

Project Information for:		L159594	
Builder:	Donald Shea O/B	Date:	4/14/2006
Lot:	N/A	Start Number:	1172
Subdivision:	888 SE Race Track Ln		
County or City:	Columbia County		
Truss Page Count:	37		

Truss Design Load Information (UNO)		Design Program: MiTek 5.2 / 6.2	
Gravity	Wind	Building Code: FBC2004	
Roof (psf): 42	Wind Standard: ASCE 7-02		
Floor (psf): 55	Wind Speed (mph): 110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)	
Owner Builder	
Address: N/A	Designer: 105
N/A	

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address: 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

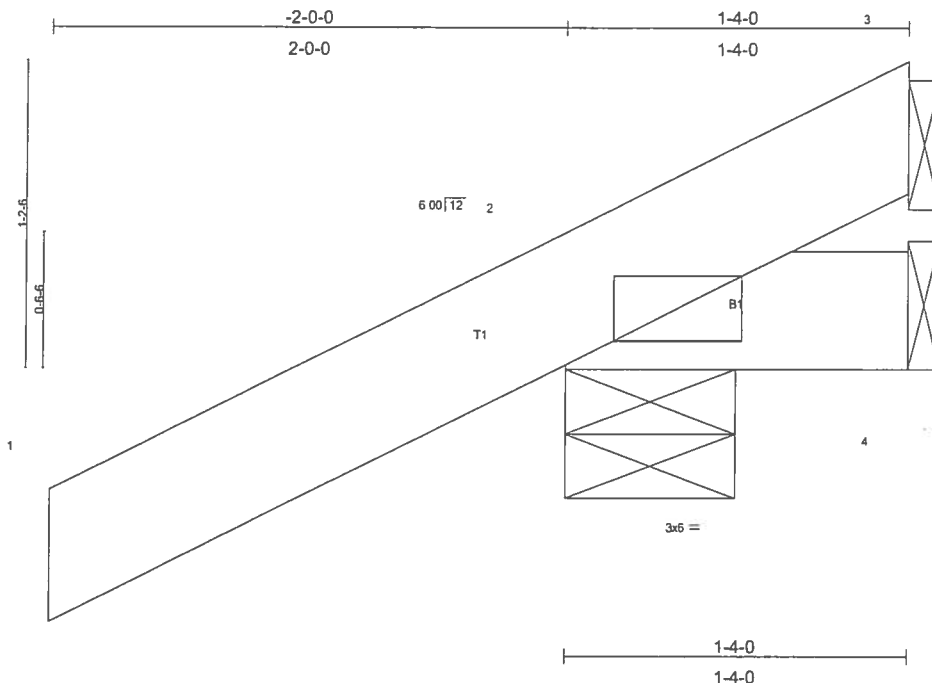
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for vertical loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0414061172	4/14/2006				
2	CJ2	0414061173	4/14/2006				
3	CJ4	0414061174	4/14/2006				
4	CJ5	0414061175	4/14/2006				
5	EJ6	0414061176	4/14/2006				
6	EJ6A	0414061177	4/14/2006				
7	EJ6B	0414061178	4/14/2006				
8	HJ9	0414061179	4/14/2006				
9	T01	0414061180	4/14/2006				
10	T01G	0414061181	4/14/2006				
11	T02	0414061182	4/14/2006				
12	T03	0414061183	4/14/2006				
13	T03G	0414061184	4/14/2006				
14	T05	0414061185	4/14/2006				
15	T05	0414061186	4/14/2006				
16	T06	0414061187	4/14/2006				
17	T07	0414061188	4/14/2006				
18	T08	0414061189	4/14/2006				
19	T09	0414061190	4/14/2006				
20	T10	0414061191	4/14/2006				
21	T11	0414061192	4/14/2006				
22	T12	0414061193	4/14/2006				
23	T13	0414061194	4/14/2006				
24	T14	0414061195	4/14/2006				
25	T15	0414061196	4/14/2006				
26	T16	0414061197	4/14/2006				
27	T16	0414061198	4/14/2006				
28	T17	0414061199	4/14/2006				
29	T18	0414061200	4/14/2006				
30	T19	0414061201	4/14/2006				
31	T20	0414061202	4/14/2006				
32	T21	0414061203	4/14/2006				
33	T22	0414061204	4/14/2006				
34	T23	0414061205	4/14/2006				
35	T24	0414061206	4/14/2006				
36	T25	0414061207	4/14/2006				
37	T26	0414061208	4/14/2006				

APR 14 2006

Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	CJ1	MONO TRUSS	8	1	Job Reference (optional)

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Scale = 1.85

LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.07	Vert(LL) -0.00 2 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.00	Vert(TL) -0.00 2 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 12 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 1-4-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=171/0-8-0, 4=13/Mechanical, 3=-39/Mechanical
Max Horz 2=63(load case 5)
Max Uplift2=-164(load case 5), 3=-39(load case 1)
Max Grav 2=171(load case 1), 4=13(load case 1), 3=57(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/31, 2-3=-41/33
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 164 lb uplift at joint 2 and 39 lb uplift at joint 3.

LOAD CASE(S) Standard

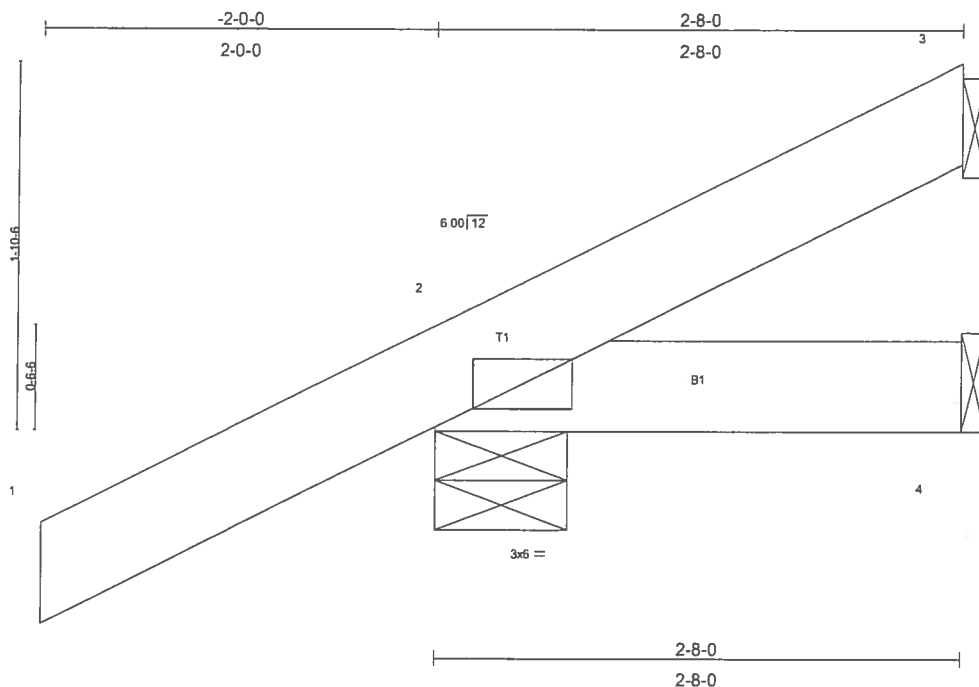
**APRIL 14, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Job L159594	Truss CJ2	Truss Type MONO TRUSS	Qty 8	Ply 1	DONALD SHEA - O/B
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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Scale = 1/11.2

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.08	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 19 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D

BOT CHORD 2 X 6 SYP No.1D

BRACING

TOP CHORD

Structural wood sheathing directly applied or 2-8-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-2/Mechanical, 2=191/0-8-0, 4=23/Mechanical

Max Horz 2=83(load case 5)

Max Uplift 3=-9(load case 6), 2=-153(load case 5)

Max Grav 3=16(load case 3), 2=191(load case 1), 4=23(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-43/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.09

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 3 and 153 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	CJ4	MONO TRUSS	8	1	Job Reference (optional)

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LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates increase 1.25	TC 0.08	Vert(LL) -0.00 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber increase 1.25	BC 0.02	Vert(TL) -0.00 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 26 lb	

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/32, 2-3=45/12
 BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

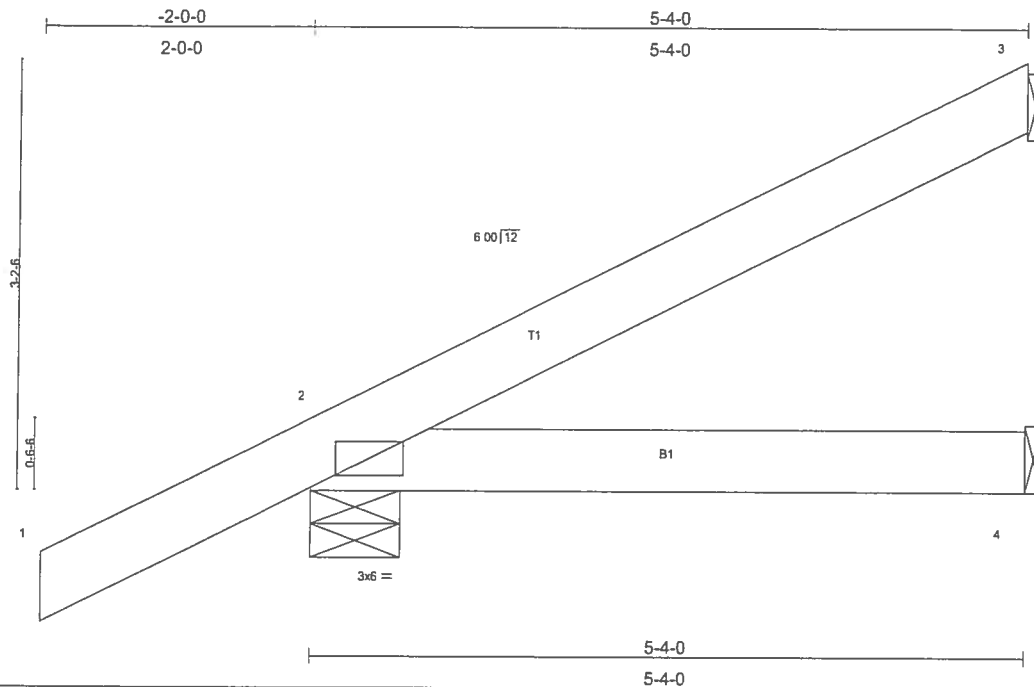
2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

Job L159594	Truss CJ5	Truss Type MONO TRUSS	Qty 8	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32056			Job Reference (optional)		

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LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.08	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.04	Vert(TL)	-0.01	2-4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 32 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=69/Mechanical, 2=242/0-8-0, 4=49/Mechanical
Max Horz 2=123(load case 5)
Max Uplift 3=60(load case 5), 2=142(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=62/24
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.11

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 3 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	EJ6	MONO TRUSS	38	1	Job Reference (optional)

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LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.09	Vert(LL) -0.01 2-4 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL) -0.02 2-4 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)			Weight: 39 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

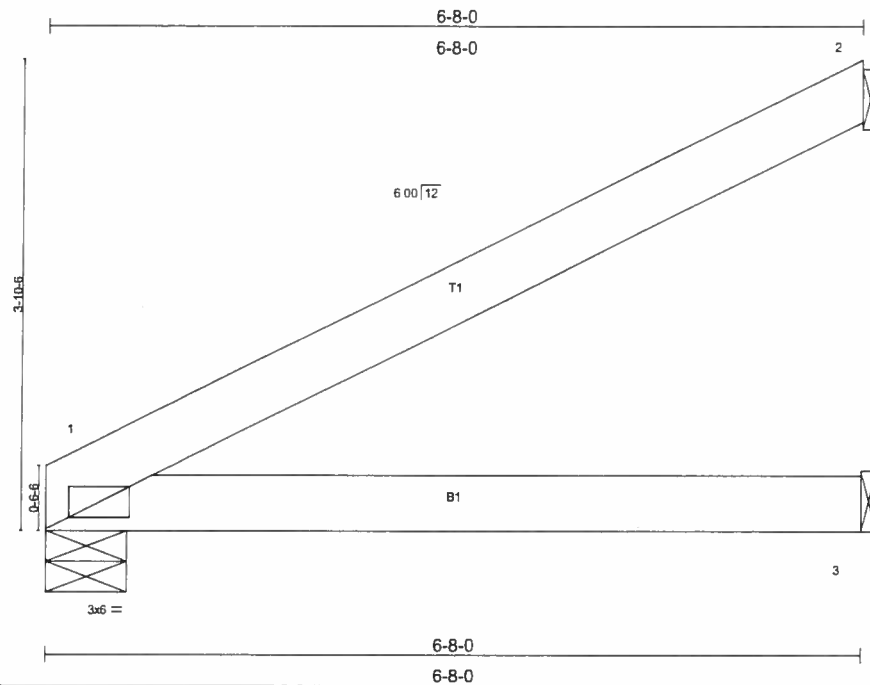
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-80/35
BOT CHORD 2-4=0/0

NOTES

LOAD CASE(S) Standard

APRIL 14, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L159594	Truss EJ6A	Truss Type MONO TRUSS	Qty 17	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:14:46 2006 Page 1		



Scale = 1/18 0

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.12	Vert(LL)	-0.01	1-3	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(TL)	-0.02	1-3	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 34 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=176/0-8-0, 2=113/Mechanical, 3=63/Mechanical
Max Horz 1=103(load case 5)
Max Uplift 1=-39(load case 5), 2=-106(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-90/43
BOT CHORD 1-3=0/0

JOINT STRESS INDEX

1 = 0.06

NOTES

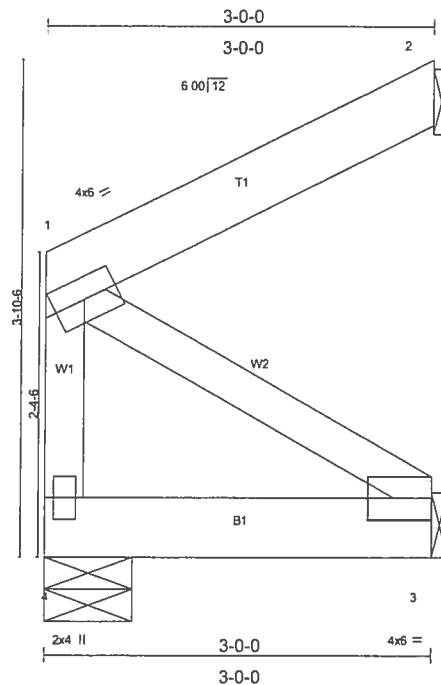
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 39 lb uplift at joint 1 and 106 lb uplift at joint 2.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	EJ6B	MONO TRUSS	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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Scale = 1/17.1

Plate Offsets (X,Y): [3:Edge,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.04	Vert(LL)	-0.00	4	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(TL)	-0.00	4	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.03	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 22 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=50/Mechanical, 4=79/0-8-0, 3=29/Mechanical
 Max Horz 4=73(load case 5)
 Max Uplift 2=46(load case 5), 3=34(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-39/18, 1-4=-50/1
 BOT CHORD 3-4=-95/0
 WEBS 1-3=-0/115

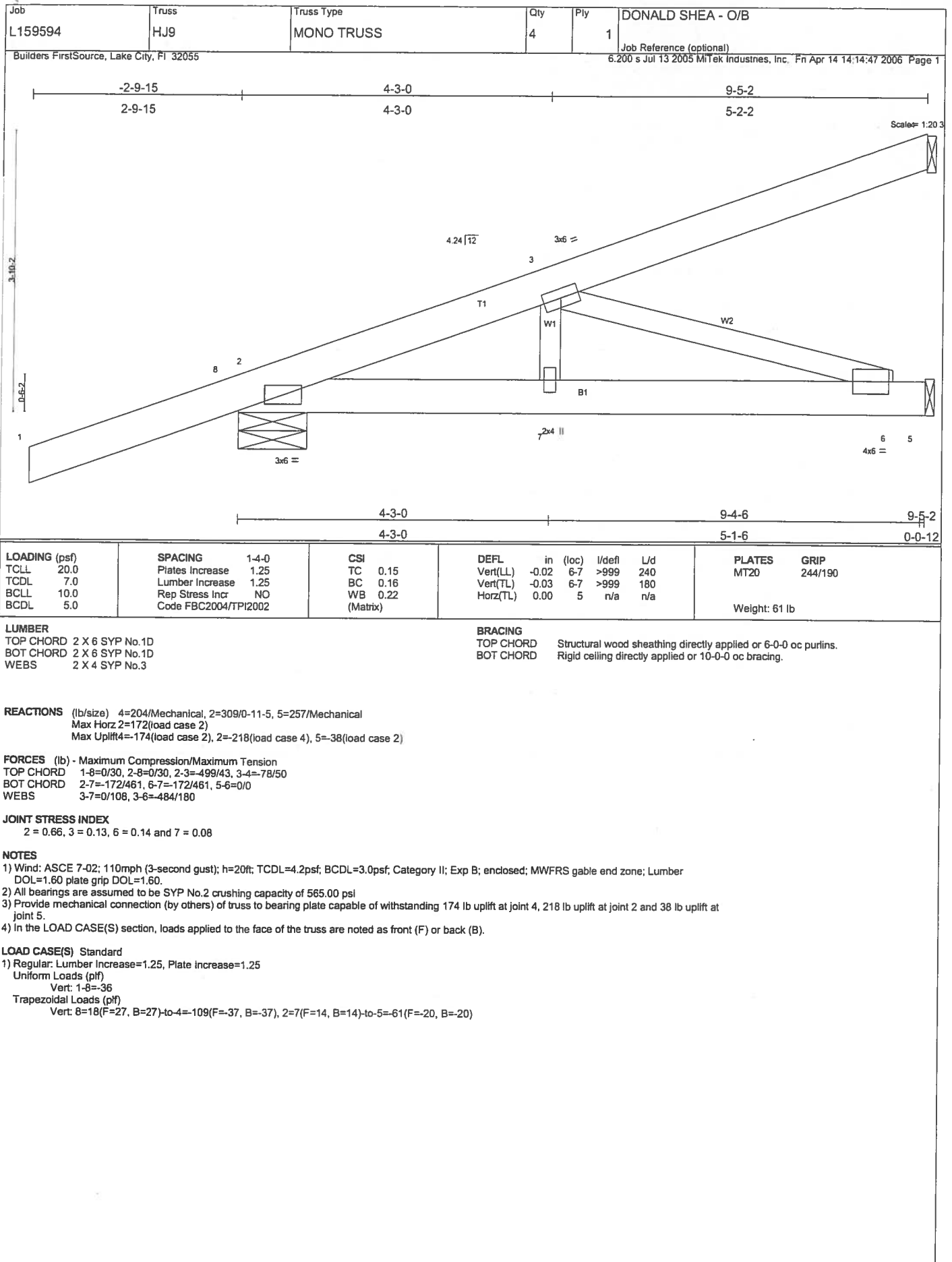
JOINT STRESS INDEX

1 = 0.04, 3 = 0.03 and 4 = 0.03

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 2 and 34 lb uplift at joint 3.

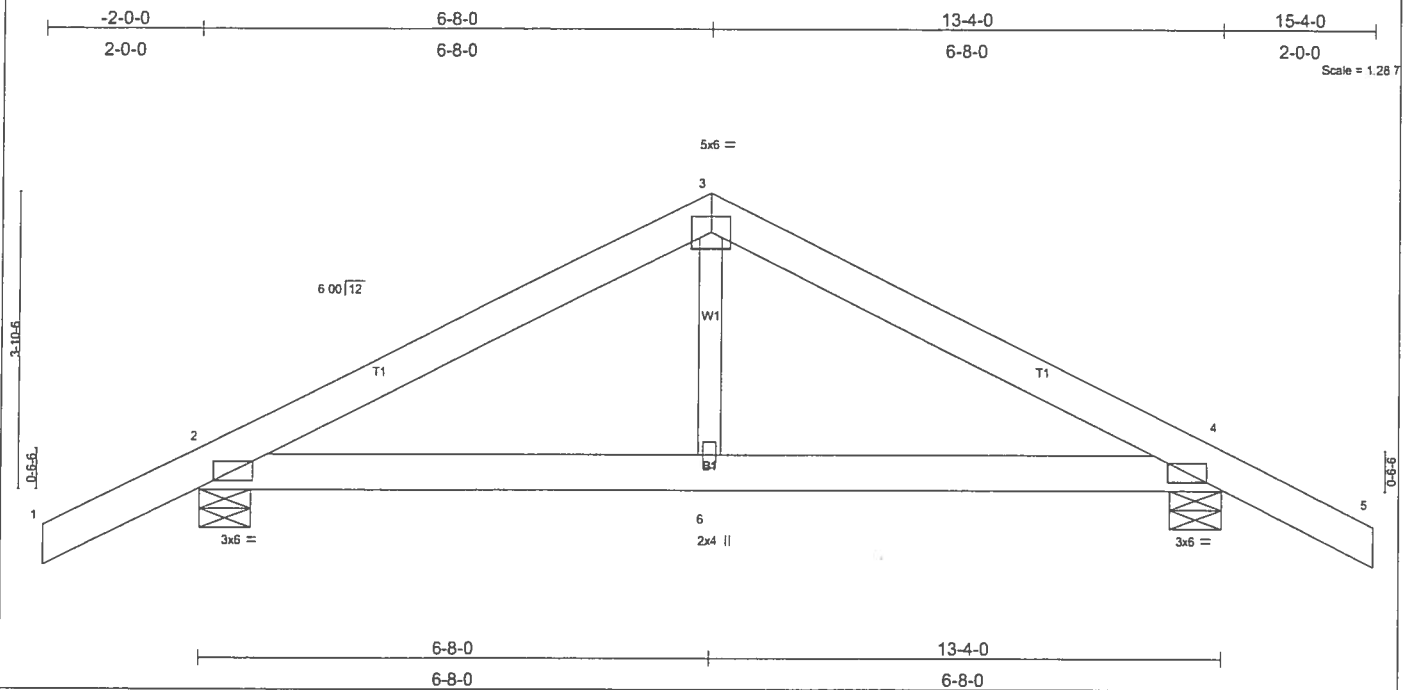
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	T01	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.09	Vert(LL) 0.02 2-6 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL) 0.02 2-6 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.05	Horz(TL) 0.00 4 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 82 lb	

LUMBER
 TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=439/0-8-0, 4=439/0-8-0
Max Horiz 2=-56(load case 6)
Max Uplift 2=-315(load case 5), 4=-315(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-462/496, 3-4=-462/496, 4-5=0/32
BOT CHORD 2-6=-321/364, 4-6=-321/364
WEBS 3-6=-246/143

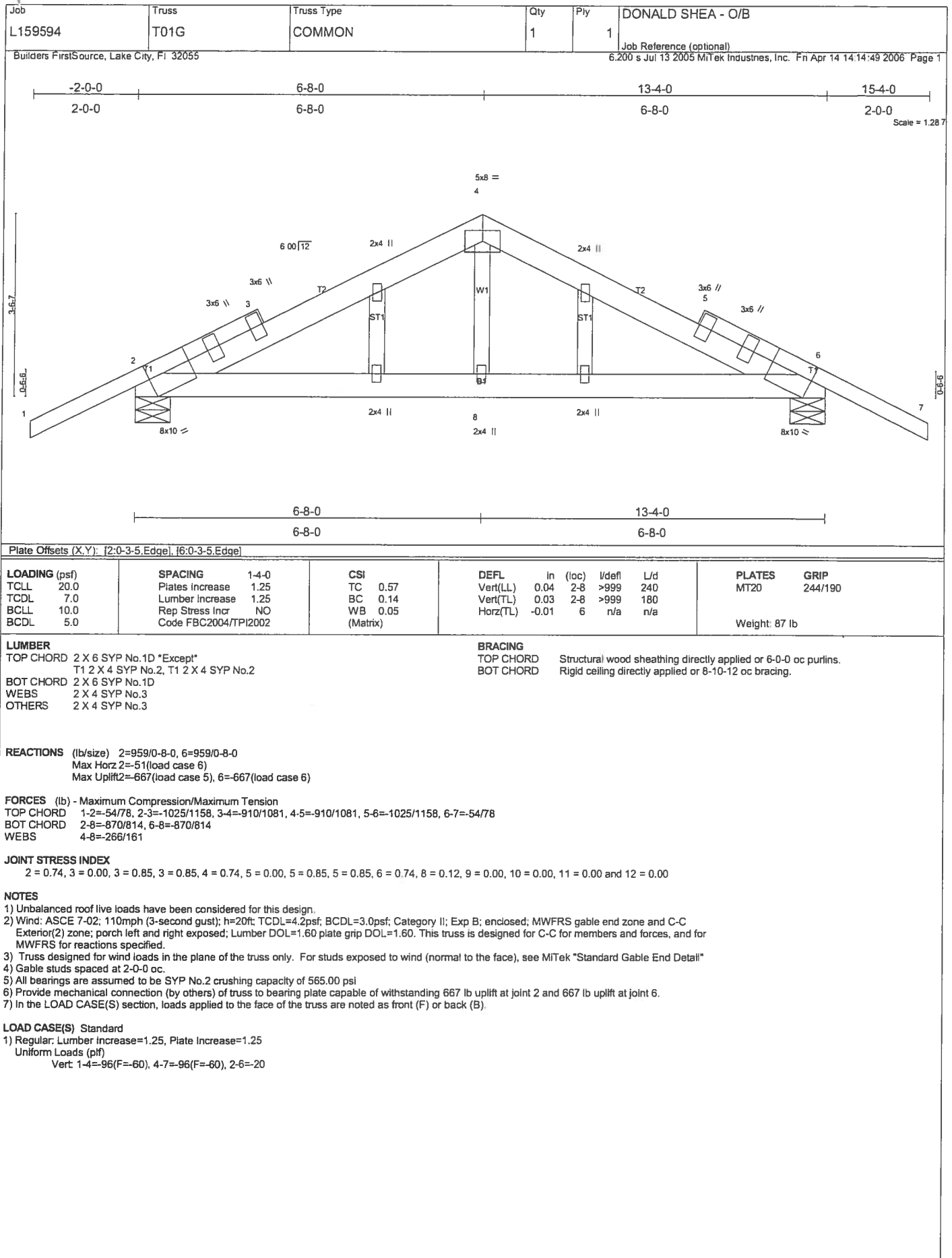
JOINT STRESS INDEX
2 = 0.46, 3 = 0.47, 4 = 0.46 and 6 = 0.10

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C exterior (2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2 and 315 lb uplift at joint 4.

LOAD CASE(S) Standard

APRIL 14, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549



Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	T02	COMMON	3	1	Job Reference (optional)

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LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.19	Vert(LL) 0.01 5-6 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.02	Vert(TL) 0.01 5-6 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.05	Horz(TL) -0.00 4 n/a n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 66 lb	

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-253/314, 2-3=-253/314, 1-6=-206/243, 3-4=-206/243
BOT CHORD 5-6=-75/68, 4-5=-72/68
WEBS 2-5=-107/42, 1-5=-132/133, 3-5=-132/133

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Wind: (2) zone; end vertical left and right exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 6 and 155 lb uplift at joint 4.

LOAD CASE(S) Standard

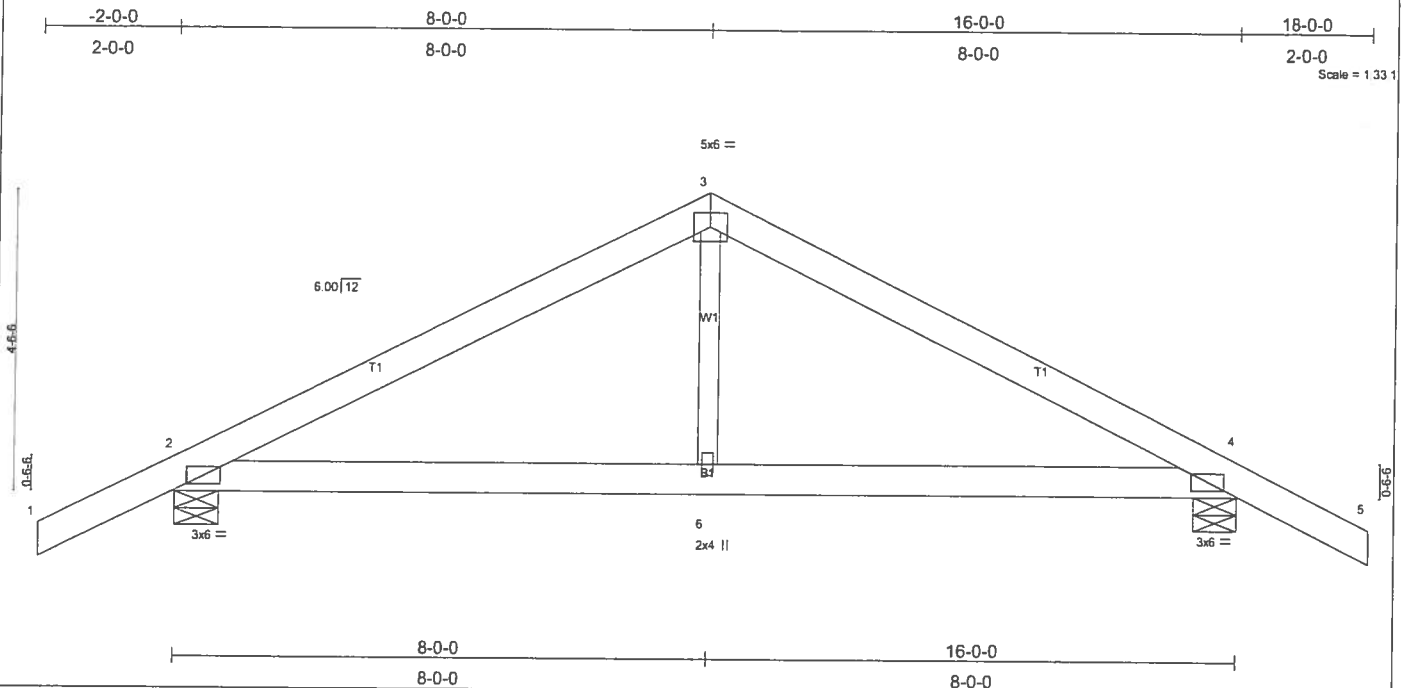
APRIL 14, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Job L159594	Truss T03	Truss Type COMMON	Qty 6	Ply 1	DONALD SHEA - O/B
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(LL) 0.04 2-6 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.08	Vert(TL) 0.03 2-6 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.01 4 n/a n/a		
	Code FBC2004/TPI2002			Weight: 96 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=513/0-8-0, 4=513/0-8-0
 Max Horz 2=-63(load case 6)
 Max Uplift 2=-360(load case 5), 4=-360(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-583/612, 3-4=-583/612, 4-5=0/32
 BOT CHORD 2-6=-418/465, 4-6=-418/465
 WEBS 3-6=-309/185

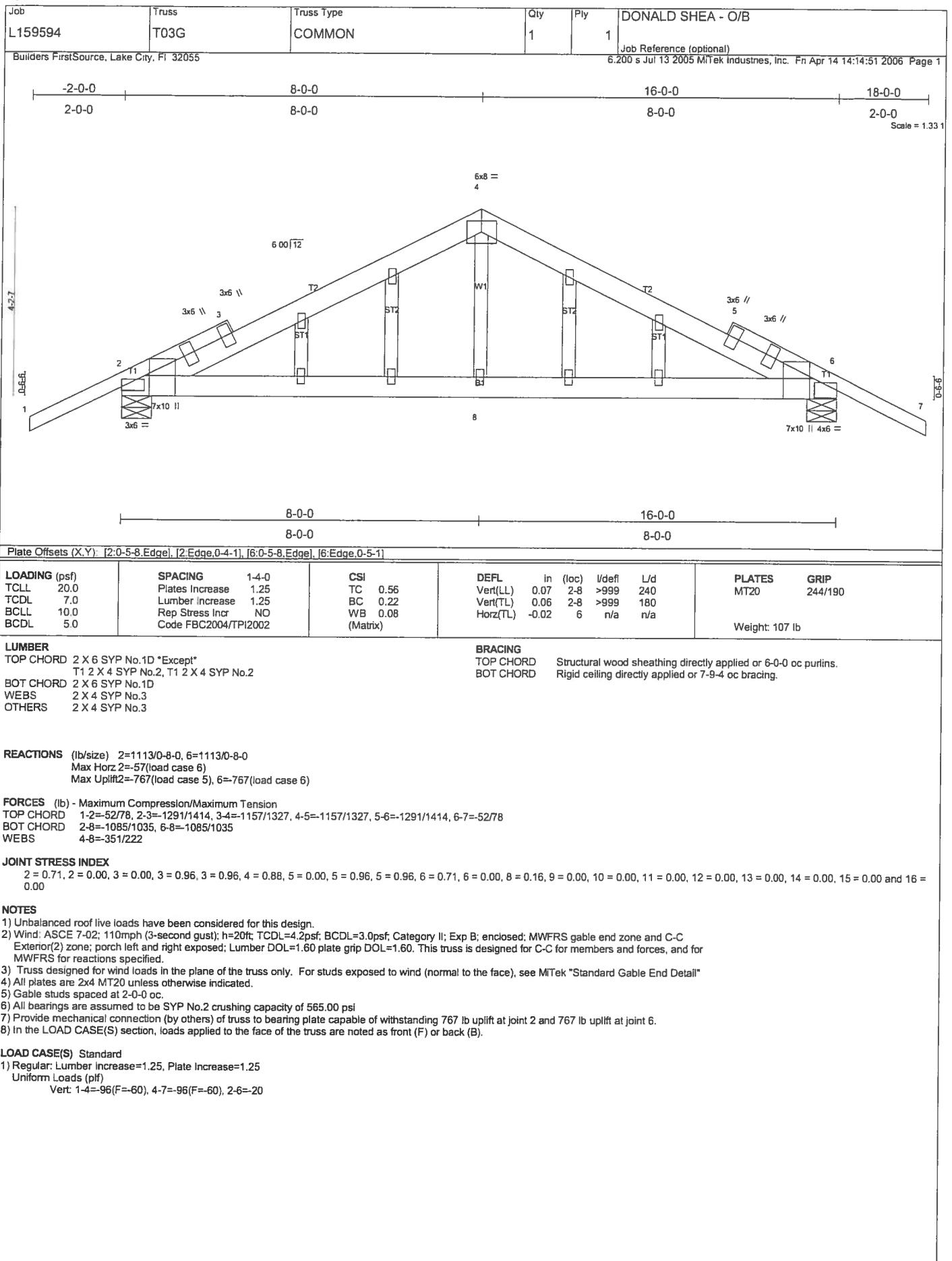
JOINT STRESS INDEX

2 = 0.46, 3 = 0.68, 4 = 0.46 and 6 = 0.13

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 360 lb uplift at joint 2 and 360 lb uplift at joint 4.

LOAD CASE(S) Standard



Job L159594	Truss T05	Truss Type HIP	Qty 1	Ply 2	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MITek Industries, Inc. Fri Apr 14 14:14:52 2006 Page 1		

2-0-0	6-8-0	12-2-15	16-8-11	22-0-0	26-4-0	30-8-0	35-11-5	40-5-1	46-0-0	52-8-0	54-8-0
2-0-0	6-8-0	5-6-15	4-5-12	5-3-5	4-4-0	4-4-0	5-3-5	4-5-12	5-6-15	6-8-0	2-0-0
Scale = 1/94.8											

6-8-0	9-8-0	14-9-14	22-0-0	26-4-0	30-8-0	37-10-2	43-0-0	46-0-0	52-8-0
6-8-0	3-0-0	5-1-14	7-2-2	4-4-0	4-4-0	7-2-2	5-1-14	3-0-0	6-8-0

Plate Offsets (X,Y): [7-0-5-0-0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.09	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.09	Vert(LL) 0.01 16-18 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.13	Vert(TL) -0.02 20-22 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 12 n/a n/a		
	Code FBC2004/TP12002			Weight: 723 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
6-0-0 oc bracing: 19-20,18-19.

REACTIONS (lb/size) 2=399/0-8-0, 23=2002/0-8-0, 20=1395/0-8-0, 18=1395/0-8-0, 15=2002/0-8-0, 12=399/0-8-0
Max Horz 2=57/(load case 5)
Max Uplift 2=238/(load case 4), 23=781/(load case 3), 20=530/(load case 2), 18=530/(load case 3), 15=776/(load case 2), 12=249/(load case 5)
Max Grav 2=399/(load case 8), 23=2002/(load case 8), 20=1400/(load case 9), 18=1400/(load case 8), 15=2002/(load case 9), 12=399/(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-3=-363/188, 3-4=-149/453, 4-5=-465/204, 5-6=-60/213, 6-7=0/58, 7-8=0/58, 8-9=-60/213, 9-10=-465/202, 10-11=-147/453,
11-12=-363/192, 12-13=0/32
BOT CHORD 2-24=-105/284, 23-24=-115/310, 22-23=-64/217, 21-22=-160/467, 20-21=-160/467, 19-20=-213/147, 18-19=-213/147, 17-18=-161/467,
16-17=-161/467, 15-16=-71/217, 14-15=-114/310, 12-14=-103/284
WEBS 3-24=-242/606, 3-23=-1150/474, 4-23=-1091/445, 4-22=-114/422, 5-22=-13/55, 5-20=-810/325, 6-20=-598/289, 6-19=-81/197,
7-19=-203/120, 8-19=-81/197, 8-18=-598/289, 9-18=-810/325, 9-16=-13/54, 10-16=-114/422, 10-15=-1091/446, 11-15=-1150/468,
11-14=-241/606

JOINT STRESS INDEX
2 = 0.19, 3 = 0.34, 4 = 0.26, 5 = 0.29, 6 = 0.25, 7 = 0.16, 8 = 0.25, 9 = 0.29, 10 = 0.26, 11 = 0.34, 12 = 0.19, 14 = 0.34, 15 = 0.26, 16 = 0.29, 17 = 0.19, 18 = 0.25, 19 = 0.28, 20 = 0.25, 21 = 0.19, 22 = 0.29, 23 = 0.26 and 24 = 0.34

NOTES
1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
2) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
6) Provide adequate drainage to prevent water ponding.
7) All plates are 4x6 MT20 unless otherwise indicated.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 2, 781 lb uplift at joint 23, 530 lb uplift at joint 20, 530 lb uplift at joint 18, 776 lb uplift at joint 15 and 249 lb uplift at joint 12.
10) Girder carries hip end with 6-8-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2

Job L159594	Truss T05	Truss Type HIP	Qty 1	Ply 2	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.200 s Jul 13 2005 Mittek Industries, Inc. Fri Apr 14 14:14:52 2006 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-36, 3-6=-99(F=-63), 6-8=-69(F=-33), 8-11=-99(F=-63), 11-13=-36, 2-24=-20, 20-24=-55(F=-35), 18-20=-40(F=-20), 14-18=-55(F=-35), 12-14=-20

Concentrated Loads (lb)

Vert: 24=-529(F) 14=-529(F)

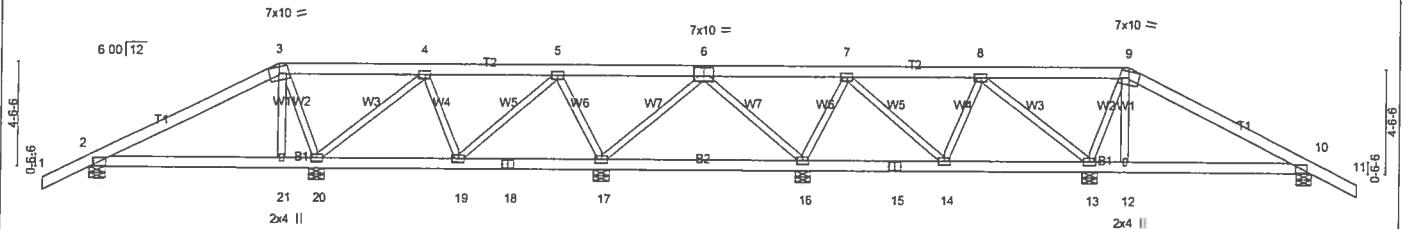
Job L159594	Truss T06	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
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2-0-0	8-0-0	14-4-4	20-1-7	26-4-0	32-6-9	38-3-12	44-8-0	52-8-0	54-8-0
2-0-0	8-0-0	6-4-4	5-9-3	6-2-9	6-2-9	5-9-3	6-4-4	8-0-0	2-0-0

Scale = 1/8" = 1'-0"



8-0-0	9-8-0	15-10-5	22-0-0	30-8-0	36-9-11	43-0-0	44-8-0	52-8-0
8-0-0	1-8-0	6-2-5	6-1-11	8-8-0	6-1-11	6-2-5	1-8-0	8-0-0

Plate Offsets (X,Y): {6-0-5-0-0-4-8}

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.13	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.08	Vert(LL) -0.02 2-21 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.22	Vert(TL) -0.03 2-21 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 359 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=309/0-8-0, 20=632/0-8-0, 17=598/0-8-0, 16=598/0-8-0, 13=632/0-8-0, 10=309/0-8-0

Max Horz 2=63 (load case 5)

Max Uplift 2=-182 (load case 5), 20=-199 (load case 4), 17=-229 (load case 3), 16=-230 (load case 4), 13=-194 (load case 3), 10=-194 (load case 6)

Max Grav 2=309 (load case 1), 20=632 (load case 1), 17=605 (load case 10), 16=605 (load case 9), 13=632 (load case 1), 10=309 (load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-129/70, 3-4=-2/83, 4-5=-155/147, 5-6=-28/121, 6-7=-29/121, 7-8=-155/147, 8-9=-5/83, 9-10=-129/70, 10-11=0/32

BOT CHORD 2-21=0/73, 20-21=0/73, 19-20=-46/159, 18-19=0/62, 17-18=0/62, 16-17=0/58, 15-16=0/62, 14-15=0/62, 13-14=-52/159, 12-13=0/71, 10-12=0/70

WEBS 3-21=0/115, 3-20=-392/157, 4-20=-314/153, 4-19=-14/62, 5-19=-45/170, 5-17=-364/196, 6-17=-167/118, 6-16=-167/118, 7-16=-364/198, 7-14=-47/170, 8-14=-14/63, 8-13=-314/152, 9-13=-392/157, 9-12=0/115

JOINT STRESS INDEX

2 = 0.25, 3 = 0.35, 4 = 0.33, 5 = 0.32, 6 = 0.16, 7 = 0.32, 8 = 0.33, 9 = 0.35, 10 = 0.25, 12 = 0.34, 13 = 0.34, 14 = 0.33, 15 = 0.11, 16 = 0.32, 17 = 0.32, 18 = 0.11, 19 = 0.33, 20 = 0.34 and 21 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C

Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) All plates are 4x6 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 lb uplift at joint 2, 199 lb uplift at joint 20, 229 lb uplift at joint 17, 230 lb uplift at joint 16, 194 lb uplift at joint 13 and 194 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L159594	Truss T07	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:14:53 2006 Page 1

2-0-0	9-4-0	17-11-0	26-4-0	34-9-0	43-4-0	52-8-0	54-8-0
2-0-0	9-4-0	8-7-0	8-5-0	8-5-0	8-7-0	9-4-0	2-0-0
							Scale = 1/94.9

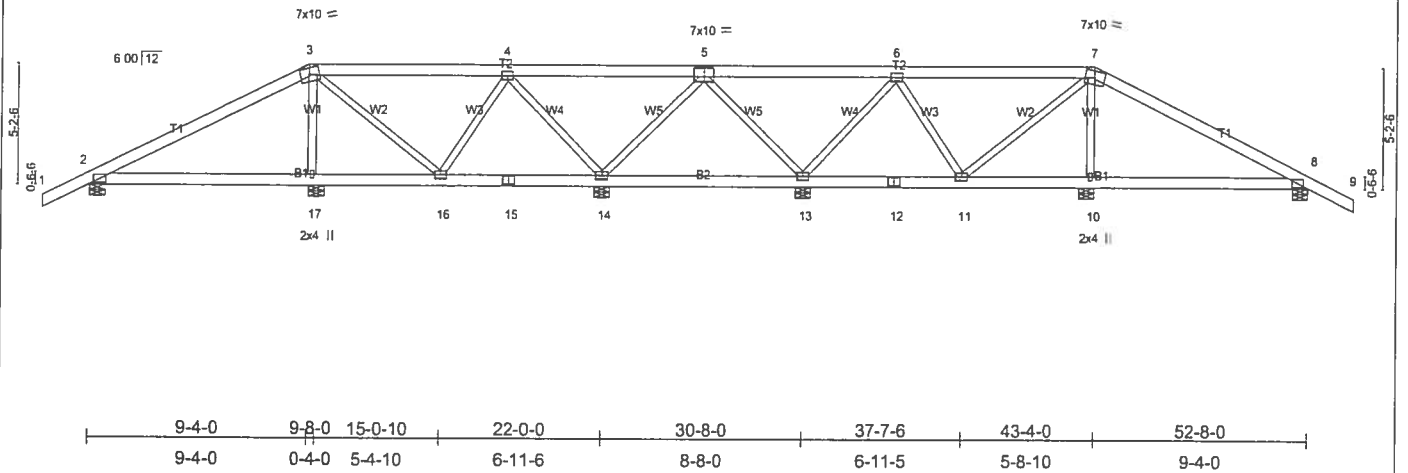


Plate Offsets (X,Y): [5.0-5.0,0.4-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.12	Vert(LL)	-0.03	8-10	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(TL)	-0.05	8-10	>999	180		
BCLL 10.0	Lumber Increase 1.25	WB 0.28	Horz(TL)	0.00	8	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TP12002							Weight: 347 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=324/0-8-0, 17=591/0-8-0, 14=625/0-8-0, 13=625/0-8-0, 10=591/0-8-0, 8=324/0-8-0

Max Horz 2=69 (load case 5)

Max Uplift 2=193 (load case 5), 17=160 (load case 4), 14=244 (load case 3), 13=245 (load case 4), 10=152 (load case 3), 8=208 (load case 6)

Max Grav 2=324 (load case 9), 17=591 (load case 9), 14=629 (load case 10), 13=629 (load case 9), 10=591 (load case 10), 8=324 (load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-119/96, 3-4=-132/156, 4-5=-36/133, 5-6=-37/133, 6-7=-132/156, 7-8=-119/96, 8-9=0/32

BOT CHORD 2-17=0/67, 16-17=0/71, 15-16=-46/135, 14-15=-46/135, 13-14=0/80, 12-13=-48/135, 11-12=-48/135, 10-11=0/65, 8-10=0/58

WEBS 3-17=400/215, 3-16=-50/153, 4-16=-7/62, 4-14=-408/228, 5-14=-209/152, 5-13=-209/152, 6-13=-408/228, 6-11=-7/61, 7-11=-50/153, 7-10=400/215

JOINT STRESS INDEX

2 = 0.34, 3 = 0.34, 4 = 0.29, 5 = 0.22, 6 = 0.29, 7 = 0.34, 8 = 0.34, 10 = 0.34, 11 = 0.29, 12 = 0.13, 13 = 0.26, 14 = 0.26, 15 = 0.13, 16 = 0.29 and 17 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C

Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

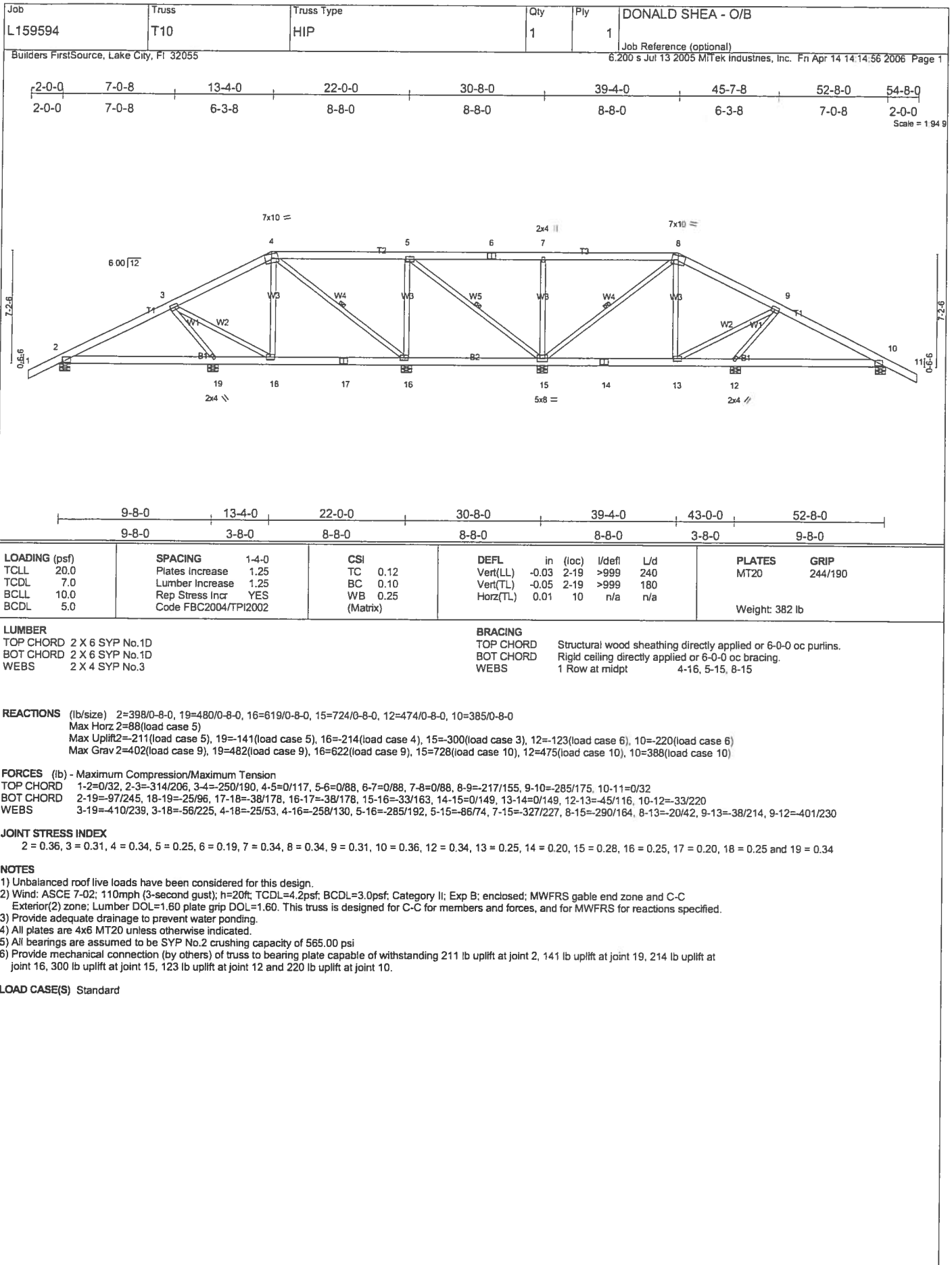
3) Provide adequate drainage to prevent water ponding.

4) All plates are 4x6 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2, 160 lb uplift at joint 17, 244 lb uplift at joint 14, 245 lb uplift at joint 13, 152 lb uplift at joint 10 and 208 lb uplift at joint 8.

LOAD CASE(S) Standard



Job L159594	Truss T12	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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2-0-0	8-2-14	16-0-0	22-0-0	30-8-0	36-8-0	44-5-2	52-8-0	54-8-0
2-0-0	8-2-14	7-9-2	6-0-0	8-8-0	6-0-0	7-9-2	8-2-14	2-0-0

Scale = 1/4" = 1'-0"

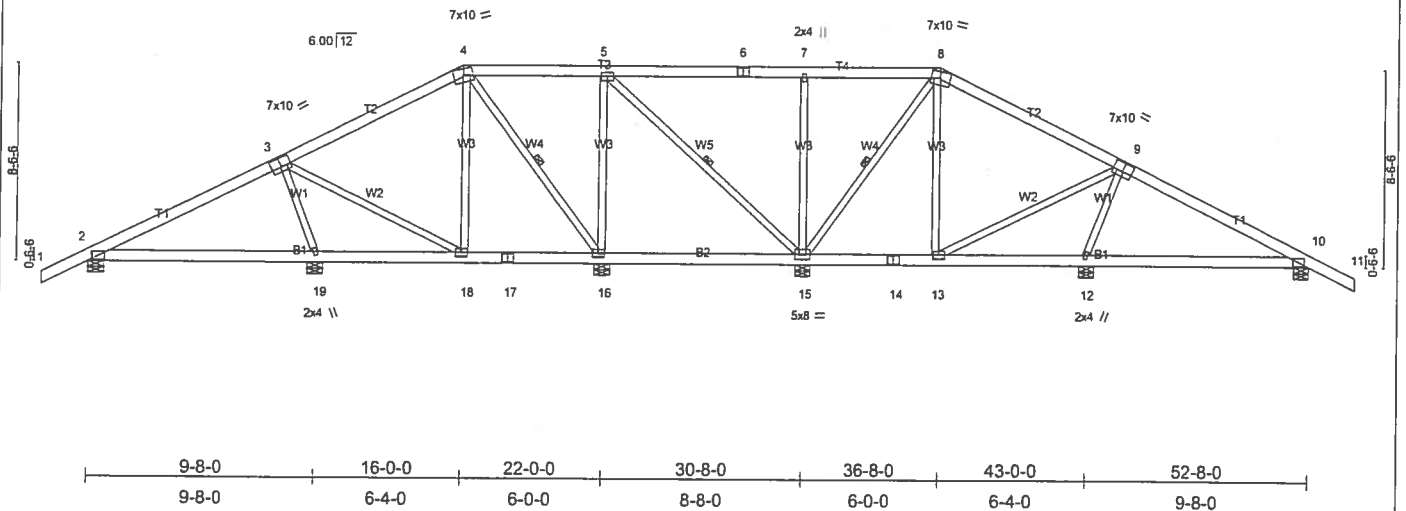


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [9:0-5-0,0-4-8]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.13	Vert(LL)	-0.03	2-19	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	-0.05	2-19	>999	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.32	Horz(TL)	0.00	10	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 394 lb

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing. Except:
 6'-0" oc bracing: 15-16, 12-13.
 WEBS 1 Row at midpt 4-16, 5-15, 8-15

REACTIONS (lb/size) 2=362/0-8-0, 19=548/0-8-0, 16=580/0-8-0, 15=706/0-8-0, 12=536/0-8-0, 10=348/0-8-0

Max Horz 2=-100(load case 6)

Max Uplift 2=-191(load case 5), 19=-187(load case 5), 16=-181(load case 4), 15=-281(load case 3), 12=-165(load case 6), 10=-206(load case 6)

Max Grav 2=365(load case 9), 19=551(load case 9), 16=581(load case 9), 15=709(load case 10), 12=539(load case 10), 10=351(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-233/150, 3-4=-212/181, 4-5=0/148, 5-6=0/93, 6-7=0/93, 7-8=0/92, 8-9=-171/141, 9-10=-203/121, 10-11=0/32

BOT CHORD 2-19=-56/158, 18-19=0/100, 17-18=-6/130, 16-17=-6/130, 15-16=-27/178, 14-15=0/144, 13-14=0/144, 12-13=-20/115, 10-12=0/132

WEBS 3-19=-388/255, 3-18=-10/132, 4-18=0/45, 4-16=-241/116, 5-16=-231/155, 5-15=-91/76, 7-15=-292/201, 8-15=-273/148, 8-13=0/52, 9-13=0/115, 9-12=-375/242

JOINT STRESS INDEX

2 = 0.32, 3 = 0.29, 4 = 0.34, 5 = 0.25, 6 = 0.24, 7 = 0.34, 8 = 0.34, 9 = 0.29, 10 = 0.31, 12 = 0.34, 13 = 0.25, 14 = 0.11, 15 = 0.28, 16 = 0.28, 17 = 0.11, 18 = 0.25 and 19 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 4x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 2, 187 lb uplift at joint 19, 181 lb uplift at joint 16, 281 lb uplift at joint 15, 165 lb uplift at joint 12 and 206 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L159594	Truss T13	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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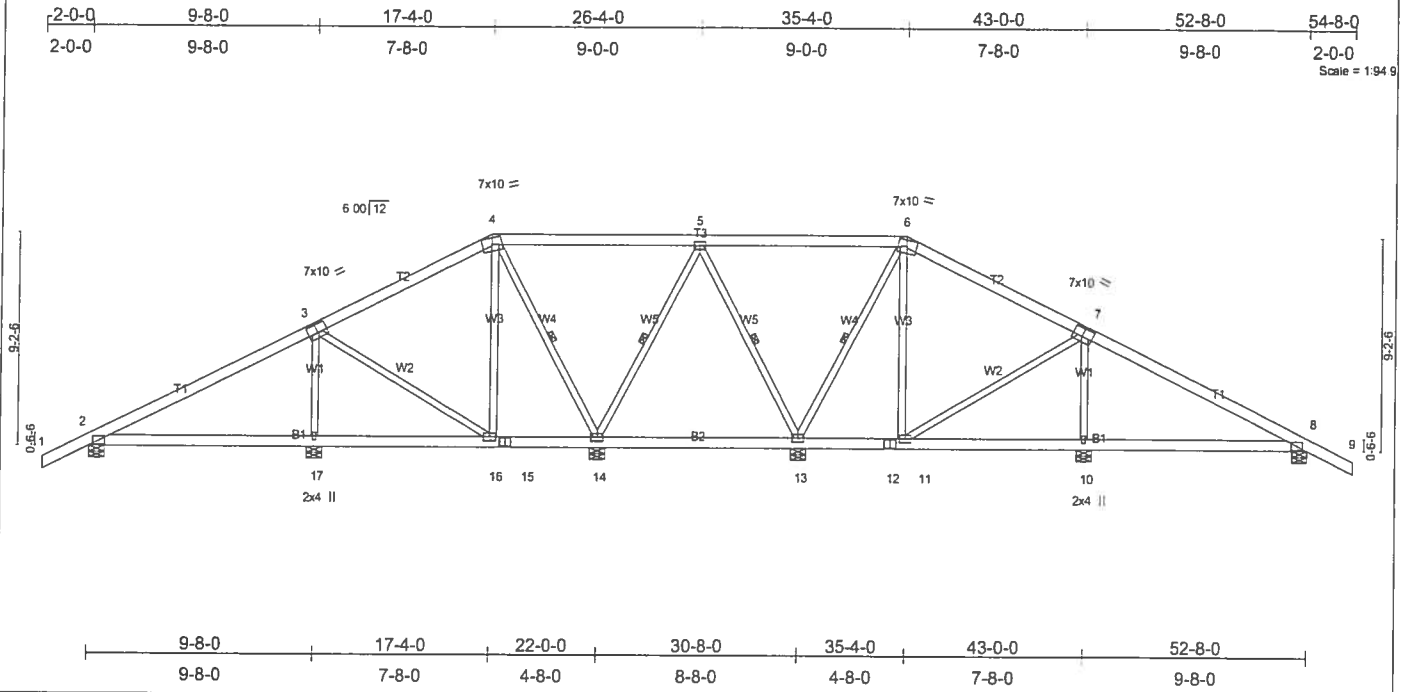


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [7:0-5-0,0-4-8]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.16	Vert(LL)	-0.04	8-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.13	Vert(TL)	-0.06	8-10	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.16	Horz(TL)	0.00	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 387 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 4-14, 5-14, 5-13, 6-13

REACTIONS (lb/size) 2=335/0-8-0, 17=619/0-8-0, 14=586/0-8-0, 13=586/0-8-0, 10=619/0-8-0, 8=335/0-8-0

Max Horz 2=106(load case 6)

Max Uplift 2=178(load case 5), 17=220(load case 5), 14=200(load case 4), 13=192(load case 3), 10=206(load case 6), 8=203(load case 6)

Max Grav 2=336(load case 9), 17=621(load case 9), 14=592(load case 9), 13=592(load case 10), 10=621(load case 10), 8=336(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-134/101, 3-4=-203/193, 4-5=0/152, 5-6=0/152, 6-7=-203/193, 7-8=-134/84, 8-9=0/32

BOT CHORD 2-17=-10/95, 16-17=-10/95, 15-16=0/142, 14-15=0/142, 13-14=-6/140, 12-13=0/123, 11-12=0/123, 10-11=0/70, 8-10=0/71

WEBS 3-17=403/265, 3-16=0/88, 4-16=-5/44, 4-14=-298/155, 5-14=-204/139, 5-13=-204/139, 6-13=-298/155, 6-11=-9/44, 7-11=-7/88, 7-10=403/265

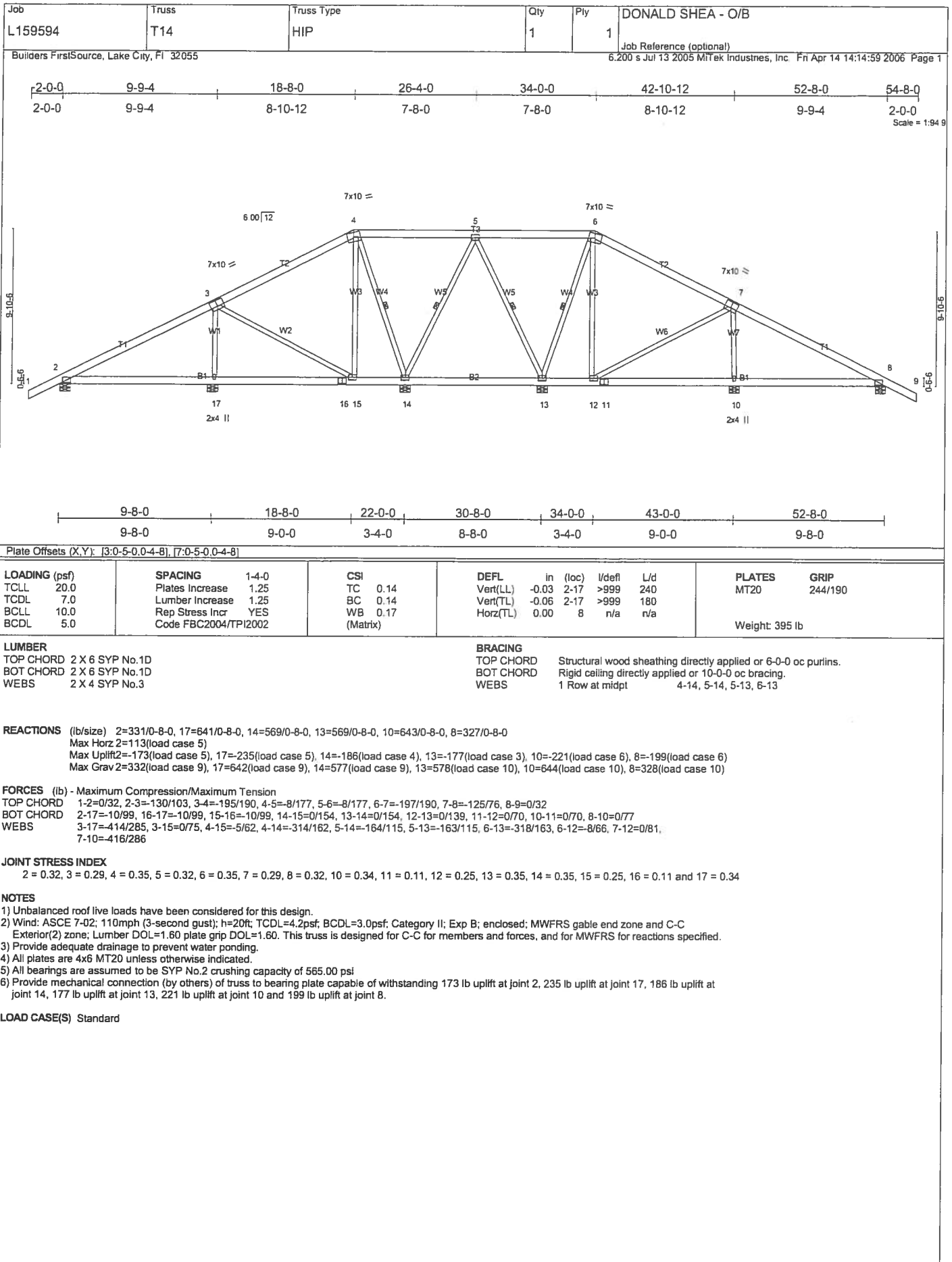
JOINT STRESS INDEX

2 = 0.35, 3 = 0.25, 4 = 0.34, 5 = 0.32, 6 = 0.34, 7 = 0.25, 8 = 0.35, 10 = 0.34, 11 = 0.25, 12 = 0.11, 13 = 0.32, 14 = 0.32, 15 = 0.11, 16 = 0.25 and 17 = 0.34

NOTES

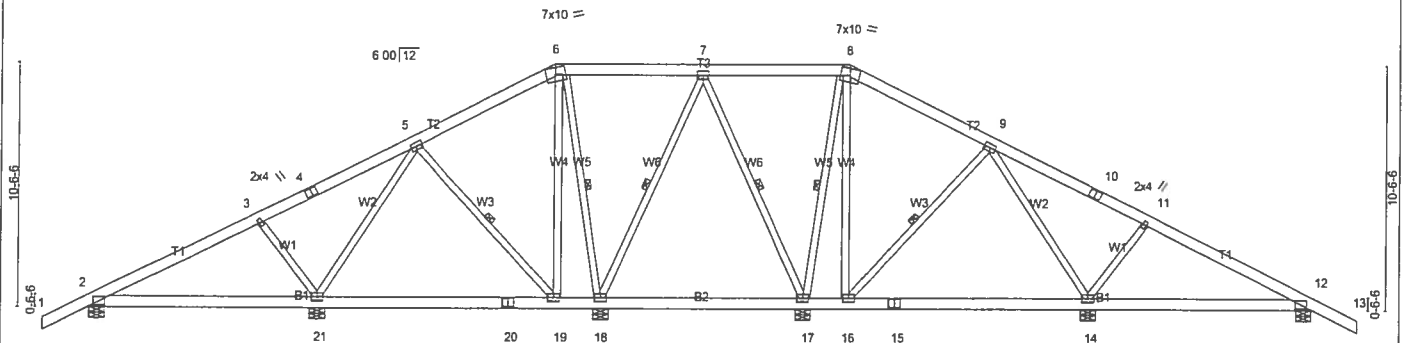
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All plates are 4x6 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 2, 220 lb uplift at joint 17, 200 lb uplift at joint 14, 192 lb uplift at joint 13, 206 lb uplift at joint 10 and 203 lb uplift at joint 8.

LOAD CASE(S) Standard



Job L159594	Truss T15	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:15:00 2006 Page 1

2-0-0	7-3-1	13-10-15	20-0-0	26-4-0	32-8-0	38-9-1	45-4-15	52-8-0	54-8-0
2-0-0	7-3-1	6-7-15	6-1-1	6-4-0	6-4-0	6-1-1	6-7-15	7-3-1	2-0-0
Scale = 1/94.9									



9-8-0	20-0-0	22-0-0	30-8-0	32-8-0	43-0-0	52-8-0
9-8-0	10-4-0	2-0-0	8-8-0	2-0-0	10-4-0	9-8-0
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.08	in (loc) l/def L/d	MT20	244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(LL) -0.03 14-16 >999 240			
BCLL 10.0	Rep Stress Incr YES	WB 0.37	Vert(TL) -0.06 14-16 >999 180			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)	Horz(TL) 0.00 12 n/a n/a			
Weight: 419 lb						

LUMBER	BRACING
TOP CHORD 2 X 6 SYP No.1D	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 6 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	WEBS 1 Row at midpt 5-19, 6-18, 7-18, 7-17, 8-17, 9-16

REACTIONS (lb/size)	2=307/0-8-0, 21=692/0-8-0, 18=541/0-8-0, 17=541/0-8-0, 14=692/0-8-0, 12=307/0-8-0
Max Horz	2=119(load case 6)
Max Uplift	2=158(load case 5), 21=260(load case 5), 18=170(load case 4), 17=155(load case 3), 14=254(load case 6), 12=184(load case 6)
Max Grav	2=308(load case 9), 21=693(load case 9), 18=548(load case 9), 17=548(load case 10), 14=693(load case 10), 12=308(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/32, 2-3=142/99, 3-4=23/82, 4-5=14/119, 5-6=144/201, 6-7=34/199, 7-8=34/199, 8-9=144/201, 9-10=0/119, 10-11=0/49, 11-12=142/79, 12-13=0/32
BOT CHORD	2-21=30/93, 20-21=7/123, 19-20=7/123, 18-19=0/173, 17-18=0/161, 16-17=0/170, 15-16=0/107, 14-15=0/107, 12-14=0/82
WEBS	3-21=242/224, 5-21=317/181, 5-19=33/124, 6-19=51/128, 6-18=313/133, 7-18=137/96, 7-17=137/96, 8-17=313/133, 8-16=51/128, 9-16=33/124, 9-14=317/181, 11-14=242/224

JOINT STRESS INDEX	
	2 = 0.28, 3 = 0.34, 4 = 0.11, 5 = 0.30, 6 = 0.37, 7 = 0.33, 8 = 0.37, 9 = 0.30, 10 = 0.11, 11 = 0.34, 12 = 0.28, 14 = 0.29, 15 = 0.12, 16 = 0.26, 17 = 0.31, 18 = 0.31, 19 = 0.26, 20 = 0.12 and 21 = 0.29

NOTES	
1) Unbalanced roof live loads have been considered for this design.	
2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.	
3) Provide adequate drainage to prevent water ponding.	
4) All plates are 4x6 MT20 unless otherwise indicated.	
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi	
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 2, 260 lb uplift at joint 21, 170 lb uplift at joint 18, 155 lb uplift at joint 17, 254 lb uplift at joint 14 and 184 lb uplift at joint 12.	

LOAD CASE(S)	Standard
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Job L159594	Truss T16	Truss Type HIP	Qty 1	Ply 2	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:15:01 2006 Page 1		

2-0-0	6-8-0	12-2-15	16-8-11	22-0-0	26-4-0	30-8-0	35-11-5	40-5-1	46-0-0	52-8-0	54-8-0
2-0-0	6-8-0	5-6-15	4-5-12	5-3-5	4-4-0	4-4-0	5-3-5	4-5-12	5-6-15	6-8-0	2-0-0
Scale = 1/94.9											

6-8-0	9-8-0	14-9-14	22-0-0	26-4-0	30-8-0	37-10-2	43-0-0	46-0-0	52-8-0
6-8-0	3-0-0	5-1-14	7-2-2	4-4-0	4-4-0	7-2-2	5-1-14	3-0-0	6-8-0

Plate Offsets (X,Y): [7:0-5-0,0-4-8]

LOADING (psf) TCCL 20.0 TCCL 7.0 BCCL 10.0 BCDL 5.0	SPACING 1-4-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.13 BC 0.13 WB 0.29 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.03 16-18 >999 240 Vert(TL) -0.06 16-18 >999 180 Horz(TL) 0.01 15 n/a n/a	PLATES MT20 Weight: 723 lb	GRIP 244/190
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LUMBER TOP CHORD 2 X 6 SYP No.1D BOT CHORD 2 X 6 SYP No.1D WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
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REACTIONS (lb/size) 2=381/0-8-0, 23=1837/0-8-0, 20=2700/0-8-0, 15=2942/0-8-0, 12=122/0-8-0
 Max Horiz 2=57(load case 5)
 Max Uplift 2=229(load case 4), 23=-831(load case 3), 20=-1265(load case 2), 15=-1343(load case 2), 12=-134(load case 5)
 Max Grav 2=381(load case 1), 23=1837(load case 1), 20=2704(load case 9), 15=2942(load case 9), 12=122(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/32, 2-3=325/175, 3-4=-223/502, 4-5=-133/86, 5-6=-399/887, 6-7=-550/300, 7-8=-550/300, 8-9=-1400/680, 9-10=-720/335, 10-11=-546/1198, 11-12=-164/346, 12-13=0/32
 BOT CHORD 2-24=-97/250, 23-24=-106/276, 22-23=-28/29, 21-22=-28/38, 20-21=-28/38, 19-20=-887/466, 18-19=-617/1400, 17-18=-520/1086, 16-17=-520/1086, 15-16=-44/54, 14-15=-253/157, 12-14=-278/166
 WEBS 3-24=-213/606, 3-23=-1173/568, 4-23=-858/507, 4-22=0/185, 5-22=0/195, 5-20=-1098/574, 6-20=-1690/990, 6-19=-875/1828, 7-19=-351/302, 8-19=-1081/485, 8-18=0/163, 9-18=-115/377, 9-16=-759/510, 10-16=-442/1247, 10-15=-1922/1005, 11-15=-1501/715, 11-14=-208/593

JOINT STRESS INDEX
 2 = 0.19, 3 = 0.34, 4 = 0.29, 5 = 0.29, 6 = 0.38, 7 = 0.16, 8 = 0.38, 9 = 0.29, 10 = 0.29, 11 = 0.34, 12 = 0.19, 14 = 0.34, 15 = 0.26, 16 = 0.29, 17 = 0.25, 18 = 0.25, 19 = 0.43, 20 = 0.25, 21 = 0.25, 22 = 0.29, 23 = 0.26 and 24 = 0.34

NOTES
 1) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 2) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
 Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 4) Unbalanced roof live loads have been considered for this design.
 5) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 6) Provide adequate drainage to prevent water ponding.
 7) All plates are 4x6 MT20 unless otherwise indicated.
 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 229 lb uplift at joint 2, 831 lb uplift at joint 23, 1265 lb uplift at joint 20, 1343 lb uplift at joint 15 and 134 lb uplift at joint 12.
 10) Girder carries hip end with 6-8-0 end setback.
 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 529 lb down and 272 lb up at 46-0-0, and 529 lb down and 272 lb up at 6-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	
L159594	T16	HIP	1	2	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:15:01 2006 Page 2

LOAD CASE(S) Standard

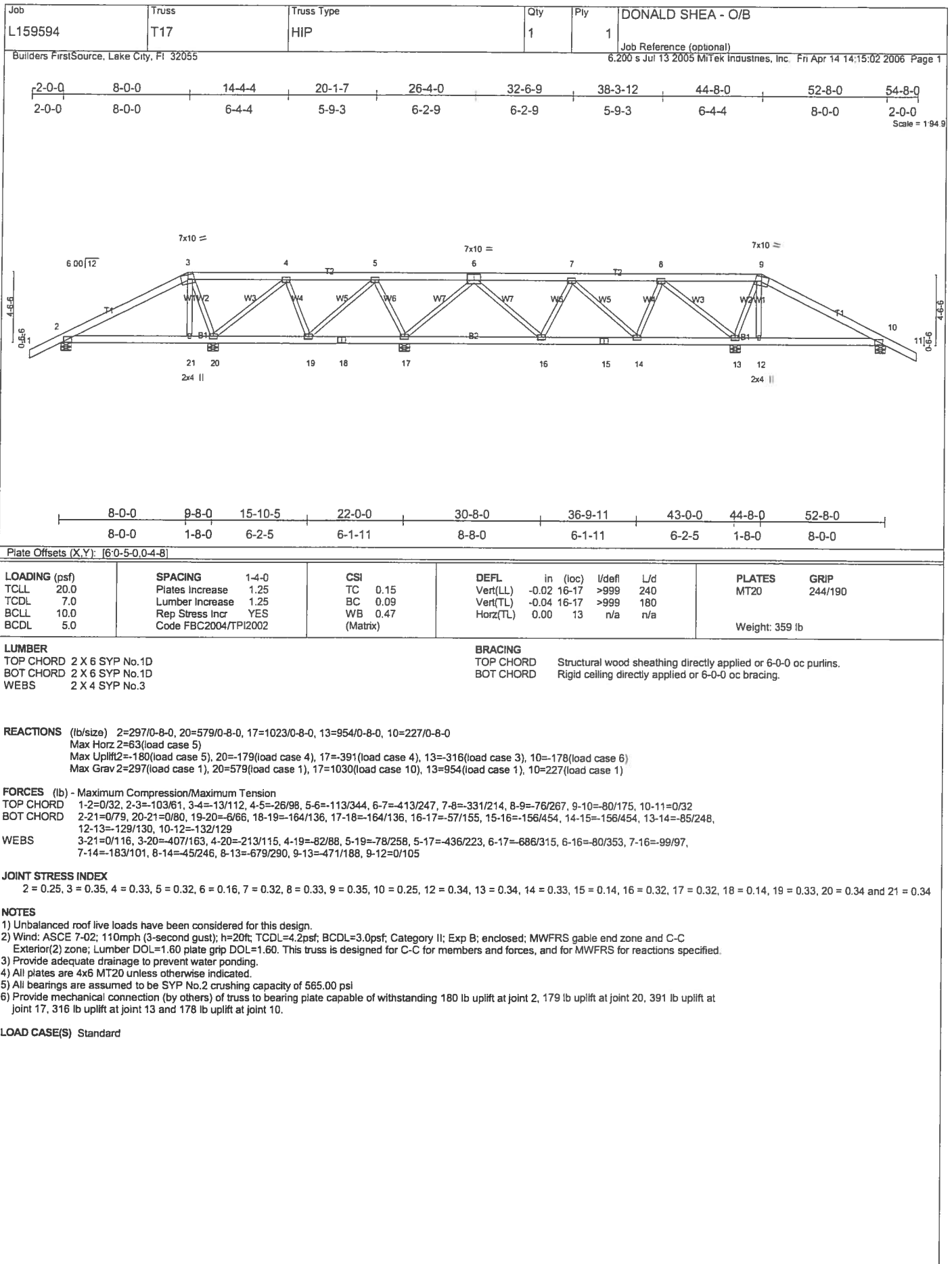
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-36, 3-11=-99(F=-63), 11-13=-36, 2-24=-20, 14-24=-55(F=-35), 12-14=-20

Concentrated Loads (lb)

Vert: 24=-529(F) 14=-529(F)



Job L159594	Truss T18	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

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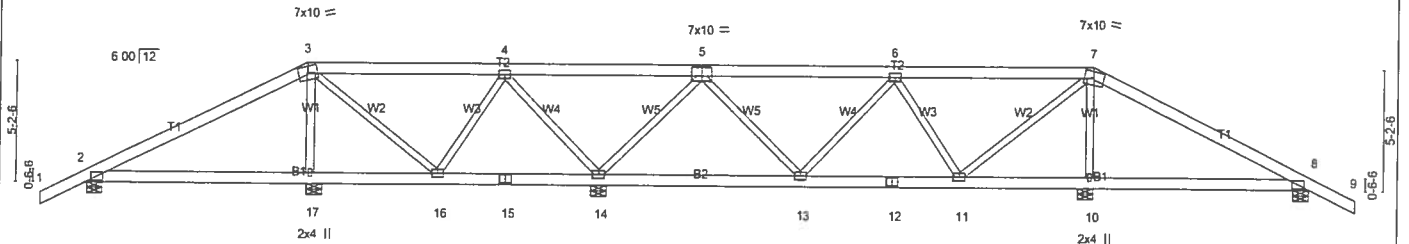


Plate Offsets (X,Y): [5-0-5-0-0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	1-4-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(LL) -0.03 2-17 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.54	Vert(TL) -0.06 2-17 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 10 n/a n/a		
	Code FBC2004/TP12002			Weight: 347 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
 10-0-0 oc bracing: 13-14,11-13.

REACTIONS (lb/size) 2=301/0-8-0, 17=549/0-8-0, 14=1085/0-8-0, 10=874/0-8-0, 8=271/0-8-0

Max Horz 2=69(load case 5)

Max Uplift 2=188(load case 5), 17=144(load case 4), 14=422(load case 3), 10=261(load case 3), 8=197(load case 6)

Max Grav 2=301(load case 1), 17=549(load case 9), 14=1088(load case 10), 10=874(load case 1), 8=271(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=68/82, 3-4=12/108, 4-5=119/346, 5-6=373/232, 6-7=301/224, 7-8=29/142, 8-9=0/32

BOT CHORD 2-17=21/77, 16-17=29/82, 15-16=32/98, 14-15=32/98, 13-14=51/152, 12-13=167/441, 11-12=167/441, 10-11=92/107, 8-10=74/97

WEBS 3-17=357/199, 3-16=12/53, 4-16=0/79, 4-14=491/260, 5-14=738/365, 5-13=65/327, 6-13=104/110, 6-11=272/165, 7-11=189/510, 7-10=679/328

JOINT STRESS INDEX

2 = 0.34, 3 = 0.34, 4 = 0.29, 5 = 0.22, 6 = 0.29, 7 = 0.34, 8 = 0.34, 10 = 0.34, 11 = 0.29, 12 = 0.18, 13 = 0.26, 14 = 0.26, 15 = 0.18, 16 = 0.29 and 17 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All plates are 4x6 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 2, 144 lb uplift at joint 17, 422 lb uplift at joint 14, 261 lb uplift at joint 10 and 197 lb uplift at joint 8.

LOAD CASE(S) Standard

Job L159594	Truss T19	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 M/Tek Industries, Inc. Fri Apr 14 14:15:03 2006 Page 1		

2-0-0	5-3-13	10-8-0	18-4-8	26-4-0	34-3-8	42-0-0	47-4-3	52-8-0	54-8-0
2-0-0	5-3-13	5-4-3	7-8-8	7-11-8	7-11-8	7-8-8	5-4-3	5-3-13	2-0-0

Scale = 1/94.9

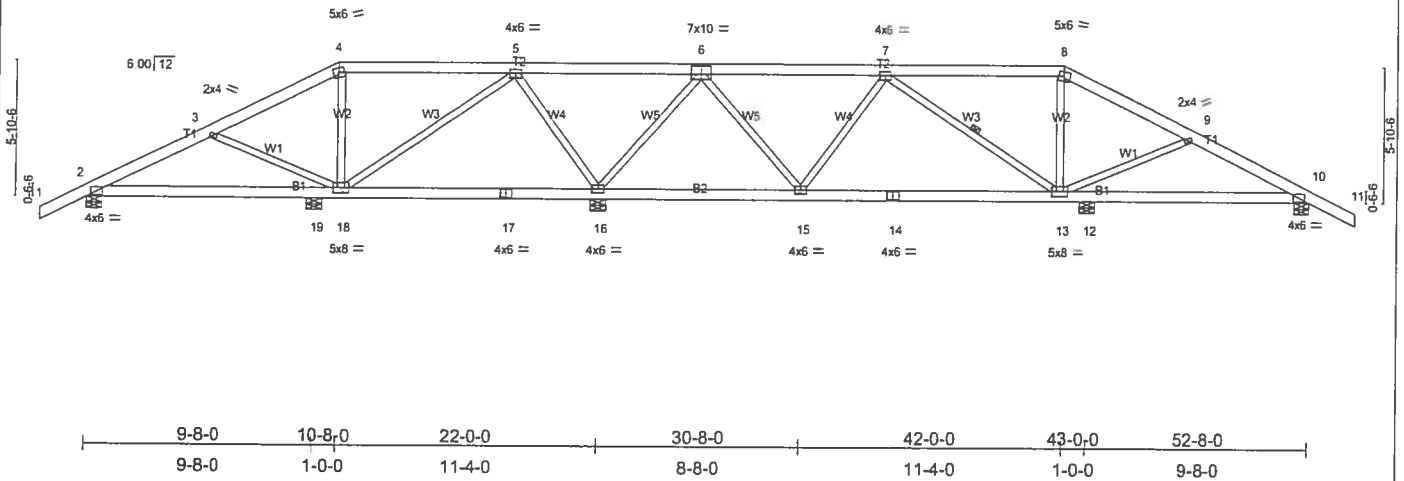


Plate Offsets (X,Y): {6-0-5-0-0-4-8}

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.09	13-15	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.29	Vert(TL)	-0.16	13-15	>999	180	244/190
BCLL 10.0	Rep Stress Incr	YES	WB 0.77	Horz(TL)	0.01	10	n/a	n/a	
BCDL 5.0	Code FBC2004/TP12002		(Matrix)						Weight: 360 lb

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
 6-0-0 oc bracing: 16-18.
 WEBS 1 Row at midpt 7-13

REACTIONS (lb/size) 2=428/0-8-0, 16=1445/0-8-0, 10=561/0-8-0, 19=239/0-8-0, 12=407/0-8-0

Max Horz 2=75(load case 5)

Max Uplift 2=232(load case 5), 16=507(load case 4), 10=274(load case 6), 19=49(load case 4), 12=107(load case 3)

Max Grav 2=435(load case 9), 16=1445(load case 1), 10=565(load case 10), 19=242(load case 9), 12=411(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=452/254, 3-4=253/162, 4-5=187/189, 5-6=139/504, 6-7=431/259, 7-8=466/304, 8-9=558/288, 9-10=733/370,

10-11=0/32

BOT CHORD 2-19=155/363, 18-19=155/363, 17-18=153/154, 16-17=153/154, 15-16=25/95, 14-15=188/571, 13-14=188/571, 12-13=211/604,

10-12=211/604

WEBS 3-18=191/170, 4-18=155/118, 5-18=132/398, 5-16=632/339, 6-16=913/426, 6-15=160/546, 7-15=260/189, 7-13=172/126, 8-13=27/66,

9-13=152/154

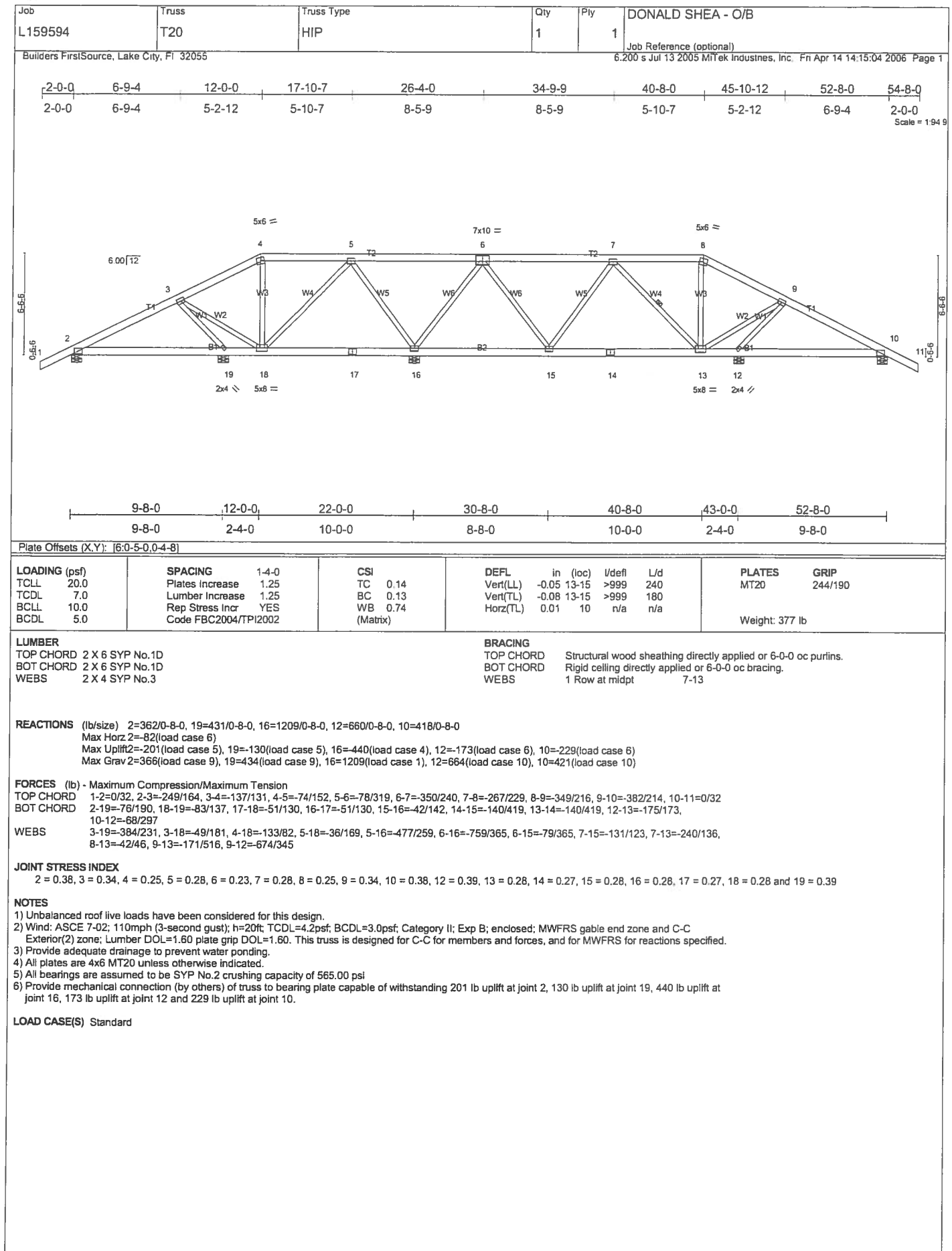
JOINT STRESS INDEX

2 = 0.33, 3 = 0.34, 4 = 0.46, 5 = 0.28, 6 = 0.22, 7 = 0.28, 8 = 0.46, 9 = 0.34, 10 = 0.33, 13 = 0.28, 14 = 0.36, 15 = 0.28, 16 = 0.28, 17 = 0.36 and 18 = 0.28

NOTES

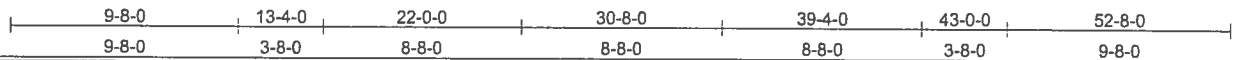
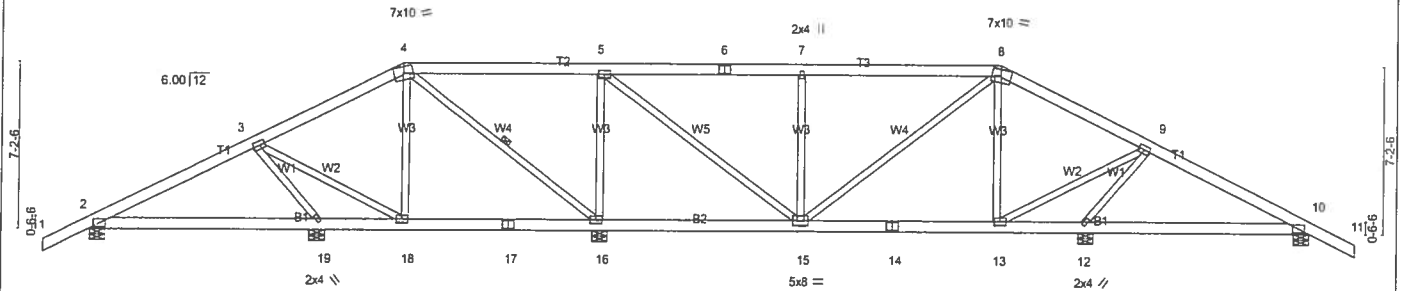
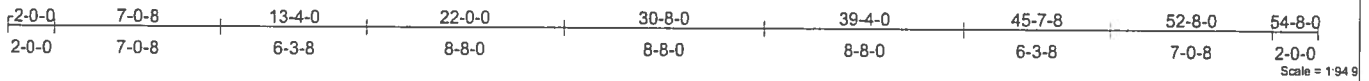
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2, 507 lb uplift at joint 16, 274 lb uplift at joint 10, 49 lb uplift at joint 19 and 107 lb uplift at joint 12.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	DONALD SHEA - O/B
L159594	T21	HIP	1	1	Job Reference (optional)

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LOADING (psf)	SPACING 1-4-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.13	Vert(LL) -0.04 13-15 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL) -0.06 13-15 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.57	Horz(TL) 0.01 10 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 382 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 4-16

REACTIONS (lb/size) 2=372/0-8-0, 19=467/0-8-0, 16=1129/0-8-0, 12=694/0-8-0, 10=417/0-8-0
 Max Horz 2=88(load case 5)
 Max Uplift 2=-204(load case 5), 19=-138(load case 5), 16=-412(load case 4), 12=-186(load case 6), 10=-229(load case 6)
 Max Grav 2=377(load case 9), 19=470(load case 9), 16=1129(load case 1), 12=697(load case 10), 10=420(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/32, 2-3=263/180, 3-4=-189/160, 4-5=0/134, 5-6=-421/288, 6-7=-421/288, 7-8=-421/288, 8-9=-420/250, 9-10=-370/209, 10-11=0/32
 BOT CHORD 2-19=85/200, 18-19=60/112, 17-18=15/124, 16-17=15/124, 15-16=134/211, 14-15=65/327, 13-14=65/327, 12-13=161/171, 10-12=63/285
 WEBS 3-19=391/230, 3-18=46/200, 4-18=19/51, 4-16=312/155, 5-16=746/400, 5-15=277/682, 7-15=319/223, 8-15=62/151, 8-13=180/112, 9-13=185/551, 9-12=681/360

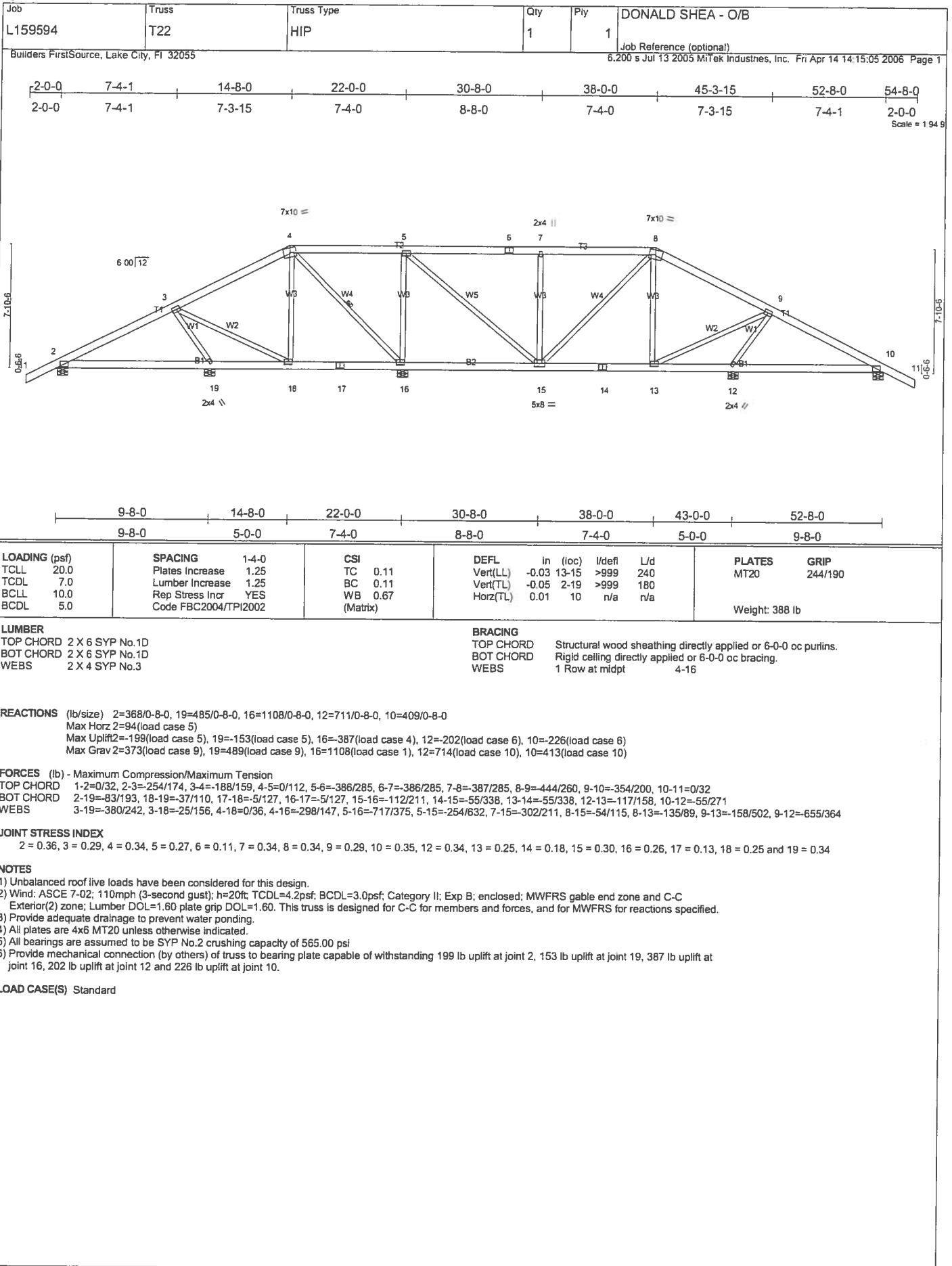
JOINT STRESS INDEX
2 = 0.36, 3 = 0.31, 4 = 0.34, 5 = 0.28, 6 = 0.21, 7 = 0.34, 8 = 0.34, 9 = 0.31, 10 = 0.34, 12 = 0.39, 13 = 0.25, 14 = 0.23, 15 = 0.32, 16 = 0.25, 17 = 0.18, 18 = 0.25 and 19 = 0.34

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 4x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2, 138 lb uplift at joint 19, 412 lb uplift at joint 16, 186 lb uplift at joint 12 and 229 lb uplift at joint 10.

LOAD CASE(S) Standard

**APRIL 14, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**



Job L159594	Truss T23	Truss Type HIP	Qty 1	Ply 1	DONALD SHEA - O/B
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Apr 14 14:15:06 2006 Page 1		

2-0-0	8-2-14	16-0-0	22-0-0	30-8-0	36-8-0	44-5-2	52-8-0	54-8-0
2-0-0	8-2-14	7-9-2	6-0-0	8-8-0	6-0-0	7-9-2	8-2-14	2-0-0

Scale = 1/4" = 1'-0"

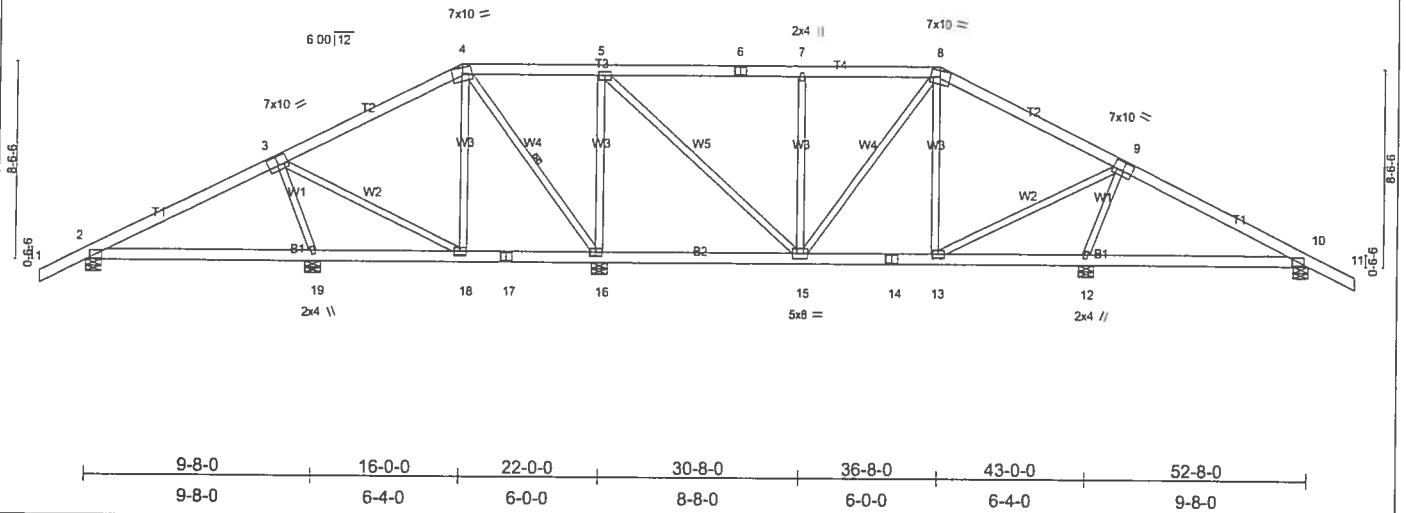


Plate Offsets (X,Y): [3:0-5-0,0-4-8], [9:0-5-0,0-4-8]

LOADING (psf)	SPACING	1-4-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.03	2-19	>999	240	
TCDL 7.0	Lumber Increase	1.25	BC 0.11	Vert(TL)	-0.05	2-19	>999	180	
BCLL 10.0	Rep Stress Incr	YES	WB 0.74	Horz(TL)	0.00	10	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 394 lb

LUMBER

TOP CHORD 2 X 6 SYP No.1D
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS 1 Row at midpt 4-16

REACTIONS (lb/size) 2=352/0-8-0, 19=538/0-8-0, 16=1039/0-8-0, 12=778/0-8-0, 10=372/0-8-0

Max Horz 2=-100(load case 6)
 Max Uplift 2=-189(load case 5), 19=-185(load case 5), 16=-346(load case 4), 12=-236(load case 6), 10=-213(load case 6)
 Max Grav 2=357(load case 9), 19=542(load case 9), 16=1039(load case 1), 12=782(load case 10), 10=376(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/32, 2-3=-215/141, 3-4=-185/167, 4-5=0/131, 5-6=-360/291, 6-7=-360/291, 7-8=-360/291, 8-9=-439/265, 9-10=-258/146, 10-11=0/32
 BOT CHORD 2-19=51/142, 18-19=12/104, 17-18=0/138, 16-17=0/138, 15-16=-63/194, 14-15=-40/328, 13-14=-40/328, 12-13=-69/139, 10-12=-9/181
 WEBS 3-19=-377/250, 3-18=-5/119, 4-18=0/46, 4-16=-257/123, 5-16=-668/340, 5-15=-218/557, 7-15=-284/198, 8-15=-58/109, 8-13=-112/73, 9-13=-118/441, 9-12=-636/363

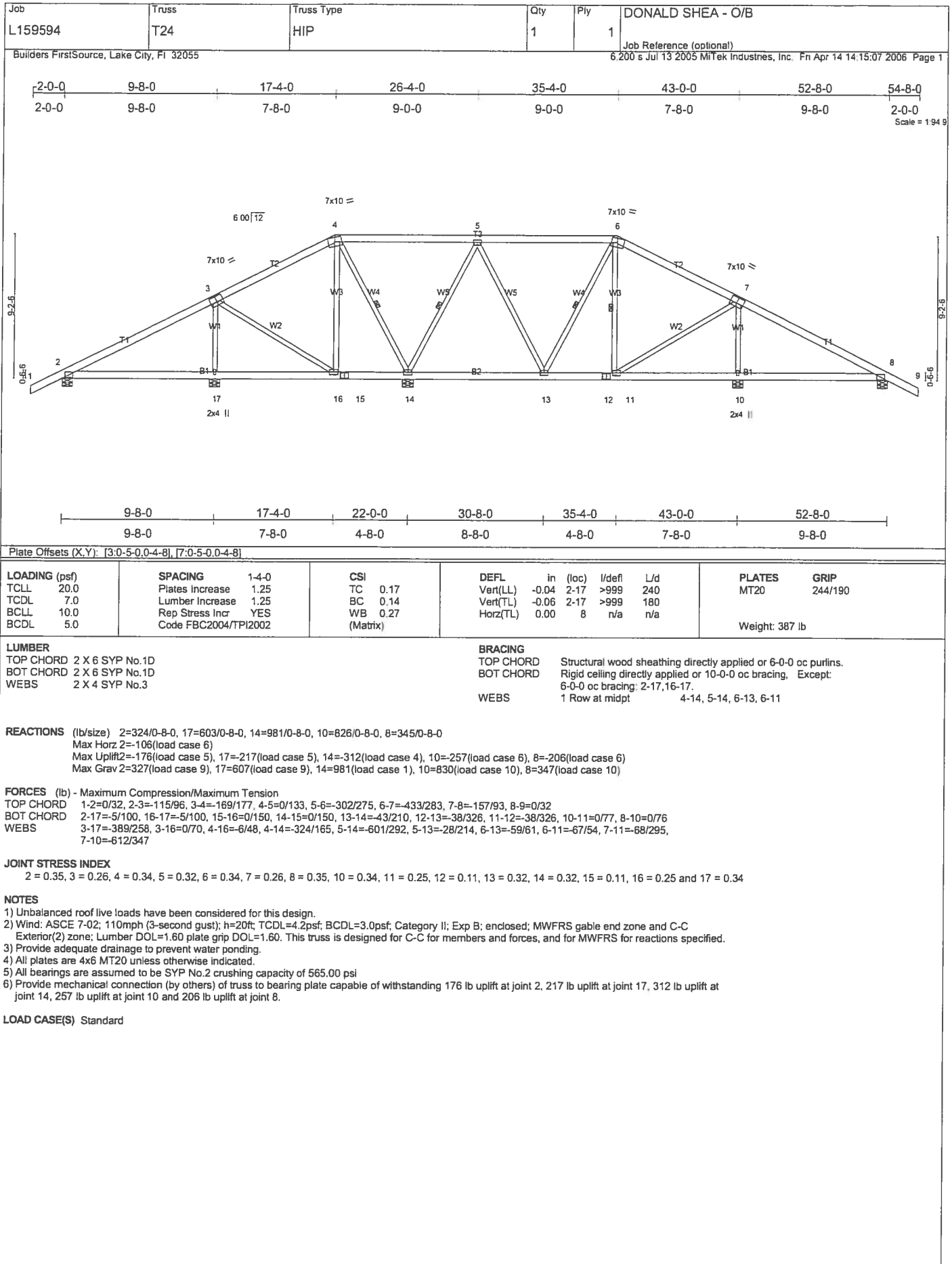
JOINT STRESS INDEX

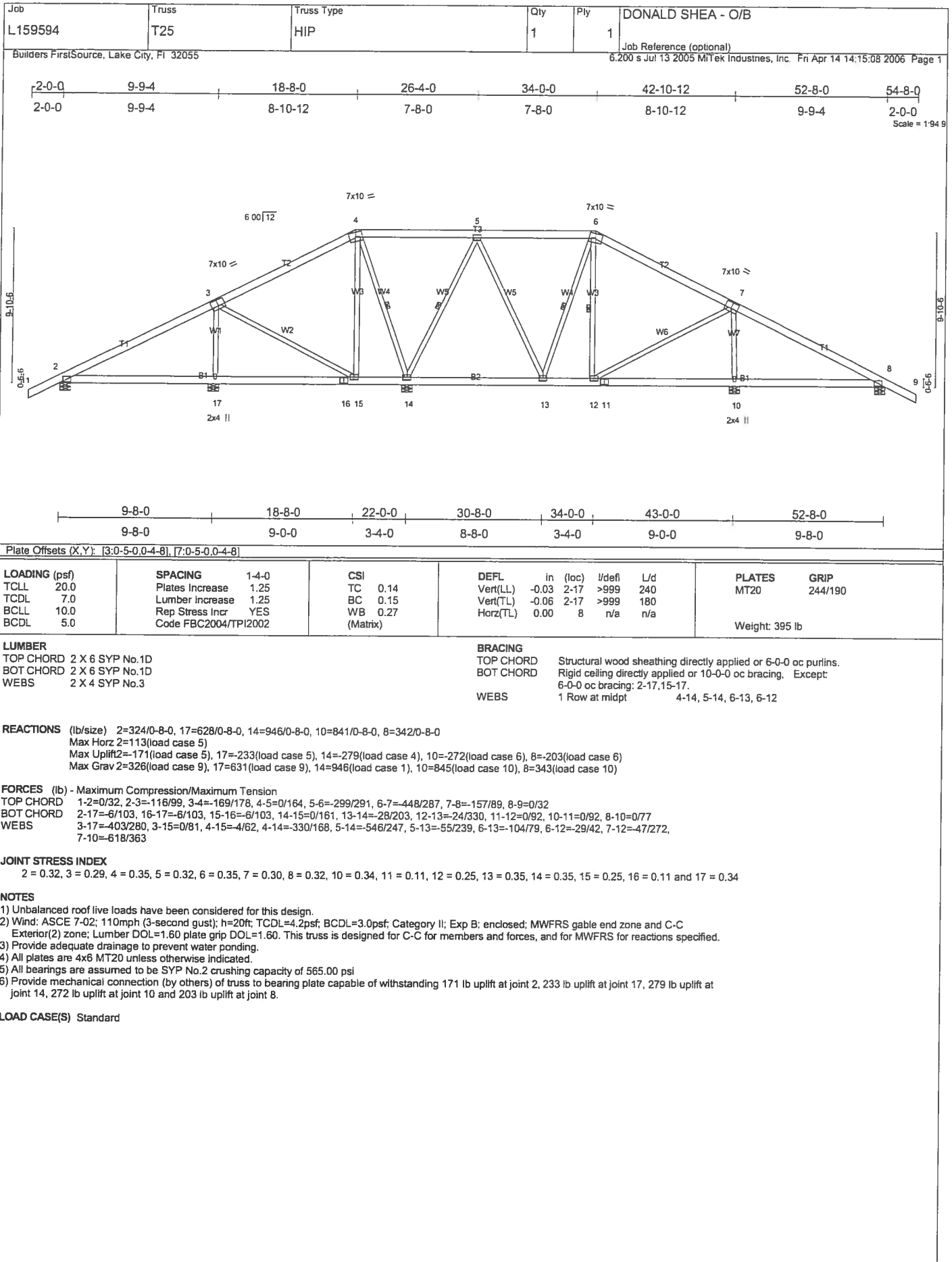
2 = 0.31, 3 = 0.29, 4 = 0.34, 5 = 0.25, 6 = 0.27, 7 = 0.34, 8 = 0.34, 9 = 0.29, 10 = 0.31, 12 = 0.34, 13 = 0.25, 14 = 0.12, 15 = 0.28, 16 = 0.28, 17 = 0.11, 18 = 0.25 and 19 = 0.34

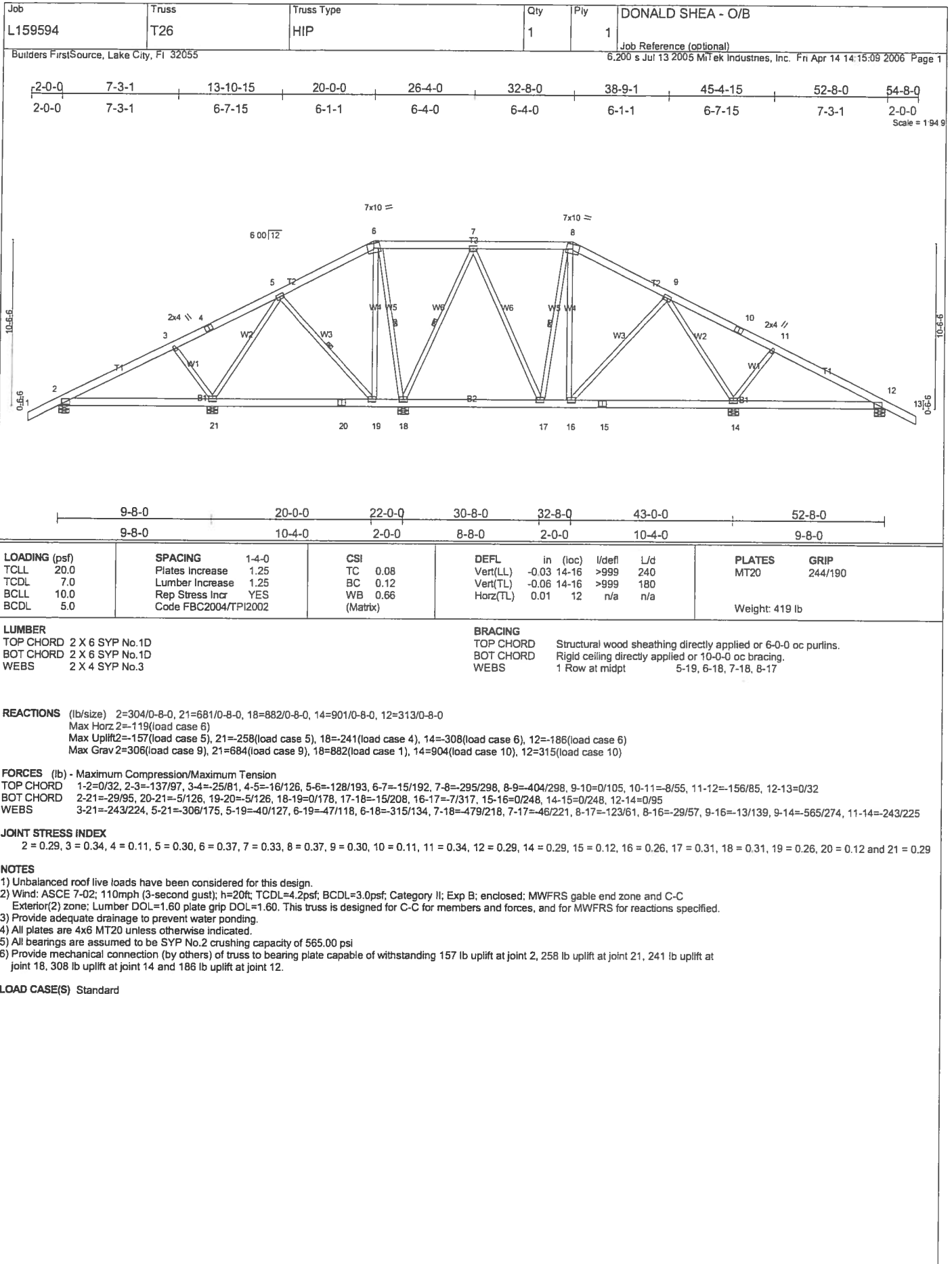
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- All plates are 4x6 MT20 unless otherwise indicated.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2, 185 lb uplift at joint 19, 346 lb uplift at joint 16, 236 lb uplift at joint 12 and 213 lb uplift at joint 10.

LOAD CASE(S) Standard





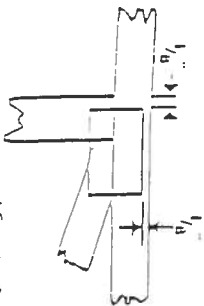


Symbols

PLATE LOCATION AND ORIENTATION



* Either plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 1 x 2 orientation, locate plates 1/8 from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE



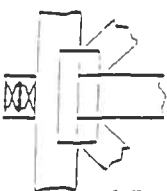
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



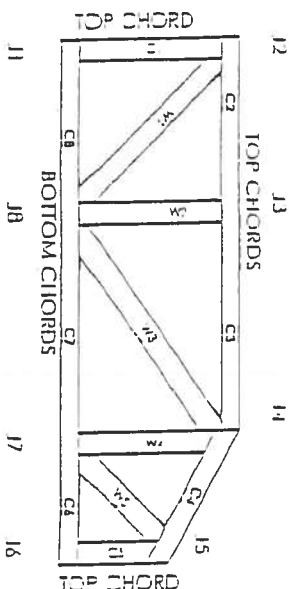
Indicates location of required continuous lateral bracing.

BEARINGS



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DIIIIR	960022-W, 970036-11
IER	561



General Safety Notes

Failure to Follow Could Cause Properly Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and weave at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (1 x 8" from adjacent joint).
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with the retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabrication. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum, plotting requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 11 ft spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to lusses are the responsibility of others unless shown.
13. Do not overload roof or floor lusses with stacks or construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of lusses.