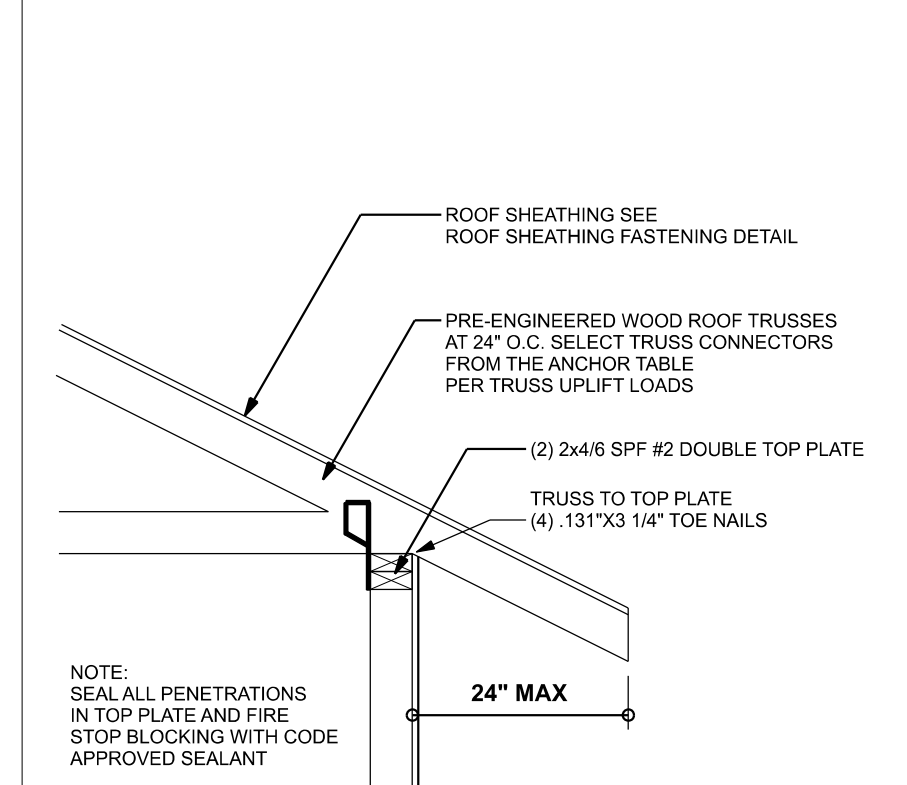
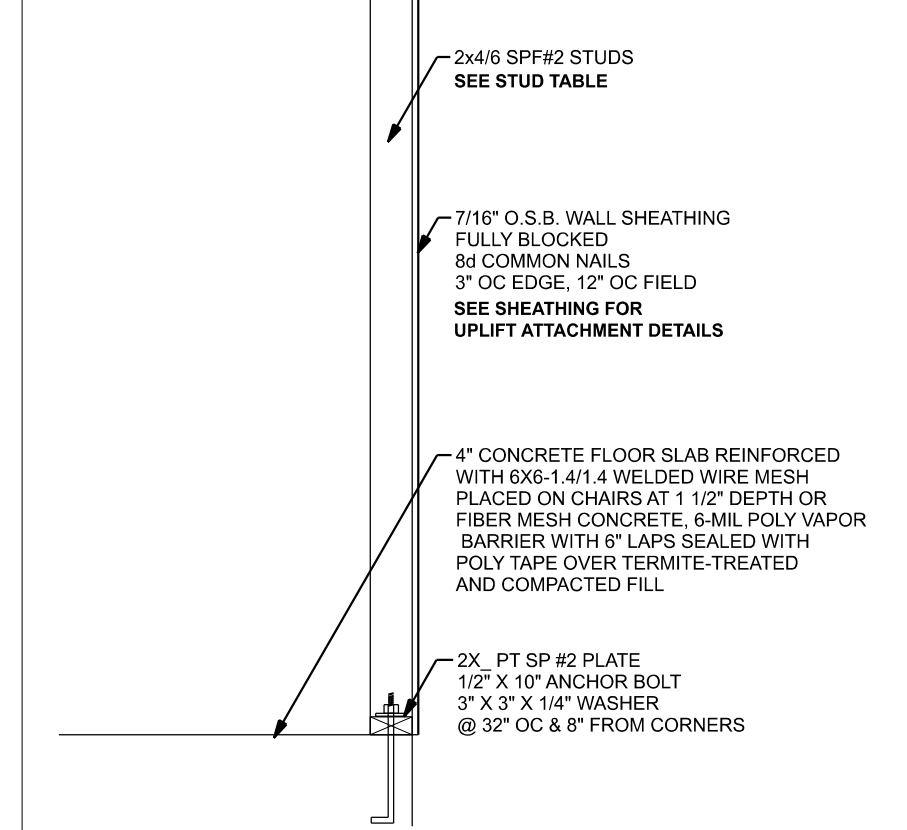
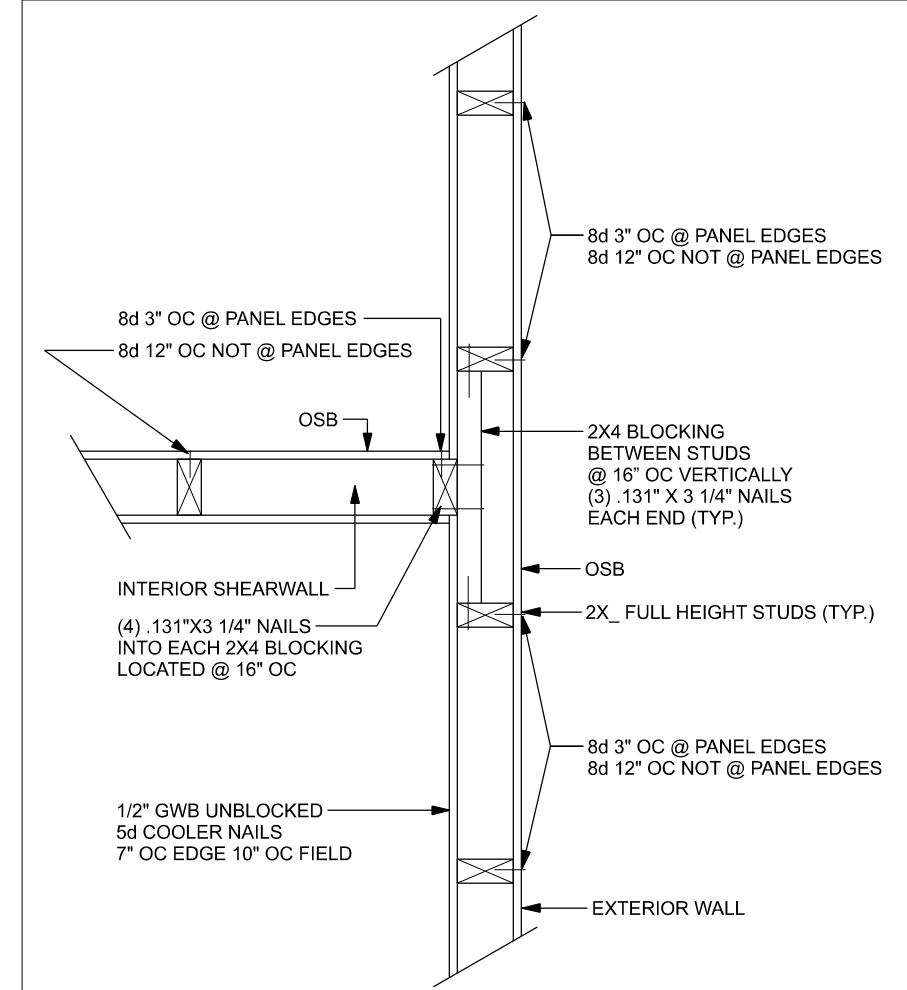




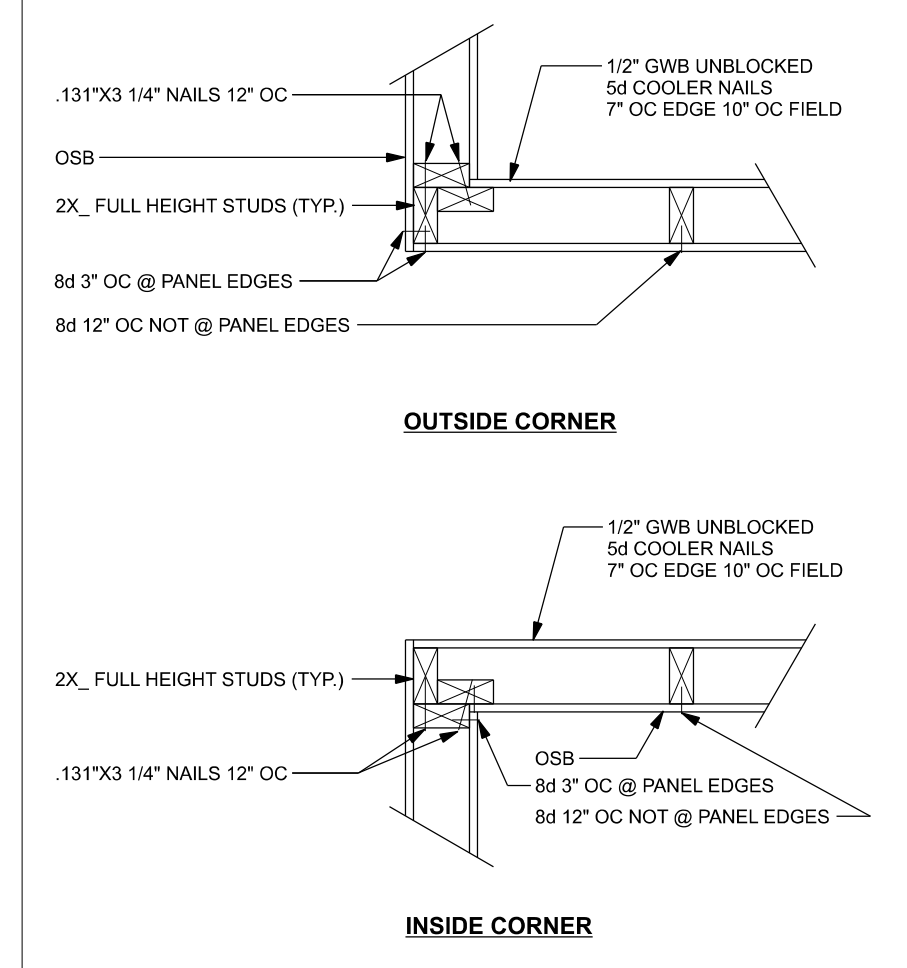
NOTE: SEAL ALL PENETRATIONS IN TOP PLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

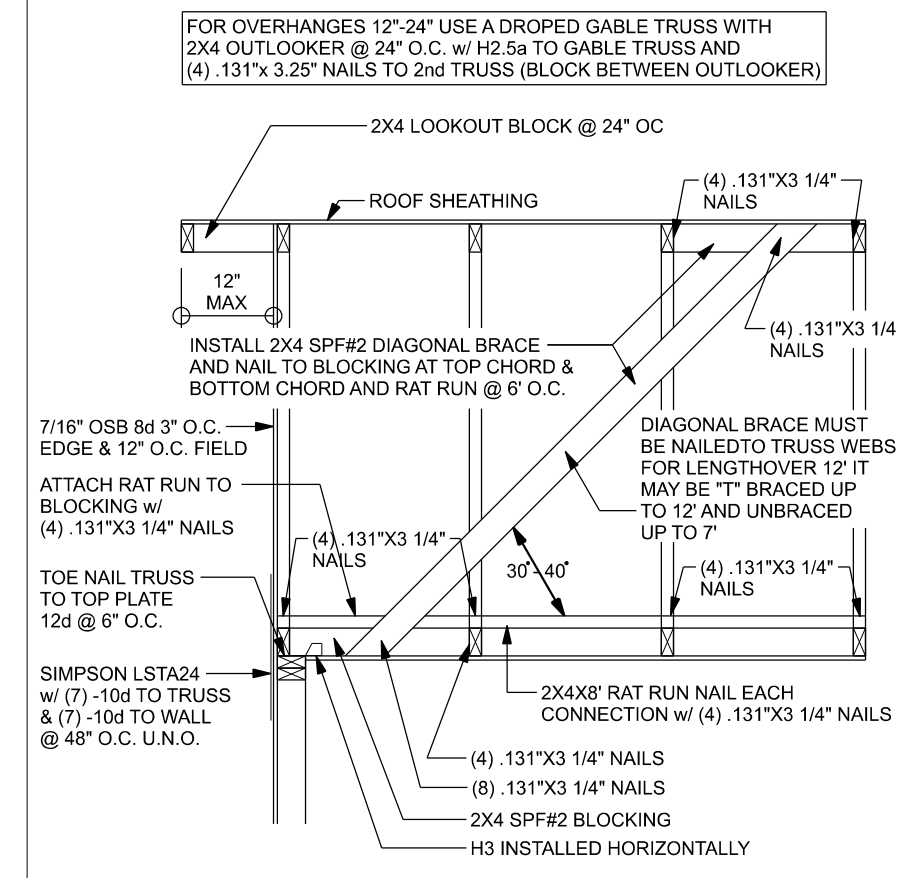


(TYP.) CORNER FRAMING
WOOD FRAME

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.

Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



(TYP.) GABLE BRACING DETAIL
WOOD FRAME

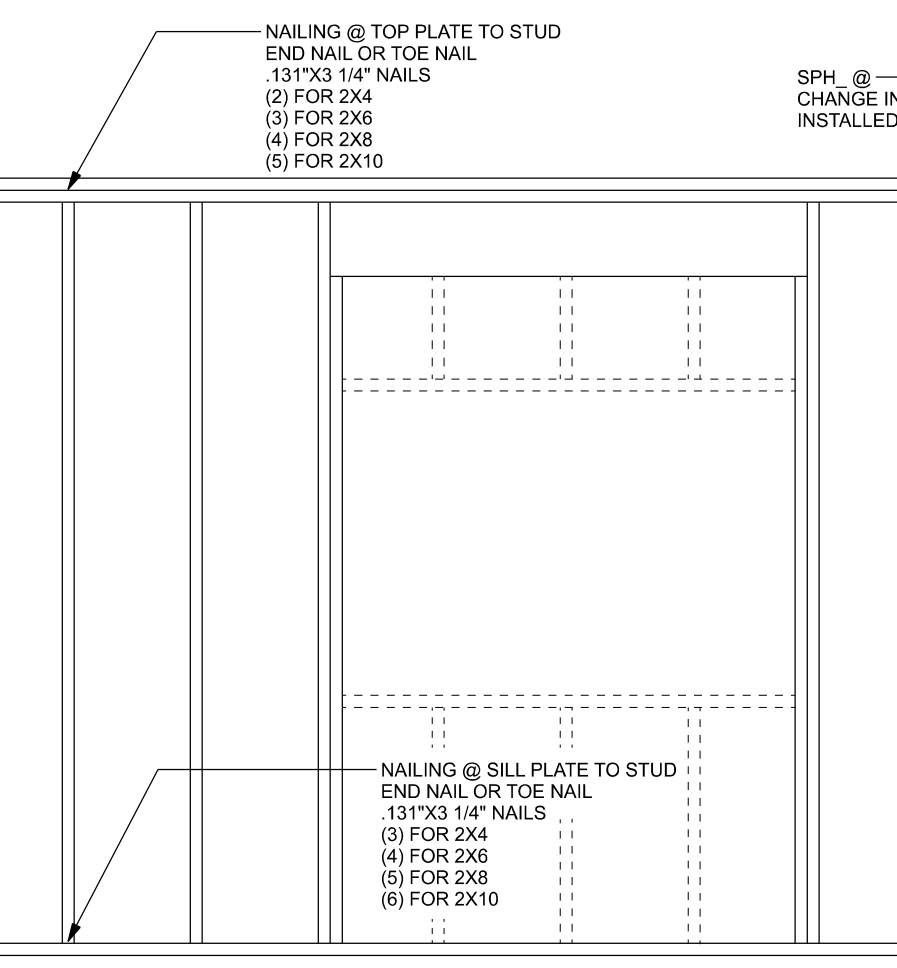
FOR OVER-HANGES 12'-24" USE A DROPPED GABLE TRUSS WITH 2X4 OUTLOOKER @ 24" O.C. w/ H2.5A TO GABLE TRUSS AND (4) 131"X3 1/4" TOE NAILS (BLOCK BETWEEN OUTLOOKER)

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

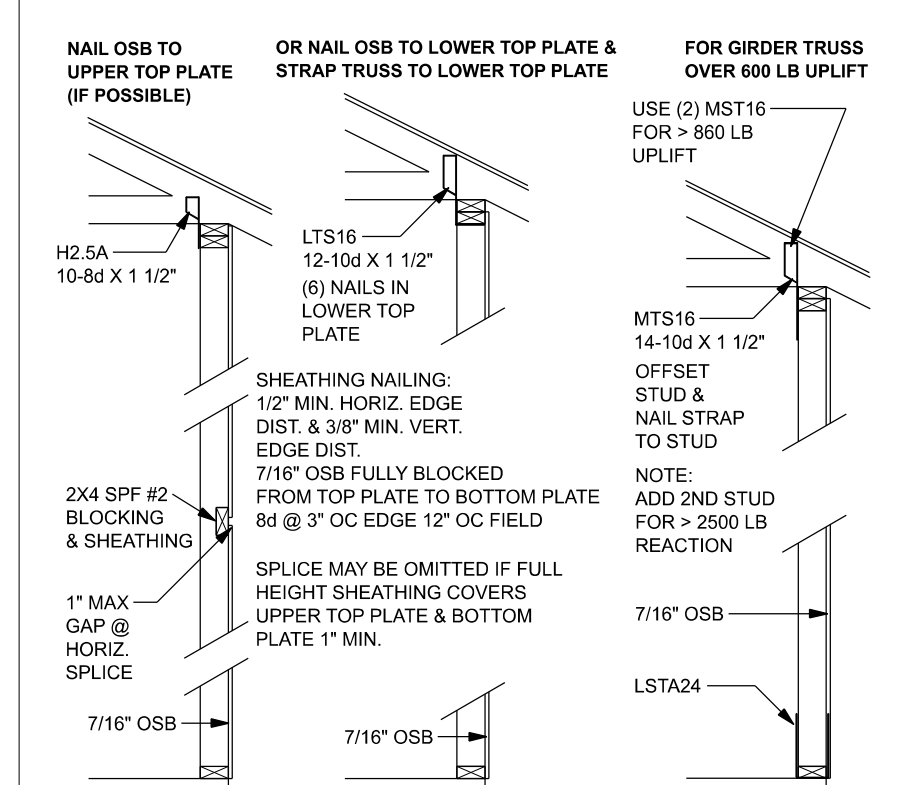
HEADER STRAP TABLE			
Uplift	Top Connection	Bottom Connection	
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate	
< 1455	MSTA24, 18-10d header to jacks	D1T22	
< 1800	(2) MST24, 18-10d header to jacks	D1T22	
< 2910	(2) MST24, 18-10d header to jacks	HTT4	

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8B		
	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6			
130 MPH EXP. C	5'-3" 7'-9" 7'-7" 11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)		

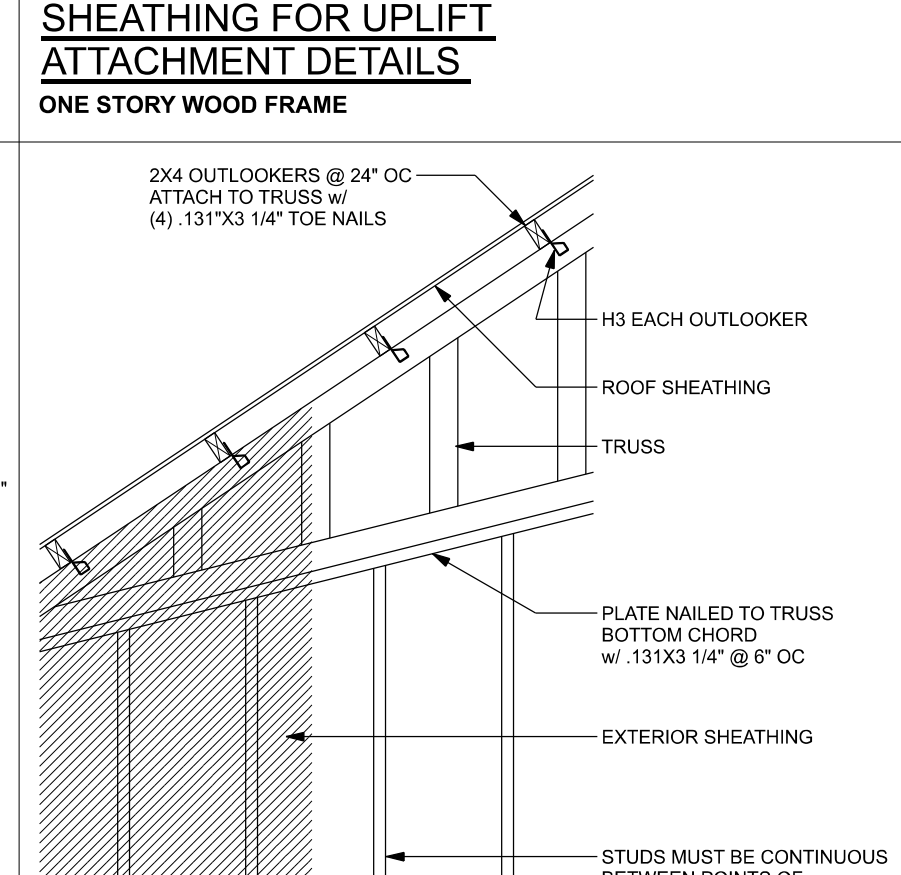
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

(TYP.) CORNER FRAMING
WOOD FRAME

(TYP.) GABLE BRACING DETAIL
WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING
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(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) INTERSECTING WALL FRAMING
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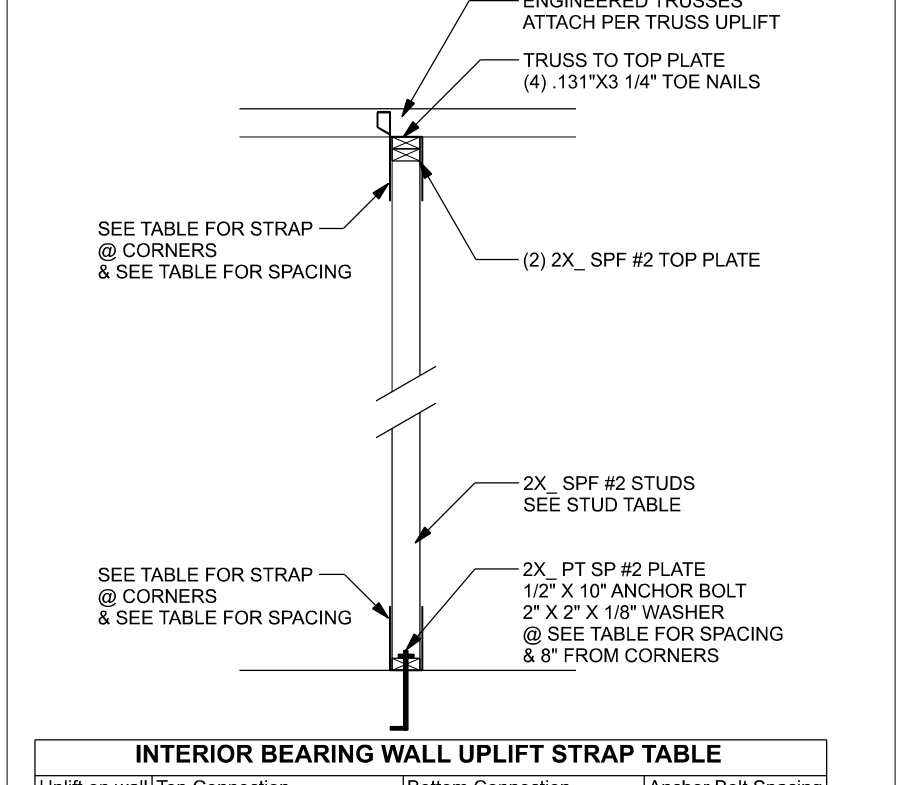
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WOOD FRAME w/ STRAPS & ANCHORS

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WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
575	496	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"	
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"	
720	620	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"	
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"	
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	6-10d	6-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	605	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	16-16d x 1 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	16-16d x 1 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor	
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) PORCH POST
ONE STORY WOOD

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

(TYP.) CORNER FRAMING
WOOD FRAME

(TYP.) GABLE BRACING DETAIL
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(TYP.) GABLE WALL w/ VAULTED CEILING
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(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVALS. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-86, U.S.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS, MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR.

IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THE TRUSS SHEETS.

DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT ²)	ZONE 5 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6 (Vasd) -27.8 (Vasd)	+25.6 (Vasd) -34.2 (Vasd)
0 - 20	+42.6 (Vult) -46.2 (Vult)	+42.6 (Vult) -57 (Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.6 (Vasd) -25.5 (Vasd)
16x7 GARAGE DOOR	+21.7 (Vasd) -24.1 (Vasd)

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

(TYP.) CORNER FRAMING
WOOD FRAME

(TYP.) GABLE BRACING DETAIL
WOOD FRAME

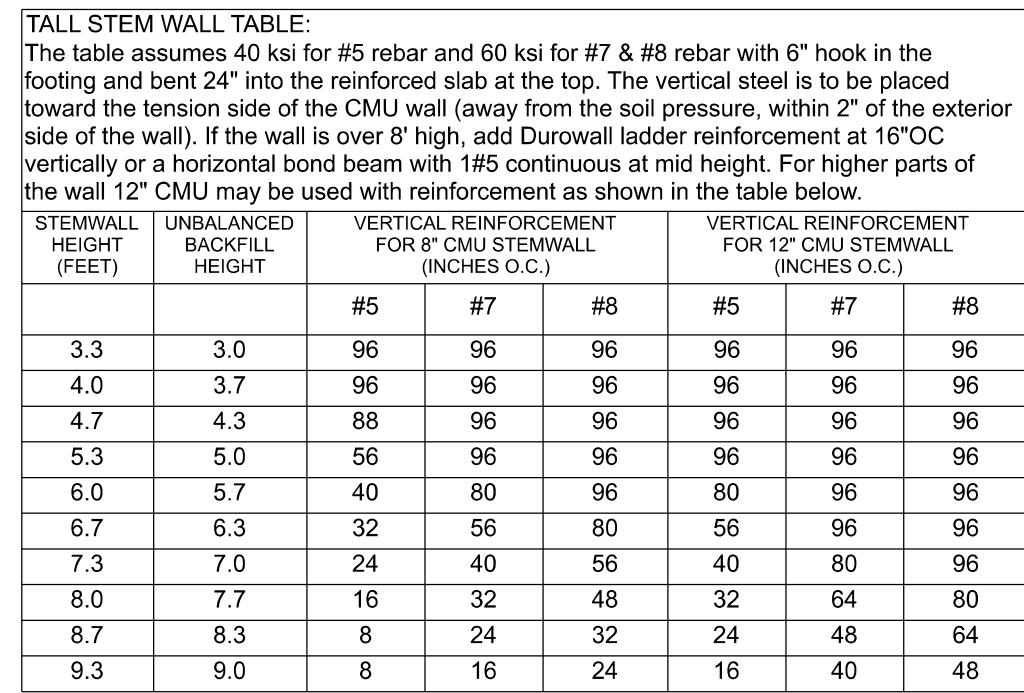
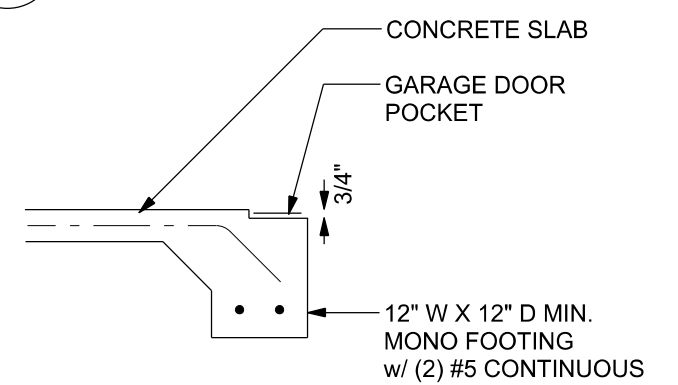
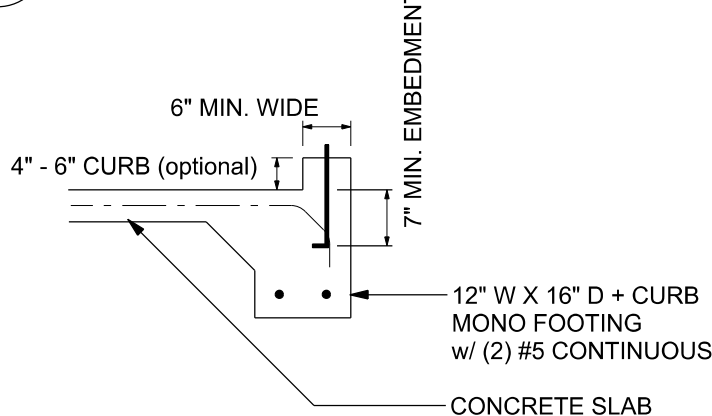
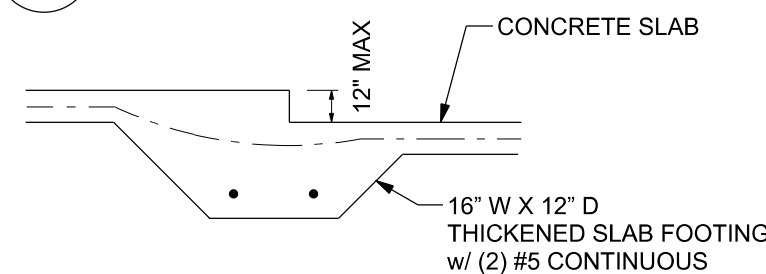
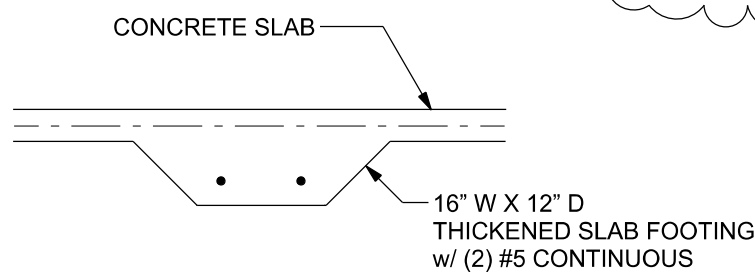
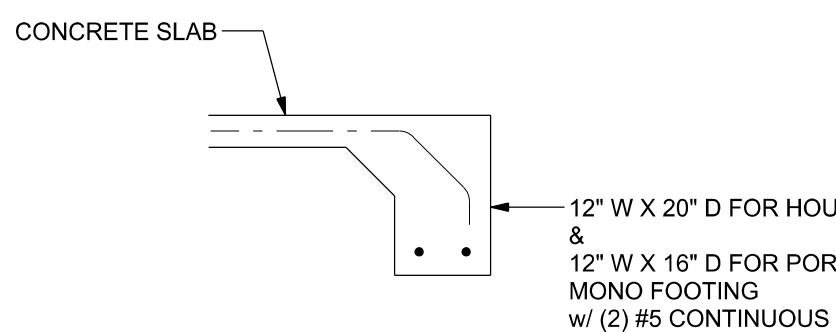
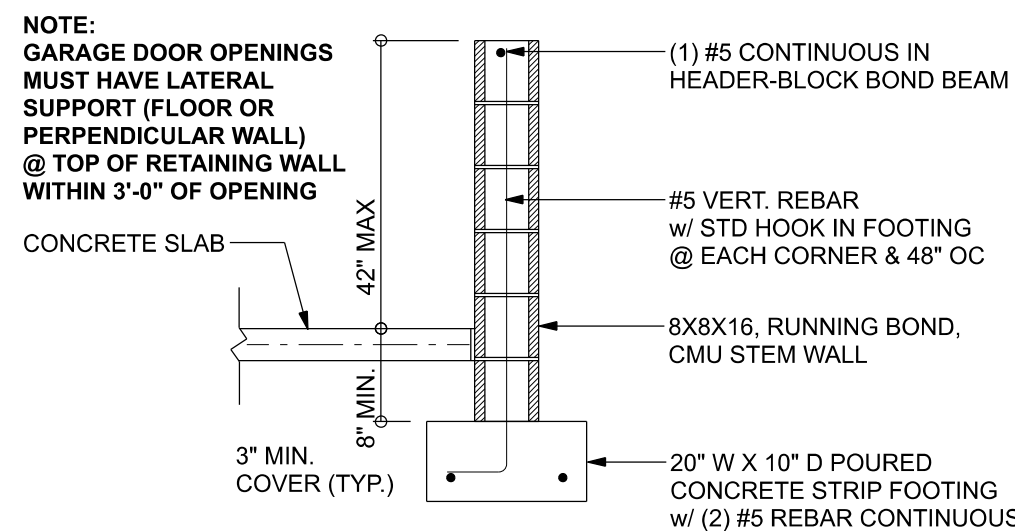
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS

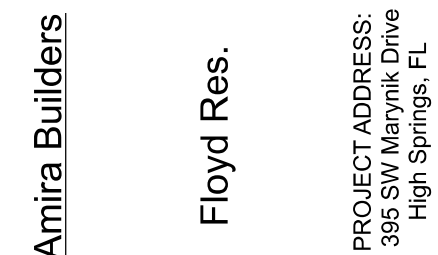
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

(TYP.) CORNER FRAMING
WOOD FRAME

(TYP.) GABLE BRACING DETAIL</



MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530/ASCE8 8/7MS 802). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530-1.02 AND THESE CONSTRUCTION DRAWINGS. ANY EXCEPTIONS TO ACI 530-1.02 MUST BE APPROVED BY THE ENGINEER IN WRITING.		
		Specific Requirements
1.AA	Compressive strength	8" block facing walls (FWS) 1" (500 psi)
2.1	Mortar	ASTM C270, Type N UNO
2.2	CMU standard	ASTM C476, admixtures require approval ASTM C90-02, Normal weight, hollow, medium surface finish, 8" x 8" x 16" turning bond and 12"x12"x16" hollow column
3.1	Clay brick standard	ASTM C216-02, GS2, SW, Type FBS, 5.5"x2.5"x11.5"
3.2	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, #3 to #4 completely embedded in mortar or grout, #5 to #11 min 40 lap dist. (GS2 for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A295, Class GFO, 0.0015 in. 245SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties in walls. Sheet metal ties not completely embedded in mortar or grout, ASTM A1153, Class BZ, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints. If not detailed on project drawings.



DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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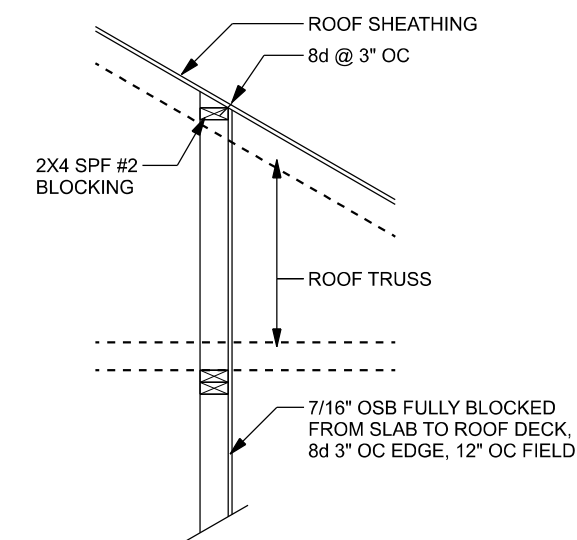


Friday, February 19, 2021

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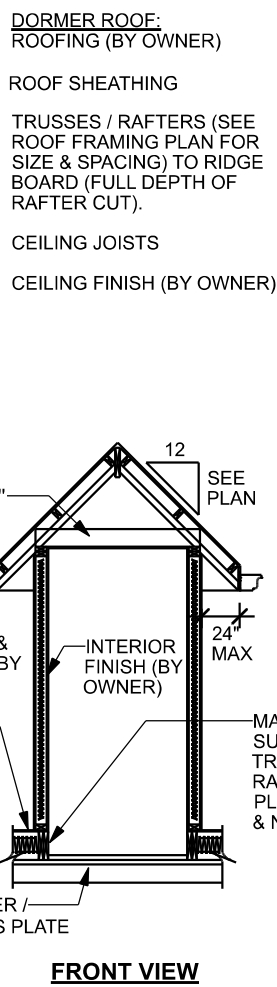
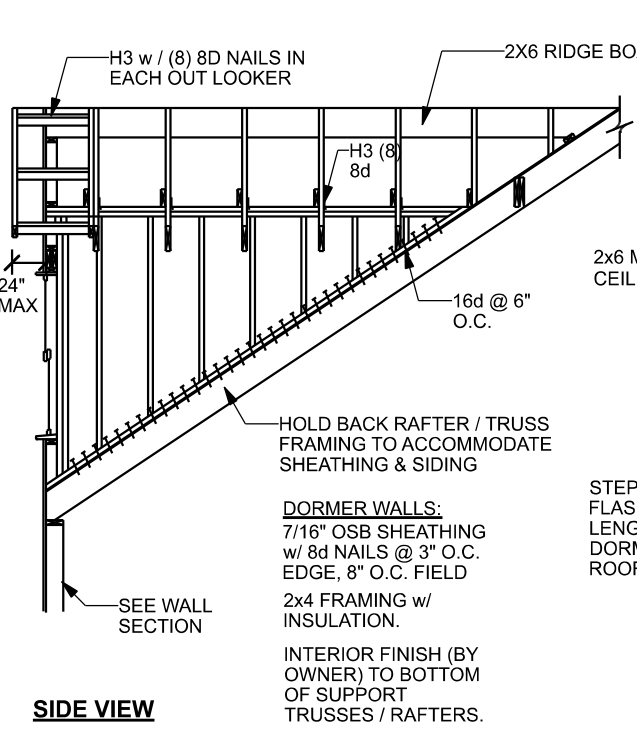
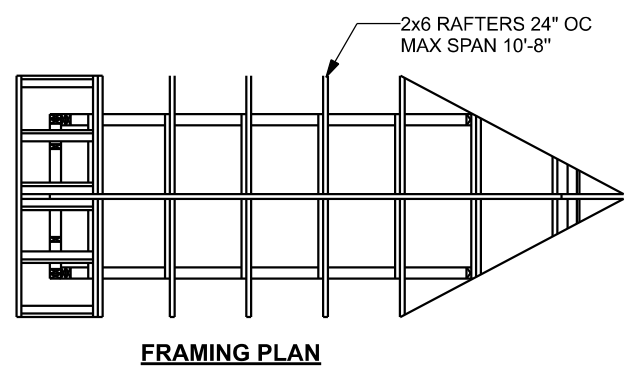
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S-2
OF 3 SHEETS



ALTERNATE IF TRUSSES
ARE PERPENDICULAR TO SHEARWALL

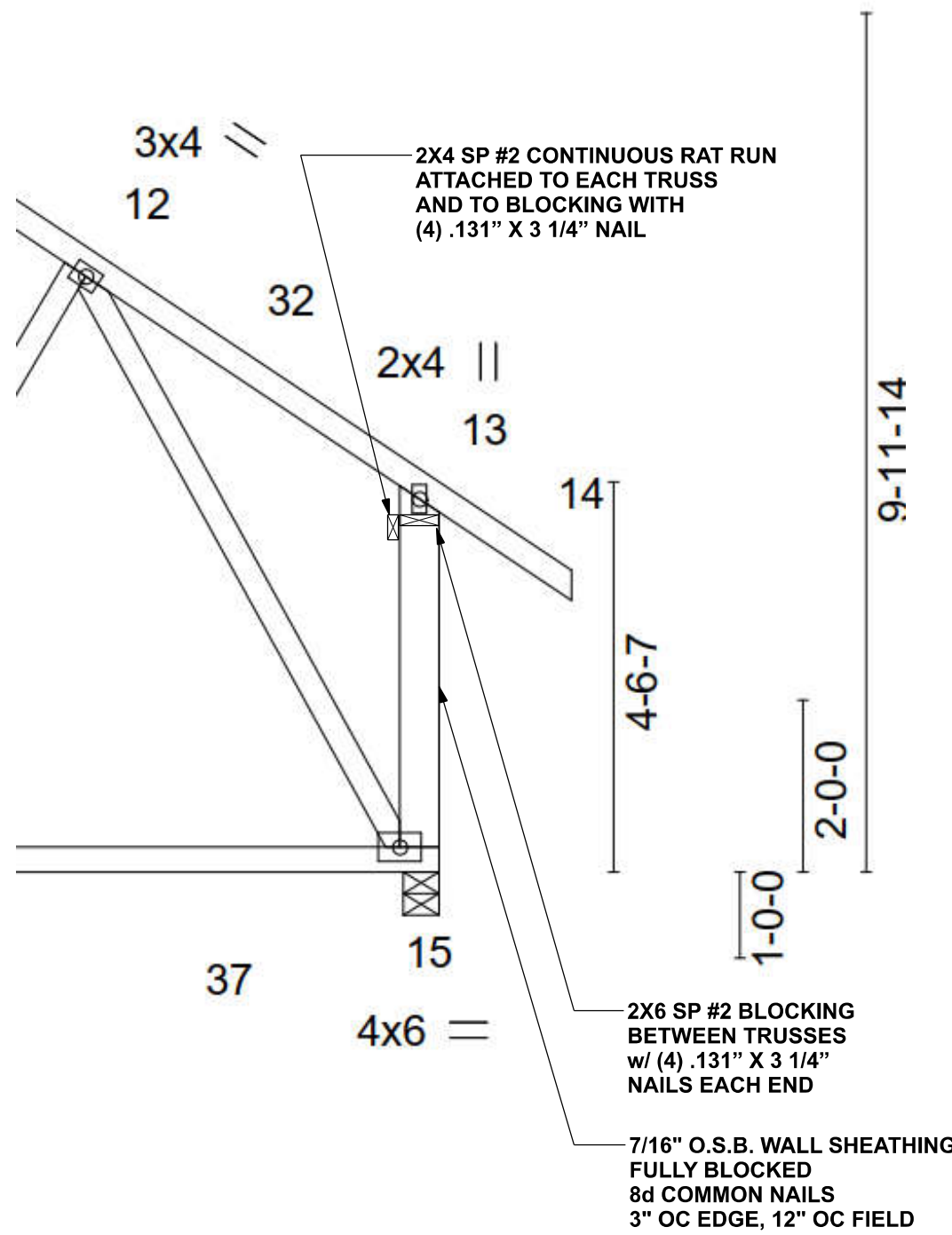
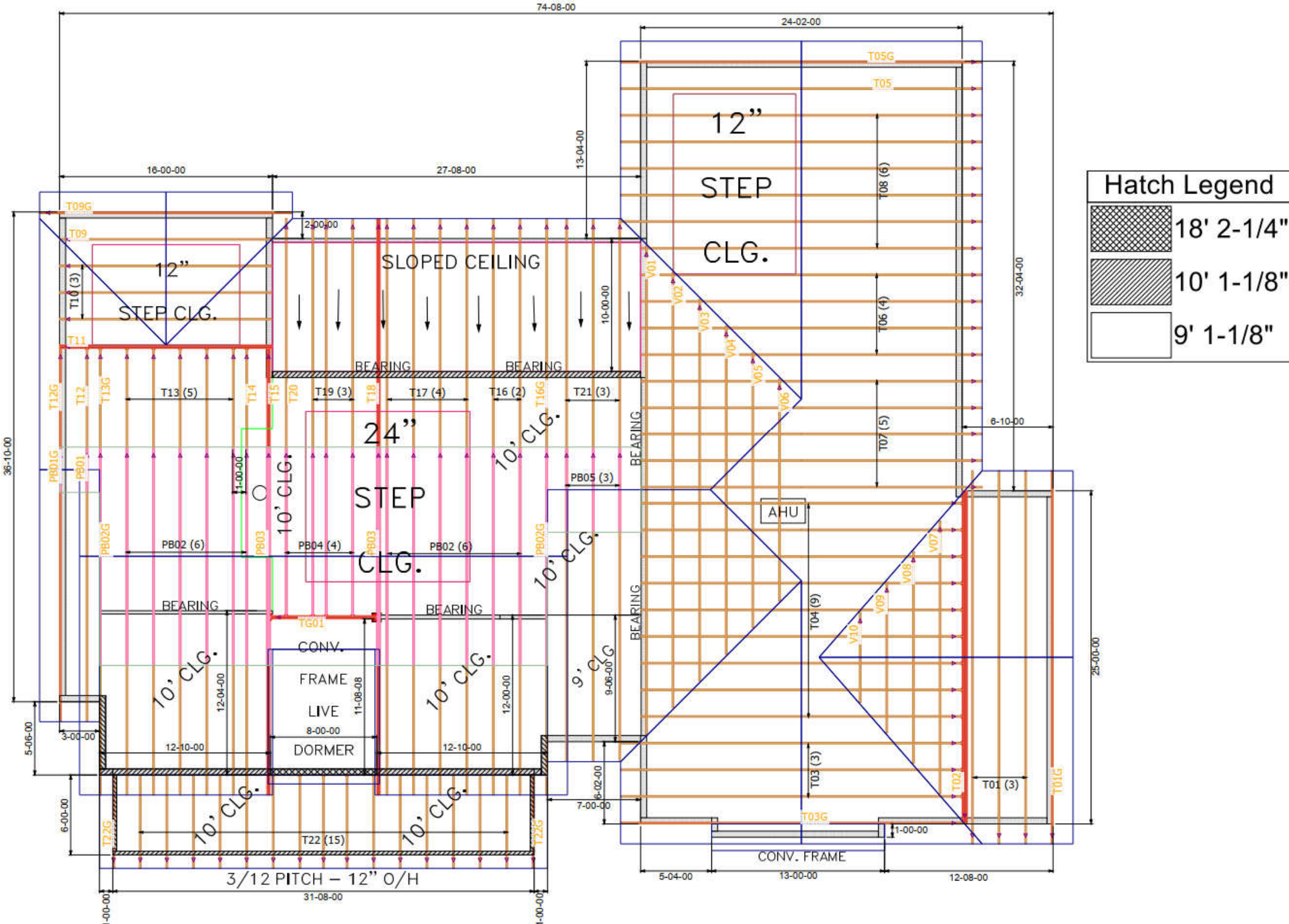
NOTE:
IF THE ABOVE DETAIL IS USED
ON THE REAR WALL
@ THE PORCH
THE PORCH CEILING DOES NOT
NEED TO BE SHEATHED



DORMER ANCHORING DETAIL (ON ROOF)

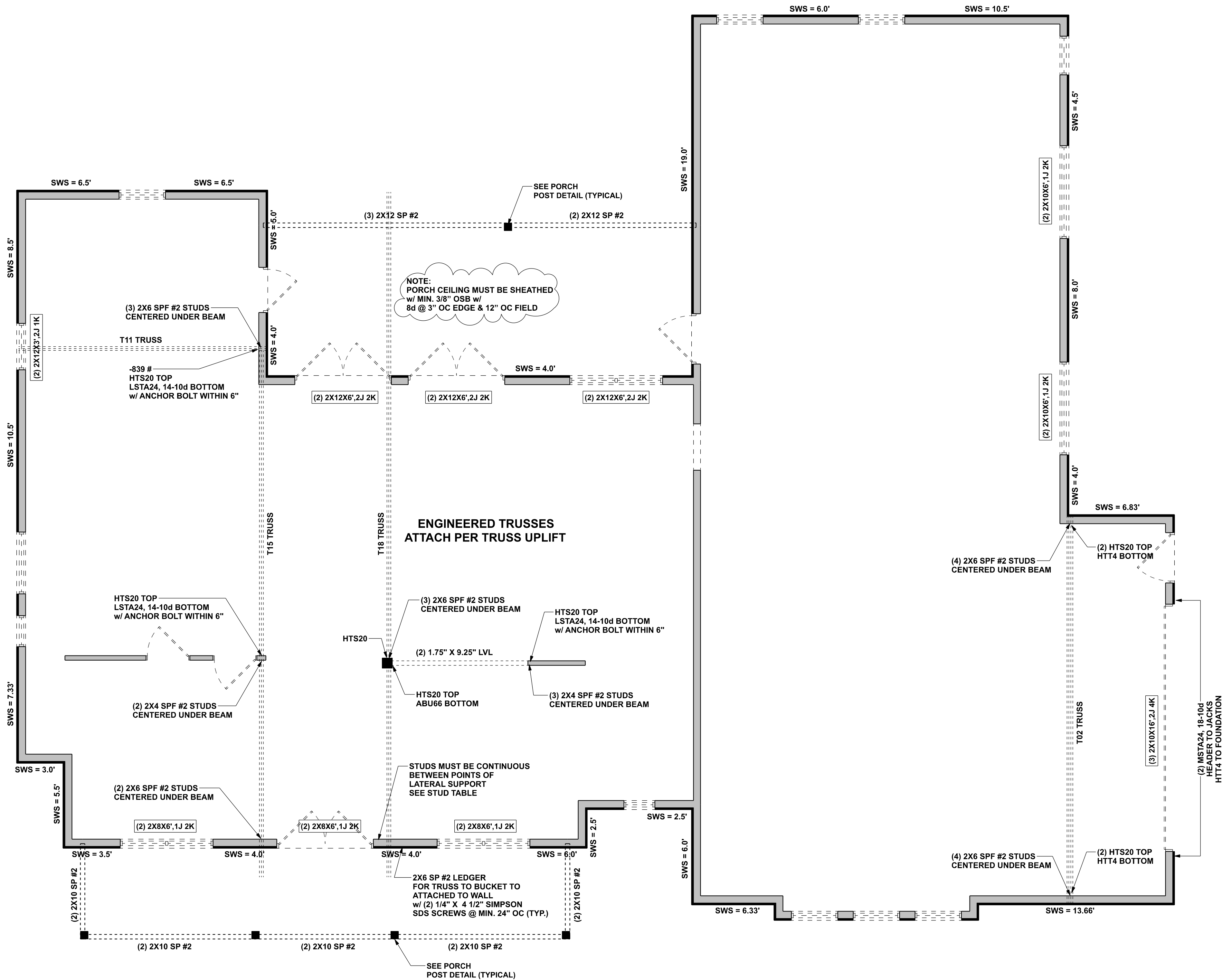
SCALE: N.T.S.

7/12 - 8/12 PITCH - 18" O/H



DETAIL @ TRUSSES WITH RAISED HEELS

SCALE: 1/2" = 1'-0"



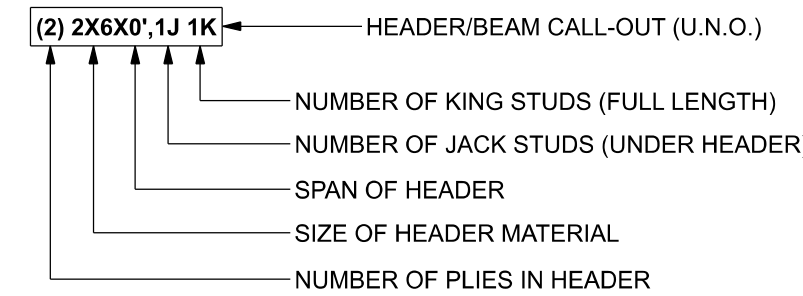
STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL. WRAP UNDER BOTTOM PLATE & OVER TOP PLATE. 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03. BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	25449 LBF	19996 LBF
REQUIRED	23356 LBF	15882 LBF

Amira Builders

Floyd Res.

PROJECT ADDRESS:
3501 Highway 100
High Springs, FL

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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S-3
OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2646866