

* ELECTRIC PLAN *

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QP	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- → -*	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
	220v OUTLET
В ая	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$	4 WAY WALL SWITCH
W _{P/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
里	GARAGE DOOR OPENER
<u> </u>	WALL HEATER

Electrical Plan Notes

E-1 Wire all appliances, HVAC units and other equipment per manufactors specifications.

E-2 Consult the owner for the number of seperate telephone lines to be installed.

E-3 All installations shall be per national electric code.
E-4 All smoke detectors shall be 120V w/battery backup of the photoelectric type, and shall be interlocked together Install inside and near all bedrooms.

E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners direction and in accordance with applicable sections or national electrical code latest edition.
E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
E-7 Entry of service underground or overhead is to be determined by the power company.
E-8 All bedroom recepticals are to be AFCI (Arc Fault Circuit Interrupt).
E-9 All outside recepticals are to be weatherproofed GFIs.

"THE GRIFFIN" CORNERSTONE DEVELOPERS

Magnolia Hills Lot 10

LAKE CITY FLORIDA

Chris W. Cox 252 N.W. Ivy Glen Lake City, Florida 32055 386-752-1711 Office 386-867-0633 Cell

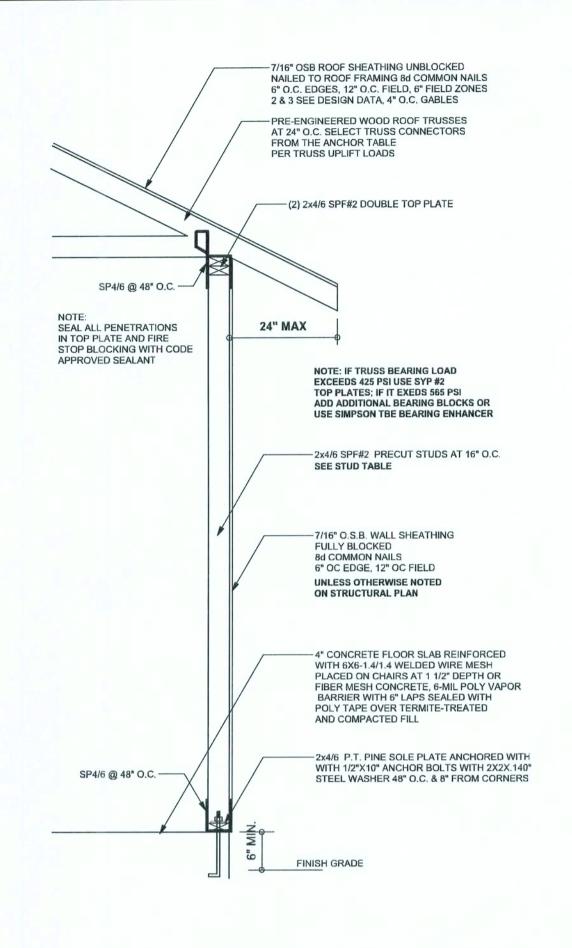
PRINTED DATE: September 06, 2006 DRAWN BY: CHECKED BY: Chris W. Cox

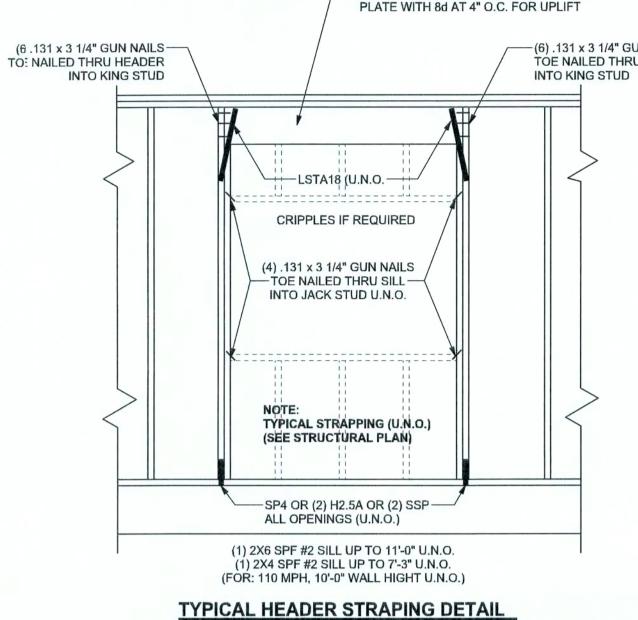
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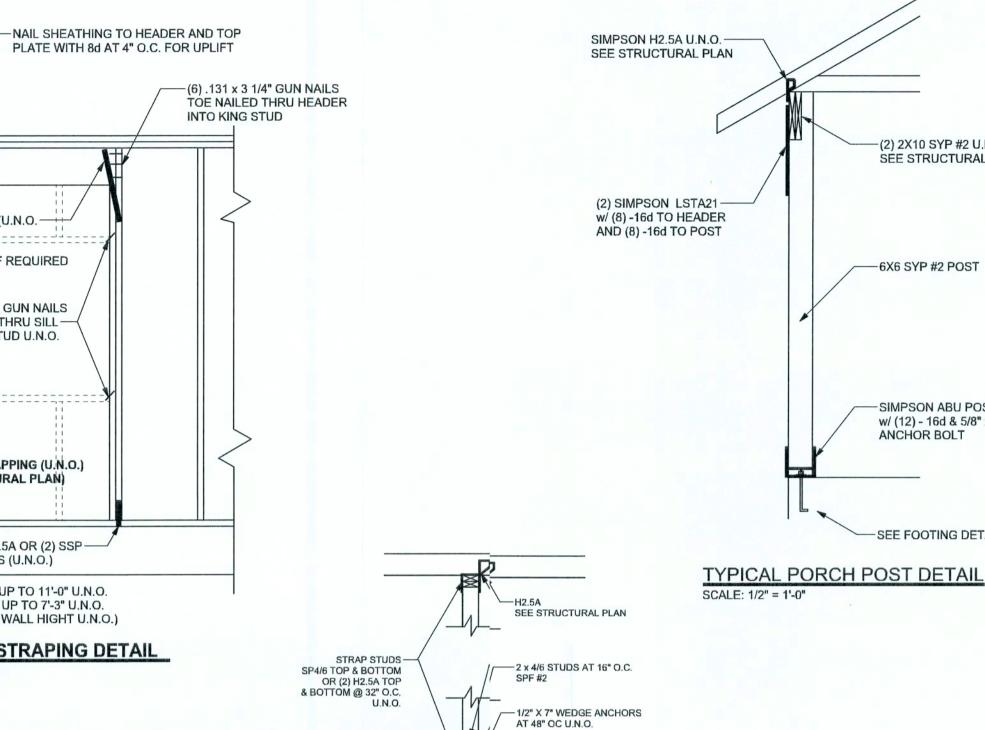
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3 OF 3 SHEETS



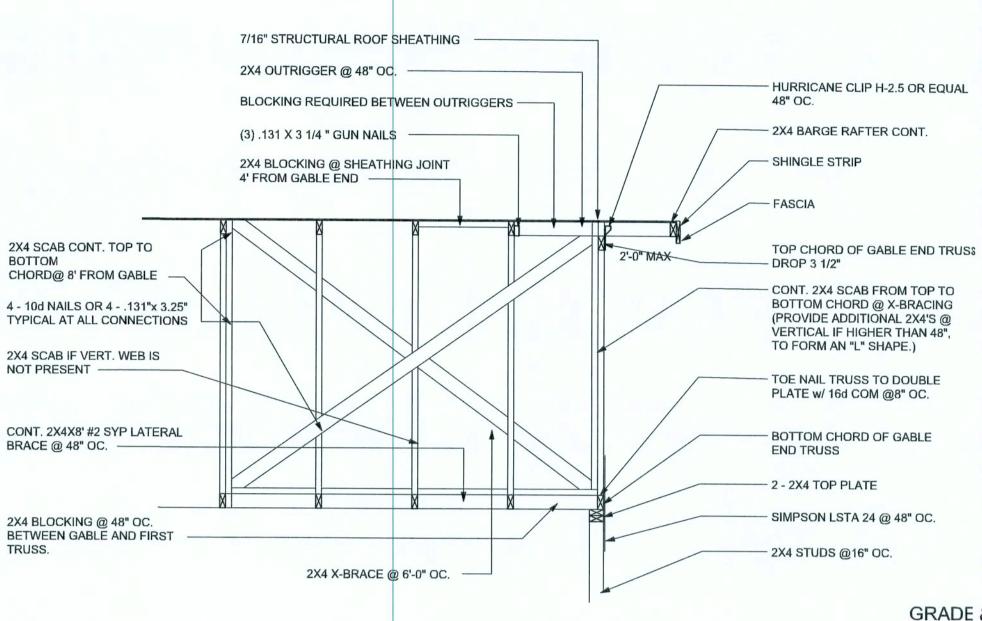




SEE FOUNDATION | DETAILS

INTERIOR BEARING WALL

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS



TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP

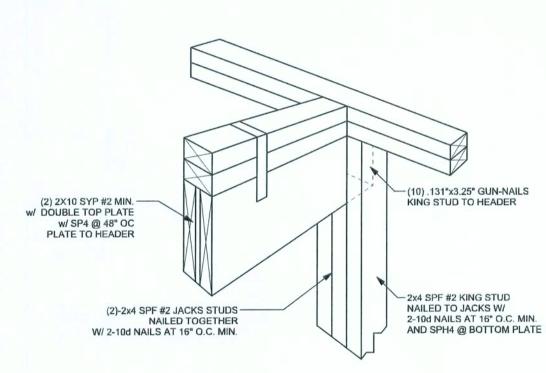
GRADE & SPECIES TABLE

Fb (psi) E (10⁶ psi) SYP #2 1200 1.6 2x10 SYP #2 1050 1.6 2x12 SYP #2 975 1.6 GLB 24F-V3 SP 2400 1.8 LSL TINBERSTRAND **UICROLAM** 2900 2.0 PARALAM 2900

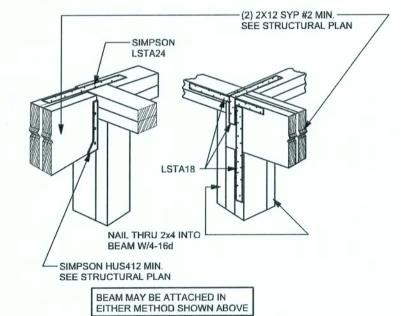
REVISIONS

SOFTPLAN

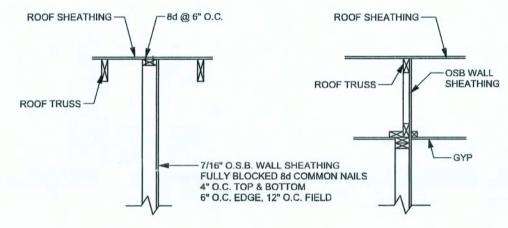
ONE STORY WALL SECTION

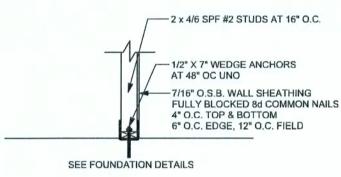


BEAM MID-WALL CONNECTION DETAIL



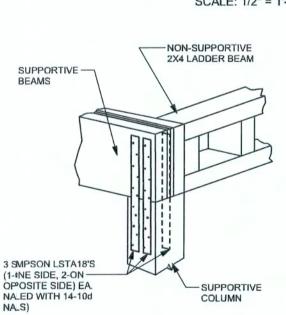
BEAM CORNER CONNECTION. DETAIL SCALE: N.T.S.



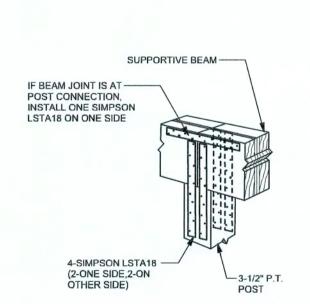


INTERIOR SHEAR WALL DETAIL SCALE: 1/2" = 1'-0" -NON-SUPPORTIVE 2X4 LADDER BEAM

SCALE: 1/2" = 1'-0"



SUPPORTIVE POST TO BEAM **DETAIL FOR SINGLE BEAM**



SUPPORTIVE CENTER POST TO BEAM DETAIL

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSSS MANUFACTURER'S ENGINEERING

< 420					
420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 11/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	1
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	-
< 2900	< 2490	2 - HTS24	12 144 1 1/2	12.174.112	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
			17 104	11 100	
		HEAVY (GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROE 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DCOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SIINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DCOUBLE TOP PLATE	6 -10d		8 -10d
-10-10-		DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 825	< 600				6-10d, 1 1/2"
< 825 < 885	< 600 < 760	SP4			0 100, 1 112
					10-10d, 1 1/2"
< 885	< 760	SP4			
< 885 < 1240	< 760 < 1065	SP4 SPH4			10-10d, 1 1/2"
< 885 < 1240 < 885	< 760 < 1065 < 760	SP4 SPH4 SP6	14-10d		10-10d, 1 1/2" 6-10d, 1 1/2"
< 885 < 1240 < 885 < 1240	< 760 < 1065 < 760 < 1065	SP4 SPH4 SP6 SPH6	14-10d 16-10d		10-10d, 1 1/2" 6-10d, 1 1/2"
< 885 < 1240 < 885 < 1240 < 1235	< 760 < 1065 < 760 < 1065 < 1165	SP4 SPH4 SP6 SPH6 LSTA18			10-10d, 1 1/2" 6-10d, 1 1/2"
< 885 < 1240 < 885 < 1240 < 825 < 1240 < 1235 < 1235	< 760 < 1065 < 760 < 1065 < 1165 < 1235	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21	16-10d		10-10d, 1 1/2" 6-10d, 1 1/2"
< 885 < 1240 < 885 < 1240 < 1240 < 1235 < 1030	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20	16-10d 18-8d		10-10d, 1 1/2" 6-10d, 1 1/2"
< 885 < 1240 < 885 < 1240 < 1240 < 1235 < 1030	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16	16-10d 18-8d 28-8d		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2"
< 885 < 1240 < 885 < 1240 < 1240 < 1235 < 1235 < 1030 < 1705	< 760 < 1065 < 760 < 1065 < 1065 < 1165 < 1235 < 1030 < 1705	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS*	16-10d 18-8d 28-8d TO STUDS		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030 < 1705	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19	16-10d 18-8d 28-8d TO STUDS 8-16d		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705 < 1350 < 2310	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030 < 1705 < 1305 < 2310	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19 LTT131	16-10d 18-8d 28-8d TO STUDS 8-16d 18-10d, 1 1/2"		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705 < 1350 < 2310 < 2775	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030 < 1705 < 1305 < 2310 < 2570	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19 LTT131 HD2A	16-10d 18-8d 28-8d TO STUDS 8-16d 18-10d, 1 1/2" 2-5/8" BOLTS		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB 1/2" AB 5/8" AB
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705 < 1350 < 2310 < 2775 < 4175	< 760 < 1065 < 760 < 1065 < 1165 < 1235 < 1030 < 1705 < 1305 < 2310 < 2570 < 3695	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19 LTT131 HD2A HTT16	16-10d 18-8d 28-8d TO STUDS 8-16d 18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB 1/2" AB 5/8" AB
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705 < 1350 < 2310 < 2775 < 4175 < 1400	< 760 < 1065 < 760 < 1065 < 1165 < 11235 < 1030 < 1705 < 1305 < 2310 < 2570 < 3695 < 1400	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19 LTT131 HD2A HTT16 PAHD42 HPAHD22	16-10d 18-8d 28-8d TO STUDS 8-16d 18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d 16-16d 16-16d		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB 1/2" AB 5/8" AB 5/8" AB
< 885 < 1240 < 885 < 1240 < 1235 < 1235 < 1030 < 1705 < 1350 < 2310 < 2775 < 4175 < 1400 < 3335	< 760 < 1065 < 760 < 1065 < 1065 < 1165 < 1235 < 1030 < 1705 < 1305 < 2310 < 2570 < 3695 < 1400 < 3335	SP4 SPH4 SP6 SPH6 LSTA18 LSTA21 CS20 CS16 STUD ANCHORS* LTT19 LTT131 HD2A HTT16 PAHD42	16-10d 18-8d 28-8d TO STUDS 8-16d 18-10d, 1 1/2" 2-5/8" BOLTS 18 - 16d 16-16d		10-10d, 1 1/2" 6-10d, 1 1/2" 10-10d, 1 1/2" TO FOUNDATION 1/2" AB 1/2" AB 5/8" AB

GENERAL NOTES:

-(2) 2X10 SYP #2 U.N.O.

----6X6 SYP #2 POST

SEE STRUCTURAL PLAN

SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

—SEE FOOTING DETAILS

ANCHOR BOLT

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO. STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

	D OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AID PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	TIONS, FOUNDATION BEARING CAPACITY, GRADE AND ND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLAN	DUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU MITS A CONTINUOUS LOAD PATH CONNECTION, CALL NEER IMMEDIATELY.
DESIGN, PLACEMEN	ANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, NNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL 3.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

	STUD SPACINGS SHALL B LOCATED WITHIN 4 FEET EXAMPLE 16" O.C. x 0.85 =	E MULTIPLIED BY OF CORNERS FOR	0.85 FC	OR FRA	MING		
DESIGN DATA							
WIND LOADS PER FLOR	IDA BUILDING CODE 20	04 RESIDENTIA	L, SE	CTIO	N R30	1.2.1	_
(ENCLOSED SIMPLE DIA MEAN ROOF HEIGHT NO ON UPPER HALF OF HIL SLOPE AND UNOBSTRU	OT EXCEEDING LEAST H L OR ESCARPMENT 60F	IORIZONTAL D T IN EXP. B. 30	IMEN:	SION NEXP	OR 60	0 FT; No	0
BUILDING IS NOT IN THE	HIGH VELOCITY HURR	ICANE ZONE					_
BUILDING IS NOT IN THE	WIND-BORNE DEBRIS	REGION					_
1.) BASIC WIND SPEED) = 110 MPH						_
2.) WIND EXPOSURE =							_
3.) WIND IMPORTANCE							_
4.) BUILDING CATEGOR							_
5.) ROOF ANGLE = 10-4	1000 000						_
6.) MEAN ROOF HEIGH							_
	RE COEFFICIENT = N/A	/ENCLOSED B	LIII DI	NG)			_
	CLADDING DESIGN W				R301.	.2(2))	_
		Zone	Effect	tive W	ind Are	ea (ft2)	_
Z A				0	1	100	
		1	19.9	-21.8	18.1	-18.1	
2 2		2	19.9	-25.5	18.1	-21.8	
1 1	7	2 O'hg		-40.6		-40.6	
2 2 2	2 5	3	19.9	-25.5	18.1	-21.8	
4		3 O'hg		-68.3		-42.4	
\ Y	4/	4		-23.6	18.5	-20.4	
555		5	21.8	-29.1	18.5	-22.6	
		Doors			21.8	-29.1	
1		(Zone	st Case 5, 10				
5	HEK.	8v7 Gar	age D	or	10 E	22.0	

		Zone	Effective Wind Area (ft.			
	A		1	0		100
\gg		1	19.9	-21.8	18.1	-18.1
2	1	2	19.9	-25.5	18.1	-21.8
-	1	2 O'hg		-40.6		-40.€
	2 2 2 1	3	19.9	-25.5	18.1	-21.8
4		3 O'hg		-68.3		-42.4
		4			18.5	-20.4
	5 5	5	21.8	-29.1	18.5	-22.6
THE STATE OF THE S	14	Doors	& Wind	dows	21.8	-29.1
			st Cas			
		(Zone	5, 10	ft2)		
2		8x7 Gar	age Do	oor	19.5	-22.9
4	/2/ 5	16x7 Ga	arage [Door	18.5	-21.0
55	2 22					
LO	ADS					
40 PSI	F (ALL OTHER DWELLING ROOMS)					
30 PSF	(SLEEPING ROOMS)					
30 PSF	(ATTICS WITH STORAGE)					
10 PSI	F (ATTICS WITHOUT STORAGE, <3:12)					
20 PSF	(FLAT OR <4:12)					
16 PS	SF (4:12 TO <12:12)					
	(,					

12 PSF (12:12 AND GREATER)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written ermission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one ouilding, at specified location. MARK DISOSWAY P.E. 53915 ornerstone Developm Zecher Bryan The Griffin Model ADDRESS: Lot #10 Magnolia Hills S/D Lake City, Florida Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE: September 06, 2006 STRUCTURAL BY

> FINALS DATE: Sept. 6, 2006

JOB NUMBER 608232 DRAWING NUMBER

EVAN BEAMSLEY

S-1 OF 2 SHEETS

