by c cal load from trusses arou	ween floor & l back 3/4" ther. Blocki above by c nd plumbin	& concrete b at exterior v ng for trans others. Odd g as noted.	y other. /all, fer of space		
CONNECTORS	Roof I A TYP: THD26 *a JUS24 B THD26-2 C THDH26-2 D THDH26-3 E THD28 F THDH28 Installation sha downs other the by others.	russ to Truss CoGTHDH28-2HTHDH28-3ITHDH210-3JGTWS2TKGTWS3TLGTWS4TII be per connectan truss to girde	M N B O or manu	s F Q R S T U V facturer's guid	loor Truss t TYP: THD4 THDH46 THD48 THDH48 THDH410 THDH610 MSH422 delines. All to be spec	o Truss Cor 6 W MSF X MSF Y MSF Z connectors a ified and su	Anectors 1422IF 1426 1426IF 1426IF and tie oplied		
UPLIFT SUMMARY	1 2 3 4 5 6 7 8 9 10 Only p	11 12 13 14 14 15 16 16 17 18 19 20 points listed abov	/e have i	reactions > 50	21 22 23 24 25 26 27 28 27 28 29 30 29 30	ift > 1000#.			
NOTES	N1       .         N2       .         N3       .         N4       .         N5       .         N6       .         N7       .         N8       .         N9       .         Diamond indicates left side of truss on truss design drawings								
Client Info	Address: Lo	t# 094 The Prese ke City . Florida	<b>D-Frar</b> erve at L	<b>NE</b> aurel Lake					
Rev.									
	Date Revised	: 1/06/25 : .	S D	cale rawn By	: 1/4" : Ste	= 1'-0" ve R.	D= 1/4		
	Sheet #	: 1 of 1	Jo	ob #	: 62	243111			

Approval Date

						Tibbotts Lumbor Ocala							Reaction Summary						
	/2						IDDetts	s Lum	IDe	er Ocala         Job Number: 6243111-F						3111-R			
			ام بلے				61	00 SE 6	8th S	St			Quoted On:						
Ucala, FL 34						3447	472 Ordered On: 11/4/2024												
LUMBER CO.							/ 00 1 Scheduled Delivery On:												
WWW.TIBBETTSLUMBER.COM								Product: Roof											
Customer Information Adams Homes of NW FL - Gainesville								J	Job Information The Preserve at Laurel Lake 094										
Address & Phone Co					Con	tact	_ ^	Address			Lot Sub-Division								
							742 SW Rosemary Dr				Sales Person Customer P.O. No.								
Phone:	Phone:					Lake City 32024					Estimator Designer								
										S	teven Robe	rts	Steven Ro	berts					
	Loading			Idina Ca	ode		Wind	Doci	ian Mot	hod			Velocity	Exp Cat	Wi	nd Ma	x		
TCLL	TCDL	BCLL	BCDL	Bui		Jue		wind	Desi	grimet	nou			velocity	Occ Cat	TCDL	В	SCDL	
20	10	0	10	FBC2	2023/TPI	2014	MWFRS (I	Directiona	al)/C-0	C hybrid	l Wir	nd ASCE 7-22	2	130 mph		4.2		6	
Root I	russe	es			0.54	Snan	TC Ditch												
label		Pr	ofilo			Height	BC Pitch	BC					Ro	actions					
Laber				-	1	32-00-00	6 /12	2 x 4	Join	nt 2 Jo	int 8		ite	actions					
A01					2-ply	4-09-15		2 x 4	24 -1	91 2 53	2399								
A02					1	32-00-00	6 /12	2 x 4	Join	nt 2 Jo	int 8								
	$\sim$				1-ply	5-09-15		2 x 4	14 -1	101 <sup>-</sup> 118	1264 -53								
A03			$\overline{\mathbf{X}}$	¥	1	32-00-00	6 /12	2 x 4	Join	nt 2 Jo	int 8								
	$\sim$				1-ply	6-09-15		2 x 4	-1	118	-53								
A04		$\sqrt{2}$		$\mathbf{h}$	1	32-00-00	6/12	$2 \times 4$	Join 14	nt 2 Jo 111 ·	int 8 1277								
					1-piy	32-00-00	6/12	$2 \times 4$	-1	113	-45								
A05	A05		1-plv	7-10-03	3/12	2 x 4	12	274	1276										
			2	32-00-00	6 /12	2 x 4	Join	-52 nt 1 Joir	-51 nt 10										
A06		5	M2		1-ply	8-04-03	3 /12	2 x 4	12	.74 <sup>·</sup>	-51								
A07					1	32-00-00	6 /12	2 x 4	Join	nt 1 Jo	int 8								
		$\sim$			1-ply	8-04-03	3 /12	2 x 4	12	-51	-49								
A08		$\wedge$		•	1	32-00-00	6 /12	2 x 4	Join 12	nt1 Jo 274 ·	int 8 1279								
		3-	<u></u> _		1-ply	8-04-03	3/12	$2 \times 4$	-	-52	-49								
A09		Æ	$\mathbb{N}$		I 1-nlv	20-00-00	3/12	$2 \times 4$ $2 \times 4$	Join 11	108	1108								
					1-piy	28-00-00	6/12	2 x 4	Join	-46 nt 1 Jo	-50 int 8								
A10		Æ	$\sim$	$\mathbf{\Sigma}$	1-ply	8-04-03	3 /12	2 x 4	11	108	1108								
					1	28-00-00	6 /12	2 x 4	Join	nt 1 Ja	int 8								
		A	$\sim$		1-ply	8-04-03	3 /12	2 x 4	11	108 -46	1108 -50								
A12		-	$\checkmark$	~	2	28-00-00	6 /12	2 x 4	Join	nt 2 Jo	int 9								
			$\sim$	2	1-ply	9-03-15	3 /12	2 x 4	-1	110	-47								
A13		s			2 1 ph/	32-00-00	6/12	$2 \times 4$	Join 14	nt 2 Jo 107 <sup>/</sup>	int 9 I271								
		-			1-piy	32-00-00	6/12	$2 \times 4$	-1	115 112 Jo	-48 int 9								
A14		AS	$\mathbb{A}$		1-plv	8-09-15	3/12	2 x 4	14	105	1273								
					1	32-00-00	6 /12	2 x 4	-1 Join	116 nt 2 Jo	-47 int 9								
A15		$\sim$	Ś	$\sim$	1-ply	7-09-15	3 /12	2 x 4	14 _1	105 ·	1273 -47								
A16	-				1	32-00-00	6 /12	2 x 4	Join	- nt 2 Jo	int 8								
	$\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ $				1-ply	6-09-15		2 x 4	15: 1	118	-53								
A17					1	32-00-00	6 /12	2 x 4	Join 14	nt 2 Jo	int 8 1264								
			<u> </u>		1-ply	5-09-15	6 /10	2 x 4	-1	110	-61								
A18					2-nlv	32-00-00 <u>4-00-15</u>	0/12	2×6	Join 24	ແ2 Jo 168 2	int 8 2649								
	<u> </u>		~		2-piy 10	1-00-00	6 /12	2 x 4	-1 Join	61 nt 2 . In	-183 int 3	Joint 4							
C1					1-ply	1-09-15		2 x 4	2	289	67	19							
I	1					1			-1	53	-100	o							



6243111-R Roof Trusses

# The Preserve at Laurel Lake 094

		Qty	Span	TC Pitch	тс				
Label	Profile	Ply	Height	BC Pitch	BC				Reactions
C1I		4	1-00-00	6 /12	2 x 4	Joint 2	Joint 3	Joint 4	
0.12		1-ply	1-09-15		2 x 4	-142	-100	-2	
C3		9	3-00-00	6 /12	2 x 4	Joint 2	Joint 3	Joint 4	
		1-ply	2-09-15		2 x 4	-85	-14	17	
СЗВ		1	3-00-00	6 /12	2 x 4	Joint 1	Joint 2	Joint 3	
		1-ply	1-10-03		2 x 4	5	-31	17	
C3C		2	3-00-00	3 /12	2 x 4	Joint 2	Joint 3	Joint 4	
		1-ply	1-06-10		2 x 4	-111	-9	-7	
C3L		4	3-00-00	6 /12	2 x 4	Joint 2	Joint 3	Joint 4	
		1-ply	2-09-15		2 x 4	-109	-14	-7	
C5		9	5-00-00	6 /12	2 x 4	Joint 2 349	Joint 3	Joint 4	
		1-ply	3-09-15		2 x 4	-70	-36	29	
C5B		1	5-00-00	6 /12	2 x 4	Joint 1 192	Joint 2 144	Joint 3 96	
		1-ply	2-10-03		2 x 4	7	-53	29	
C5C		2	5-00-00	3 /12	2 x 4	Joint 2 349	Joint 3 115	Joint 4 96	
		1-ply	2-00-10		2 x 4	-120	-27	-12	
E5L		3	5-00-00	6/12	2 x 4	Joint 2 349	Joint 3 115	Joint 4 96	
		1-ply	3-09-15	0.//0	2 x 4	-111	-36	-12	
E7		27	7-00-00	6/12	2 x 4	Joint 2 421	Joint 3 183	Joint 4 136	
		1-ply	4-09-15	0.//0	2 x 4	-63	-62	41	
G01		1	20-00-00	6/12	2 x 4	Joint 2 1504	Joint 6 1528		
		1-ply	4-09-15	0.//0	2 x 4	-79	-88		
G02		1	20-00-00	6/12	2 X 4	Joint 2 1217	Joint / 1217		
		1-ріу	5-09-15	6/10	2 X 4	127	127		
G03		2 1 mh/	20-00-00	0/12	2 X 4	1139	1139		
		i-piy	20-00-00	6 /12	$2 \times 4$	127	127		
G04			6-03-15	0712	$2 \times 4$	1003	1146		
		1-piy 2	7-00-02	4 24 /12	$2 \times 4$	191 Joint 2	123	Joint 5	
H5L		1-ply	3-09-07	7.27/12	$2 \times 4$	417	127	122	
		1	7-08-09	2 12 /12	$2 \times 4$	-229	-31 Joint 5	-53	
H6C		1-ply	2-01-15	2.12/12	$2 \times 4$	532	409		
		5	9-10-01	4 24 /12	$2 \times 4$	-199 Joint 2	-97 Joint 4	Joint 5	
H7		1-plv	4-09-07	,	$2 \times 4$	582	165	272	
		1	14-00-00	6 /12	2 x 4	-170 Joint 2	-51 Joint 5	18	
L01		1-plv	3-09-15		2 x 4	913	913		
		2	14-00-00	6 /12	2 x 4	-216 Joint 2	-216 Joint 4		
L02		1-ply	4-09-15		2 x 4	677	677		
		1	11-09-00	6 /12	2 x 4	Joint 1	Joint 3	Joint 4	
LV1		1-ply	2-11-04		2 x 4	193	193	459	
1.1/0		1	7-09-00	6 /12	2 x 4	Joint 1	Joint 3	Joint 4	
		1-ply	1-11-04		2 x 4	131	131	257	
1.1/2		1	3-09-00	6 /12	2 x 4	Joint 1	Joint 3	11	
		1-ply	11-04		2 x 4	100	100		
M4		8	5-06-08	3 /12	2 x 4	Joint 2	Joint 5		
		1-ply	2-02-04		2 x 4	365 -123	184 -45		
Ma	/1	1	9-06-08	3 /12	2 x 4	Joint 2	Joint 6	Joint 7	
		1-ply	3-02-04		2 x 4	379 -131	178 -28	312 -25	
Ma		4	9-06-08	3 /12	2 x 4	Joint 2	Joint 6		
11/13		1-ply	3-02-04		2 x 4	514 -156	355 -95		