

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID: ITVO8228Z0206160442

REVIS 512

Truss Fabricator: Anderson Truss Company

Job Identification: 9-136--Doug Edgley Tuwuana Rossin -- , **

Truss Count: 69

Model Code: Florida Building Code 2007 and 2009 Supplement

Truss Criteria: FBC2007Res/TPI-2002(STD)

Engineering Software: Alpine Software, Version 9.02.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-05 -Closed

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Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Seal Date: 10/06/2009

-Truss Design Engineer-

Doug Fleming

Florida License Number: 66648

1950 Marley Drive

Haines City, FL 33844

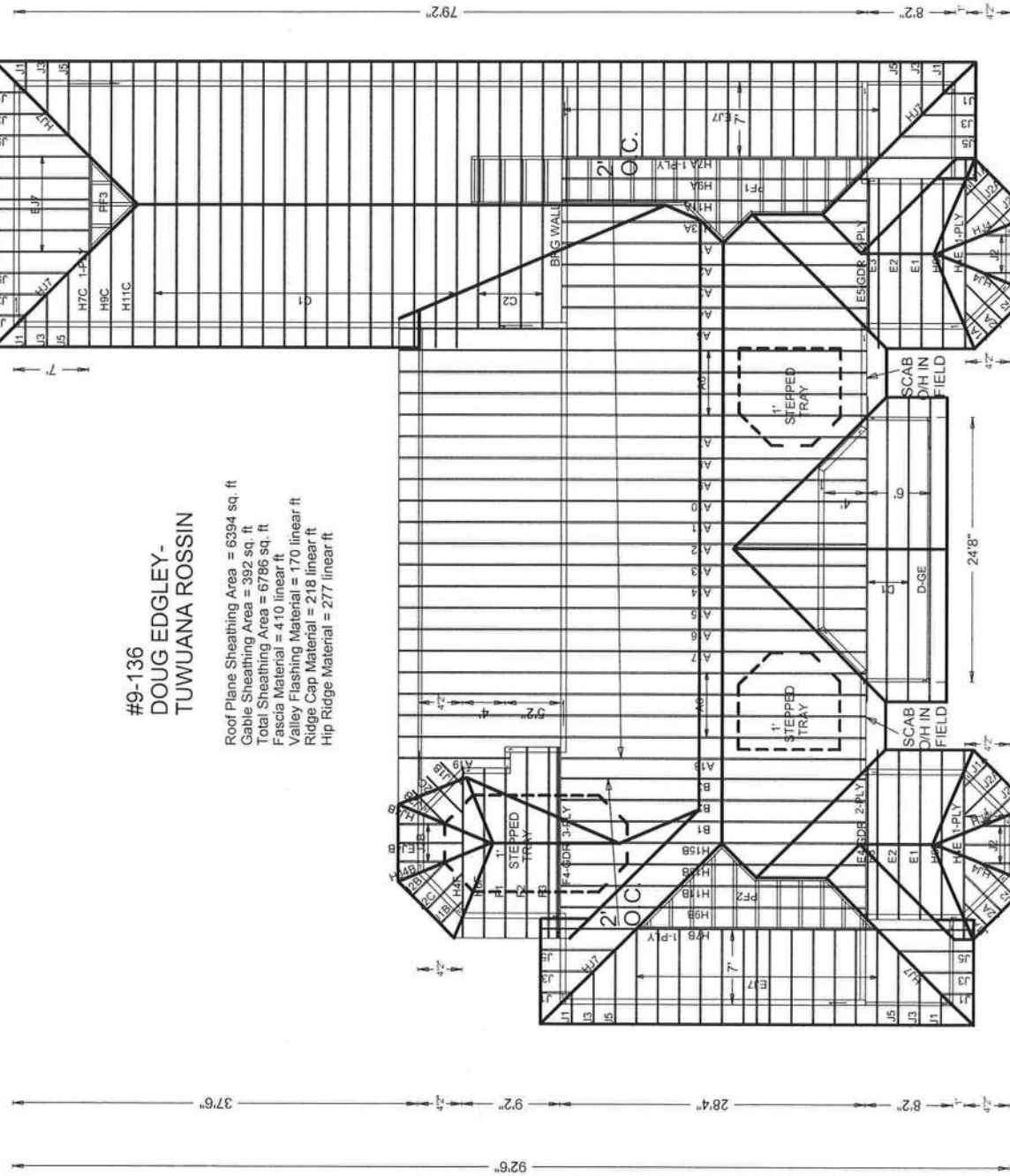
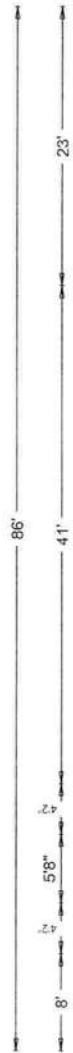
Details: BRCLBSUB-A140GC020109-A140GS020109-HIPFRAME-

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1	43231--A19		09279016	10/06/09
2	43232--A5		09279033	10/06/09
3	43233--A6		09279050	10/06/09
4	43234--A8		09279046	10/06/09
5	43235--A14		09279035	10/06/09
6	43236--A15		09279036	10/06/09
7	43237--A16		09279007	10/06/09
8	43238--A17		09279049	10/06/09
9	43239--A18		09279006	10/06/09
10	43240--A7		09279048	10/06/09
11	43241--A9		09279041	10/06/09
12	43242--A10		09279040	10/06/09
13	43243--A11		09279039	10/06/09
14	43244--A12		09279009	10/06/09
15	43245--A13		09279034	10/06/09
16	43246--H7A		09279027	10/06/09
17	43247--H9A		09279008	10/06/09
18	43248--H11A		09279055	10/06/09
19	43249--H13A		09279010	10/06/09
20	43250--A1		09279002	10/06/09
21	43251--A2		09279011	10/06/09
22	43252--A3		09279012	10/06/09
23	43253--A4		09279013	10/06/09
24	43254--H7B		09279044	10/06/09
25	43255--H9B		09279018	10/06/09
26	43256--H11B		09279014	10/06/09
27	43257--H13B		09279022	10/06/09
28	43258--H15B		09279015	10/06/09
29	43259--B1		09279016	10/06/09
30	43260--B2		09279017	10/06/09
31	43261--B3		09279023	10/06/09
32	43262--H7C		09279001	10/06/09
33	43263--H9C		09279010	10/06/09
34	43264--H11C		09279021	10/06/09
35	43265--C1		09279018	10/06/09
36	43266--C2		09279019	10/06/09

#	Ref	Description	Drawing#	Date
37	43267--D1		09279013	10/06/09
38	43268--D-GE		09279015	10/06/09
39	43269--H4E		09279004	10/06/09
40	43270--H6E		09279005	10/06/09
41	43271--E1		09279053	10/06/09
42	43272--E2		09279054	10/06/09
43	43273--E3		09279009	10/06/09
44	43274--E4-GDR		09279020	10/06/09
45	43275--E5-GDR		09279037	10/06/09
46	43276--H4F		09279052	10/06/09
47	43277--H6F		09279017	10/06/09
48	43278--F4-GDR		09279021	10/06/09
49	43279--F1		09279020	10/06/09
50	43280--F2		09279012	10/06/09
51	43281--F3		09279011	10/06/09
52	43282--HJ7		09279014	10/06/09
53	43283--J1		09279019	10/06/09
54	43284--J3		09279023	10/06/09
55	43285--J5		09279031	10/06/09
56	43286--EJ7		09279032	10/06/09
57	43287--J1A		09279029	10/06/09
58	43288--J2A		09279030	10/06/09
59	43289--J2		09279025	10/06/09
60	43290--HJ4		09279024	10/06/09
61	43291--EJ4		09279026	10/06/09
62	43292--J1B		09279045	10/06/09
63	43293--J2C		09279051	10/06/09
64	43294--J2B		09279042	10/06/09
65	43295--HJ4B		09279038	10/06/09
66	43296--EJ4B		09279043	10/06/09
67	43297--PF2		09279047	10/06/09
68	43298--PF3		09279003	10/06/09
69	43299--PF1		09279028	10/06/09

Permit # 28035





**#9-136
DOUG EDGLEY-
TUWUANA ROSSIN**

Roof Plane Sheathing Area = 6394 sq. ft
 Gable Sheathing Area = 392 sq. ft
 Total Sheathing Area = 6786 sq. ft
 Fascia Material = 410 linear ft
 Valley Flashing Material = 170 linear ft
 Ridge Cap Material = 218 linear ft
 Hip Ridge Material = 277 linear ft

JOB DESCRIPTION: Doug Edgley
 / Tuwuana Rossin

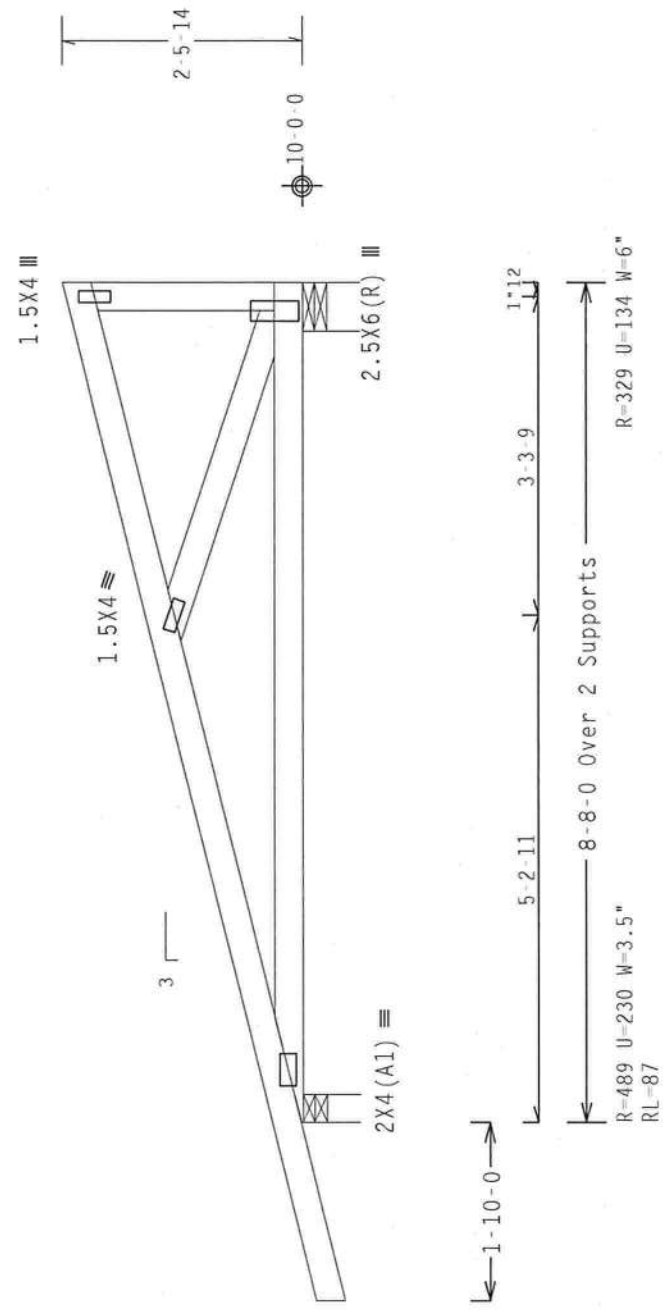
JOB NO:
 9-136

PAGE NO:
 1 OF 1

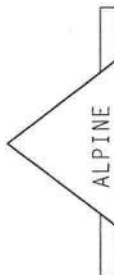
(9-136--Doug Edgley Tuwuana Rossin -- , ** - A19)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55
Wind reactions based on MWFRS pressures.
Right end vertical not exposed to wind pressure.
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278		QTY: 1		Scale = 5" / Ft.	
			TC LL	20.0 PSF	REF	R8228- 43231
			TC DL	10.0 PSF	DATE	10/06/09
			BC DL	10.0 PSF	DRW	HCUSR8228 09279016
			BC LL	0.0 PSF	HC-ENG	KD/AP
			TOT.LD.	40.0 PSF	SEQN-	48718
			DUR.FAC.	1.25	FROM	AH
			SPACING	24.0"	JREF-	1TV08228Z02

110 mph wind, 15.00 ft mean ht, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.55

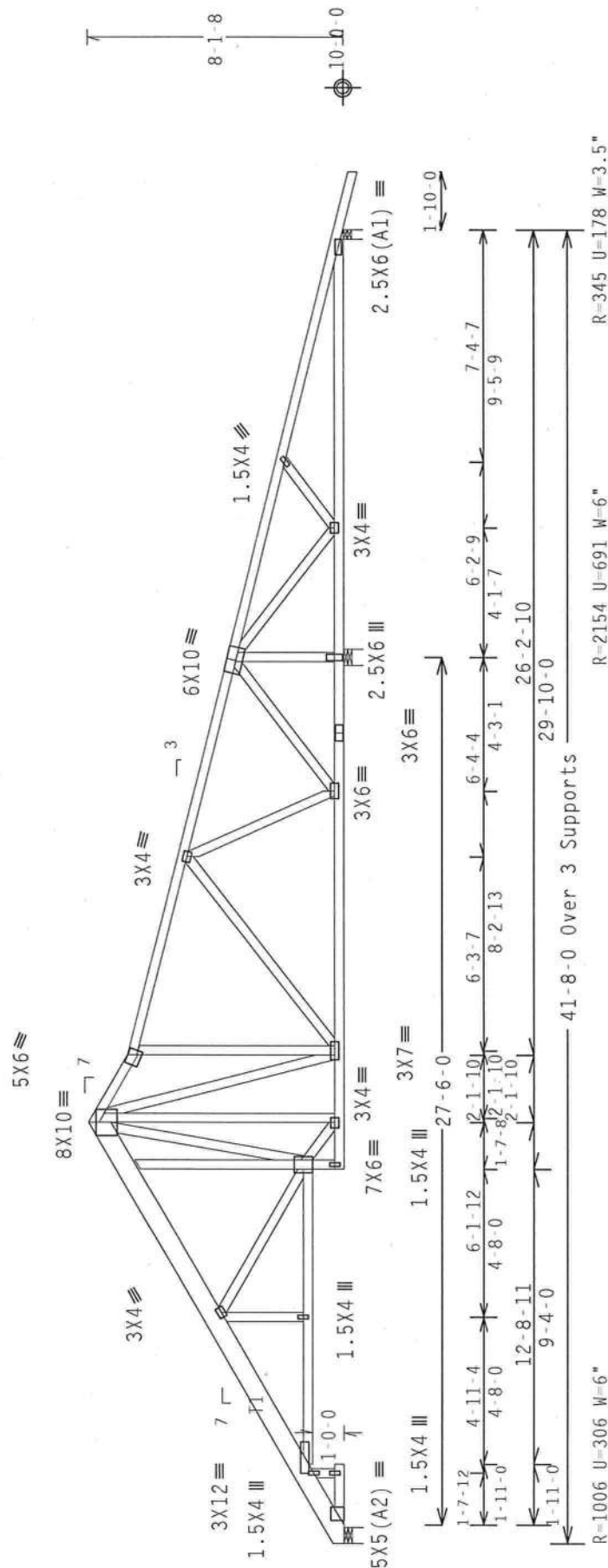
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

Bottom chord checked for 10.00 psf non-concurrent live load.

NOTE: Laterally brace bottom chord above filler at 2'0" o.c.

MAX. INCLUDING A LATERAL BRACE AT CHORD ENDS.

Design Crit: FBC2007Res/TPI - 2002(STD)
 $FT/RT = 10\% (0\%) / 0 (0)$

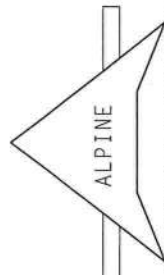
PLT TYP. Wave

OTY:8 FL/-/5/-/-/R/-
Scale = 1875"/Ft.

TC LL	20.0 PSF	REF	R8228 - 43233
TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCUSR8228 09279050
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN -	48989
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF -	1TV08228202

[illegible]

BE RESPONSIBLE FOR THE INSTALLATION CONTRACTOR, THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS DESIGN SPECIFICATIONS, OR ANY FAILURE TO FOLLOW THE TRUSS DESIGN SPECIFICATIONS. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS DESIGN SPECIFICATIONS, OR ANY FAILURE TO FOLLOW THE TRUSS DESIGN SPECIFICATIONS.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCPI (+/-)=0.55

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

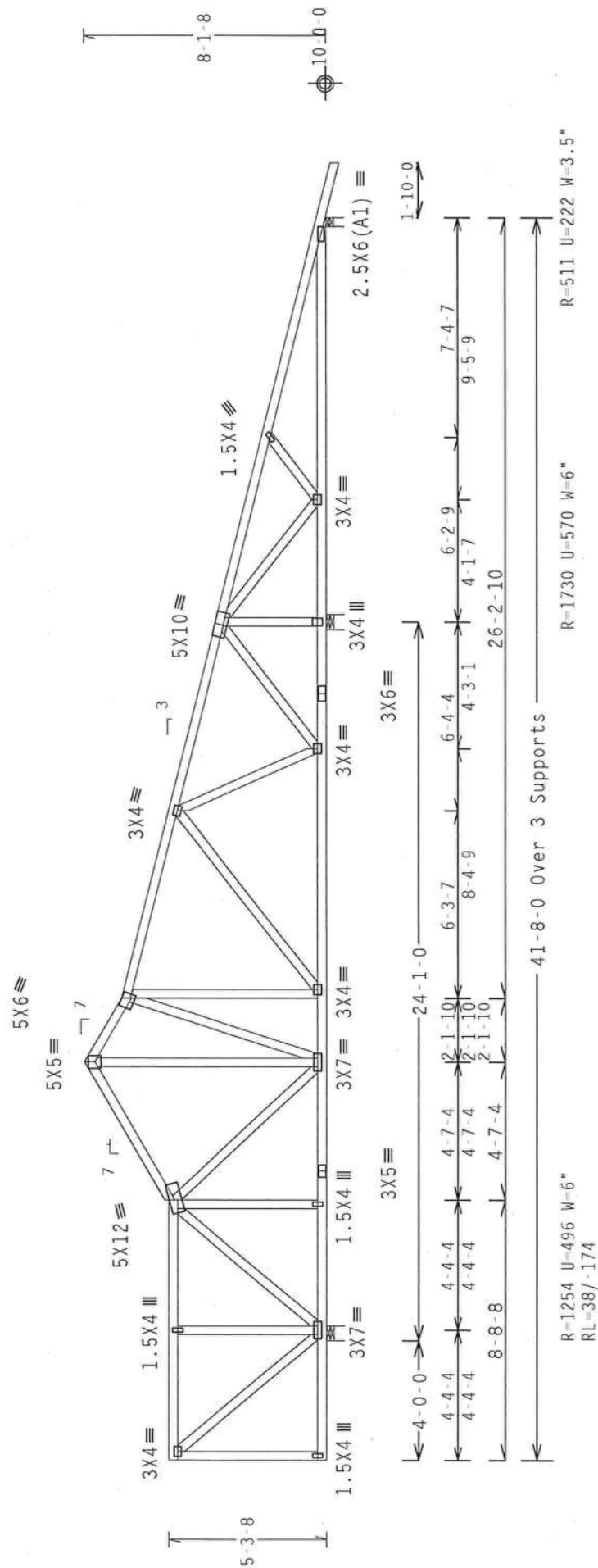
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 9.02.00

Scale = .1875"/Ft.

WARNING THOMSONS REQUIRE EXTREME CARE IN FABRICATING, AND THE SHIPPING, INSTALLING AND BRACING OF TRUSSES. THOMSONS HAS A REPUTATION FOR EXCELLENCE IN THE DESIGN AND CONSTRUCTION OF STEEL TRUSSES. THOMSONS IS LOCATED AT 6000 LITTLE STREET, SUITE 319, ALEXANDRIA VA, 22314) AND WE CAN PROVIDE TRUSS CHAIRS OF AMERICA, 5000 EMBLETTIE LANE, SUITE 517, 53719). TO PREFORM THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLS.

TC LL	20.0	PSF	REF R8228 - 43235
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUR8228 09279035
BC LL	0.0	PSF	HC-ENG KD/AP
TOT.LD.	40.0	PSF	SEQN - 49062
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF - 1TV08228Z02



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{Cpi}(v/-)=0.55$

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

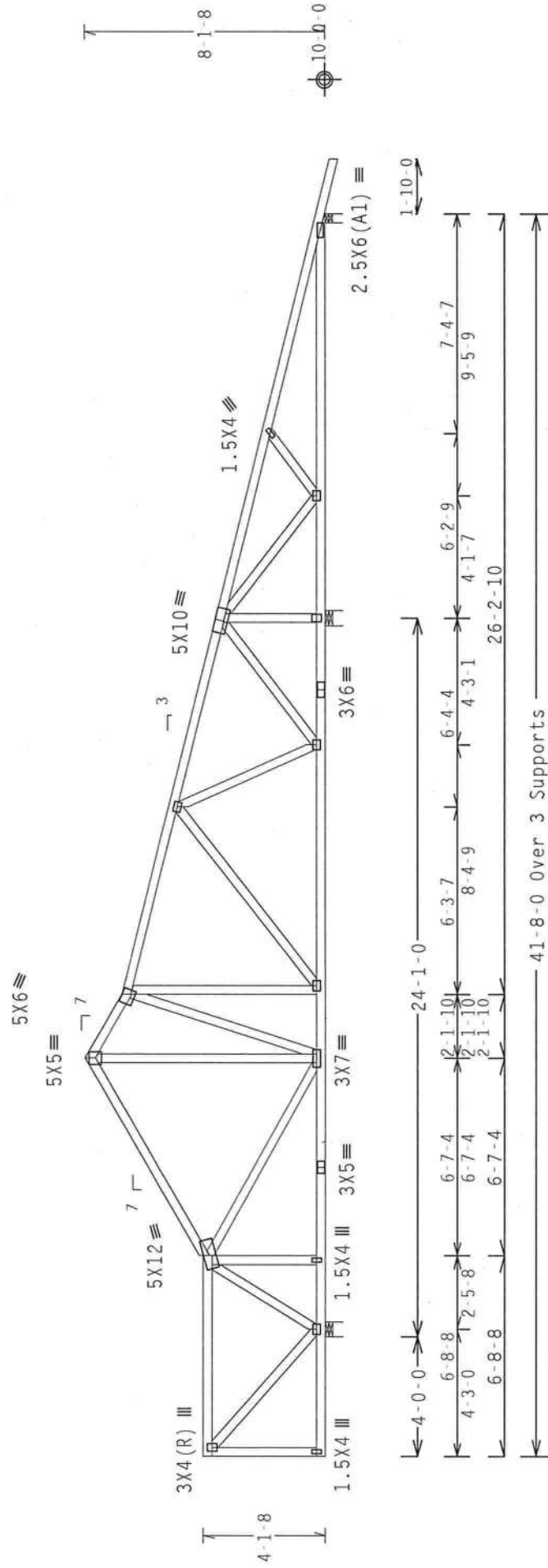
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1254 U=444 W=6"

RL=65/-185

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD

FT/RT=10% (0%) / 0 (0)

9.02.00

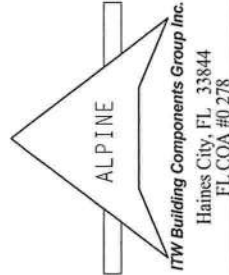
QTY:1 FL/-/5/-/-/R/-/

Scale = .1875"/Ft.

9.02.00

QTY:1 FL/-/5/-/-/R/-/

Scale = .1875"/Ft.

[illegible][illegible]

TC LL	20.0 PSF	REF	R8228- 43236
TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCUSR8228 09279036
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	49072
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TV08228Z02

(9-136-Doug Edgley Tuwana Rossin -- , ** - A16)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

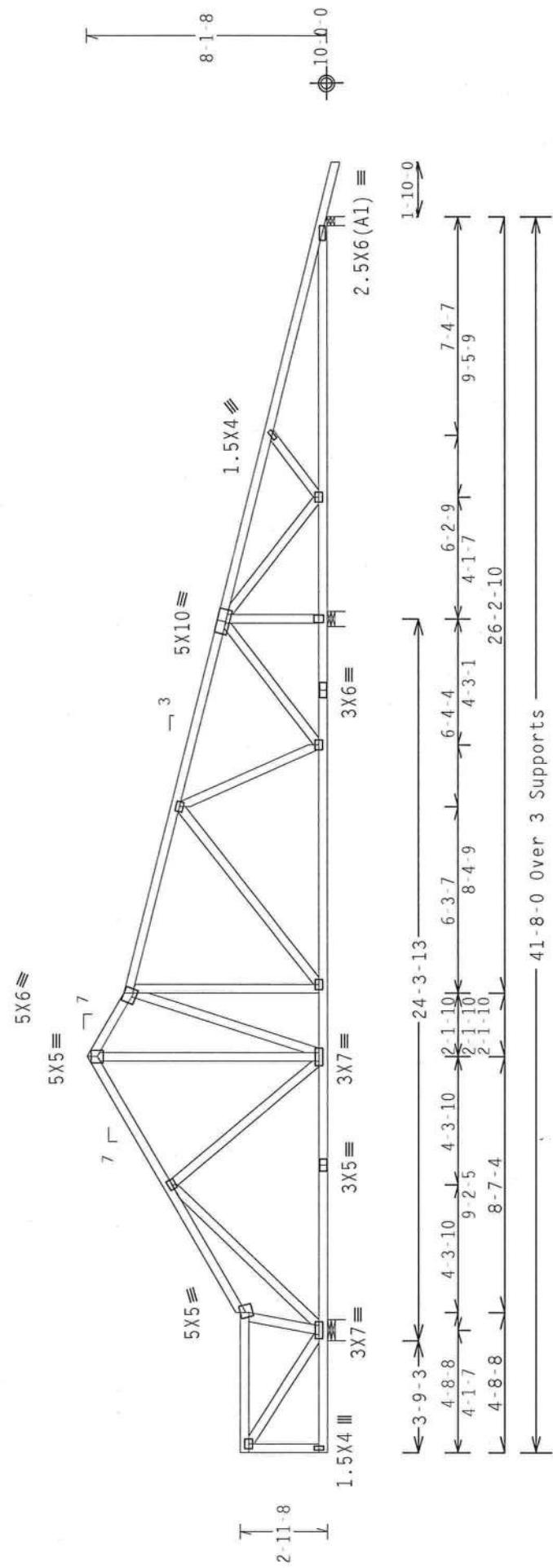
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.55$

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=1250 U=386 W=8.485"
RL=91/-197

R=1747 U=586 W=6"

R=508 U=222 W=3.5"

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

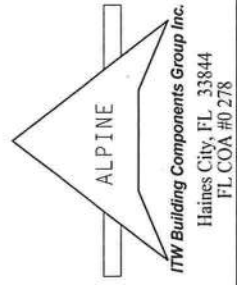
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		BC LL	0.0 PSF	HC-ENG	KD/AP
		TOT.LD.	40.0 PSF	SEQN-	49081
		DUR.FAC.	1.25	FROM	AH
		SPACING	24.0"	JREF-	1TV08228Z02



****WARNING**** TRUSSES ROUTINE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESIDENTIAL BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48071) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY AIA/ASCE AND TPI). TPI BCG CONNECTOR PLATES ARE MADE OF 2018/16GA (4-H/55/K) ASIN A553 GRADE 40/60 (4, 4/8, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) DESIGN GUIDE 9, THE STEEL EDITION OF THE AISC 13TH EDITION, AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER. THE DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



	Top	Bot	Dense
chord	2x4	2x4	SP #2
chord	2x4	2x4	SP #2
webs	2x4	2x4	SP #3

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCpi(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

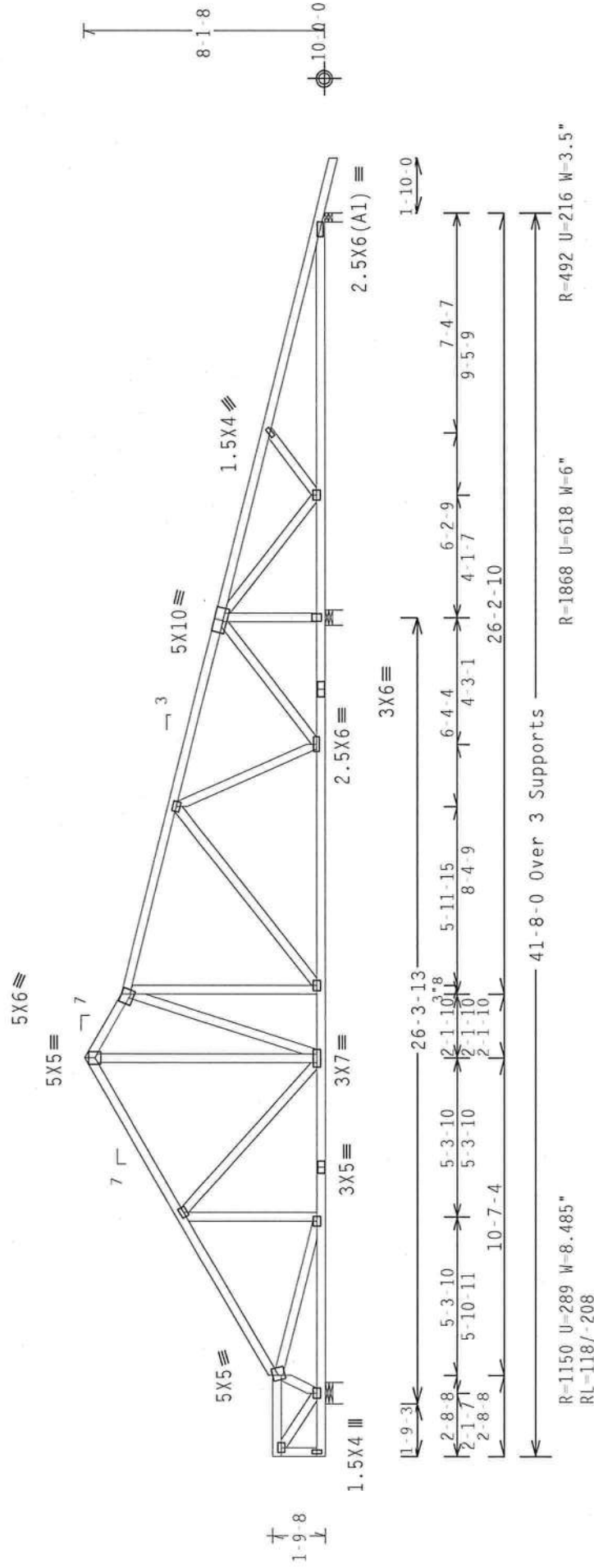
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

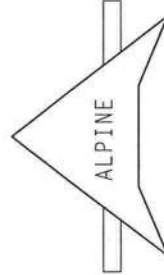
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

1.002 (316)
9.02.00

QTY:1 FL/-/5/-/-/R/-/

Scale = .1875"/Ft.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

*****WARNING***** TROSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO UCSTI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY UCSTI (CHASSIS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND A-6000 (FLOOR JOIST CONNECTIONS FOR CONCRETE JOISTS), PUBLISHED BY UCSTI (CHASSIS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) FOR PROPER TROSS DESIGN, FABRICATION, AND INSTALLATION. TROSSES SHOULD BE PROPERLY ATTACHED TO THE TOP OF THE JOIST. OTHERS INDICATED FOR CEILING SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GIRDERS SHALL HAVE PROPERLY ATTACHED PLATE, ETC.



TC LL	20.0	PSF	REF	R8228-	43238
TC DL	10.0	PSF	DATE	10/06/09	
BC DL	10.0	PSF	DRW	HCHSR8228	09279049
BC LL	0.0	PSF	HC-ENG	KD/AP	
TOT.LD.	40.0	PSF	SEQN-	49089	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TV08228202	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

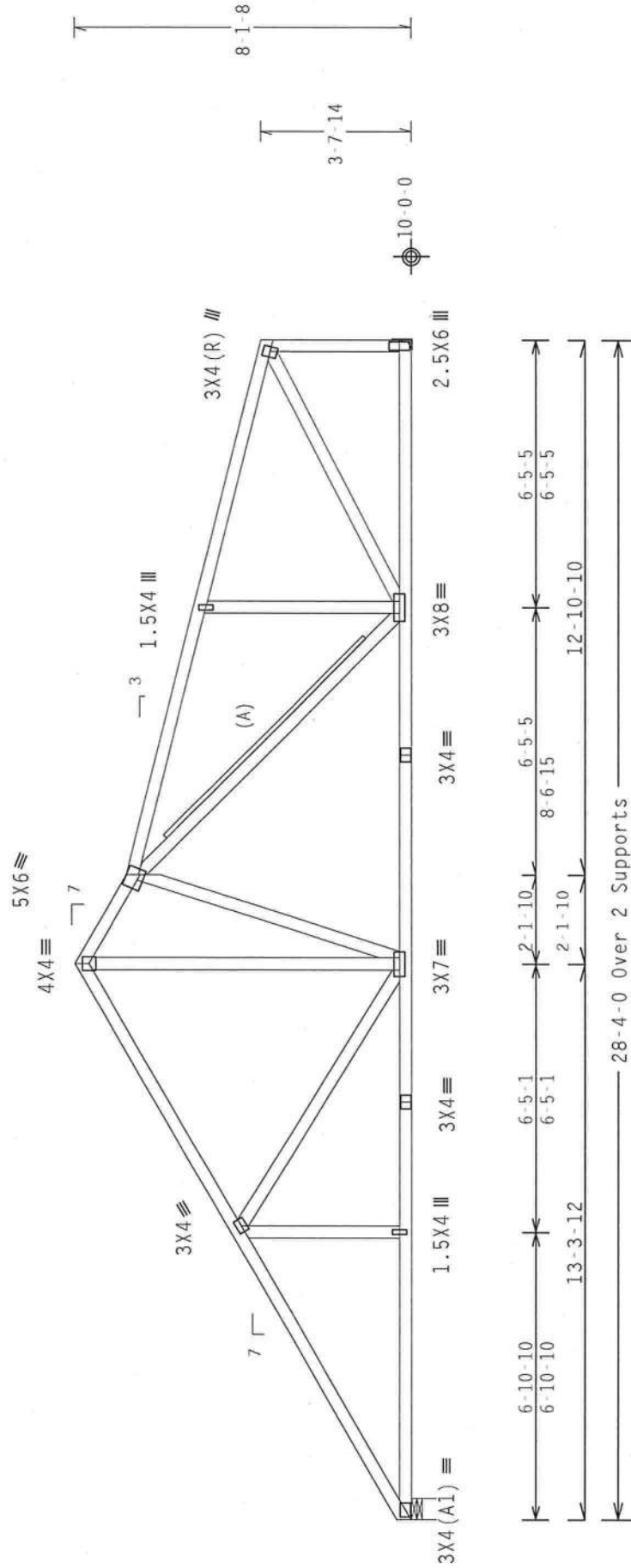
Deflection meets L/240 live and L/180 total load.

	Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense	
	Webbs	2x4	SP	#3	

(J) hanger connection not found in inventory file for this condition. Provide connection.

(A) 1x4 #3SRB SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=1177 U=126 W=6"
RL=163/-144

R=1147 U=184 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (312X0 min. (H)

QTY:1 FL1-151-1-R1- Scale = 25"/Ft.

Design Crit: FBC2007Res/TPI - 2002(STD)
FT/RT=10%(0%) / 0(0)

3CZ007/Kes/1FT-200Z (310)
FT/RT=10%(0%)/0(0) 9.02.02

PLT TYP. Wave

[illegible]

****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR FOR ITR BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1-0 OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGNER'S COMMENTS: PROVIDE LIGHT PROTECTIVE COATINGS (RATIONAL DESIGNER SEE THE ACPA) AND TYP. 150 BCG TRUSS COMPANY, 1700 E. 9TH AVENUE, DENVER, COLORADO 80202-6000. USE 1/2" STEEL PLATES TO ATTACH TRUSS TO WALLS.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. AN INSPECTION OF PLATES FOLLOWED BY (U) SHALL BE PER AMBER A3 OF TP1-2002 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THIS PROJECT. NO OTHER PARTS OF THIS CONSTRUCTION ARE COVERED BY THIS DRAWING.

FOR THIS COMMENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMBER TP1-1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 43239
TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCU8228 09279006
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	49097
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TV08228Z02

	Top	Bot	Dense
chord	2x4	2x4	SP #2
chord	2x4	2x4	SP #2
webs	2x4	2x4	SP #3

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

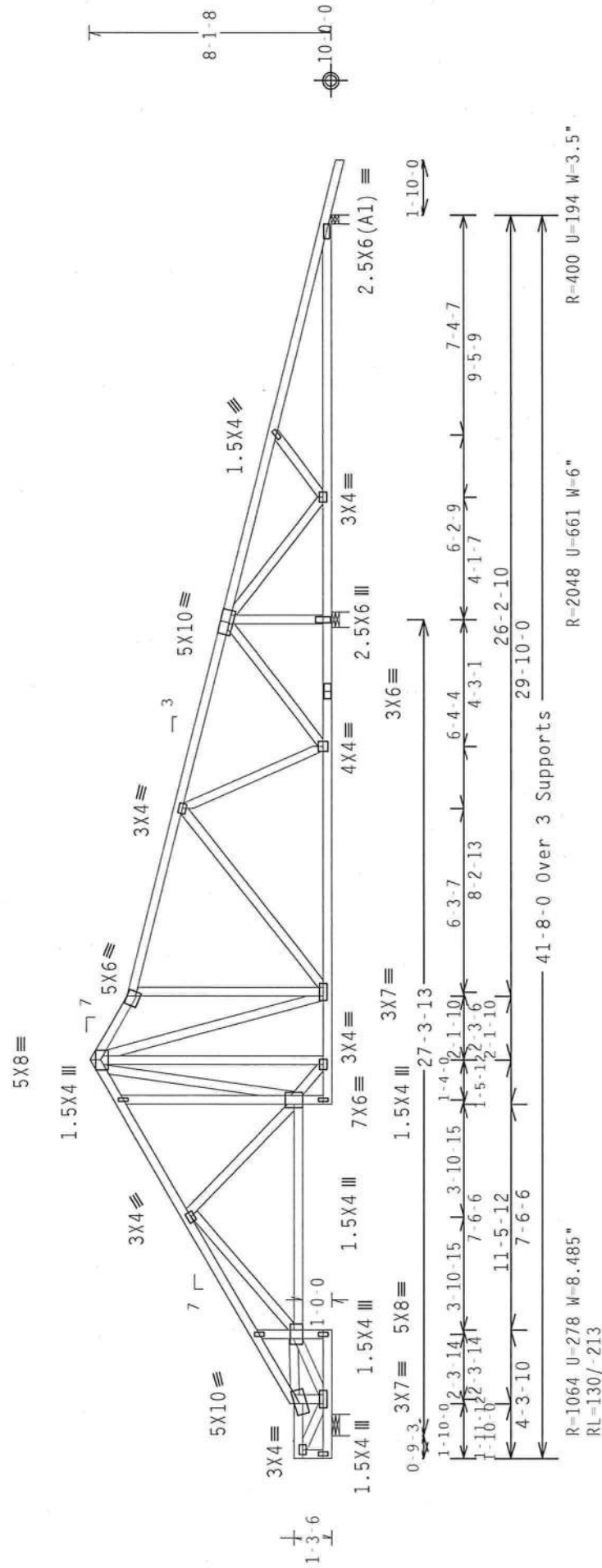
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

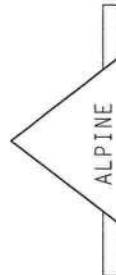
9.02.00

QTY:1 FL/-/5/-/-/R/-/

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY IPSS (BRIDGE PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 530 NORTH DEARBORN STREET, CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TRUSSES SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOLTS. CHILD SHALL HAVE PROPERLY ATTACHED PANELS AND BOLTS.

	TC LL	20.0	PSF	REF	R8228 -	43240
	TC DL	10.0	PSF	DATE	10/06/09	
	BC DL	10.0	PSF	DRW	HCU8R8228	09279048
	BC LL	0.0	PSF	HC-ENG	KD/AP	
	TOT.LD.	40.0	PSF	SEQN-	49003	
	DUR.FAC.	1.25		FROM	AH	
SPACING			24.0"	JREF-	1TV08228Z02	



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55

Left end vertical not exposed to wind pressure.

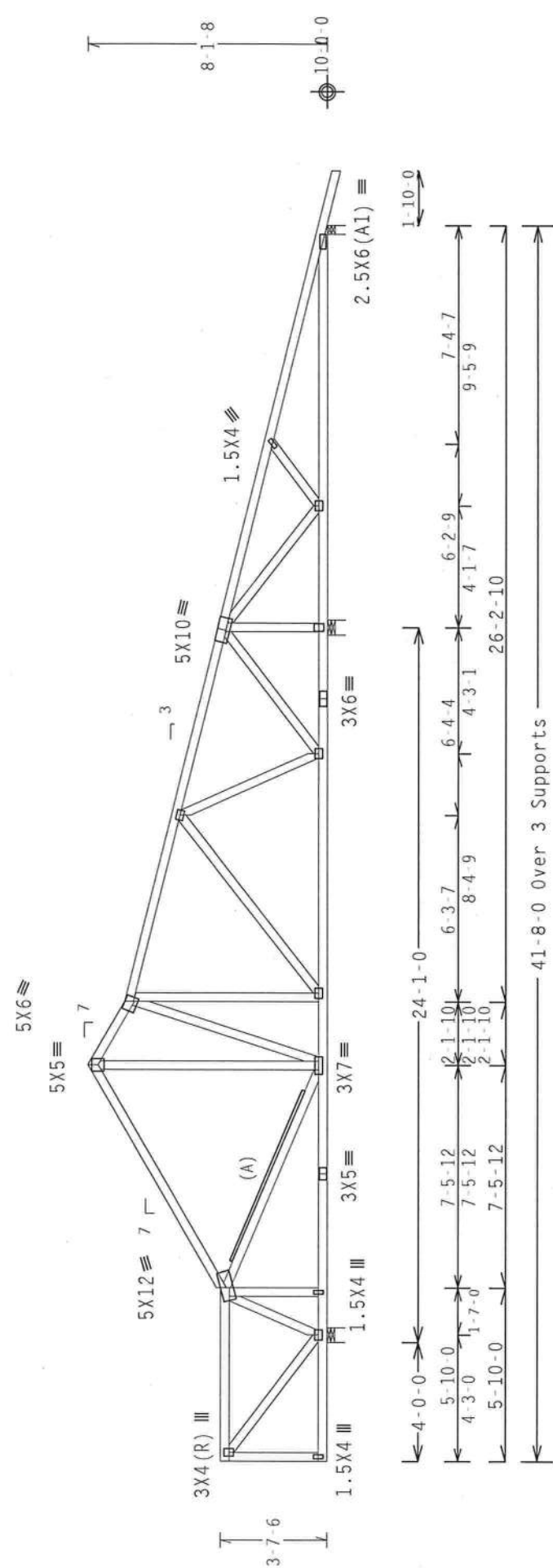
Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat IC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



R=1257 U=420 W=6"
RL=76/-190

R=1731 U=581 W=6"

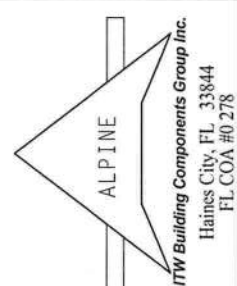
R=514 U=223 W=3.5"

Note: All Plates Are 3x4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave QTY: 1 FL/-5/- / - / R / - Scale = .1875" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. EACH TRUSS MUST BE CHECKED FOR CORRECTNESS OF MATERIALS, DIMENSIONS, AND CONNECTIONS. ALL TRUSSES MUST BE INSPECTED AND APPROVED BY A LICENSED PROFESSIONAL ENGINEER BEFORE INSTALLATION. THE INSTALLATION CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS IN CONFORMANCE WITH THE DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



REF	R8228- 43241
DATE	10/06/09
DRW	HCUSR8228 09279041
HC-ENG	KD/AP
SEQN	49023
FROM	AH
JREF	1TV08228Z02

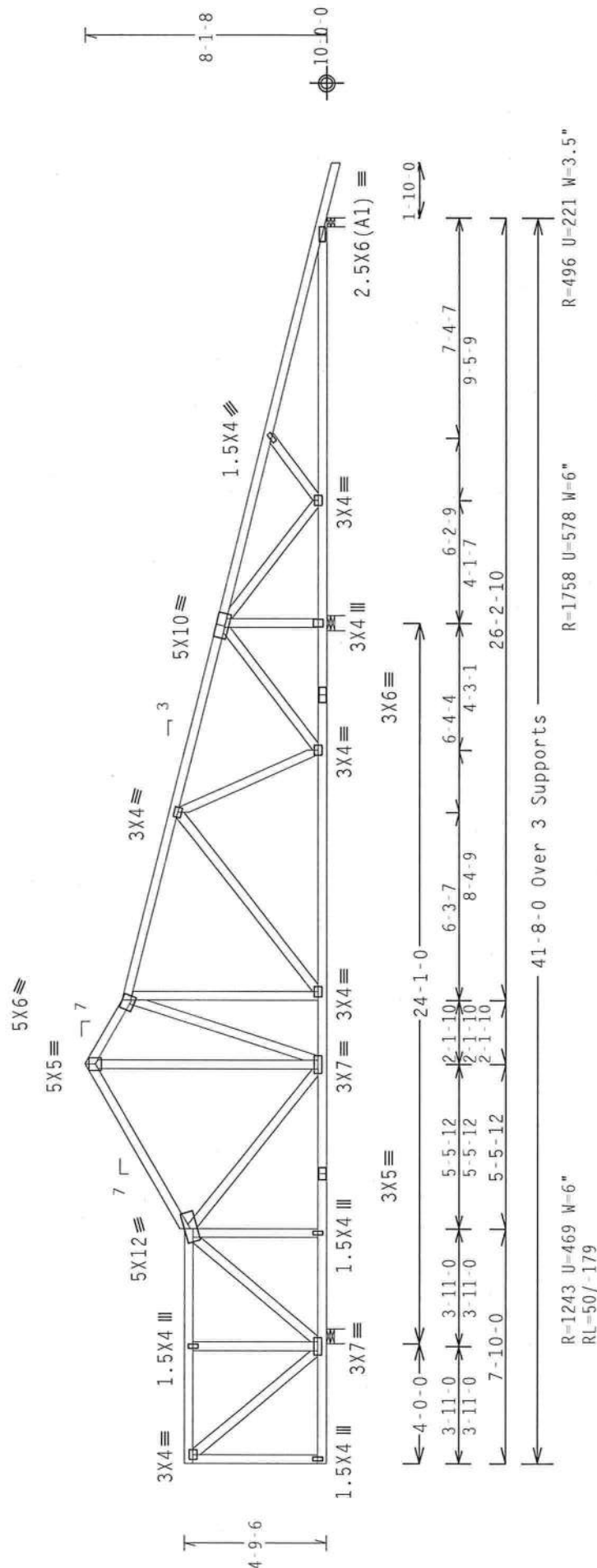
1110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART-ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, wind BC DL=5.0 psf (+/-)=0.55

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 30.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

00

QTY:1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

*****WARNING***** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRACING. REFER TO DECSI BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TRUSS PLATE INSTITUTE, 2100 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 FOR TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF	REF	R8228- 43242
TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCUSR8228 0927904
BC LL	0.0 PSF	HC-ENG	KD/AP
TOT.LD.	40.0 PSF	SEQN-	49033
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	ITV08228702



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FLCOA #0 278

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpi (+/-)=0.55

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

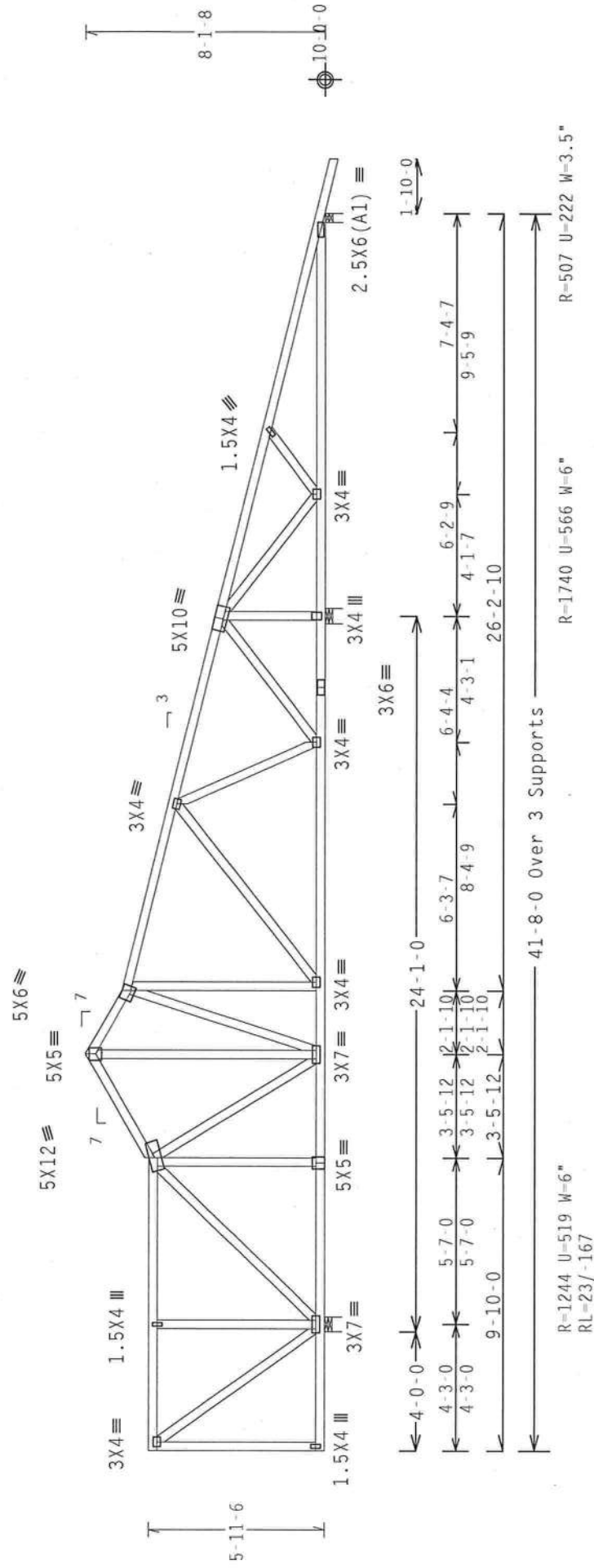
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0) QTY: 1 FL/-5/-/-R/
Scale = .1875" / Ft.



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. BEFORE THE TRUSSES ARE USED FOR ANY PURPOSE, THE PUBLISHED INFORMATION, INCLUDING THE TRUSS MANUFACTURER'S INSTRUCTIONS, MUST BE READ AND UNDERSTOOD. THE TRUSSES ARE NOT TO BE USED FOR ANY PURPOSES OTHER THAN THOSE FOR WHICH THEY WERE DESIGNED. THE TRUSSES ARE NOT TO BE USED FOR ANY PURPOSES OTHER THAN THOSE FOR WHICH THEY WERE DESIGNED. THE TRUSSES ARE NOT TO BE USED FOR ANY PURPOSES OTHER THAN THOSE FOR WHICH THEY WERE DESIGNED.

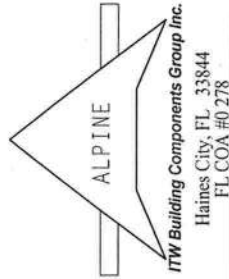
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS IN COMPLIANCE WITH THE FOLLOWING REQUIREMENTS:

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. FOR STEEL AND TP1. THE BCG DESIGNER CONFIRMS THAT ALL FABRICATING, HANDLING, SHIPPING, INSTALLING, OR WELDING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 2010/18GA. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. PLATES TO EACH FACE OF TRUSS AND, IF NECESSARY, ADDITIONAL PLATES TO EACH FACE OF THE TRUSS COMPONENT.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX# 43 OF TP1-2902 SEC. 3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ABX#1/TP1 SEC. 2.



Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

11110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.55

Left end vertical not exposed to wind pressure.

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

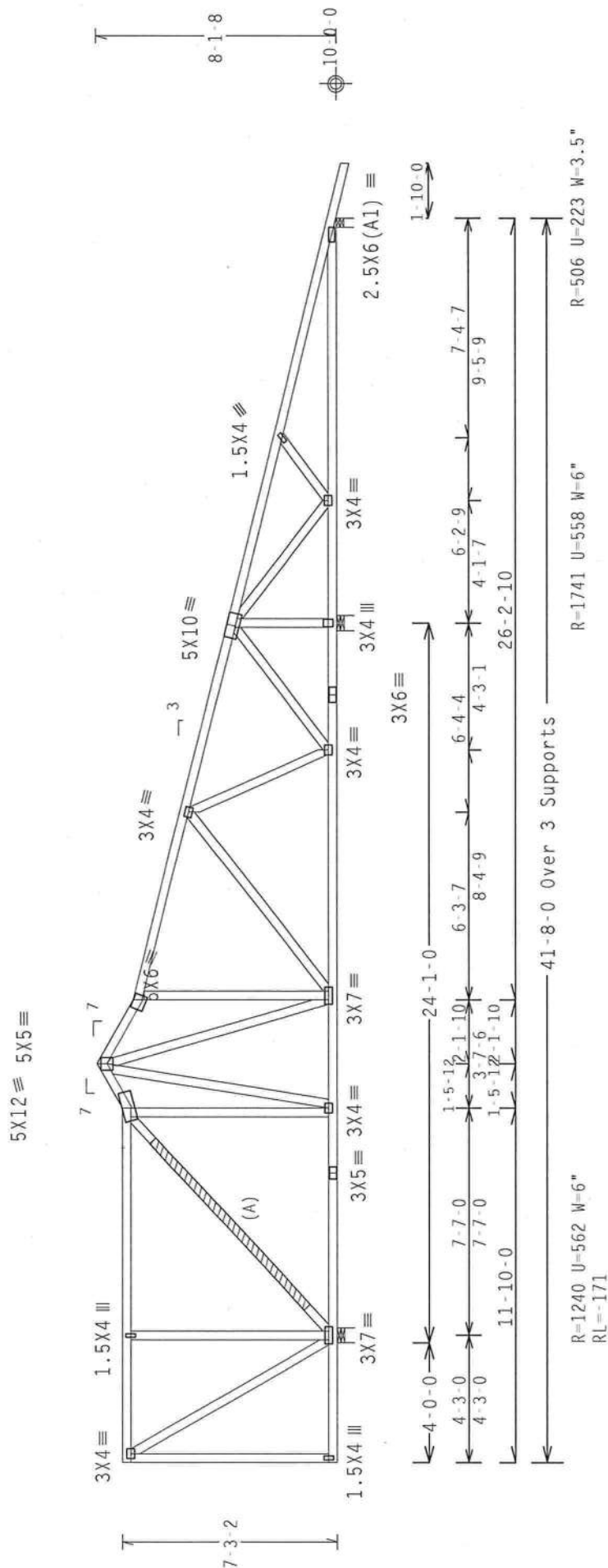
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MMWFRS loads based on trusses located at least 30.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

00

QTY:1 FL/-/5/-/-/R/-

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESIGNS (INCLUDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PLATE INSTITUTE, 210 WEST CHERRY STREET, PITTSBURGH, PA 15222 FOR THE PROPERLY USED SAFETY CONNECTIONS. THESE FUNCTIONS ARE NOT COVERED BY THIS CERTIFICATE LABEL. MODEL SGN-14-A, 537219 FOR THE SAFETY PRINCIPLES REQUIRED TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

FF B8228- 43244

DATE 10/06/09

10/06/09

IRW HCUSR8228 09279009

IC-ENG DF/DF

67878 FON-

[illegible]

ROM AH

REF- ITV08228Z02

(9-136--Doug Edgley Tuwauana Rossin -- , ** - A13)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

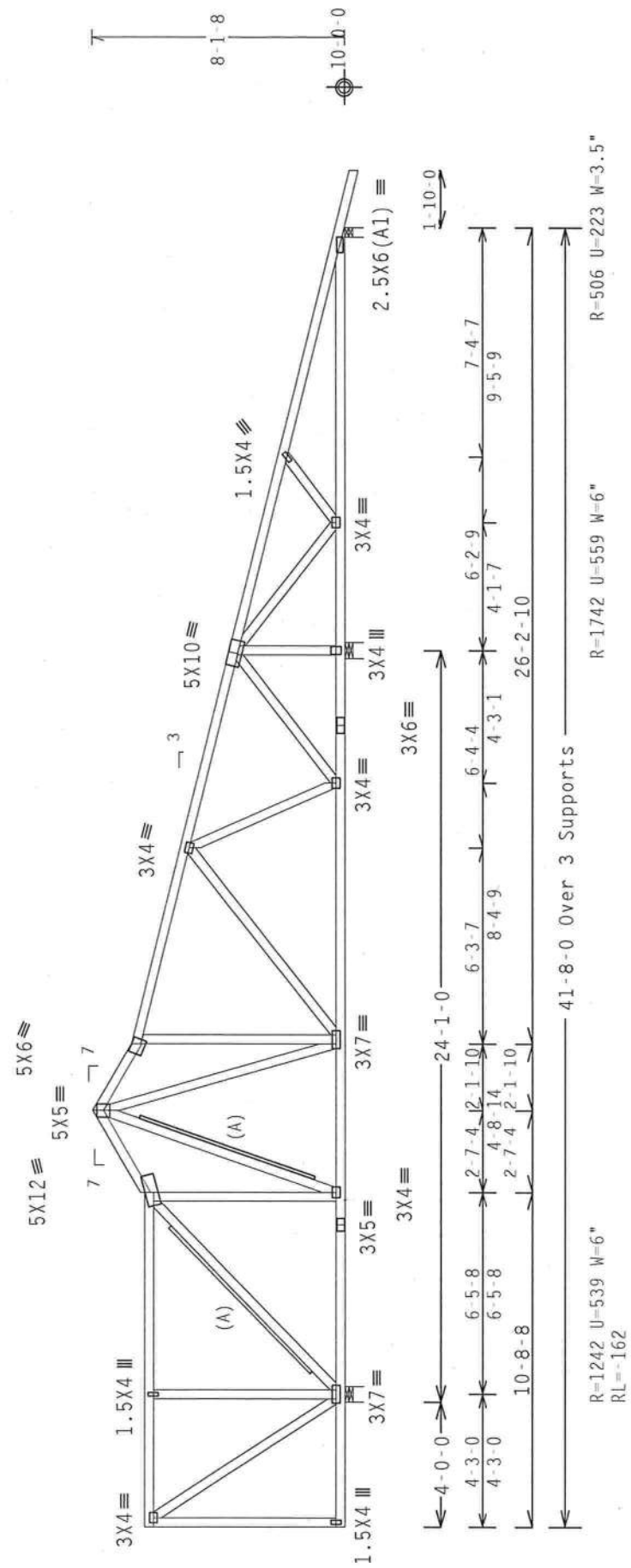
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.



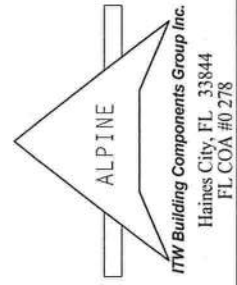
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

QTY:1	FL/-/5/-/-/R/-	Scale = .1875"/Ft.
TC LL	20.0 PSF	REF R8228- 43245
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279034
BC LL	0.0 PSF	HC-ENG KD/AP
TOT.LD.	40.0 PSF	SEQN- 49055
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THE DESIGNER IS NOT RESPONSIBLE FOR THE TRUSS FABRICATOR'S FAILURE TO FOLLOW THE TRUSS MANUFACTURER'S INSTRUCTIONS. THE TRUSS FABRICATOR SHALL BE RESPONSIBLE FOR THE TRUSS FABRICATOR'S FAILURE TO FOLLOW THE TRUSS MANUFACTURER'S INSTRUCTIONS. THE TRUSS FABRICATOR SHALL BE RESPONSIBLE FOR THE TRUSS FABRICATOR'S FAILURE TO FOLLOW THE TRUSS MANUFACTURER'S INSTRUCTIONS. THE TRUSS FABRICATOR SHALL BE RESPONSIBLE FOR THE TRUSS FABRICATOR'S FAILURE TO FOLLOW THE TRUSS MANUFACTURER'S INSTRUCTIONS.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.



Haines City, FL 33844
FL COA #0 278

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W9 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

(B) #3 or better scab brace. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

(A) (2) #3 or better scab braces. Same size & 80% length of web member. Attach one to each face w/10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load.

Negative reaction(s) of -372# MAX. (See below) from a non-wind load case requires uplift connection.

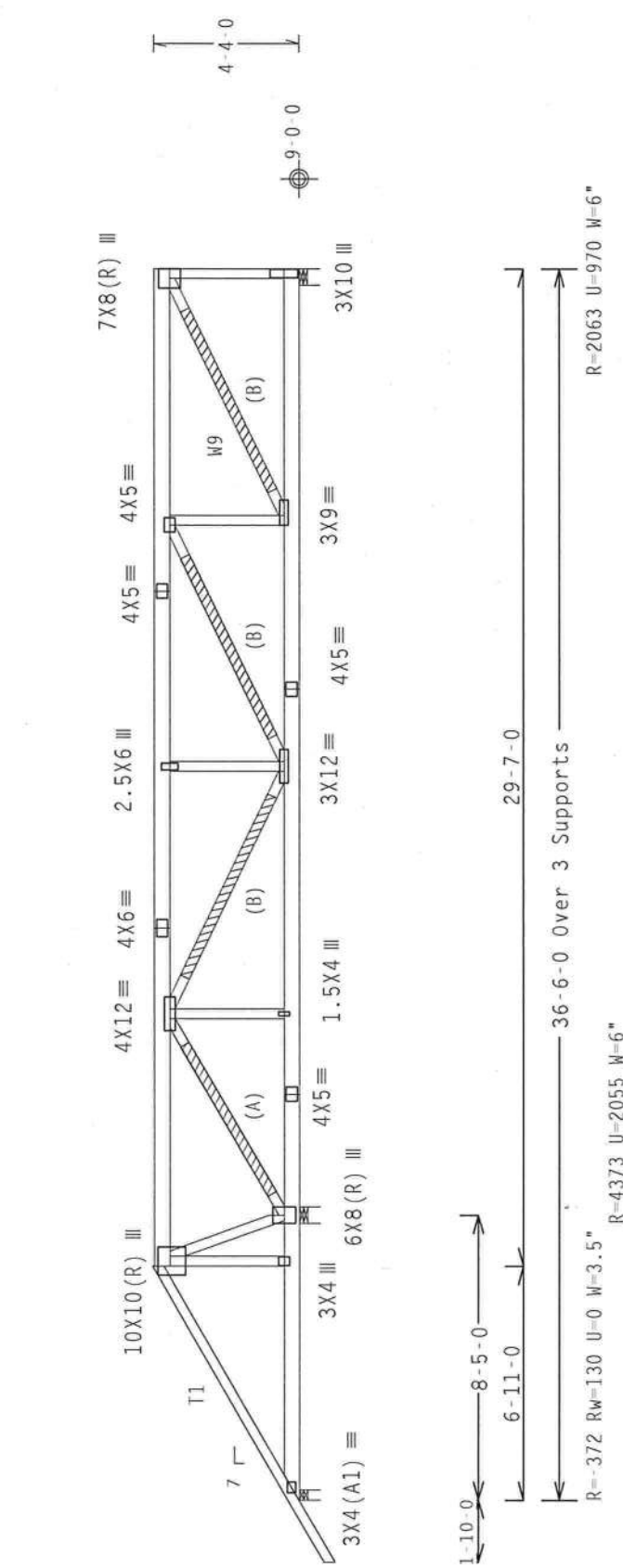
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg. Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 G_{Cpi}(+/-)-0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. Right side jacks have 0-0-0 setback with 0-0-0 cant and 0-0-0 overhang.

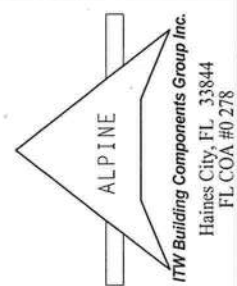


Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0) 9.02.00



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESIDENTIAL BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CROSS PLATE INSTITUTE, 218 STREET, SUITE 102, ALCOCK, MA 01909, FOR TRUSS SAFETY INFORMATION. ALL TRUSSES MUST BE INSTALLED AND BRACED IN ACCORDANCE WITH THE TPI INSTRUCTIONS. OTHERWISE, THE TRUSS MANUFACTURER SHALL NOT BE RESPONSIBLE FOR THE TRUSS BEING USED. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BIDS (NATIONAL DESIGN SPEC, BY AREA) AND TPI. THE REG. INCORPORATOR PLATES ARE MADE OF 70/18/16GA (W/H/55/P) ASTM A653 GRADE 40/60 (W. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



QTY:1	FL/-/5/-/-/R/-	Scale =.1875"/Ft.
TC LL	20.0 PSF	REF R8228- 43246
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279027
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 46650
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02

110 mph wind, 15.00 ft mean hgt., ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, wind BC PL=1.00 GCPI(+/-)-0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

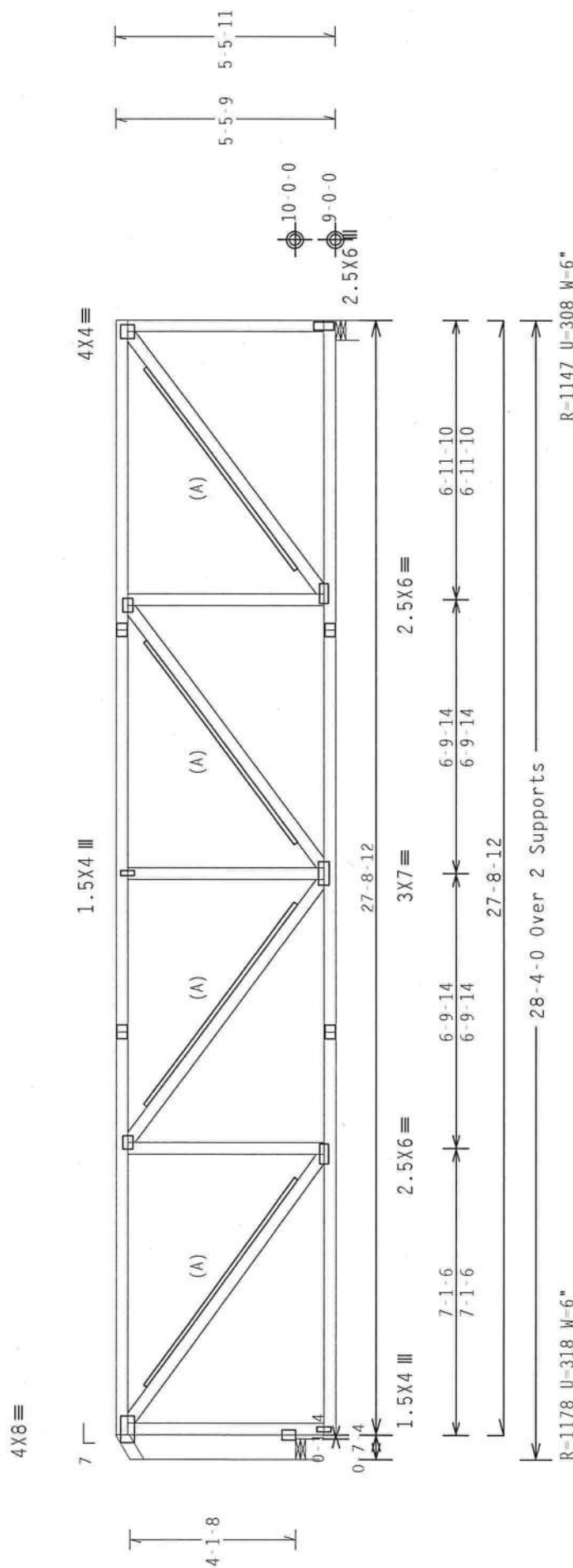
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	
:Lt	Bearing	Leg	2x8	SP	#1	

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" C.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

 $FT/RT=10\%(0\%)/0(0)$

00.00

QTY:1 FL/-/5/-/-/R/-/

Scale = .25"/Ft.

WARNING: THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND REACHING. ALWAYS USE PROPER LIFTING TECHNIQUES. THUSSES ARE NOT TO BE USED FOR LIFTING. THUSSES ARE THE PROPERTY OF NORTH LEE STREET, SUITE 312, ALBANY, NY 12214, AND VICA TRUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MOBILE, AL 36619. 5327191. PROPER PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO THE BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY LTV CURTAIN PANEL INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND AISC (4000 TRUSS COUNCIL OF AMERICA, 63000 ENTERPRISE LANE, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. LTV BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH THE SPECIFICATIONS, FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTORS) DESIGN SPEC. (BY AISC) AND TPI. (TOP CHORD PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10A-2, 10A-3, 10A-4, 10A-5, 10A-6, 10A-7, 10A-8, 10A-9, 10A-10, 10A-11, 10A-12, 10A-13, 10A-14, 10A-15, 10A-16, 10A-17, 10A-18, 10A-19, 10A-20, 10A-21, 10A-22, 10A-23, 10A-24, 10A-25, 10A-26, 10A-27, 10A-28, 10A-29, 10A-30, 10A-31, 10A-32, 10A-33, 10A-34, 10A-35, 10A-36, 10A-37, 10A-38, 10A-39, 10A-40, 10A-41, 10A-42, 10A-43, 10A-44, 10A-45, 10A-46, 10A-47, 10A-48, 10A-49, 10A-50, 10A-51, 10A-52, 10A-53, 10A-54, 10A-55, 10A-56, 10A-57, 10A-58, 10A-59, 10A-60, 10A-61, 10A-62, 10A-63, 10A-64, 10A-65, 10A-66, 10A-67, 10A-68, 10A-69, 10A-70, 10A-71, 10A-72, 10A-73, 10A-74, 10A-75, 10A-76, 10A-77, 10A-78, 10A-79, 10A-80, 10A-81, 10A-82, 10A-83, 10A-84, 10A-85, 10A-86, 10A-87, 10A-88, 10A-89, 10A-90, 10A-91, 10A-92, 10A-93, 10A-94, 10A-95, 10A-96, 10A-97, 10A-98, 10A-99, 10A-100, 10A-101, 10A-102, 10A-103, 10A-104, 10A-105, 10A-106, 10A-107, 10A-108, 10A-109, 10A-110, 10A-111, 10A-112, 10A-113, 10A-114, 10A-115, 10A-116, 10A-117, 10A-118, 10A-119, 10A-120, 10A-121, 10A-122, 10A-123, 10A-124, 10A-125, 10A-126, 10A-127, 10A-128, 10A-129, 10A-130, 10A-131, 10A-132, 10A-133, 10A-134, 10A-135, 10A-136, 10A-137, 10A-138, 10A-139, 10A-140, 10A-141, 10A-142, 10A-143, 10A-144, 10A-145, 10A-146, 10A-147, 10A-148, 10A-149, 10A-150, 10A-151, 10A-152, 10A-153, 10A-154, 10A-155, 10A-156, 10A-157, 10A-158, 10A-159, 10A-160, 10A-161, 10A-162, 10A-163, 10A-164, 10A-165, 10A-166, 10A-167, 10A-168, 10A-169, 10A-170, 10A-171, 10A-172, 10A-173, 10A-174, 10A-175, 10A-176, 10A-177, 10A-178, 10A-179, 10A-180, 10A-181, 10A-182, 10A-183, 10A-184, 10A-185, 10A-186, 10A-187, 10A-188, 10A-189, 10A-190, 10A-191, 10A-192, 10A-193, 10A-194, 10A-195, 10A-196, 10A-197, 10A-198, 10A-199, 10A-200, 10A-201, 10A-202, 10A-203, 10A-204, 10A-205, 10A-206, 10A-207, 10A-208, 10A-209, 10A-210, 10A-211, 10A-212, 10A-213, 10A-214, 10A-215, 10A-216, 10A-217, 10A-218, 10A-219, 10A-220, 10A-221, 10A-222, 10A-223, 10A-224, 10A-225, 10A-226, 10A-227, 10A-228, 10A-229, 10A-230, 10A-231, 10A-232, 10A-233, 10A-234, 10A-235, 10A-236, 10A-237, 10A-238, 10A-239, 10A-240, 10A-241, 10A-242, 10A-243, 10A-244, 10A-245, 10A-246, 10A-247, 10A-248, 10A-249, 10A-250, 10A-251, 10A-252, 10A-253, 10A-254, 10A-255, 10A-256, 10A-257, 10A-258, 10A-259, 10A-260, 10A-261, 10A-262, 10A-263, 10A-264, 10A-265, 10A-266, 10A-267, 10A-268, 10A-269, 10A-270, 10A-271, 10A-272, 10A-273, 10A-274, 10A-275, 10A-276, 10A-277, 10A-278, 10A-279, 10A-280, 10A-281, 10A-282, 10A-283, 10A-284, 10A-285, 10A-286, 10A-287, 10A-288, 10A-289, 10A-290, 10A-291, 10A-292, 10A-293, 10A-294, 10A-295, 10A-296, 10A-297, 10A-298, 10A-299, 10A-300, 10A-301, 10A-302, 10A-303, 10A-304, 10A-305, 10A-306, 10A-307, 10A-308, 10A-309, 10A-310, 10A-311, 10A-312, 10A-313, 10A-314, 10A-315, 10A-316, 10A-317, 10A-318, 10A-319, 10A-320, 10A-321, 10A-322, 10A-323, 10A-324, 10A-325, 10A-326, 10A-327, 10A-328, 10A-329, 10A-330, 10A-331, 10A-332, 10A-333, 10A-334, 10A-335, 10A-336, 10A-337, 10A-338, 10A-339, 10A-340, 10A-341, 10A-342, 10A-343, 10A-344, 10A-345, 10A-346, 10A-347, 10A-348, 10A-349, 10A-350, 10A-351, 10A-352, 10A-353, 10A-354, 10A-355, 10A-356, 10A-357, 10A-358, 10A-359, 10A-360, 10A-361, 10A-362, 10A-363, 10A-364, 10A-365, 10A-366, 10A-367, 10A-368, 10A-369, 10A-370, 10A-371, 10A-372, 10A-373, 10A-374, 10A-375, 10A-376, 10A-377, 10A-378, 10A-379, 10A-380, 10A-381, 10A-382, 10A-383, 10A-384, 10A-385, 10A-386, 10A-387, 10A-388, 10A-389, 10A-390, 10A-391, 10A-392, 10A-393, 10A-394, 10A-395, 10A-396, 10A-397, 10A-398, 10A-399, 10A-400, 10A-401, 10A-402, 10A-403, 10A-404, 10A-405, 10A-406, 10A-407, 10A-408, 10A-409, 10A-410, 10A-411, 10A-412, 10A-413, 10A-414, 10A-415, 10A-416, 10A-417, 10A-418, 10A-419, 10A-420, 10A-421, 10A-422, 10A-423, 10A-424, 10A-425, 10A-426, 10A-427, 10A-428, 10A-429, 10A-430, 10A-431,



TC LL	20.0	PSF	REF R8228- 43247
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUSR8228 09279008
BC LL	0.0	PSF	HC-ENG KD/AP
TOT.LD.	40.0	PSF	SEQN- 48761
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF- 1TV08228Z02

Top chord 2x4 SP #2 Dense

Bot chord 2x4 SP #2 Dense

Webs 2x4 SP #3

: Lt Bearing Leg 2x8 SP #1 Dense:

(A) 1x4 #3SR8 SPF-S or better "T" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

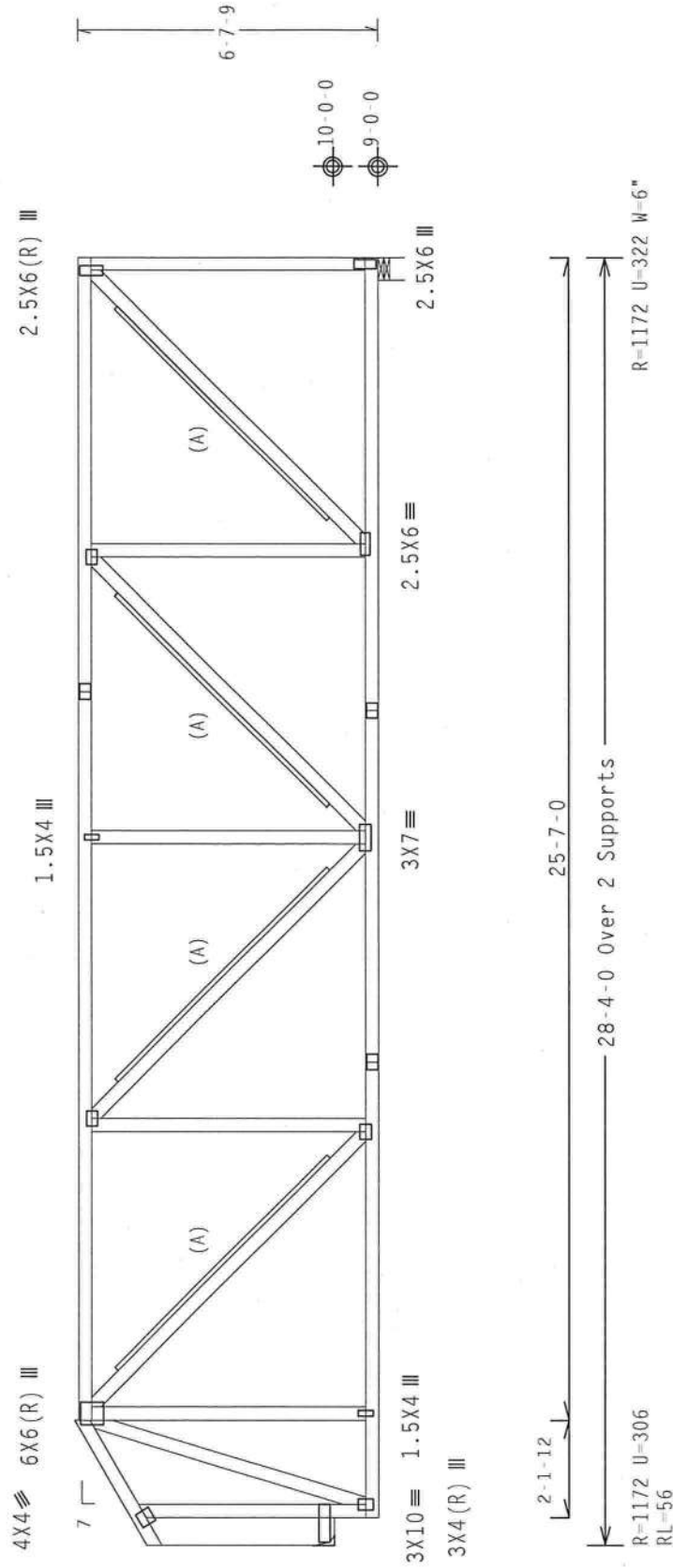
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

FL/-/5/-/-/R/-

Scale = .25"/Ft.

[illegible]

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDED INSTRUCTIONS FOR INSTALLING AND BRACING REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY THE FOLLOWING: TRUSS INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA 4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 48219 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID BEAMING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITEM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH THIS DESIGN OR FABRICATING, HANDLING, SHIPPING, INSTALLING, BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE REQUIREMENTS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) CONNECTION PLATES ARE MADE OF 70/18/16GA (44/MS/AT) GALV A653 GRADE 40/60 (44/MS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-272 THROUGH 100A-275. ALL FABRICATING AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION) CONNECTION PLATES TO BE USED FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TP1 1 SEC. 2.



TC 11	20.0 PSE
-------	----------

[illegible]

70 21 10.0 PSF

BC DL 10.0 PSF

BC LL 0.0 PSF

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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101:LD: 40.0 PSF

DUR.FAC. 1.25

SPACING 24.0"

ALPINE

ITW Building Components Group Inc.

City, FL 33844

FL COA #0 278

1110 mph wind, 15.51 ft mean hgt. ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In Tie of structural panels use purlins to brace all flat TC @ 24" OC.

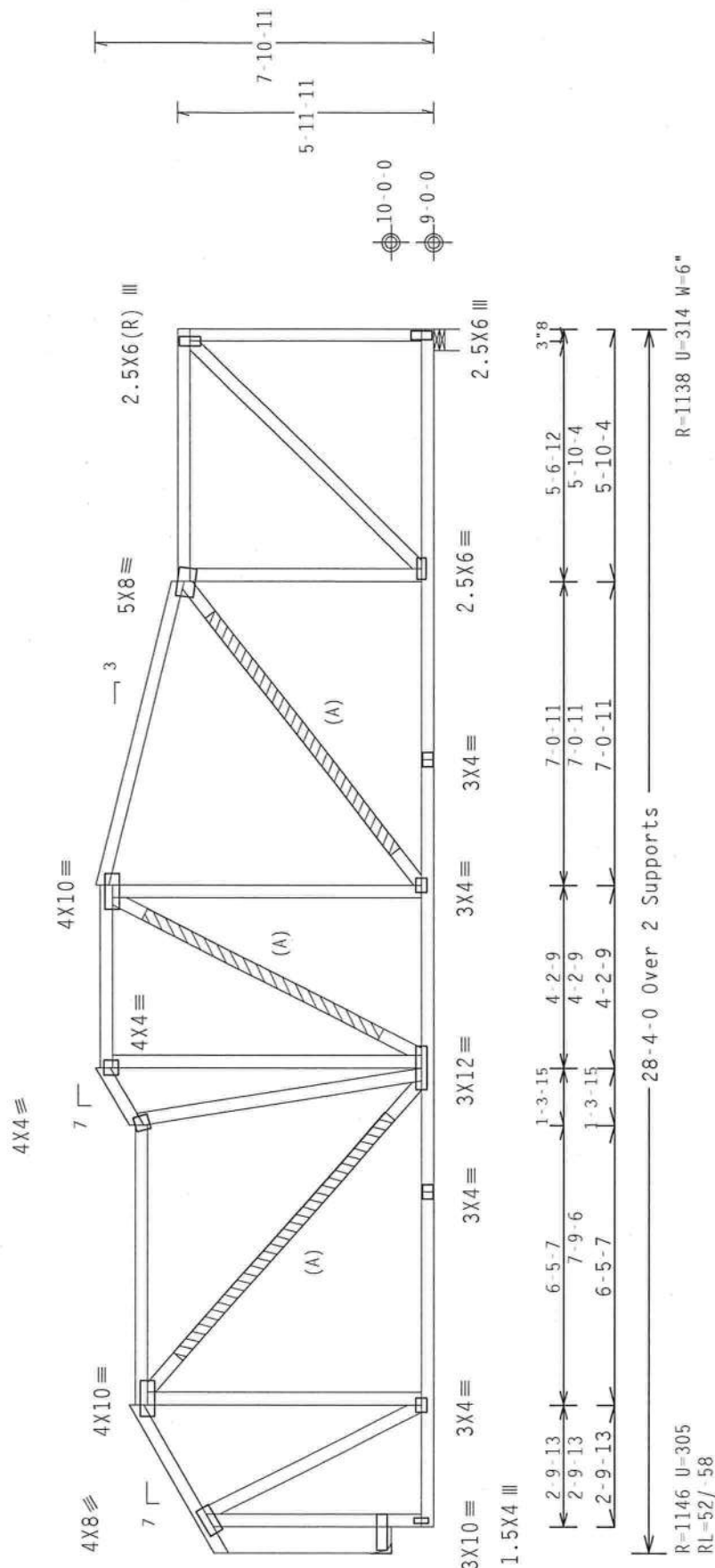
MMFRS loads based on trusses located at least 7.75 ft. from roof edge.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

00

QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

[illegible]

TC LL	20.0 PSF
-------	----------

FF R8228- 43249

TC DI 10 0 PSE

DATE 10/06/09

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
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CO/NO/OT 715

BC DL 10.0 PSF

KW HCUSR8228 0927901

BC LL 0.0 PSF

IC-ENG DF/DF

TOT.LD. 40.0 PSF

EQN- 67862

DUR. FAC. 1.25

ROM AH

SPACING 24.0"

IRFF- 1TV08228702



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Bearing Leg 2x8 SP #1 Dense:

(A) 1x4 #3SR8 SPF-5 or better "J" brace, 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.96 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI(+/-)=0.18

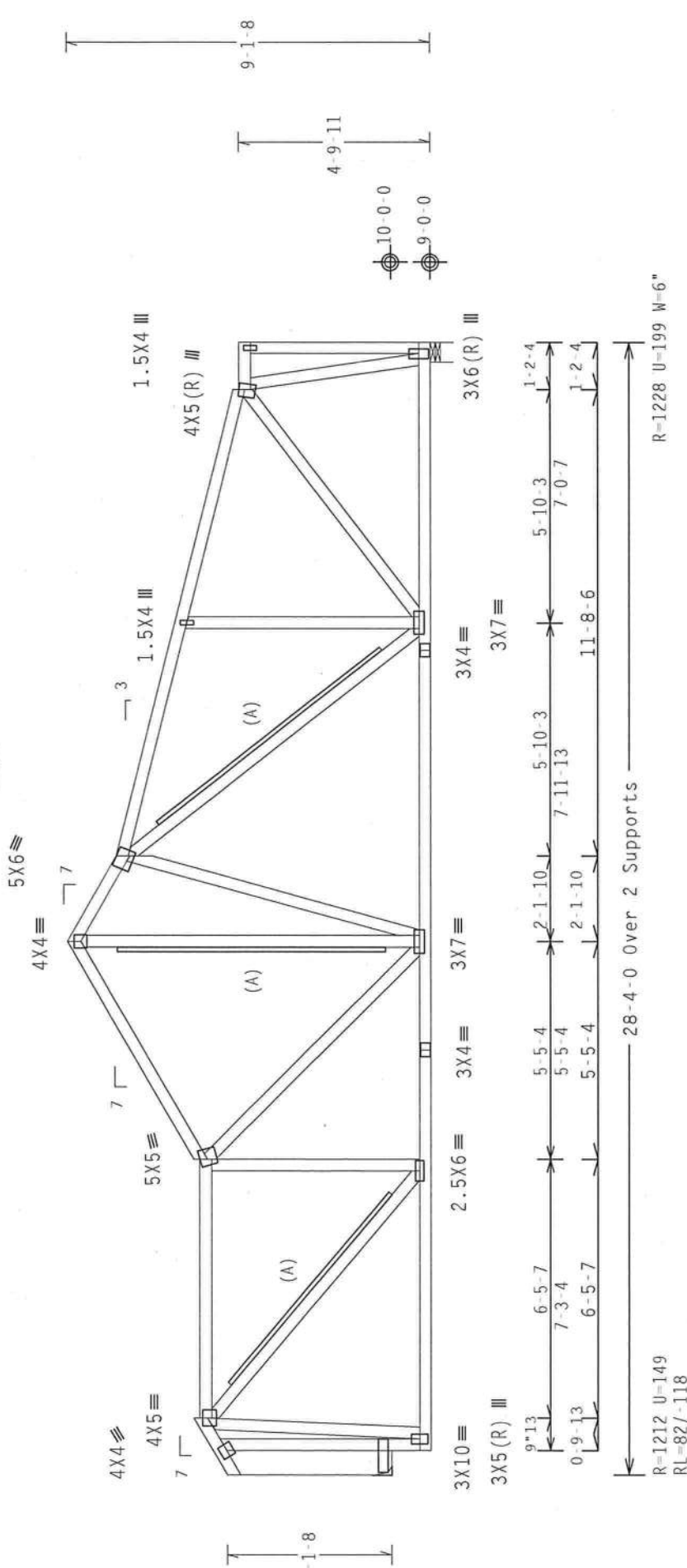
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.96 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY:1	FL/-/5/-/R/-	Scale = .25"/Ft.
	TC LL	20.0 PSF	REF R8228- 43250
	TC DL	10.0 PSF	DATE 10/06/09
	BC DL	10.0 PSF	DRW HCUR8228 09279002
	BC LL	0.0 PSF	HC-ENG KD/AP
	TOT.LD.	40.0 PSF	SEQN- 48917
	DUR.FAC.	1.25	FROM AH
	SPACING	24.0"	JREF- 1TV08228Z02

1110 mph wind, 15.88 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

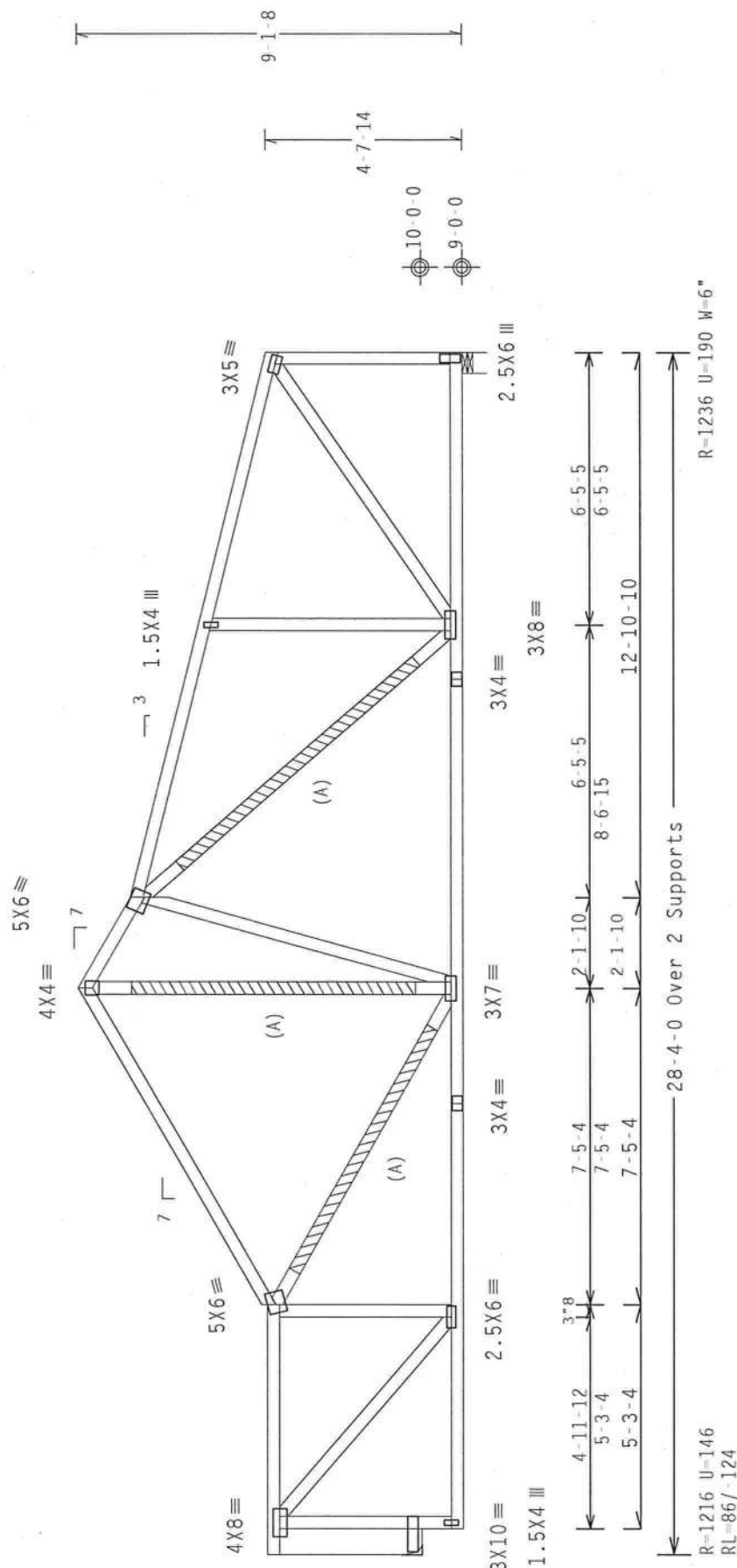
Deflection meets L/240 live and L/180 total load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.88 ft. from roof edge.



R=1216 U=146
RL=86/-124

28-4-0 Over 2 Supports

R=1236 U=190 W=6"

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

9.02.00.0

QTY:1 FL/-/5/-/-/R/-

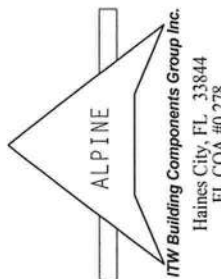
Scale = .25"/Ft.

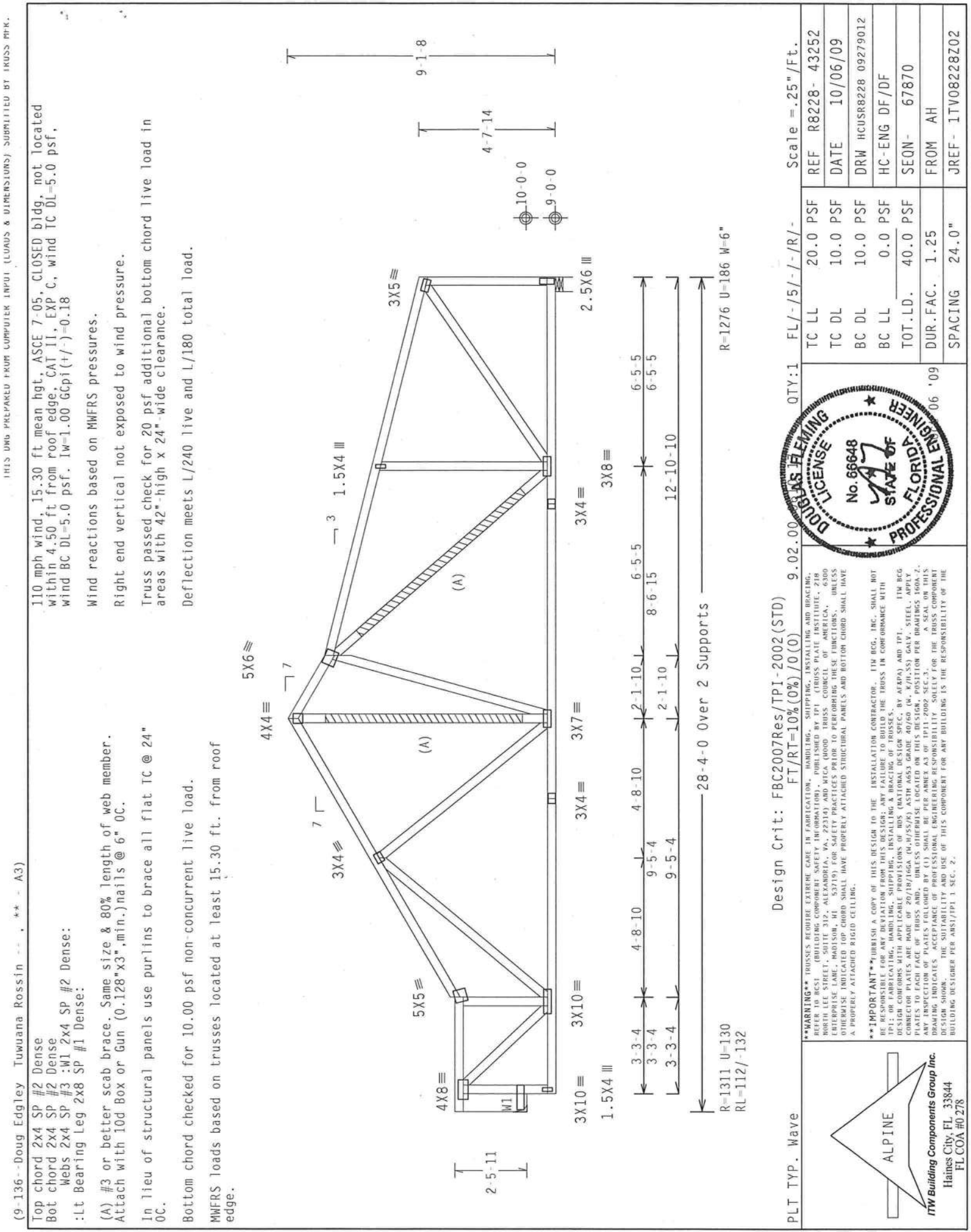
TC LL	20.0 PSF	REF R8228- 43251
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279011
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 67866
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02



WARNING TRUSSES REQUIRING EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND PRACICING RELATE TO THE FOLLOWING COMPANIES' SAFETY INFORMATION: PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304; AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, WAUWATOSA, WI 53219. ALL FABRICATORS MUST BE TRAINED TO PERFORM THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN CONDITIONS WITHIN THE PROVISIONS OF THIS NATIONAL DESIGN SPEC. (IN AEP/A) AND TP1. ITW BCG DESIGN CONDITIONS WITHIN THE PROVISIONS OF THIS NATIONAL DESIGN SPEC. (IN AEP/A) AND TP1. ITW BCG CONNECTOR PLATES ARE MADE OF 2019/T18/AL OR 4725/PC LOCATED 40/60 IN. K/35/5 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED IN THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBA A.3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNING DESIGNER PER A541/TP1 1 SEC. 2.





1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCpi(+/-)=0.18

Wind reactions based on MwFRS pressures.

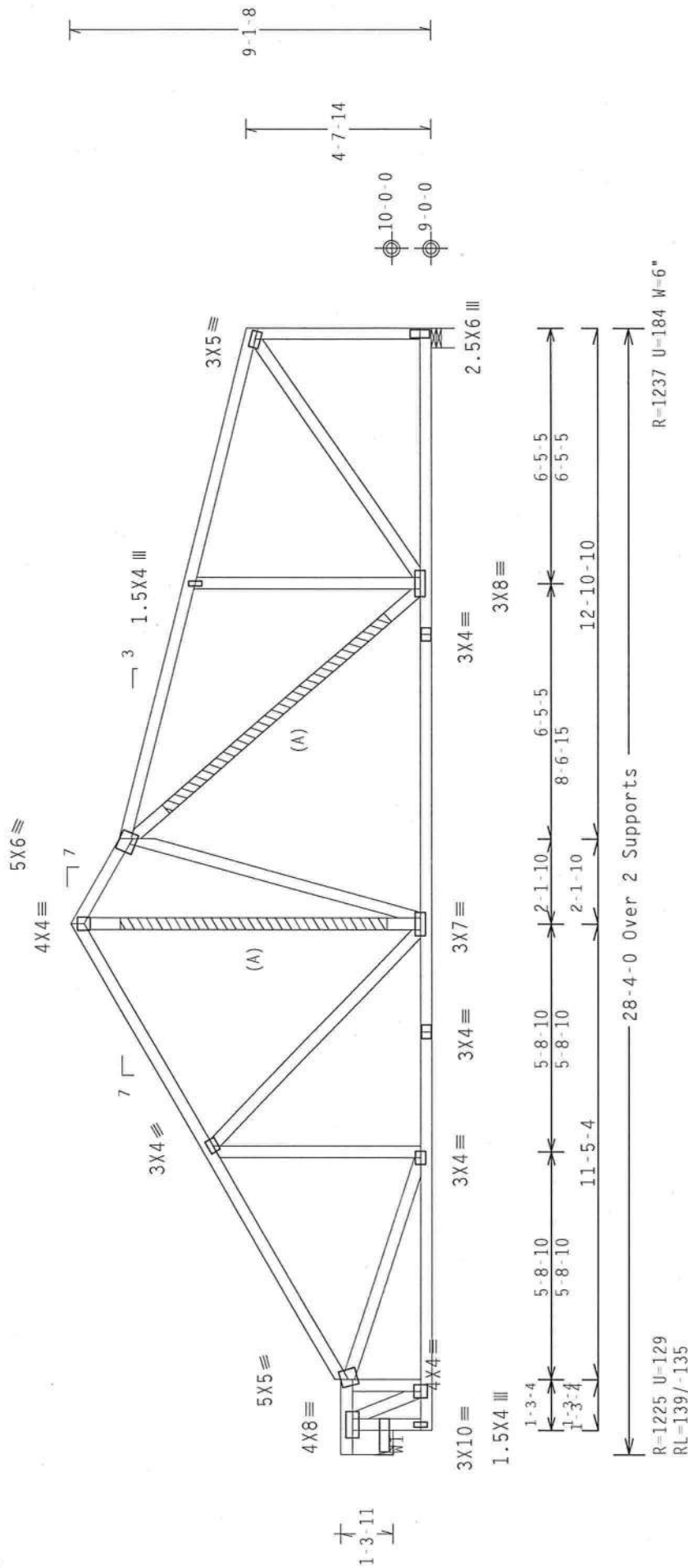
(A) #3 of better scap brace. Same size & 60% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

9.02.00
(0)

QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

WARNING THUSSES REQUIRE EXTREME CARE IN ERECTION, HANDLING, SHIPPING, UNLOADING AND BRACING. ALWAYS FOLLOW THE MANUFACTURER'S RECOMMENDED SAFETY INFORMATION. PUBLISHED BY THE THUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALICHA, ARIZONA, VA 22314, AND WCA THUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORD.

WARNING—THOUSSES FLOORING EXTREME CARE IN FABRICATION, SHIPPING, INSTALLING AND BRACING. BE SURE TO BESEI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PAPER INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCCA (WOOD TRUSS COUNCIL OF AMERICA, 10000 WOODHURST DRIVE, SUITE 100, FORT WORTH, TEXAS 76116) FOR THE PROPER BRACING OF TRUSSES. TRUSSES SHOULD BE PROPERLY IDENTIFIED AND PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IFW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AND FAILURE TO BUILD THE TRUSS IN ACCORDANCE WITH

****WARNING**** TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENTS SAFETY INFORMATION), PUBLISHED BY ITW (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 WINTERPARK LANE, WADSWORTH, MI 53219) FOR TRUSS SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DETAILING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES. THE DESIGNER CANNOT ASSUME RESPONSIBILITY FOR MODIFICATIONS OF THIS NATIONAL DESIGN SPEC. BY AEPFO AND TPI. ITW BCG DESIGN CHANGES WITH APPLICABLE PROVISIONS OF THIS NATIONAL DESIGN SPEC. SHALL BE INDICATED BY STEEL DESIGNER'S PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-2 THROUGH 160A-5. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI-2009 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE SUBMITTAL AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER. PER ANNEX A/TPI 1 SEC. 2.



TC LL	20.0	PSF	REF R8228- 43253
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUSR8228 09279013
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 67874
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF- 1TV08228Z02

(9-136--Doug Edgley Tuwuana Rossin --, ** - H7B)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B3 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W8 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Negative reaction(s) of .385# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg. Located anywhere in roof, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang.

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

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DATE 10/06/09

DRW HCUSR8228 09279044

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SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

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SEQN- 46965

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PLT TYP. Wave

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FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

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FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

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FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

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FL COA #0278

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FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

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SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

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Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

Scale = .1875"/Ft.

QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044

HC-ENG JB/DF

SEQN- 46965

FROM AH

JREF- 1TV08228202

PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0278

Design Crit: FBC2007Res/TPI-2002 (STD)

FT/RT=10% (0%) /0(0)

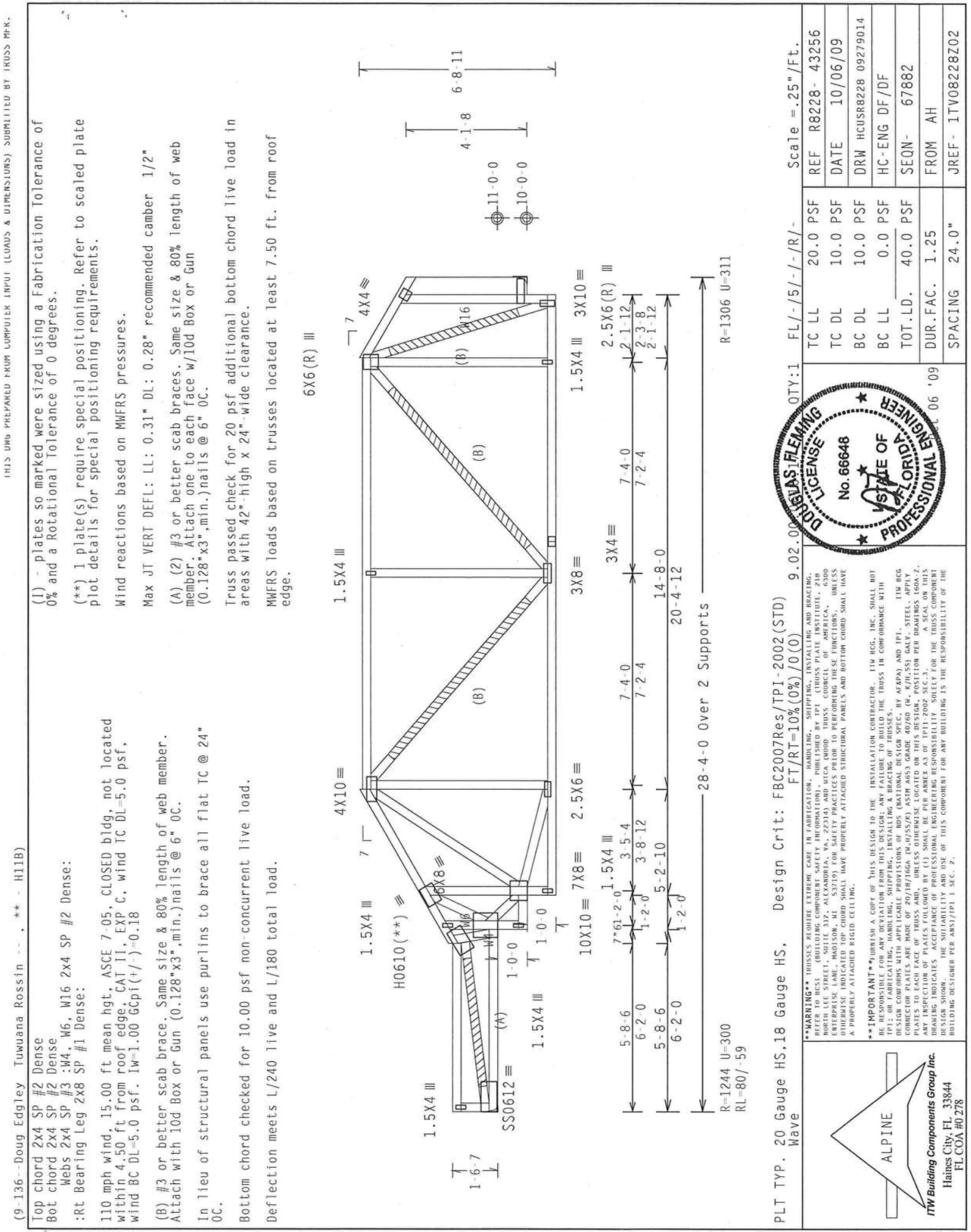
Scale = .1875"/Ft.

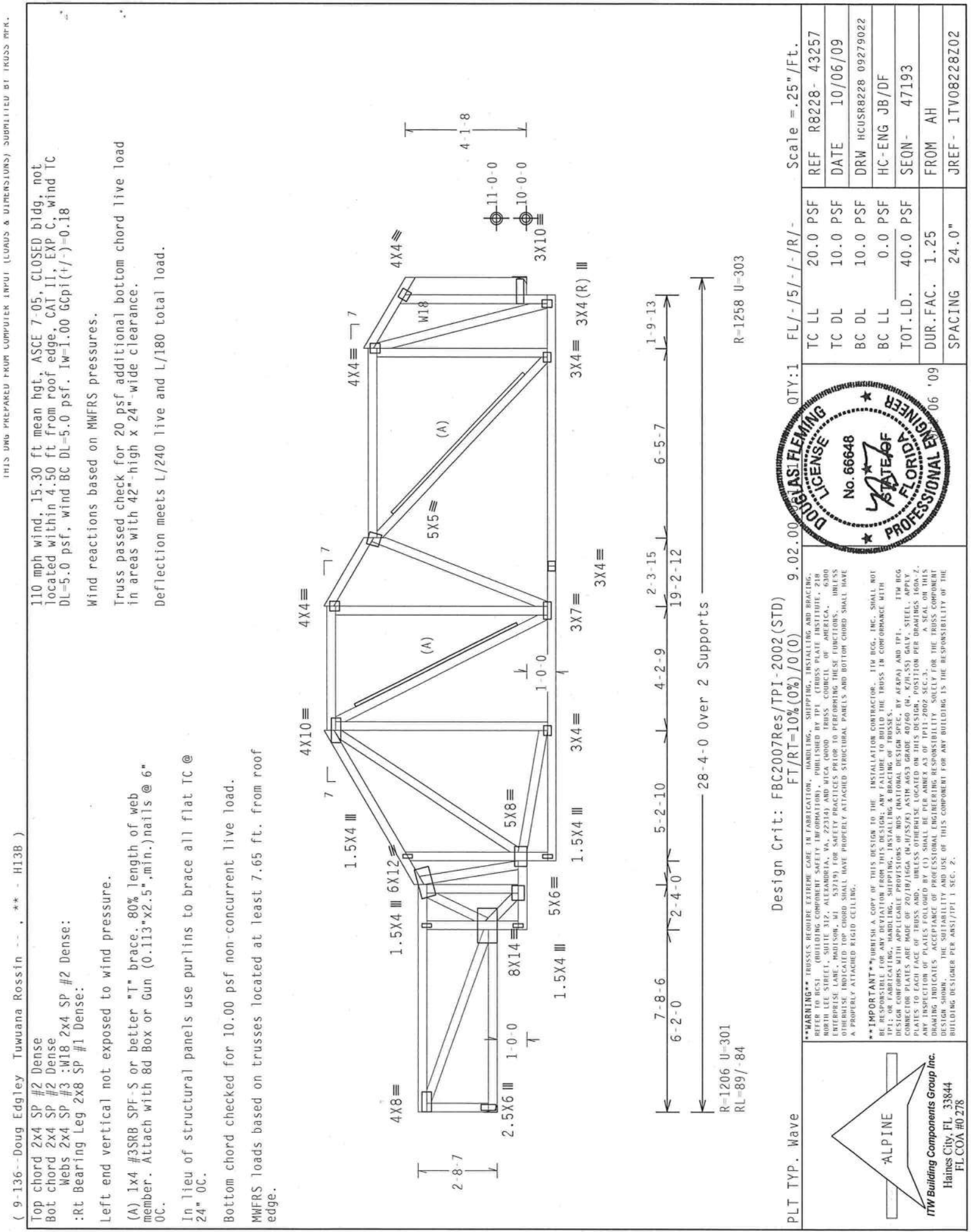
QTY:1

REF R8228- 43254

DATE 10/06/09

DRW HCUSR8228 09279044





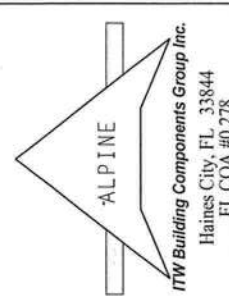
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)



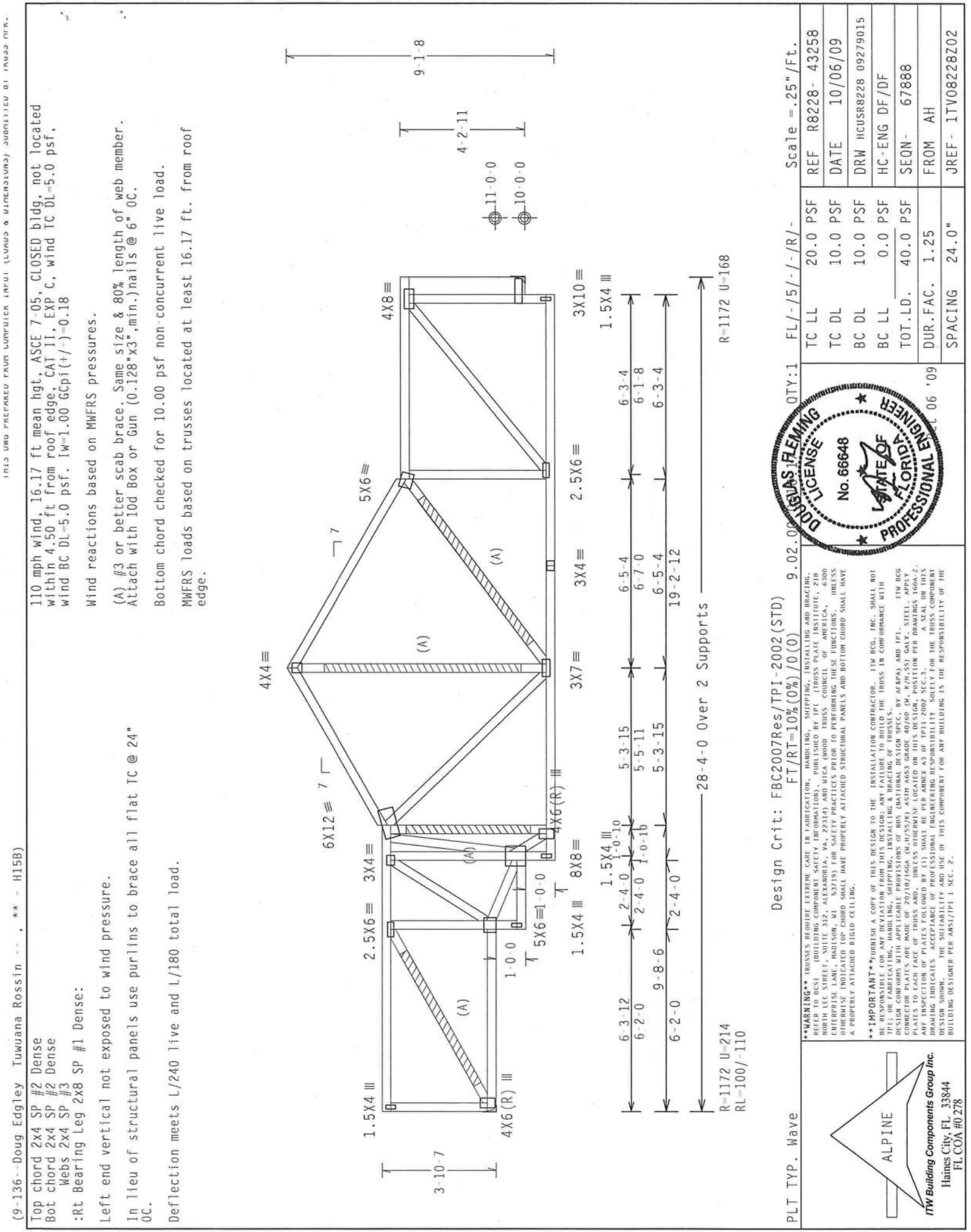
QTY:1	FL/-/5/-/-/R/-	Scale =.25"/Ft.
TC LL	20.0 PSF	REF R8228 - 43257
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279022
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 47193
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI CROSS PLATE INSTITUTE, 2180 W. 10TH AVE., SUITE 100, ALPHARETTA, GA 30004-4000, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI'S OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AIA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2019/16GA (40/60 (4, 6, 8, 10, 12, 14, 16, 18, 20, 22, 24, 26, 28, 30, 32, 34, 36, 38, 40, 42, 44, 46, 48, 50, 52, 54, 56, 58, 60, 62, 64, 66, 68, 70, 72, 74, 76, 78, 80, 82, 84, 86, 88, 90, 92, 94, 96, 98, 100) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave



THIS WAS PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUPPLIED BY THE USER.

(9-136--Doug Edgley Tuwana Rossin --, ** - H15B)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Bearing Leg 2x8 SP #1 Dense:
Left end vertical not exposed to wind pressure.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 16.17 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18
Wind reactions based on MWFRS pressures.
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 16.17 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)
Scale = .25" / Ft.

PLT TYP. Wave
R=1172 U=214
RL=100/-110
R=1172 U=168

28-4-0 Over 2 Supports

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .25" / Ft.

PLT TYP. Wave
R=1172 U=214
RL=100/-110
R=1172 U=168

28-4-0 Over 2 Supports

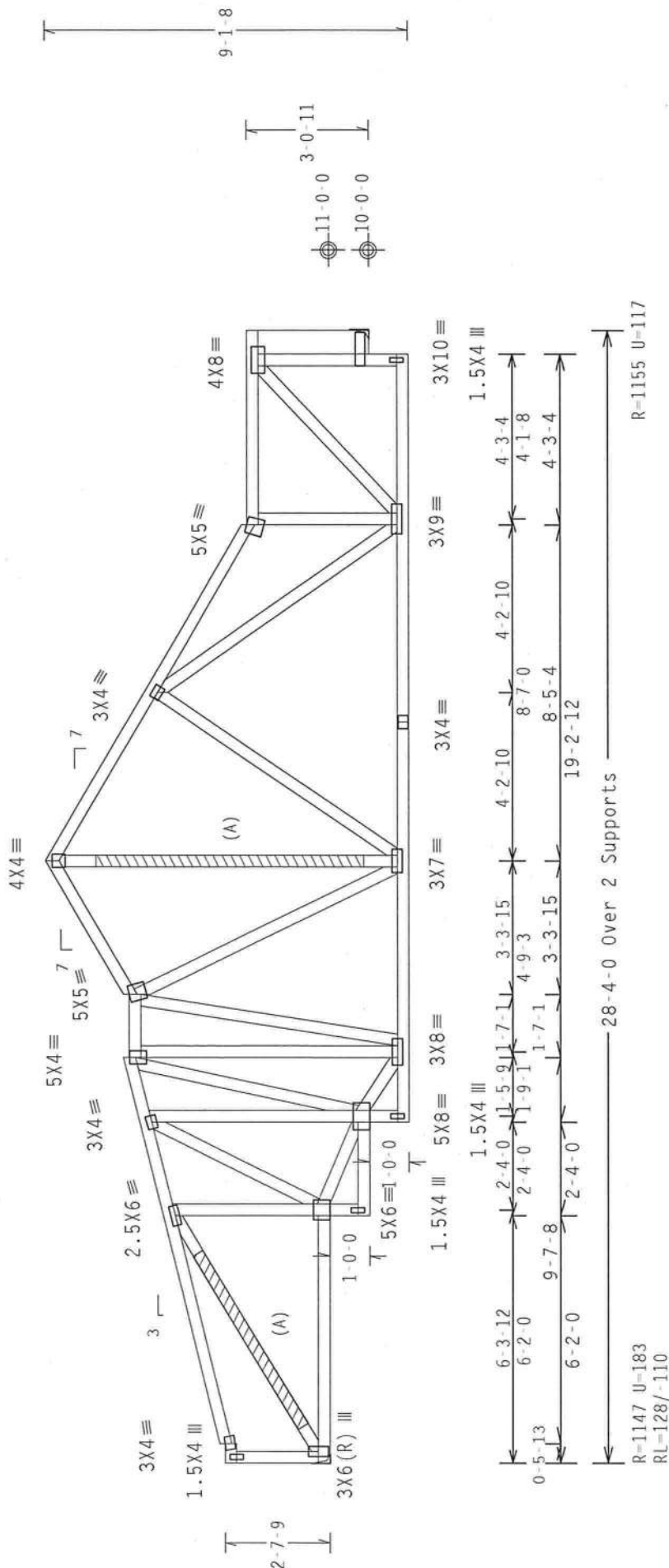
11110 mph wind, 15.59 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

THESE RESULTS WERE REPRODUCED BY

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

00

QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

[illegible]

WARNING-**TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING TO BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI WILL BE RESPONSIBLE FOR ANY DAMAGE TO PROPERTY OR PERSONS. IF YOU ARE NOT A MEMBER OF THE IRONWORKERS' UNION, PLEASE REFER TO RCSC (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY ITP (TRUSS PLATE INSTITUTE), 210 WILSON LEE STREET, SUITE 312, ALXANDRIA, VA, 22314) AND AFCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 NORTHERN LIFEWAY LANE, WALTONSDALE, MI 49789) FOR ADDITIONAL INFORMATION ON SAFE TRUSS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CORREL.

*IMPORTANT**-**UNLESS A COPY OF THIS DESIGN IS TO THE INSTALLATION CONTRACTOR, ITC BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH ITPI(1) OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

**CONNECTIONS COMBINS WITH APPLICABLE PORTALS (JOINTS) AS PER NATIONAL INSIST SPEC. BY ATCPA) AND IPTI. WITH BCGG CONNECTION PLATES ARE MADE OF 2018/1866 (WJDSST) ASTM A653 GRADE 50 GALVALUMETAL GALVA STEEL. APPLICATION OF CONNECTION PLATES FOLLOWED BY IT SHALL BE PER ADHES AT PT-1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THE DESIGN SHOWING. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER/DRAWN PER ANTI/TPT 1 SEC. 2.



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

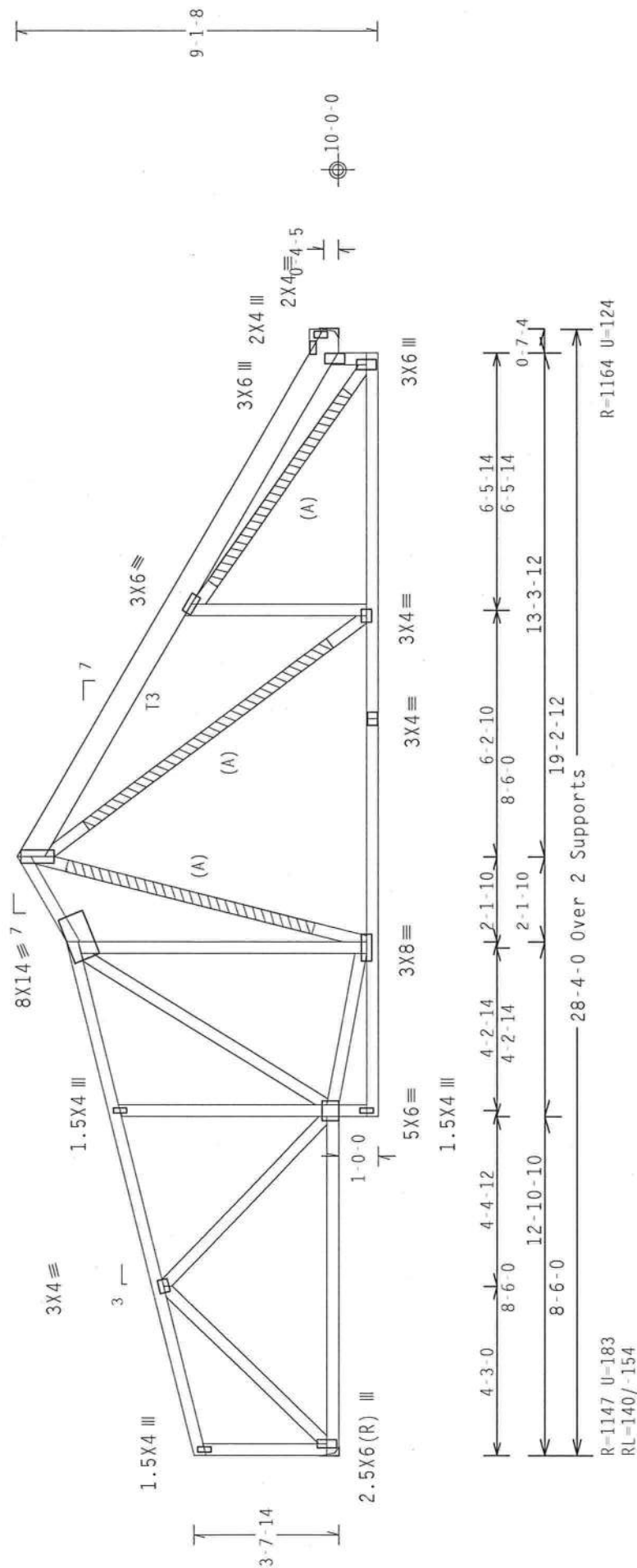
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

4X10(R) III



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. 20 Gauge HS.Wave

Scale = .25"/Ft.

WARNING: THUSSE'S FLOOR, EXTREME CARE IN ERECTION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO PAGES 10-11 FOR MORE INFORMATION. PUBLISHED BY THE STEEL PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304, AND AISC, 900 THIRD STREET, SUITE 100, PITTSBURGH, PA 15222. THUSSE'S FLOOR IS A REGISTERED TRADEMARK OF THUSSE'S FLOOR, INC., 6300 INTERSTATE LAKE, SUITE 500, MI 48219. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHANGING.

TC LL	20.0 PSF
-------	----------

REF R8228- 43261

DATE 10/06/09

7146 50/100/01

0RW HCUSR8228 092790Z

IC-ENG DF/DF

67908 EQN-

FROM AH

URFF- 1TV08228702

1
0
1
0
1
1
0
0
1
1
4
1
1
1

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FLCOA #0218

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

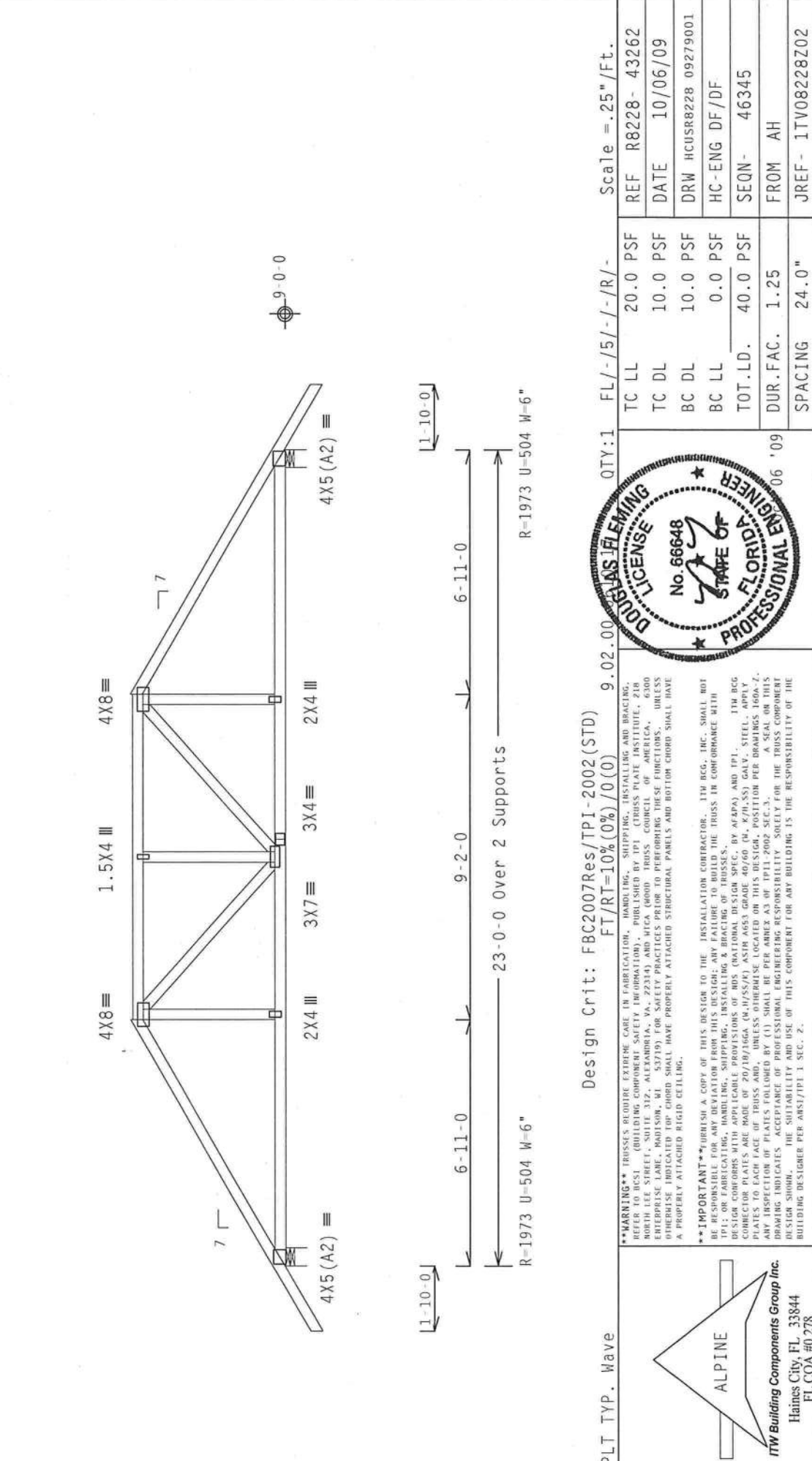
Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

Left side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. End jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang. Right side jacks have 7-0-0 setback with 0-0-0 cant and 1-10-0 overhang.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0)/0(0)

Scale = .25" / Ft.

QTY:1	FL / - / 5 / - / - / R / -	TC LL	20.0 PSF	REF	R8228 - 43262
		TC DL	10.0 PSF	DATE	10/06/09
		BC DL	10.0 PSF	DRW	HCUSR8228 09279001
		BC LL	0.0 PSF	HC-ENG	DF/DF
		TOT.LD.	40.0 PSF	SEQN	46345
		DUR.FAC.	1.25	FROM	AH
		SPACING	24.0"	JREF	1TV08228202

DOUBLE FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

06 '09

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA - 22304), AND NTC (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY AFAPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/756GA (4-H/55/51) ASTM A653 GRADE 40/60 (0.4, 8/10/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE RESPONSIBILITY OF THE TRUSS ORDERER. (2) SHALL BE THE RESPONSIBILITY OF THE TRUSS ORDERER. (3) SHALL BE THE RESPONSIBILITY OF THE TRUSS ORDERER. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.0$ GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

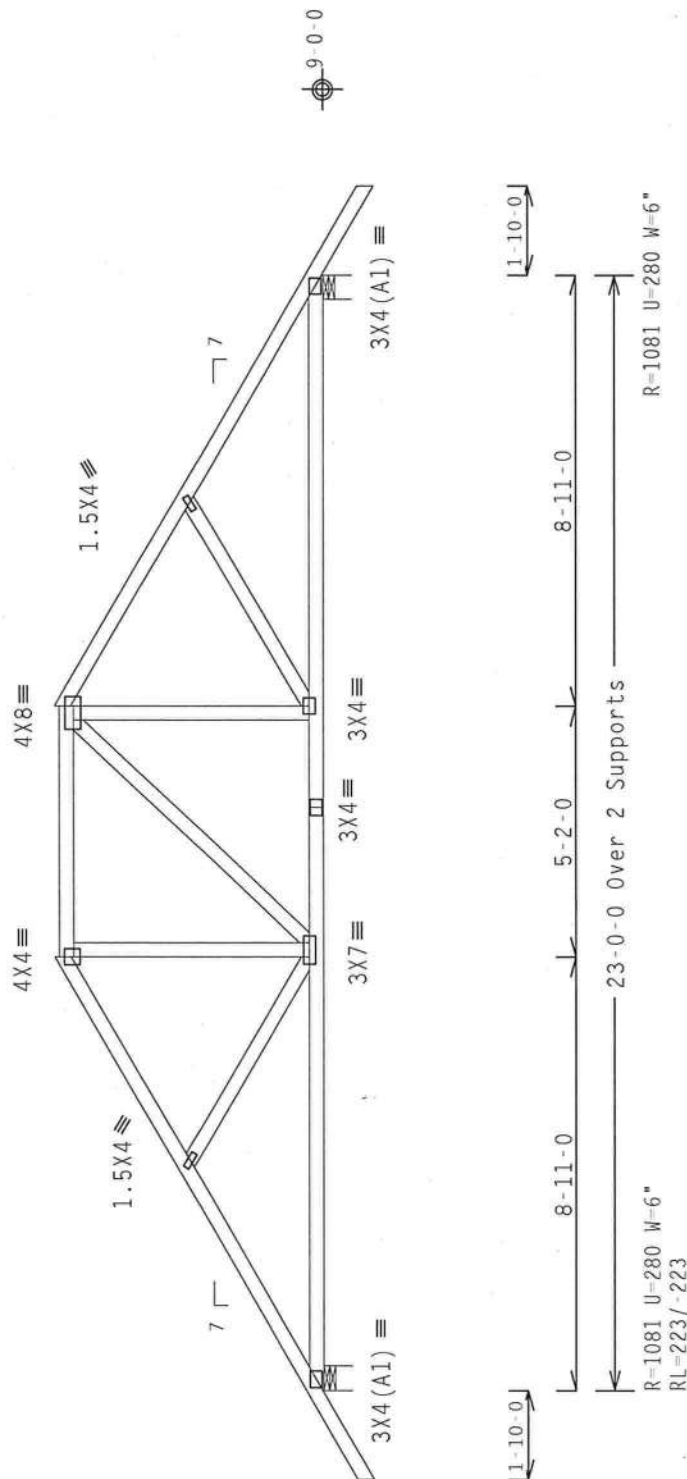
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

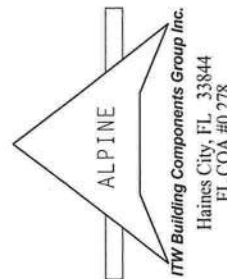
PLT TYP. Wave

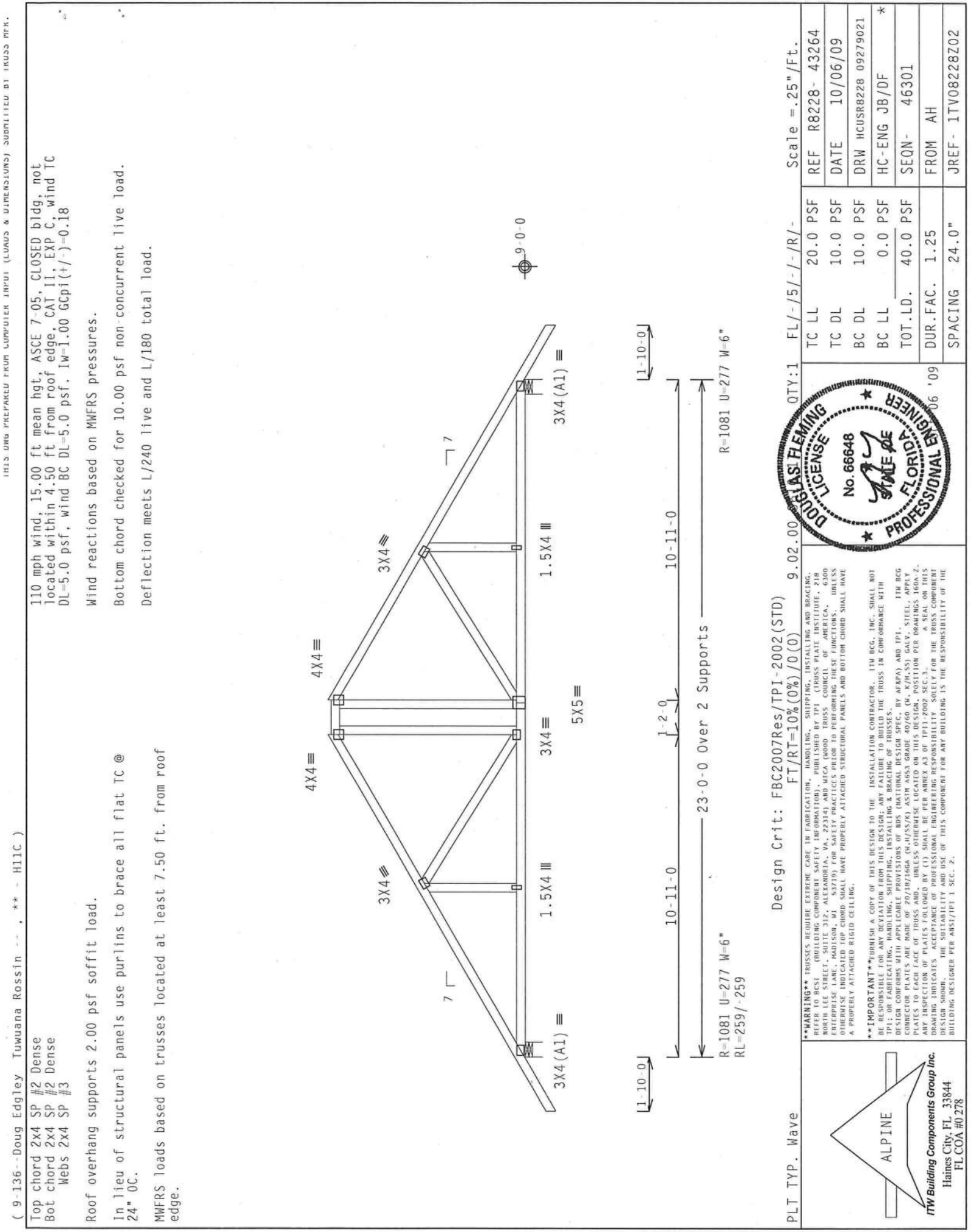
9.02.08

QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0	PSF	REF	R8228-	43263
TC DL	10.0	PSF	DATE	10/06/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09279010
BC LL	0.0	PSF	HC-ENG	JB/DF	*
TOT.LD.	40.0	PSF	SEQN-	46311	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TV08228Z02	

[illegible][illegible]



Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Special loads

		Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From	63 plf at 0.00 to	63 plf at 11.50
TC - From	63 plf at 11.50 to	63 plf at 24.83
BC - From	20 plf at 0.00 to	20 plf at 7.46
BC - From	70 plf at 7.46 to	70 plf at 9.42
BC - From	110 plf at 9.42 to	110 plf at 12.00
BC - From	110 plf at 12.00 to	110 plf at 13.67
BC - From	70 plf at 13.67 to	70 plf at 15.54
BC - From	20 plf at 15.54 to	20 plf at 23.00
BC - From	5 plf at 23.00 to	5 plf at 24.83

Wind reactions based on MWFRS pressures.

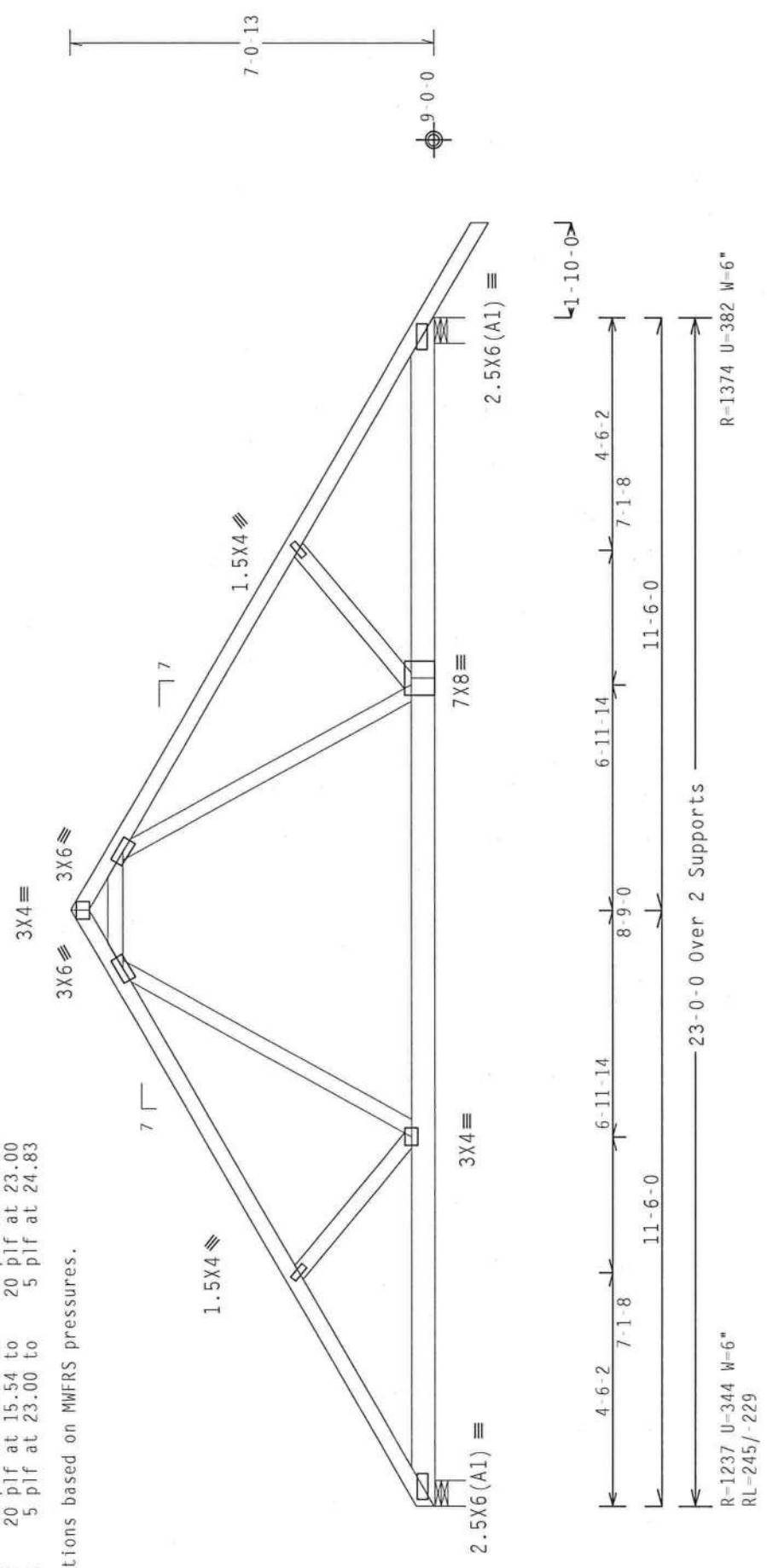
** The maximum horizontal reaction is 245# **

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / 0 (0)

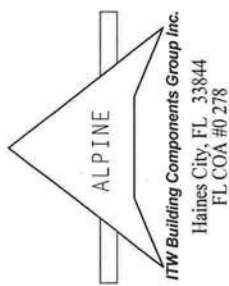
QTY: 4	FL / - / 5 / - / - / R / -	Scale = .3125" / Ft.
TC LL	20.0 PSF	REF R8228 - 43266
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279019
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN - 67856
DUR.FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF - 1TV08228202



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CROSS PLATE INSTITUTE, 218 INDUSTRIAL STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND TPI CROSS PLATE INSTITUTE'S WEBSITE, www.tpiinc.com, FOR ADDITIONAL INFORMATION. UNLESS OTHERWISE INDICATED FOR CHORD, CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** UNLESS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, ITM BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RES (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 20/10/166A (4.0/55/5) ASTM A653 GRADE 40/60 (4, 8/21/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

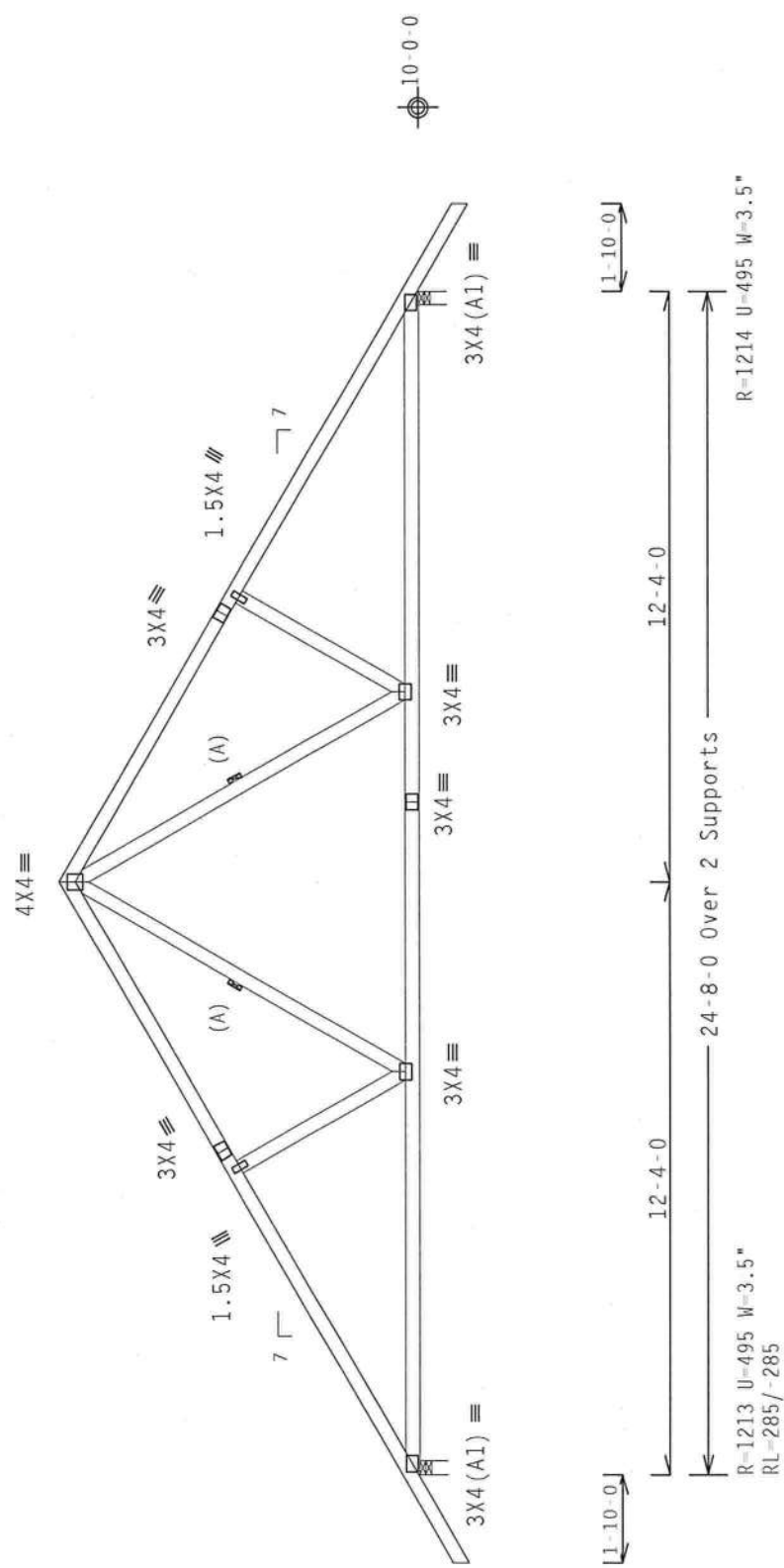
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.55

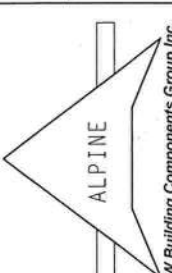
Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

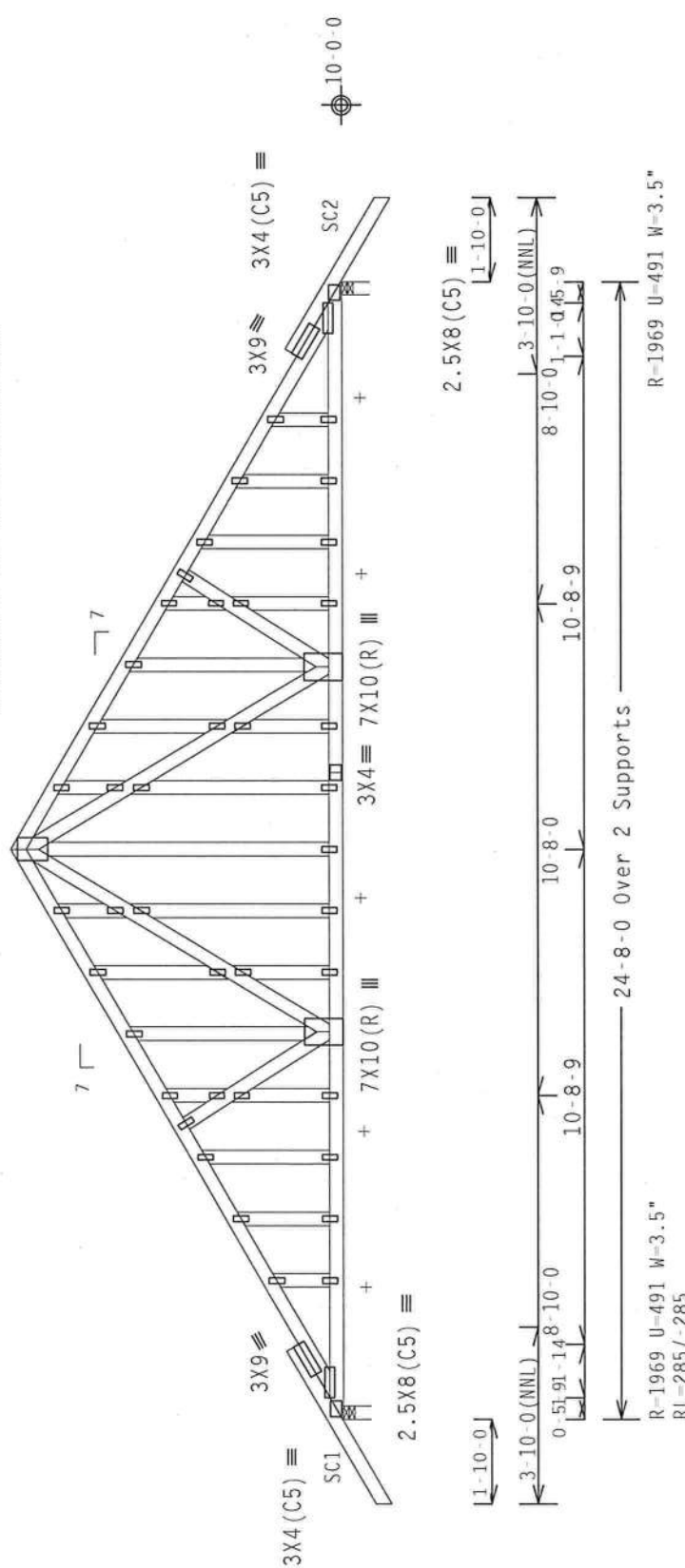
Deflection meets L/240 live and L/180 total load.



PLT TYP. Wave		Design Crit: FBC2007Res/TPI-2002(STD) FT/RT=10%(0%)/0(0)		9.02.00.00		QTY:3		FL/-/5/-/-/R/-		Scale =.25"/Ft.	
 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278		**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2318 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 CRISPVIEW LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL TRUSSES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.									
		IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.									
		DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PSA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 70/18/15GA (W/H/S/S/E) ASTM A653 GRADE 40/60 (W, K/B, S/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.									
		ANY INSPECTION OF PLATES FOLLOWED BY (3) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) FOR THE DESIGN SUBMITTAL. THE DESIGN SUBMITTAL SHALL BE REVIEWED FOR THE DESIGN OF THE TRUSS. A SEAL ON THIS DESIGN SUBMITTAL SHALL BE REQUIRED FOR THE TRUSS. THE SUBMITTAL AND USE OF ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMST/TPI 1 SEC. 2.									
											

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART-ENC. bldg,
Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind
BC DL=5.0 psf. lw=1.00 Gcpi(+/-)=0.55
Wind reactions based on MWFRS pressures.
Truss spaced at 24.0" OC designed to support 1-6-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord
must not be cut or notched.
Truss passed check for 20 psf additional bottom chord live load in
areas with 42"-high x 24"-wide clearance.
Bottom chord checked for 10.00 psf non-concurrent live load.
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE
ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND
SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS
LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE
DESIGNED BY THE BUILDING DESIGNER.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.
See DWGS A140G020109 & A140G020109 for more requirements.
Stacked top chord must NOT be notched or cut in area (NNL). Attach
stacked top chord (SC) to dropped top chord in notched area
using 3x4 tie plates 24" o.c. Center plate on stacked/dropped
chord interface, plate length perpendicular to chord length.
Splice top chord in notched area using 3x6.
Deflection meets L/240 live and L/180 total load.
+ MEMBER TO BE Laterally Braced for Out of Plane Wind Loads.
BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

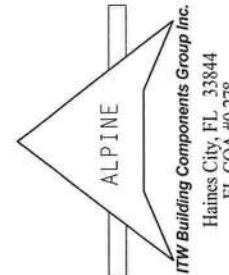


Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%) 70(0) 9.02-00



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD CONNECTIONS ASSOCIATION), 1000 ENTERPRISE LANE, SUITE 1500, FARMINGTON, CT 06031 FOR BEST PRACTICES. ALL FABRICATORS, SHIPPERS, UNLESS OTHERWISE SPECIFIED, SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCSI, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PDA) AND TPI. THE BCSI CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/SS/P) ASIM A653 GRADE 40/60 (W, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 16GA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



QTY: 1	FL / - / 5 / - / - / R / -	Scale = .25" / Ft.
TC LL	20.0 PSF	REF R8228- 43268
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279015
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEON- 46101
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228202

Top chord	2x4	SP #2	Dense
Bot chord	2x4	SP #2	Dense
Webbs	2x4	SP #3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWERS pressures.

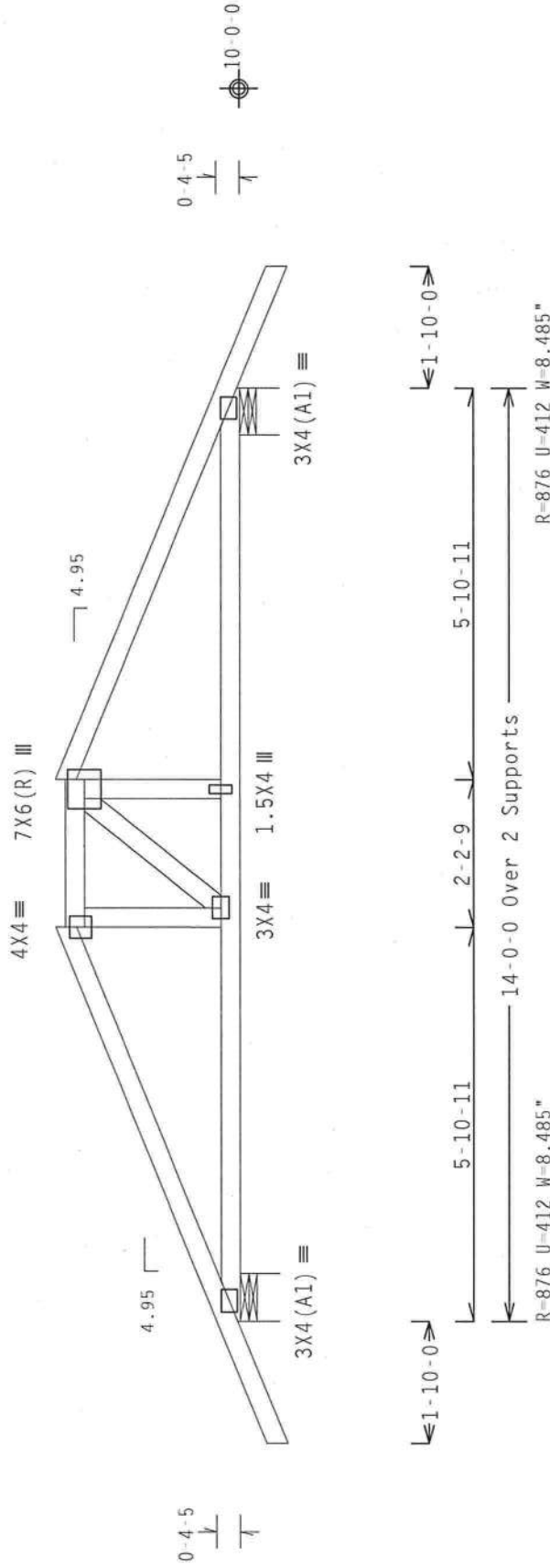
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.

Special loads

[illegible]Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

9.02.00

QTY:2 FL/-/5/-/-/R/-

Scale = .375"/Ft.

WARNING: THESE REQUIREMENTS CAME INTO EFFECT, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RESIST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CEILING PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, AXMINSTER, VA 22014, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ILM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE OR INJURY TO PERSONS OR PROPERTY THAT MAY OCCUR DUE TO THE DESIGN OR THE INSTALLATION OF THIS DESIGN. ANY FAILURE TO BUILD THE THRUSS IN CONFORMANCE WITH THE DESIGN OR THE INSTALLATION OF THIS DESIGN, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

DESIGN COMPONENTS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISC) AND TPI.

1.1M BCG CONNECTOR PLATES ARE MADE OF 2010/18/16GA. (MAY/25/2K) ALUM A653 GRADE 40/60 (N. K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF THRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION THRU DRUMINGS 160A-2. A SEAL ON THIS DRUMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE THRUSS COMPONENT OF THIS DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIAA/TPI 1 SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpi(+/-)=0.18

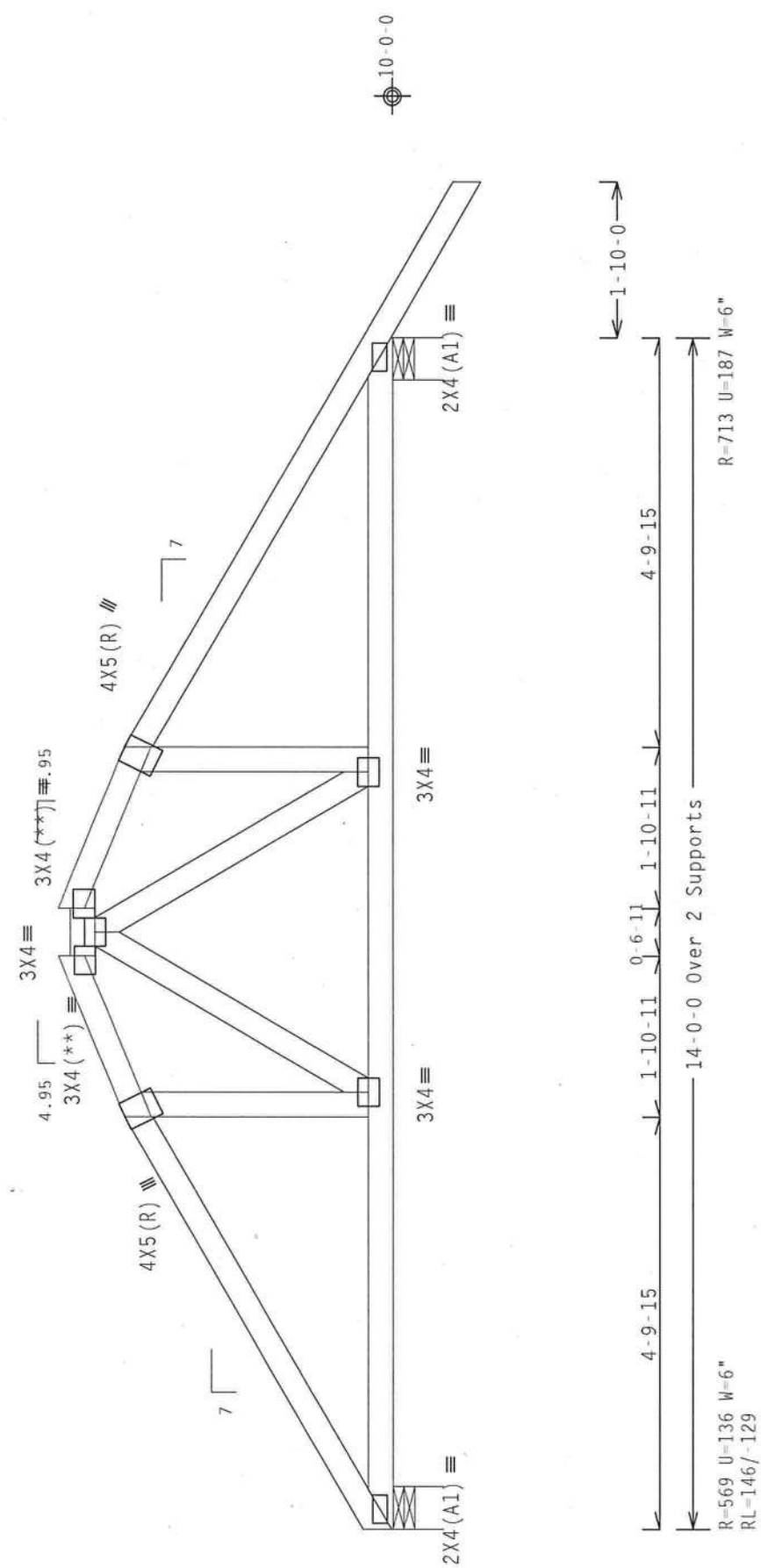
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

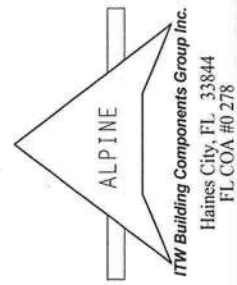
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .5"/Ft.

TC LL	20.0	PSF	REF R8228- 43270
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUR8228 09279005
BC LL	0.0	PSF	HC-ENG JB/DF
TOT.LD.	40.0	PSF	SEQN- 46381
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF- 1TV08228Z02



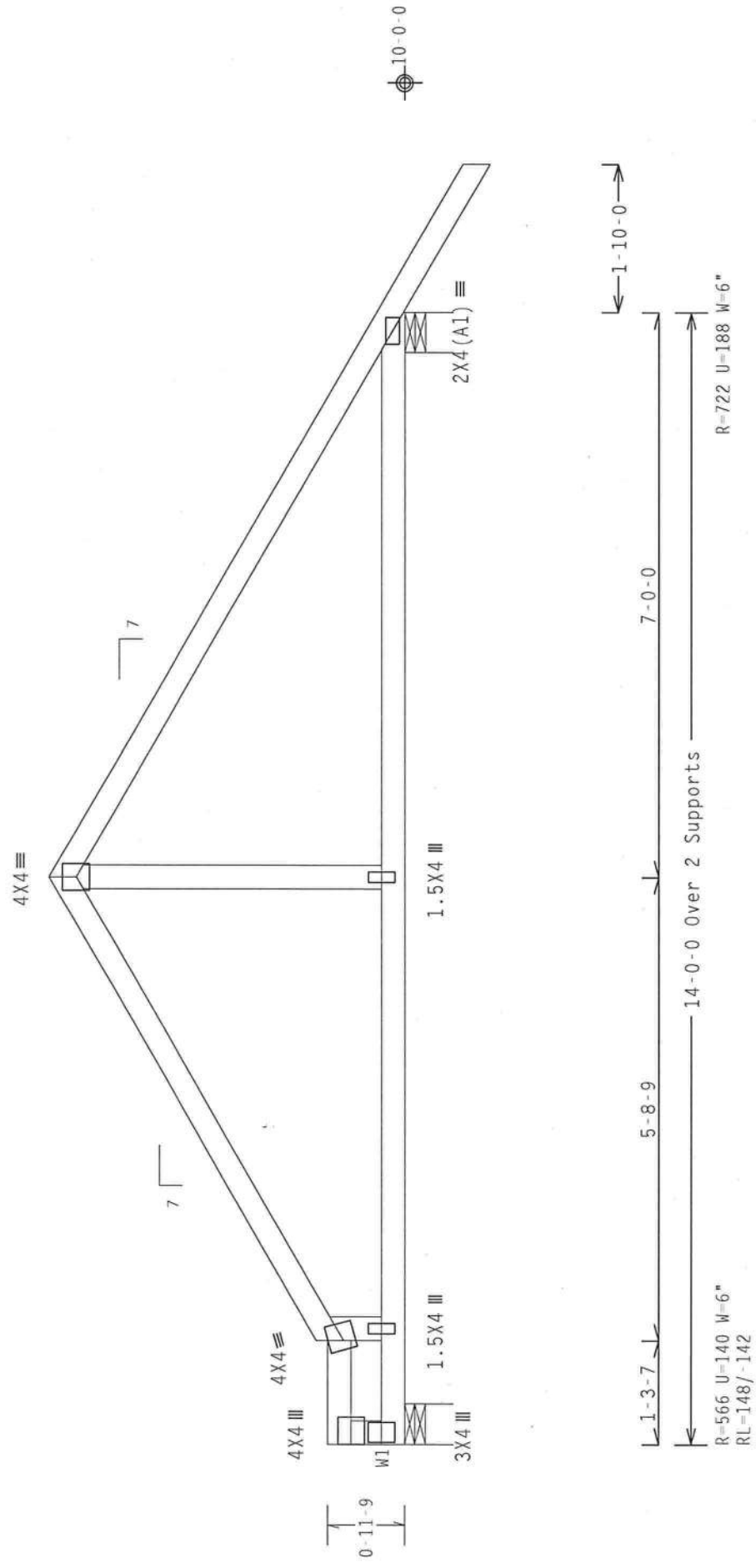
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DECSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MICHIGAN AVE., CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHANGING.

[illegible]

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W1 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

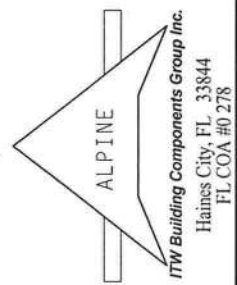


Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave	QTY: 2		FL / - / 5 / - / - / R / -	Scale = .5" / Ft.
	TC LL	20.0 PSF	REF R8228- 43271	
	TC DL	10.0 PSF	DATE 10/06/09	
	BC DL	10.0 PSF	DRW HCUSR8228 09279053	
	BC LL	0.0 PSF	HC-ENG JB/DF	*
TOT.LD.			SEQN- 46387	
DUR.FAC.			FROM AH	
SPACING			JREF- 1TV08228202	



****WARNING**** TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. THESE TRUSSES ARE THE PROPERTY OF ITW BUILDING COMPONENTS GROUP INC. AND ARE TO BE USED ONLY FOR THE PROJECT AND LOCATION SPECIFICALLY INDICATED ON THIS DRAWING. ANY OTHER USE IS PROHIBITED. THE USER OF THESE TRUSSES SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM DAMAGE AND LOSS. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THEFT AND VANDALISM. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE WEATHER. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE FIRE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE COLLAPSE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE OVERLOAD. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE IMPACT. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CORROSION. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DEFORMATION. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CRACKING. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SPLITTING. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE BUCKLING. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE TENSILE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE COMPRESSIVE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SHEAR FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE BOLT FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE WELD FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE FASTENER FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CONNECTION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DETAILING FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE MANUFACTURING FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE TRANSPORTATION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE STORAGE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE HANDLING FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE INSTALLATION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE MAINTENANCE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DEMOLITION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DISPOSAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RECYCLING FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ENVIRONMENTAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SOCIAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ECONOMIC FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE POLITICAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CULTURAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RELIGIOUS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ETHNIC FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE LANGUAGE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE NATIONALITY FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CITIZENSHIP FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RESIDENCE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE OCCUPATION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE EDUCATION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE EMPLOYMENT FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE INCOME FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE WEALTH FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE STATUS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE REPUTATION FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CREDIT FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RISK FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE LIABILITY FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE INSURANCE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE TAX FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE LEGAL FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE REGULATORY FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE COMPLIANCE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE STANDARDS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CODES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SPECIFICATIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE REQUIREMENTS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE CONDITIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE LIMITATIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RESTRICTIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE PROHIBITIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE MANDATES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ORDINANCES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DECREES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RESOLUTIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ORDERS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE DIRECTIVES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE INSTRUCTIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE GUIDELINES FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RECOMMENDATIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SUGGESTIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ADVICE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE OPINIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE RECOMMENDATIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE SUGGESTIONS FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE ADVICE FAILURE. THE USER SHALL BE RESPONSIBLE FOR THE PROTECTION OF THE TRUSSES FROM THE OPINIONS FAILURE.



Top chord	2x4	SP #2	Dense
Bot chord	2x4	SP #2	Dense
Webbs	2x4	SP #3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf Iw=1.00 GCpi(+/-)=0.18

Left end vertical not exposed to wind pressure.

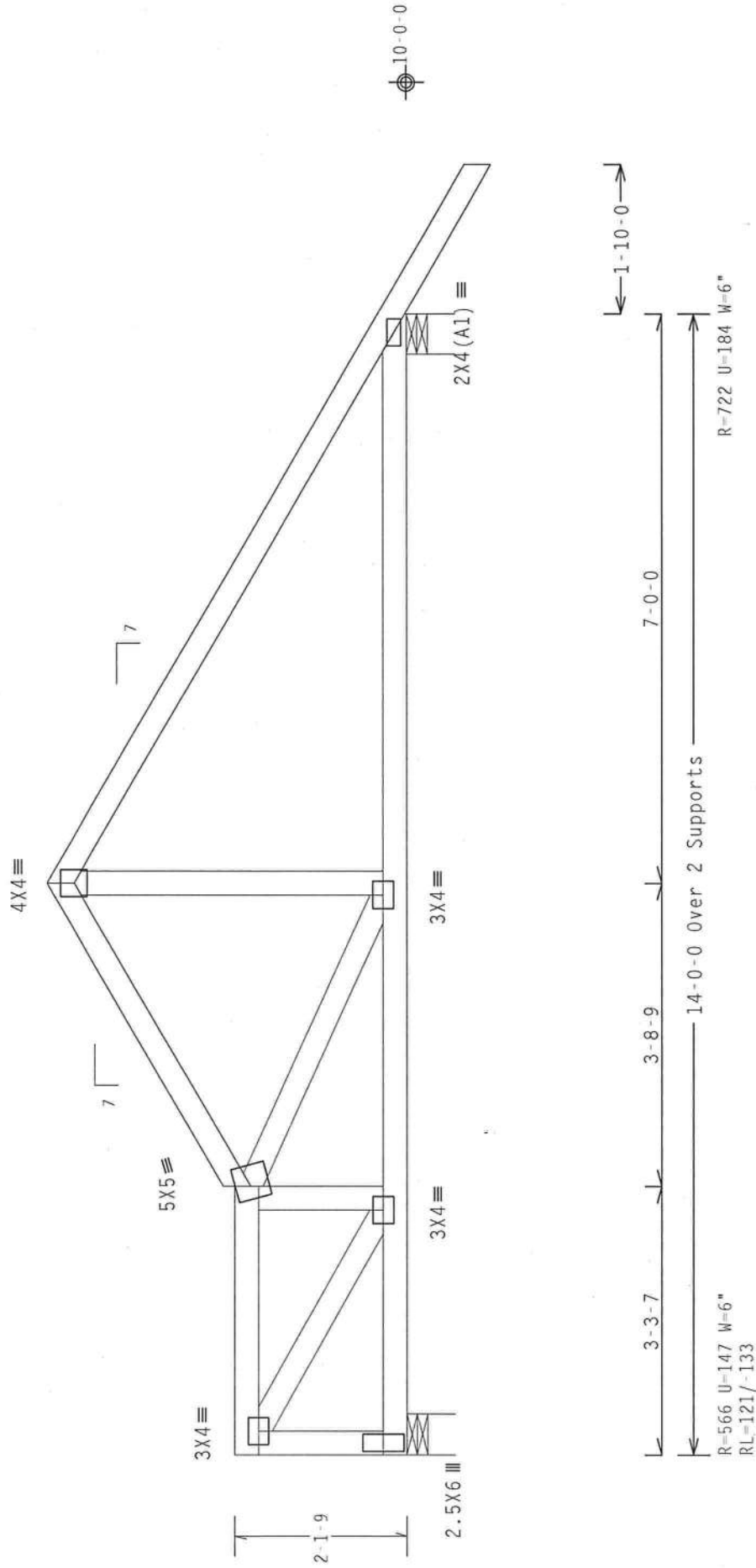
Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Design Crit: FBC2007Res/TPI-2002(STD)
 $ET/RT=10\%(0\%)/0(0)$

design: c:\rc\p2007\res\p1_2002\3109

FT/RT=10%(0%)T(0) 9

***WARNING** TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BEST BUILDING COMPONENT SHEET INFORMATION FOR PUBLISHED INFO. CROSS PLATE INSTITUTION, 218 CHILDRIDGE LANE, MADISON, MI 48219 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE JOINTS. UNLESS OTHERWISE INDICATED FOR GIRDERS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID JOINTS.

[illegible]

PIT TYP. Wave



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



FL/-/5/-/-/R/-	Scale = .5" / Ft.	
TC LL 20.0 PSF	REF R8228 -	43272
TC DL 10.0 PSF	DATE 10/06/09	
BC DL 10.0 PSF	DRW HCUSR8228	09279054
BC LL 0.0 PSF	HC-ENG JB/DF	*
TOT.LD. 40.0 PSF	SEQN -	46391
DUR.FAC. 1.25	FROM AH	
SPACING 24.0"	JREF -	1TV08228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

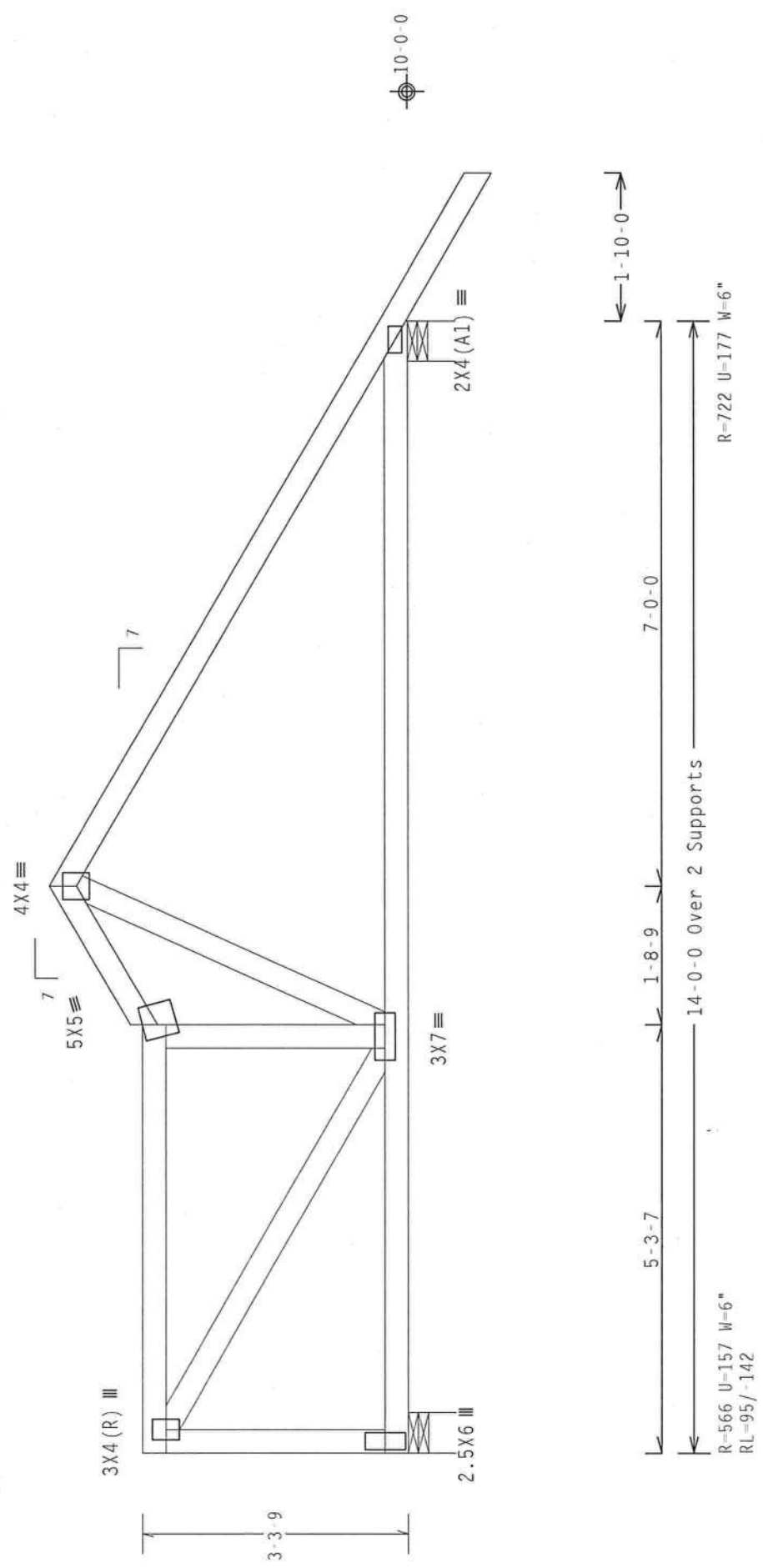
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

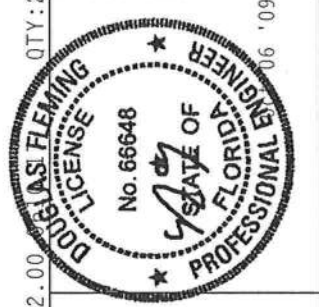
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

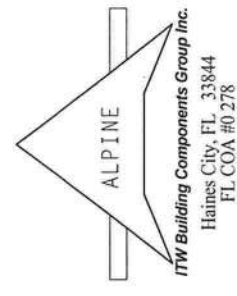
QTY: 2	FL/-5/-/-/R/-	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R8228- 43273
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279009
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN - 46397
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TV08228Z02



****WARNING**** TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION), 500 N. MERRILL STREET, PITTSBURGH, PA, 15222 FOR PROPER FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/P&A) AND TPI. 1TH BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/H/SS/K) ASTM A653 GRADE 40/60 (N, K/H/SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIMENSIONS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI-2002 SEC.3. A SEAL OR THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3

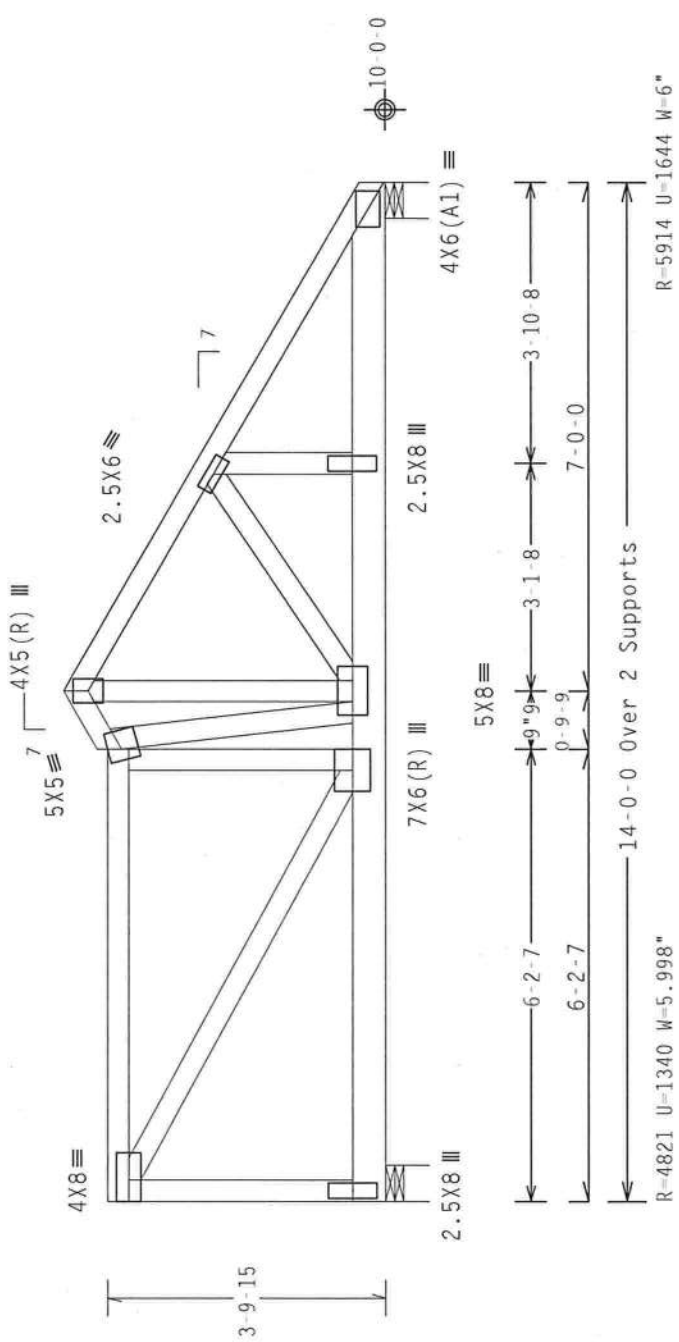
Special loads

- (Lumber Dur. Fac.=1.25 / Plate Dur. Fac.=1.25)
- TC - From 63 pif at 0.00 to 63 pif at 6.20
 - TC - From 63 pif at 6.20 to 63 pif at 7.00
 - TC - From 63 pif at 7.00 to 63 pif at 14.00
 - BC - From 20 pif at 0.00 to 20 pif at 14.00
 - BC - 1178 lb Conc. Load at 1.06
 - BC - 1306 lb Conc. Load at 3.06
 - BC - 1258 lb Conc. Load at 5.06
 - BC - 1172 lb Conc. Load at 7.06
 - BC - 1155 lb Conc. Load at 9.06
 - BC - 1158 lb Conc. Load at 11.06
 - BC - 2344 lb Conc. Load at 13.06

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{Cpf}(+/-)=0.18$
Wind reactions based on MWFRS pressures.
Left end vertical not exposed to wind pressure.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) 70(0)

PLT TYP. Wave



ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 STREET, SUITE 312, ALPHARETTA, GA 30009, (770) 962-1111, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, MODIFYING OR REMOVING ANY PART OF THE TRUSS OR ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AF&PA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (N/A/SS/K) ASTM A653 GRADE 40/60 (H, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABX 43 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

QTY: 1	FL / - / 5 / - / - / R / -	Scale = .375" / Ft.
TC LL	20.0 PSF	REF R8228- 43274
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279020
BC LL	0.0 PSF	HC-ENG DF/DF
TOT. LD.	40.0 PSF	SEON- 67916
DUR. FAC.	1.25	FROM AH
SPACING	SEE ABOVE	JREF- 1TV08228Z02

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails

Top Chord: 1 Row @12.00" o.c.

Bot Chord: 2 Rows @ 6.00" o.c. (Each Row)

Webs : 1 Row @ 4" 0.c.

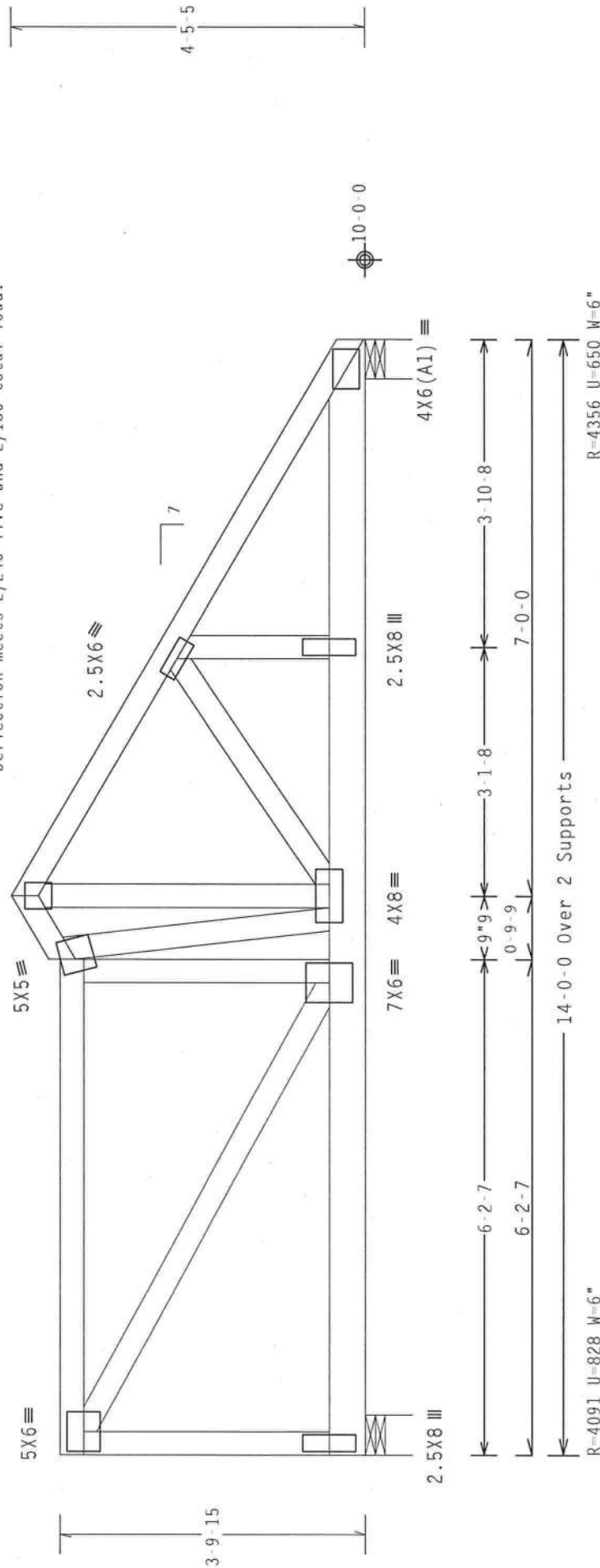
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)[illegible]

*****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, PERFORMED BY THE TRUSS MANUFACTURER, WILL BE AT THEIR OWN RISK AND WITHOUT LIABILITY TO ITM BCG.

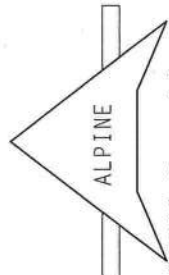
CORRUGATED PLATES ARE MADE OF 26/18/10/16GA OR 26/18/10/14 GA, AL-5052 H14 GALV. STEEL - APPLY PLATES TO EACH FACE OF TRUSS AND, WHEN APPLICABLE, OVER THE JOINTS. POSITION PER DRAWINGS 16DA-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBEX 4.3 OF TP11-2002 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER SUBMITTAL/TP11 SEC. 2.

PLOT TYP. Wave

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R8228- 43275
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279037
BC LL	0.0 PSF	HC-ENG KD/AP
TOT.LD.	40.0 PSF	SEQN- 49101
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02



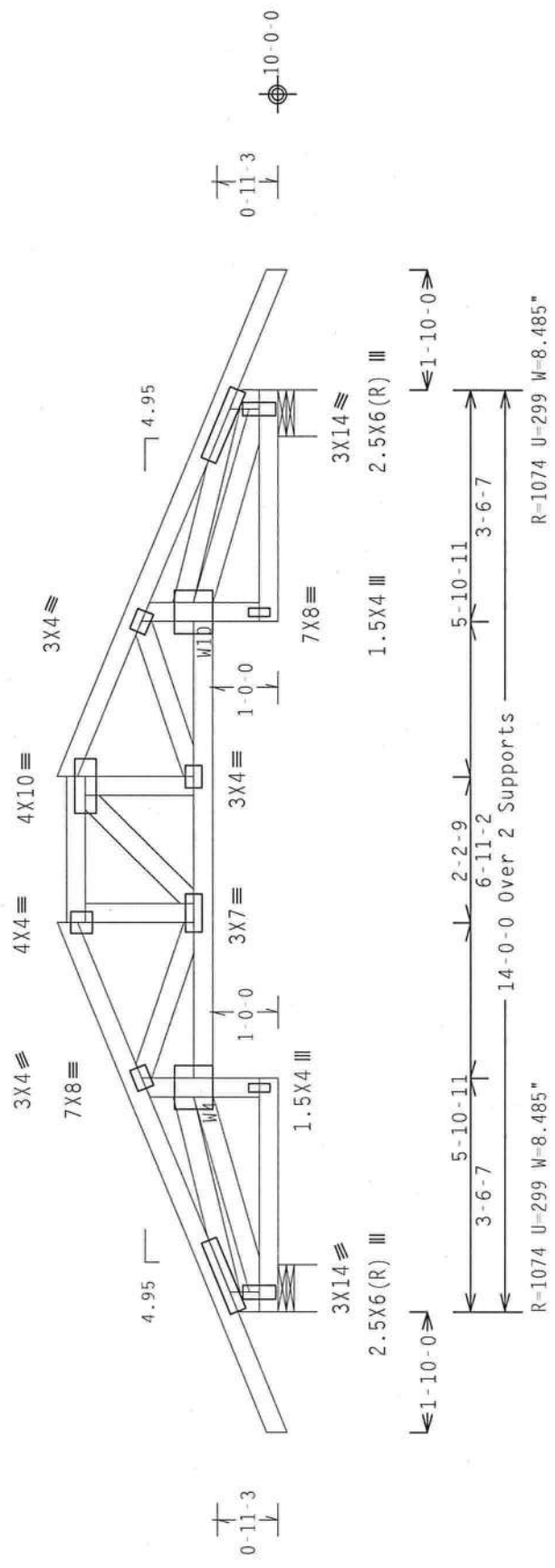
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

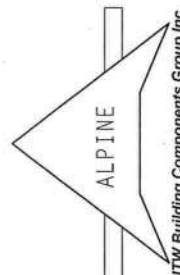
(9-136--Doug Edgley Tuwana Rossin -- , ** - H4F)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W4, W10 2x4 SP #2 Dense:
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Wind reactions based on MWFRS pressures.
#1 hip supports 5-10-11 jacks W/2 panel TC and no end vert.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18





ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

Scale = .375"/Ft.

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENT SAFETY PROGRAM) PUBLISHED BY THE CRUISE PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (M/M/SS)K1 ASIN A653 GRADE 40/60 (M. K/M-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABCEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELLY FOR THE TRUSS COMPONENT MANUFACTURER. THE ACCEPTANCE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DORCAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

06 '09

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R8228- 43276
DATE 10/06/09
DRW HCUSR8228 09279052
HC-ENG JB/DF
SEON- 46472
FROM AH
JREF- 1TV08228Z02

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

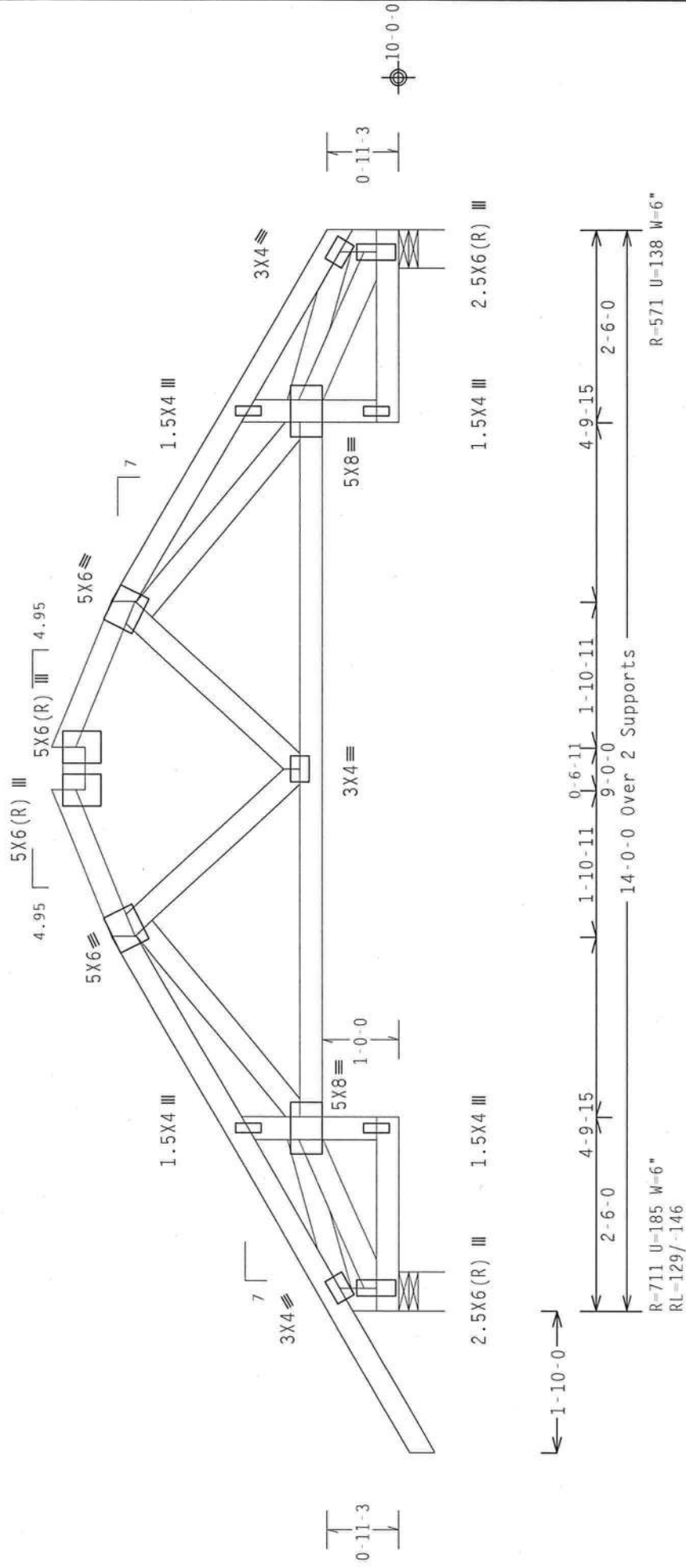
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI - 2002(STD)
FT/RT=10%(0%) / 0(0)

PLT TYP. Wave

QTY:1 FL/-/5/-/-/R/-

Scale = .5"/Ft.

WARNING TRUSSES, PLACING THE EXTERIOR CABLE IN PARTICULATION, HARD INCS., SHIPPING, INSTALLING AND BRACING REFER TO RESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CHIEFS OF PLANT INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALXANDRIA, VA, 22314, FOR A SAFE (GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719), FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0	PSF	REF R8228 - 43277
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUSR8228 09279017
BC LL	0.0	PSF	HC-ENG JB/DF
TOT.LD.	40.0	PSF	SEQN - 46551
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF - 1TV08228Z02



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

3 COMPLETE TRUSSES REQUIRED

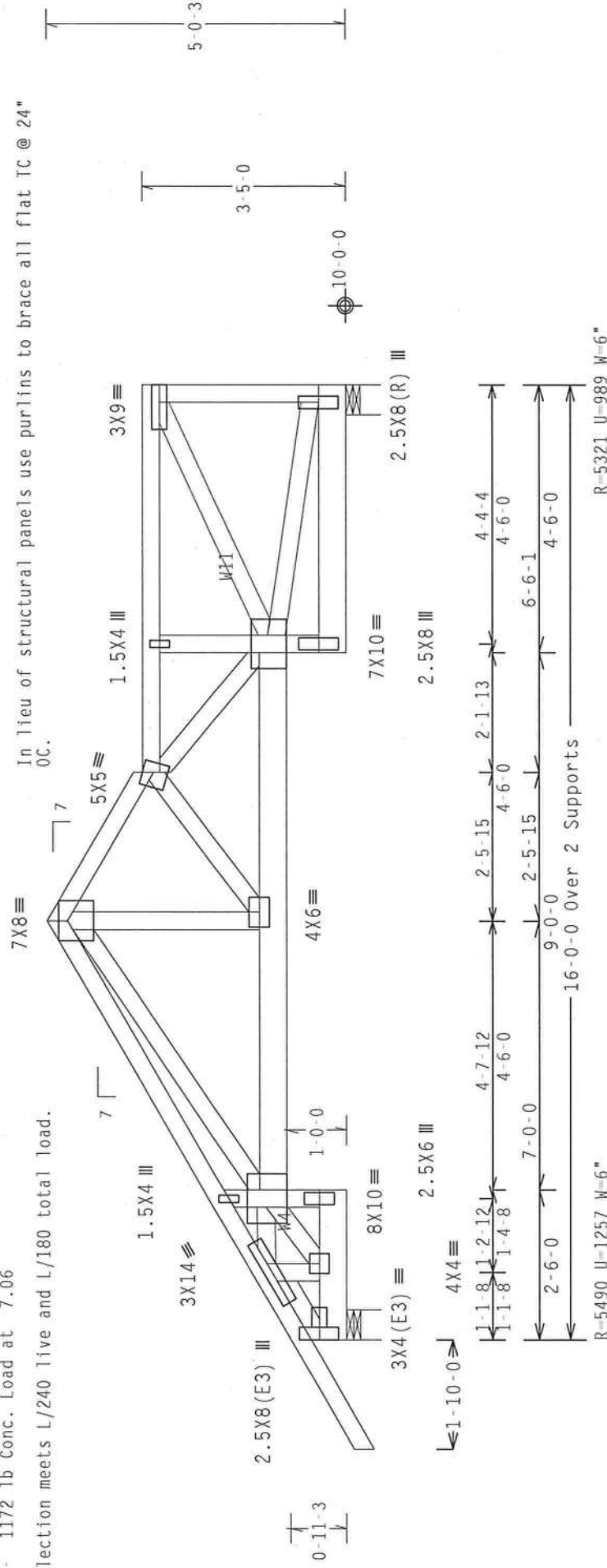
Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing
between rows and stagger nails in each row to avoid splitting.
110 mph wind, 15.00 ft mean hgt. ASCE 7-05, CLOSED bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0
psf. Iw=1.00 GCpf(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



R=5490 II=1257 W=6"

R-5321 U-989 W-6"

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

FT/RT=10%(0%)/0(0)

QTY:1 FL/-/5/-/-/R/-

Scale = .375"/Ft.

WARNING* BROSSES REQUIRE EXTREME CARE IN FABRICATION, DRESSING, SHIPPING, INSTALLING AND BRACING. IF THESE COMPONENTS ARE NOT PROPERLY PREPARED, THE WEIGHT OF THE PANELS WITH THEIR PIVOT POINTS WILL CAUSE THEM TO FALL, RESULTING IN PERSONAL INJURY OR DEATH. CONTACT THE LOCAL AFGA HUBBES COUNCIL FOR AFGA ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOLD CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHORD.

TC LL	20.0	PSF	REF	R8228-	43278
TC DL	10.0	PSF	DATE	10/06/09	
BC DL	10.0	PSF	DRW	HCUSR8228	09279021
BC LL	0.0	PSF	HC-ENG	DF/DF	
TOT.LD.	40.0	PSF	SEQN-	67913	
DUR.FAC.	1.25		FROM	AH	
SPACING	24.0"		JREF-	1TV08228Z02	

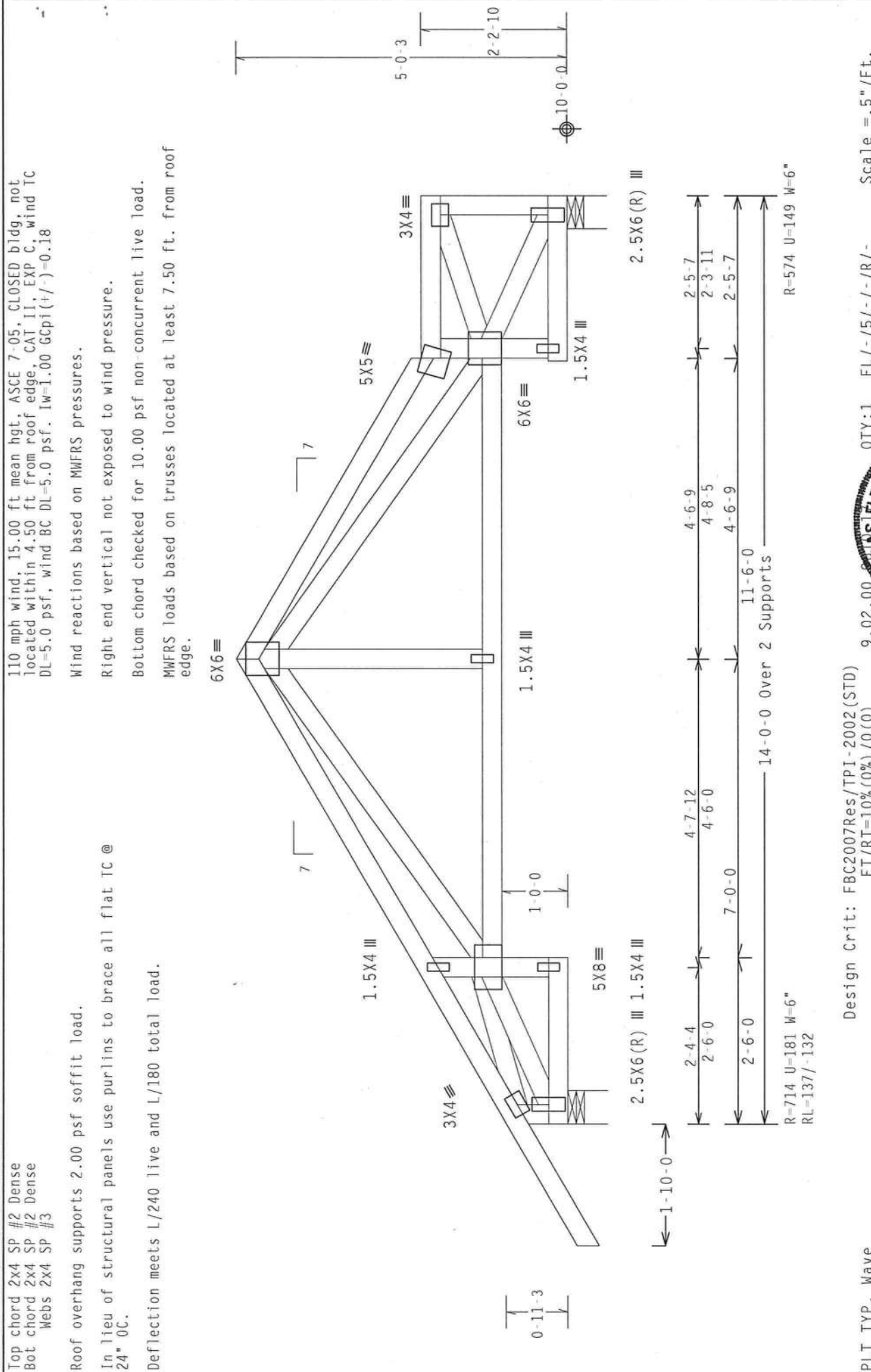
DOUGLAS FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE STEEL INSTITUTE, 2101 LEXINGTON AVENUE, PITTSBURGH, PA 15222, FOR TRUSS SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC, BY AREA) AND TPI. 1TH BCG CORRELATION PLATES ARE MADE OF 70/10/166A (4-10/55/16) ASH 6053 GRADE 40/60 (14, 4/21/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS. NON-INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE A BASIS FOR ANY CLAIMS FOR THE TRUSS COMPONENTS. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE LIABILITY AND THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI SEC. 2.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

9.02.00

QTY:1

FL/-/5/-/-/R/-

Scale = .5" / Ft.

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	
SPACING	24.0"	

DOBBS FLEMING
LICENSE
No 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

06 '09

****WARNING**** TRUSSES ROUTINE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HOUSTON, TX 77061) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, ALL DIMENSIONS AND MATERIALS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AIA/AS) GALV. STEEL. APPLY CORRECTOR PLATES ARE MADE OF 70/10/16GA (40/10/16GA) ASTM A653 GRADE 40/60 (40/60) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

REF R8228 - 43279

DATE 10/06/09

DRW HCUSR8228 09279020

HC-ENG KD/AP

SEQN- 48705

FROM AH

JREF- 1TV08228Z02

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-) -0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

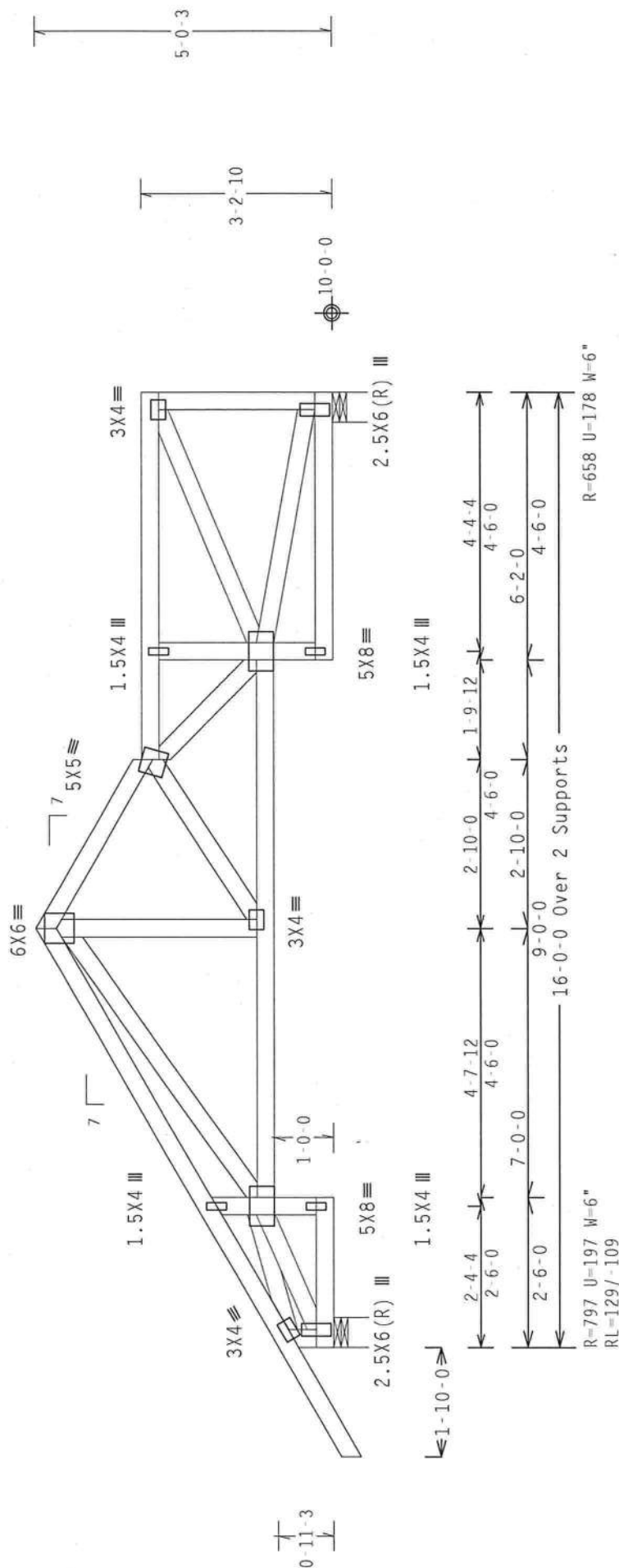
Bottom chord checked for 10.00 psf non-concurrent live load.

MMWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI -2002(STD)
FT/RT=10%(0%)/0(0)

QTY:1 FL/-/5/-/-/R/-
Scale =.375"/Ft.

PLT TYP. Wave

TC LL	20.0	PSF	REF R8228- 43281
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCUSR8228 09279011
BC LL	0.0	PSF	HC-ENG KD/AP *
TOT.LD.	40.0	PSF	SEQN- 48727
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF- 1TV08228Z02

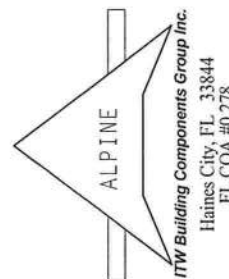


WARNING THUSSES REQUIRE TIGHTENING CARE IN INSTALLATION. HANDLING, SHIPPING, UNLOADING AND UNPACKING MUST BE DONE WITH EXTREME CARE TO PREVENT DAMAGE TO THE THUSSES. PUBLISHED BY THE THUSSES PLANT, LITTLETON, COLORADO 80120. COMPANY, SAFETY INFORMATION, 221A W. 10TH AVE., DENVER, COLORADO 80202. THUSSES PLANT, LITTLETON, COLORADO 80120. NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314. AND WCA, 6000 THUSS, COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, SUITE 508, MI 53119. FOR SAFETY PRACTICES PRIOR TO PERFORMING THE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IIT BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THIS DESIGN, INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING, SHALL BE THE SOLE RESPONSIBILITY OF THE DESIGNER AND/OR THE INSTALLATION CONTRACTOR:

- ALL CONNECTIONS MUST BE MADE IN ACCORDANCE WITH THE DESIGN SPECIFICATIONS.
- ALL TROSS PLATES TO WHICH CORROSION RESISTANT PROTECTION IS APPLIED MUST BE TYPE 304 STAINLESS STEEL OR ALUMINUM.
- ALL TROSS PLATES ARE MADE OF 2608/AL6061 OR A MINIMUM GRADE 60/60 (H.K./SI) GALV. STEEL, APPLY TO EACH FACE OF TROSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBIX A3 OF 1971-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TROSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASHRAE/TPI 1 SEC. 2.



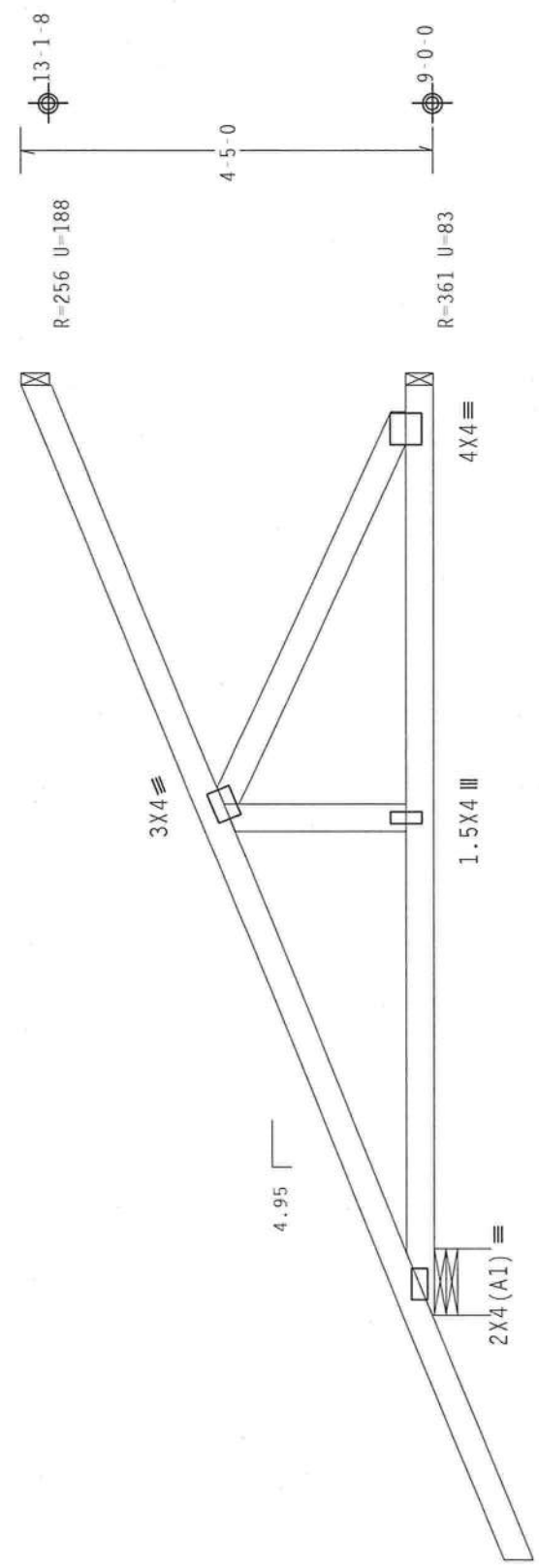
(9-136--Doug Edgley Tuwuana Rossin -- ** - HJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg.
Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind
BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

Hipjack supports 7'-0" setback jacks with no webs.
Deflection meets L/240 live and L/180 total load.



2-7-2
9-10-13 Over 3 Supports
R=518 U=265 W=8.485"

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / 0 (0)



QTY: 5	FL / - / 5 / - / - / R / -	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R8228 - 43282
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279014
BC LL	0.0 PSF	HC-ENG JB/DF
TOT. LD.	40.0 PSF	SEQN - 46337
DUR. FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TV08228202

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CROSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22304 AND TPI'S TRUSS DESIGN SPEC. (TDS) FOR TRUSS DESIGN. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AWS (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (4-10/55/K) ASTM A653 GRADE 40/60 (4-8/10/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AWS/TPI 1 SEC. 2.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

PLT TYP. Wave

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

110 mph wind, 15.00 ft mean htg, ASCE 7-05, PART-ENC. bldg.
Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind
BC DL=5.0 psf. $I_w=1.00$ GCpi (+/-)=0.55

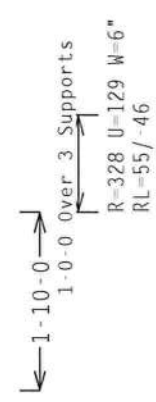
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.

R=94 Rw=63 U=94



R=1 Rw=32 U=36



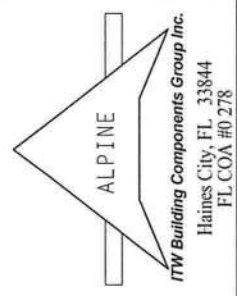
R=328 U=129 W=6"
RL=55/-46

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / 0(0)

PLT TYP. Wave	QTY:10		FL/-/5/-/-/R/-		Scale = .5" / Ft.	
	TC LL	20.0 PSF	REF	R8228- 43283		
	TC DL	10.0 PSF	DATE	10/06/09		
	BC DL	10.0 PSF	DRW	HCUSR8228 09279019		
	BC LL	0.0 PSF	HC-ENG	JB/DF		
	TOT.LD.	40.0 PSF	SEQN	46327		
		DUR.FAC.	1.25	FROM	AH	
		SPACING	24.0"	JREF	1TV08228Z02	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AS) AND TPI-1. ITW BCG CONNECTOR PLATES ARE MADE OF 2019/16GA (4-H/55/18) ASTM A553 GRADE 40/60 (4, 8/10, 55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED OR THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS PER 11-2002 SPEC.3. A QUAL APPROVED DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense

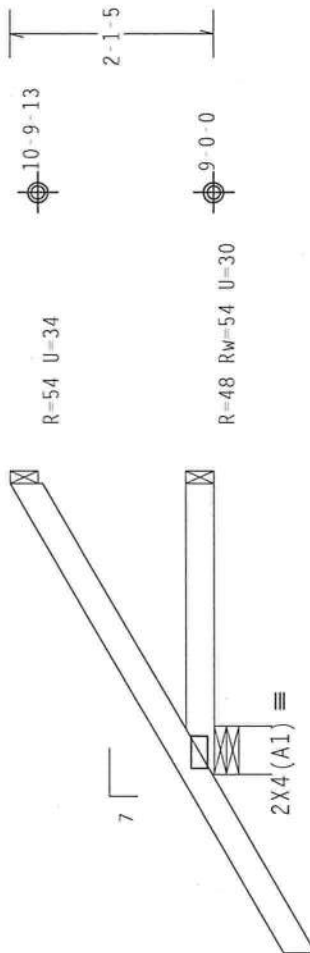
11110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.55

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



← 1-10-0 →
3-0-0 Over 3 Supports
R=302 U=97 W=6"
RL=95/-55

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

QTY: 10 FL / - / 5 / - / - / R / -
Scale = .5" / Ft.

•WARNING• TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RECSI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE CHUOIS PLATE INSTITUTE, 218 NORTH LEE ROAD, SUITE 312, ALEXANDRIA, VA 22314, AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, BRIGHTON, MA 02135) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0 PSF	REF R8228 - 43284
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279023
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 46330
DDUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02

60, 90

WARNING**TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP (BUILDING COMPONENTS SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, HAMILTON, MI 48379) FOR VARIOUS TRUSS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT**SUBMIT A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING SPECIFICATIONS WILL BE AT THE USER'S RISK. IF ANY CONNECTOR PLATES ARE MADE OF 20/18/16GA (.040/.035/.030 ASIN A563 GRADE 40/60 (A, K/HJ/SLS) GALV., STEEL, ALL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ABXZ OF TP1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT MANUFACTURING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ABXZ/TP1 1 SEC. 2.

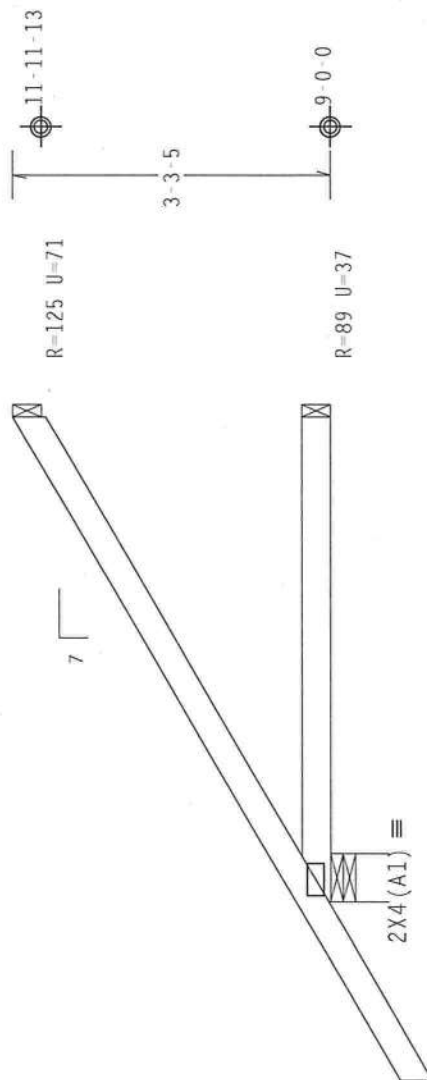


ITW Building Components Group Inc.
Haines City, FL 33844
EI COA #0 278

1110 mph wind, 15.00 ft mean hgt., ASCE 7-05, PART_ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 GCpi (+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



$\leftarrow 1 \cdot 10 \cdot 0 \rightarrow$
 $\leftarrow 5 \cdot 0 \cdot 0 \text{ Over } 3 \text{ Supports} \rightarrow$
 $R = 366 \text{ U} - 114 \text{ W} = 6"$
 $RI = 135 / 64$

Design Crit: FBC2007Res/TPI - 2002 (STD)
FT/RT=10%(0%)/0(0)

Scale = .5" / Ft.

9.02.00

QTY:10 FL/-/5/-/-/R/-/-

Scale = .5"/Ft.

OFF B8228- 43285

DATE 10/06/00

DATE 10/06/09

ORW HCUSR8228 0927903

IC-ENG JB/DF

46333

	ALL
EDQM	ALL

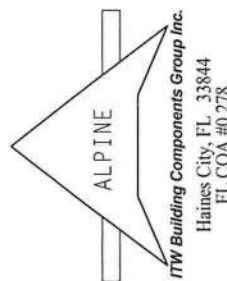
RUM AH

[illegible]

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

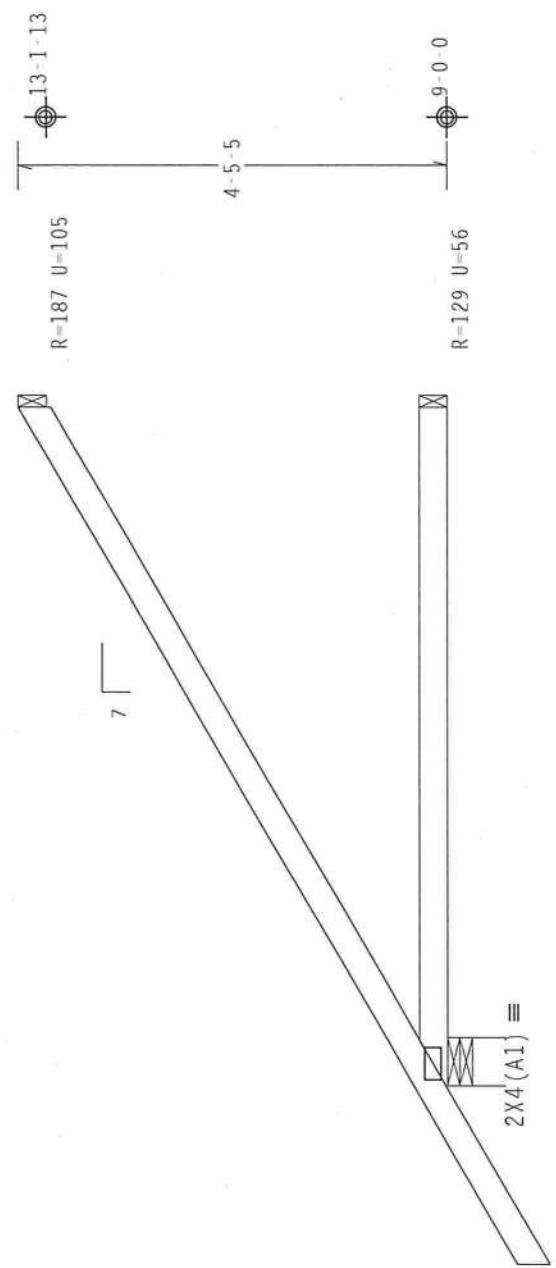
DESIGNER CONFORMS WITH APPLICABLE PROVISIONS OF NATIONAL DESIGN SPEC., BY AISC/AISI AND TPI. ALL CONNECTION PLATES ARE MADE OF 20/10/16GA (W. JESS/6) ASTM A563 GRADE 40/60 (W. JESS) GALV. STEEL. ALL PLATES TO EACH FACE OF THISS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMBIX AS OF TPI1 2002 SEC.3).

THE FOLLOWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE THISS COMPONENT DESIGNER SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGNER. (SEE DRAWING DESIGNER PER AMBIX/TPI 1 SEC. 2).



(9-136--Doug Edgley Tuwana Rossin -- , ** - EJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, PART ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind IC DL=5.0 psf, wind BC DL=5.0 psf. $1w=1.00 GCpi(+/-)=0.55$
Wind reactions based on MWFRS pressures.
Deflection meets L/240 live and L/180 total load.



1-10-0
7-0-0 Over 3 Supports
R=441 U=136 W=6
RL=175/-73

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

QTY: 35 FL/-5/-/R/- Scale = .5"/Ft.

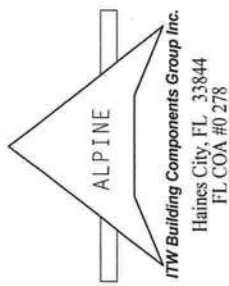
TC LL	20.0 PSF	REF	R8228 - 43286
TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCUSR8228 09279032
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	46340
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1TV08228Z02



****WARNING**** TROSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH EL STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AFCA (AMERICAN FABRICATING COMPANY, 10000 WILSON AVENUE, SUITE 100, WILSON, CA 94093). THESE TROSSES ARE DESIGNED TO PERFORM THESE FUNCTIONS UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BEG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ITM BEG CORRELATOR PLATES ARE MADE OF 20/18/16GA (4-H/55/K) ASTM A653 GRADE 40/60 (4, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Wave

(9-136--Doug Edgley Tuwuana Rossin -- , ** - J1A)

Top chord 2x4 SP #2 Dense.
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.

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Deflection meets L/240 live and L/180 total load.

Wind reactions based on MMFRS pressures.

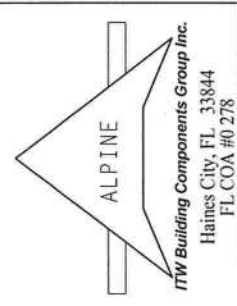
Deflection meets L/240 live and L/180 total load.

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 2100 W. 10TH AVE., SUITE 100, ALBUQUERQUE, NM 87105-4000, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURN IN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. BY AISC AND TPI. THE REG. INCORPORATES PLATES ARE MADE OF 20/18/16GA (W, H/SS/S) ASTM A653 GRADE 40/60 (W, H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



QTY: 4	FL / - / 5 / - / - / R / -	Scale = .5" / Ft.
TC LL	20.0 PSF	REF R8228 - 43287
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279029
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN - 46348
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TV08228Z02



1-1-10-0
1-1-1 Over 3 Supports
R-317 U=114 W=6"
RL=57/-46

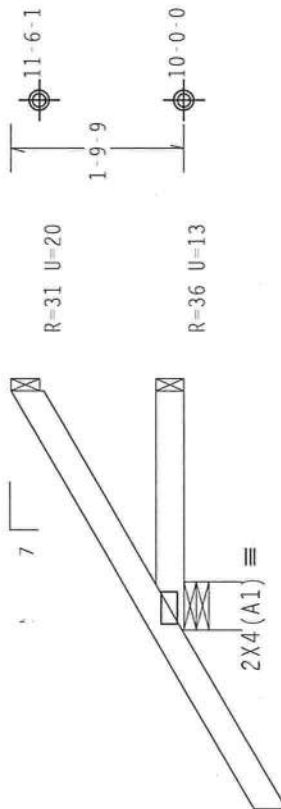
R=-78 Rw=55 U=81

R-3 Rw=25 U=28

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



$\leftarrow 1-10-0 \rightarrow$
 $\leftarrow 2.5-9 \text{ Over } 3 \text{ Supports} \rightarrow$
 $R=290 \quad U=75 \quad W=6''$
 $RL=85/-52$

Design Crit: FBC2007Res/TPI-2002(STD)
 $FT/RT=10\%(0\%)/0(0)$

PLT TYP. Wave

9.02.00

QTY: 8

Scale = .5"/Ft.

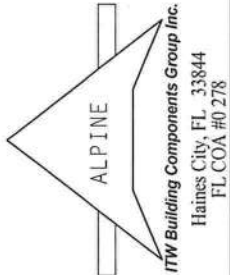
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TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279025
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 46356
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02

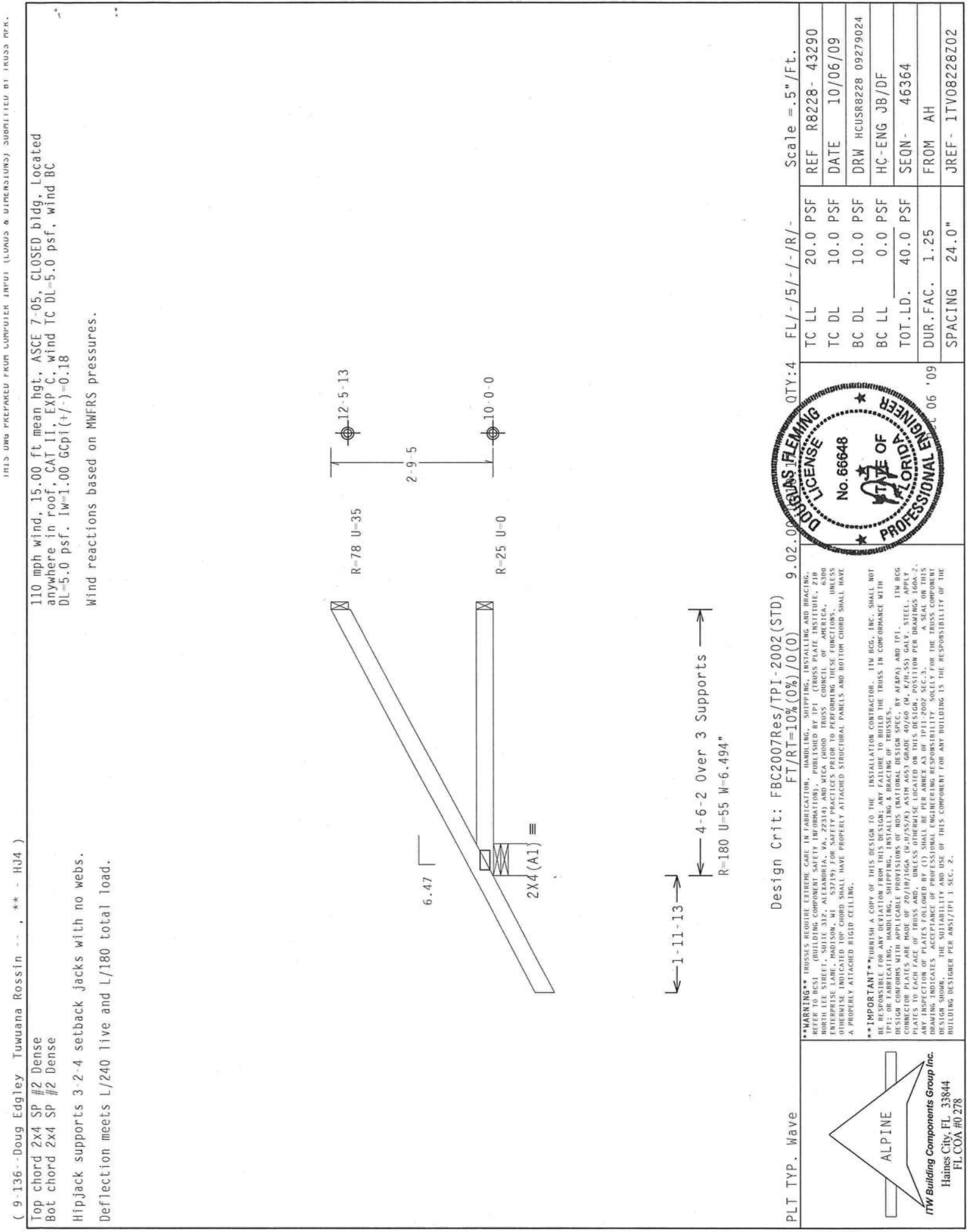


****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR-TTW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY INVITATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SPECIFICATIONS WILL BE AT THE USER'S RISK.

DESIGN COMPONENTS ARE AVAILABLE PRODUCTIONS OF WDS (NATIONAL DESIGN SPEC., K/F/H) AND TPI. 11W BCG CONNECTOR PLATES WITH APPLICABLE DIMENSIONS ASH A65-3 GRADE 40/46 (C, A7/A8-55) GALV. STEEL, APPLIED TO PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. A SEAL ON THIS INSPECTION OF PLATES FOLLOWED BY 1) SHALL BE PER APPROX AS OF 1711 2002 SEC.3. A SEAL ON THIS DESIGN SHOWS THE QUALITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASH A65-3 2 SEC. 2.





THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS PERK.

(9-136--Doug Edgley Tuwuana Rossin -- , ** - HJ4)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Hipjack supports 3-2-4 setback jacks with no webs.
Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18
Wind reactions based on MMFRS pressures.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP., Wave

Scale = .5" / Ft.

QTY: 4	FL / - / 5 / - / - / R / -	TC LL	20.0 PSF	REF	R8228 - 43290
		TC DL	10.0 PSF	DATE	10/06/09
		BC DL	10.0 PSF	DRW	HCUSR8228 09279024
		BC LL	0.0 PSF	HC-ENG	JB/DF
		TOT.LD.	40.0 PSF	SEQN	46364
		DUR.FAC.	1.25	FROM	AH
		SPACING	24.0"	JREF	1TV08228Z02

ALPINE
Haines City, FL 33844
FL COA #0278

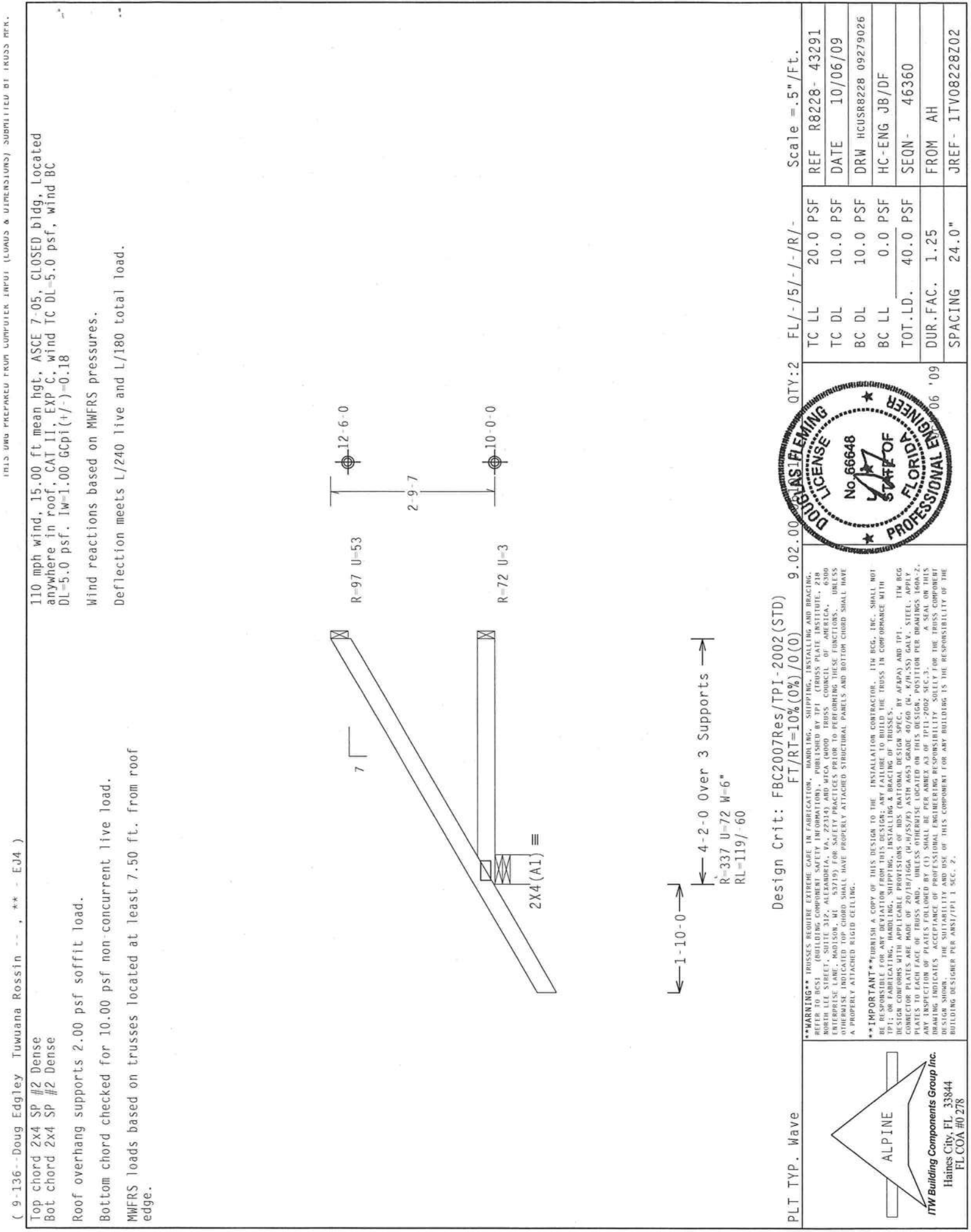
ITW Building Components Group Inc.

Professional Engineer
No. 66648
STATE OF FLORIDA

DOUGLAS FLEMING
LICENSE

06 '09

IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TPI. ITW BCG CORRELATES PLATES ARE MADE OF 20/18/16GA (U-1755/A) ASTM A653 GRADE 40/60 (4, K/H-SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-2. ANY RESPECTION OF PLATES FOLLOWED BY (C) SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUSS. CONFORMIT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



THIS WAS PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY IKUSS MEK.

(9-136--Doug Edgley Tuwana Rossin --, ** - EJ4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MWFRS pressures.
Deflection meets L/240 live and L/180 total load.

PLT TYP. Wave
Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10%(0%)/0(0)

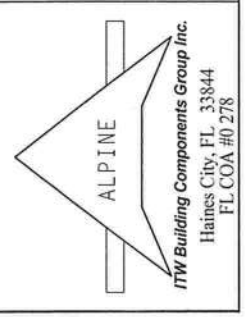
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TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279026
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 46360
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TV08228Z02

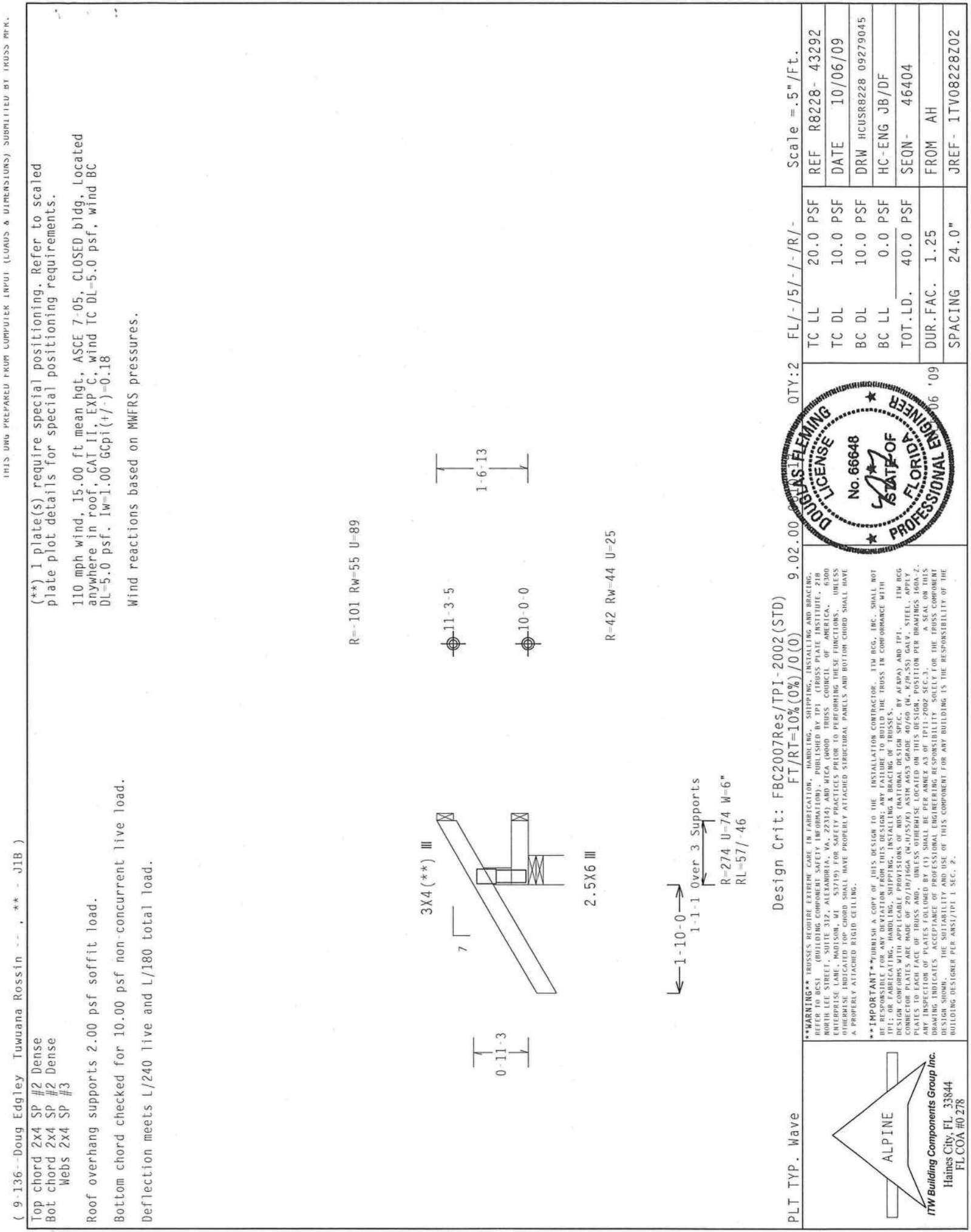


****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DESIGN (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2100 W. 10TH AVE., SUITE 100, GAINESVILLE, FL 32609, FOR ALL INFORMATION CONCERNING TRUSS SAFETY. ALWAYS FOLLOW THE DESIGNER'S RECOMMENDATIONS FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC., BY AIA/ASA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/1666A (4-H/55/PS) ASTM A653 GRADE 40/60 (4, 8/H-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT DESIGNER, THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.





Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Laterally brace chord above/below filler @ 24" O.C.
(or as designed) including a brace on chord directly
above/below both ends of filler (if no rigid diaphragm
exists at that point)

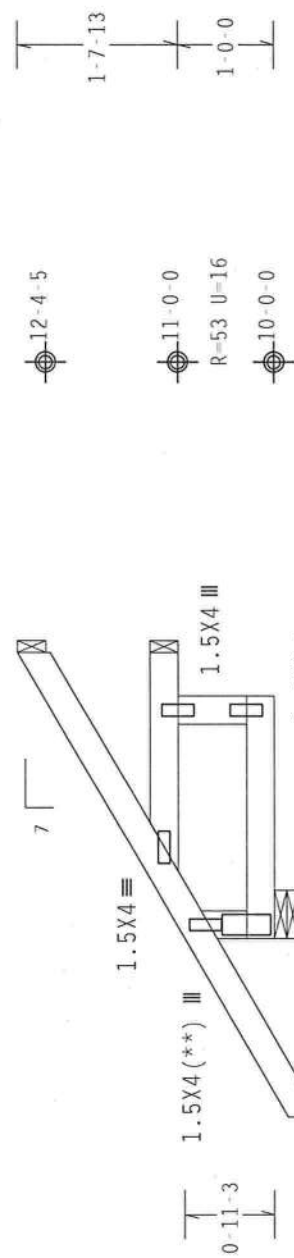
(**) 1 plate(s) require special positioning. Refer to scaled
plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg. located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Fasten rated sheathing to one face of this frame.

R-58 U-34



0-8-5 1-9-11 0-5-6
12-11-6 Over 3 Supports

R-285 U-51 W=6"
RL-94/-54

Design Crit: FBC2007Res/TPI-2002 (STD)
FT/RT=10% (0%) / 0 (0)

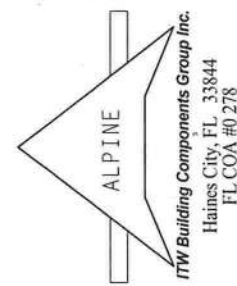
QTY: 2 FL/-/5/-/-/R/- Scale = .5" / Ft.

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TC DL	10.0 PSF	DATE	10/06/09
BC DL	10.0 PSF	DRW	HCUSR8228 09279051
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	46414
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1TV08228Z02



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURN IN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING FROM THE TRUSS IN COMPLIANCE WITH TPI'S OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG DESIGN CODES WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC, BY AISC) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (4-11/8X5/8) ASTM A653 GRADE 40/60 (4, 8/11.55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

() 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.**

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

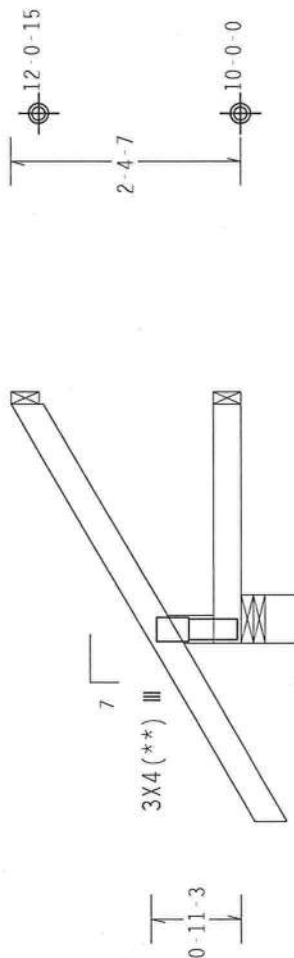
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

R=19 RW=24 U=26

R=54 U=13



← 1-10-0 →
2.5-9 Over 3 Supports
R=273 U=51 W=6"
RL=85 / 52

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

QTY: 4 FL / - / 5 / - / - / R / -
Scale = .5" / Ft.

WARNING-TRUSSES BEARING EXTERIOR GORE IN FABRICATION. MARDLING, SHIPPING, INSTALLING AND BRACING. TRUSS COMPANY, COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CRUSS PAIL INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314, AND MICA, 4000 TRUSS, COURCEL OF AMERICA, 6300 ENTERPRISE LANE, WADSWORTH, MI 48093. 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BRIDGE THE TRUSS IN CONFORMANCE WITH THE MARDLING, SHIPPING, INSTALLING AND BRACING INSTRUCTIONS.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCG'S (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

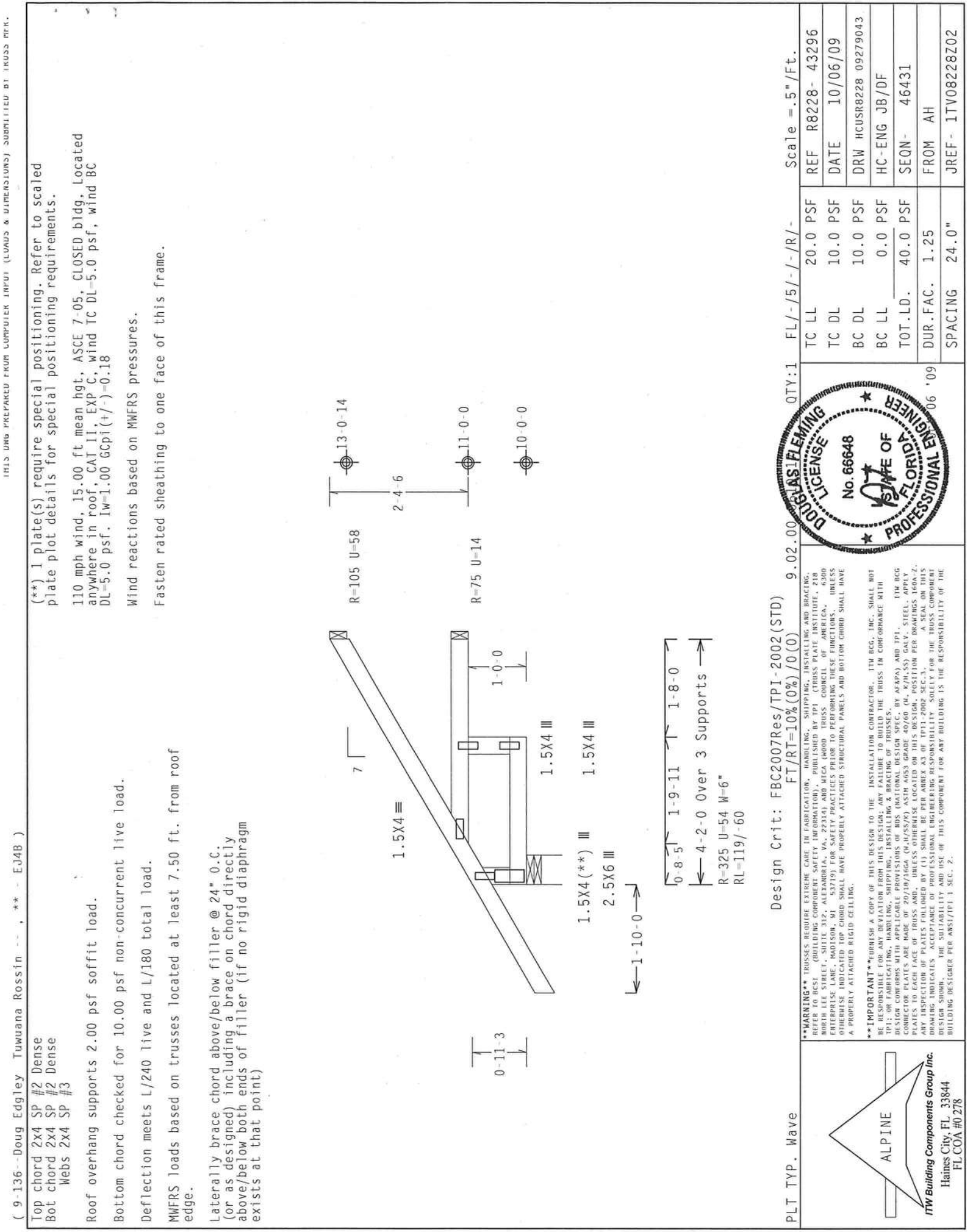
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI (TPI) OR FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALL BCG CONNECTION PLATES ARE MADE OF 2010/1816GA (0.1875") ASTM A653 GRADE 50. 11" (W-55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIMENSIONS 100A-2, 100B-2 AND 100C-2. (SEE DIMENSIONAL DRAWING 100A-2, 100B-2 AND 100C-2 FOR TRUSS COMPONENT DESIGN SHOWING THE SUBMITTABLES FOR THE TRUSS COMPONENT DESIGNER'S USE.)

USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER SUBMITTAL 1, SEC. 2.



TC LL	20.0 PSF	REF R8228- 43294
TC DL	10.0 PSF	DATE 10/06/09
BC DL	10.0 PSF	DRW HCUSR8228 09279042
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN - 46421
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF - 1TV08228Z02



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS PERK.

(9-136--Doug Edgley Tuwana Rossin --, ** - EJ4B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.

Laterally brace chord above/below filler @ 24" O.C.
(or as designed) including a brace on chord directly
above/below both ends of filler (if no rigid diaphragm
exists at that point)

(**) 1 plate(s) require special positioning. Refer to scaled
plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg. Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC
DL=5.0 psf. IW=1.00 GCpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Fasten rated sheathing to one face of this frame.

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0)

Scale = .5" / Ft.

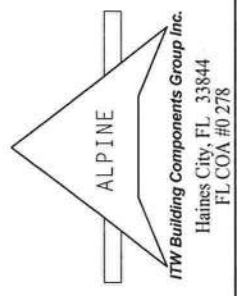
QTY:1	FL/-/5/-/-/R/-	REF R8228- 43296
TC LL	20.0 PSF	DATE 10/06/09
TC DL	10.0 PSF	DRW HCUSR8228 09279043
BC DL	10.0 PSF	HC-ENG JB/DF
BC LL	0.0 PSF	SEQN- 46431
TOT.LD.	40.0 PSF	FROM AH
DUR.FAC.	1.25	JREF- 1TV08228Z02
SPACING	24.0"	



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI, TRUSS PLATE INSTITUTE, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND MICA 14000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI, 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

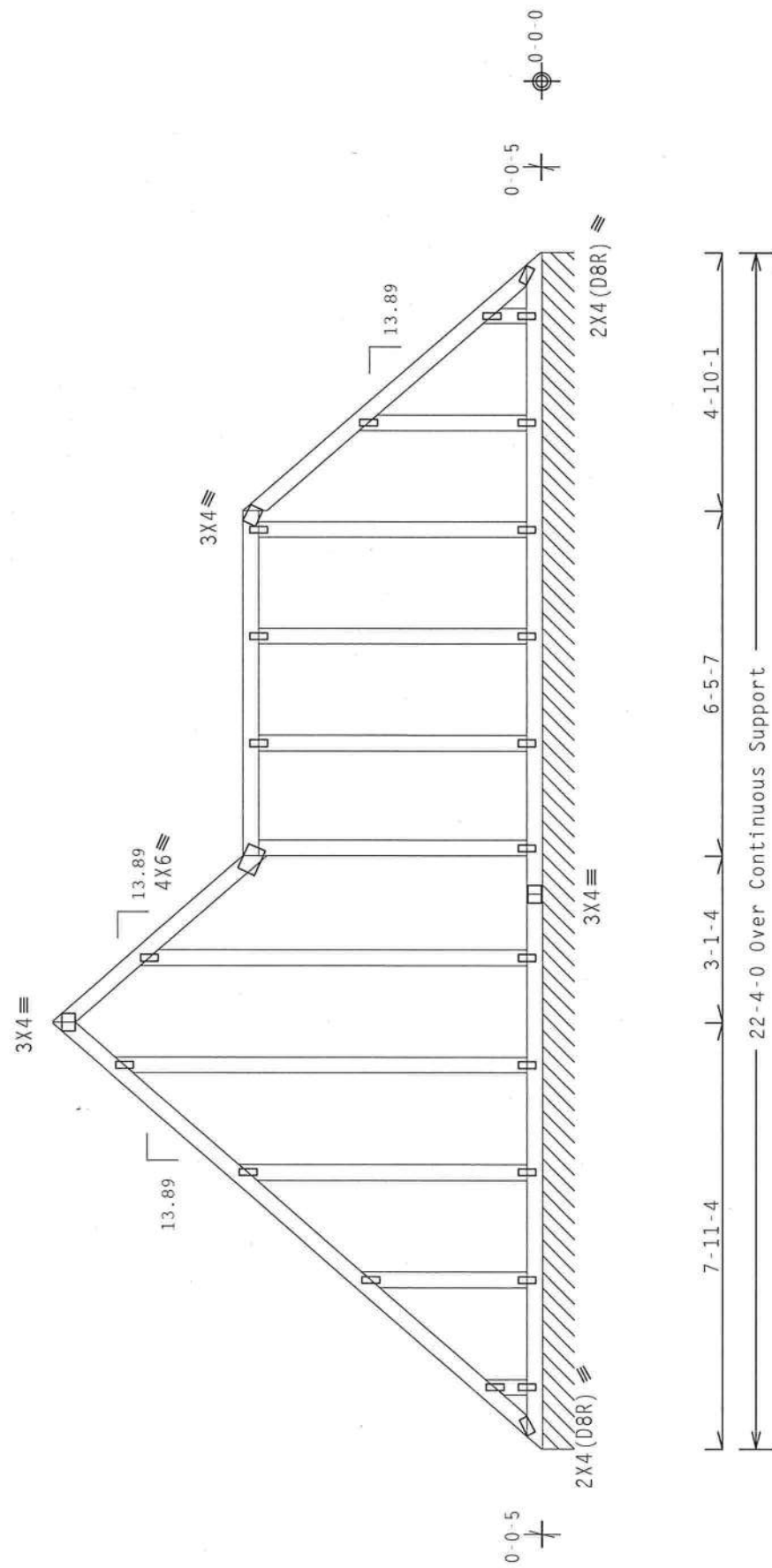
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF RDS (NATIONAL DESIGN SPEC. BY AIA/ASCE) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 2017/16GA (4.4/55/54) ASTM A653 GRADE 40/60 (4.4/55/54) GALV. STEEL. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS T00N-2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



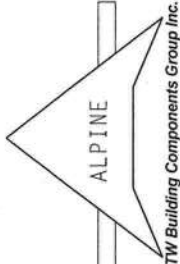
PLT TYP. Wave

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.

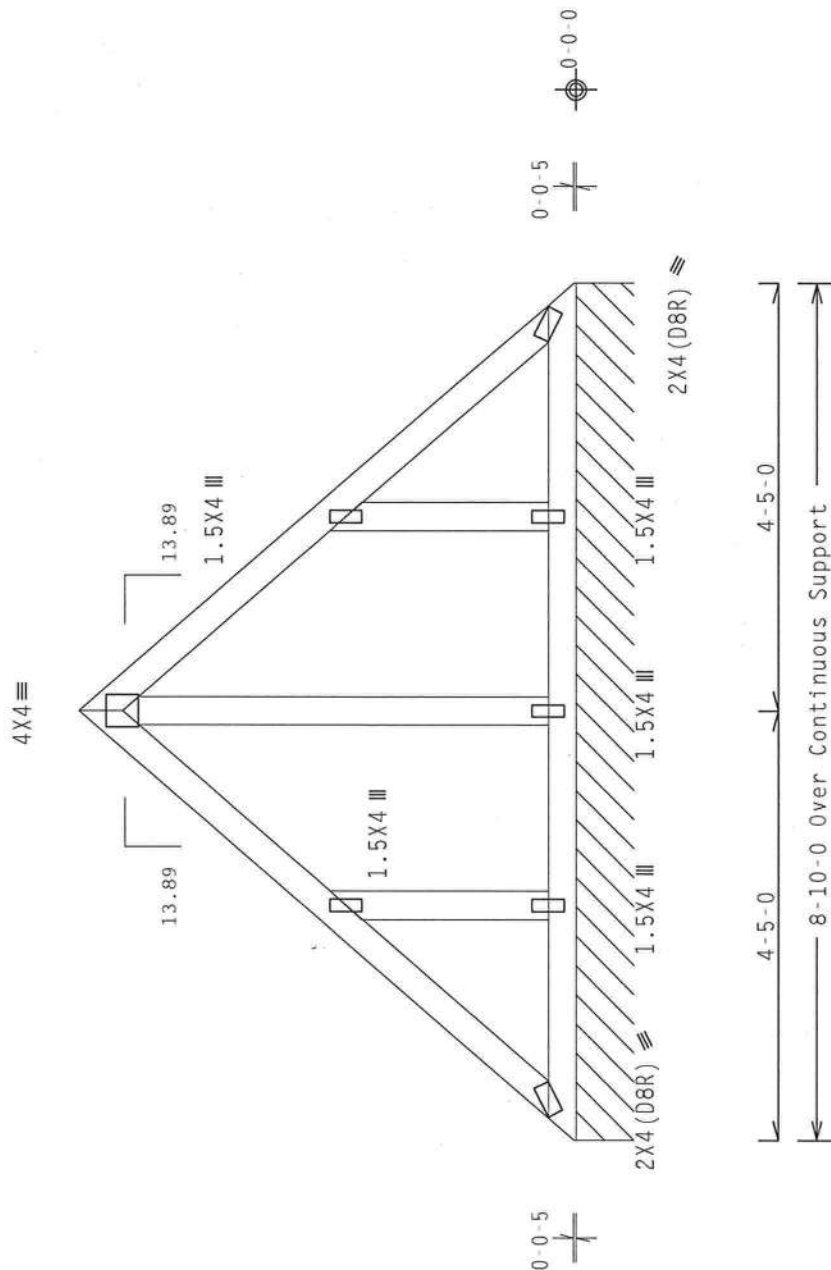


Note: All Plates Are 1.5X4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0) 9.02.00

	Scale = .3125" / Ft.	
	REF	R8228 - 43297
	DATE	10/06/09
	DRW	HCUSR8228 09279047
	HC-ENG	JB/DF
	SEQN	47407
 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	TC LL	20.0 PSF
	TC DL	10.0 PSF
	BC DL	10.0 PSF
	BC LL	0.0 PSF
	TOT.LD.	40.0 PSF
	DUR.FAC.	1.25
	SPACING	24.0"
FROM AH		JREF - 1TV08228Z02

Top	chord	2x4	SP	#2	Dense
30t	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME. SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.

Design Crit: FBC2007Res/TPI - 2002 (STD)
 $FT/RT = 10\% (0\%) / 0 (0)$

PLT TYP. Wave

9.02.00

OTY:1

Scale = .5"/Ft.

TC LL	20.0	PSF	REF R8228- 43298
TC DL	10.0	PSF	DATE 10/06/09
BC DL	10.0	PSF	DRW HCURSR8228 09279003
BC LL	0.0	PSF	HC-ENG JB/DF
TOT.LD.	40.0	PSF	SEQN- 47410
DUR.FAC.	1.25		FROM AH
SPACING	24.0"		JREF- 1TV08228Z02



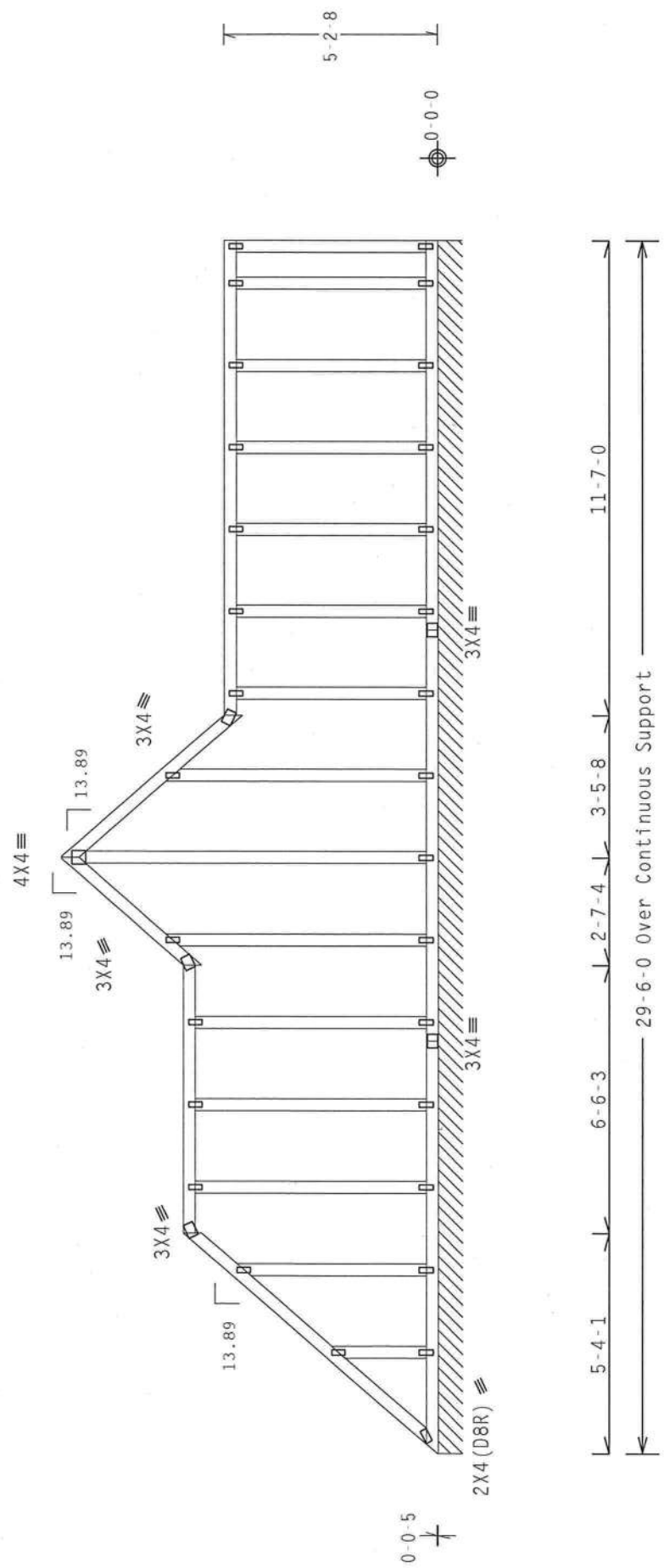
WARNING THUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST AVAILABLE SAFETY INFORMATION. PUBLISHED BY THE CONCRETE PUMPING INSTITUTE, 2718 WILLOW LANE, SUITE 312, ALEXANDRIA, VA 22304 AND HCA 40000 PERFORM THESE ACTIONS CAREFULLY. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID JOINTS.

ALPINE

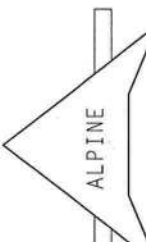
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

NOTE: THIS DESIGN IS NOT TO BE USED AS A ROOF TRUSS. IT IS ONLY TO BE USED AS A ROOF HIP FRAME.
SEE DETAIL HIPFRAME0109 FOR MORE INFORMATION.



Note: All Plates Are 1.5x4 Except As Shown.
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=10%(0%)/0(0) 9.02.0

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	PLT TYP. Wave		QTY: 1		FL / - / 5 / - / R / -		Scale = .25" / Ft.	
	TC LL		20.0 PSF		REF		R8228 - 43299	
	TC DL		10.0 PSF		DATE		10/06/09	
	BC DL		10.0 PSF		DRW		HCUSR8228 09279028	
	BC LL		0.0 PSF		HC-ENG		JB/DF	
	TOT.LD.		40.0 PSF		SEQN		47388	
DUR.FAC.		1.25		FROM		AH		
SPACING		24.0"		JREF		1TV08228Z02		

design crit. FB2007RES/TP1-2002 (00)

FT/RT=10%(0%)70(0)

9.02.00

QTY: 1

FL / - / 5 / - / R / -

Scale = .25" / Ft.

TC LL 20.0 PSF REF R8228 - 43299

TC DL 10.0 PSF DATE 10/06/09

BC DL 10.0 PSF DRW HCUSR8228 09279028

BC LL 0.0 PSF HC-ENG JB/DF

TOT.LD. 40.0 PSF SEQN - 47388

DUR.FAC. 1.25 FROM AH

SPACING 24.0" JREF - 1TV08228Z02

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO HCS1 (BUILDING COMPONENT SOCIETY INFORMATION), PUBLISHED BY TP1 (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MIGA (GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR DIVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN REQUIREMENTS OF THIS DRAWING SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ITW BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASA AND TP1. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/J/S/S/E/K) ASTM A563 GRADE 40/60 (W, E/H/S/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/A3 OF TP1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIA/TP1 1 SEC. 2.

DRG 15 FLEMING

LICENSE

No. 66648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

06 '09



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SOCIETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2108 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 FARMERS BUILDING, CHICAGO, IL 60606) FOR PROPER BRACING AND CONNECTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 2018/166A (W-17/SS/RS) A572 A553 GRADE 40/60 (W, F, H, S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AISC A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/TPI 1 SEC. 2.

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



TWM
Building Components Group Inc.

Earth City, MO 63045

..WARNING.. READ AND FOLLOW ALL NOTES ON THIS SHEET

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET:**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow HCSI (Building Component Safety Institute), by TPI and WTC) for safety practices prior to performing functions. Trusses are designed for use in a temporary condition. Trusses shall have properly attached girders. Locations shown for permanent lateral restraint of webs shall have bracing installed per HCSI sections B3 & B7. See this job's general notes page for more information.

••IMPORTANT•• FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

Building Components Group Inc. (TWBCG) shall not be responsible for any deviation from this design, and shall not be responsible for any failure to build the truss in conformance with TPI, or fabricating, shipping, installing & bracing of trusses. TWBCG connector plates are made of 2018/16GA (WH/S/K) ASTM A653 grade 37/40/90 (K/W/H) galv. steel. Apply plates at each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

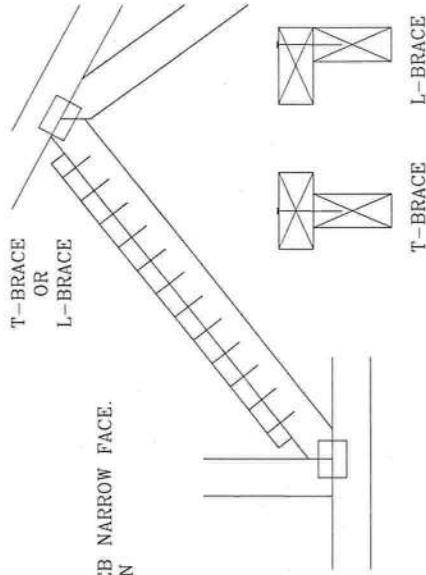
responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCC: www.itwben.com; TPI: www.tinst.com; WTC: www.shel

responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCC: www.itwbcc.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3", MIN) NAILS.

BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

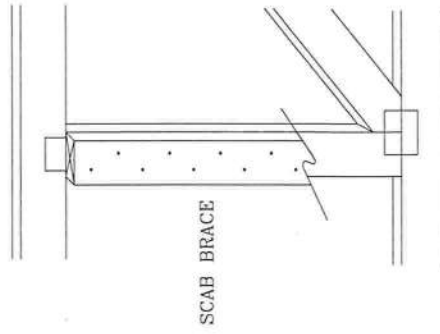
APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.

NO MORE THAN (1) SCAB PER
ATTACH WITH 10d BOX OR GUN

(0.128" x 3" MIN) NAILS.

AT 6" O.C.

BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



SCAB BRACE

TC LL	REF	CLB SUBST.
PSF		

TC DL	PSF	DATE	1/1/09
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BC DL	PSF	DRWG	BRCCLBSUB0109
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BC 1J. PSF

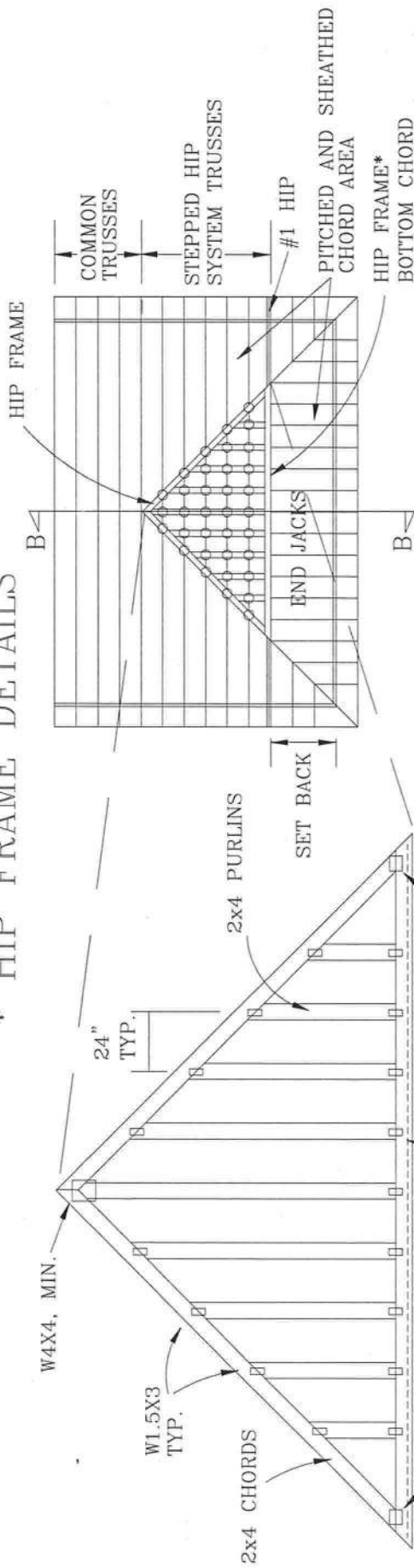
DC	mm	PSF
TOT	ID	

TOT. LD.
DUR. FAC.

SPACING



* HIP FRAME DETAILS



o- ATTACH HIP FRAME TO FLAT CHORDS OF STEPPED HIP AT ALL OVERLAPPING POINTS WITH 2-10d (0.148"x3") COMMON NAILS. BOTTOM CHORD OF HIP FRAME TO BE ATTACHED TO #1 HIP WITH 10d COMMON (0.148"x3") NAILS @ 6" O.C. MAXIMUM SPACING.

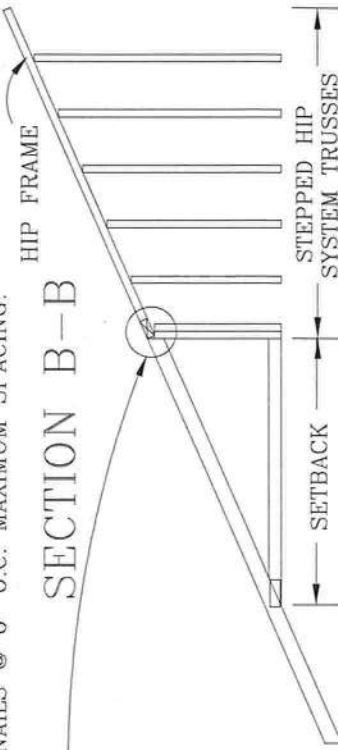
HIP FRAME STOPS AT PLUMB CUT OF JACKS TO MAINTAIN PITCH CONTINUITY.

USE THIS DETAIL FOR ASCE 7-02 OR ASCE 7-05 WIND, 110 MPH, 30' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=4.2 PSF, Kzt=1.00

* HIP FRAME LUMBER IS SPF, SO. PINE, HF, OR DFL STANDARD, STUD GRADE, OR BETTER.

SEE ENGINEER'S SEALED DESIGN FOR SETBACK, LUMBER, PLATING, LOADING AND DURATION FACTOR REQUIRED.

'R' HIP FRAME CHORDS MAY BE TRIMMED UP TO 2" TO FIT. PURLINS MUST BE INTACT AND PROPERLY ATTACHED.



HIP FRAME - PROVIDED BY TRUSS MANUFACTURER. HIP FRAME IS DESIGNED TO PROVIDE BRACING FOR FLAT TOP CHORDS OF HIP FRAME SYSTEM WHERE INDICATED. STRUCTURAL PANELS MUST BE PROPERLY ATTACHED DIRECTLY TO HIP FRAME PURLINS.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow ITW Building Components Safety Information, by TPI and WCA for safety practices prior to performing installation. Trusses shall be properly attached to structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per ECSI sections B3 & B7. See this job's general notes page for more information.

****IMPORTANT**** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall be responsible for any deviation from this design. Trusses shall be installed in accordance with TPI's instructions for installing & bracing of trusses. ITWBCG connector plates are made of 2019/16GA (W/H/S/K) ASTM A563 grade 37/40/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. ITW-BCC: www.itwbcg.com; TPI: www.tpiusa.com; WCA: www.structure.com; ICC: www.iccsafe.org



Earth City, MO 63045

REF	HIP FRAME
DATE	1/1/09
DRWG	HIPFRAME0109

