Columbia County Building Permit Application ~~455.50~~ 480.50 Revised 9-23-0

For Office Use Only Application # 0511-53 Date Received 11/15/05 By G Permit # 1002/24231
Application Approved by - Zoning Official BLK Date 12.12.05 Plans Examiner OK JTH Date 12-09-05
Flood Zone X Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Dev.
Comments Per PLAT
NOC needed

Applicants Name CHRIS COX Phone 386-752-1711
Address 252 NW IVY GLEN LAKE CITY FL 32055
Owners Name Cornerstone Developers LLC. Phone 752.1711
911 Address 176 SW Fieldstone Ct LAKE CITY FL
Contractors Name Bryan Zecher Construction Phone 752-8653
Address PO Box 815 Lake City FL 32056
Fee Simple Owner Name & Address Cornerstone Developers, LLC.
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address MARK DISOSWAY PO BOX 868 LAKE CITY FL 32056
Mortgage Lenders Name & Address NA

Circle the correct power company – (FL Power & Light) – Clay Elec. – Suwannee Valley Elec. – Progressive Energy
Property ID Number 33-3S-16-02438-166 Estimated Cost of Construction 60,000.00
Subdivision Name Emerald Cove Lot 66 Block ____ Unit -- Phase 1
Driving Directions Take Hwy 90 W - go two miles past I-75 - subdivision is on the left. TL on Heathridge, TR Fieldstone Ct, 4th on right.

Type of Construction FRAME & BRICK Number of Existing Dwellings on Property Ø
Total Acreage 1/2 ACRE Lot Size 1/2 ACRE Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 27' Side 20' Side 11' Rear 120'
Total Building Height 17'-10 1/2" Number of Stories 1 Heated Floor Area 1430 sqft Roof Pitch 6/12
Porches 219 Garage 400 Total 2049

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Chris W. Cox
Owner Builder or Agent (Including Contractor)

Chris Cox
Contractor Signature
Contractors License Number CBC054575
Competency Card Number ____
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

SUSAN ANN FAIR
MY COMMISSION # DD 342845
EXPIRES: August 14, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me
this 15 day of Nov 20 05.
Personally known ✓ or Produced Identification ____

Susan Ann Fair
Notary Signature

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2214

Property Appraiser's 02438-000
Identification Number 02421-000

TM File No: 05-652

WARRANTY DEED

This Warranty Deed, made this 18th day of October, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor, and CORNERSTONE DEVELOPERS, LLC, a Florida Limited Liability Company, whose post office address is P.O. Box 815, Lake City, Florida 32056, grantee.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 65,66,67,68,71,72,73,74,93,94,95,96,97 & 98, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

(Signature of First Witness)

DeEtte F. Brown

(Typed Name of First Witness)

(Signature of Second Witness)

Karen M. Wright

(Typed Name of Second Witness)

D D P CORPORATION

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 18th day of October, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is/are personally known to me or who has/have produced _____ as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

KAREN M. WRIGHT
Notary Public - State of Florida
My Commission Expires Jan 23, 2006
Commission # DD066234
Bonded By National Notary Assn.

Inst:2005026450 Date:10/24/2005 Time:13:06
Doc Stamp-Deed : 3628.80
DC,P.DeWitt Cason,Columbia County B:1062 P:2215

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
904 NW Main Blvd.
LAKE CITY, FLORIDA 32055

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1134N

---------- PART II - SITEPLAN ----------

Scale: 1 inch = 50 feet.

See Attached

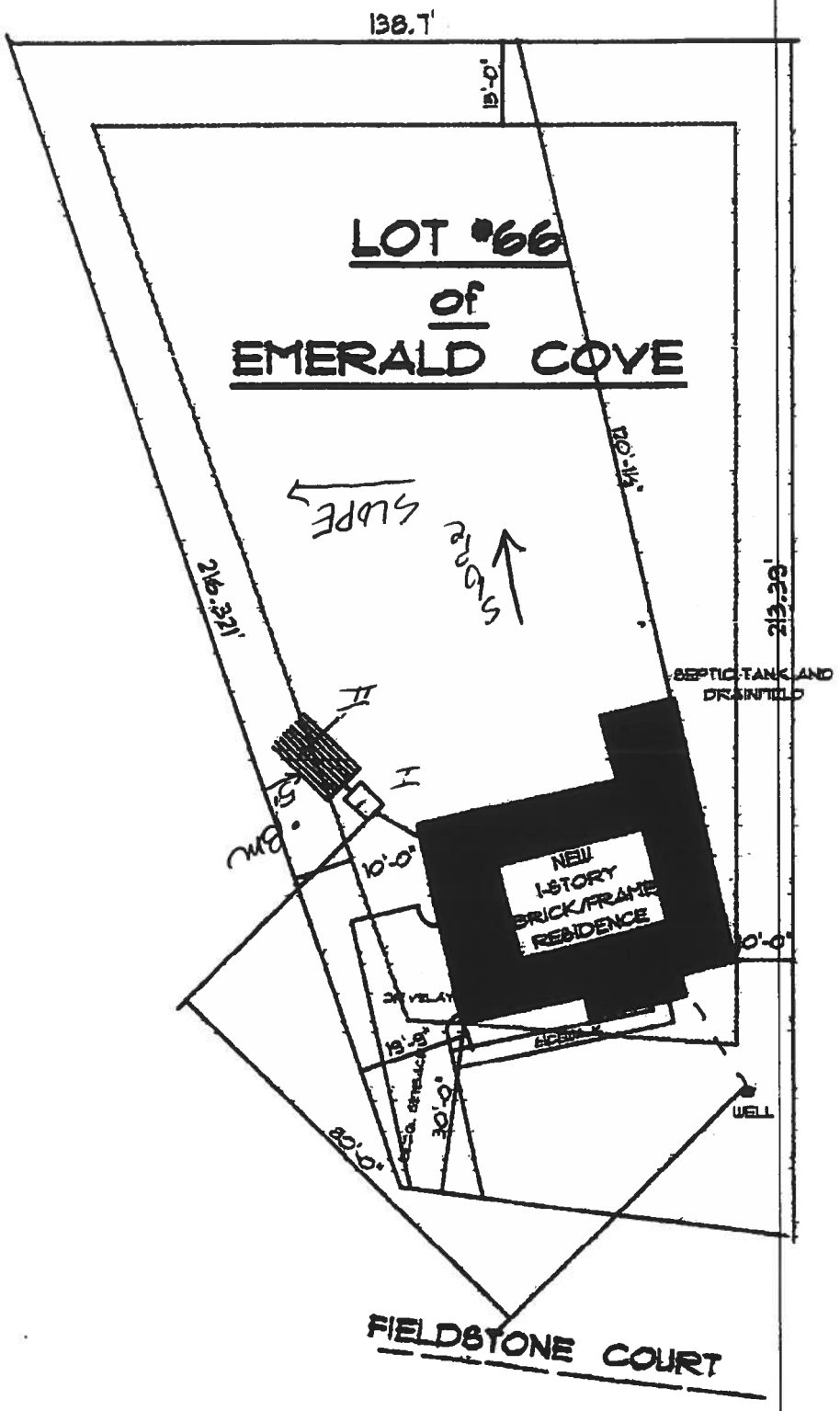
Notes: _____

Site Plan submitted by: _____ MASTER CONTRACTOR

Plan Approved X Not Approved ____ Date OCT 24 2005

By _____ COLUMBIA County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



• SITE PLAN •

SCALE : 1" = 20'-0'

Rock D 7-0

FORM 600A-2004 EnergyGauge® 4.0

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**TheVictoriaModel510319**	Builder:	**Bryan Zecher**
Address:	**Lot: 66, Sub: Emerald Cove, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	24231
Owner:	**Model Home**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 28.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 10.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	1430 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 61.0 ft²	13. Heating systems	
b. SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 61.0 ft²	a. Electric Heat Pump	Cap: 28.0 kBtu/hr
			HSPF: 7.00
8. Floor types		b. N/A	
a. Slab-On-Grade Edge Insulation	R=0.0, 206.0(p) ft		
b. N/A		c. N/A	
c. N/A			
9. Wall types		14. Hot water systems	
a. Frame, Wood, Adjacent	R=13.0, 348.0 ft²	a. Electric Resistance	Cap: 40.0 gallons
b. Frame, Wood, Exterior	R=13.0, 1109.0 ft²		EF: 0.92
c. N/A		b. N/A	
d. N/A			
e. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a. Under Attic	R=30.0, 1460.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 140.0 ft		
b. N/A			

Glass/Floor Area: 0.08 Total as-built points: 25230 Total base points: 26137 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature]

DATE: 11-7-05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [signature]

DATE: 11-15-05

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	**1430.0**	**20.04**	**5158.3**	Double, Clear	E	1.5	1.5	3.0	42.06	0.52	65.3
				Double, Clear	E	1.5	5.5	45.0	42.06	0.90	1696.4
				Double, Clear	E	1.5	7.5	20.0	42.06	0.95	798.0
				Double, Clear	S	1.5	1.5	3.0	35.87	0.52	56.0
				Double, Clear	S	1.5	5.5	15.0	35.87	0.83	447.7
				Double, Clear	W	1.5	5.5	30.0	38.52	0.90	1036.6
				As-Built Total:				**116.0**			**4100.1**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	348.0	0.70	243.6	Frame, Wood, Adjacent			13.0	348.0	0.60		208.8
Exterior	1109.0	1.70	1885.3	Frame, Wood, Exterior			13.0	1109.0	1.50		1663.5
Base Total:	**1457.0**		**2128.9**	**As-Built Total:**				**1457.0**			**1872.3**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	20.0	2.40	48.0	Exterior Insulated				20.0	4.10		82.0
Exterior	40.0	6.10	244.0	Exterior Insulated				20.0	4.10		82.0
				Adjacent Insulated				20.0	1.60		32.0
Base Total:	**60.0**		**292.0**	**As-Built Total:**				**60.0**			**196.0**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1430.0	1.73	2473.9	Under Attic			30.0	1460.0	1.73 X 1.00		2525.8
Base Total:	**1430.0**		**2473.9**	**As-Built Total:**				**1460.0**			**2525.8**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	206.0(p)	-37.0	-7622.0	Slab-On-Grade Edge Insulation			0.0	206.0(p	-41.20		-8487.2
Raised	0.0	0.00	0.0								
Base Total:			**-7622.0**	**As-Built Total:**				**206.0**			**-8487.2**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1430.0	10.21	14600.3					1430.0	10.21		14600.3

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT						
Summer Base Points: 17031.4			**Summer As-Built Points:**						**14807.3**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points	
			(sys 1: Central Unit 28000 btuh ,SEER/EFF(10.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)						
			14807	1.00	(1.09 x 1.147 x 1.00)	0.341	1.000	6318.3	
17031.4	**0.4266**	**7265.6**	**14807.3**	**1.00**	**1.250**	**0.341**	**1.000**	**6318.3**	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1430.0	12.74	3279.3	Double, Clear	E	1.5	1.5	3.0	18.79	1.29	72.5
				Double, Clear	E	1.5	5.5	45.0	18.79	1.04	880.6
				Double, Clear	E	1.5	7.5	20.0	18.79	1.02	384.5
				Double, Clear	S	1.5	1.5	3.0	13.30	2.73	109.0
				Double, Clear	S	1.5	5.5	15.0	13.30	1.15	228.8
				Double, Clear	W	1.5	5.5	30.0	20.73	1.03	639.3
				As-Built Total:				**116.0**			**2314.7**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	348.0	3.60	1252.8	Frame, Wood, Adjacent			13.0	348.0	3.30		1148.4
Exterior	1109.0	3.70	4103.3	Frame, Wood, Exterior			13.0	1109.0	3.40		3770.6
Base Total:	**1457.0**		**5356.1**	**As-Built Total:**				**1457.0**			**4919.0**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	20.0	11.50	230.0	Exterior Insulated				20.0	8.40		168.0
Exterior	40.0	12.30	492.0	Exterior Insulated				20.0	8.40		168.0
				Adjacent Insulated				20.0	8.00		160.0
Base Total:	**60.0**		**722.0**	**As-Built Total:**				**60.0**			**496.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X WCM =		Points
Under Attic	1430.0	2.05	2931.5	Under Attic			30.0	1460.0	2.05 X 1.00		2993.0
Base Total:	**1430.0**		**2931.5**	**As-Built Total:**				**1460.0**			**2993.0**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	206.0(p)	8.9	1833.4	Slab-On-Grade Edge Insulation			0.0	206.0(p	18.80		3872.8
Raised	0.0	0.00	0.0								
Base Total:			**1833.4**	**As-Built Total:**				**206.0**			**3872.8**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1430.0	-0.59	-843.7					1430.0	-0.59		-843.7

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**13278.6**	**Winter As-Built Points:**						**13751.8**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
			(sys 1: Electric Heat Pump 28000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0						
			13751.8	1.000	(1.069 x 1.169 x 1.00)	0.487	1.000	8371.6	
13278.6	**0.6274**	**8331.0**	**13751.8**	**1.00**	**1.250**	**0.487**	**1.000**	**8371.6**	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
4		2635.00	10540.0	40.0	0.92	4		1.00	2635.00	1.00	10540.0
				As-Built Total:							**10540.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points +	Heating Points +	Hot Water Points =	Total Points	Cooling Points +	Heating Points +	Hot Water Points =	Total Points
7266	**8331**	**10540**	**26137**	**6318**	**8372**	**10540**	**25230**

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.2

The higher the score, the more efficient the home.

Model Home, Lot: 66, Sub: Emerald Cove, Plat: , Lake City, FL,

1.	New construction or existing	New	
2.	Single family or multi-family	Single family	
3.	Number of units, if multi-family	1	
4.	Number of Bedrooms	4	
5.	Is this a worst case?	Yes	
6.	Conditioned floor area (ft²)	1430 ft²	
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		
a.	U-factor: (or Single or Double DEFAULT) 7a.	Description Area (Dble Default) 61.0 ft²	
b.	SHGC: (or Clear or Tint DEFAULT) 7b.	(Clear) 61.0 ft²	
8.	Floor types		
a.	Slab-On-Grade Edge Insulation	R=0.0, 206.0(p) ft	
b.	N/A		
c.	N/A		
9.	Wall types		
a.	Frame, Wood, Adjacent	R=13.0, 348.0 ft²	
b.	Frame, Wood, Exterior	R=13.0, 1109.0 ft²	
c.	N/A		
d.	N/A		
e.	N/A		
10.	Ceiling types		
a.	Under Attic	R=30.0, 1460.0 ft²	
b.	N/A		
c.	N/A		
11.	Ducts		
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 140.0 ft	
b.	N/A		

12.	Cooling systems		
a.	Central Unit	Cap: 28.0 kBtu/hr SEER: 10.00	
b.	N/A		
c.	N/A		
13.	Heating systems		
a.	Electric Heat Pump	Cap: 28.0 kBtu/hr HSPF: 7.00	
b.	N/A		
c.	N/A		
14.	Hot water systems		
a.	Electric Resistance	Cap: 40.0 gallons EF: 0.92	
b.	N/A		
c.	Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15.	HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: 11-15-05

Address of New Home: 176 SW FIELDSTONE CT. City/FL Zip: LAKE CITY, FL

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

BUILDING INPUT SUMMARY REPORT

PROJECT

Title:	TheVictoriaModel510319	**Family Type:**	Single	**Address Type:**	Lot Information
Owner:	Model Home	**New/Existing:**	New	**Lot #:**	66
# of Units:	1	**Bedrooms:**	4	**Subdivision:**	Emerald Cove
Builder Name:	Bryan Zecher	**Conditioned Area:**	1430	**Platbook:**	(blank)
Climate:	North	**Total Stories:**	1	**Street:**	N/A
Permit Office:	(blank)	**Worst Case:**	Yes	**County:**	(blank)
Jurisdiction #:	(blank)	**Rotate Angle:**	(blank)	**City, St, Zip:**	Lake City, FL,

FLOORS

#	Floor Type	R-Val	Area/Perimeter	Units
1	Slab-On-Grade Edge Insulation	0.0	206.0(p) ft	1

DOORS

#	Door Type	Orientation	Area	Units
1	Insulated	Exterior	10.0 ft²	2
2	Insulated	Exterior	20.0 ft²	1
3	Insulated	Adjacent	20.0 ft²	1

CEILINGS

#	Ceiling Type	R-Val	Area	Base Area	Units
1	Under Attic	30.0	1460.0 ft²	1430.0 ft²	1

Credit Multipliers: None

COOLING

#	System Type	Efficiency	Capacity
1	Central Unit	SEER: 10.00	28.0 kBtu/hr

Credit Multipliers: None

WALLS

#	Wall Type	Location	R-Val	Area	Units
1	Frame - Wood	Adjacent	13.0	348.0 ft²	1
2	Frame - Wood	Exterior	13.0	1109.0 ft²	1

HEATING

#	System Type	Efficiency	Capacity
1	Electric Heat Pump	COP: 7.00	28.0 kBtu/hr

Credit Multipliers: None

WINDOWS

#	Panes	Tint	Ornt	Area	OH Length	OH Hght	Units
1	Double	Clear	N	3.0 ft²	1.5 ft	1.5 ft	1
2	Double	Clear	N	15.0 ft²	1.5 ft	5.5 ft	3
3	Double	Clear	N	10.0 ft²	1.5 ft	7.5 ft	2
4	Double	Clear	E	3.0 ft²	1.5 ft	1.5 ft	1
5	Double	Clear	E	15.0 ft²	1.5 ft	5.5 ft	1
6	Double	Clear	S	15.0 ft²	1.5 ft	5.5 ft	2

DUCTS

#	Supply Location	Return Location	Air Handler Location	Supply R-Val	Supply Length
1	Uncond.	Uncond.	Garage	6.0	140.0 ft

Credit Multipliers: None

WATER

#	System Type	EF	Cap.	Conservation Type	Con. EF
1	Electric Resistance	0.92	40.0	None	0.00

REFR.

#	Use Default?	Annual Operating Cost	Electric Rate
1	Yes	N/A	N/A

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001002

DATE 03/14/2006 PARCEL ID # 33-3S-16-02438-166

APPLICANT SUSAN FAIR PHONE 752-1711

ADDRESS 224 SW VERNON WAY LAKE CITY FL 32024

OWNER CORNERSTONE DEVELOPERS PHONE 752-1711

ADDRESS 176 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 752-8653

LOCATION OF PROPERTY 90W, TL ON HEATHRIDGE, TR ON FIELDSTONE COURT, 4TH ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 66 1

SIGNATURE Susan Fair

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY, FLORIDA

CORNERSTONE - LOT 66

NOV 29 2005

Project Information for:	L138117		
Builder:	BRYAN ZECHER	Date:	11/29/2005
Lot:	LOT 66 EMERALD COVE	Start Number:	1702
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	30		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	120		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
ALACHUA,FL 32615
Designer: 137

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1129051702	11/29/2005				
2	CJ3	1129051703	11/29/2005				
3	CJ5	1129051704	11/29/2005				
4	EJ3	1129051705	11/29/2005				
5	EJ5	1129051706	11/29/2005				
6	EJ7	1129051707	11/29/2005				
7	EJ7A	1129051708	11/29/2005				
8	HJ3	1129051709	11/29/2005				
9	HJ5	1129051710	11/29/2005				
10	HJ7	1129051711	11/29/2005				
11	T01	1129051712	11/29/2005				
12	T02T	1129051713	11/29/2005				
13	T03T	1129051714	11/29/2005				
14	T04T	1129051715	11/29/2005				
15	T05T	1129051716	11/29/2005				
16	T06T	1129051717	11/29/2005				
17	T07P	1129051718	11/29/2005				
18	T08P	1129051719	11/29/2005				
19	T09P	1129051720	11/29/2005				
20	T10	1129051721	11/29/2005				
21	T11	1129051722	11/29/2005				
22	T12	1129051723	11/29/2005				
23	T13	1129051724	11/29/2005				
24	T14	1129051725	11/29/2005				
25	T15P	1129051726	11/29/2005				
26	T16P	1129051727	11/29/2005				
27	T17P	1129051728	11/29/2005				
28	T18H	1129051729	11/29/2005				
29	T19P	1129051730	11/29/2005				
30	T20P	1129051731	11/29/2005				

Licensee Details

Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN (Primary Name)**
	BRYAN ZECHER CONSTRUCTION INC (DBA
Main Address:	**P O BOX 815** **LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST** **LAKE CITY, FL 32055 United States Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

View Related License Information

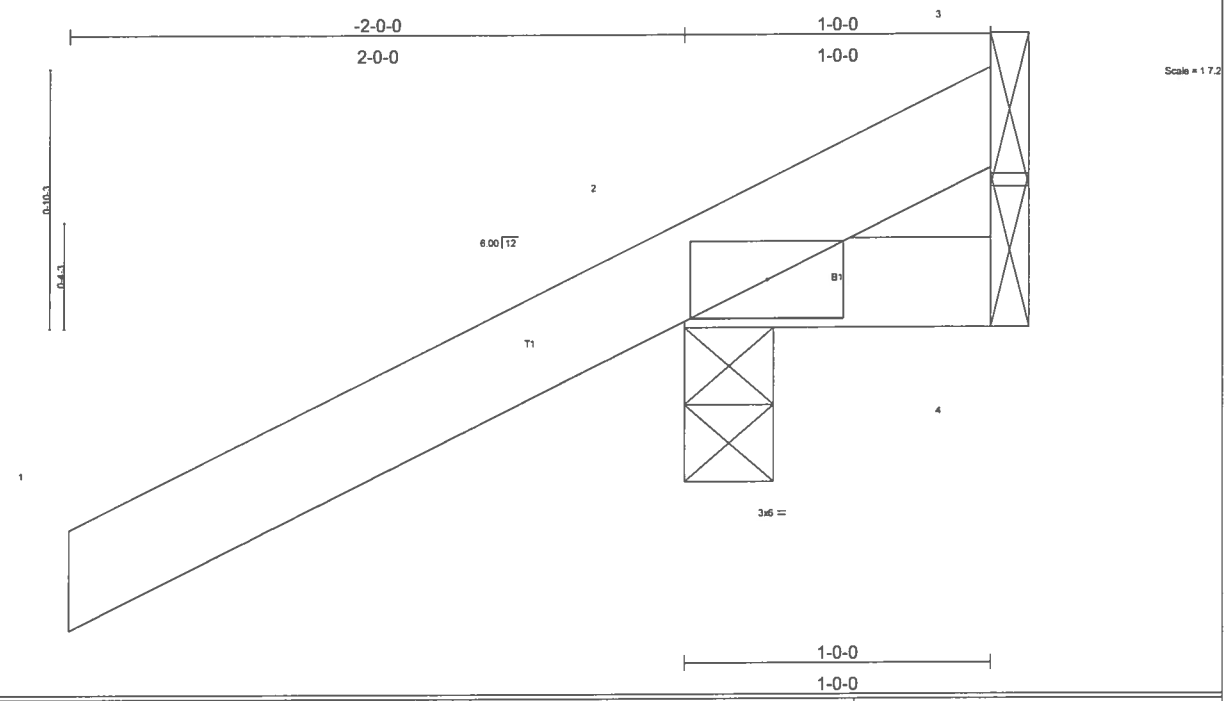
View License Complaint

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051702

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	CJ1	MONO TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:54 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

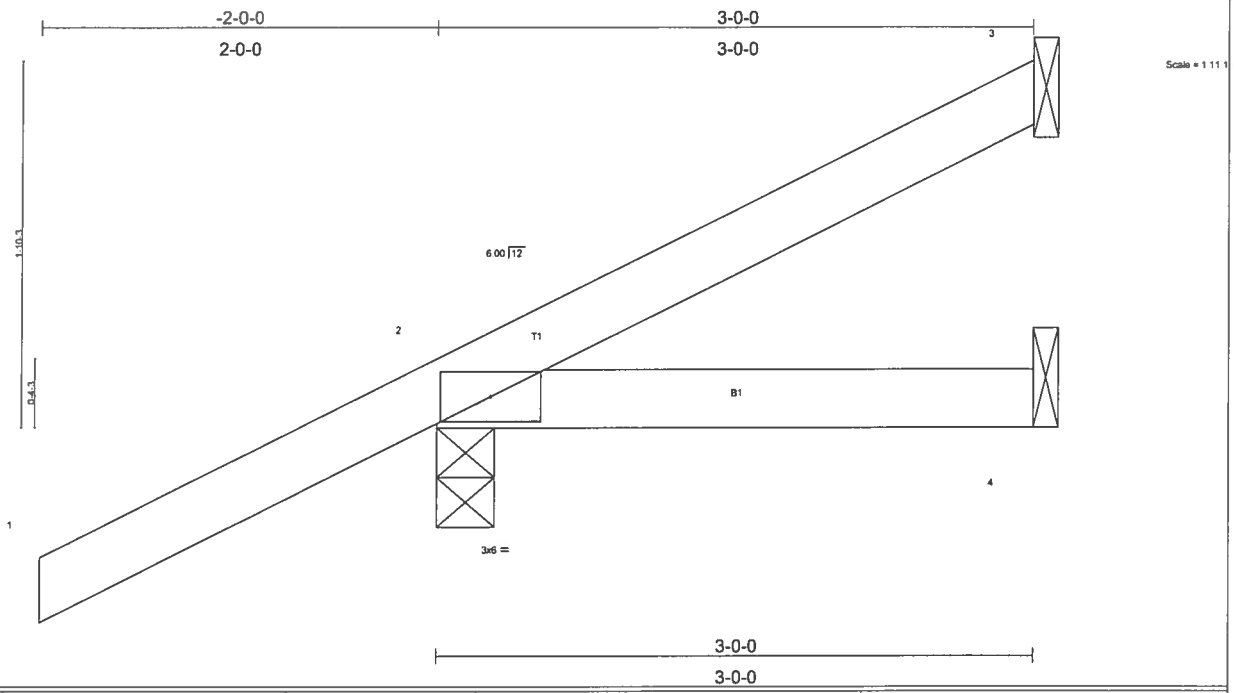
LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051703

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:54 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

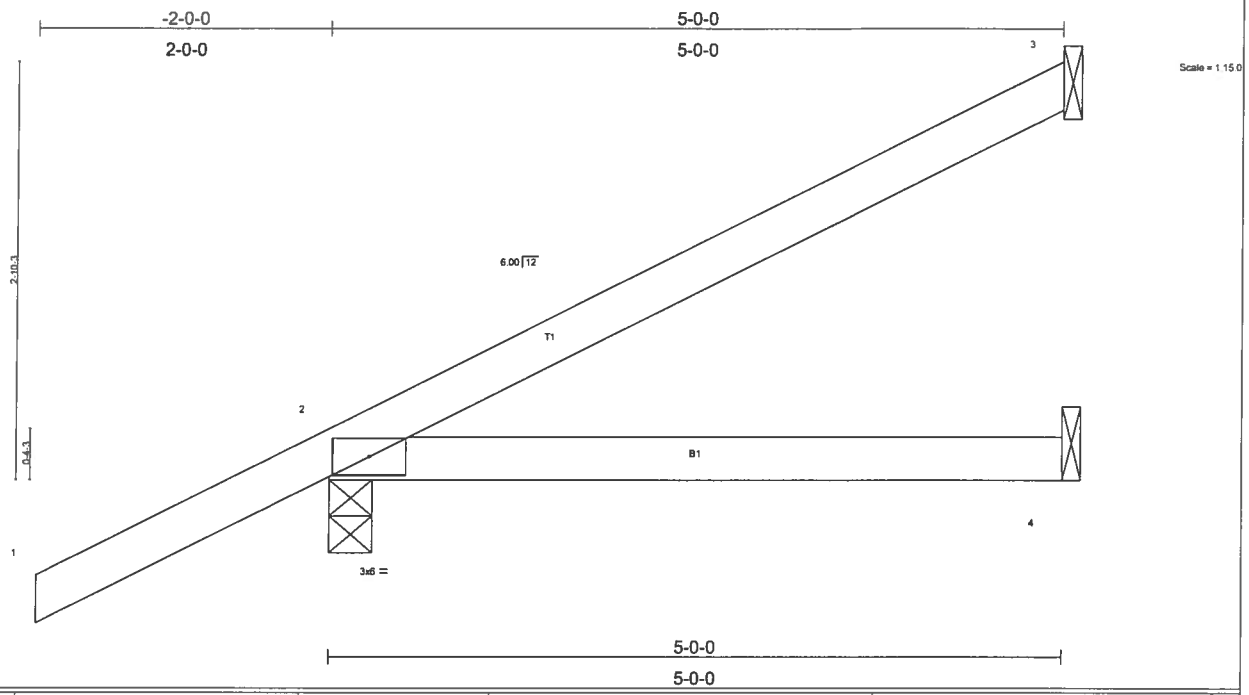
LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051704

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	CJ5	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:55 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051705

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	EJ3	MONO TRUSS	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:55 2005 Page 1

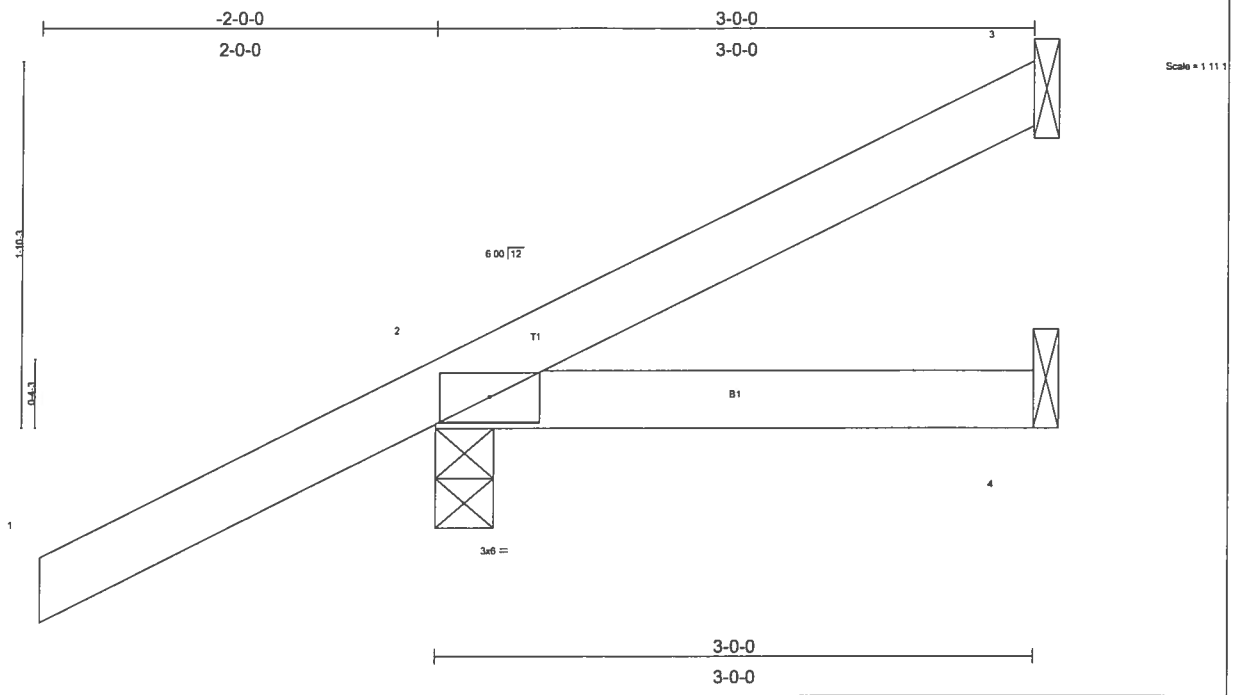


Plate Offsets (X,Y): [2:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

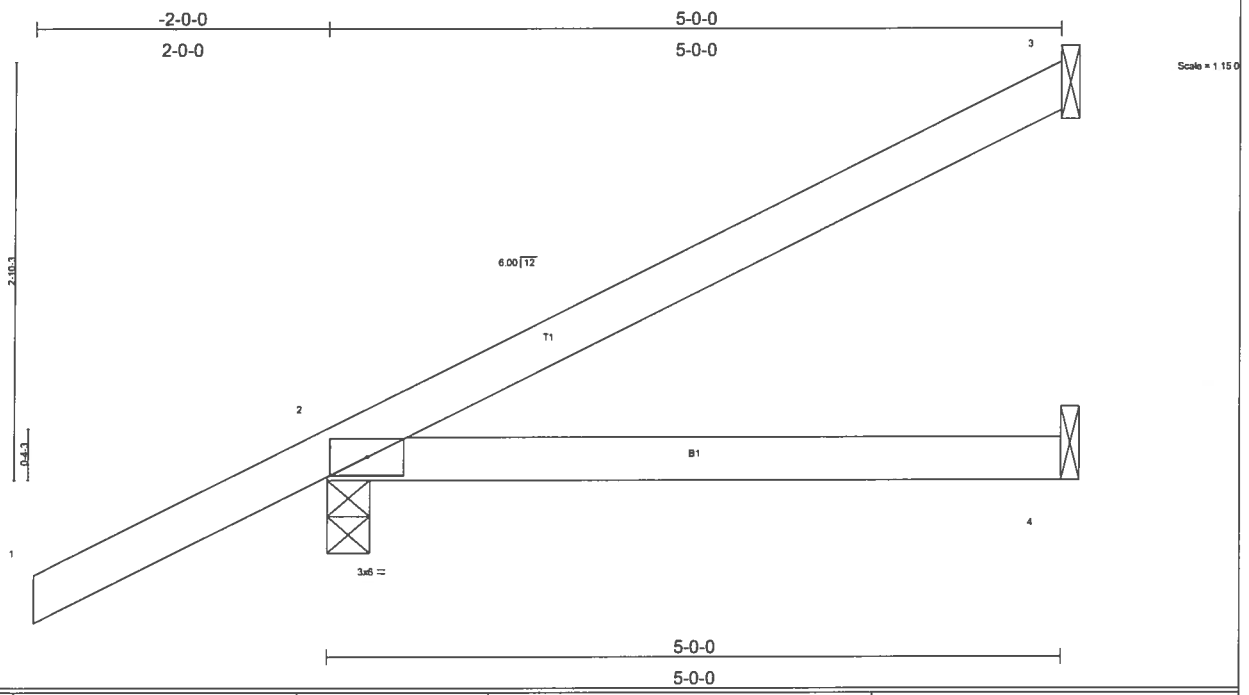
LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051706

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	EJ5	MONO TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:56 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051707

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	EJ7	MONO TRUSS	24	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:56 2005 Page 1

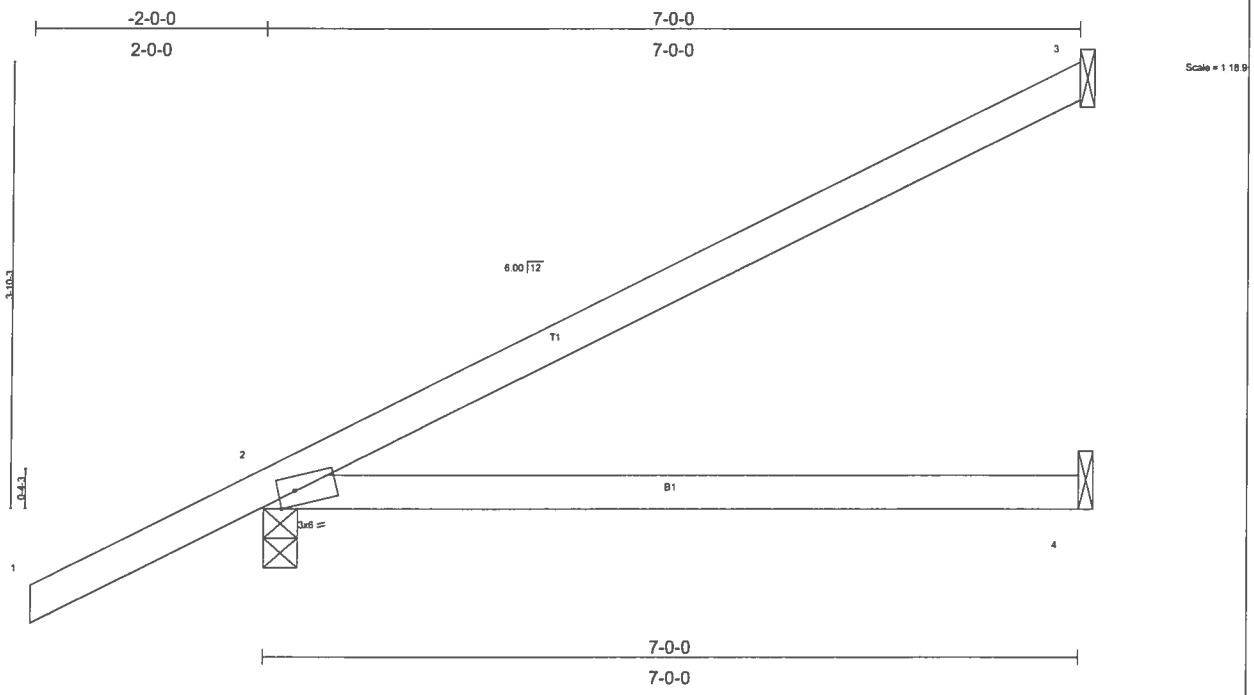


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

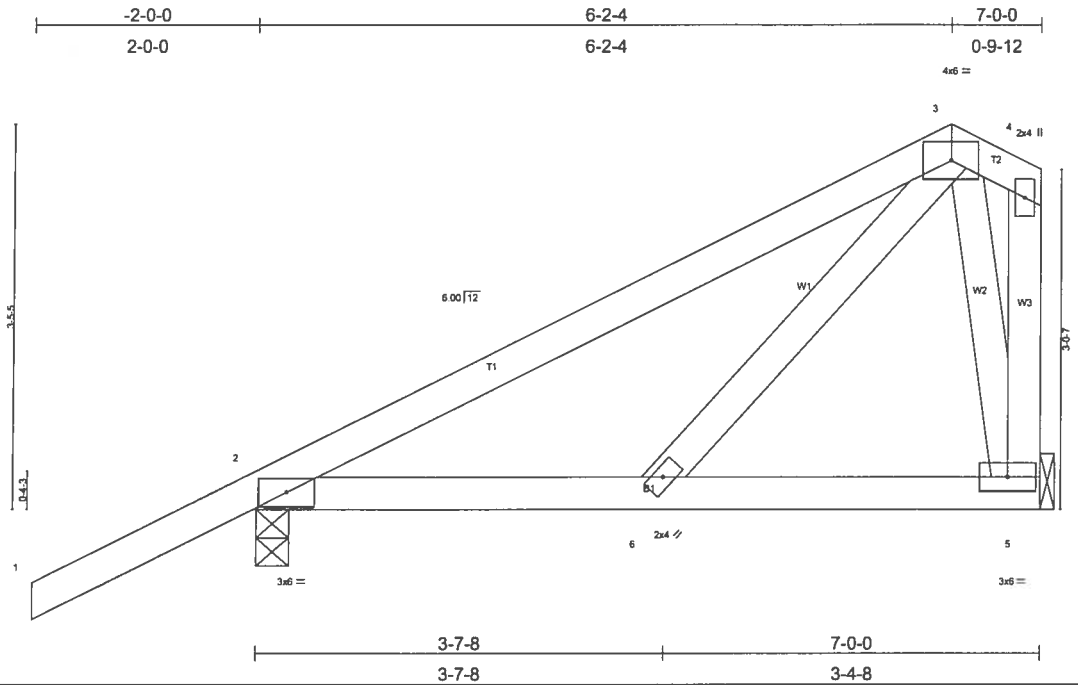
LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051708

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	EJ7A	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:52:57 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	-0.01	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 40 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=416/0-3-8, 5=263/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-221(load case 5), 5=-103(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-301/19, 3-4=-110/83, 4-5=-251/186
BOT CHORD 2-6=-84/201, 5-6=-96/95
WEBS 3-6=0/162, 3-5=-399/448

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2 and 103 lb uplift at joint 5.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051709

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	HJ3	MONO TRUSS	2	1	Job Reference (optional)

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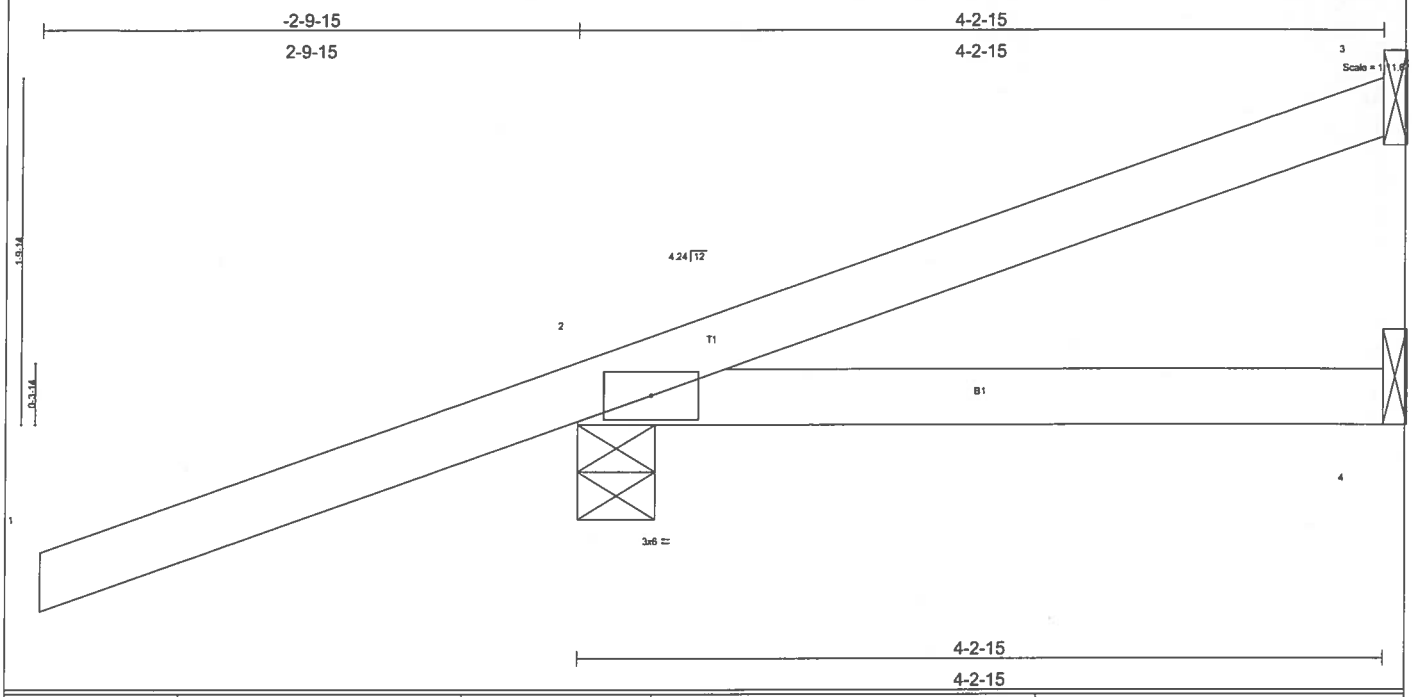

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=15/Mechanical, 2=289/0-4-15, 4=42/Mechanical
Max Horz 2=98(load case 2)
Max Uplift3=-6(load case 4), 2=-302(load case 2), 4=-41(load case 2)
Max Grav 3=32(load case 5), 2=289(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-37/10
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 6 lb uplift at joint 3, 302 lb uplift at joint 2 and 41 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051710

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	HJ5	MONO TRUSS	2	1	Job Reference (optional)

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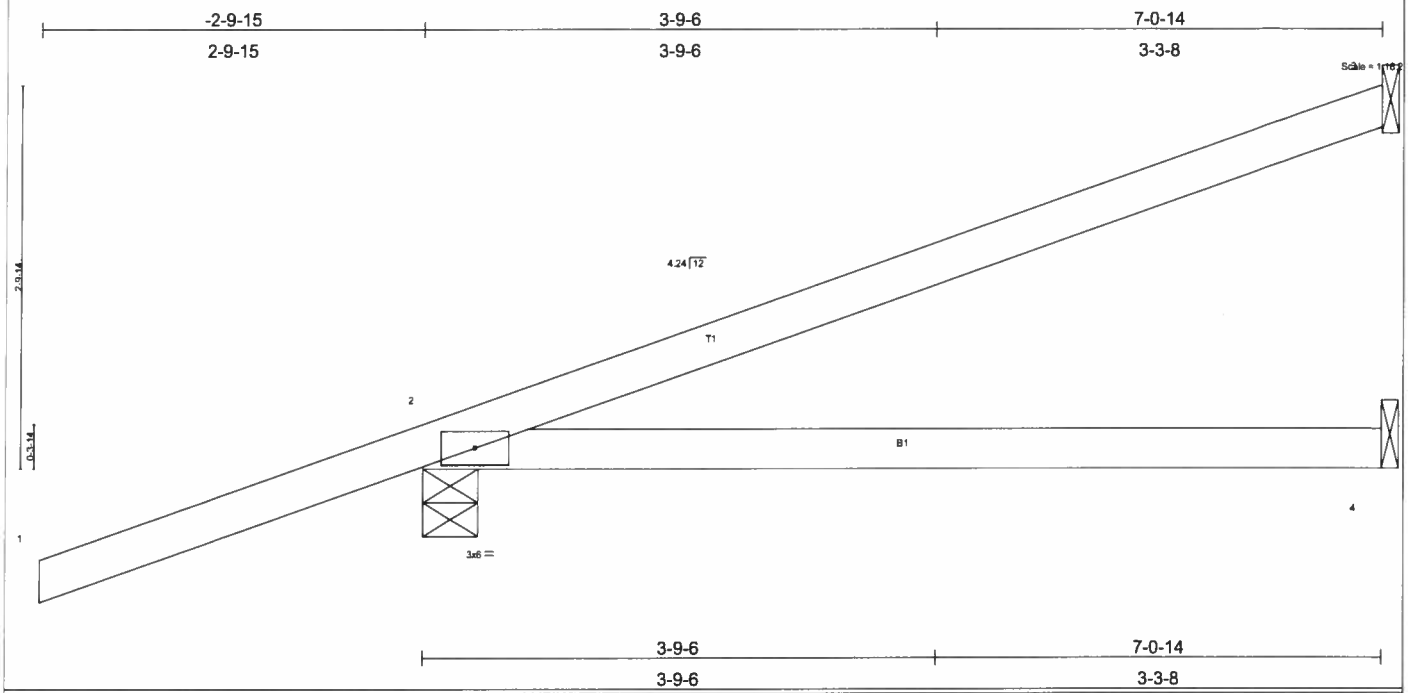

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.08	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.13	2-4	>609	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=192/Mechanical, 2=375/0-4-15, 4=111/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-144(load case 2), 2=-248(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-68/45
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 248 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-4=-53(F=-12, B=-12)

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051711

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	HJ7	MONO TRUSS	3	1	Job Reference (optional)

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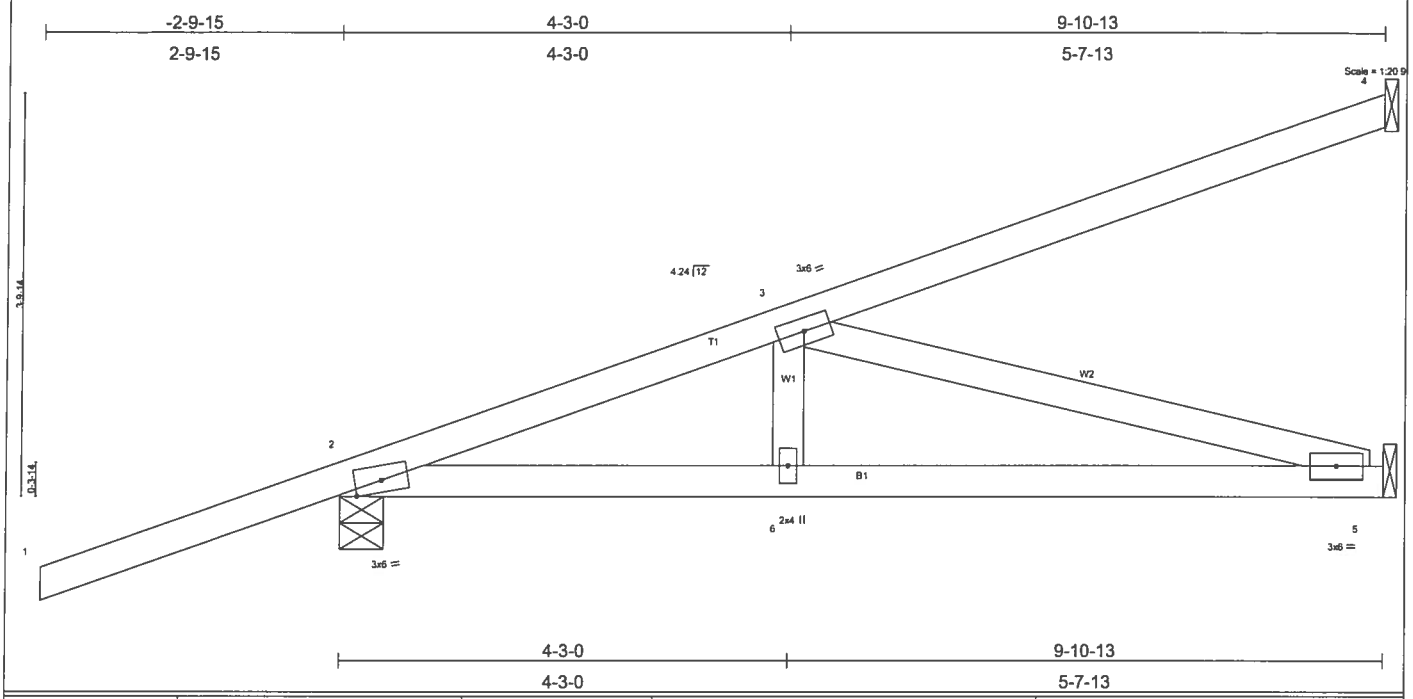

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.08	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.14	5-6	>818	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.51	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=531/0-4-15, 5=377/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-278(load case 2), 5=-64(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-877/116, 3-4=-105/65
BOT CHORD 2-6=-304/814, 5-6=-304/814
WEBS 3-6=0/163, 3-5=-844/316

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 278 lb uplift at joint 2 and 64 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051712

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T01	HIP	1	2	Job Reference (optional)

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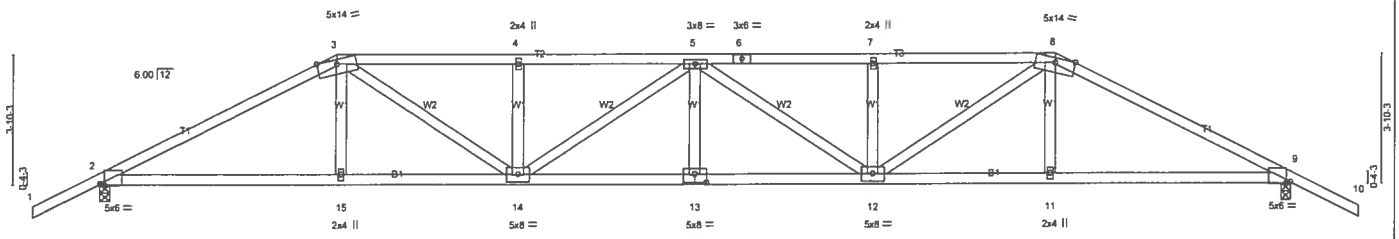
-2-0-0	3-9-4	7-0-0	12-4-10	17-7-8	22-10-6	28-3-0	31-5-12	35-3-0	37-3-0
2-0-0	3-9-4	3-2-12	5-4-10	5-2-14	5-2-14	5-4-10	3-2-12	3-9-4	2-0-0

Scale = 1:65.1



3-9-4	7-0-0	12-4-10	17-7-8	22-10-6	28-3-0	31-5-12	35-3-0
3-9-4	3-2-12	5-4-10	5-2-14	5-2-14	5-4-10	3-2-12	3-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [9:0-1-11,Edge], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.40	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.63	13-14	>662	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(TL)	0.18	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 348 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-6 oc bracing.

REACTIONS (lb/size) 2=3169/0-3-8, 9=3169/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-1336(load case 4), 9=-1336(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-6163/2622, 3-4=-7615/3369, 4-5=-7614/3369, 5-6=-7614/3369, 6-7=-7614/3369, 7-8=-7615/3369, 8-9=-6163/2622, 9-10=0/47
BOT CHORD 2-15=-2293/5407, 14-15=-2302/5441, 13-14=-3578/8315, 12-13=-3578/8315, 11-12=-2261/5441, 9-11=-2251/5407
WEBS 3-15=-225/831, 3-14=-1261/2695, 4-14=-644/539, 5-14=-877/390, 5-13=0/340, 5-12=-877/390, 7-12=-644/539, 8-12=-1261/2695, 8-11=-224/831

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1336 lb uplift at joint 2 and 1336 lb uplift at joint 9.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 28-3-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-118(F=-64), 8-10=-54, 2-15=-30, 11-15=-65(F=-35), 9-11=-30
Concentrated Loads (lb)
Vert: 15=-539(F) 11=-539(F)

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051713

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T02T	SPECIAL	1	1	Job Reference (optional)

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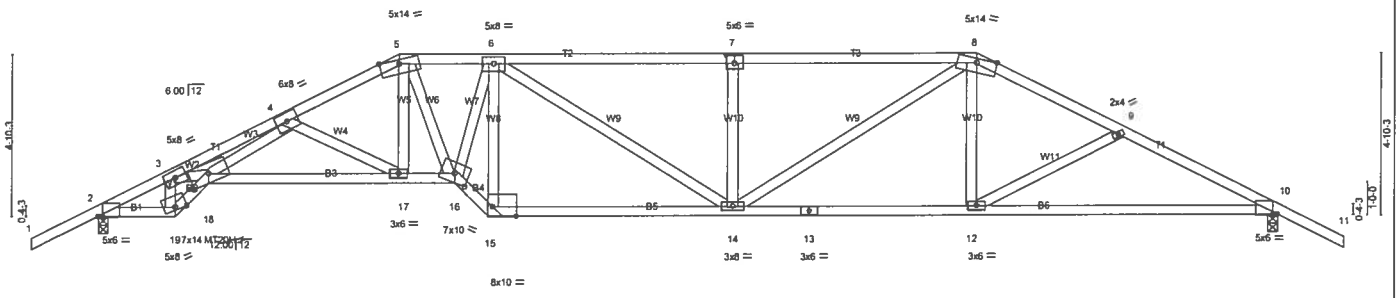
-2-0-0 | 2-3-8 | 5-7-12 | 9-0-0 | 11-8-0 | 18-11-8 | 26-3-0 | 30-5-12 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 3-4-4 | 3-4-4 | 2-8-0 | 7-3-8 | 7-3-8 | 4-2-12 | 4-9-4 | 2-0-0

Scale = 1:65.8



11-8-0

2-3-8 | 3-3-8 | 9-0-0 | 10-8-0 | 18-11-8 | 26-3-0 | 35-3-0

2-3-8 | 1-0-0 | 5-8-8 | 1-8-0 | 1-0-0 | 7-3-8 | 7-3-8 | 9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-3-0,0-3-0], [10:0-1-11,Edge], [15:0-8-9,Edge], [16:0-5-0,0-3-4], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.39	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.63	14-15	>664	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.31	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-537(load case 5), 10=-537(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2702/964, 3-4=-6141/2075, 4-5=-3102/1209, 5-6=-3004/1243, 6-7=-2845/1227, 7-8=-2849/1230, 8-9=-2479/1004, 9-10=-2691/1093, 10-11=0/47
BOT CHORD 2-19=-697/2282, 18-19=-820/2750, 17-18=-1198/3596, 16-17=-813/2754, 15-16=-1071/3290, 14-15=-797/2549, 13-14=-647/2190, 12-13=-647/2190, 10-12=-807/2349
WEBS 3-19=-1774/594, 3-18=-1072/3460, 4-18=-628/2439, 4-17=-961/436, 5-17=-202/742, 5-16=-310/629, 6-16=-480/1580, 6-15=-1989/749, 6-14=-179/430, 7-14=-417/292, 8-14=-361/865, 8-12=-29/350, 9-12=-195/184

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 2 and 537 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051714

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T03T	SPECIAL	1	1	Job Reference (optional)

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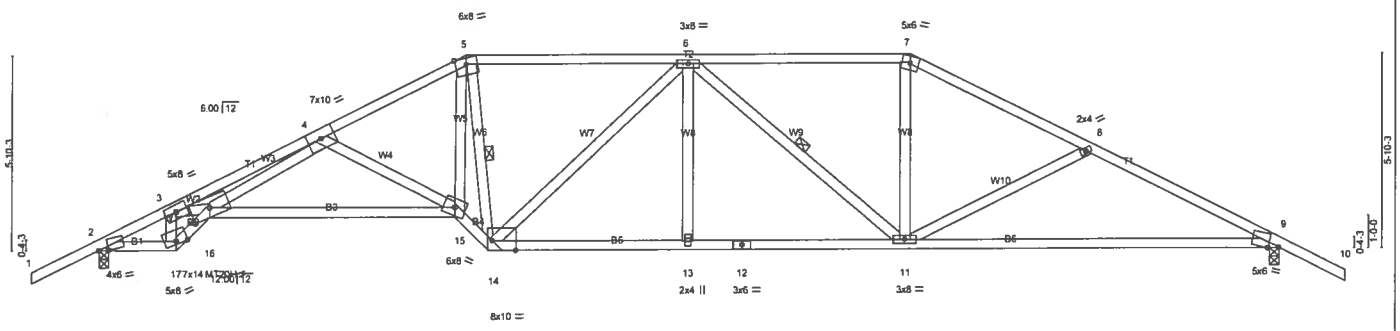
-2-0-0	2-3-8	6-7-12	11-0-0	17-7-8	24-3-0	29-5-12	35-3-0	37-3-0
2-0-0	2-3-8	4-4-4	4-4-4	6-7-8	6-7-8	5-2-12	5-9-4	2-0-0

Scale = 1:65.8



2-3-8	3-3-8	10-8-0	11-8-0	17-7-8	24-3-0	35-3-0
2-3-8	1-0-0	7-4-8	1-0-0	5-11-8	6-7-8	11-0-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [9:0-3-13,Edge], [14:0-8-9,Edge], [16:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.52	15-16	>802	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.85	15-16	>495	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.35	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 6-11

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-555(load case 5), 9=-555(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2681/994, 3-4=-6248/2108, 4-5=-2752/1120, 5-6=-2028/943, 6-7=-2044/935, 7-8=-2331/970, 8-9=-2641/1112, 9-10=0/47
BOT CHORD 2-17=-729/2263, 16-17=-861/2786, 15-16=-1076/3184, 14-15=-905/3213, 13-14=-708/2332, 12-13=-708/2330, 11-12=-708/2330, 9-11=-817/2311
WEBS 3-17=-1870/628, 3-16=-1057/3578, 4-16=-765/2938, 4-15=-877/437, 5-15=-799/2769, 5-14=-1871/529, 6-14=-522/224, 6-13=0/165, 6-11=-491/236, 7-11=-179/698, 8-11=-325/278

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 555 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051715

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T04T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:01 2005 Page 1

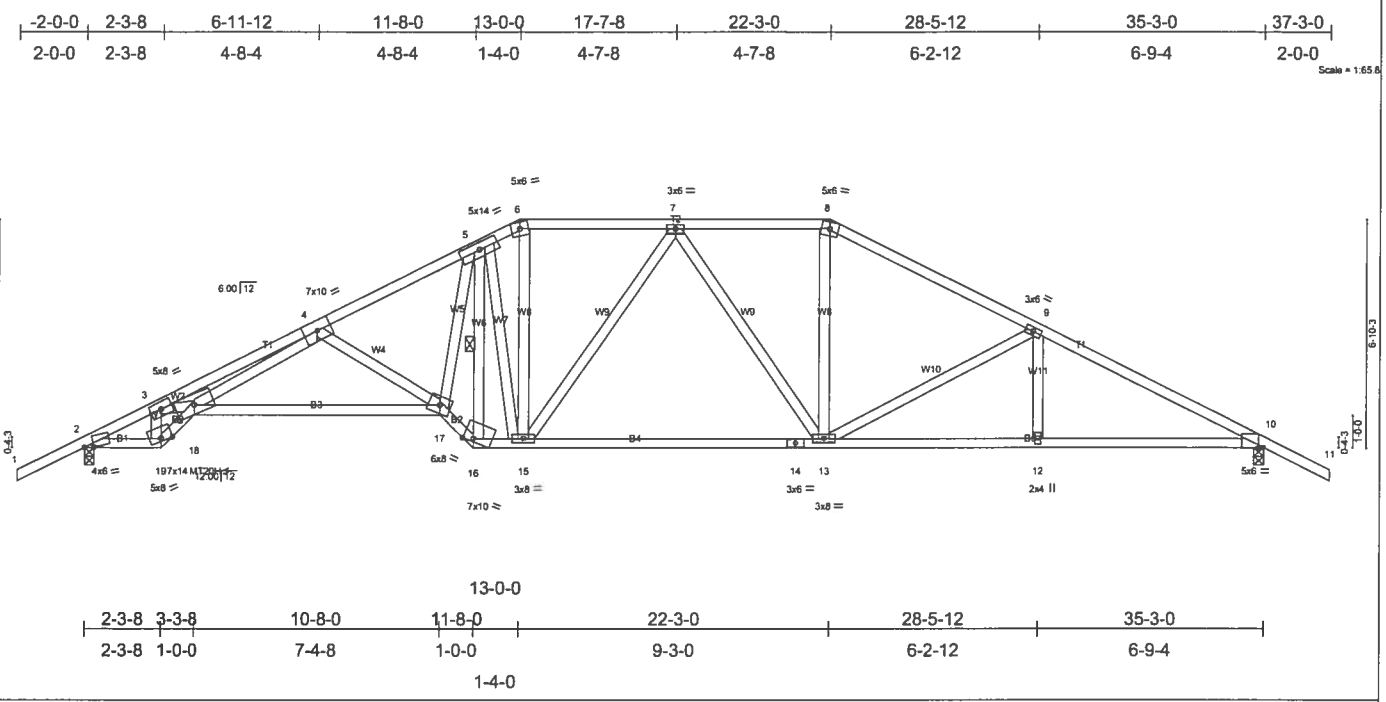


Plate Offsets (X,Y): [2:0-3-5,0-0-11], [10:0-1-11,Edge], [16:0-3-11,Edge], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.51	17-18	>817	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.83	17-18	>505	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.34	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-16

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-571(load case 5), 10=-571(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2684/1013, 3-4=-6181/2132, 4-5=-2813/1162, 5-6=-2038/978, 6-7=-1850/899, 7-8=-1857/911, 8-9=-2141/942, 9-10=-2718/1089, 10-11=0/47
BOT CHORD 2-19=-748/2264, 18-19=-879/2767, 17-18=-1077/3136, 16-17=-722/2669, 15-16=-564/1945, 14-15=-546/1928, 13-14=-546/1928, 12-13=-789/2347, 10-12=-789/2347
WEBS 3-19=-1832/637, 3-18=-1057/3510, 4-18=-775/2886, 4-17=-819/444, 5-17=-706/2501, 5-16=-1762/382, 5-15=-470/348, 6-15=-306/691, 7-15=-252/162, 7-13=-264/163, 8-13=-181/620, 9-13=-577/333, 9-12=0/192

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 571 lb uplift at joint 2 and 571 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051716

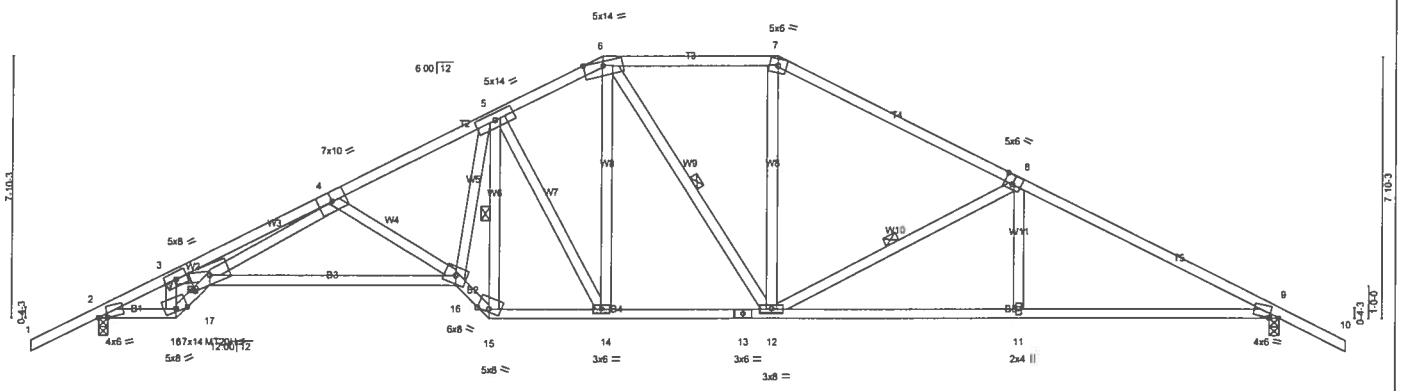
Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T05T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:02 2005 Page 1

-2-0-0 | 2-3-8 | 7-1-12 | 11-8-0 | 15-0-0 | 20-3-0 | 27-5-12 | 35-3-0 | 37-3-0

2-0-0 | 2-3-8 | 4-10-4 | 4-6-4 | 3-4-0 | 5-3-0 | 7-2-12 | 7-9-4 | 2-0-0

Scale = 1:65.8



2-3-8 | 3-3-8 | 10-8-0 | 11-8-0 | 15-0-0 | 20-3-0 | 27-5-12 | 35-3-0

2-3-8 | 1-0-0 | 7-4-8 | 1-0-0 | 3-4-0 | 5-3-0 | 7-2-12 | 7-9-4

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [8:0-3-0,0-3-4], [9:0-3-5,0-0-11], [17:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	-0.51	16-17	>824	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.82	16-17	>509	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.34	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 212 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-15, 6-12, 8-12

REACTIONS (lb/size) 2=1584/0-3-8, 9=1584/0-3-8
Max Horz 2=-143(load case 6)
Max Uplift2=-586(load case 5), 9=-586(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2684/1027, 3-4=-6183/2167, 4-5=-2815/1182, 5-6=-1877/934, 6-7=-1686/889, 7-8=-1968/907, 8-9=-2683/1091, 9-10=0/47
BOT CHORD 2-18=-759/2265, 17-18=-893/2768, 16-17=-1101/3146, 15-16=-745/2646, 14-15=-572/1936, 13-14=-409/1657, 12-13=-409/1657, 11-12=-780/2310, 9-11=-780/2310
WEBS 3-18=-1833/645, 3-17=-1079/3513, 4-17=-786/2879, 4-16=-822/447, 5-16=-737/2505, 5-15=-1717/483, 5-14=-592/345, 6-14=-256/571, 6-12=-127/201, 7-12=-127/493, 8-12=-723/403, 8-11=0/255

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 586 lb uplift at joint 2 and 586 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051717

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T06T	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:03 2005 Page 1

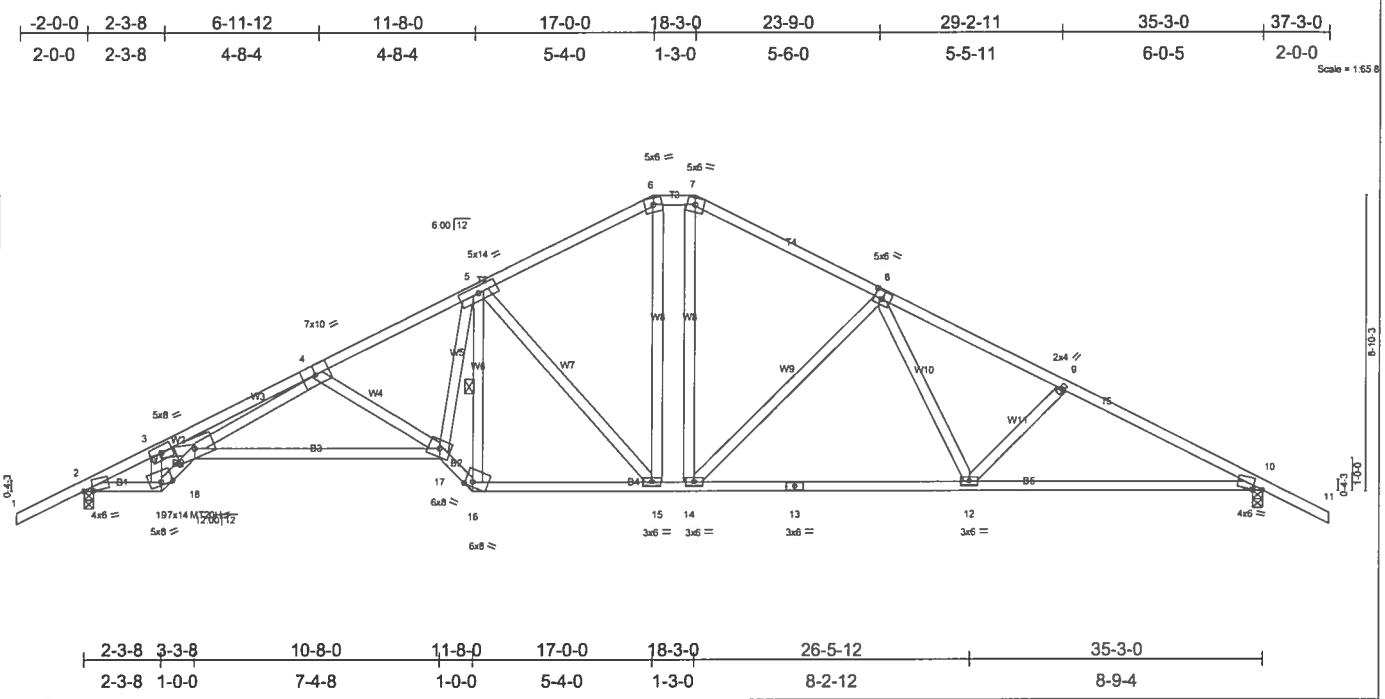


Plate Offsets (X,Y): [2:0-3-5,0-0-11], [8:0-3-0,0-3-0], [10:0-3-5,0-0-11], [16:0-2-11,Edge], [18:0-7-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.51	17-18	>827	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.82	17-18	>511	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.34	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-16

REACTIONS (lb/size) 2=1584/0-3-8, 10=1584/0-3-8
Max Horz 2=157(load case 5)
Max Uplift2=-598(load case 5), 10=-598(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2687/1045, 3-4=-6185/2202, 4-5=-2818/1205, 5-6=-1736/883, 6-7=-1508/850, 7-8=-1747/882, 8-9=-2482/1090, 9-10=-2700/1143, 10-11=0/47
BOT CHORD 2-19=-775/2268, 18-19=-912/2772, 17-18=-1116/3142, 16-17=-782/2656, 15-16=-591/1943, 14-15=-337/1508, 13-14=-606/1937, 12-13=-606/1937, 10-12=-844/2340
WEBS 3-19=-1837/658, 3-18=-1096/3510, 4-18=-804/2885, 4-17=-809/436, 5-17=-768/2504, 5-16=-1724/534, 5-15=-694/388, 6-15=-261/525, 7-14=-255/624, 8-14=-650/392, 8-12=-140/533, 9-12=-265/258

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 598 lb uplift at joint 2 and 598 lb uplift at joint 10.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051718

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T07P	SPECIAL	1	2	Job Reference (optional)

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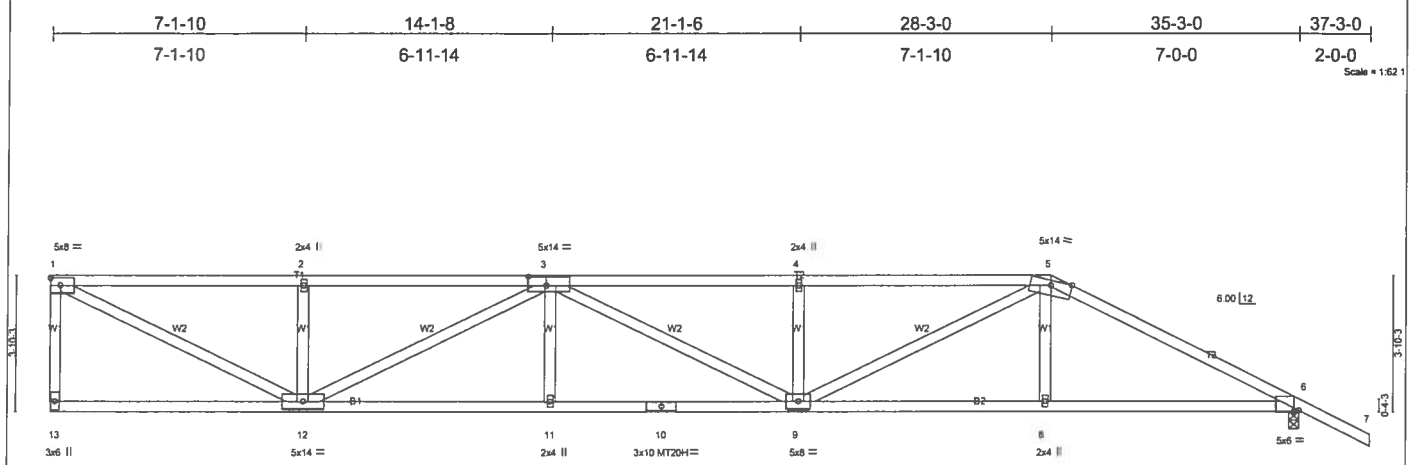

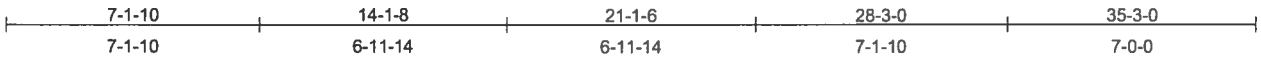

Plate Offsets (X,Y): [3:0-6-0,0-3-0], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.41	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.66	9-11	>635	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.88	Horz(TL)	0.15	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 356 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-9-7 oc bracing.

REACTIONS (lb/size) 13=3101/Mechanical, 6=3043/0-3-8
Max Horz 13=-226(load case 5)
Max Uplift13=-1321(load case 2), 6=-1203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-13=-2874/1346, 1-2=-5077/2160, 2-3=-5077/2160, 3-4=-7665/3257, 4-5=-7665/3256, 5-6=-5929/2403, 6-7=0/47
BOT CHORD 12-13=0/184, 11-12=-3083/7528, 10-11=-3081/7543, 9-10=-3081/7543, 8-9=-2070/5241, 6-8=-2063/5205
WEBS 1-12=-2334/5490, 2-12=-792/628, 3-12=-2760/1177, 3-11=0/435, 3-9=-97/135, 4-9=-819/660, 5-9=-1245/2706, 5-8=-190/875

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1321 lb uplift at joint 13 and 1203 lb uplift at joint 6.
8) Girder carries hip end with 7-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 255 lb up at 28-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-113(F=-58), 5-7=-54, 8-13=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F)

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051719

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T08P	SPECIAL	1	1	Job Reference (optional)

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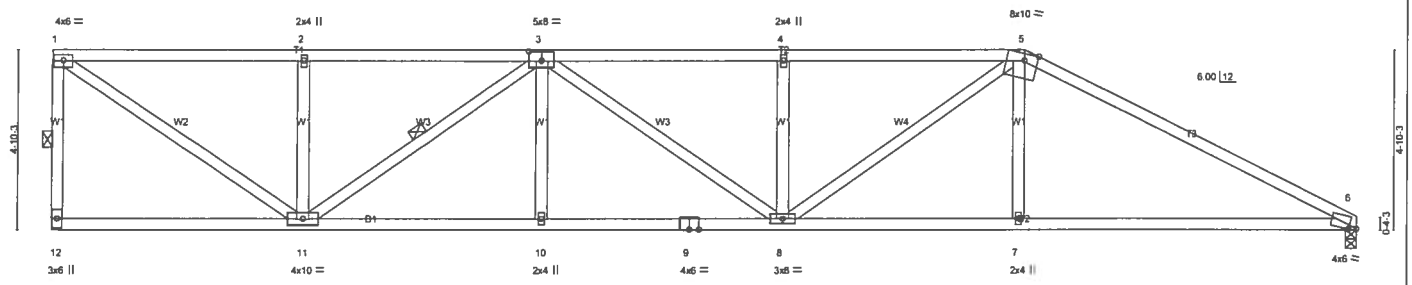
6-9-6	13-3-4	19-9-2	26-3-0	30-5-12	35-3-0
6-9-6	6-5-14	6-5-14	6-5-14	4-2-12	4-9-4

Scale = 1:59.4



6-9-6	13-3-4	19-9-2	26-3-0	35-3-0
6-9-6	6-5-14	6-5-14	6-5-14	9-0-0

Plate Offsets (X,Y): [3:0-4-0,0-3-0], [5:0-4-3,Edge], [6:0-2-9,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.35	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.57	6-7	>734	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.11	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-11 oc bracing.
WEBS 1 Row at midpt 1-12, 3-11

REACTIONS (lb/size) 12=1468/Mechanical, 6=1468/0-3-8
Max Horz 12=-210(load case 6)
Max Uplift12=-543(load case 3), 6=-408(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-12=-1366/605, 1-2=-1796/729, 2-3=-1796/729, 3-4=-2815/1177, 4-5=-2815/1177, 5-6=-2639/1029
BOT CHORD 11-12=0/209, 10-11=-883/2704, 9-10=-882/2705, 8-9=-882/2705, 7-8=-788/2272, 6-7=-788/2263
WEBS 1-11=-856/2110, 2-11=-362/264, 3-11=-1111/472, 3-10=0/204, 3-8=-76/134, 4-8=-302/255, 5-8=-369/663, 5-7=0/297

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 543 lb uplift at joint 12 and 408 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051720

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T09P	SPECIAL	1	1	Job Reference (optional)

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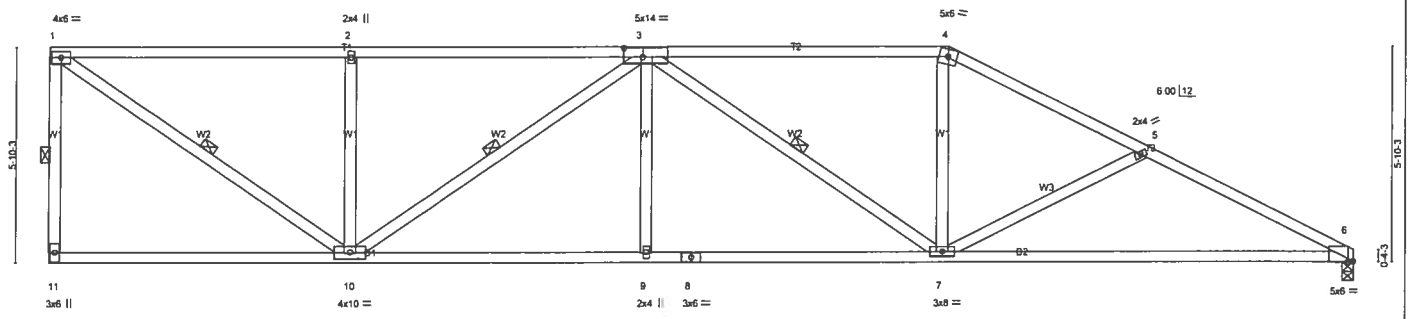
8-1-9	16-1-7	24-3-0	29-5-12	35-3-0
8-1-9	7-11-13	8-1-9	5-2-12	5-9-4

Scale = 1:59.5



8-1-9	16-1-7	24-3-0	35-3-0
8-1-9	7-11-13	8-1-9	11-0-0

Plate Offsets (X,Y): [3:0-6-0,0-3-0], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.43	6-7	>987	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.72	6-7	>585	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 1-11, 1-10, 3-10, 3-7

REACTIONS (lb/size) 11=1468/Mechanical, 6=1468/0-3-8
Max Horz 11=-256(load case 6)
Max Uplift11=-537(load case 3), 6=-418(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-1351/618, 1-2=-1710/708, 2-3=-1710/708, 3-4=-2082/925, 4-5=-2369/958, 5-6=-2678/1116
BOT CHORD 10-11=0/258, 9-10=-727/2362, 8-9=-726/2366, 7-8=-726/2366, 6-7=-909/2350
WEBS 1-10=-832/2009, 2-10=-438/321, 3-10=-798/370, 3-9=0/212, 3-7=-345/247, 4-7=-142/660, 5-7=-320/304

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 537 lb uplift at joint 11 and 418 lb uplift at joint 6.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051721

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:05 2005 Page 1

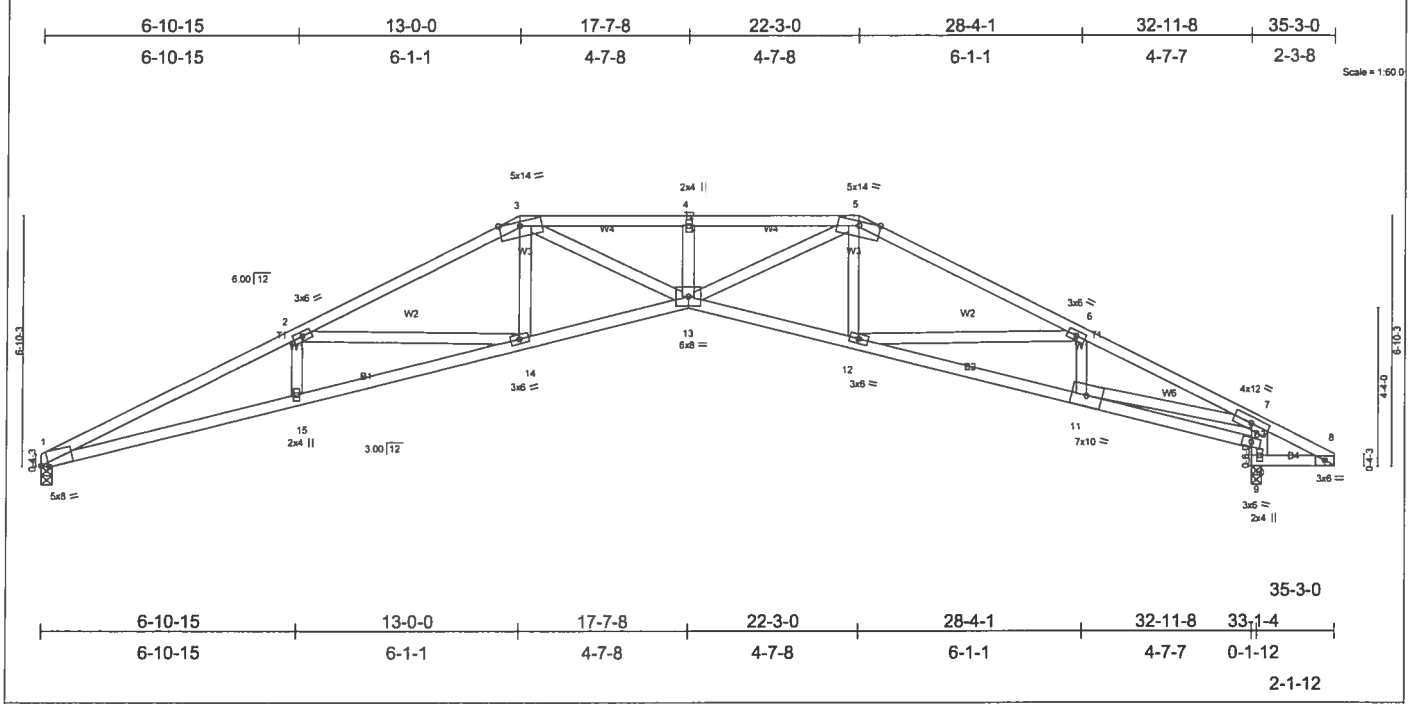


Plate Offsets (X,Y): [1:0-2-7,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.63	13-14	>627	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-1.01	13-14	>392	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.70	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 169 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B3 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-0-9 oc bracing.

REACTIONS (lb/size) 1=1382/0-3-8, 9=1566/0-3-8
Max Horz 1=96(load case 4)
Max Uplift1=-427(load case 5), 9=-532(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4771/1921, 2-3=-3740/1457, 3-4=-5074/1914, 4-5=-5074/1914, 5-6=-3494/1345, 6-7=-3509/1368, 7-8=-124/156
BOT CHORD 1-15=-1664/4318, 14-15=-1662/4314, 13-14=-1073/3404, 12-13=-970/3173, 11-12=-1139/3196, 10-11=-71/357, 9-10=-1521/706, 7-10=-1493/695, 8-9=-109/126
WEBS 2-15=0/207, 2-14=-903/575, 3-14=-147/460, 3-13=-600/1968, 4-13=-185/165, 5-13=-710/2215, 5-12=-44/237, 6-12=-127/226, 6-11=-245/207, 7-11=-1113/2752

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 1 and 532 lb uplift at joint 9.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051722

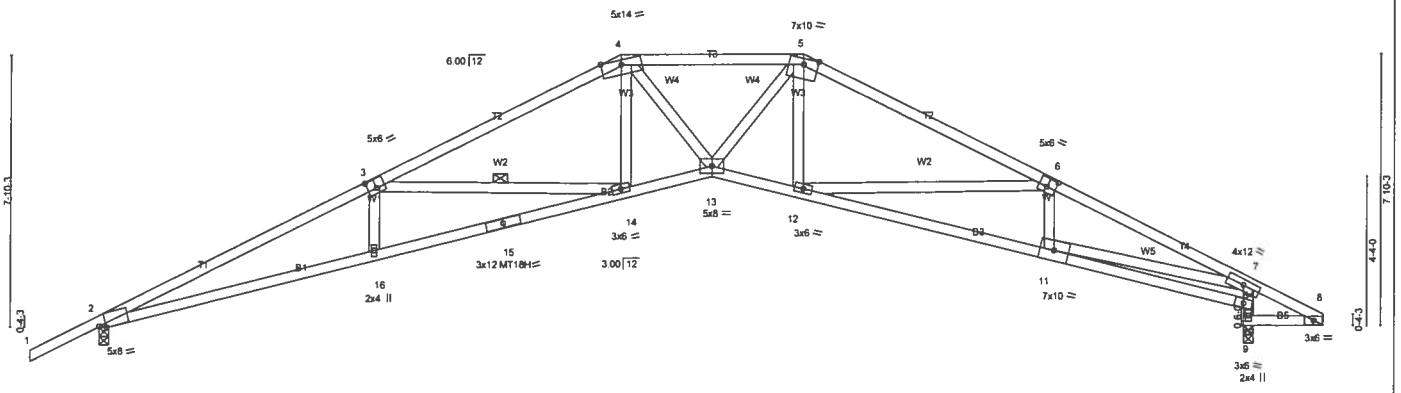
Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:06 2005 Page 1

-2-0-0 | 7-10-15 | 15-0-0 | 20-3-0 | 27-4-1 | 32-11-8 | 35-3-0

2-0-0 | 7-10-15 | 7-1-1 | 5-3-0 | 7-1-1 | 5-7-7 | 2-3-8

Scale: 3/16"=1'



35-3-0

7-10-15 | 15-0-0 | 17-7-8 | 20-3-0 | 27-4-1 | 32-11-8 | 33-1-4

7-10-15 | 7-1-1 | 2-7-8 | 2-7-8 | 7-1-1 | 5-7-7 | 0-1-12

2-1-12

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-4], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.53	14-16	>744	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.86	14-16	>462	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.59	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 173 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except* B4 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-14

REACTIONS (lb/size) 9=1567/0-3-8, 2=1498/0-3-8
Max Horz 2=167(load case 5)
Max Uplift9=-549(load case 6), 2=-565(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4636/1802, 3-4=-3373/1287, 4-5=-3463/1370, 5-6=-3261/1239, 6-7=-3600/1408, 7-8=-127/155
BOT CHORD 2-16=-1536/4192, 15-16=-1538/4161, 14-15=-1532/4188, 13-14=-872/3036, 12-13=-830/2930, 11-12=-1163/3277, 10-11=-79/380, 9-10=-1526/726, 7-10=-1499/724, 8-9=-101/121
WEBS 3-16=0/240, 3-14=-1113/648, 4-14=-162/533, 4-13=-223/803, 5-13=-288/962, 5-12=-72/347, 6-12=-384/349, 6-11=-160/192, 7-11=-1131/2822

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 549 lb uplift at joint 9 and 565 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

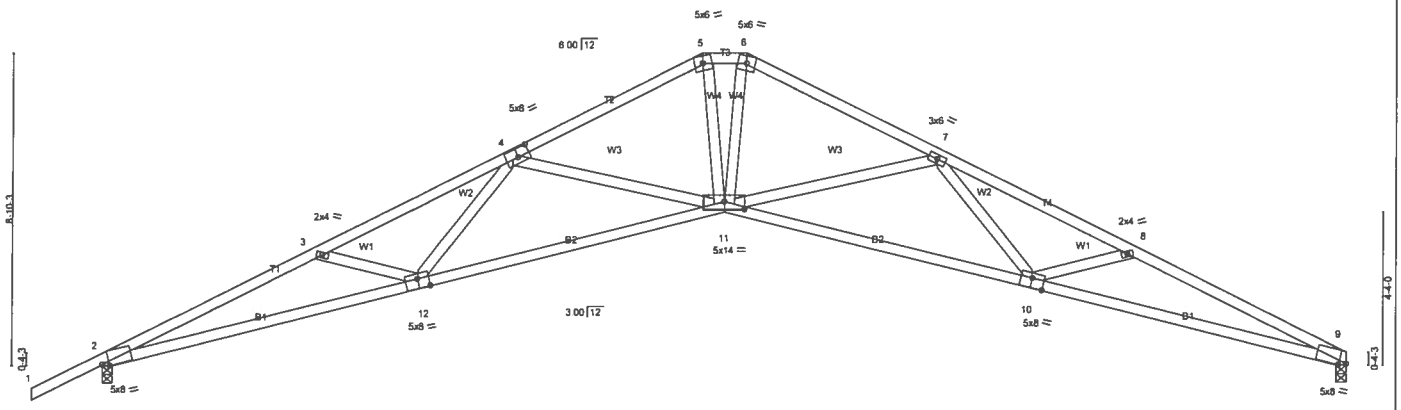
Dwg.#1129051723

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:07 2005 Page 1

-2-0-0	6-2-10	11-7-5	17-0-0	18-3-0	23-7-11	29-0-6	35-3-0
2-0-0	6-2-10	5-4-11	5-4-11	1-3-0	5-4-11	5-4-11	6-2-10

Scale = 1:62.4



8-11-0	17-7-8	26-4-0	35-3-0
8-11-0	8-8-8	8-8-8	8-11-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-12,0-3-0], [9:0-2-7,Edge], [10:0-4-0,0-3-4], [11:0-7-0,0-2-12], [12:0-4-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.70	11-12	>596	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-1.13	11-12	>370	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.76	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-5 oc bracing.

REACTIONS (lb/size) 9=1465/0-3-8, 2=1588/0-3-8
Max Horz 2=180(load case 5)
Max Uplift9=-472(load case 6), 2=-599(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4999/2069, 3-4=-4661/1858, 4-5=-3320/1313, 5-6=-3099/1297, 6-7=-3320/1313, 7-8=-4701/1923, 8-9=-5040/2161
BOT CHORD 2-12=-1794/4533, 11-12=-1381/3893, 10-11=-1406/3909, 9-10=-1891/4592
WEBS 3-12=-289/322, 4-12=-161/640, 4-11=-873/556, 5-11=-460/1300, 6-11=-452/1295, 7-11=-889/581, 7-10=-213/672, 8-10=-312/359

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 9, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 472 lb uplift at joint 9 and 599 lb uplift at joint 2.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

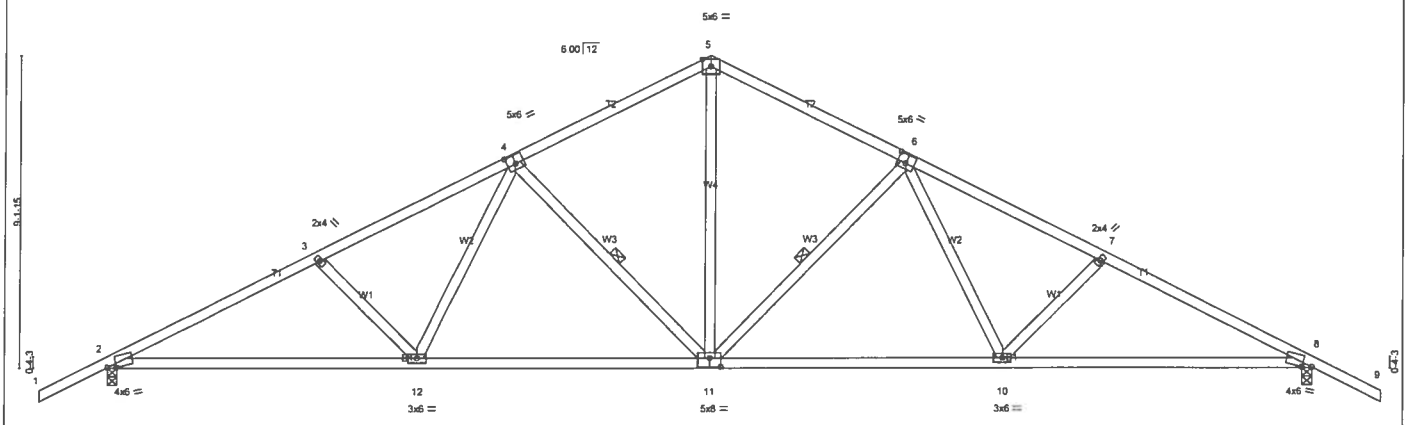
Dwg.#1129051724

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T13	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:07 2005 Page 1

-2-0-0	6-2-13	11-11-3	17-7-8	23-3-13	29-0-3	35-3-0	37-3-0
2-0-0	6-2-13	5-8-5	5-8-5	5-8-5	5-8-5	6-2-13	2-0-0

Scale: 3/16"=1'



9-1-0	17-7-8	26-2-0	35-3-0
9-1-0	8-6-8	8-6-8	9-1-0

Plate Offsets (X,Y): [2:0-3-5,0-0-11], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-3-5,0-0-11], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.23	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.38	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-7 oc bracing.
WEBS 1 Row at midpt 4-11, 6-11

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=162(load case 5)
Max Uplift2=-601(load case 5), 8=-601(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2673/1147, 3-4=-2457/1090, 4-5=-1713/876, 5-6=-1713/876, 6-7=-2457/1090, 7-8=-2673/1147, 8-9=0/47
BOT CHORD 2-12=-846/2330, 11-12=-591/1903, 10-11=-591/1903, 8-10=-846/2330
WEBS 3-12=-289/275, 4-12=-153/540, 4-11=-641/397, 5-11=-531/1154, 6-11=-641/397, 6-10=-153/540, 7-10=-289/275

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

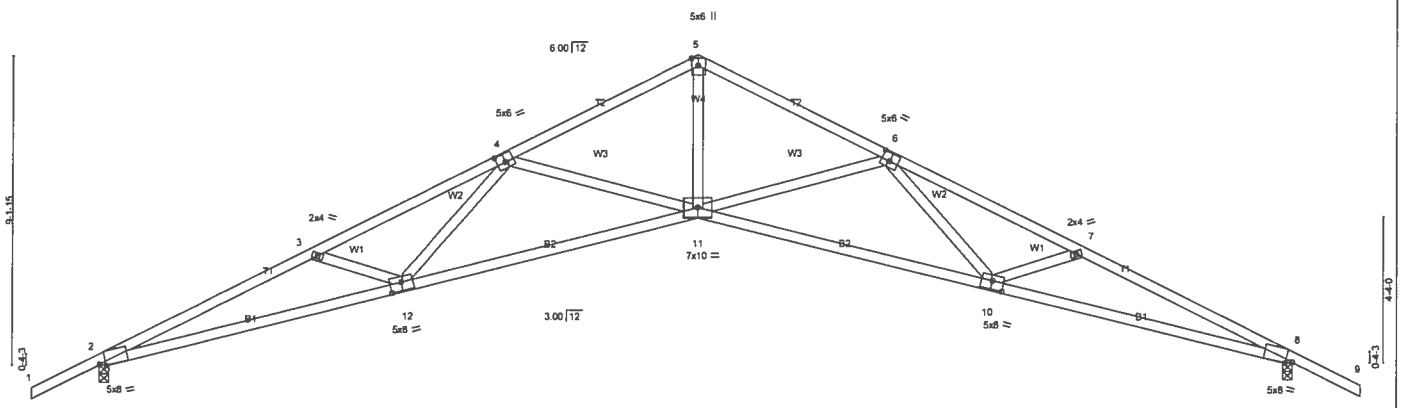
Dwg.#1129051725

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T14	SCISSOR	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:08 2005 Page 1

-2-0-0	6-5-2	12-0-5	17-7-8	23-2-11	28-9-14	35-3-0	37-3-0
2-0-0	6-5-2	5-7-3	5-7-3	5-7-3	5-7-3	6-5-2	2-0-0

Scale = 1:65.1



8-11-0	17-7-8	26-4-0	35-3-0
8-11-0	8-8-8	8-8-8	8-11-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-2-7,Edge], [10:0-4-0,0-3-0], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.69	11-12	>608	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-1.11	11-12	>377	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.74	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-2-11 oc bracing.

REACTIONS (lb/size) 2=1584/0-3-8, 8=1584/0-3-8
Max Horz 2=161(load case 5)
Max Uplift2=-601(load case 5), 8=-601(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4985/1974, 3-4=-4646/1772, 4-5=-3296/1217, 5-6=-3296/1217, 6-7=-4646/1772, 7-8=-4985/1974, 8-9=0/46
BOT CHORD 2-12=-1629/4520, 11-12=-1181/3813, 10-11=-1181/3813, 8-10=-1629/4520
WEBS 3-12=-293/330, 4-12=-204/704, 4-11=-845/539, 5-11=-843/2579, 6-11=-845/539, 6-10=-204/704, 7-10=-293/330

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 601 lb uplift at joint 2 and 601 lb uplift at joint 8.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051726

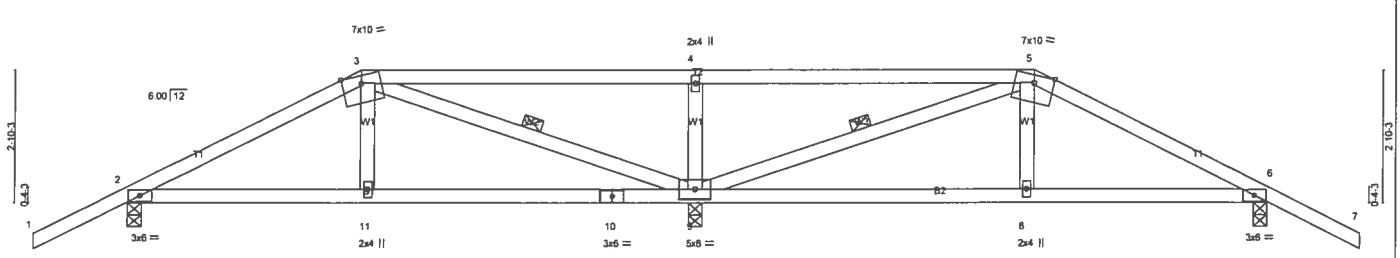
Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T15P	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:08 2005 Page 1

-2-0-0 | 5-0-0 | 12-1-12 | 19-4-8 | 24-4-8 | 26-4-8

2-0-0 | 5-0-0 | 7-1-12 | 7-2-12 | 5-0-0 | 2-0-0

Scale = 1:47.1



5-0-0 | 12-1-12 | 19-4-8 | 24-4-8

5-0-0 | 7-1-12 | 7-2-12 | 5-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.09	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.14	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-3 oc bracing.
WEBS 1 Row at midpt 3-9, 5-9

REACTIONS (lb/size) 2=789/0-3-8, 9=1860/0-3-8, 6=795/0-3-8
Max Horz 2=-73(load case 5)
Max Uplift2=-532(load case 4), 9=-922(load case 3), 6=-389(load case 5)
Max Grav 2=790(load case 8), 9=1860(load case 1), 6=796(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1138/632, 3-4=-31/130, 4-5=-31/130, 5-6=-1156/407, 6-7=0/47
BOT CHORD 2-11=-535/966, 10-11=-548/993, 9-10=-548/993, 8-9=-311/1011, 6-8=-309/983
WEBS 3-11=-241/472, 3-9=-1191/686, 4-9=-686/532, 5-9=-1207/452, 5-8=-51/476

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 2, 922 lb uplift at joint 9 and 389 lb uplift at joint 6.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 19-4-8, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

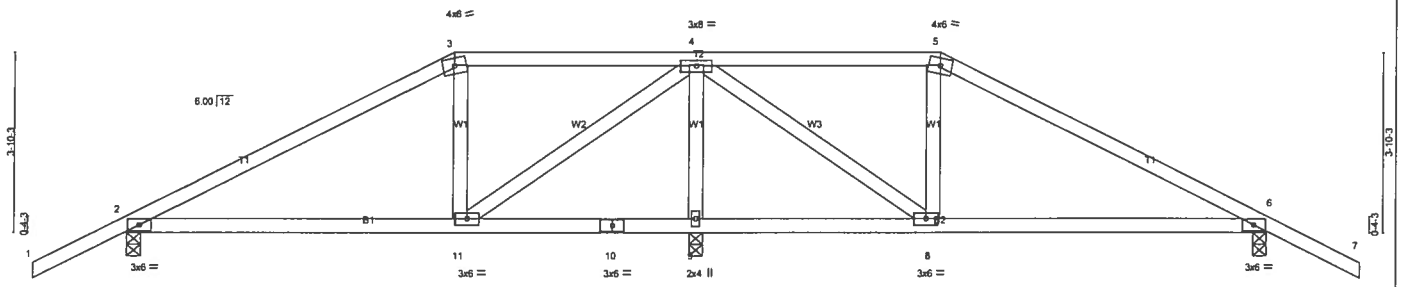
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-86(F=-31), 5-7=-54, 2-11=-30, 8-11=-47(F=-17), 6-8=-30
Concentrated Loads (lb)
Vert: 11=-245(F) 8=-245(F)

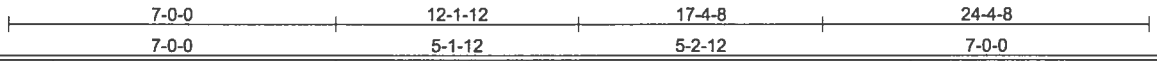
NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051727

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T16P	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Nov 29 15:53:09 2005 Page 1





LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.08	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	-0.13	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.26	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=568/0-3-8, 9=1115/0-3-8, 6=572/0-3-8
Max Horz 2=-87(load case 6)
Max Uplift2=-300(load case 5), 9=-286(load case 4), 6=-317(load case 6)
Max Grav 2=575(load case 9), 9=1115(load case 1), 6=578(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-509/240, 3-4=-376/281, 4-5=-384/284, 5-6=-518/243, 6-7=0/47
BOT CHORD 2-11=-79/382, 10-11=-174/143, 9-10=-174/143, 8-9=-174/143, 6-8=-54/390
WEBS 3-11=-169/155, 4-11=-203/659, 4-9=-997/391, 4-8=-205/666, 5-8=-167/156

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2, 286 lb uplift at joint 9 and 317 lb uplift at joint 6.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051728

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T17P	HIP	1	1	Job Reference (optional)

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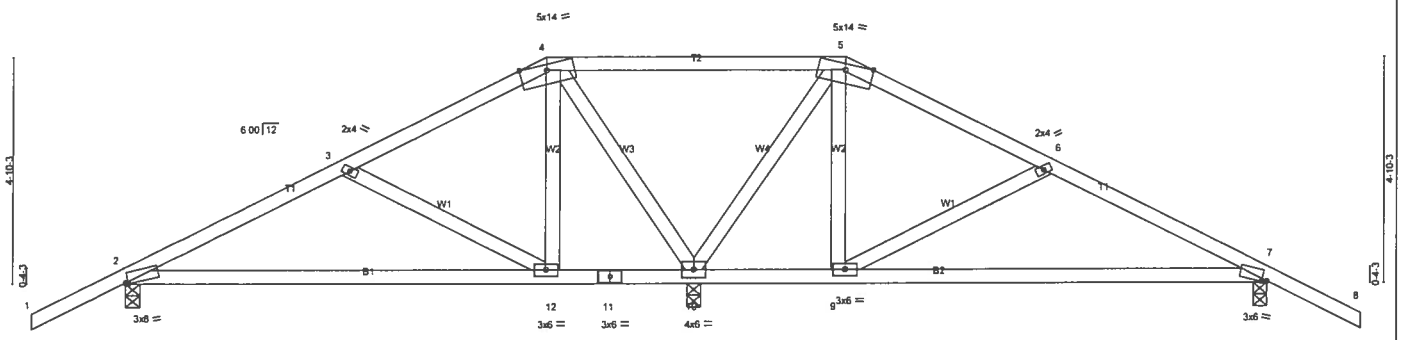
-2-0-0	4-9-4	9-0-0	15-4-8	19-7-4	24-4-8	26-4-8
2-0-0	4-9-4	4-2-12	6-4-8	4-2-12	4-9-4	2-0-0

Scale = 1:47.1



9-0-0	12-1-12	15-4-8	24-4-8
9-0-0	3-1-12	3-2-12	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.30	2-12	>474	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	0.25	2-12	>582	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=495/0-3-8, 10=1260/0-3-8, 7=500/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-400(load case 5), 10=-517(load case 5), 7=-270(load case 6)
Max Grav 2=529(load case 9), 10=1260(load case 1), 7=533(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-435/420, 3-4=-179/259, 4-5=-120/391, 5-6=-188/57, 6-7=-443/156, 7-8=0/47
BOT CHORD 2-12=-242/356, 11-12=-77/131, 10-11=-77/131, 9-10=0/235, 7-9=-18/364
WEBS 3-12=-273/305, 4-12=-502/379, 4-10=-798/732, 5-10=-798/336, 5-9=-45/378, 6-9=-273/216

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 2, 517 lb uplift at joint 10 and 270 lb uplift at joint 7.

LOAD CASE(S) Standard

NOVEMBER 29, 2005 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051729

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T18H	HIP	1	1	Job Reference (optional)

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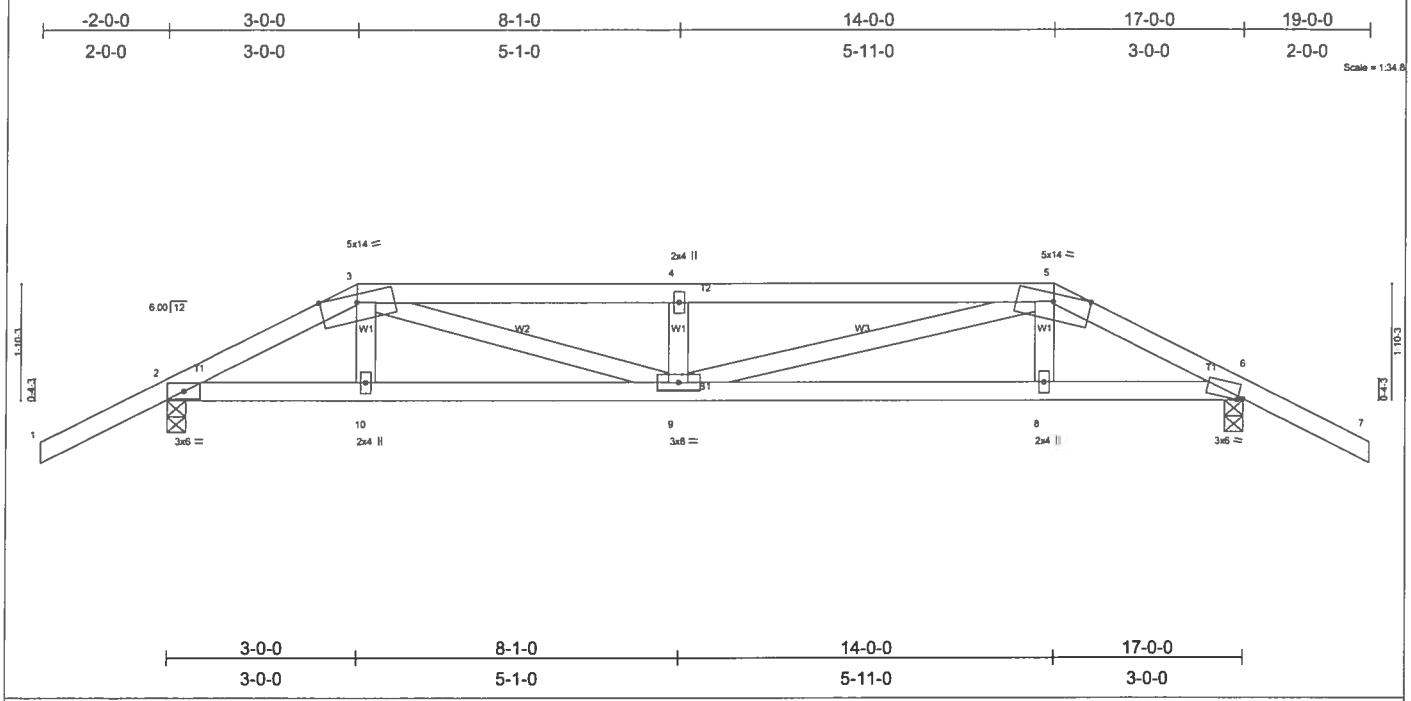

Plate Offsets (X,Y): [6:0-1-0,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.15	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.22	8-9	>914	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.

REACTIONS (lb/size) 2=962/0-3-8, 6=962/0-3-8
Max Horz 2=-59(load case 5)
Max Uplift2=-595(load case 4), 6=-595(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1543/939, 3-4=-2338/1484, 4-5=-2338/1484, 5-6=-1590/969, 6-7=0/47
BOT CHORD 2-10=-797/1327, 9-10=-807/1344, 8-9=-829/1397, 6-8=-818/1377
WEBS 3-10=-108/194, 3-9=-703/1051, 4-9=-358/264, 5-9=-663/987, 5-8=-122/221

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 6.
5) Girder carries hip end with 3-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 14-0-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-64(F=-10), 5-7=-54, 2-10=-30, 8-10=-35(F=-5), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-63(F) 8=-63(F)

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Dwg.#1129051730

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T19P	HIP	1	1	Job Reference (optional)

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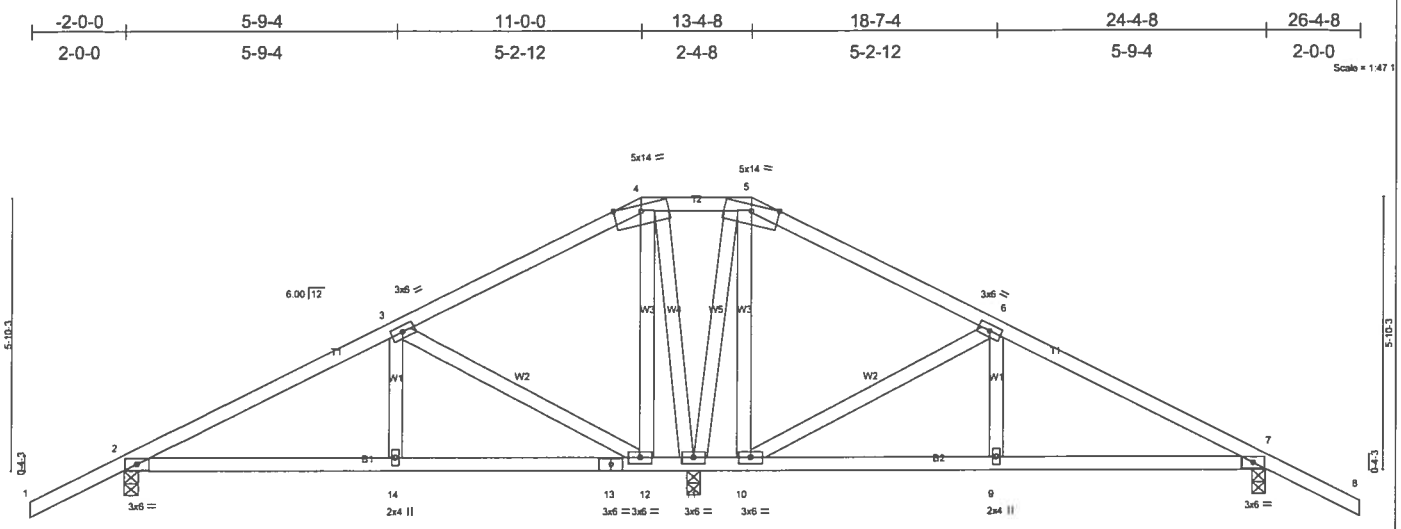

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.05	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=455/0-3-8, 11=1341/0-3-8, 7=459/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-263(load case 5), 11=-367(load case 5), 7=-287(load case 6)
Max Grav 2=496(load case 9), 11=1341(load case 1), 7=500(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-422/156, 3-4=-19/326, 4-5=0/353, 5-6=-20/316, 6-7=-431/160, 7-8=0/47
BOT CHORD 2-14=-68/316, 13-14=-68/316, 12-13=-68/316, 11-12=-233/272, 10-11=-224/268, 9-10=-23/324, 7-9=-23/324
WEBS 3-14=0/189, 3-12=-544/281, 4-12=-62/290, 4-11=-605/202, 5-11=-609/206, 5-10=-66/297, 6-10=-543/281, 6-9=0/189

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 263 lb uplift at joint 2, 367 lb uplift at joint 11 and 287 lb uplift at joint 7.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1129051731

Job	Truss	Truss Type	Qty	Ply	BRYAN ZECHER- LOT 66 EMERALD COVE
L138117	T20P	SPECIAL	1	2	Job Reference (optional)

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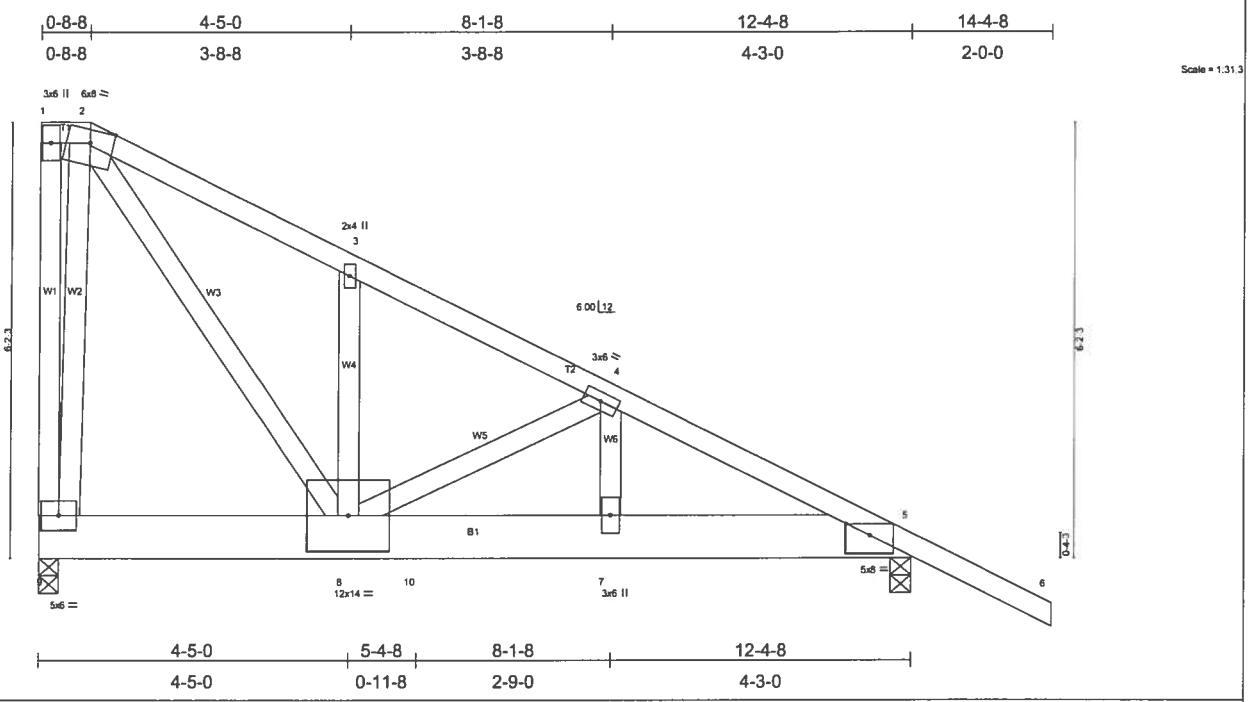

Plate Offsets (X,Y): [5:0-4-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.07	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.12	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 201 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 9=4690/0-3-8, 5=2498/0-3-8
Max Horz 9=-336(load case 5)
Max Uplift9=-1816(load case 5), 5=-967(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=-345/135, 1-2=-66/27, 2-3=-4050/1516, 3-4=-4058/1419, 4-5=-4953/1683, 5-6=0/53
BOT CHORD 8-9=-7/362, 8-10=-1395/4400, 7-10=-1395/4400, 5-7=-1395/4400
WEBS 2-9=-2977/1229, 2-8=-2286/5932, 3-8=-202/186, 4-8=-924/374, 4-7=-171/595

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1816 lb uplift at joint 9 and 967 lb uplift at joint 5.
6) Girder carries tie-in span(s): 35-3-0 from 0-0-0 to 4-4-8
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3101 lb down and 1171 lb up at 5-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-6=-54, 8-9=-722(F=-692), 5-8=-30
Concentrated Loads (lb)
Vert: 10=-3101(F)

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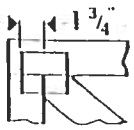
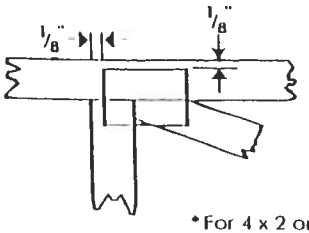
Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

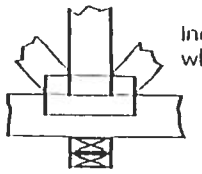
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

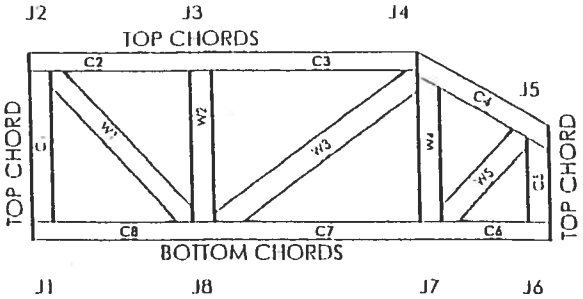
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transfering connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** (see NOTE 1)
		a) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
☑	☐	d) Provide a full legal description of property.
		Wind-load Engineering Summary, calculations and any details required
		a) Plans or specifications must state compliance with FBC Section 1606
		b) The following information must be shown as per section 1606.1.7 FBC
		a. Basic wind speed (MPH)
		b. Wind importance factor (I) and building category
		c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
		d. The applicable internal pressure coefficient
		e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

a) Rooms labeled and dimensioned

b) Shear walls

c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)

d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth

e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails

f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing

b) All posts and/or column footing including size and reinforcing

c) Any special support required by soil analysis such as piling

d) Location of any vertical steel

Roof System:

a) Truss package including:

1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

b) Conventional Framing Layout including:

1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

NA

Wall Sections including:

a) Masonry wall

1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

NA

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ ☐ ~~c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)~~ N/A

Floor Framing System:

☐ ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ ☐ ~~b) Floor joist size and spacing~~

☐ ☐ c) Girder size and spacing N/A

☐ ☐ d) Attachment of ~~joist~~ to girder

☐ ☐ ~~e) Wind~~ load requirements where applicable

☐ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☑ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment N/A

Disclosure Statement for Owner Builders

Notice Of Commencement

Private Potable Water — private water system

a) Size of pump motor

b) Size of pressure tank

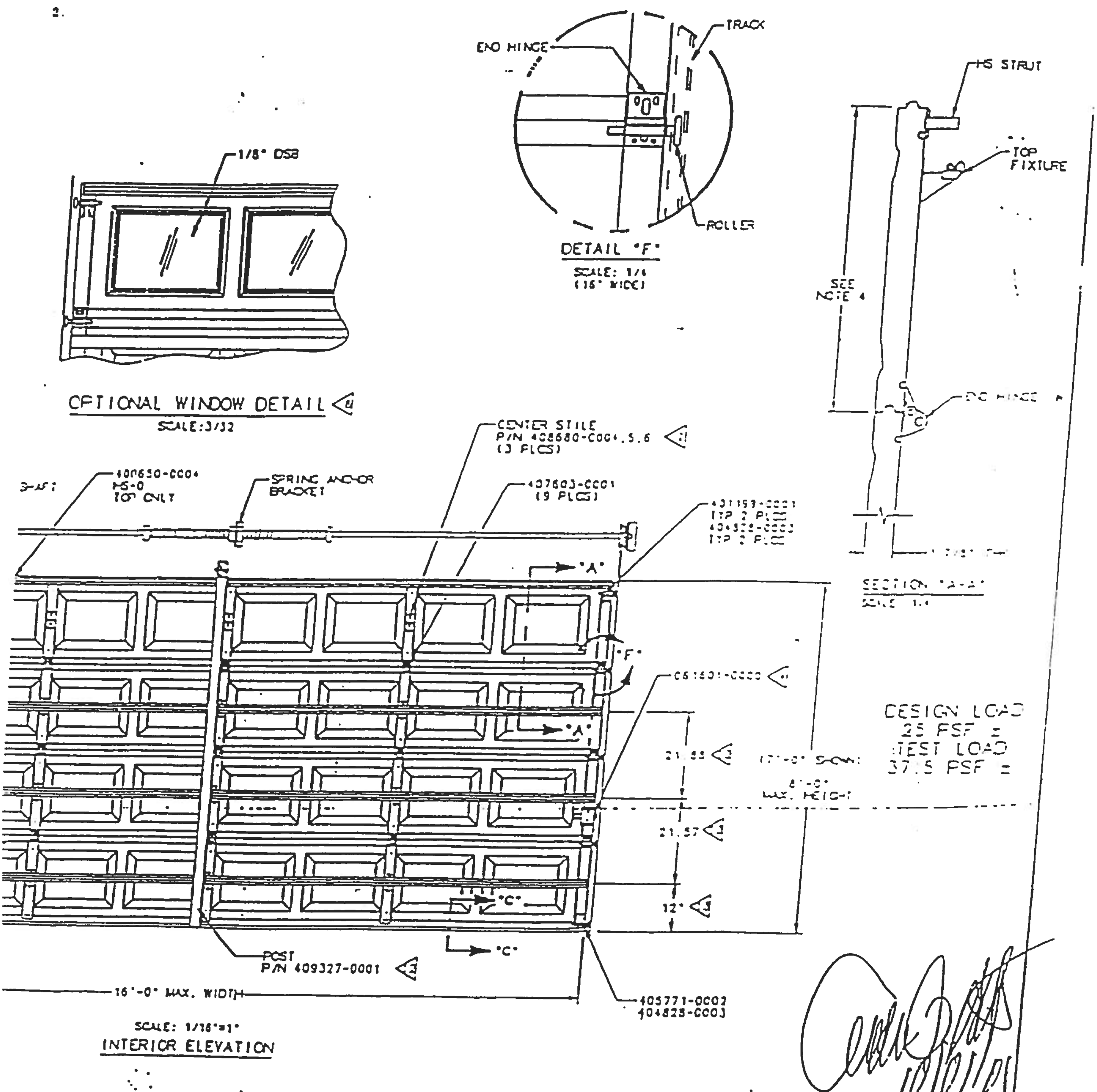
c) Cycle stop valve if used

ɹCTED WITH 5 OR MORE SECTIONS MUST BE
ƐS/RU'S FOR THE ADDITIONAL INTERMEDIATE
ON THE THIRD SECTION.
..L SECTIONS LESS THAN 20.812" MUST BE
THE ACTUAL SECTION HEIGHT & 20.812".
¼ X 5/8" LONG TYPE AB HEX HEAD METAL SCREW
4 WIND WARNINGS ISSUED.

2.

GARAGE DOORS

REVISIONS			
LETTER	DESCRIPTION	DATE	APPROVED
A	REV PER EN 10130	3/06/98	DL
B	REV PER EN 10141	3/13/98	DL
C	REV PER EN 10132	[illegible]	DL

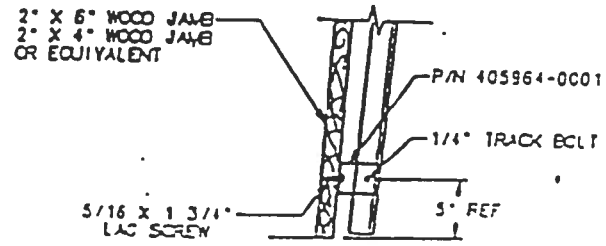


OVERHEAD DOOR, DALLAS, TEXAS

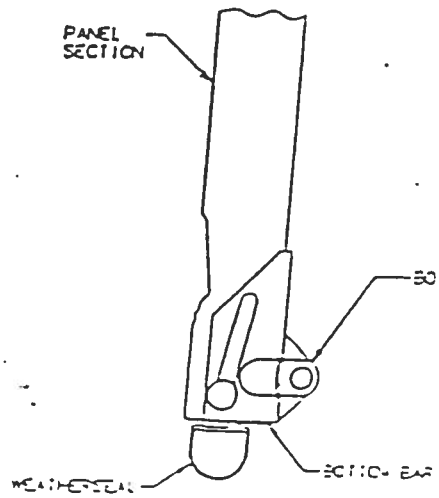
DRAWN BY: M. YOUNIS	01/13/98	SERIES 280 & 180 RESI STL DR. 16'-0" MAX WIDTH, WINDLOAD
MATERIAL: N/A		
CHECKED BY: DAVID FAW	02/19/98	DRAWING NUMBER: D-409335
SCALE: NONE / UNIT OF MEASURE: EACH	APPROVED BY: DAVID FAW	02/19/98

NOTES:

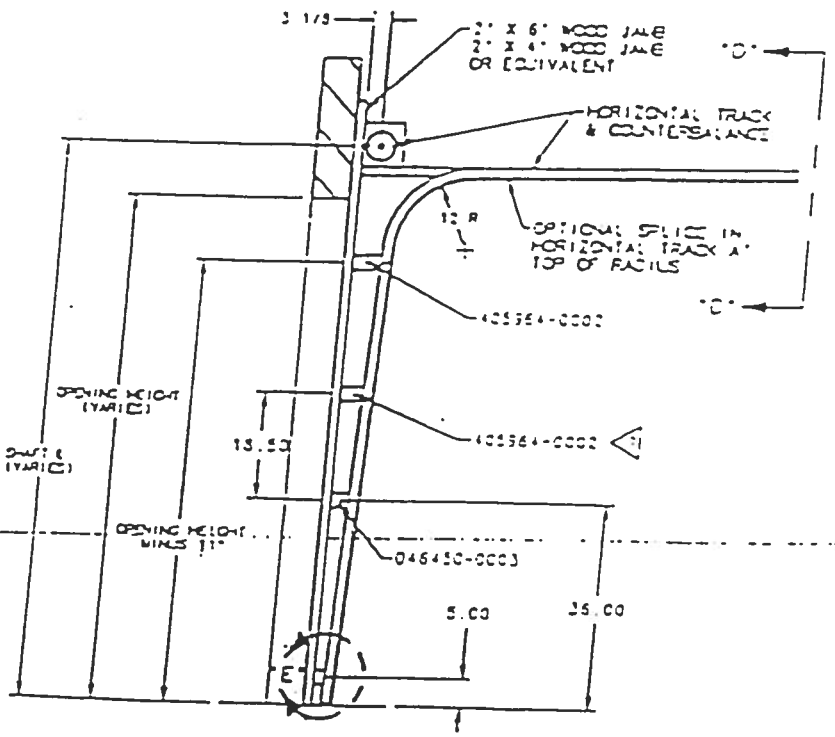
1. TESTED IN ACCORDANCE WITH STANDARD BUILDING CODE, CHAPTER 17, TO A POSITIVE AND NEGATIVE 37.5 PSF.
2. DASH NUMBERS REPRESENT VARIOUS SECTION HEIGHTS.
3. FOUR SECTION 7' HIGH DOOR SHOWN. 8' HIGH DOORS HAVE FIVE SECTIONS.
4. SECTION HEIGHT OF 20.312, 19.00 & 16.75 ARE AVAILABLE AND MAY BE USED IN COMBINATION TO ACHIEVE VARIOUS HEIGHT DOORS.
5. EMBOSSMENT PATTERN OF 14.50 X 20.375 SHOWN. ALTERNATE PATTERNS OF 12.50 X 43.375 AND 12.50 X 20.375 MAY BE USED.
6. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
7. USE THIS BRACKET, REF. P/N 405964-0002, ON 8' HIGH DOORS ONLY.
8. WINDOW MAY BE INSTALLED IN THE TOP SECTION OR THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
9. THE STRUT PLACEMENT ON D CONSISTENT WITH THE DOOR SECTIONS ARE TO BE PLACED
10. THE STRUT PLACEMENT DIMEN REDUCED BY THE DIFFERENCE
11. SCREW P/N 605911-0001 IS
12. POST TO BE INSTALLED ONLY
13. STRUT PLACEMENTS CAN VARY
14. QUANTITY FOR LOCKS CAN BE



DETAIL "E"
SCALE: 1/3

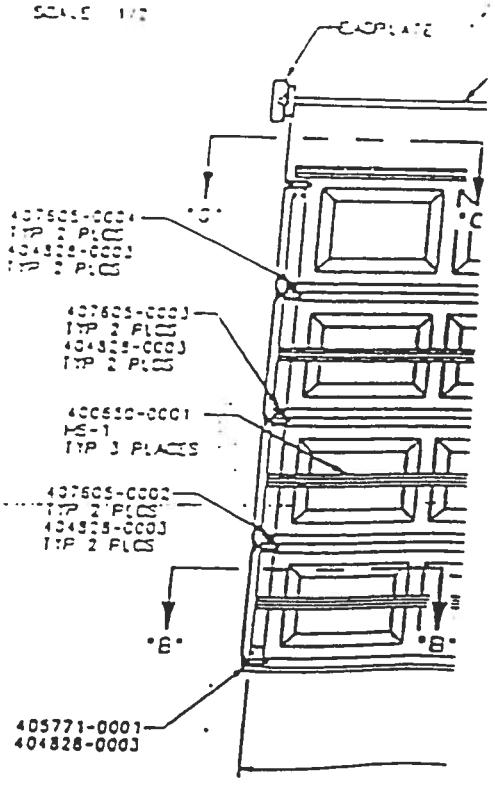


VIEW "D-
2" TRACK
.049 THK (VERT



STANDARD TRACK DETAIL FOR 16'
SCALE: 1/16" = 1"

SECTION "C-C"
SCALE 1/2



SERIES 280 THRU 289 ARE EQUIVALENT CONSTRUCTION 25GA STEEL
SERIES 180 ARE SAME CONSTRUCTION AS SERIES 280 ONLY 24GA STEEL AND END CAPS

DOOR TESTED WAS 281 SERIES.

DOOR WIDTH	CENTER STILE	END STILE	ROLLER SHAFT BRACKET	STRUTS/SECT.	ROLLER	VERTICAL TRACK GAGE	JAMB LOAD (# PER FT-HT)	HARDWARE
16'	3	SINGLE		MS1 MS0	2" X 7/16"	.049"	100	STD.

SHINGLES

ELK Premium Roofing

March 6, 2002

Subject: Elk Product Approval Information

All Prestique® and Capstone® products manufactured in Tuscaloosa, AL are certified under the Miami – Dade County Building Code Office (BCCO). These products also meet the requirements for the Florida Building Code since they are MD approved. The following test protocols must be passed by each of the products in order for MD product certification:
ASTM D3462
PA 100 (110 mph uplift and wind driven rain resistance)
PA 107 (Modified ASTM D3161 - 110 mph wind uplift resistance)

The nailing patterns that were used during the PA 100 and PA 107 wind test protocols for the Prestique and Capstone products are listed below. Also listed below are the Miami – Dade Notice of Acceptance Numbers (NOA)

Raised Profile, Prestique High Definition, Prestique 25, or Prestique 30 –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.04

Prestique I 35 or Prestique I* –
PA 100 = 4 nails
PA 107 = 5 nails
MD NOA# = 01-1226.05

Prestique Plus or Prestique Gallery Collection* –
PA 100 = 4 nails
PA 107 = 4 nails
MD NOA# = 01-1226.03

Capstone*
PA 100 = 4 Nails
PA 107 = 4 Nails
MD NOA# = 01-0523.01

* As per the Elk Limited Warranty, six nails are required for the Elk high wind warranty

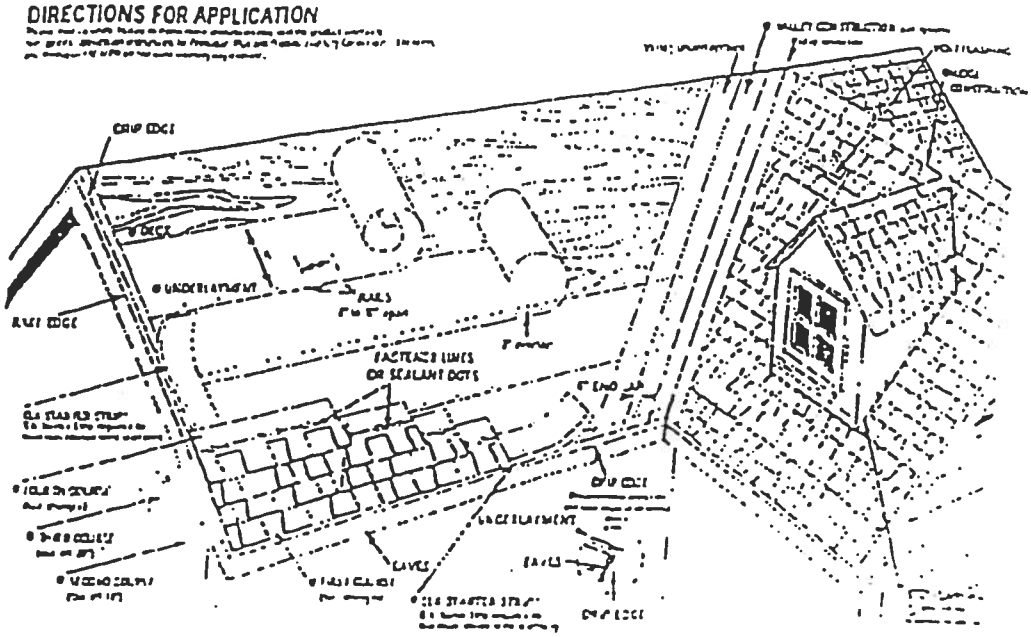
If there are any questions please contact:

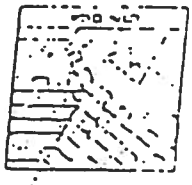
Mike Reed – Technical Manager (205) 342-0287

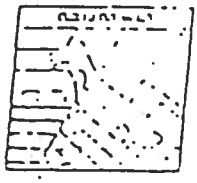
or

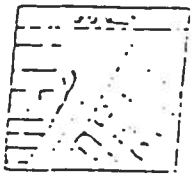
Daniel DeJarnette – QA Engineer (205) 342-0293

DIRECTIONS FOR APPLICATION









ELK

www.elkcorp.com

ROOFING PRODUCTS SPECIFICATIONS - TUSCALOOSA, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE™

[illegible]

HIP AND RIDGE SHINGLES

[illegible]

All Prestique and Raised Profile shingles meet UL Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790) and ASTM Specifications D 3018, Type I, D 3161, Type I, E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles meet the latest Miami-Dade building code requirements.

[illegible]

SOUTHEAST & ATLANTIC OFFICE: 800.945.5551

CORPORATE HEADQUARTERS: 800.354.7732

PLANT LOCATION: 800.945.5545

ELK
www.elkcorp.com

WINDOWS

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:mlb

STATE OF FLORIDA PROFESSIONAL ENGINEER

1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.architest.com

Allen G. Reum
1 APRIL 2002

REGISTERED PROFESSIONAL ENGINEER
STATE OF FLORIDA
NO. 19354

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

ALLEN N. REEVES CERTIFICATE NO. 13734 STATE OF FLORIDA PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft²	0.3 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE NO. 18377
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Optional Performance

Paragraph	Title of Test - Test Method	Results	Allowed
4.3	Water Resistance (ASTM E 547-00)		
	(with and without screen)		
	WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the meeting rail)		
	(Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Paragraph	Title of Test - Test Method	Results	Allowed
	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.18" max.
	@ 70.8 psf (negative)	0.05"	0.18" max.

ALLEN N. REEVES
CERTIFICATE
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:clb
01-41134.01

ALLEN N. REEVES
CERTIFICATE NO. 17924
STATE OF FLORIDA
PROFESSIONAL ENGINEER

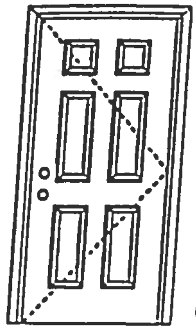
X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
limited water unless special threshold design is used

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7 [illegible] state or local building codes specify the edition required

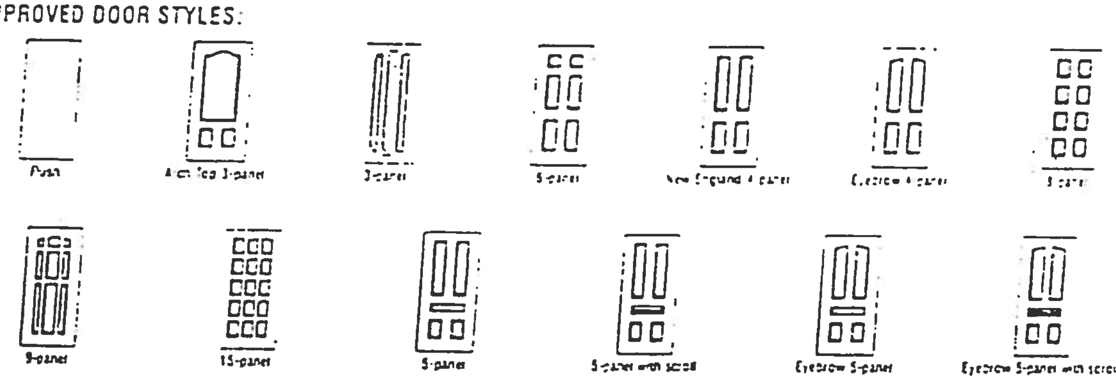
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02

APPROVED DOOR STYLES:



Johnson EntrySystems
June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice

PREMDOR Collection Premium Quality Doors
Exclusively from Masonite
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

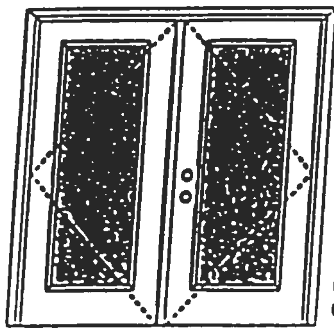
PREMDOOR
Exclusively from Masonite
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Double Door
[illegible]

Design Pressure
+40.5/-40.5
[illegible]

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED

[illegible]

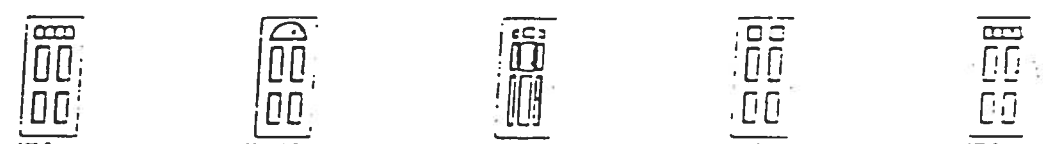
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02

MINIMUM INSTALLATION DETAIL:

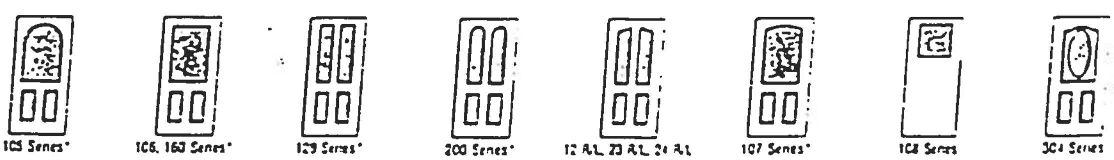
Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02

APPROVED DOOR STYLES.
1/4 GLASS:



100 Series | 120, 125 Series | 134 Series | 630 Series | [illegible] Series

1/2 GLASS:



105 Series* | 106, 160 Series* | 129 Series* | 200 Series* | 12 R.L, 23 R.L, 24 R.L Series* | 107 Series* | 108 Series | 304 Series

*This glass kit may also be used in the following door styles: 5-panel, 5-panel with scroll, Eyebrow 5-panel, Eyebrow 5-panel with scroll

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOOR
Exclusively from Masonite
Masonite International Corporation

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


145 Series


320 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12, NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Penney, P.E. / 16258

Unit Tested in Accordance with Miami-Dade ECCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from [illegible]. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in [illegible] plastic lip lite surround

Frame constructed of wood with an extruded aluminum threshold

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE ECCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3025447A and CCP/Test Report Validation Matrix #3025447A-001 provides additional information - available from the ITS/WH website (www.ritswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Johnson EntrySystems

June 17, 2002

PREMDOR

Masonite

Masonite International Corporation

GARAGE HEADER (84) $26F_b$ - 1.9E

Anthony POWER HEADER®

ENGINEERED WOOD SECTION PROPERTIES AND LOAD CAPACITIES

ALLOWABLE DESIGN STRESSES (PSI):

FLEXURAL STRESS (F_b) =	2600
COMPRESSION PERP. TO GRAIN ($F_{c\perp}$) =	740
HORIZONTAL SHEAR (F_v) =	225
MODULUS OF ELASTICITY (MOE) =	1.9×10^6

	7.7	9.0	10.4	11.7	12.9	14.2	15.5
	326	514	789	1115	1521	2014	2604
	8865	12015	15996	20145	24772	29877	35460
	3908	4550	5250	5892	6533	7175	7817

NOTES:

1. Beam weights are based on 38 pcf.
2. Moment capacities are based on a span of 21 feet and must be modified for other spans.
3. Flexural Stress, F_b, shall be modified by the Volume Factor, C_v, as outlined in AITC 117 - Design 1593 and the NDS for Wood Construction 1597.
4. Allowable design properties and load capacities are based on a load duration of 100 percent and dry use conditions.
5. The AITC NER 466 was used in calculating the above allowable design stresses for POWER HEADER®.

GARAGE HEADER COMPARISONS

	810 / 540	3-1/2" x 8-3/8"	3-1/2" x 9-5/8"	3-1/2" x 9"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	990 / 720	3-1/2" x 9-3/4"	3-1/2" x 9-5/8"	3-1/2" x 10-1/2"	3-1/2" x 9-1/4"	3-1/2" x 11-1/4"**
	610 / 400	3-1/2" x 12-5/8"	3-1/2" x 13-3/4"	3-1/2" x 13-1/2"	3-1/2" x 14"	3-1/2" x 14"*
	765 / 510	3-1/2" x 14"	3-1/2" x 15-1/8"	3-1/2" x 15"	3-1/2" x 14"	3-1/2" x 16"*
	750 / 490	3-1/2" x 15-3/8"	3-1/2" x 16-1/2"	3-1/2" x 16-1/2"	3-1/2" x 16"	3-1/2" x 18"*
	900 / 600	3-1/2" x 16-3/4"	3-1/2" x 17-7/8"	3-1/2" x 18"	3-1/2" x 16"	—

For more information on POWER HEADER®, or other laminated structural products from Anthony Forest Products Company please call 1-800-221-2326 or FAX at 870-862-6502.

POWER HEADER® is a trademark of
Anthony Forest Products Company
Post Office Box 1877 • El Dorado, Arkansas 71731
Internet address: http:// www.anthonyforest.com
e-mail: info@anthonyforest.com
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Distributed by:

WOODFORD PLYWOOD, INC.
"Structural Wood Products"

11960 West Beaver Street
Jacksonville, Florida 32220

(904) 695-9080
(800) 447-6568
FAX (904) 695-9160

Anthony POWER HEADER®

$26F_b$ - 1.9E

3-1/2" WIDTH GARAGE HEADER PLF CAPACITY

844	896		1216		1573						
161	207	254	330	390	510	552	669	752	824		
114	145	180	231	277	359	391	510	534	653	707	789

844	975		1322								
161	207	254	330	390	510	552	724	752	897		
114	145	180	231	277	359	391	510	534	699	693	

562	778	888	1056	1363	1367		1582						
107	153	169	245	260	380	368	540	501	715	664	864	840	
76	107	120	171	185	267	261	380	356	521	471	684	609	813

NOTES:

1. Values shown are the maximum uniform loads in pounds per lineal foot (PLF) that can be applied to the header. Header weight has been subtracted from the allowable total load.
2. Tables are based on simple span uniform load conditions using a design span equal to the center-to-center of bearing. Non-shaded areas are based on 3" of bearing at each support, shaded areas on 4.5" of bearing, and shaded & outlined areas on 6" of bearing at supports.
3. Headers are assumed to be loaded on the top edge with continuous lateral support along compression edge.
4. When no live load is listed, total load controls.
5. Deflection limits are listed within the PLF table heading.

GARAGE HEADER SIZING USING PLF TABLES:

To size a garage header supporting roof only, determine the total load & live load in pounds per lineal foot (PLF). Check the appropriate PLF table for a header supporting roof loads only (125% Non-Snow vs. 115% Snow) and select a member with a total load and live load capacity which meets or exceeds the design load for the rough opening size. For a garage header supporting roof, wall, and floor framing, determine the total load and live load in pounds per lineal foot (PLF). Select a header size from the roof, wall, and floor table (100% load duration) which has a total load and live load capacity equal to or greater than the design load for the appropriate rough opening.

Residential System Sizing Calculation

Summary

Model Home

Project Title:
TheVictoriaModel510319

Class 3 Rating
Registration No. 0
Climate: North

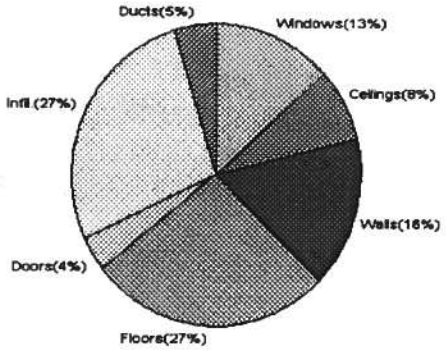
11/7/2005

Location for weather data: Gainesville - Defaults: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	93	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	18	F
Total heating load calculation	**24321**	**Btuh**	**Total cooling load calculation**	**21361**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	115.1	28000	Sensible (SHR = 0.75)	136.9	21000
Heat Pump + Auxiliary(0.0kW)	115.1	28000	Latent	116.3	7000
			Total (Electric Heat Pump)	131.1	28000

WINTER CALCULATIONS

Winter Heating Load (for 1430 sqft)

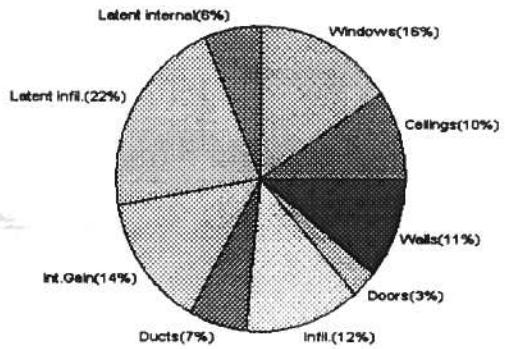
Load component			Load	
Window total	116	sqft	3283	Btuh
Wall total	1457	sqft	3995	Btuh
Door total	60	sqft	921	Btuh
Ceiling total	1460	sqft	1898	Btuh
Floor total	206	ft	6510	Btuh
Infiltration	153	cfm	6557	Btuh
Subtotal			**23163**	**Btuh**
Duct loss			1158	Btuh
TOTAL HEAT LOSS			**24321**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1430 sqft)

Load component			Load	
Window total	116	sqft	3328	Btuh
Wall total	1457	sqft	2292	Btuh
Door total	60	sqft	608	Btuh
Ceiling total	1460	sqft	2073	Btuh
Floor total			0	Btuh
Infiltration	134	cfm	2648	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**13949**	**Btuh**
Duct gain			1395	Btuh
Total sensible gain			**15343**	**Btuh**
Latent gain(infiltration)			4638	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**6018**	**Btuh**
TOTAL HEAT GAIN			**21361**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.

PREPARED BY: [signature]

DATE: 11-7-05

System Sizing Calculations - Winter

Residential Load - Component Details

Model Home

Project Title:
TheVictoriaModel510319

Class 3 Rating
Registration No. 0
Climate: North

,

Reference City: Gainesville (Defaults) Winter Temperature Difference: 39.0 F

11/7/2005

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Metal, DEF	N	3.0	28.3	85 Btuh
2	2, Clear, Metal, DEF	N	45.0	28.3	1274 Btuh
3	2, Clear, Metal, DEF	N	20.0	28.3	566 Btuh
4	2, Clear, Metal, DEF	E	3.0	28.3	85 Btuh
5	2, Clear, Metal, DEF	E	15.0	28.3	424 Btuh
6	2, Clear, Metal, DEF	S	30.0	28.3	849 Btuh
	Window Total		116		3283 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Adjacent	13.0	348	1.6	557 Btuh
2	Frame - Exterior	13.0	1109	3.1	3438 Btuh
	Wall Total		1457		3995 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		20	18.3	367 Btuh
2	Insulated - Exter		20	18.3	367 Btuh
3	Insulated - Adjac		20	9.4	188 Btuh
	Door Total		60		921Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1460	1.3	1898 Btuh
	Ceiling Total		1460		1898Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	206.0 ft(p)	31.6	6510 Btuh
	Floor Total		206		6510 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.80	11440(sqft)	153	6557 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			153	6557 Btuh

Totals for Heating	**Subtotal**	**23163 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1158 Btuh**
	Total Btuh Loss	**24321 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Model Home

Project Title:
TheVictoriaModel510319

Class 3 Rating
Registration No. 0
Climate: North

Reference City: Gainesville (Defaults) Summer Temperature Difference: 18.0 F 11/7/2005

Window	Type Panes/SHGC/U/InSh/ExSh	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	1.5	3.0	0.0	3.0	22	22	66	Btuh
2	2, Clear, DEF, N, N	N	1.5	5.5	45.0	0.0	45.0	22	22	990	Btuh
3	2, Clear, DEF, N, N	N	1.5	7.5	20.0	0.0	20.0	22	22	440	Btuh
4	2, Clear, DEF, N, N	E	1.5	1.5	3.0	2.5	0.5	22	72	92	Btuh
5	2, Clear, DEF, N, N	E	1.5	5.5	15.0	0.0	15.0	22	72	1080	Btuh
6	2, Clear, DEF, N, N	S	1.5	5.5	30.0	30.0	0.0	22	37	660	Btuh
	Window Total				116					3328	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Adjacent	13.0	348.0	1.0	362	Btuh
2	Frame - Exterior	13.0	1109.0	1.7	1930	Btuh
	Wall Total		1457.0		2292	Btuh

Doors	Type	Area	HTM	Load	
1	Insulated - Exter	20.0	10.1	203	Btuh
2	Insulated - Exter	20.0	10.1	203	Btuh
3	Insulated - Adjac	20.0	10.1	203	Btuh
	Door Total	60.0		608	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1460.0	1.4	2073	Btuh
	Ceiling Total		1460.0		2073	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	206.0 ft(p)	0.0	0	Btuh
	Floor Total		206.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.70	11440	133.7	2648	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			134	2648	Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load	
	6	X 300 +	1200	3000	Btuh

Totals for Cooling	**Subtotal**	**13949**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**1395**	**Btuh**
	Total sensible gain	**15343**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**4638**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**21361**	**Btuh**

Manual J Summer Calculations

Residential Load - Component Details (continued)

Model Home

Project Title:
TheVictoriaModel510319

Class 3 Rating
Registration No. 0
Climate: North

11/7/2005

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

April 4, 2006

Cornerstone Developers, LLC
180 NW Amenity Court
Lake City, Florida 32055

Attention: Frank Soucinek

Reference: Proposed Residences
Emerald Cove, Phase I, Lots 66 and 67
Columbia County, Florida
Cal-Tech Project No. 06-207

Dear Mr. Soucinek,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lots 66 and 67 of Emerald Cove, Phase I in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of homes to be constructed on these lots and to provide recommendations for selecting finished floor elevations.

Based upon the U. S. Coast and Geodetic Survey marker "BP19" located near the intersection of U. S. 90 and Brown Road, elevations of the roadway centerline and proposed finished floor for each residence are provided in the following table. For both residences the proposed finished floor elevation is below the adjacent roadway centerline elevation.

Lot Number	Centerline Elevation (ft)	Proposed Floor Elevation (ft)
66	128.06	126.67
67	126.43	126.22

24231
23913

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the FEMA flood map for Columbia County, the drainage basin in which the proposed home sites are located is not a flood area; therefore, flooding of the proposed homes should not be expected if floors are constructed at the proposed elevations. If for some reason however flooding did occur within this drainage basin, flooding to an elevation of 112 feet would produce flood depths on the order of 25 feet within portions of the basin. Flooding to this depth is highly unlikely.

It should be noted a relatively large, topographically isolated flood area is located approximately one-half mile south southeast of the building sites. The flood elevation for this area has not been determined by FEMA; however, based upon the area delineated by the flood map, this flood elevation is estimated to be about 112.0 feet. The proposed finished floor elevations are each roughly 14 feet above this flood elevation; therefore, flooding should not be expected.

Elevating the floor of either residence to 12 inches above the adjacent roadway should not be required; however, we recommend for each site the finished floor be a minimum of 12 inches above the finished surface grade at the perimeter of the residence.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer
4/4/06
52612

PMD

Rec. 35.50
Cert. Copy 5.50

24231

Prepared by and after
recording return to:

William L. Joel
Stoneburner Berry & Simmons, P.A.
841 Prudential Drive, Suite 1400
Jacksonville, FL 32207

Permit No. _____
Tax Folio No. _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA

COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvements will be made to certain real property, and in accordance with section 713 Florida Statutes, the following information is provided in this notice of commencement.

1. Description of property (legal description and address, if available):

   See Exhibit "A" attached hereto.

2. General description of improvements: Construction of residential dwellings

3. Owner Information:

   (a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
   180 NW Amenity Court
   Lake City, Florida 32025

   (b) Owner's interest in the site of the improvements (if other than fee simple title holder):

   (c) Name and Address of fee simple title holder (if other than owner):

4. Contractor:

   Name and Address: Bryan Zecher Construction Inc.
   465 NW Orange Street
   Lake City, FL 32055

   (b) Phone No. 386-752-8653 Fax No. _____ (Optional, if service by fax is acceptable)

5. Surety on any payment bond: N/A

   (a) Name and Address:
   (b) Phone No. _____ Fax No. _____ (Optional, if service by fax is acceptable)

Inst:2006006010 Date:03/13/2006 Time:09:12
_____ DC,P.DeWitt Cason,Columbia County B:1076 P:2461

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Teagle
Deputy Clerk
Date 3-13-06

COLUMBIA COUNTY, FLORIDA CLERK CIRCUIT COURT

(c) Amount of bond $_____

6. Lender making loan for the construction of the improvements:

(a) Name and Address: First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256
Attn: James J. O'Connor, Jr.

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

7. Persons within the State of Florida designated by owner upon whom notices may be served as provided by Section 713.13(1)(a)7, Florida Statutes:

(a) Name and Address: Cornerstone Developers, LLC a Florida limited liability company
180 NW Amenity Court
Lake City, Florida 32025

(b) Phone No. (386) 752-1711 Fax No. _____ (Optional, if service by fax is acceptable)

8. In addition to himself, Owner designates the following person to receive a copy of the lienor's notice as provided in Section 713.13(1)(b), Florida Statute:

(a) Name and Address: James J. O'Connor, Jr.
First Horizon Home Loan Corporation
1051 Deerwood Park Boulevard
Building 200, Suite 115
Jacksonville, FL 32256

(b) Phone No. 904-998-5300 Fax No. _____ (Optional, if service by fax is acceptable)

9. Expiration date of notice of commencement (the expiration date is one (1) year from the date of recording unless a different date is specified):

CORNERSTONE DEVELOPERS, LLC

By: _____
Frank Soucinek, its sole Manager

(SEAL)

(OWNER)

Inst:2006006010 Date:03/13/2006 Time:09:12
_____DC,P.DeWitt Cason,Columbia County B:1076 P:2462

Sworn to and Subscribed before me this 8th
day of MARCH, 2006, by Frank Soucinek, the
Manager of Cornerstone Developers, LLC. He
is personally known or has produced _____
as identification.

_____
Notary Public, State and County Aforesaid
Print Name:
My Commission Expires:
My Commission No.:

TERRY MCDAVID
MY COMMISSION # DD 500788
EXPIRES: January 16, 2010
Bonded Thru Notary Public Underwriters

(NOTARIAL SEAL)

POST A CERTIFIED COPY OF THE RECORDED NOTICE ON CONSTRUCTION SITE

Inst:2006006010 Date:03/13/2006 Time:09:12
_____DC,P.DeWitt Cason,Columbia County B:1076 P:2463

EXHIBIT A

Lot 66, Emerald Cove, Phase 1, according to the map or plat thereof as recorded in Plat Book 8, Page(s) 35 and 36, Public Records of Columbia County, Florida.

Inst:2006006010 Date:03/13/2006 Time:09:12
DC,P.DeWitt Cason,Columbia County B:1076 P:2464

#24231

MARK DISOSWAY P.E.
Ph 386-754-5419
POB868, Lake City, FL 32056
PE No FL-53915

TABLE R404.1.1(1)
PLAIN CONCRETE AND PLAIN MASONRY FOUNDATION WALLS

MAXIMUM WALL HEIGHT (feet)	MAXIMUM UNBAL-ANCED BACKFILL HEIGHT[c] (feet)	PLAIN CONCRETE MINIMUM NOMINAL WALL THICKNESS (inches)			PLAIN MASONRY[a] MINIMUM NOMINAL WALL THICKNESS (inches)		
		Soil classes[b]				SANDY FILL	
		GW, GP, SW and SP	GM, GC, SM, SM-SC and ML	SC, MH, ML-CL and inorganic CL	GW, GP, SW and SP	GM, GC, SM, SM-SC and ML	SC, MH, ML-CL and inorganic CL
5	4	6	6	6	6 solid[d] or 8	6 solid[d] or 8	6 solid[d] or 8
	5	6	6	6	6 solid[d] or 8	8	10
6	4	6	6	6	6 solid[d] or 8	6 solid[d] or 8	6 solid[d] or 8
	5	6	6	6	6 solid[d] or 8	8	10
	6	6	8[g]	8[g]	8	10	12
7	4	6	6	6	6 solid[d] or 8	8	8
	5	6	6	8[g]	6 solid[d] or 8	10	10
	6	6	8	8	10	12	10 solid[d]
	7	8	8	10	12	10 solid[d]	12 solid[d]
8	4	6	6	6	6 solid[d] or 8	6 solid[d] or 8	8
	5	6	6	8	6 solid[d] or 8	10	12
	6	8[g]	8	10	10	12	12 solid[d]
	7	8	10	10	12	12 solid[d]	Footnote e
	8	10	10	12	10 solid[d]	12 solid[d]	Footnote e
9	4	6	6	6	6 solid[d] or 8	6 solid[d] or 8	8
	5	6	8[g]	8	8	10	12
	6	8	8	10	10	12	12 solid[d]
	7	8	10	10	12	12 solid[d]	Footnote e
	8	10	10	12	12 solid[d]	Footnote e	Footnote e
	9	10	12	Footnote f	Footnote e	Footnote e	Footnote e

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square inch = 6.895 Pa.

a. Mortar shall be Type M or S and masonry shall be laid in running bond. Ungrouted hollow masonry units are permitted except where otherwise indicated.

b. Soil classes are in accordance with the Unified Soil Classification System. Refer to Table R405.1.

c. Unbalanced backfill height is the difference in height of the exterior and interior finish ground levels. Where an interior concrete slab is provided, the unbalanced backfill height shall be measured from the exterior finish ground level to the top of the interior concrete slab.

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FOUNDATION LETTER
CORNER STONE
JOB # 510319A
EMERALD COVE
LOT # 66

THIS CODE TABLE IS FOR PLAIN MASONRY FOUNDATION WALL (NO REBAR)

d. Solid grouted hollow units or solid masonry units.
e. Wall construction shall be in accordance with Table R404.1.1(2) or a design shall be provided.
f. A design is required.
g. Thickness may be 6 inches, provided minimum specified compressive strength of concrete, *fc*, is 4,000 psi.

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MWD 20206006

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CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 33-3S-16-02438-166 **Building permit No.** 000024231

Use Classification SFD,UTILITY **Fire:** 17.76

Permit Holder BRYAN ZECHER **Waste:** 36.75

Owner of Building CORNERSTONE DEVELOPERS **Total:** 54.51

Location: 176 SW FIELDSTONE COURT(EMERALD COVE,LOT 66)

Date: 07/21/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)