

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

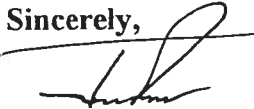
**From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068**

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

This design is for an individual building component and has been based on information provided by the client. The engineer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/APT 1-1995 and RDMS-971 to be incorporated as part of a building design by a Building Designer (registered architect or professional engineer). When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records and wind or snow loads, project specifications or special requirements. Unless otherwise stated, truss has not been designed for storage or occupancy loads. The design assumes unbraced chord ends (top or bottom) are fixed. Where compression chords (top or bottom) are pinned, where a maximum top chord height is not fully braced laterally by a minimum vertical post, rafter or ceiling, they should be braced at a maximum spacing of 10'-0". Connector plates shall be manufactured from 6061-T6 aluminum alloy. Truss shall be manufactured from 20 gauge hot dipped galvanized steel per ASTM A 653. Grade 40, unless otherwise shown.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabricating. Plates shall not be installed over boobies, boots or distorted framing. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both sides of the gusset with fully fluted imbedded shall be 5/16" wide at the joint unless otherwise shown. A 5/16" plate is 1/2" wide at 1" long. A 3/8" plate is 3/4" wide at 1" long. Double (nails) on parallel to the plate length specified. Slight cholla on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This unit is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TFPI 1-1993

All bracing and erection recommendations are to be followed in accordance with "Handling, Lifting and Bracing," HB-31. Trusses are to be handled with particular care during bending and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Qty: 1

SPANS UP TO 60'-0"

Cont. Support Studs @ 6'-0"-0 O.C.

Profile Path: C:\TEE-LOK\Work\Jobs\MARONDA SYSTEMS\VT.prx

TC Live	16.0	psf	Lbr DF: 1.25
TC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2- 0- 0
BC Dead	2.0	psf	Trl-02/FBC-04
TOTAL	35.0	psf	Code: FLA
			v4.7.2-4355

Job:

Customer:

WO: HIPDETAIL

TI: HIP

Qty: 1

DESIGN INFORMATION

A design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or designs furnished to the truss designer by the client. The correctness or accuracy of this information as it applies to a specific project and accepts no responsibility or creates no control with regard to erection, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and AS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and loads, project specific data not be designed for or occupancy loads. The design assumes proper bracing or chords (top or bottom) are continuously applied to the truss. The design assumes that the truss is not fully braced laterally by a system applied rigid ceiling, they should be braced at a minimum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

ABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the following notes and to realize a continuing responsibility for accurate fabrication. Any discrepancies are to be written before cutting or welding. Plates are to be cut to the correct length and beveled or chamfered. Members shall be cut for light fitting wood to wood joints. Connector plates shall be located on both faces of joints unless otherwise shown. A 3/4" plate is 5" wide x 10' long. A 6" plate is 6" wide x 8" long. Slots (holes) in parallel to the plate length specified. Double cuts on members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1993

RECAUTIONARY NOTES

If bracing and erection recommendations are to be followed in accordance with accepted industry practices, trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and section bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during erection. The supervision of erection of trusses shall be the responsibility of the person experienced in the erection of trusses. Professional advice shall be sought if design loads shall not be applied to trusses at any time to loads other than the weight of the trusses shall be applied to trusses until all fastening and bracing is completed.

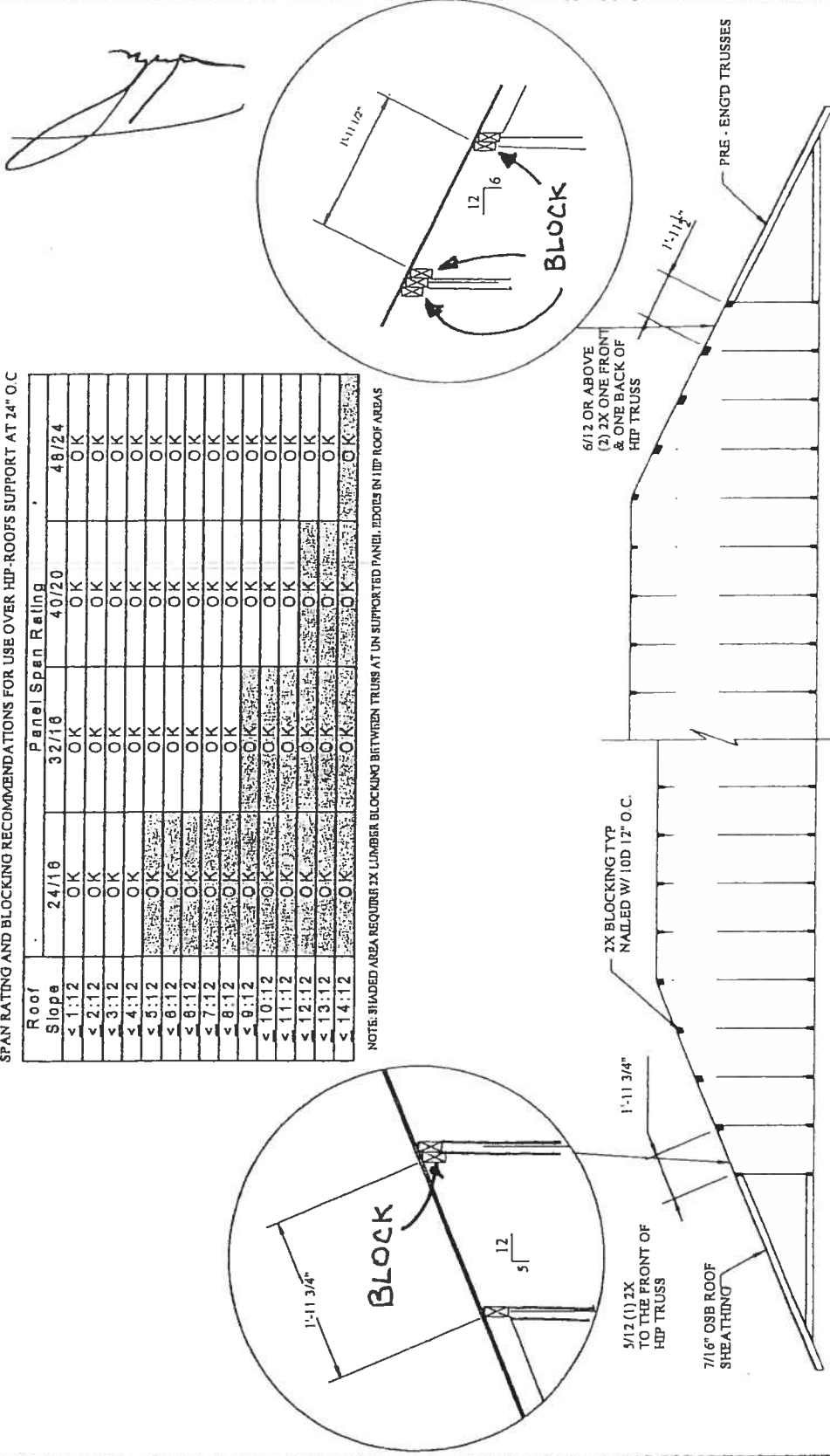
HIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083

SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C.

Roof Slope	24/16	32/16	40/20	48/24
≤ 1:12	OK	OK	OK	OK
≤ 2:12	OK	OK	OK	OK
≤ 3:12	OK	OK	OK	OK
≤ 4:12	OK	OK	OK	OK
≤ 5:12	OK	OK	OK	OK
≤ 6:12	OK	OK	OK	OK
≤ 7:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 9:12	OK	OK	OK	OK
≤ 10:12	OK	OK	OK	OK
≤ 11:12	OK	OK	OK	OK
≤ 12:12	OK	OK	OK	OK
≤ 13:12	OK	OK	OK	OK
≤ 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRES 2X LUMBER BLOCKING BETWEEN TRUSSES AT UN-SUPPORTED PANEL EDGES IN HIP ROOF AREAS



Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\Job-1\COMM-1.prx

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of trusses members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 or TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 353 Orono Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenton, NC 27932

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: HIPDETAIL

TI: HIP

11/2/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-0

is design for the individual building component and the design of the building as a whole. The design team must provide the information needed by the client. The design team must assume responsibility for damages as a result of faulty or incorrect information, specifications or design. *For* design furnished to the truss designer by the client for the construction of trusses, this information as it relates to a specific project and controls no responsibility or overrules no concept with regard to erection, handling, shipment and installation of trusses. If a truss has been designed as an individual building component in accordance with ANSI/TPI-1:1993 and ANSI/TPI-2:1993 and is incorporated as part of the building design, the building designer, the design engineer and the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for the local building, project specifications or special and or snow loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where a truss is designed for tension, it must be braced laterally by a system applied rigid ceiling, they should be braced by a minimum spacing of 6' on center. Trusses are to be installed in accordance with ASTM A 653, Grade 40, hot dipped galvanized steel.

For information to the fabricator, call review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over kneeholes, knots or distorted members. Connector plates shall be cut for tight fitting wood to wood joints. Members shall be located on both faces of the joint unless rails fully imbedded and shall be 5/8" above x joint unless otherwise specified. A 5x4 plate shall be 5/8" above x joint unless rails fully imbedded and shall be 5/8" above x joint unless otherwise specified. A 6x4 plate shall be 6" wide x 8" long. Splice holes (nails) parallel to the plate length specified. Double ends on all members shall meet at the centroid of the web(s) unless otherwise shown. Connector plate sizes are minimum based on the forces shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated with fire retardant treated member unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1995.

ANSI/TPI-

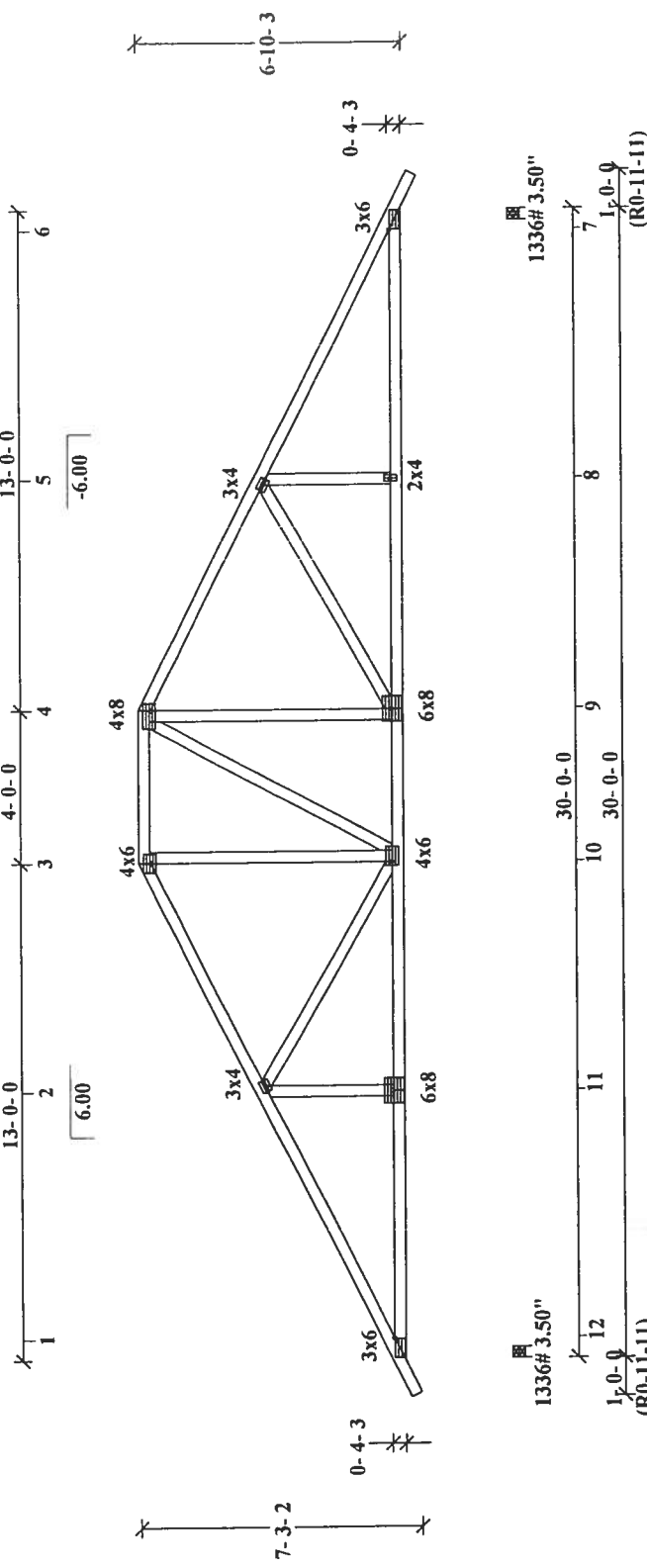
II bracing and erection recommendations are to be followed in accordance with accepted industry observations. Trusses are to be handled with particular care during handling, transport, and erection. They are to be handled with care to prevent damage during handling, transport, and erection. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during erection and installation between trusses to avoid toppling and buckling. The supervision of erection of the trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought and needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

-MAX. REACTIONS PER BEARING LOCATION-									
X-Loc	Bset	Var	Horiz	Uplift	Y	Type			
0-1-12	1	1336	0	-674	B	F			
23-10-4	1	1336	0	-674	B	H	R		
PROVIDE UPLIFT CONNECTION PER SCHEDULE									
-WEB...FORCE...CSI...WEB...FORCE...CSI...									
11-2	276	0.09	C	4	9	509	0		
2-10	-628	0.51	C	9	5	-626	0		
3-10	509	0.10	C	5	8	275	0		
10-4	5	0.00	C						

TT

	in./Ratio	in./Ratio	Jnt(s).
I.Span	-0.12/999	-0.24/999	5-4
Horiz.	0.05	0.10	NA
Max DL Deflection L/999 = -0.12			
Long term deflection factor = 1.50			

1.6	TC	FORCE, CSI	Cr	BC	FORCE, CSI
1	2	-2284	0.48	A 12-11	1988 0.61
2	3	-1691	0.46	A 11-10	1982 0.61
3	4	-1484	0.27	A 10-9	1442 0.40
4	5	-1686	0.46	A 9-8	1982 0.60
5	6	-2284	0.48	A 8-7	1988 0.61



EXCEPT AS SHOWN PLATES ARE MITEK MT20
(K0-11-11)

Scale = 0.1998

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
927 Modesto Bl. Ft. Lauderdale, FL 33766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not considered bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Tus Platte Institute, Inc. TPI is located in 383 O'Neale Drive, Madison, Wisconsin 53719).
C. A. RILEY, P.E., Registered Professional Engineer, License No. 10731

Eng Job:	WO: SUNILS
Dwg:	TI: H1
Dsgnr: TLY	6/6/2006
Chk:	Lbr DF: 1 1 paf
	Plt DF: 1 1 paf
	O.C.: 2 -
	TPI-02/FB/
	Code: FLA
	v4. 32-4
	43.0 paf
	43.0 paf

Job: SUNBURY L 3

Customer:

WO: SUNL3

TI: H2A

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. (This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and is not to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specific based on their designed loads. Unless shown, the design assumes no storage or occupancy (or both) are continuously applied to the truss.)

Where indicated by a truss designer, the truss shall be fully braced laterally by a system of rigid or flexible bracing. The truss shall be designed for a minimum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be per in writing before fabrication. Fabrication plates shall not be cut or modified without the truss designer's approval. Members shall be cut for tight fitting wood to wood joints. Connector plates shall be located on both faces of the web with nuts fully imbedded and shall be sym. about the web unless otherwise shown. A 5x4 plate is 5" wide x 1" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bonding. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight line position and for raising them from the ground shall be designed and installed by others. Careful lifting is essential and erection bracing is always required. Noisy precautionary action for trusses requires strict adherence to the following instructions. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADS -- This design is the composite result of multiple loads.

Windload per ASCE 7-02, CSc, V= 125mph, H= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
Bld Type= encl L= 40.0 ft W= 30.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-2295/1065, 2-3=-1863/943, 3-4=-1625/902, 4-5=-1625/898, 5-6=-1863/938, 6-7=-2295/1046
BOT CHORD 13-12=-864/2000, 12-11=-864/1996, 11-10=-677/1691, 10-9=-841/1996, 9-8=-841/2000, 8-7=-1065/1996, 7-6=-1065/1996, 6-5=-1065/1996, 5-4=-1065/1996, 4-3=-1065/1996, 3-2=-1065/1996, 2-1=-1065/1996
Webs 12-2=0/195, 2-11=-462/295, 3-11=-220/630, 11-4=-115/97, 4-10=-115/101, 10-5=-214/630, 5-6=-462/272, 6-9=-0/195, 9-10=-214/630, 10-11=-462/272, 11-12=-864/2000, 12-13=-864/2000

MAX. REACTIONS PER BEARING LOCATION

X-Loc Bset Vert Horiz Uplift Y Type
0- 1-12 1 1290 14 -617 B H Roll
29-10- 4 1 1336 0 -691 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
WEB... FORCE... CSI... Cr... WEB... FORCE... CSI... Cr...
12- 2 195 0.06 C 4-10 -115 0.09
2-11 -462 0.29 C 10- 5 630 0.20
3-11 630 0.20 C 10- 6 -462 0.29
11- 4 -115 0.09 C 6- 9 195 0.06

GLORIAL MAX DEFLECTIONS

LL TL
I. Span in./Ratio in./Ratio Jnt(s)
Horiz. -0.19/999 -0.38/932 10-11
0.05 0.10 NA
Max DL Deflection L/999 = -0.19
Long term deflection factor = 1.50

Joint Locations

1) 0- 0- 0 6) 23- 7- 4 11) 11- 0- 0
2) 6- 4-12 7) 30- 0- 0 12) 6- 4-12
3) 11- 0- 0 8) 30- 0- 0 13) 0- 0- 0
4) 15- 0- 0 9) 23- 7- 4
5) 19- 0- 0 10) 19- 0- 0

Diagram

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.2070

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\H2A.prx

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 581 D'Orofino Drive, Madison, Wisconsin 53719)
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Eng Job:

WO: SUNL3

Dwg:

TI: H2A

Dsgnr: TLY

Chk:

6/6/2006

TC Live

16.0 psf

TC Dead

7.0 psf

BC Live

10.0 psf

BC Dead

10.0 psf

TOTAL

43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

V4.7.32-46657

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: H3A

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or liability for any errors or omissions in the design, fabrication, shipping, and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with the rearward trued lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADS -- This design is the composite result of multiple loads.

WindLoad per ASCE 7-02, Exp. Cat. B, V=125mph, W=21.0 ft, I=1.00, Exp. Cat. B, Kzt=1.0
Bld Type= encl I= 40.0 ft W= 30.0 ft Truss
in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-2288/1130, 2-3=-2065/1000,
3-4=-2128/1092, 4-5=-2128/1092, 5-6=-2065/989,
6-7=-2288/1105,
BOT CHORD 12-11=-942/2016, 11-10=-713/1799,
10-9=-704/1799, 9-8=-912/2016,
C Webs 2-11=-243/262, 11-3=-105/439,
C3-10=-179/409, 10-4=-291/235, 10-5=-186/409,
5-9=-91/439, 9-6=-243/238.

MAX. REACTIONS PER BEARING LOCATION

X-Loc Bset Vert Horiz Uplift Y Type
09-1-12 1 1290 14 -627 B Pin
20-10-4 1 1336 0 -701 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
WEB...FORCE...CSI...Cr...BC...FORCE...CSI...
2-11 262 0.12 C 10-5 409 0.17 C10-9=-704/1799, 9-8=-912/2016,
11-3 439 0.14 C 5-9 439 0.14 C Webs 2-11=-243/262, 11-3=-105/439,
3-10 409 0.16 C 9-6 -243 0.12 C3-10=-179/409, 10-4=-291/235, 10-5=-186/409,
10-4 -291 0.12 C 5-9=-91/439, 9-6=-243/238.

GLOBAL MAX DEFLECTIONS

LL TL
in./Ratio in./Ratio Jnt(s)
I Span -0.18/999 -0.36/993 8-9
Boxiz. 0.05 0.10 NA
Max DL Deflection L/999 = -0.18
Long term deflection factor = 1.50

Joint Locations

1) 0-0-0 5) 21-0-0 9) 21-0-0
2) 4-9-4 6) 25-2-12 10) 15-0-0
3) 9-0-0 7) 30-0-0 11) 9-0-0
4) 15-0-0 8) 30-0-0 12) 0-0-0
In-Plant Quality Assurance with Cq=1
In-Plant Quality Assurance, per
sec. 3.2.4 of TPI-1-02, for joint(s):
1,7
...TC...FORCE...CSI...Cr...BC...FORCE...CSI...
1-2 -2288 0.33 A 12-11 2016 0.73
2-3 -2065 0.31 A 11-10 1799 0.70
3-4 -2128 0.37 A 10-9 1799 0.70
4-5 -2128 0.37 A 9-8 2016 0.73
5-6 -2065 0.30 A
6-7 -2288 0.31 A

Diagram

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.2118

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\H3A.prx

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to ready for the building designer. Additional bracing may be required. (See UHS-01 of TPI) For specific joint bracing, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Eng Job:

WO: SUNL3

Dwg:

TI: H3A

Dsgnr: TLY

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FIA

v4.7.32-46654

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to transportation, handling, shipment and installation of trusses. This truss has been designed as an individual building component to be incorporated as part of the building design. It is not to be incorporated as part of the building design without the approval of the designer. When used in the building design, the design loadings shown must be checked to be sure that the data shown are in agreement with the snow loading conditions, local climatic records and/or snow loads, project specifications or special stipulated loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly stipulated rigid ceiling, they should be braced in the maximum spacing. 20 gauge box clipped, galvanized steel trusses are available in 20 gauge box clipped, galvanized steel. See ASTM A 653. 30 gauge box clipped, galvanized steel trusses are available in 30 gauge box clipped, galvanized steel. See ASTM A 653. 40 gauge box clipped, galvanized steel trusses are available in 40 gauge box clipped, galvanized steel. See ASTM A 653.

FABRICATION NOTES

prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies will be put in writing before cutting or fabrication. Plates shall be cut to the correct size, be free of cracks, knots or distortions. The steel shall be cut for light fitting forces to avoid bending. Members shall be cut to be located on both faces of the plate, unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double angles are members that meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for corrosion allowing for additional stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TP1

PRECAUTIONARY NOTES

all bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

EXCEPT AS SHOWN PLATES ARE MITek MT20

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
167 Medallion Pl. Chuluota, FL 32766

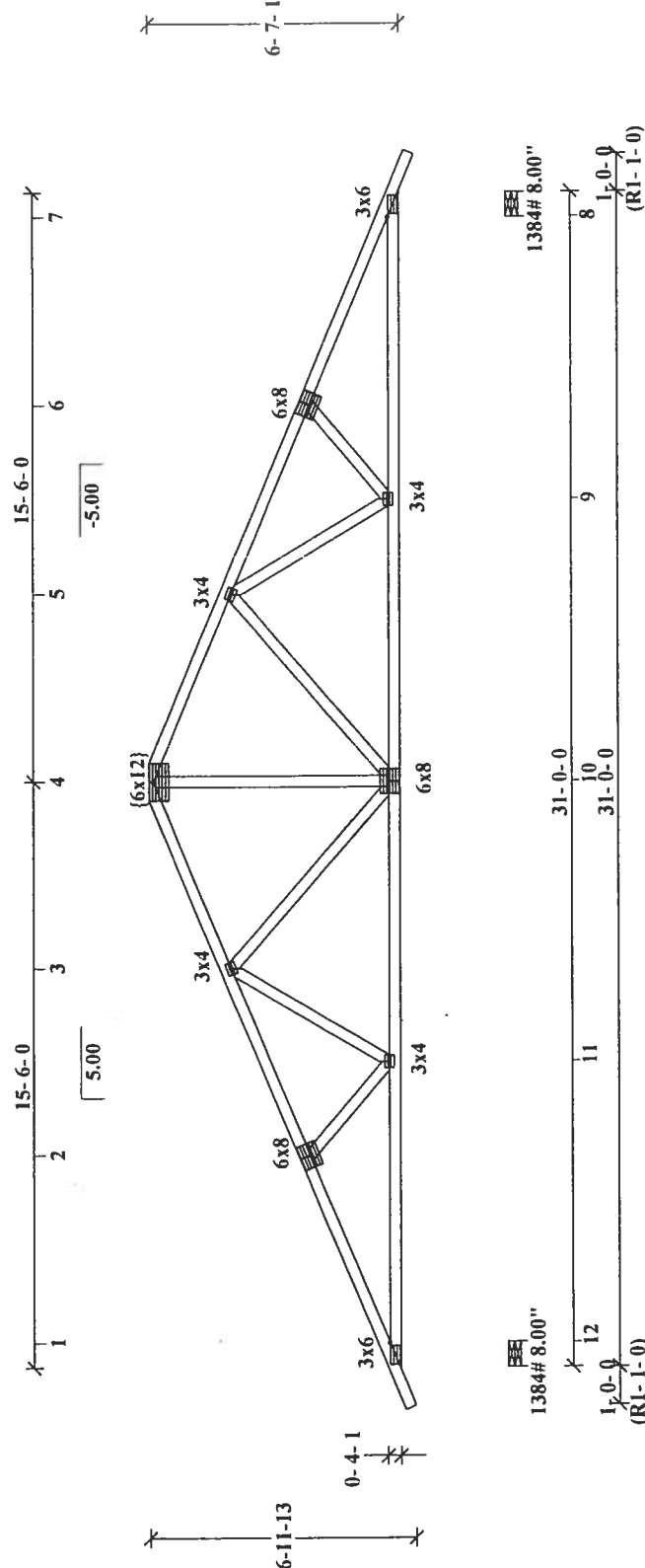
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI). For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 983 D Onofrio Drive, Madison, Wisconsin 53719).

D'Onofrio Drive, Madison, Wisconsin 53719)
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\H6.prx

Scale = 0.1980



Eng Job:	WO: SUNL3
Dwg:	TI: H6
Dsgnr:ECH	6/6/2006
Chk:	
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf
	Lbr DF: 1.25
	Plt DF: 1.25
	O.C.: 2 - 0 - 0
	TPI-02/FBC-04
	Code: FLA
	v4 7 32-46821

This design is for an individual building component and is not intended to be used for any other building. The user has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or design furnished to the truss designer by the client and/or designees. The user warrants that the information as it relates to a specific project and accepts no responsibility or exercises no control with regard to construction, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ASCE 7-99 and design loads are to be incorporated as part of the building design. Building Designing: When used for any other building, the design loadings shown must be used as a guideline for the design loadings shown must be used to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special stipulated loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by abutting units otherwise specified. Where a bottom chord in tension are not fully braced laterally by a system applied rigid ceiling, they should be braced by a maximum spacing of 10 ft. The design is based on the assumed ASTM A 36, Grade 50, unless otherwise specified. The design is based on the assumed ASTM A 36, Grade 50, unless otherwise specified.

When fabricating, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are not to be installed before cutting or fabricating. Plates will be put in varying order of knotholes, knots or distorted members. Members shall be cut for tight fitting wood to wood joining. Connector plates shall be tight on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3/4" plate is 5" wide x 10' long. A 6" plate is 16" wide x 8' long. Sizes (holes) on a parallel to the plate length specified. Double cut on members shall meet at the extremity of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry obligations. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of personnel experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P. E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

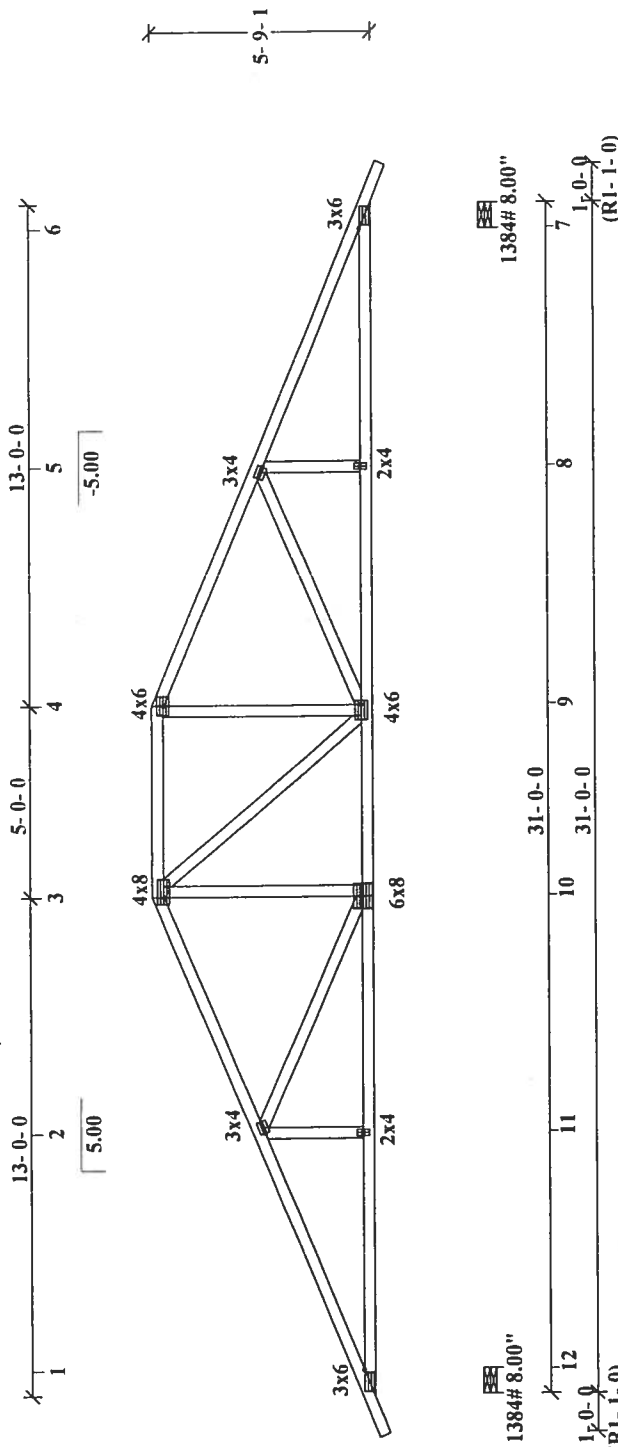
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is notension bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91) (TPI) for specific truss bracing requirements. Contact Building Designer (Tuscarora Plate Institute, TPI is located at 383 Commerce Building, Erie, PA 16501, 814/398-1371).

COMPANY Fabricator by: Tuscarora Engineering Corp., P.A., 818 Southside Rd., Edison, NJ 07032.

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\H7.prx

WO: SUNL3	WO: SUNL3	WO: SUNL3
TI: H7	TI: H7	TI: H7
6/6/2006	6/6/2006	6/6/2006
Lbr DF: 1	Lbr DF: 1	Lbr DF: 1
Plt DF: 1	Plt DF: 1	Plt DF: 1
O.C.: 2-	O.C.: 2-	O.C.: 2-
TPI-02/FB	TPI-02/FB	TPI-02/FB
Code: FLA	Code: FLA	Code: FLA
V4: 32-4	V4: 32-4	V4: 32-4
43.0 psf	43.0 psf	43.0 psf
TOTAL	TOTAL	TOTAL

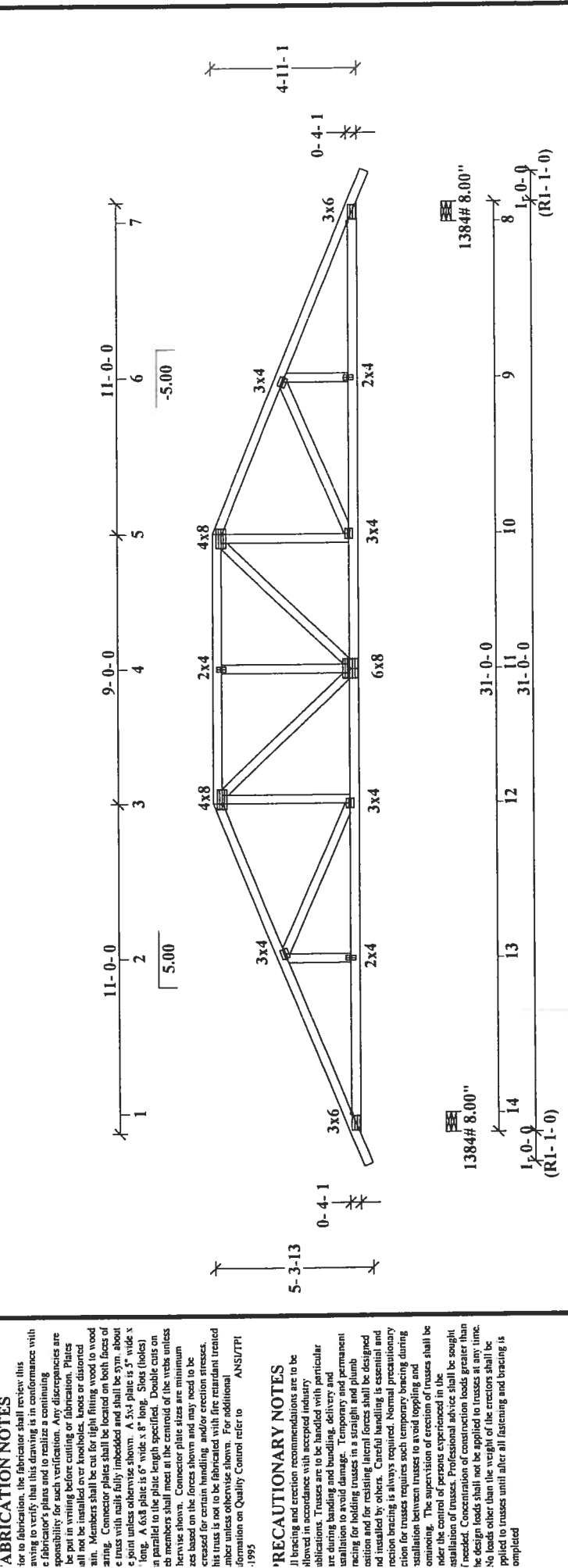


Scale = 0.1980

DESIGN INFORMATION
This design is for an individual building component and is based on information provided by the client. The engineer disclaims any responsibility for damages as a result of failure or incorrect information, specifications or design changes furnished to the client by the client or the correctness or accuracy of this information as it relates to a specific project and accepts no responsibility for erection or construction with regard to installation, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and 305-97 to be incorporated as part of the building design. A Building Designer, when reviewing for approval by a building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special piled loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes no truss bracing (top or bottom) are continuously used by sheathing unless otherwise specified. Where open applied righting, the sheathing shall be installed by a minimum of 10 gage hot dipped galvanized steel sheathing ASTM A 653, Grade 40, unless otherwise shown.

ABRICATION NOTES
For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with a fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates are to be installed over knotholes, knots or disjuncts and not be installed over knots or disjuncts. Members shall be cut for tight fitting, with force of a truss with nuts fully installed. A 5/8" plate 13" wide x 1/2" thick shall be installed over the top of the joint unless otherwise shown. A 5/8" plate 13" wide x 1/2" thick shall be installed over the bottom of the joint unless otherwise shown. Double cut on members shall meet in the center. Connector plates shall be furnished from the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

RECAUTIONARY NOTES
If bracing and erection recommendations are to be followed in accordance with accepted industry publications, trusses are to be handled with particular care during handling and erection. Delivery and erection shall be in accordance with the following: Trusses are to be delivered in a straight and plumb condition. Trusses shall be handled by the lifting points indicated on the drawing. Trusses shall be braced by others. Careful handling is essential and no truss shall be braced by others. Normal precautionary erection for trusses requires such temporary bracing and stabilization between trusses to avoid toppling and overturning. The supervision of erection of trusses shall be under the control of persons experienced in the erection of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.



EXCEPT AS SHOWN PLATES ARE Mitek MT20

Scale = 0.1980

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

DESIGN: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\H8.prx

Eng Job:	WO: SUNL3
Dwg:	TI: H8
Dsgnr: ECH	Chk: 6/6/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall truss may be required. (See HUB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Ondre Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 418 Soundside Rd. Edenton, NC 27932

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it relates to a specific project and accepts no

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the applicable codes, plans and to realize a continuing responsibility to the fabricator's health and safety. Any discrepancies or omissions in writing before cutting or fabrication. Plates shall not be installed over buckholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood connections. Connector plates shall be located on both faces of the main member. The joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double cut on plates shall be parallel to the plate length specified. Scribe cut on plates shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum dimensions based on the forces shown and may need to be increased for certain handling and/or erection stresses. This must not be fabricated with the retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TFP 1-1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

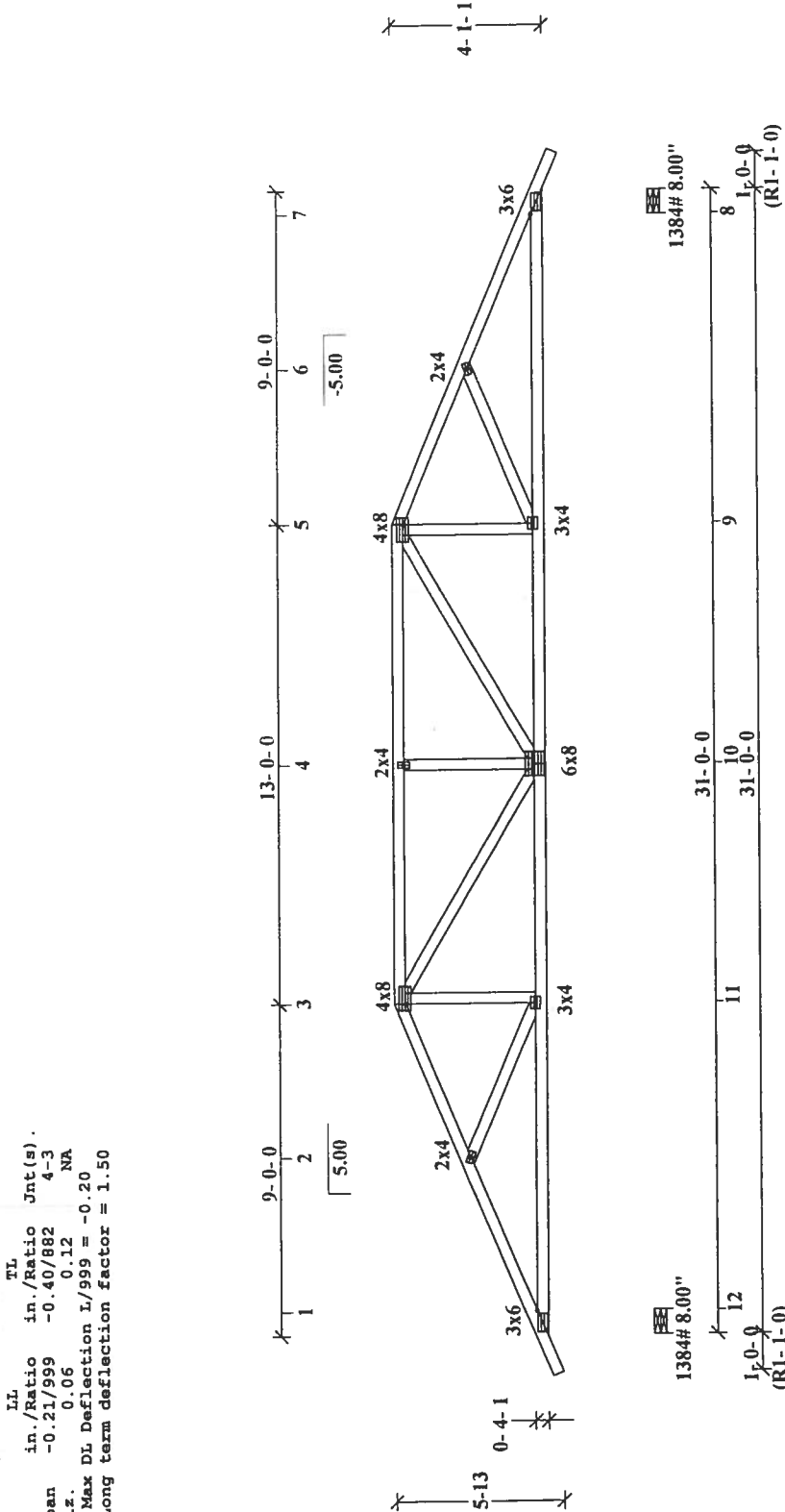
4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
67 Medallion Pl. Chuluota, FL 32766

D'Onofrio Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., #18 Southside Rd. Edenton, NC 27932

Design: Matrix Analysis

TTC Live	16.0	psf	Lbr DF: 1.25
TTC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2-0-0
BC Dead	10.0	psf	TPI-02/FBC-04
TOTAL	43.0	psf	Code: FLA
			v4.7.32-46834

	LL		TL	
	in./Ratio	in./Ratio	in./Ratio	Jnt(s)
I Span	-0.21/999	-0.40/882		4-3
Horiz.	0.06	0.12	NA	NA
	Max DL Deflection L/999 = -0.20			
	Long term deflection factor = 1.50			



DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client. The accuracy or correctness of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the manufacturer's plans and to realize a conjoining of the fabricator's plans with verification. Any discrepancies are to be put in writing before cutting or fabricating. Plates shall be installed over knobholes, knots or distortion grain. Members shall be cut for tight fitting, void to void joints. Connector plates shall be located on both faces of the truss with male fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is "x" wide x "y" long. A 6x8 plate is 6" wide x 8" long. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/ITP 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses in any time. No loads other than the weight of the erectors in any time is applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2

WO: SUNL3

Otv: 1

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

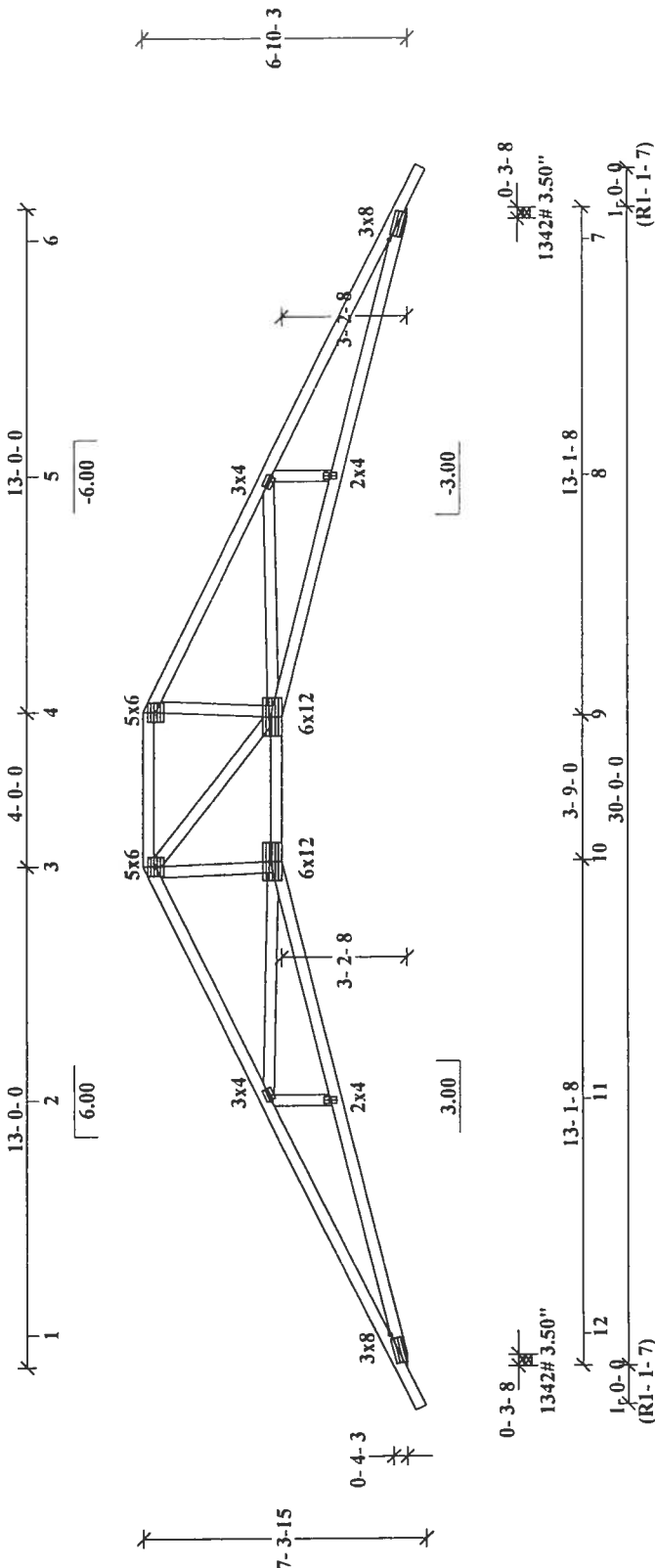
In-Plant Quality Assurance with Cq= 1

In-Plant Quality Assurance, per sec. 3.2.4 of TFI-1-02, for joint(s):

	TC	FORCE	CSI	Cr	BC	12	11	328	0.93
1-2	-4196	0.62	A	12	-11	328	0.93		
2-3	-3180	0.53	A	11	-10	3819	0.86		
3-4	-2813	0.36	A	10	-9	2814	0.58		
4-5	-3180	0.53	A	9	-8	3820	0.86		
5-6	-4196	0.62	A	8	-7	3828	0.93		

1,6
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-4196/1752, 2-3=-3180/1376,
3-4=-2813/1311, 4-5=-3180/1373, 5-6=-4196/1753,
BOT CHORD 12-11=-1506/3928, 11-10=-1500/3819,
10-9=-968/2814, 9-8=-1501/3820, 8-7=-1507/3828,
Webs 11-2=0/217, 2-1=0/-683/476,
3-10=-376/1165, 3-9=2-7, 9-4=-375/51165,
9-5=-684/481, 5-6=0/217,

	LL	TL
	in./Ratio	in./Ratio
I.Span	-0.42/833	-0.83/423
Horiz.	0.30	0.58
	NA	NA
Max DL Deflection	L/862 = -0.41	
Long term deflection factor =	1.50	



EXCEPT AS SHOWN PLATES ARE MITAK M20

Scale = 0.2038

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ALL AFFECTING CONTRACTOR. BRACING WARNING:
Framing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified members only as determined by the building designer. Additional bracing of the overall structure may be required [See HUD-91 (TP1) for specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI) is located at 583 D. O'Brien Drive, Madison, Wisconsin 53719].
C. A. 818 Emeraldale Park, Edinburg, NC 27031

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNL3\HS1.prx

Eng Job:	WO: SUNL3
Dwg:	TI: HS1
Dsgnr: TLY	9/14/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf
	lbr DF: 1.25
	Plt DF: 1.25
	O.C.: 2- 0- 0
	TPI-02/FBC-04
	Code: FLA
	v4.7.32-59740

This design is for an individual building component and has been based on information provided by the client. The designer does not accept responsibility for any consequences arising from a reliance on such information, specifications or drawings for design purposes. The design is intended as a guide for the construction of the building component, by the client or their agent, and the correctness and accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to transportation, storage, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANS/TPI 1-1995 and ENDS-57 to be incorporated as part of the building design by a Building Designer. The design requires approval by a Building Designer. When loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special engineered loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a floor joist or other member, they shall be braced at a maximum spacing applied right cutting, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be fabricated in accordance with ASTM A 633, grade 40, unless galvanized steel sheet piling ASTM A 653, Grade 40, dipped galvanneal steel.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the applicable code and all applicable regulations, and the responsibility for such verification. Any discrepancies are to be reported in writing before cutting or fabrication. Plates are not to be installed over welds, cracks, knots or distorted framing. Members shall be cut for tight fitting voids to wood framing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3/4" plate is 3" wide x 4' long. A 5/8" plate is 6" wide x 3" long. Double cut on top parallel to the plate length specified. Double cut on bottom members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire resistant treated lumber unless otherwise shown. For additional information refer to the American Institute of Steel Construction Quality Control refer to ANSI/TPI 1-1985.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be hoisted with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Maronda Systems

Design: Matrix Analysis

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenilton, NC 27932
D Onofrio Drive, Madison, Wisconsin 53719

TOTAL	43.0	psf
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v4.7.32-46891

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MULTIPLE LOADS -- This design is the
composite result of multiple loads.
Windload per ASCE 7-02,          C6C, W = 125mph,
Bld Ft = 21.0 ft, I = 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type = encl I = 40.0 ft W= 30.0 ft Truss
in INT zone, TCDDL= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req
FORCES (lb) - Max Compression/Max Tension
CHORD 1-2=-4157/1951, 2-3=-3840/1686,
3-4=-4114/1869, 4-5=-4114/1869, 5-6=-3841/1686,
6-7=-4158/1951,
BOB CHORD 12-11=-1701/3817, 11-10=-1322/3438,
10-9=-1323/3439, 9-8=-1702/3817,
Wabs 2-11=-265/319, 3-11=-390/1200,
3-10=-307/719, 10-4=-260/225, 10-5=-307/718,
9-5=-391/1200, 9-6=-264/319,
-----GLOBAL MAX DEFLECTIONS-----
            LL              TL
in./Ratio  in./Ratio  Jnt(s).
I.Span    -0.47/758    -0.92/385      4
Horiz.     0.29        0.57      NA
Max DL Deflection L/784 = -0.45
Long term deflection factor = 1.50

```

	Joint Locations								
1)	0-0-0	5)	21-0-0	9)	20-10-8				
2)	4-10-15	6)	25-1-1	10)	15-0-0				
3)	9-0-0	7)	30-0-0	11)	9-1-8				
4)	15-0-0	8)	30-0-0	12)	0-0-0				

In-Plant Quality Assurance with Cq= 1
 In-Plant Quality Assurance, per
 sec. 3.2.4 of TPI-1-02, for joint(s):
 1.3.5.7

TC.	FORCE.	CSI.	CF.	BC.	FORCE.	CSI.
1-	2	-4157	0.49	A 12-11	3817	0.96
2-	3	-3840	0.45	A 11-10	3438	0.74
3-	4	-4114	0.48	A 10-9	3439	0.74
4-	5	-4114	0.48	A 9-8	3817	0.96
5-	6	-3841	0.45	A		
6-	7	-4158	0.49	A		

	LL		TL	
	in./Ratio	in./Ratio	in./Ratio	Jnt(s)
I.Span	-0.47/758	-0.92/385		4
Horiz.	0.29	0.57		NA
Max DL Deflection	L/784 = -0.45			
Long term deflection	factor = 1.50			

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: HSGRD

Qty: 3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for erection or control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in rigid ceiling, they are not fully braced laterally by a properly applied rigid ceiling. They should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both sides of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2

2x 8 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

2x 4 SP #2

2- 8, 4- 8

2- 3, 3- 4

MULTIPLE LOADS -- This design is the composite result of multiple loads. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. 3-PLY TRUSS: Fasten w/3"x 0.120" nails in staggered pattern per nailing schedule: TC- 2 per ft. BC- 2 per ft. WEBS- 2 per ft Repeat nailing as each ply is applied. Distribute loads equally to each ply. .TC...FORCE...CSI, Cr...BC...FORCE...CSI, Cr... 1- 2 -8215 0.17 A 10- 9 7624 0.33 2- 3 -14246 0.50 A 9- 8 8717 0.62 3- 4 -14246 0.50 A 8- 7 8710 0.62 4- 5 -8210 0.17 A 7- 6 7618 0.33 3 PLYS REQUIRED Lineal Footage = 170.1

[]: Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

This 3-PLY Hip Master designed to carry 5'-0" open jacks (no webs) across center and Hip Jack Truss framed to BC Wndload per ASCE 7-02, Cfc, V= 125mph, B= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl L= 40.0 ft W= 30.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req Supported span based on int. wind zone. In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION----- X-Loc Bset Vert Horiz Uplift Y Type 0- 1-12 1 2180 0 -681 B Pin 29-10- 4 1 2179 0 -680 B H Roll PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB...FORCE...CSI, Cr .WEB...FORCE...CSI, Cr 2- 9 2884 0.31 C 8- 4 5560 0.34 2- 8 5554 0.34 C 7- 4 2882 0.31 8- 3 -520 0.05 C

-----GLOBAL MAX DEFLECTIONS----- LL in./Ratio in./Ratio Jnt(s) I.Span -0.60/589 -1.18/298 3 Horiz. 0.17 0.34 NA Max DL Deflection L/606 = -0.58 Long term deflection factor = 1.50

Joint Locations 1) 0-0-0 5) 30-0-0 9) 5-4-8 2) 5-0-0 6) 30-0-0 10) 0-0-0 3) 15-0-0 7) 24-7-8 4) 25-0-0 8) 15-0-0

-----TOTAL DESIGN LOADS----- Uniform PLF From PLF To TC Vert L+D -46 -1-1-9 -46 5-0-0 TC Vert L+D -77 5-0-0 -77 25-0-0 TC Vert L+D -46 25-0-0 -46 31-1-9 BC Vert L+D -40 0-0-0 -40 5-0-0 BC Vert L+D -67 5-0-0 -67 25-0-0 BC Vert L+D -40 25-0-0 -40 30-0-0 Concentrated LBS Location BC Vert L+D -255 5-0-0 BC Vert L+D -255 25-0-0 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-8215/2534, 2-3=-14246/4402, 3-4=-14246/4402, 4-5=-8210/2532, C8-7=-2548/8710, 7-8=-2306/7618, 8-9=-2550/8717, Cwebs 2-9=-648/2884, 2-8=-1861/5554, 8-3=-520/485, 8-4=-1863/5560, 7-4=-648/2882,

Multiple Plys

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 83 D'Ondrino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Seaside Rd, Edenton, NC 27932

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNL3

TI: HSGRD

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FTA

V4.7.32-46894

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050066

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\HSGRD.prx

EXCEPT AS SHOWN PLATES ARE MITAK MT20

Scale = 0.1982

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J12

Qty: 2

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSIRTP1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local building official and/or other applicable codes. Proper truss has not been designed for storage or shipping loads. The design assumes connector chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chord in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over existing wood or other materials. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 1" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIRTP1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and building. Delivery and installation to avoid damage. Temporary and permanent bracing (not including truss) shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

...TC...FORCE..CSI..Cr...BC...FORCE..CSI..1-2-125 0.78 A 1-5-9 0.81

2-3 186 0.39 A 5-4-0 0.81

..WEB...FORCE..CSI..Cr..5-2-332 0.10 C

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 377 98 -471 B Pin

6-11-4 1 118 257 -134 T Pin

6-11-4 1 158 0 -223 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

Joint Locations

1) 0-7-13 3) 7-0-0 5) 4-11-14

2) 4-11-14 4) 7-0-0 6) 0-7-13

WindLoad per ASCE 7-02, C&C, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 31.0 ft W= 7.0 ft Truss in INT zone, TCOL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-125/68, 2-3=-45/186,

BOT CHORD 1-5=-9/2, 5-4=0/0,

Webs 5-2=-62/332,

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s) .

I. Span 0.60/xxx 0.22/xxx 5

Horiz. -0.09 -0.17 NA

Max DL Deflection L/xxx = -0.37

Long term deflection factor = 1.50

377# 8.00"

158# 1.50"

3-7-13

0-4-1

3-3-1

Attach with (4) 16d Toe-Nails

Attach with (3) 16d Toe-Nails

3-3-1

3

2

1

7-0-0

5.00

2x4

3x4

2x4

1-0-0

(R1-1-0)

7-0-0

7-0-0

4

5

6

Over 3 Supports

Scale = 0.4576

Eng Job:

DWG:

Dsgnr: ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNL3

TI: J12

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-53785

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J12.prx

Maronda Systems		4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl. Chuluota, FL 32716	
Design: Matrix Analysis		Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J12A.prx	
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenport, NC 27932			
Eng Job:	WO: SUNL3		
Dwg:	TI: J12A		
Dsgmr:	ECH	Chk:	6/6/2006
TC Live	16.0	psf	Lbr DF: 1.25
TC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2- 0- 0
BC Dead	10.0	psf	TPT-02/FBC-04
TOTAL	43.0	psf	Code: FLA
			v4.7.32-53787

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J13

Qty: 3

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=0/45,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, CSc, V= 125mph,

H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl I= 32.0 ft W= 5.0 ft Truss

in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC...FORCE..CSI..Cr..BC...FORCE..CSI..

1- 2 45 0.60 A 1- 3 0 0.48

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.13/xxx 0.05/xxx 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/xxx = -0.08

Long term deflection factor = 1.50

Joint Locations

1) 0- 7-13 3) 5- 0- 0

2) 5- 0- 0 4) 0- 7-13

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 266 161 -397 B Pin

4-11- 4 1 113 124 -274 T Pin

4-11- 4 1 103 0 -41 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

2- 9-13

0- 4- 1

5- 0- 0

2

5.00

Attach with (3)

16d Toe-Nails

2- 5- 1

Attach with (2)

16d Toe-Nails

3x4

103# 1.50"

266# 8.00"

1- 0- 0

4

5- 0- 0

5- 0- 0

(R1-1-0)

EXCEPT AS SHOWN PLATES ARE M/Tek MT20

Over 3 Supports Scale = 0.4677

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or extends no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes temporary loads (top or bottom) are shown. Where action chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted grain. Members shall be cut to right angles to wood framing. Connector plates shall be fastened to both faces of the truss with nails fully bedded and shall be 2" wide x 1" long at 6" pitch in tension. A 5/4 plate is 2" wide x 1" long at 6" pitch in tension. Double cut on web members shall meet at the extremity of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry applications. Trusses are to be handled with particular care during loading and building, delivery and installation. Temporary bracing and temporary component bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and damage. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until all fastening and bracing is completed

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3013

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chulueta, FL 32766

WARNING:

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Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oneofrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenon, NC 27932

Eng Job:

WO: SUNL3

Dwg:

TI: J13

Dgnr: ECH

Chk:

6/6/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-4677

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J13.prx

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J14

Qty: 3

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-5/27,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, Exp.Cat. B, Kzt=1.0, H=21.0 ft, I=1.00, Exp.Cat. B, Kzt=1.0, Bid Type= encl I= 32.0 ft W= 3.0 ft Truss in END zone, TCCL= 4.2 psf, BCCL= 6.0 psf Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 27 0.22 A 1- 3 0 0.17

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection I/xxx = -0.01

Long term deflection factor = 1.50

1) 0- 7-13 3) 3- 0- 0

2) 3- 0- 0 4) 0- 7-13

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 4- 0 1 180 99 -282 B Pin

2-11- 4 1 68 77 -170 T Pin

2-11- 4 1 62 0 -24 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

DESIGN INFORMATION

This design is for an individual building component and is not to be used for any other purpose. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, shipping and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rafter tie, they should be braced at the maximum spacing of 10'-0" to prevent lateral-torsional buckling. Minimum 20 gauge box shaped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be 3" in. about the joint unless otherwise shown. X-X plate is 3" x 1/2" x 1" long. (End plate is 6" wide x 8" long. See (holes) parallel to the joint with specified double cut web member shall meet at the centroid of the web unless otherwise shown. Connector plate size is minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and action for use of any such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

WO: SUNL3

TI: J14

Qty: 3

Eng Job:

Dwg:

Dsgnr: ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Over 3 Supports

Scale = 0.5469

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00050068

367 Medallion Pl Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J14.prx

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J14A

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-5/27,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C&C, V= 125mph, R= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl I= 32.0 ft W= 3.0 ft Truss in END zone, TCCL= 4.2 psf, RCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req ..TC...FORCE..CSI..Cr ..BC...FORCE..CSI.. 1- 2 27 0.22 A 1- 3 0 0.17

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.02/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/xxx = -0.01

Long term deflection factor = 1.50

Joint Locations

1) 0- 7-13 3) 3- 0- 0

2) 3- 0- 0 4) 0- 7-13

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 129 99 -185 B Pin

2-11- 4 1 68 77 -170 T Pin

2-11- 4 1 62 0 -24 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSUTPI 1-1993 and VDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed or checked for seismic loads. The design assumes truss is braced in all directions. The design assumes truss is braced by sheathing unless otherwise specified. Where seismic chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

• Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for light fitting wood to wood kneeling. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. (3x4 plate is 3" wide x 11" long. A 6x6 plate is 6" wide x 8" long. Short (holes) on panels in plates length specified. Double cuts on web members shall be in the middle of the member unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSUTPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry applications. Trusses are to be handled with particular attention to bracing and handling. Delivery and erection are to be done in accordance with design and permanent marking for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and tipping. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until all fastening and bracing is completed.

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5937

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co. P.A. 818 Soundside Rd. Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3513

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J14A.prx

Eng Job:

WO: SUNL3

TI: J14A

Dsgnr:ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Eng Job:

WO: SUNL3

TI: J14A

Dsgnr:ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Eng Job:

WO: SUNL3

TI: J14A

Dsgnr:ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J15

Qty: 3

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-9/0,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless otherwise specified.

Windload per ASCE 7-02. Csc, V= 125mph, H= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 32.0 ft W= 1.0 ft Truss

in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

TC...FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 -9 0.03 A 1- 3 0 0.01

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Unt(s).

I.Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

long term deflection factor = 1.50

1) 0- 7-13 3) 1- 0- 0

2) 1- 0- 0 4) 0- 7-13

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-11- 4 1 23 27 -59 B Pin

0-11- 4 1 20 0 -6 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

1- 0- 0 1 2

5.00

C/L: 0-11- 4

23# 1.50"

3x4

0- 4- 1

1- 1- 13

0- 9- 1

Attach with (2)

16d Toe-Nails

94# 8.00" 20# 1.50"

1- 0- 0 4 3

1- 0- 0 1- 0- 0

(R1- 1- 0)

Attach with (2)

16d Toe-Nails

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Over 3 Supports Scale = 0.6564

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HUB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

387 Medallion Pl Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOR\Work\Jobs\SUNL3\SUNBURY L 3\J15.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipping and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords are not fully braced laterally by a properly applied rigid ceiling, they should be braced as a compression member (100% C.C.). Connector plates shall be manufactured from 70 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over handholes, doors or ducted joints. Members shall be cut for tight fitting and to avoid gapping. Connector plates shall be located on both faces of the joint with ends fully embedded and shall be sym. about the joint unless otherwise shown. A 1/4 plate is 1" wide x 4" long. A 1/2 plate is 6" wide x 8" long. Slots (holes) cut parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in place and holding them in position and for erecting them shall be designed and installed by others. Careful handling is essential and erection is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

Job: SUNBURY L 3Customer:WU: SUNL3TI: J15AQty: 1

TC: 2x 4 SP #2
BC: 2x 4 SP #2
MULTIPLE LOADS -- This design is the composite result of multiple loads.
This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-9/0,
BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Windload per ASCE 7-02, C6C, V= 125mph,
B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
Bld Type= encl L= 32.0 ft W= 1.0 ft Truss
in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req
..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..
1- 2 -9 0.03 A 1- 3 0 0.01
-----GLOBAL MAX DEFLECTIONS-----
LL
in./Ratio in./Ratio Jnt(s)
I. Span 0.00/999 0.00/999 2-1
Horiz. 0.00 0.00 NA
Max DL Deflection L/999 = 0.00
Long term deflection factor = 1.50

1) 0- 7-13 3) 1- 0- 0
2) 1- 0- 0 4) 0- 7-13
In-Plant Quality Assurance with Cq= 1
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 4- 0 1 43 33 -62 B Pin
0-11- 4 1 23 27 -59 T Pin
0-11- 4 1 20 0 -6 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE

DESIGN INFORMATION
This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.
This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Interior bottom chords in rafter areas are not fully braced and shall be properly applied with a minimum of 100% bracing. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 633, Grade 40, unless otherwise shown.

FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5' wide x 4' long. A 6x6 plate is 6' wide x 8' long. Slots (holes) run parallel to the plate length specified. Provide cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are to be based on the provisions and requirements of the minimum sizes for truss members and end connections.
This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal preliminary installation requires such temporary bracing during shipment. The provisions of erection of trusses shall be in the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Consideration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.7251

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 581 D'Oroff Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Eng Job:
Dwg:
Dsgn: ECH Chk:
TC Live 16.0 psf
TC Dead 7.0 psf
BC Live 10.0 psf
BC Dead 10.0 psf
TOTAL 43.0 psf

WO: SUNL3
TI: J15A
6/6/2006
Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FLA
v4.7.32-46815

Profile Path: C:\TEB-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J15A.prx

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1) 0-6-8	3) 3-0-0
2) 3-0-0	4) 0-6-8

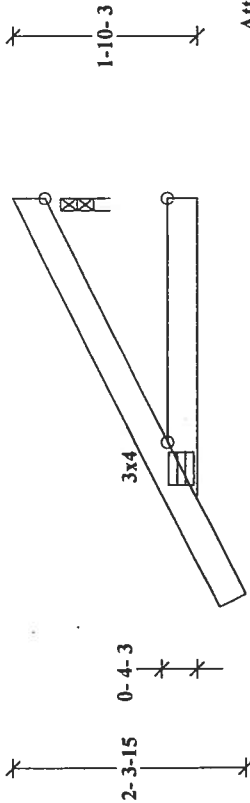
In-Plant Quality Assurance with Cq=1

-----MAX. REACTIONS PER BEARING LOCATION-----

K-Loc	Bset	Vert	Horiz	Uplift	Y Type
0-1-12	1	181	119	-279	B Fin
2-11-4	1	67	92	-169	T Fin
2-11-4	1	62	0	-27	B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Attach with (2) 16d Toe-Nails



Attach with (2) 16d Toe-Nails

Over 3 Supports Scale = 0.5116

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not considered bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members and roof purlins during erection and installation of the building. Additional bracing of the overall structure may be required. (See HIFR-91 locations defined to be used by the building designer.)

(TP1) For specific truss bracing requirements, contact building designer (Trus Plate Institute, TPI is located at 883 O'Neale Drive, Madison, Wisconsin 53719).

Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd. Edenport, NC 27932.

WO: SUNL3	TI: J2	6/6/2006	1br DF: 1.25
			Plt DF: 1.25
			O.C.: 2 - 0 - 0
			TPI-02/FBC-04
			Code: FLA
			v4.7.32-46796

Job: SUNBURY L 3

Customer:

WO: SUNL3

TI: J2A

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or design furnished to the client. The design is the property of the designer and shall remain confidential as it may relate to specific property and shall not be used for any other project without the written consent of the designer. The designer shall not be held responsible for any errors or omissions in the design, fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies shall not be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 1" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-6/31,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C6C, V= 125mph, H= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0, Bld Type= encl L= 32.0 ft W= 3.0 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req. TC...FORCE..CSI. Cr ..BC...FORCE..CSI. 1- 2 31 0.24 A 1- 3 0 0.19

-----GLOBAL MAX DEFLECTIONS-----

LL TL in./Ratio in./Ratio Jnt(s) I. Span 0.02/xxx 0.01/xxx 3-1 Horiz. 0.00 0.00 NA Max DL Deflection L/xxx = -0.01 Long term deflection factor = 1.50

1) 0- 6- 8 3) 3- 0- 0

2) 3- 0- 0 4) 0- 6- 8

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 1-12 1 129 119 -183 B Pin

2-11- 4 1 67 92 -169 T Pin

2-11- 4 1 62 0 -27 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5617

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion PL Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing. Erection bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 581 D Onofrio Drive, Madison, Wisconsin 53719).

Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd. Edenon, NC 27932

Eng Job:

WO: SUNL3

Dwg:

TI: J2A

Dsgnr: ECH

Chk:

6/6/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

V4.7.32-46802

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J2A.prx

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: J3A

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-2/11,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Welded per ASCE 7-02, C6C, V= 125mph,

B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 32.0 ft W= 1.0 ft Truss

in END zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 11 0.03 A 1- 3 0 0.02

-----GLOBAL MAX DEFLECTIONS-----

in./Ratio in./Ratio Jnt(s).

I.Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Joint Locations

1) 0- 6- 8 3) 1- 0- 0

2) 1- 0- 0 4) 0- 6- 8

In-Plant Quality Assurance with Cqf 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 1-12 1 43 40 -61 B Pin

0-11- 4 1 23 32 -58 T Pin

0-11- 4 1 21 0 -7 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

1-0-0 1 2

6.00

C/L: 0-11-4

23# 1.50"

3x4

0-10-3

0-4-3

Max DL Deflection L/999 = 0.00

Attach with (2)

16d Toe-Nails

0-10-3

43# 3.50"

21# 1.50"

1-0-0 4 3

Attach with (2)

16d Toe-Nails

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.7073

WARNING:

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Bracing shown on this drawing is not erection bracing, wind bracing, or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEB-LOK\Work\Jobs\SUNL3\SUNBURY L 3\J3A.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid diaphragm, they should be braced as a compression chord. Connections (plates) shall be maximum spacing of 16" on center. Plates shall be minimum 3/16" thick and 30" long. No clipped or notched (not meeting ASTM A 653, Grade 40, unless otherwise shown).

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 8" long. Shims (holes) run parallel to the plate length specified. Do not cut or notch members and pieces at the center of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation to prevent trusses from tipping and falling and to prevent the trusses from being damaged. The provisions of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: JGRD1

Qty: 2

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or inaccurate information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and except no responsibility of erection or control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed with an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a building designer. When reviewed for approval by a building designer, the design loadings shown must be checked to be sure that the loads shown are in agreement with the local building codes. Deflections are recorded for wind or occupancy loads. The design assumes applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and specifications. The fabricator shall be responsible for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over leadpipes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/4 plate is 6" wide x 8" long. Slot (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADS -- This design is the composite result of multiple loads. Right End vertical designed for wind. In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 5-11 1 285 281 -592 B Pin

9- 8-13 1 536 0 -325 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

..TC.. FORCE..CSI.. Cr ..BC.. FORCE..CSI..

1- 2 613 0.46 C 6- 5 -754 0.25

5- 2 206 0.07 C 4- 3 -152 0.25 C

2- 4 826 0.27 C

---GLOBAL MAX DEFLECTIONS---

LL TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.10/999 0.10/999 2-1

Horiz. -0.01 -0.02 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, Exp. Cat. B, Kzt= 1.0

Bld Type= encl L= 32.0 ft W= 9.8 ft Truss

In END zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

Supported span based on int. wind zone.

..WEB.. FORCE..CSI.. Cr

5- 2 206 0.07 C

2- 4 826 0.27 C

Joint Locations

1) 0- 0- 0 3) 9-10- 1 5) 5- 3-12

2) 5- 3-12 4) 9-10- 1 6) 0- 0- 0

----- TOTAL DESIGN LOADS -----

Uniform PLF From PLF To

TC Vert L+D -11 -1- 6- 2 0 0- 0- 0

TC Vert L+D 0 0- 0- 0 -88 9-10- 1

BC Vert L+D 0 0- 0- 0 -76 9-10- 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-607/613, 2-3=-50/123,

BOT CHORD 6-5=-754/560, 5-4=-752/549,

Wabs 5-2=-37/206, 2-4=-603/826,

4-3=-152/22,

285# 11.31"

536# 2.50"

4-3-7

0-3-14

3x4

2x4

3x4

3x4

3-9-10

9-10-1

9-10-1

9-10-1

1-5-0

6

5

4

1-6-0

4.24

EXCEPT AS SHOWN PLATES ARE MITek MT20

Scale = 0.3325

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chulueta, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\JGRD1.prx

Eng Job:

Dwg:

Dsgnr: ECH

Chk:

WO: SUNL3

TI: JGRD1

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-46667

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27912

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: JS3

Qty: 4

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-11/0,

BOT CHORD 1-3=-2/4,

Analysis based on Simplified Analog Model.

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-1-12 1 89 43 -174 B Pin

0-11- 4 1 23 35 -63 T Pin

0-11- 4 1 21 0 -13 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Joint Locations

1) 0- 9-15 3) 1- 0- 0

2) 1- 0- 0 4) 0- 9-15

WnLoad per ASCE 7-02, C&C, V= 125mph,

H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 1.0 ft W= 1.0 ft Truss

in END zone, TCDF= 0.0 psf, BCDL= 0.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr ..BC..FORCE..CSI..

1- 2 -11 0.03 A 1- 3 4 0.01

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I.Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for exercises no control with regard to fabrication, handling, shipping, and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid diaphragm, they should be braced at a maximum spacing of 10'-0". Compression chords shall be braced with 2x4 or 2x6 grade hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5' wide x 4' long. A 6x6 plate is 6' wide x 8' long. Slots (holes) run parallel to the plate length specified. Bolts cut on when members shall meet at the centroid of the members unless otherwise shown. Connections and fastenings are minimum based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and damage. The supervision of erection of trusses shall be under the control of persons experienced in the erection of trusses. Professional advice shall be sought if needed. Connections and erection loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

Diagram

1-3-2

0-4-3

0-8-1

0-2-2 Attach with (2) 16d Toe-Nails

3x4

6.00

C/L: 0-11- 4

23# 1.50"

3.00

0-3-8

89# 3.50"

21# 1.50"

1-0-0

4 3

1-0-0

1-0-0

(R0-11-11)

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Over 3 Supports Scale = 0.7438

Maronda Systems

4005 MARONDA WAY

Sunford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, post bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNL3

TI: JS3

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-46912

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\JS3.prx

Job: SUNBURY L 3

Customer: WO:SUNL3

TI:PI

Qty:13

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..1- 2 45 0.62 A 1- 3 1 0.53

-----GLOBAL MAX DEFLECTIONS-----

in./Ratio

in./Ratio

in./Ratio

Jnt(s)

Horiz.

0.00

0.07/xxx

3-1

NA

Max DL Deflection L/xxx = -0.06

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, Exp. Cat. B, V=125mph, H=21.0 ft, I=1.00, Exp. Cat. B, Kzt=1.0

Bld Type= part I= 50.0 ft W= 5.0 ft Truss in INT zone, TCDF= 4.2 psf, BCDF= 6.0 psf

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=0/45, 1-3=0/1,

BOT CHORD 1-3=0/1,

Joint Locations

1) 0- 7-13 3) 5- 0- 0

2) 5- 0- 0 4) 0- 7-13

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 215 163 -441 B Pin

4-11- 4 1 107 126 -279 T Pin

4-11- 4 1 94 0 -79 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

2- 9-13

0- 4- 1

2- 5- 1

Attach with (3)

16d Toe-Nails

Attach with (2)

16d Toe-Nails

5.00

C/L: 4-11-4

107# 1.50"

94# 1.50"

215# 8.00"

3x4

1-0-0

5-0-0

5-0-0

3

4

(R1-1-0)

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\PI.prx

WARNING:

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Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made for the lateral bracing at ends and specified at locations determined by the building designer. Additional bracing of the truss may be required. (See HUB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 383 D'Orofino Drive, Madison, Wisconsin 53719)

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipping, storage and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked with the local building codes, local officials for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting void to avoid bearing. Connector plates shall be located in both faces of the truss members and shall be about 1/2" from the joint unless otherwise shown. A 5x4 plate 3' x 4' long. A 6x8 plate is 6" wide x 8' long. Slots (holes) run parallel to the plate length specified. Double cut on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bonding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for loading trusses in a straight and plane position and for resisting erection forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid tipping and damage. The supervision of erection of trusses shall be done by the client or a professional engineer. If needed, Consultation of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

allow bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during loading and bundling. Delivery and permanent installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and undermining. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentrication of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
DMAS PONCE P.E. LICENSE #0050068
57 Medallion Pl. Chuluota, FL 32766

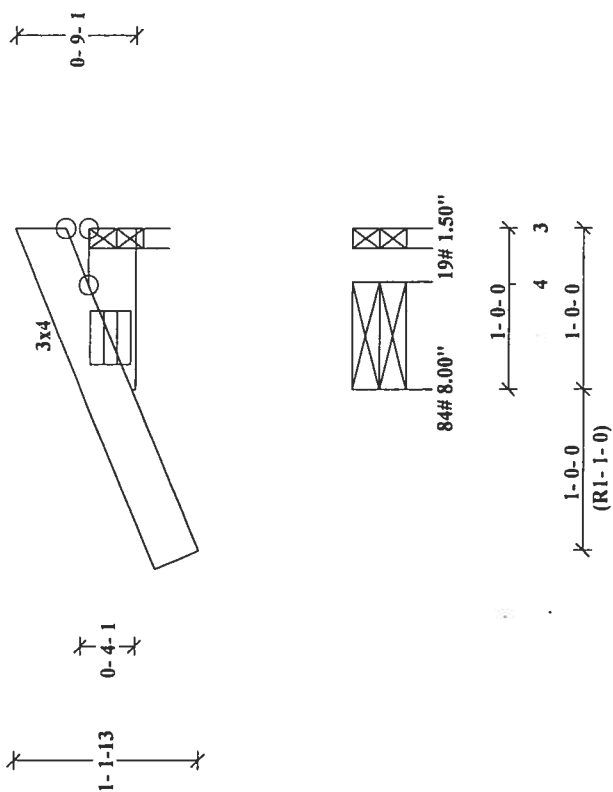
Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY I. 3\p2 prx

Component Engineering by Truss Engineering Co., P. A., 818 Soundside Rd., Edenton, NC 27932

TTC Live	16.0	psf
TTC Dead	7.0	psf
BBC Live	0.0	psf
BBC Dead	10.0	psf
TOTAL	33.0	psf

6/6/2006 2:21 PM

ILbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TTPI-02/FBC-04
Code: FLA



Attach with (2)
6d Toe-Nails

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for any claims or damages relating to the fabrication, erection, installation and/or use of the trust component in accordance with ANS/UL1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced against lateral movement. The bottom chord is braced at ends in space and not fully braced laterally by a property applied floor ceiling, they should be braced at a maximum spacing of 10'-0" c/c. Connector plates shall be manufactured from 20 gauge 40 unpainted galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to verify a continuing responsibility for each fabrication. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over holes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be girt, about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x4 plate is 6" wide x 4" long. Steel plates are to parallel to the plate length specified. Double (two) web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or reaction stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/UTPI 1985.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling. Delivery and installation is to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary order for trusses requires each temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction load greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

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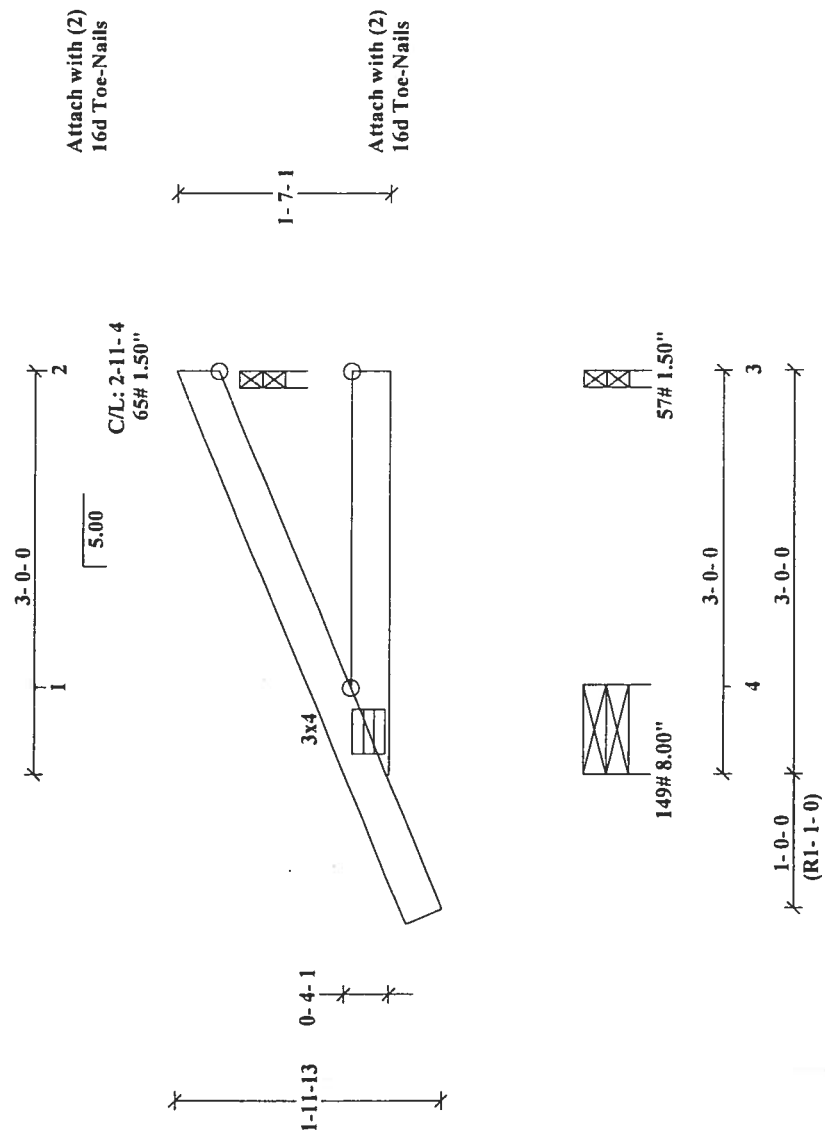
TC:  2x 4  SP #2
BC:  2x 4  SP #2
MULTIPLE LOADS  -- This design is the
                    composite result of multiple loads.
This truss is designed to bear on multiple
supports. Interior bearing locations should
be marked on truss. Shim or wedge if
necessary to achieve full bearing.
..TC...FORCE..CSI..Cr..BC...FORCE..CSI.
1- 2 27 0.23 A 1- 3 1 0.19
-----GLOBAL MAX DEFLECTIONS-----
                                LL          TL
                                in./Ratio  in./Ratio  Jnt(s).
I.Span      0.02/xxx      0.01/xxx      3-1
Horiz.       0.00          0.00          NA
Max DL Deflection L/xxx = -0.01
Long term deflection factor = 1.50

```

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
Wbnd per ASCE 7-02, CSc, V= 125mph, B= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= part L= 40.0 ft W= 3.0 ft Truss
In end zone, TCDL= 4.2 psf, BCDL= 6.0 psf
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-5/27,
BOT CHORD 1-3=0/1.

Y Y	MY	ALL MAX DEFLECTIONS-----
1	1	1
2	2	2
3	3	3
4	4	4
5	5	5
6	6	6
7	7	7
8	8	8
9	9	9
10	10	10
11	11	11
12	12	12
13	13	13
14	14	14
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91	91	91
92	92	92
93	93	93
94	94	94
95	95	95
96	96	96
97	97	97
98	98	98
99	99	99
100	100	100

	IL	IL
	in./Ratio	in./Ratio
I. Span	0.02/xxx	0.01/xxx
Horiz.	0.00	0.00
		NA
	Max DL Deflection	L/xxx = -0.01
	Long term deflection factor	= 1.50



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Over 3 supports Scale = 0.6935

Maronda Systems
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367 Medallion Pl. Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING: The bracing, portal bracing or similar bracing which is a part of
the structural design of the building and which must be provided by the building designer, shall be shown on the
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91
of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 483
O'Connell Drive, Madison, Wisconsin 53719).

Eng Job:	WO: SUNL3	
Dwg:	TI: P3	
Dsgnr:	ECH	6/6/2006
TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	0.0	psf
BC Dead	10.0	psf
TOTAL	33.0	psf
		Lbr DF: 1.25
		Plt DF: 1.25
		O.C.: 2- 0- 0
		TPI-02/FBC-04
		Code: FLA
		v4.7.32-46858

DESIGN INFORMATION
This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and VDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special uplift loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where action chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies shall not be installed over knockholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5'-4" plate is 5" wide x 4" long. A 6'-8" plate is 6" wide x 8" long. Slot holes run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES
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MULTIPLE LOADS -- This design is the composite result of multiple loads.
Right End vertical designed for wind.
In-Plant Quality Assurance with Cq= 1
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 5-11 1 105 181 -538 B Pin
6-10-14 1 186 0 -235 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
..TC.. FORCE..CSI.. Cr ..BC.. FORCE..CSI..
1- 2 72 0.77 C 4- 3 62 0.60
---GLOBAL MAX DEFLECTIONS---
TL
in./Ratio in./Ratio Jnt(s)
I. Span 0.27/265 0.27/265 2-1
Horiz. -0.03 -0.07 NA
Max DL Deflection L/999 = 0.00
Long term deflection factor = 1.50

Joint Locations
1) 0- 0- 0 3) 7- 0- 2
2) 7- 0- 2 4) 0- 0- 0
---TOTAL DESIGN LOADS---
Uniform PLF From PLF To
TC Vert L+D -11 -1- 5-15 0 0- 0- 0
TC Vert L+D 0 0- 0- 0 -56 7- 0- 2
BC Vert L+D 0 0- 0- 0 -24 7- 0- 2
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-51/72,
BOT CHORD 4-3=0/62,
Webs 3-2=-122/179,

Customer: WO: SUNL3
Job: SUNBURY L 3
Customer: WO: SUNL3
Job: SUNBURY L 3

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Job: SUNBURY L 3
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Job: SUNBURY L 3

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
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TOMAS PONCE P.E. LICENSE #0050068
367 Madison Pl. Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown in a part of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See TUB-9) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroffino Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenton, NC 27932

EXCEPT AS SHOWN PLATES ARE MITEX MT20 Scale = 0.4422

Eng Job:	WO: SUNL3
Dwg:	TI: PJGRD
Dsgnr: ECH	Chk: 6/6/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	0.0 psf
BC Dead	10.0 psf
TOTAL	33.0 psf

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\PJGRD.prx
Design: Matrix Analysis

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: PMH1

Qty: 1

DESIGN INFORMATION

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FABRICATION NOTES

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PRECAUTIONARY NOTES

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TC: 2x 4 SP #2

2x 6 SP #2

BC: 2x 4 SP #2

2x 4 SP #3

WB: 2x 4 SP #2

3- 5, 5- 6

3- 9, 5- 9, 6- 8

MULTIPLE LOADS -- This design is the composite result of multiple loads. Right End vertical designed for wind. In-Plant Quality Assurance with Cq= 1 In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 1, 6, 8

FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-1847/1491, 2-3=-1639/1342, 3-4=-1792/1429, 4-5=-1791/1429, 5-6=-1277/1006, BOT CHORD 12-11=-1501/1723, 11-10=-1500/1719, 10-9=-1291/1511, 9-8=-1004/1265, 8-7=-66/75, WEBS . . . FORCE . . . CSI . Cr 11-2= 574 0.42 C 9-5 574 0.04 C 5-8 -601 0.10 C 8-6 1439 0.05 C 7-6 -866 0.37 C 7-6 -866/710, 19-0-0

GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s)

I. Span -0.13/999 -0.23/999 9-10

Horiz. 0.02 0.06 NA

Max DL Deflection L/999 = -0.10

Long term deflection factor = 1.50

Joint Locations

1) 0-0-0 5) 21-8-0 9) 15-4-0

2) 4-9-14 6) 28-0-0 10) 9-0-0

3) 9-0-0 7) 28-0-0 11) 4-9-14

4) 15-4-0 8) 21-8-0 12) 0-0-0

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 982 193 -870 B H Roll

27-8-0 1 931 0 -747 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..

1-2 -1847 0.36 A 12-11 1723 0.49

2-3 -1639 0.35 A 11-10 1719 0.45

3-4 -1792 0.27 A 10-9 1511 0.46

4-5 -1791 0.26 A 9-8 1265 0.46

5-6 -1277 0.23 A 8-7 75 0.27

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.2250

982# 8.00"

931# 8.00"

19-0-0

4-5-13

0-4-1

12

11

10

9

8

7

6

5

4

3

2

1

3x4

3x4

3x4

5x10

2x4

2x4

2x4

6x8

3x6

3x6

3x6

2x4

2x4

3x6

4-1-1

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only; to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-9) of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 483 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Seaside Rd., Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\PMH1.prx

Eng Job:

Dwg:

Dsgnr: ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 0.0 psf

BC Dead 10.0 psf

TOTAL 33.0 psf

WO: SUNL3

TI: PMH1

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

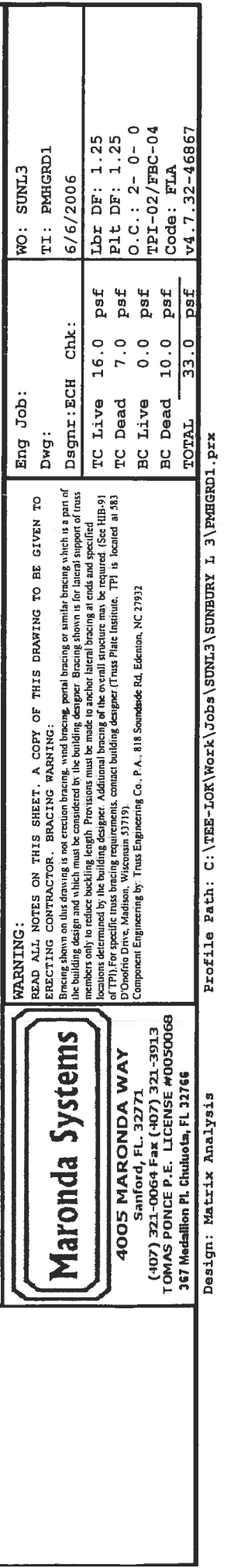
Code: FLA

v4.7.32-46875

WARNING:	
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Ondro Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenport, NC 27932	
Eng Job:	WO: SUNL3
Dwg:	TI: PMH2
Dsgnr: ECH Chk:	6/6/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	0.0 psf
BC Dead	10.0 psf
TOTAL	33.0 psf
Lbr DF:	1.25
Pbr DF:	1.25
O.C.:	2 - 0 - 0
TPI-02/FBC-04	
Code:	FLA
v4.7.32-46871	

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\PMH2.prx

Design: Matrix Analysis



DESIGN INFORMATION
This design is for an individual building component and is not to be used for any other purpose. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or design. The client is responsible for the accuracy of the information as it may relate to a specific project and for the proper use of the information. The client is responsible for the accuracy of the information as it may relate to a specific project and for the proper use of the information. The client is responsible for the accuracy of the information as it may relate to a specific project and for the proper use of the information.

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TC:	2x 4	SP	#2
BC:	2x 4	SP	#2
WB:	2x 4	SP	#3
GBL:	2x 4	SP	#3
MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.			
Windload per ASCE 7-02, CEC, V= 125mph, B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0			
Bid Type= encl I= 50.0 ft W= 20.7 ft Truss			
in END zone, TCCL= 0.0 psf, BCDL= 0.0 psf			
Impact Resistant Covering and/or Glazing Req			
..TC.. .FORCE..CSI..Cr..BC.. .FORCE..CSI..			
1- 2 -62 0.24 A 1-17 0 0.09			
2- 3 -38 0.11 A 17-16 0 0.06			
3- 4 45 0.08 A 16-15 0 0.02			
4- 5 114 0.12 A 15-14 0 0.02			
5- 6 114 0.11 A 14-13 0 0.02			
6- 7 45 0.06 A 13-12 0 0.02			
7- 8 -38 0.13 A 12-11 0 0.06			
8- 9 -62 0.16 A 11- 9 0 0.09			

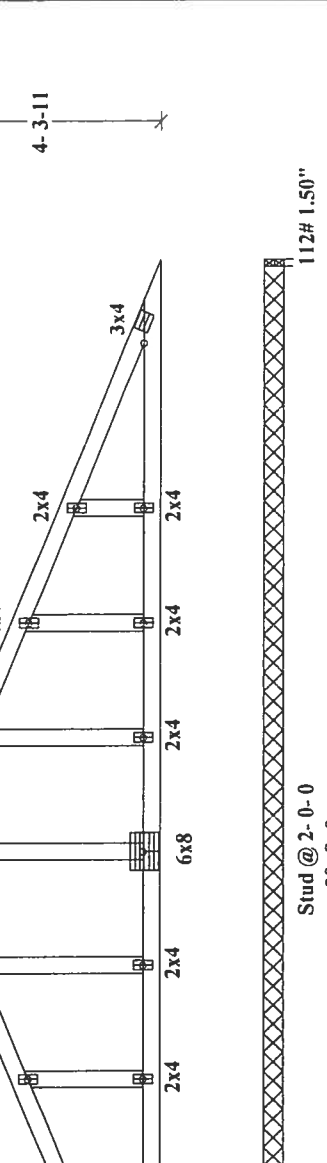
FABRICATION NOTES
Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's capabilities. Any discrepancies are to be noted and resolved prior to fabrication. The fabricator shall be responsible for the accuracy of the information as it may relate to a specific project and for the proper use of the information. The client is responsible for the accuracy of the information as it may relate to a specific project and for the proper use of the information.

FABRICATION NOTES
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in./Ratio	in./Ratio	Jnt(s)
I. Span	0.02/999	2-1
Horiz.	0.00	0.01
Max DL Deflection L/999 = 0.00		
Long term deflection factor = 1.50		

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be handled with particular care during handling and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until all fastening and bracing is completed.

PRECAUTIONARY NOTES
All bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be handled with particular care during handling and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until all fastening and bracing is completed.



Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chulueta, FL 32766

Design: Matrix Analysis

EXCEPT AS SHOWN PLATES ARE Mitek MT20
WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HUB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719)
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenport, NC 27932

Eng Job:	WO: SUNL3
Dwg:	TI: RG3
Dsgnr: TLY	Chk: 6/6/2006
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	2.0 psf
TOTAL	35.0 psf
Lbr DF: 1.25	
Plt DF: 1.25	
O.C.: 2- 0- 0	
TPI-02/FBC-04	
Code: FLA	
V4.7.32-53810	

responsibility or exercises no control with regard to the responsibility, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/UPA 1-1995 and is not to be incorporated as part of the building design (ANSI-90-7) to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the design shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special requirements for the building. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes unbraced connection chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where connection chords in tension are not fully braced by a companion chord in tension, the chord must be braced by a companion chord of 100% C. Connector plates shall be designed in accordance with AISC 308. Connector plates are not recommended from 20 gauge but dipped galvanized steel, minimum ASTM A 653, 80 unspliced galvanneal steel.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted members. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slot (holes) are parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/TP1

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and bundling, delivery and installation to avoid damage. Temporary permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and buckling. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until all fastening and bracing is completed.

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
67 Medallion Pl. Chuluota, FL 32766

Assignment: Matrix Analysis

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) For specific truss bracing requirements, contact Building Designer (Truss Plate Design, TPD) is located at 3831 Thompson Avenue, Westborough, MA 01581, Tel. 508/336-5719.

COMPONENT FABRICATING BY: Toms Engineering, Inc. P.O. Box 818, Southside Rd. Edison, NJ 07032.

Component Engineering by Truss Engineering Co., P. A., 818 Soundside Rd. Edenton, NC 27932

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\T1.prx

Dwg:	TI: T1
------	--------

Deser: M.V. Chy: 300/2006

16	0	000	1
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IC LIVE	10.0	psi	Plt DF: 1

	TC Dead	7.0 psi
O.C.: 2-		

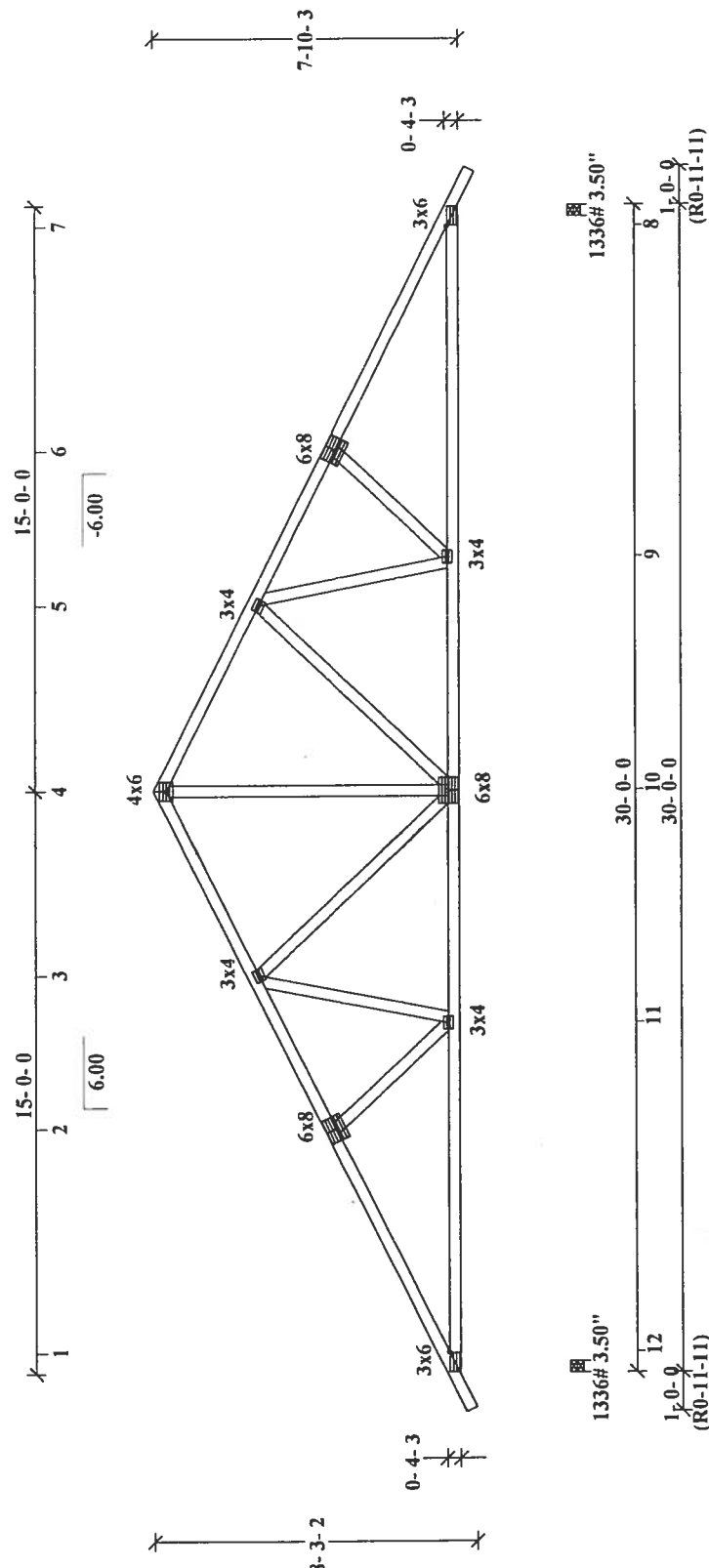
BC Live	10.0	psf
TPI-02/FBC		

BC Dead	10.0	psf	Code: FLA
---------	------	-----	-----------

TOTAL	43.0	psf	v4.7.32-40
-------	------	-----	------------

Max DL Deflection L/999 = -0.19

meeting ASTM A 653, Grade 40, unless otherwise shown.



Scale = 0.2054

Tak MT20

This truss has been designed as an individual building component in accordance with ANSI/TPI-1-1995 and NDS-97. It is incorporated as part of the building design by a building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the building code requirements, local climatic records for wind or snow loads, product specifications or special approved loads. Unless shown, truss has not been designed for storage or occupancy for any design loadings. The truss is designed to be installed bottom chord continuously supported by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to verify a continuing responsibility for each realization. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be fabricated over isobutles, knobs or distorted metal. Members shall be cut for tight fitting wood to wood bearing. Members shall be located on both faces of the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" high. All plates shall be fabricated and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Size (holes) shall be the same as the plate length specified. Double cuts on members shall meet at the center of the plate. All members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This must be not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

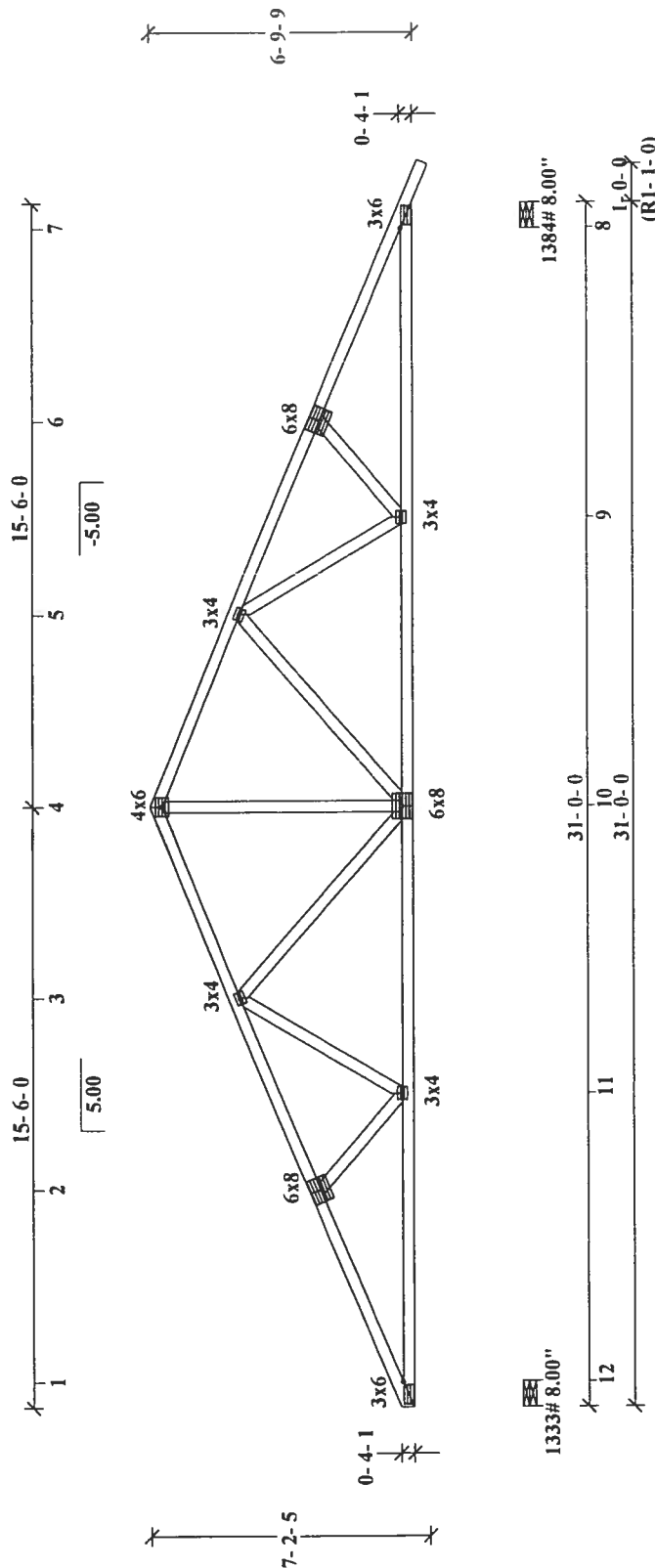
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses at top and toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Desion: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\T5A.prx

Job:	WO: SUNIL3
:	TI: T5A
War: ECH	6/6/2006
Chk:	Lbr DF: 1
	Plt DF: 1
	O.C.: 2
	TPI-02/FB
	Code: FLA
	v4.7.32-5
	AL 43.0
	16.0 psf
	Live
	Dead 7.0 psf
	ive 10.0 psf
	Dead 10.0 psf
	AL 43.0



Scale = 0.2047

MULTIPLE LOADCASES --This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1
In-Plant Quality Assurance, per
sec. 3.2.4 of TPI-1-02, for joint(s):
1,7.

FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-2634/1092,2-3=-2466/1042,
3-4=-1740/852,4-5=-1740/852,5-6=-2466/1042,
6-7=-2634/1092,
BOTTOM CHORD 12-11=-930/2384,11-10=-763/2002,
10-9=-763/2002,11-8=-930/2384,
webs
2-11=-201/180,11-5=-107/488,
3-10=-584/298,10-4=-375/1083,10-5=-584/298,
5-9=-96/488,9-6=-201/168,

I Span	in./Ratio	in./Ratio	TL	Jnt(s).
	-0.19/999	-0.38/926		9-10
Horiz.	0.06	0.12		NA
			Max DL Deflection	L/999 = -0.19
			Long term deflection factor	= 1.50

	IL	TL
I Span	in./Ratio -0.19/999	in./Ratio -0.38/926
Horiz.	0.06	0.12
	Max DL Deflection L/999 = -0.19	NA
	Long term deflection factor = 1.50	

Job: SUNBURY L 3

Customer:

WO: SUNL3

TT: V20

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

Multiple Loads -- This design is the composite result of multiple loads.

Right End vertical designed for wind.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-25-29/10,

BOT CHORD 1-3-0/0,

Webs 3-2-0/0.

-----GLOBAL MAX DEFLECTIONS-----

LL

TL

I. Span in./Ratio in./Ratio Jnt(s)

Horiz. 0.00/xxx 0.00/xxx 3-1

0.00 0.00 NA

Max DL Deflection L/xxx = 0.00

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C&C, V= 125mph,

B= 21.0 ft, I= 1.00, Exp Cat. B, Kzt= 1.0

Bld Type= encl L= 2.0 ft W= 2.0 ft Truss

in END zone, TCDL= 4.2 psf, BCDL= 1.2 psf

Impact Resistant Covering and/or Glazing Req

..TC.. FORCE..CSI.. Cr ..BC.. FORCE..CSI.

1- 2 -29 0.03 A 1- 3 0 0.02

3- 2 -2 0 0.01 C

Joint Locations

1) 0- 0- 0 3) 2- 0- 0

2) 2- 0- 0 4) 0- 0- 0

In-Plant Quality Assurance with Cg= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 37 36 -75 B Pin

1- 8- 0 1 18 -16 -14 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..WEB.. FORCE..CSI.. Cr ..WEB.. FORCE..CSI.. Cr

3- 2 -2 0 0.01 C

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to the information provided by the client.

This truss has been designed with individual building components in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by the building designer. When required for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the design and specifications and that the truss is suitable for the intended application. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during building and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

Diagram 1: Top view of a truss section showing dimensions 2-0-0, 1, 2, 5.00, and 0-10-0.

Diagram 2: Side view of a truss section showing dimensions 2x4, 3x4, 0-10-0, and 2x4.

Diagram 3: Detail view of a truss section showing dimensions 37# 8.00", Stud @ 2-0-0, 18# 8.00", 2-0-0, 4, 3.

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.9004

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing. Wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Ondrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A. 818 Soundside Rd. Edenon, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3513

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Eng Job:

Dwg:

Dsgnr: ECH

Chk:

WO: SUNL3

TI: V20

6/6/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

TOTAL 35.0 psf

47.32-46888

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V20.prx

Design: Matrix Analysis
Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V26.prx

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: V28

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or cures no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSITR 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to the local building codes, local climate in agreement with the local building codes, local climate in agreement with wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

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PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during erection and handling. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuous braced unless noted otherwise.

Windload per ASCE 7-02, C&C, V= 125mph, B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 11.0 ft W= 11.0 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 1.2 psf

Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=36/109, 2-3=30/72, 3-4=42/72, 4-5=89/71

BOT CHORD 1-9=0/0, 9-8=0/0, 8-7=0/0, 7-5=0/0, 5-4=0/0, 4-3=124/255, 3-2=124/255, 2-1=124/255, 1-0=124/255

WEBS 9-2=124/255, 8-3=71/93, 7-4=124/255

GLOBAL MAX DEFLECTIONS-----

IL TL

I. Span in./Ratio in./Ratio Jnt(s) 5-4

Horiz. 0.01/999 0.01/999 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Joint Locations

1)	0-0-0	5)	11-0-0	9)	3-6-0
2)	3-6-0	6)	11-0-0	10)	0-0-0
3)	5-6-0	7)	7-6-0		
4)	7-6-0	8)	5-6-0		

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0-4-0 1 85 -101 -88 B Pin

10-8-0 1 85 -99 -88 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB...FORCE. CSI. Cr .WEB...FORCE. CSI. Cr

9-2 255 0.06 C 7-4 255 0.06 C

8-3 93 0.17 C

Diagram

Scale = 0.4383

EXCEPT AS SHOWN PLATES ARE Mitek MT20

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V28.prx

Eng Job:

WO: SUNL3

Dwg:

TI: V28

Dsgn:

ECH Chk:

6/6/2006

TC Live

16.0 psf

TC Dead

7.0 psf

BC Live

10.0 psf

BC Dead

2.0 psf

TOTAL

35.0 psf

Scale = 0.4383

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Job: SUNBURY I. 3

Customer: WO: SUNL3

TT: V29

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-46/135, 2-3=-66/47,

BOT CHORD 1-5=0/0, 5-3=0/0,

WEBS 5-2=-131/256,

WEB...FORCE..CSI..Cr

5-2 256 0.12 C

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02. C6C, V= 125mph, B= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl L= 7.0 ft W= 7.0 ft truss In END zone, TCDF= 4.2 psf, BCDI= 1.2 psf Impact Resistant Covering and/or Glazing Req

..TC...FORCE..CSI..Cr...BC...FORCE..CSI..1-2 135 0.24 A 1-5 0 0.06 2-3 -66 0.14 A 5-3 0 0.06

-----GLOBAL MAX DEFLECTIONS-----

LL TL in./Ratio in./Ratio Jnt(s).

I. Span 0.01/999 0.01/999 3-2

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

1) 0-0-0 3) 7-0-0 5) 3-6-0

2) 3-6-0 4) 7-0-0 6) 0-0-0

Wind loads have been checked perpendicular to the face of the truss.

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 85 -125 -118 B Pin

6-8-0 1 85 -119 -118 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5' wide x 4" long. A 6x8 plate is 6' wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and downrailing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITAK MT20

Scale = 0.5928

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).

Component Engineering by Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Eng Job:

Dwg:

Dsgnr: ECH

Chk:

WO: SUNL3

TI: V29

6/6/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

V4.7.32-53793

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY I. 3\V29.prx

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: V30

Qty: 1

DESIGN INFORMATION

his design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages to the building or its contents resulting from the use of the design information furnished to the designer by the client or the client's representative.

---MAX. REACTIONS PER BEARING LOCATION---

X-LOC	BSET	VART	HORIZ	UPLIFT	Y	Type
0-4-0	1	1	-12	-5	B	Pin
2-8-0	1	0	0	0	B	Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C&C, V=125mph, B=21.0 ft, I=1.00, Exp.Cat. B, Kzt=1.0

Bld Type= encl L= 3.0 ft W= 3.0 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 1.2 psf Impact Resistant Covering and/or Glazing Req .TC...FORCE...CSI. Cr ..BC...FORCE...CSI.

1-2 -6 0.00 A 3-3 0 0.00

3-2 0 0.00 A

Joint Locations

1)	0-0-0	3)	3-0-0
2) <td>1-6-0</td> <td>4)<td>0-0-0</td></td>	1-6-0	4) <td>0-0-0</td>	0-0-0

Wind loads have been checked perpendicular to the face of the truss.

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

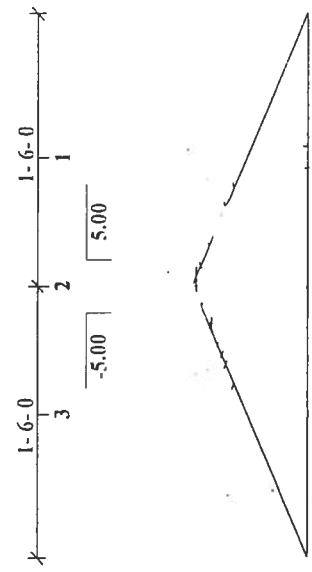
TOP CHORD 1-2=-6/0, 3-2=0/0,

BOT CHORD 3-3=0/0,

-----GLOBAL MAX DEFLECTIONS-----

LL	TL
in./Ratio	in./Ratio
I. Span 0.00/999	0.00/999
Horiz. 0.00	0.00
Max DL Deflection L/999 = 0.00	NA
Long term deflection factor = 1.50	

0-7-8

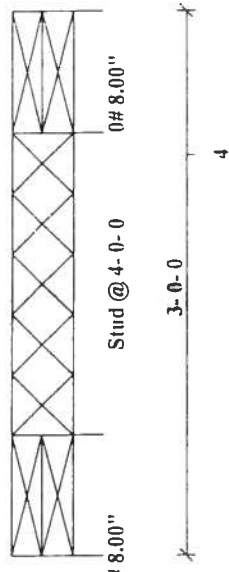


1-6-0 3 2 1

-5.00 5.00

0-7-8

0-7-8



Stud @ 4-0-0 0# 8.00"

3-0-0 4

0-7-8

0-7-8



0-7-8

0-7-8

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.9360

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Ondra Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27912

Maronda Systems

4005 MARONDA WAY

Saunder, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V30.prx

FABRICATION NOTES

rior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood joints. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 3" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) in parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fine random treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIT/PTI-1995.

RECAUTIONARY NOTES

ut bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and bottoming. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

Job: SUNBURY L 3

Customer: WO: SUNL3

TI: V6

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

MULTIPLE LOADS -- This design is the composite result of multiple loads.

Windload per ASCE 7-02, C&C, V= 125mph, R= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl L= 8.0 ft W= 8.0 ft Truss in END zone, TCDFL= 4.2 psf, BCDFL= 1.2 psf

Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s).

I. Span 0.03/999 0.02/999 3-2

Horiz. 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-69/190, 2-3=-135/66,

BOT CHORD 1-5=0/0.5-3=0/0,

Webs 5-2=-149/266, .TC...FORCE..CSI..

1- 2 190 0.42 A 1- 5 0 0.09

2- 3 -135 0.25 A 5- 3 0 0.09

Joint Locations

1) 0- 0- 0 3) 8- 0- 0 5) 4- 0- 0

2) 4- 0- 0 4) 8- 0- 0 6) 0- 0- 0

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 4- 0 1 108 -170 -138 B Pin

7- 8- 0 1 108 -170 -138 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB...FORCE..CSI.. Cr .WEB...FORCE..CSI.. Cr

5- 2 266 0.25 C

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by the building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

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PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.6773

WARNING:

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Component Engineering by: Truss Engineering Co., P. A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V6.prx

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-69/190, 2-3=-135/66,

BOT CHORD 1-5=0/0.5-3=0/0,

Webs 5-2=-149/266, .TC...FORCE..CSI..

1- 2 190 0.42 A 1- 5 0 0.09

2- 3 -135 0.25 A 5- 3 0 0.09

Joint Locations

1) 0- 0- 0 3) 8- 0- 0 5) 4- 0- 0

2) 4- 0- 0 4) 8- 0- 0 6) 0- 0- 0

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 4- 0 1 108 -170 -138 B Pin

7- 8- 0 1 108 -170 -138 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB...FORCE..CSI.. Cr .WEB...FORCE..CSI.. Cr

5- 2 266 0.25 C

DESIGN INFORMATION

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PRECAUTIONARY NOTES

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EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.6773

WARNING:

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Component Engineering by: Truss Engineering Co., P. A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNL3\SUNBURY L 3\V6.prx

	Joint Locations				
1)	0-1-8	14)	21-10-12	27)	19-9-4
2)	1-1-0	15)	22-2-8	28)	16-7-8
3)	1-9-8	16)	24-0-0	29)	14-8-0
4)	3-8-8	17)	25-1-8	30)	14-0-0
5)	6-0-2	18)	25-5-4	31)	11-4-8
6)	8-1-10	19)	27-5-8	32)	9-6-8
7)	9-1-8	20)	27-10-8	33)	8-1-10
8)	10-1-8	21)	29-10-8	34)	6-0-2
9)	12-7-8	22)	29-10-8	35)	3-0-8
10)	14-0-0	23)	27-10-8	36)	1-1-0

	PLF	From	PLF	To
Uniform	-100	0-0-0	-100	29-9-0
BC Vest I+D	-10	0-0-0	-10	29-9-0
Concentrated	-1347	29-7-8		
TC Vest I+D				

4x6
 x3
 3x6 W=3x6
 [6x12]
 +[8x10] 6x12
 0-9-4
 [3x8]
 +[3x4]
 3x4
 x6

Over 3 Supports

Eng Job: SUNF	WO: SUNF
Dwg:	TI: FB
Dsgnr: TLY	5/11/2007
TC Live 40.0 psf	Lbr DF: 1.00
BC Dead 10.0 psf	Plt DF: 1.00
BC Live 0.0 psf	O.C.: 2-0-0
BC Dead 5.0 psf	FBC-04/TPI-02/NDS-01
TOTAL 55.0 psf	Code:
	v54.18-92217

Profile Path: C:\TEE-LOK\Work\Jobs\SUNF\SUNBURY FLOOR\SUNF\FB.prx

DESIGN INFORMATION

[illegible]

ABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the applicable code and standards. The fabricator shall be responsible for verifying the dimensions and tolerances of the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be noted in writing before cutting or fabrication. Plates are not to be installed over knotholes, knots or distorted wood. Members shall be cut for tight fitting wood to wood joints. Connector plates shall be located on both faces of the joints with nails fully imbedded and shall be 3" wide x 8' long unless otherwise shown. A 3/4 plate shall be 3" wide x 8' long. A 6/8 plate is 6" wide x 8' long. Double cut on members shall meet at the center of the webs unless otherwise shown. Connector plate sizes are minimum based on the forces shown and may need to be increased for certain loading, and/or erection stresses. All truss is not to be fabricated with fire retardant treated wood unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-98.

RECAUTIONARY NOTES

bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be handled with particular care during loading and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and trussing is always required. Normal precautionary bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and overturning. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought and concentrated concentration of construction loads greater than design loads shall not be applied to trusses at any time. Design loads other than the weight of the erector shall be applied to trusses until after all fastening and bracing is completed.

TC: 4x 2 SP #2
 BNC: 4x 2 SP #1 D
 WB: (2) 4x 2 SP #3
 4x 2 SP #3

2-17, 2-16, 3-16
 3-15, 4-15, 4-14, 5-13, 6-13, 6-12, 7-12
 7-11, 8-11, 8-10, 9-10, 0-0

Unbraced load case(s) have been checked.
 MULTIPLE LOADCASES -- This design is the
 composite result of multiple loadcases.
 All COMPRESSOR Chords are assumed to be
 continuously braced unless noted otherwise.

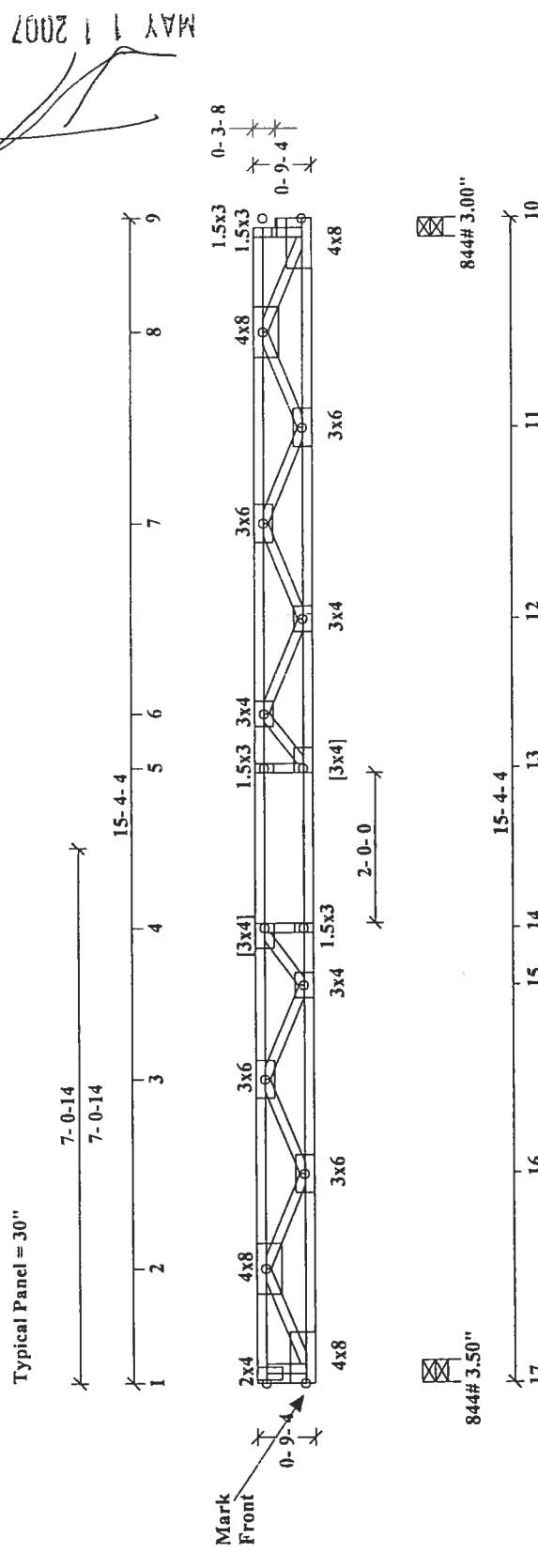
	in./Ratio	in./Ratio	TL	Jnt(s).
I.Span	-0.40/443	-0.63/283		13
Horiz.	0.05	0.07		NA
				Long term deflection factor = 1.50

[[] or []: Plate(s) OFFSET from joint center.
The Joint Detail Report must be included
with any submittal, inspection, and/or
fabrication documentation.

This truss is designed in accordance with
the 2004 Florida Building Code section
2306.1 and referenced standards, U.N.O.
In-Plant Quality Assurance with Cq = 1.11
In-Plant Quality Assurance, per
sec. 3.2.4 of TPI-1-02, for joint(s):
2, 3, 7, 8, 11, 15, 16

-----MAX. REACTIONS PER BEARING LOCATION-----						
X-X-Loc	BSet	Vert	Horiz	Uplift	Y	Type
0- 1-12	1	844	0	0	B	Pin
15- 2-12	1	844	0	0	B	H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE



EXCEPDM IS SHOWN PLATES ARE MITEK MT20

Scale = 0.4144

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE SELECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, gable bracing or similar bracing which is a part of the building structure. It is intended to be used as a temporary bracing system for the building during construction. The members only to resist buckling length. Provisions must be made to anchor lateral bracing at ends and specified intermediate locations determined by the building designer. Additional bracing of the overall structure may be required. (See HEB-91 for TPI) for specific rules bracing requirements. Contact building designer (T'ruan Pincus Institute, TPI) is located at 343 Connecticut Expressway, P.O. Box 818, South Plainfield, NJ 07080. (Tel. 201-791-1100).

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
OMAS PONCE P.E. LICENSE #0050068
67 Medallion Pl, Chuluota, FL 32766

Eng Job: SUNF

1

Dwg: Dsgnr: TLY Chk:

TC Live	40.0	psf
TC Dead	10.0	psf
BC Live	0.0	psf
BC Dead	5.0	psf
TOTAL	55.0	psf

WO: SUNF

U.S. DEPT. OF AGRICULTURE

11: FC
5/11/200

ULbr DF: 1.00
PLPt DF: 1.00
O.C.: 2- 0- 0
FFBC-04/TPI-02
Code:
v5.4.18-92238

