

BUILDING LOAD SUMMARY

BUILDING CODE: FLORIDA BUILDING CODE

ED: 2010

JOB LOCATION: GAINESVILLE, FL

>>> ROOF LOADS:

Dead Load: 0.97 psf

Live Load: 20 psf

Tributary Reduction: X Y N

Collateral Load: 0 psf

Sprinkler Load: 0 psf

Ground Snow Load: N/A psf

Flat roof Snow load: N/A psf

Snow exposure: N/A

Snow Importance: N/A

Thermal Factor: N/A

>>> WIND LOADS:

Wind Velocity (ULT): 130 mph

Fastest Mile: N/A

3-sec Gust: X

Wind velocity (ASD): 101 mph

Risk Category: II

Wind Exposure: B

Enclosure: Encl Partial Open

Internal pressure coefficient: 0.18

Components and Cladding:

Zone1 12.36 / 16.11 psf

Zone 4 16.50 / 17.89 psf

Zone2 12.36 / 26.48 psf

Zone 5 16.50 / 21.87 psf

Zone3 12.36 / 42.4 psf

>>> EARTHQUAKE LOADS:

Base Seismic Force Resisting System:

Design Base Shear: N/A

Analysis Used: N/A

Seismic Importance: N/A

Risk Category: N/A

Site Class: N/A

Seismic Design Category: N/A

S(S): N/A g

S(1): N/A g

C(S): N/A

R: N/A

>>> MEZZANINE LOADS:

Dead Load: psf

Live Load: psf

Reduction: %

>>> OTHER LOADS:

NONE

DOOR PRESSURES (PSF)					
QTY		DESCRIPTION	APPROVAL #	DESIGN INTERIOR	DESIGN EDGE
1	3070S-W	SOLID WALK DOOR WITH LEVER LOCKSET	FL10294	+17.4 / -18.9	+17.4 / -22.7
				+50.0 / -50.0	

>>> DEAN STEEL BUILDINGS, INC. ACTS AS A RESELLER OF THE ABOVE LISTED ACCESSORIES
>>> DEAN STEEL BUILDINGS, INC. CERTIFIES THE LOADS APPLIED DO NOT EXCEED THE MANUFACTURER'S STATED RESISTANCE PRESSURE.

SHEETING					FRAMING SCREW		STITCH SCREW		
	PANEL TYPE	APPROVAL #	MATL.	COLOR	MARK	PART #	SPACING	MARK	PART #
ROOF	RIB-12	FL3774.1	24 GAGE	GL2		HW5510	12" O.C.		HW5502
WALLS	RIB-12	FL3775.1	26 GAGE	GR2		HW5515	12" O.C.		HW5511

ALTHOUGH WE TRY TO MAKE ALL DRAWINGS AS ACCURATE AS POSSIBLE, ANY DRAWING MARKED AS PERMIT OR APPROVAL HAS NOT BEEN FINAL DETAILED OR CHECKED. THE CUSTOMER ACCEPTS ALL RESPONSIBILITY FOR USING PERMIT OR APPROVAL DRAWINGS FOR ANY PURPOSE OTHER THAN OBTAINING A BUILDING PERMIT OR REVIEWING THE DETAILS FOR CONTRACT COMPLIANCE.

FLASHING		
	MATL.	COLOR
GUTTER	26 GAGE	AS
EAVE FLASHING	26 GAGE	AS
RAKE	26 GAGE	AS
DOWNSPOUTS	26 GAGE	AS
CORNERS	26 GAGE	AS
FRAMED OPENINGS	26 GAGE	AS
OTHER	26 GAGE	AS

IMPORTANT --- PLEASE READ THE FOLLOWING:

>>> General
Dean Steel Buildings, Inc. standard product specifications apply, unless stipulated otherwise on the Dean Steel Buildings' purchase order. Dean Steel Buildings design practices, manufacturing processes, accessories and warranties will govern the work. A Dean Steel Buildings Standard Specifications Brochure is available upon request.

The builder/contractor is responsible for the setting of the anchor bolts and the erection of the steel building components in accordance with Dean Steel Buildings, Inc. "For Construction" drawings.

>>> Unloading Check-out and Shortages:
It is essential, while unloading your Dean Steel Building, to verify that all components listed on the shipping papers were actually shipped. The actual quantities of the items on each truck are circled on the shipping papers. Any items found to be shipped short, or in direct conflict with the shipping papers should be noted on the driver's report. This serves two purposes: 1) You, as the erector, will be aware that a particular item is missing and can temporarily work around it; 2) it lets us know that we have a problem and allow us to react quickly. This system eliminates the discovery that something is missing at the time you need it to erect the building. Taking the time to properly unload the job and check it allows for proper placing of the parts around the job site, which should expedite the erection process. Dean will do its utmost to fill any reported shortages as quickly as possible. Once the erector, or owner, signs that he has received the goods and accepted them as being complete, we can only assume that shortages after this point are missing due to job site theft. All claims for damage or shortage must be presented, in writing, to the carrier--either Dean Steel Buildings, Inc., or common carrier, within seven days after receipt of materials by purchaser. Failure to do so voids any claim.

>>> Storage and Protection of Materials:
A Galvanic action known as "white rust" may result when aluminum, galvanized, or the galvanized pre-painted coating on piled flat sheets or nested formed sheets becomes wet from rain, condensation, or other causes. Under certain weather conditions this "white rust" can happen in as little as 24-48 hours. Galvalume coated sheet is susceptible, as is galvanized sheet, to wet storage staining. However, due to the composition of the coating, the sheet surface will develop a dark gray discoloration as opposed to the white oxide that develops on galvanized. Formed pre-painted sheets must be protected from moisture, in the same manner as plain galvanized or Galvalume sheets, if they are in contact with other sheets. The sheets must be properly packaged and stored. It is important upon receipt of material to examine packages for damage. Builders should take prompt action where cuts, tears, or other damage is evident. If moisture is present the panel should be dried at once. Panels that cannot be stored out of the elements should be re-stacked individually and spacers put between the panels, so that individual panels can have air circulated around them (non-metallic spacers; i.e., wood, cardboard, etc.). Bundled panels should be off the ground sufficiently to prevent rising water from coming in contact with the panels. Bundled panels should also be slanted so that any condensation may be drained off. All bundled panels should be thoroughly covered with a waterproof canvas tarp. Do not use non-breathing materials such as plastic because they prevent air passage and tend to trap moisture in the bundle. Roof and side panels should be erected as soon as possible after their arrival at the job site. If prolonged job site storage will be required, the builder is advised to seek storage of the panels out of the elements.

>>> Proper Erection Practices:

Dean requires that erection of its products be done by experienced pre-engineered metal building assemblers. Erecting and construction methods should be performed as outlined in the "American Institute of Steel Construction Code of Standard Practices for Steel Buildings and Bridges, Part 5." In addition, the erection policies and practices of both the SBA Independent Erectors Division, and MBMA erection practices must be understood and adhered to. The quality of erection has a direct bearing on the quality of the end product. If there are any questions as to these drawings on the Dean Steel Building system, please do not hesitate to contact our Customer Service Department (239) 334-1051.

Temporary supports or bracing required for the building erection is the responsibility of the erector to determine, furnish and install.

>>> Back Charge Claim Procedure:

Dean Steel Buildings, Inc., follows the back charge claim procedure adopted by MBMA and as outlined in the MBMA Low Rise Building Systems Manual, Common Industry Practices Section 6.10. The Customer Service Manager must be notified at once when a condition becomes apparent, which may result in back charge by the builder or erector. Notification by phone must be confirmed in writing. Some approximation of the amount of the back charge must be established at this time, and a written authorization from the Customer Service Manager must be secured before the work is started.

Dean will not honor any field corrections or back charges unless prior notice has been given and agreed upon. All discrepancies must be agreed upon, in writing, and accompanied with a Dean purchase order number before Dean will honor any back charges. Dean will then pay this agreed amount upon presentation of a final claim. Payment will be by credit memo to the Builder's account.

Any work which is undertaken without such notification and authorization, for which the builder expects to back charge Dean, will not be honored as a back charge.

Should a discrepancy exist, Dean may elect to do one of the following:

- Ship material from its plant for field correction (freight allowed)
- Purchase material locally (or allow builder to do so) for field correction
- Modify existing materials previously shipped to conform to requirements
- Return material to Dean's plant for exchange or modification requirements

When delivery is contracted by Dean, it is our carrier's intent to arrive on the job site at a pre-designated time and every effort will be made to do so. However, Dean will not accept any back charges due to late arrivals.

Dean Steel Buildings, Inc., shall not, however, be liable to builder in any way or for any reason on account of any change in Dean Steel Buildings, Inc., product lines. Dean will not pay any back charges for delays that may be incurred due to shortages. Dean will not pay any claims on improper unloading of material, or delays or damages caused by improper erection techniques. Dean Steel Buildings, Inc., may make changes from time to time in their product lines by discontinuing, altering, or modifying any or all of the products included therein and by adding new and additional products thereto.



DIETTE ELAINE

2023

STATE OF FLORIDA

PROFESSIONAL ENGINEER

0

FOR CONSTRUCTION

13/13

RMQ

ISSUE

REVISION

DATE

BY

THIS SET OF DRAWINGS IS FOR

☐ PERMIT

☒ CONSTRUCTION

☐

DEAN STEEL BUILDINGS, INC.

2929 INDUSTRIAL AVE. FORT MYERS, FLORIDA 33901

JOB NUMBER

TV01345

DRAWN

DATE

MAZ 9/6/13

CHECKED

DATE

MAZ 9/10/13

SHEET NUMBER

1 OF 4

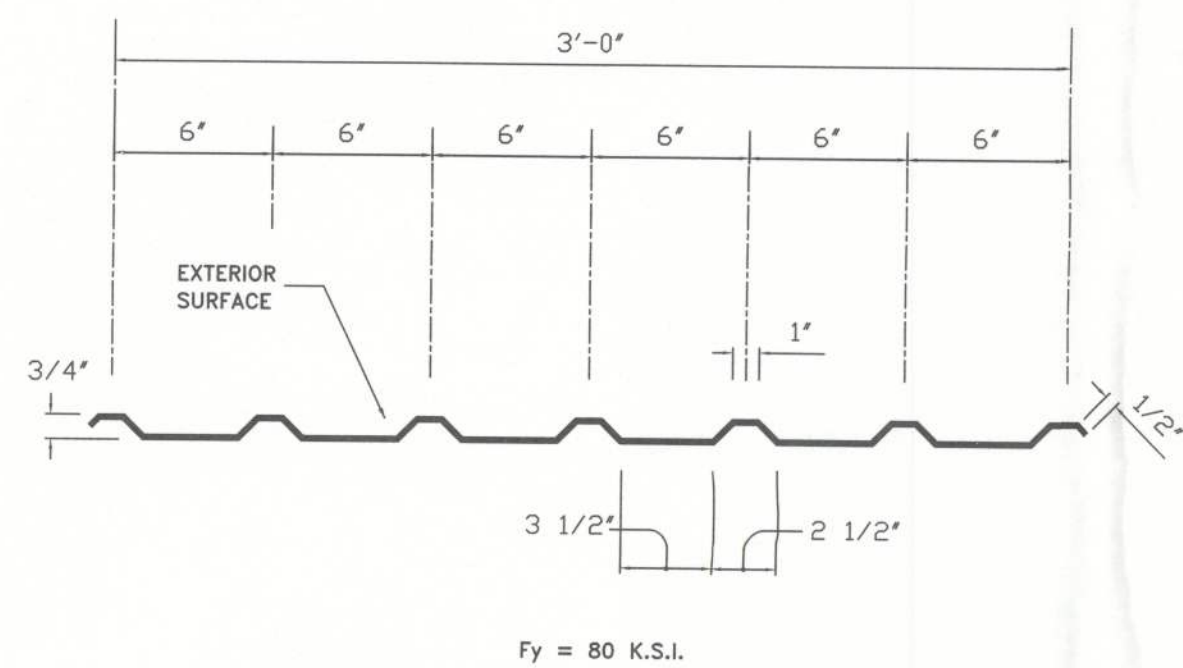
CUSTOMER : S.M. HOLWAY CONSTRUCTION INC.

PROJECT NAME : THOMPSON BUILDING

DESCRIPTION :

JOB INFORMATION

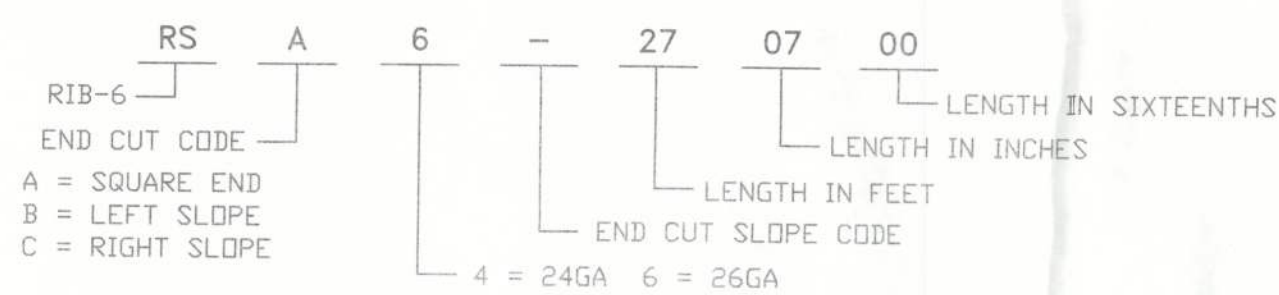
RIB - 6 PANELS



F_y = 80 K.S.I.

RIB-6 PANEL SECTION PROPERTIES					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0182	0.0331	0.0127	0.0314
24 GA	0.0258	0.0258	0.0486	0.0179	0.0464

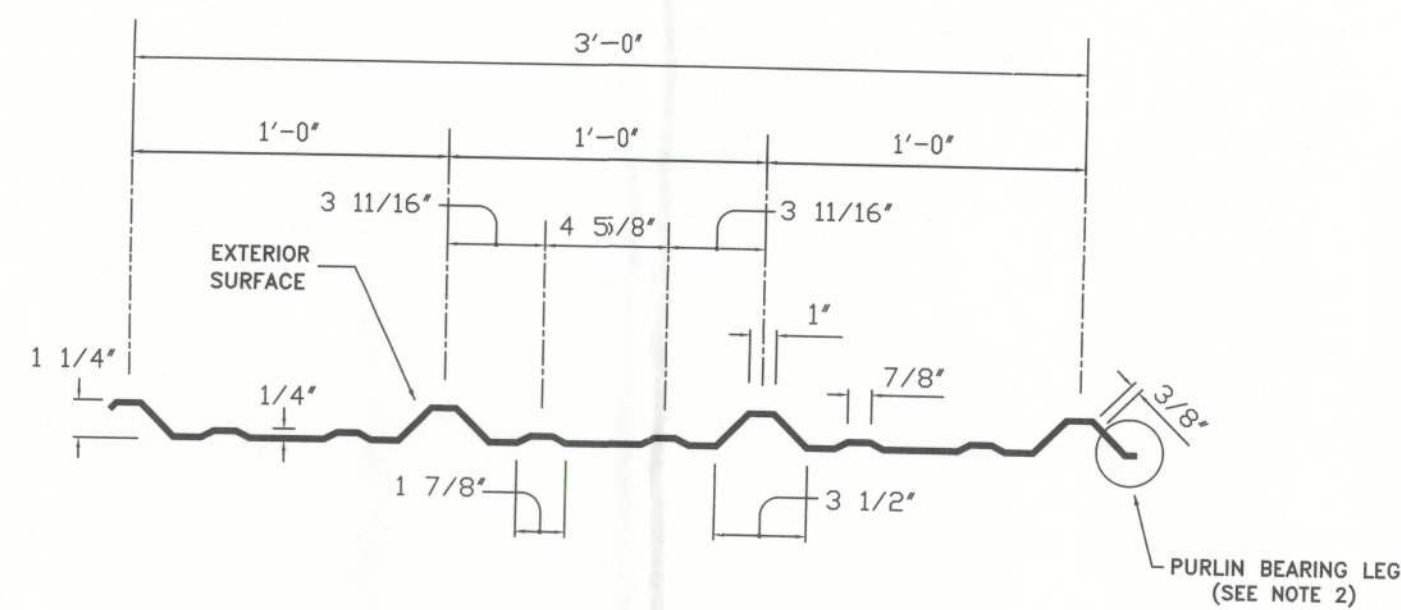
THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTED BY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMBER.



NOTE

- 1) SECTIONS PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
- 2) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, 690 1.25 OZ./PER SF. MEETING ASTM A525 OR GALVALUME™ A255, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.

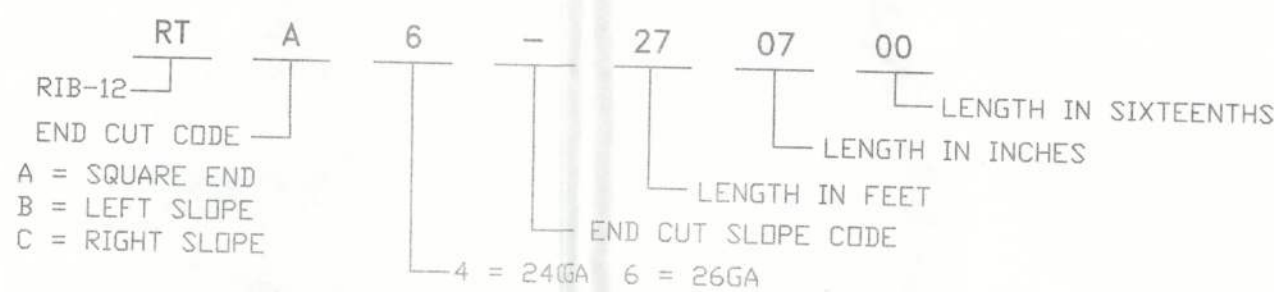
RIB - 12 PANELS / SHADOW PANELS



PANEL SECTION PROPERTIES 80,000 PSI					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0294	0.0283	0.0371	0.0500
24 GA	0.0258	0.0465	0.0481	0.0520	0.0671

PANEL SECTION PROPERTIES 50,000 PSI					
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
24 GA	0.0258	0.0547	0.0589	0.0542	0.0677

THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTED BY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMBER.



NOTE

- 1) SECTIONS PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
- 2) PANELS WITH PURLIN BEARING LEGS ARE AVAILABLE IN RIB-12 GALVALUME ONLY
- 3) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, 690 1.25 OZ./PER SF. MEETING ASTM A525 OR GALVALUME™ A255, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.

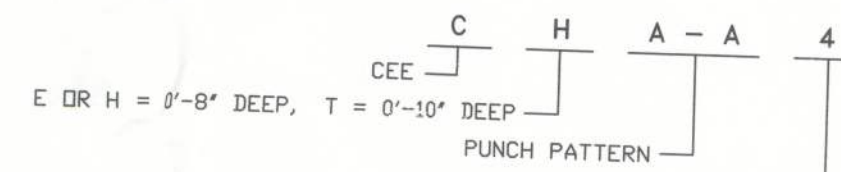
CEE SECTION PROPERTIES

D	*B*	MATERIAL	*f*	*d*	IX in. ⁴	SX
0'-8"	2 1/2"	14 GA	0.0747	3/4"	9.716	2.1
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.311	3.3
0'-10"	3 1/2"	14 GA	0.0747	3/4"	20.863	4.1
0'-10"	4"	12 GA	0.1046	3/4"	29.949	5.8

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER.

NOTE

- 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
- 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)



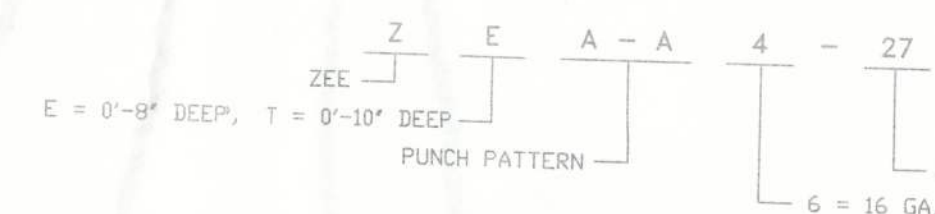
ZEE SECTION PROPERTIES

D	*B*	MATERIAL	*f*	*d*	IX in. ⁴	SX
0'-8"	2 1/2"	16 GA	0.0598	3/4"	8.120	2.1
0'-8"	2 1/2"	15 GA	0.0673	3/4"	9.099	2.1
0'-8"	2 1/2"	14 GA	0.0747	3/4"	10.058	2.1
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.847	3.3
0'-10"	3 1/2"	14 GA	0.0747	3/4"	21.960	4.1
0'-10"	3 1/2"	12 GA	0.1046	3/4"	30.520	6.1

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER.

NOTE

- 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
- 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)



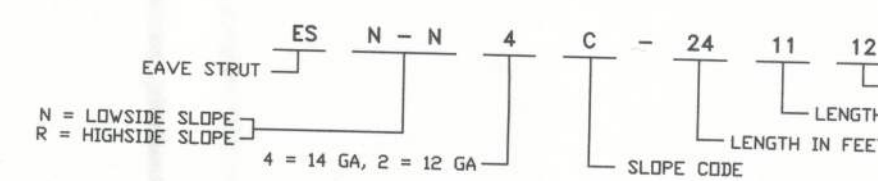
EAVE STRUT SECTION PROPERTIES (POSITIVE BENDING)

D	*B*	MATERIAL	*f*	*d*	IX in. ⁴	SX in. ³	IY in.
0'-8"	0'-5"	14 GA	0.0747	3/4"	15.581	3.895	6.624
0'-8"	0'-5"	12 GA	0.1046	3/4"	21.335	4.979	9.341

THE SLOPE AND MATERIAL THICKNESS OF ALL EAVE STRUTS ARE DENOTED BY THE THIRD, FIFTH, AND SIXTH CHARACTER OF THE PART NUMBER.

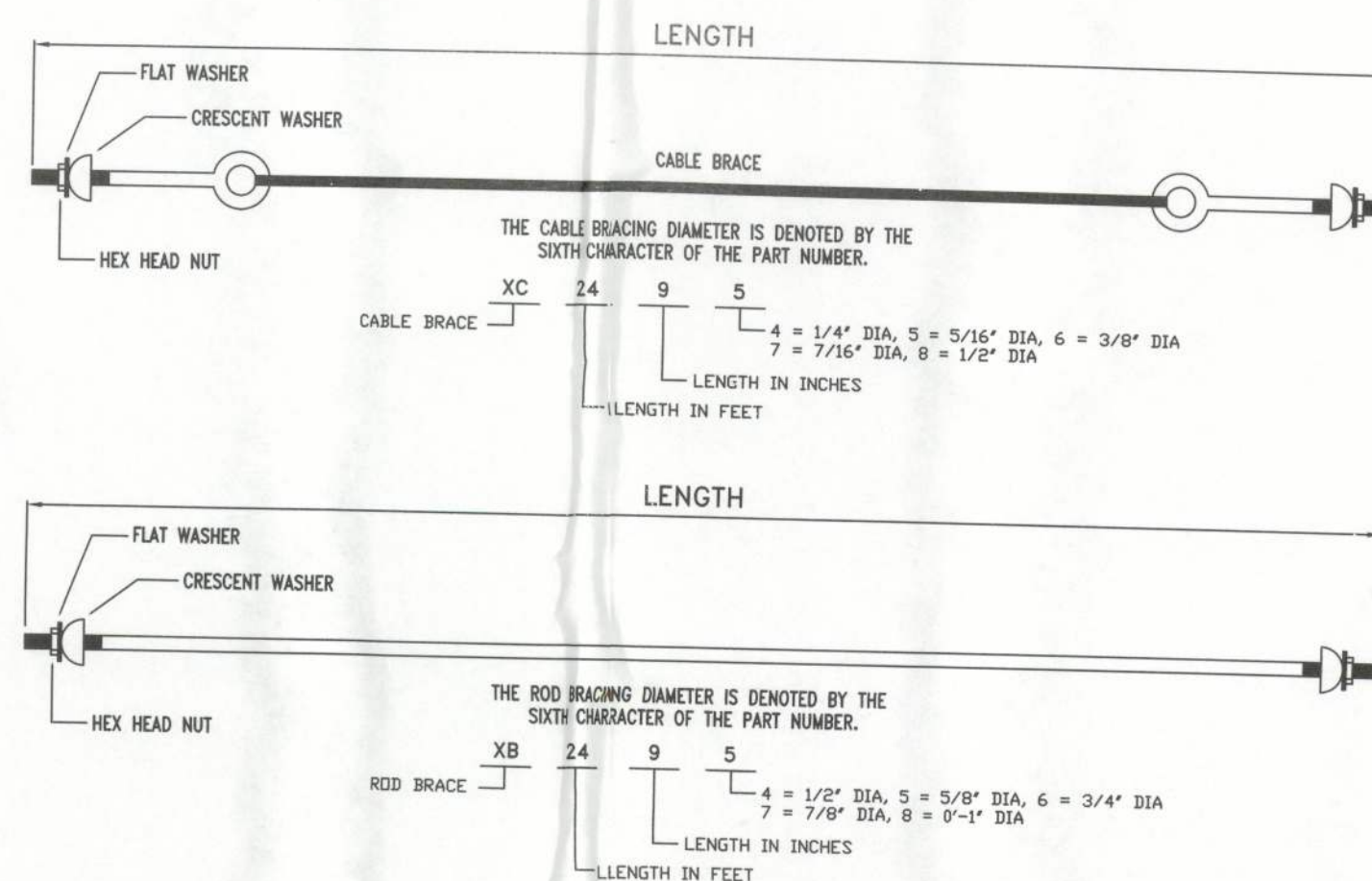
NOTE

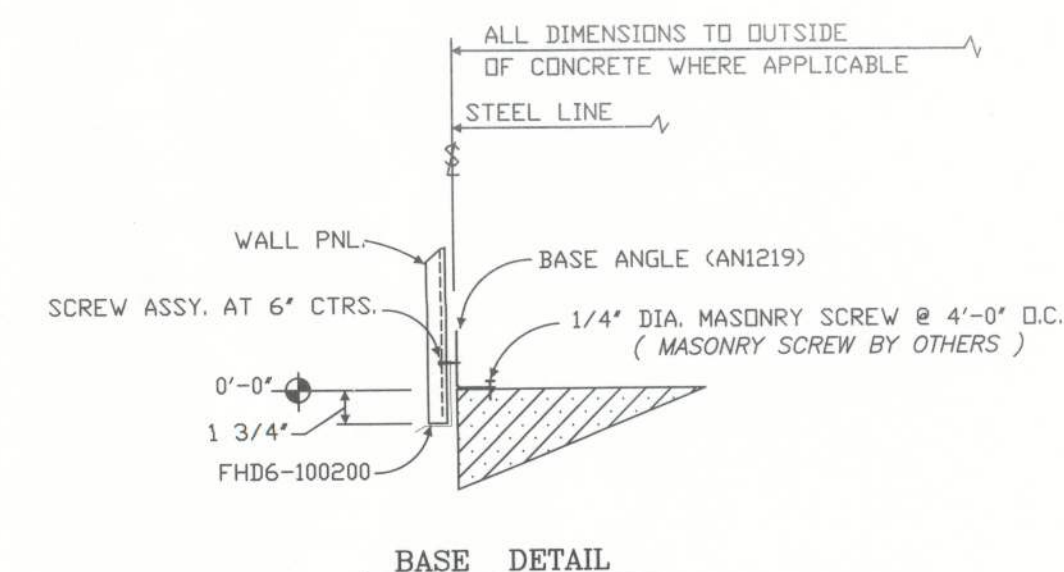
- 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE 1986 AISI COLD-FORMED STEEL DESIGN MANUAL INCLUDING 1989 ADDENDUMS.
- 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)



SLOPE CODE SCHEDULE

A = 0.012	H = 3.512	Q = 7.012
B = 0.512	J = 4.012	R = 7.512
C = 1.012	K = 4.512	S = 8.012
D = 1.512	L = 5.012	T = 8.512
E = 2.012	M = 5.512	U = 9.012
F = 2.512	N = 6.012	V = 9.512
G = 3.012	P = 6.512	W = 10.012

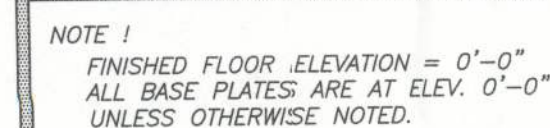





	QTY.	PART #	BOLT SIZE	"A"	"B"	"C"	"D"	PROJ.
	8	AB-1	1/2" X 0'-6"	1/2"	2"	0'-6"	1 1/4"	1 1/2"
	16	AB-2	3/4" X 1'-3"	3/4"	3"	1'-3"	3"	2"
		AB-3	7/8" X 1'-6"	7/8"	4"	1'-3"	3 1/2"	2"
		AB-4	1" X 1'-3"	1"	4"	1'-6"	4"	3"
		AB-5	1" X 2'-2"	1"	4"	2'-2"	4"	3"
		AB-6	1 1/4" X 2'-0"	1 1/4"	4"	2'-0"	3 3/4"	3"

DEAN STEEL BUILDINGS, INC. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIALS, AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBODIMENT. ADEQUATE PROVISIONS SHALL BE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING AND ANY OTHER IMPOSED LOADS, ACCOUNTING FOR THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

THE DISTANCE OF THE ANCHOR BOLTS FROM THE EDGE OF THE SLAB IS BASED ON DSB STANDARDS. THE ACTUAL SLAB SIZE MAY BE MODIFIED TO ACCOMMODATE ACTUAL SITE CONDITIONS AND FOUNDATION DESIGN.



 <p>2929 INDUSTRIAL AVE. FORT MYERS, FLORIDA 33901</p>	JOB NUMBER	TV01345
	DRAWN	DATE
	MAZ	9/6/13
	CHECKED	DATE
	MAZ	9/6/13
SHEET NUMBER		3 OF

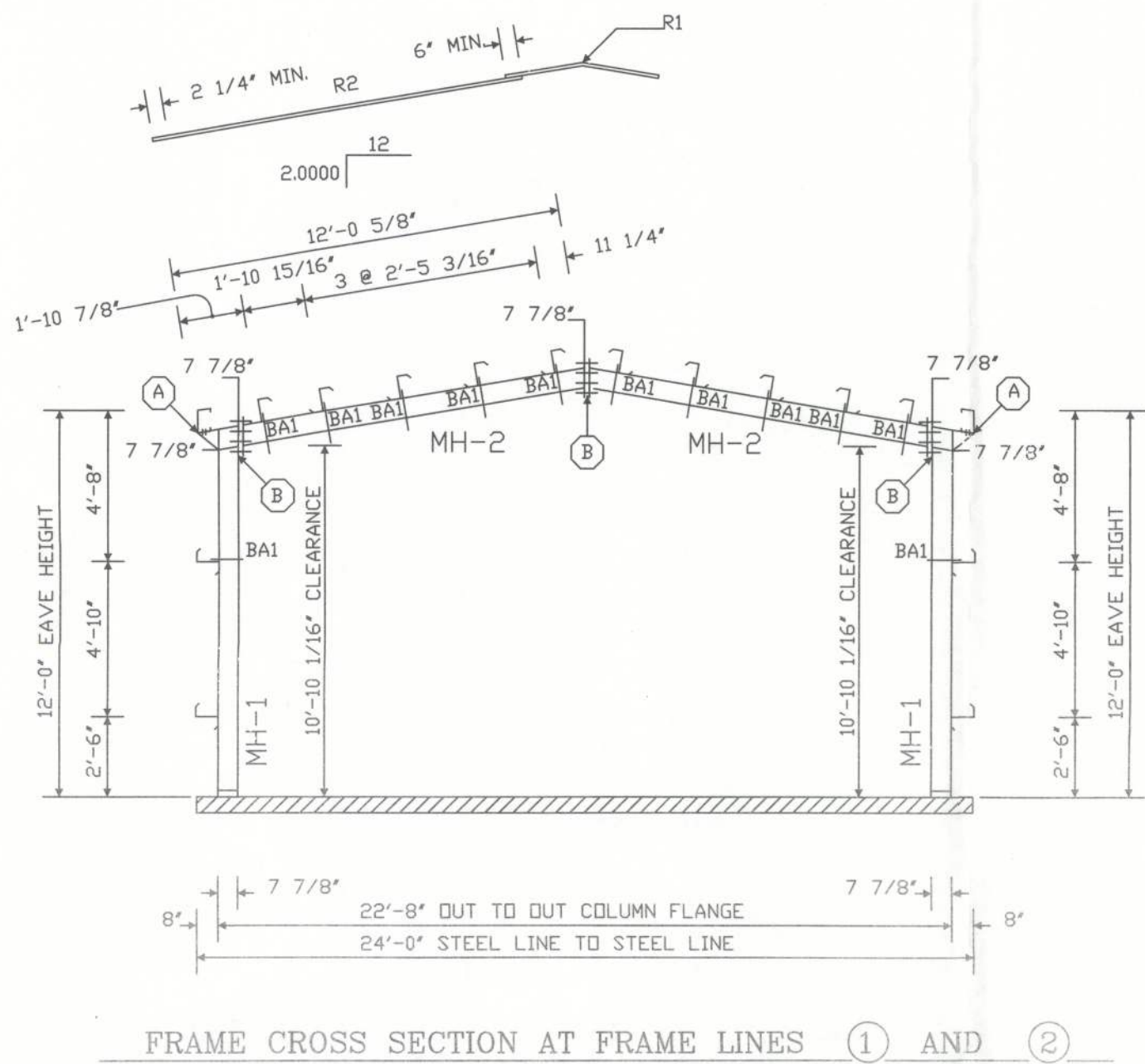
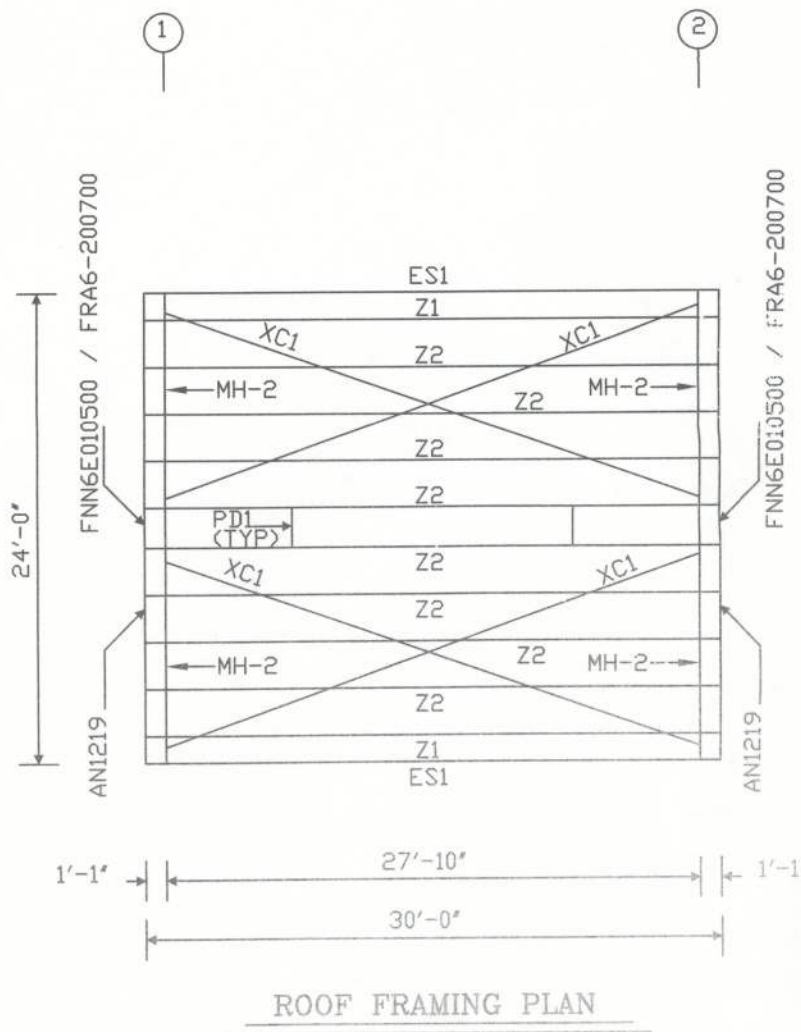
0	FOR CONST.	9/6/13	RMQ
ISSUE	REVISION	DATE	BY

CUSTOMER :	S.M. HOLWAY CONSTRUCTION INC
PROJECT NAME :	THOMPSON BUILDING
DESCRIPTION :	ANCHOR BOLT PLAN BASE PLATE DETAILS FRAME REACTIONS

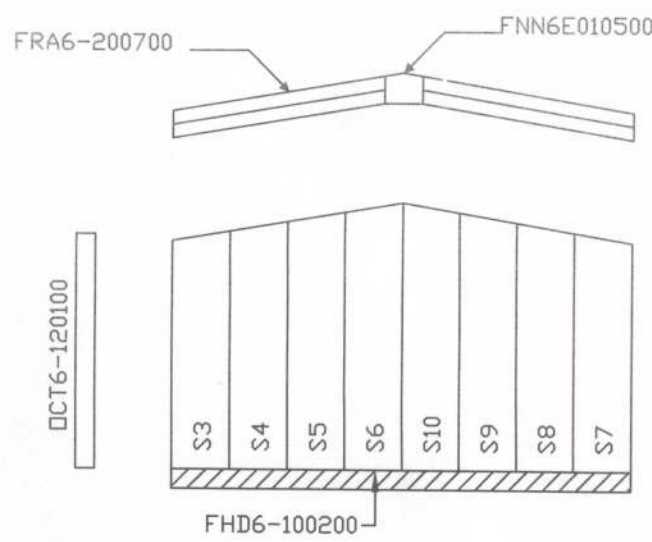
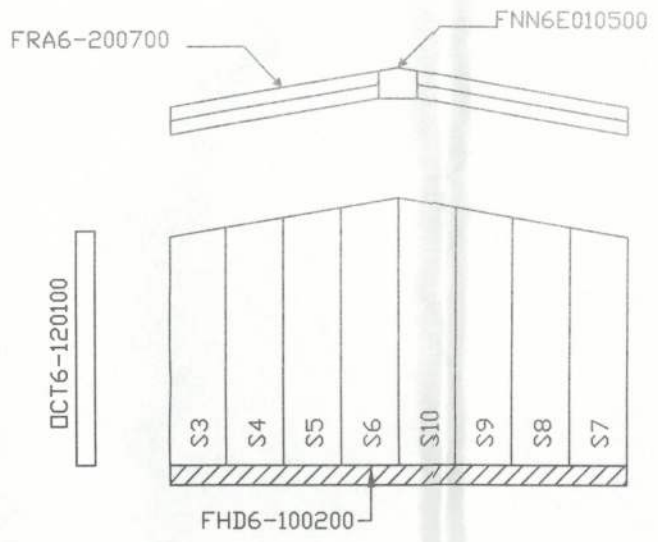
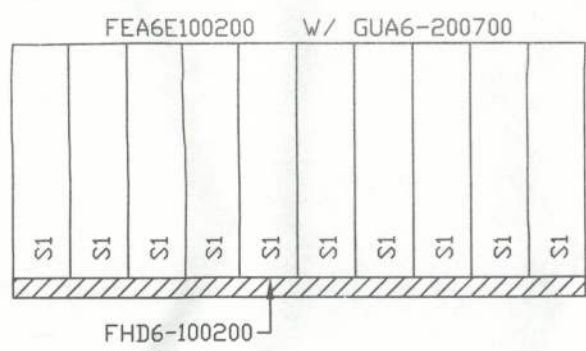
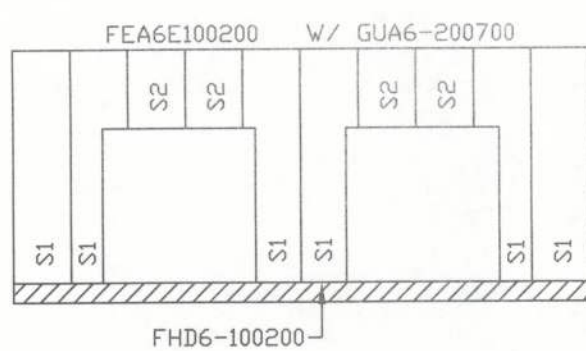
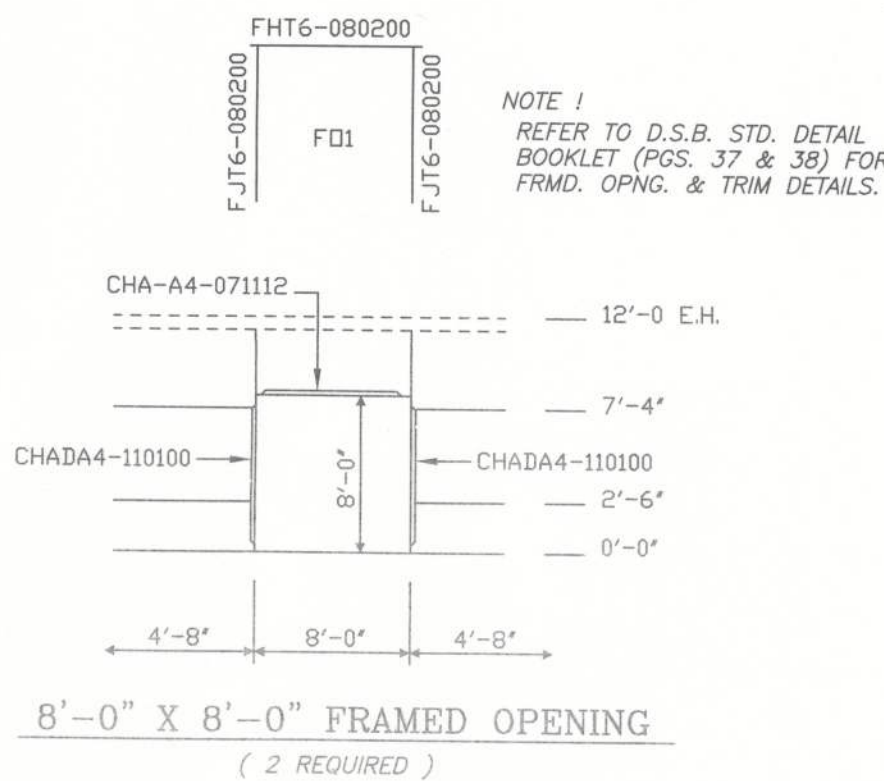
FIELD LOCATED ACCESSORY SCHEDULE		
QTY.	MK.	DESCRIPTION
1	A	3070 CEDD FOAM INSULATED DOOR LEAF W/FRAME & LEVER LOCKSET (CYLINDRICAL OPERATOR) (+50.00 / -50.00 PSF)

NOTE 1
REFER TO D.S.B. STD. DETAIL
BOOKLET (PG. 36) FOR
WALK DOOR INSTALLATION DETAILS.

NOTE:
RAKE ANGLE (AN1219) IS ATTACHED
TO THE BOTTOM OF THE PURLIN ZEES.
REFER TO D.S.B. STANDARD DETAILS
BOOKLET (PGS. 4, 8 & 20) FOR
MORE DETAILS.



- NOTE:
1. THE HIGH STRENGTH BOLTS & WASHERS SPECIFIED FOR USE AT THE COLUMN & RAFTER CONNECTIONS CONFORM TO A.S.T.M. SPECIFICATION A-325. THESE BOLTS MUST BE TIGHTENED BY THE "TURN OF THE NUT" METHOD AS DESCRIBED IN SECTION 5(C) OF THE A.I.S.C. SPECIFICATION STRUCTURAL JOINTS USING A.S.T.M. A-325 OR A-490 BOLTS.
 2. ALL BOLTS TO BE HIGH STRENGTH WITH NUTS AND WASHERS. EXCEPT CONNECTION "A" REQUIRES A WASHER ON THE EAVE STRUT SIDE ONLY.
 3. ALL DEPTHS SHOWN ARE WEB DEPTHS ONLY AND DO NOT INCLUDE FLANGE THICKNESSES.
 4. FLANGE BRACES ARE 1 3/4" X 1 3/4" X 14 GA. ANGLE ALL FLG. BRACES ARE LOCATED ON BOTH SIDES OF THE FRAME UNLESS NOTED OTHERWISE. PEAK PURLIN IS FLANGE BRACED ON ONE SIDE ONLY. FRAMES AT BLDG. ENDS REQUIRE FLG. BRACE ON ONE SIDE ONLY.
 5. RIGID FRAME RAFTER PART NUMBERS ARE LOCATED ON THE TOP FLANGE OF THE UPHILL END OF THE RAFTER.
 6. RIGID FRAME COLUMN PART NUMBERS ARE LOCATED ON THE OUTER FLANGE AT THE TOP OF THE COLUMN.
 7. DIMENSIONS SHOWN ON CROSS-SECTION ARE MEASURED AT THE COLUMN BASES ONLY. THE WIDTH MEASURED AT THE EAVE CAN BE WIDER DEPENDING ON COLUMN OUTER FLANGE THICKNESS CHANGES.
 8. RIGID FRAME IS SYMMETRICAL ABOUT CTR. LINE.



- NOTE 1
DIMENSIONS ON THIS SHEET ARE STEEL DIMENSIONS AND DO NOT INCLUDE SHEET LEDGES, ETC. FOR CONCRETE DIMENSIONS SEE THE ANCHOR BOLT PLAN.
- NOTE 2
SHEETING MUST BE ATTACHED TO ALL SECONDARY FRAMING PER DEAN STANDARD DETAILS. THIS INCLUDES ALL DOUBLE PURLINS AND GIRTS REGARDLESS OF LOCATION.
- NOTE 3
THE TYPE AND MATL. THICKNESS OF ALL FLASHING IS DENOTED BY THE 1ST, 2ND, 3RD, AND 4TH CHARACTERS OF PART NUMBER. (EXAMPLE: FJT6-140100)
- FJT = JAMB FLASHING FOR RIB-12, FRA = RAKE FLASHING (STD.), GUA = SCULPTURED GUTTER, FVR = WALL TO ROOF FLASHING, OCT = OUTSIDE CORNER FLASHING FOR RIB-12, ETC.

BOLT SCHEDULE		
CONN.	QTY.	SIZE
A	4	1/2" X 1 1/4" HW5532
B	8	3/4" X 2" HW5537

NOTE: ALL BOLTS TO BE HIGH STRENGTH WITH NUT AND WASHER. CONNECTION "A" REQUIRES WASHER ON EAVE STRUT SIDE ONLY.

MALETTE ELAINE LICENSED PROFESSIONAL ENGINEER FLORIDA 00000000			
0	FOR CONST.	09/13/13	RMQ
ISSUE	REVISION	DATE	BY

DEAN STEEL BUILDINGS, INC. 2969 INDUSTRIAL AVE. FORT MYERS, FLORIDA 33901	JOB NUMBER TV01345	
	DRAWN MAZ	DATE 9/6/13
	CHECKED MAZ	DATE 9/10/13
	SHEET NUMBER 4 OF 4	

CUSTOMER: S.M. HOLWAY CONSTRUCTION INC.	
PROJECT NAME: THOMPSON BUILDING	DESCRIPTION: ROOF FRAMING PLAN FRAME CROSS SECTION ENDWALL & SIDEWALL ELEVATIONS