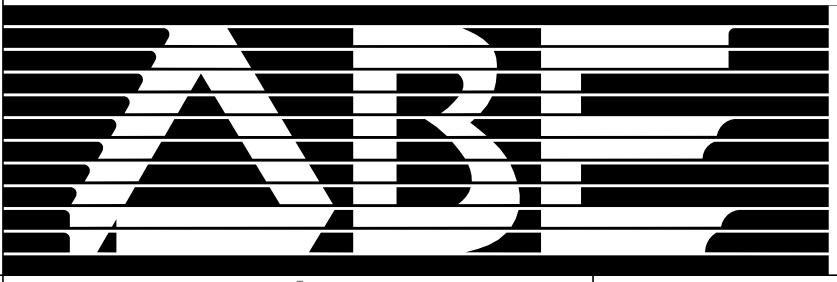
COLUMBIA COUNTY I-10 REST AREA 02/09/21



SHEET LIST	
HEET NUMBERS	
CV	COVER SHEET
SP-1	SURVEY
SP-2	SITE PLAN
A-1	PROPOSED FLOOR PLAN
A-2	ELEVATION
A-3	ELEVATION
S-0	STRUCTURAL NOTES
S-1	FOUNDATION FRAMING PLAN
S-2	STRUCTURAL PLAN
S-3	ROOF FRAMING PLAN
S-4	STRUCTURAL SECTIONS
S-5	STRUCTURAL SECTIONS

# ABE BORUJERDI, P.E. 19.020



# AMERICAN BUILDING ENGINEERS

ARCHITECTURE - ENGINEERING - INTERIOR DESIGN - GENERAL CONTRACTOR LICENSES # AA26003224, #PE38553, # IB26001734, CGC #044407 1842 E. OAKLAND PARK BLVD. - FORT LAUDERDALE, FL - 33306 (305) 681-5563 FAX (954) 739-5153

Email: info@abeincusa.com www.abeincusa.com

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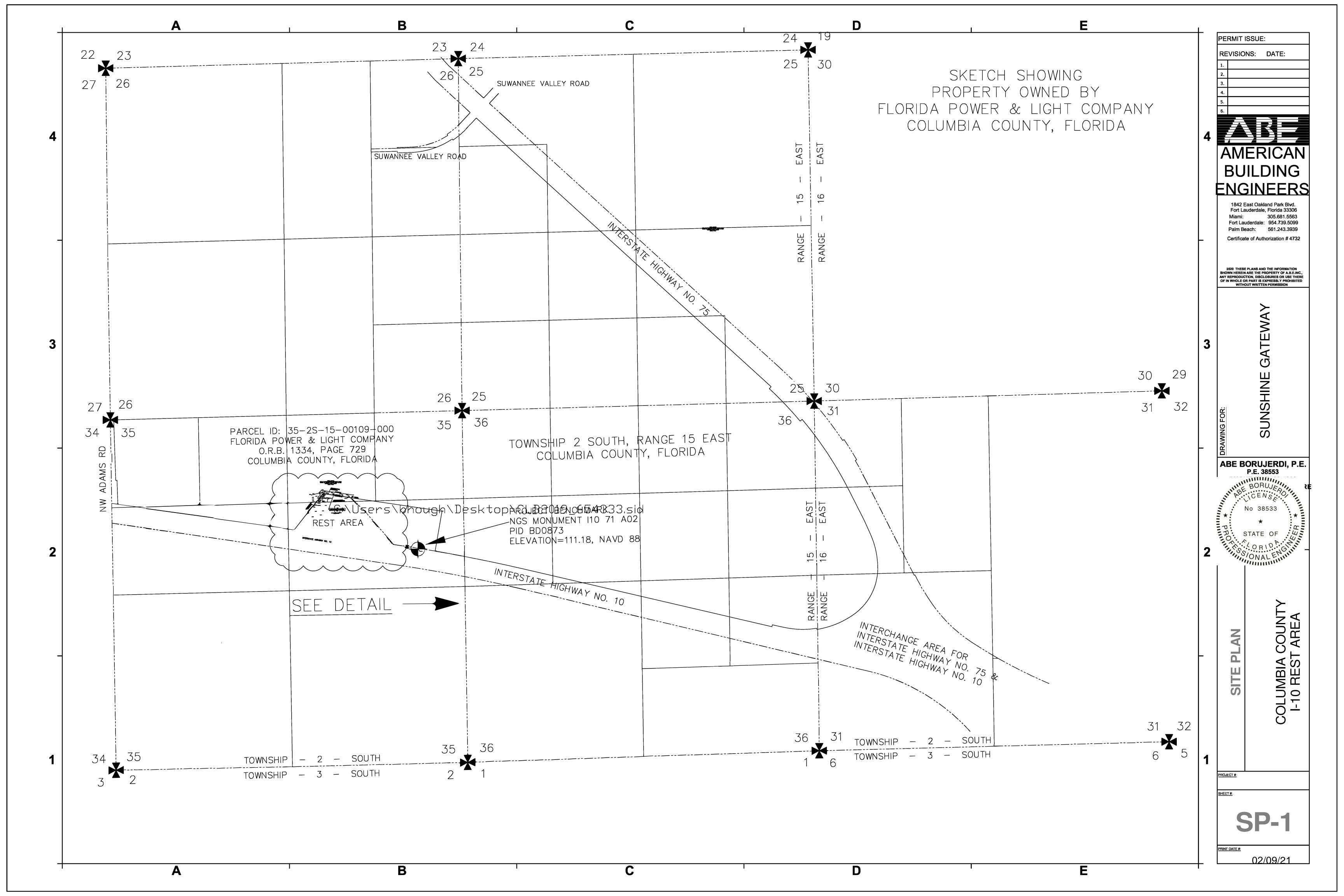
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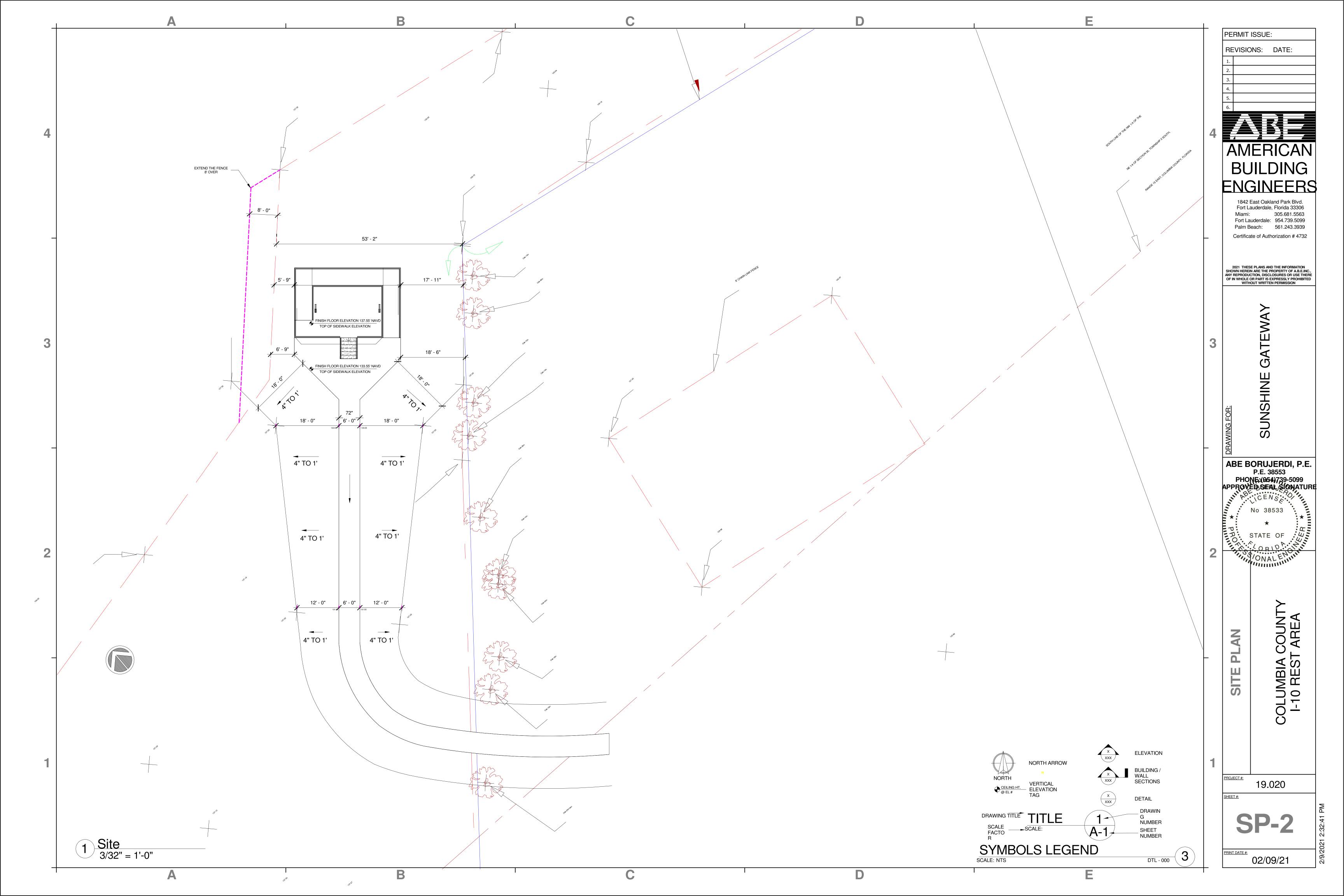
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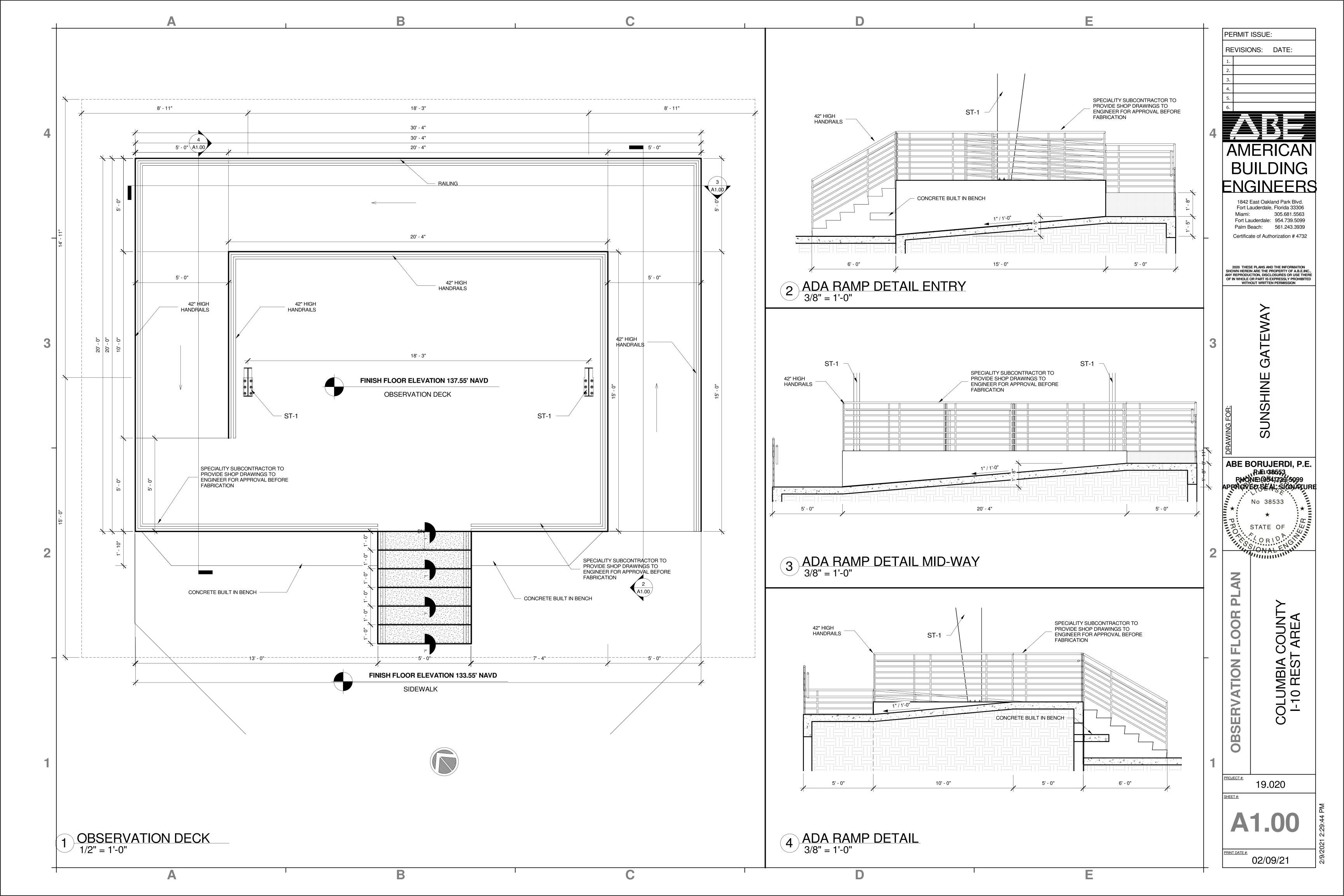
BUILDING **ENGINEERS** 

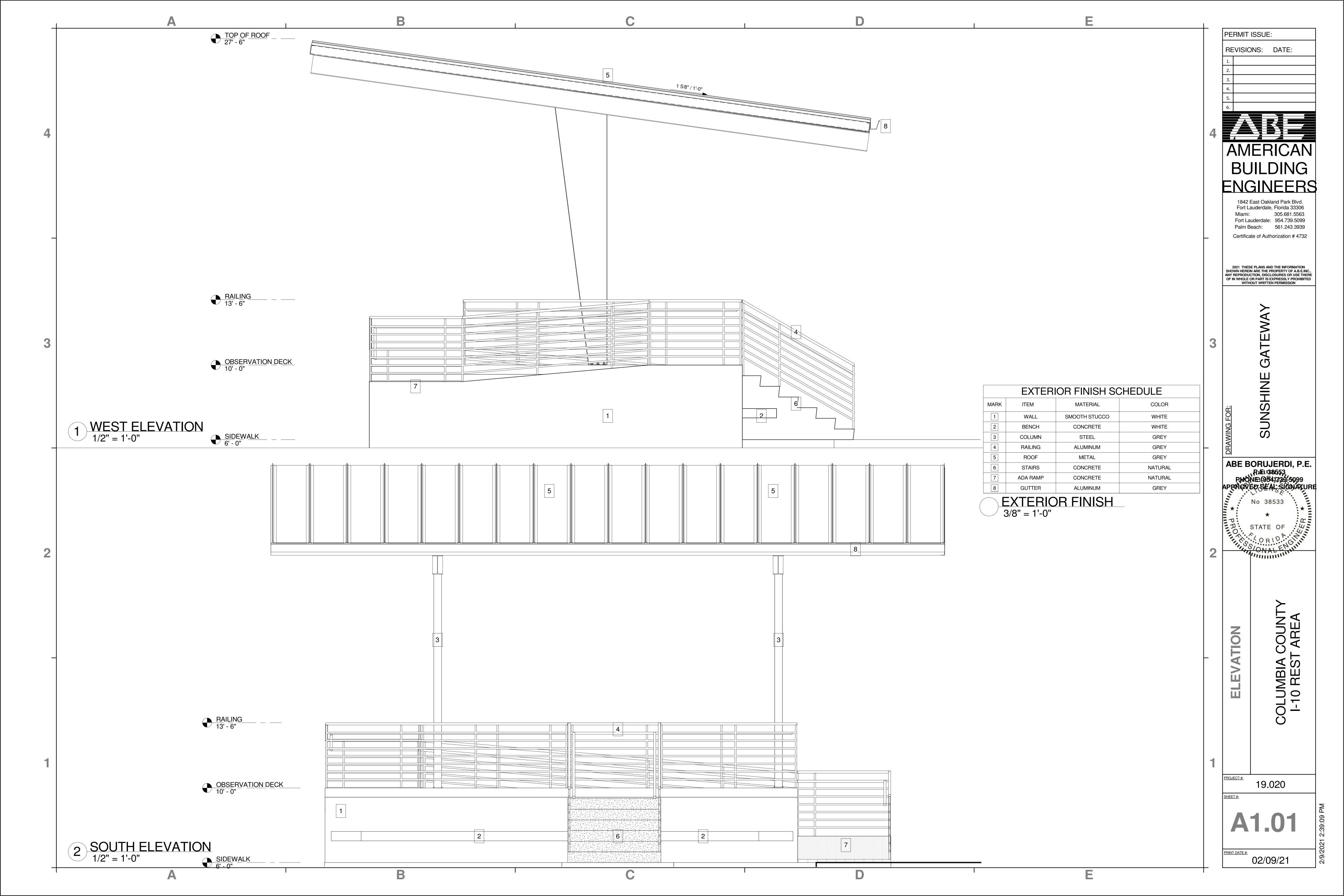
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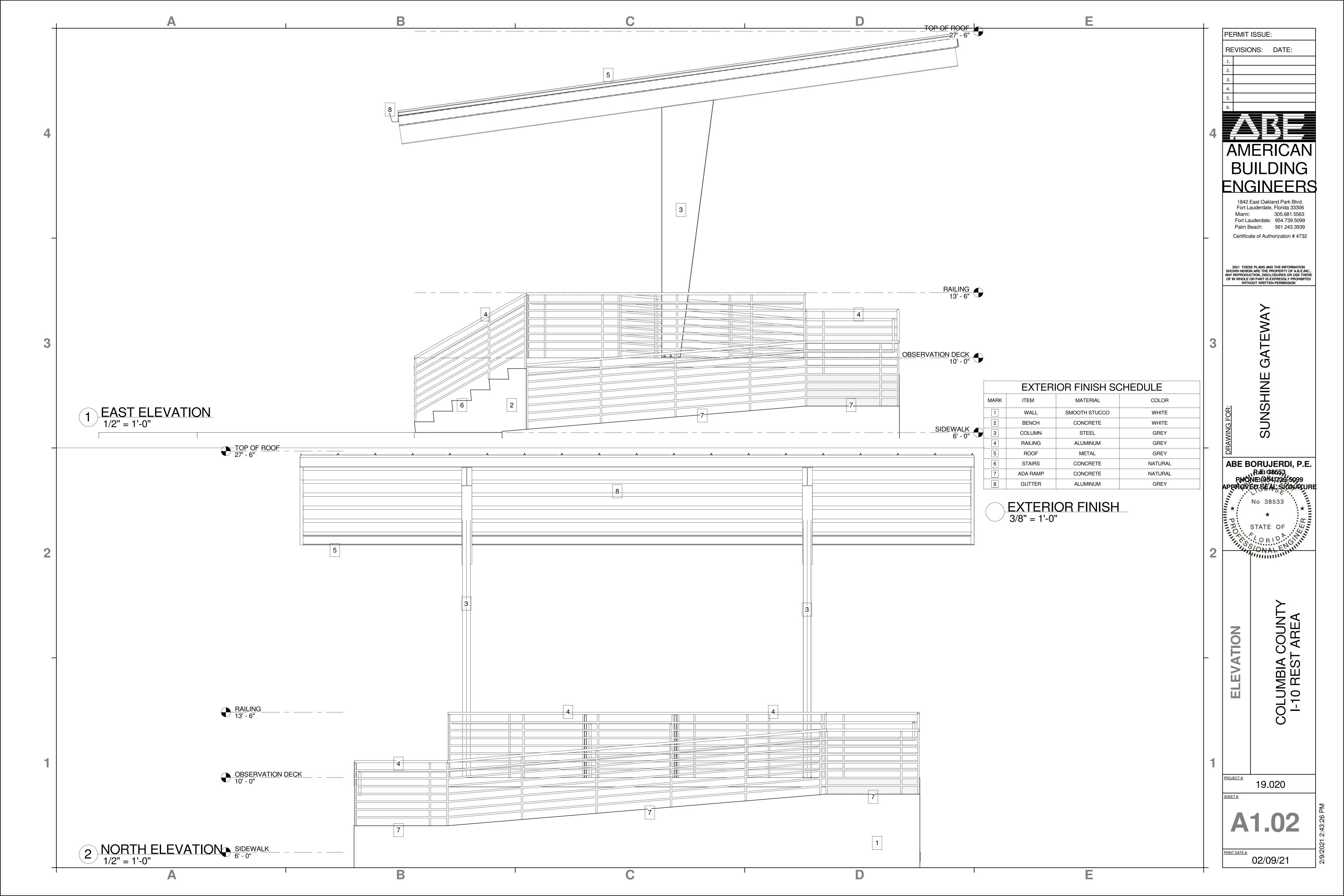
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### **GENERAL:**

ALL INFORMATION THAT IS STRUCTURAL IN NATURE SHOWN IN THESE STRUCTURAL SHEETS SHALL TAKE PRECEDENCE OVER STRUCTURAL ITEMS IN OTHER TRADES PLANS.

EXCEPTION: NOTES IN OTHER TRADES PLANS ADDRESSING SPECIAL ITEMS (SUCH AS MINIMUM CONCRETE STRENGTH FOR FIRE RATING) SHALL GOVERN WHEN MORE STRINGENT THAN HERE SHOWN.

### PROJECT SPECIFIC CODES USED:

ALL STRUCTURAL ELEMENTS FOR THIS PROJECT HAVE BEEN DESIGNED PER F.B.C. 2017. THIS IMPLIES THE USE OF THE BELOW LISTED CODES (FBC TAKES PRECEDENCE OVER CONFLICTING ITEMS.)

### **REQUEST FOR INFORMATION:**

PLEASE USE HAND SKETCHES AND OTHER VISUAL / WRITTEN METHODS TO COMMUNICATE A CONSTRUCTION QUESTION. SEND TO US VIA EMAIL.

### **CONTRACTING NOTES:**

- THE CONTRACTOR SHALL INCLUDE IN HIS INITIAL BID 1% ADDITIONAL (BASED ON THE TOTAL AMOUNT OF THE SHELL BID) AS AN ALLOWANCE FOR ADDITIONAL REINFORCEMENT. CONCRETE, WOOD, STRAPPING, FORMWORK, STRUCTURAL STEEL PARTS, WELDING, CONNECTORS. SAWCUTTING OR GRINDING CONCRETE AND ITS ASSOCIATED LABOR SAID ALLOWANCE SHALL BE USED FOR REWORKS REQUIRED BY THE INSPECTOR OR ENGINEER IN ORDER TO ENSURE THE INTERGRITY OF SHELL AND CORRECT ANY MISINTERPRATIONS OR INCONSISTENCES IN THE PLANVIEWS, NOTES AND DETAILS.
- GENERAL CONTRACTOR MUST FAMILIARIZE HIM OR HER SELF WITH THE STRUCTURAL PLANS AND HOW THEY INTERACT WITH OTHER SPECIALTIES PLANS (ARCHITECTURAL, ETC.) ALTHOUGH TRADE COORDINATION HAS BEEN PERFORMED, THIS OFFICE SHALL BE CONTACTED IN CASE THE CONTRACTOR FINDS MISSING, AMBIGUOUS, UNCLEAR OR CONFLICTING INFORMATION.
- THE CONTRACTOR SHALL ALWAYS KEEP ON SITE THE LATEST REVISION OF THE SIGNED AND SEALED STRUCTURAL PLANS, APPROVED SHOP DRAWINGS AND PRODUCT APPROVALS.
- GENERAL CONTRACTOR IS SOLEY RESPONSIBLE FOR ALL ACTIVITIES RELATED TO WORK SAFETY.
- CONSTRUCTION LOADS SHOULD BE CAREFULLY DISTRIBUTED TO AVOID LOADING ANY MEMBERS IN EXCESS OF THEIR CAPACITY.
- THE APPROVAL BY THE STRUCTURAL INSPECTOR OF ANY WORK DOES NOT RELIEVE THE CONTRACTOR FOR COMPLYING WITH THE FLORIDA BUILDING CODE AND THE APPROVED STRUCTURAL PLANS.

# PROJECT PLANS, DETAILS & ARCHITECTURAL BACKGROUNDS:

ALL DETAILS AND SECTIONS SHOWN ARE INTENDED TO BE TYPICAL UNLESS OTHERWISE SPECIFIED. DETAILS THAT REFERENCE SPECIFIC SCHEDULES ARE INTENDED TO BE SCHEDULES SHALL TAKE PRECEDENCE IN ALL CONFLICTING ITEMS. ALL ELEMENTS INTENDED TO ORIGINATE FROM OTHER TRADE'S DRAWINGS (ELEVATIONS, PARTITION LOCATIONS, ETC) HAVE BEEN INCLUDED FOR THEIR RELATION TO STRUCTURAL ITEMS ONLY. REFER TO THE CORRESPONDING TRADE'S PLANS FOR FINAL LAYOUTS AND DETAILS. DO NOT USA A SCALE TO DETERMINE THE DIMENSIONS OF ANY PLAN OR DETAIL DRAWINGS. IF THE DIMENSION NEEDED IS UNAVAILABLE, PLEASE CONTACT THIS OFFICE.

GRAVITY LOADING (ASCE 7-10, CHAPTER, TABLE C3-1 & C3-2):

SEE GRAVITY LOADING CHART FOR DESIGN LOADS.

REQUEST ACTUAL PRODUCT CUT SHEETS FROM SUPPLIERS FOR ANY EQUIPMENT HEAVIER THAN 200 LB.

### **DEEP FOUNDATION - SEE SOILS REPORT**

A SOILS REPORT IS AVAILABLE TO THE STRUCTURAL ENGINEER. THE DESIGN OF THE FOUNDATION AND MUST BE DESIGNED FOR BY THE SOILS ENGINEERS. AT THE TIME OF CONSTRUCTION, A FLORIDA REGISTERED SOILS ENGINEER SHALL BE HIRED BY THE OWNER TO INVESTIGATE THE SOIL STRATA BY MEANS OF A SIGNED AND SEALED REPORT BASED ON A SOILS BORING TEST.

### **GRADE BEAMS:**

LOWER REBAR CAGE 1 1/2" ON GRADE BEAMS UNDER EXTERIOR WALLS TO ALLOW FOR MINIMUM CONCRETE COVERAGE OF 1.5" UNDER DOOR RECESS (SEE BEAM RECESS & CONCRETE COVER DETAIL PROVIDED). E.E.: STIRRUPS SHOULD BE PLACED ON EACH END OF EACH INTERMEDIATE SUPPORT. BALANCE REFERS TO THE SPACING OF THE REMAINING TIES AFTER THE TIES AT EACH END HAVE BEEN INSTALLED. IF NO END TIES SHOWN, USE BALANCE THROUGHOUT THE ENTIRE LENGTH OF THE BEAM.

### CONCRETE STRENGTH (ACI 318-14, 315-08, LRFD):

MONOLITHIC OR STEMWALL FOOTINGS: 5000 PSI SLABS SUPPORTED ON GRADE LEVEL WHEN REINFORCED WITH W.W.M.: 5000 PSI COLUMNS, CAST IN PLACE WALLS AND SHEARWALLS: 5000 PSI ELEVATED BEAMS AND SLABS: 5000 PSI MASONRY GROUT IN CELLS: 3000 PSI

# 5000 PSI CONCRETE:

THIS SECTION TO BE USED WHEN SPECIFIED UNDER THE "CONCRETE STRENGTH" STRUCTURAL NOTES.

CEMENT SHALL BE TYPE I CONFORMING TO ASTM C-150. NOMINAL MAXIMUM COARSE AGGREGATE SIZE TO BE 3/4". THE FOLLOWING SPECIFICATION FOR CONCRETE STRENGTH IS BASED ON

ACI 318, TABLE 4.2.2. A MAXIMUM OF 0.15 (PERCENT BY WEIGTH OF CEMENT) WATER SOLUBLE CHLORIDE ION CONTENT SHALL BE ALLOWED IN THE CONCRETE MIX. TO BE DETERMINED ON THE CONCRETE MIXTURE BY ASTM C1216 AT AGE BETWEEN 28 AND 42 DAYS,

PROVIDE TEST RESULTS TO THIS OFFICE. ALL CONCRETE TO BE READY MIX CONCRETE TO ACHIEVE A 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI (AND WATER TO CEMENT RATIO OF 0.4, PLEASE NOTE THIS W/C USUALLY RESULTS IN HIGHER CONCRETE STRENGTHS AND THEREFORE GOVERNS THE CONCRETE STRENGTH REQUIREMENT) FOR ALL STRUCTURAL ELEMENTS (INCLUDING, BUT NOT LIMITED TO FOOTINGS, SLABS, BEAMS, COLUMNS) CUTTING HOLES IN PRE-ENGINEERED FLOOR SYSTEMS SHALL BE COORDINATED BY THE GENERAL CONTRACTOR AND THE SYSTEM SUPPLIER BEFORE PRODUCTION. CONCRETE MIX SUBMITTALS SHALL DESCRIBE WHICH LOCATION IN THE STRUCTURE THE MIX IS TO BE PLACED.

### CONCRETE PLACEMENT:

ACI347.2R-10, 4.5: NO WORKER SHALL BE ALLOWED DIRECTLY UNDER THE FORMS DURING CONCRETE PLACEMENT. CURING AGENTS USED ON ROOFS SHALL BE CHECKED FOR COMPATIBILITY WITH ROOFING MATERIALS.

# **CONCRETE REINFORCEMENT:**

TEMPERATURE REINFORCEMENT IN STRUCTURAL SLABS SHALL BE INSTALLED CONTINUOUSLY (WITHOUT BREAKS OR INTERRUPTIONS) IN ALL AREAS OF THE SLAB AND RUNS PERPENDICULAR TO THE MAIN REINFORCEMENT. OVERLAP TEMPERATURE BARS WHERE NEEDED A MIN OF 24"

CONCRETE REINFORCEMENT BARS, STIRRUPS AND TIES SHALL BE IN ACCORDANCE WITH ASTM A-615 (GRADE 60), REINFORCING BARS THAT ACT AS LAP DOWELS AND ARE TO BE WELDED TO STRUCTURAL STEEL SHALL CONFORM TO ASTM A-706 (GRADE 60). REINFORCING BARS CONTAINING A LIGHT COAT OF RUST SHALL BE ALLOWED.

ACI HOOK TO BE PROVIDED FOR ALL REQUIRED CONDITIONS PER DETAILS PRESENTED IN THIS PROJECT.

FIELD WATER IS NOT PERMITTED TO BE ADDED TO THE CONCRETE MIX.

ALL EXTERIOR SLAB REBAR SHALL BE SEPARATED BY PLASTIC CHAIRS FROM ANY CONCRETE FACES. WELDED REINFORCEMENT SHALL CONFORM TO AWS D1.4 OR

BE ASTM A706 GRADE 60. ANCHOR EMBEDMENT LENGTH INCLUDE STRUCTURAL CONCRETE ONLY (DOES NOT INCLUDE STUCCO). ALL CONCRETE SURFACES TO BE KEPT IN A MOIST CONDITION FOR 7 DAYS AFTER POUR.

CONCRETE COVER SHOULD BE AS FOLLOWS UNLESS NOTED OTHERWISE:

CONCRETE PLACED DIRECTLY IN CONTACT WITH GROUND:

ALL BAR SIZES

FACE OF CONCRETE ELEMENT EXPOSED TO WEATHER OR GROUND AFTER FORM REMOVAL:

#5 BARS OR SMALLER 1-1/2"

ALL OTHER BAR SIZES

FACE OF CONCRETE ELEMENT NOT EXPOSED TO WEATHER OR GROUND:

SLABS, CAST IN PLACE WALLS 3/4

BEAMS AND COLUMNS

COLUMN AND FILLED CELL LAP SPLICES SHALL BE 48 BAR DIAMETERS. THE CONTRACTOR SHALL ALLOW FOR AN ADDITIONAL 1% AMOUNT OF REBAR TO BE USED AT THE DISCRETION OF THE STRUCTURAL INSPECTOR IN ORDER TO RECTIFY OR STRENGTHEN ANY CONDITIONS FOUND TO BE CRITICAL DURING CONSTRUCTION.

ALL OTHER TRADES PLANS SHALL BE VERIFIED TO IDENTIFY THE LOCATION OF DEPRESSIONS, RECESSES, PIPES, PENETRATIONS, SLEEVES, ETC. SAID PENETRATIONS, SPECIALLY WHEN THEY CROSS CONCRETE BEAMS AND COLUMNS, MUST BE BLOCKED OUT FROM THE POUR, DISPLACING THE LEAST AMOUNT OF CONCRETE, PRIOR TO THE POUR. ANY PENETRATIONS LARGER THAN 5" DIAMETER MUST BE APPROVED BY THIS OFFICE.

ALL FRESH CONCRETE TO EXISTING CONCRETE CONTACT FACES SHALL HAVE AN APPROVED FRESH CONCRETE EPOXY BONDING AGENT APPLIED AS PER THE MANUFACTURER S SPECIFICATIONS.

ALL FORMS SHALL BE CLEANED WITH COMPRESSED AIR IN ORDER TO REMOVE ANY DIRT. LAITANCE. DEBRIS. ETC. PRIOR TO THE PLACEMENT OF ANY CONCRETE.

FOR PIPES THAT RUN ON THE INSIDE OF THE REINFORCING CAGE OF CONCRETE COVER AGAINST PVC PIPES DUE TO INEVITABLE FIELD COORDINATION WITH PLUMBING, MUST BE FIELD SPRAY GALVANIZED 6" TO EACH SIDE OF EACH PIPE. NO REBAR SHALL HAVE LESS THAN 3/4" COVER AGAINST A PIPE.

ANY CONSTRUCTION JOINT BETWEEN TWO SLABS REQUIRES CONNECTION DOWELS TO PREVENT DIFFERENTIAL SETTLEMENT THAT COULD CRACK FLOORING OR OTHER FINISHES. UNLESS NOTED OTHERWISE, USE #3 REBAR, SINGLE PIECE, EXTENDING 12" INTO EACH OF BOTH SLABS. SPACED @12" O/C AND 4" FROM EACH

# **CONCRETE TESTING:**

QUALIFIED FIELD TESTING TECHNICIANS SHALL PREPARE SPECIMENS REQUIRED FOR CURING UNDER FIELD CONDITIONS AND RECORD THE TEMPERATURE OF THE FRESH CONCRETE WHEN PREPARING SPECIMENS FOR LABORATORY STRENGTH

ACCOUNTING SEPARATELY FOR EACH CLASS OF CONCRETE AND EACH DAY, ONE TEST (CONSISTING OF BREAKING (2) 6"0 DIAMETER x 12" HIGH CONCRETE CYLINDER SAMPLES, MOLDED AS PER ASTM C31 AND TESTED PER ASTM C39) SHALL BE PERFORMED ACCORDING TO THE MOST FREQUENT OF:

- ONCE A DAY
- ONCE EVERY 150 CUBIC YARDS
- ONCE EVERY 5000 SQUARE FEET OF SLAB OR WALL SURFACE AREA

7 DAY, 14 DAY AND 28 DAY TEST REPORTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW. THE CLIENT SHALL RETAIN A REPUTABLE TESTING COMPANY TO BE ON SITE AT THE TIME OF CONCRETE POUR AND INDEPENDENTLY COLLECT CYLINDERS AND PREFORM COMPRESSIVE TESTING.

# **ANCHORING SPECIFICATIONS:**

CONCRETE SCREWS SHALL BE HILTI KWIK-CON II (NOA#12-0625.05) EXPANSION BOLTS SHALL HAVE A LENGTH IDENTIFYING LABEL AND BE HILTI KWIK **BOLT 3 OR EQUAL** 

### **CONCRETE TIE BEAM:**

OCCASIONALLY, A LOWER CONCRETE HEADER BEAM IS SPECIFIED, THEN CMU BLOCK ISSPECIFIED ON TOP WITH A TIE BEAM AT THE TOP OF THE WALL. THIS IS DONE WITH THE INTENT TO ECONOMIZE IN THE FORMING, REBAR AND CONCRETING. HOWEVER, DUE TO THIS REQUIRING SEPARATE CONCRETE POURS, THE CONTRACTOR MAY ELECT TO FULLY FORM THE BEAM BY DOING ONE SINGLE DEEP BEAM. THIS IS ALWAYS ACCEPTABLE AS LONG AS 2#5 MID BARS ARE ADDED FOR EVERY 12" OF BLOCK HEIGHT BEING REPLACED. ALSO THE LOW HEADER BEAM SPECIFIED STIRRUP SPACING MUST BE MADE FULL HEIGHT.

GENERAL CONTRACTOR IS TO VERIFY BEAM DEPTH & ELEVATIONS WITH ARCHITECTURAL PLANS & NOTIFY ARCHITECT OF ANY DISCREPANCIES. COORDINATE MASONRY OPENINGS WITH WINDOW MANUFACTURER & N.O.A.

FOR ARCHED BEAMS ADD (2) # 5'S DIAGONAL EACH END OF THE BEAM. ADD (1) EXTRA #5 DIAGONAL EACH SIDE FOR EVERY 4" EXTRA WIDTH. BEAM DEPTHS ARE MINIMUM REQUIRED, ADD 2#5 MIDDLE FOR EVERY ADDITIONAL 12" OF DEPTH

TYPICAL TIE BEAM OVER WALLS TO BE (PER FBC 2121.2.3.2) 8"x12" w/ (2) #5's TOP AND BOTTOM. (4) #3 TIES @ EACH SIDE OF EVERY BEND. CORNER AND TRANSITION. TIE BALANCE @48" O/C. TIE BEAM DROPS DOWN TO TOP OF OPENING - SEE FRAMING

FOR WIDER THAN 8" TIE BEAMS, INCREASE TOP AND BOTTOM LONGITUDINAL STEEL BY (1) # 5 PER EXTRA 4" OF WIDTH.

"PCL" REFERS TO PRECAST LINTEL MANUFACTURED BY CASTCRETE WITH NOA # 14-0903.03. USE 12" WIDE LINTEL FOR 12" THICK WALLS TIES AT INTERMEDIATE AND END SUPPORT SPACING REFERS TO TIES TO EACH SIDE OF EACH INTERMEDIATE COLUMN OR WALL SUPPORT

BALANCE REFERS TO THE SPACING OF THE REMAINING TIES AFTER THE TIES AT EACH END HAVE BEEN INSTALLED. IF NO END TIES SHOWN, USE BALANCE THROUGHOUT THE ENTIRE LENGTH OF THE BEAM.

GC TO VERIFY DEPTH OF BEAMS WITH WINDOW AND DOOR HEIGHTS. COORDINATE WITH GLAZING MANUFACTURER.

WHEN THE BEAM SPECIFIED IN THE STRUCTURAL PLAN RUNS OVER A CMU WALL, IT'S ACTUAL WIDTH IS TO BE THE ACTUAL WIDTH OF THE WALL, WHICH WOULD BE 7-5/8" OVER 8" CMU.

### NOMINAL AND TRUE THICKNESS MEMBERS:

THE FOLLOWING ARE MEMBERS IN WHICH THE NOTED THICKNESS AND WIDTH IN PLAN AND SCHEDULE IS I NTENDED TO BE THE EXACT MEASUREMENT OF THE ACTUAL BUILT MEMBERS.

- \* ALL CONCRETE SLABS
- \* INTERIOR CONCRETE BEAMS AND COLUMNS FOOTINGS, GRADE BEAMS AND PILES
- \* STRUCTURAL STEEL MEMBERS (COLUMNS, BEAMS AND CONNECTION MEMBERS)

THE FOLLOWING MEMBERS HAVE THEIR NOMINAL DIMENSIONS NOTED AND THE **ACTUAL DIMENSION VARIES:** 

\* CONCRETE COLUMNS AND TIE BEAMS IN-LINE WITH MASONRY WALLS, WHERE THE COLUMN THICKNESS SHALL MATCH THE WALL NET THICKNESS. DRESSED LUMBER

# STRUCTURAL STEEL (AISC LRFD):

CONTRACTOR SHALL RETAIN A STRUCTURAL STEEL FABRICATOR THAT WILL MODEL THE STEEL MEMBERS. THEIR GEOMETRY AND THE CONNECTIONS BETWEEN EACH IN ORDER IN A 3D STRUCTURAL STEEL DETAILING PROGRAM IN ORDER TO GENERATE SHOP DRAWINGS SHOWING THE CONNECTIONS TO SCALE. THESE SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL TO THIS OFFICE PRIOR TO FABRICATION. THIS OFFICE WILL NOT BE RESPONSIBLE FOR FABRICATION ERRORS ARISING FROM FABRIADED STEEL WITHOUT A STAMPED SHOP DRAWING APPROVAL FROM THIS OFFICE.

ALL STRUCTURAL STEEL SHALL COMPLY WITH AISC LRFD 13TH EDITION. STEEL BEAMS, C-CHANNELS AND MC-CHANNELS SHALL COMPLY WITH ASTM A992 (GRADE 50).

STEEL RECTANGULAR / SQUARE COLUMN SECTIONS - ASTM A-500 (Fv=46 KSI). GRADE B. STEEL CIRCULAR COLUMN SECTIONS - ASTM A-500 (Fv=42 KSI), GRADE B.

STEEL ANGLES AND S-SHAPES SHALL CONFORM TO ASTM A36. ALL STEEL BOLTS AND THREADED RODS, UNLESS NOTED OTHERWISE ON THE PLANS TO BE ASTM A307, GRADE A AND INSTALLED TO A SNUG TIGHT CONDITION (DEFINED AS BEING TIGHTENED BY THE FULL EFFORT OF A WORKER USING A REGULAR SPUD WRENCH). BOLT NUTS TO BE ASTM A563.

ALL STRUCTURAL STEEL (NOT INCLUDING REBAR) TO HAVE A SHOP COAT OF RUST INHIBITIVE PAINT.

AFTER INSTALLATION A FIELD TOUCH UP SHALL BE APPLIED. UNLESS THE JOBSITE IS FURTHER THAN 2 MILES FROM THE OCEAN, ANY STRUCTURAL STEEL THAT IS TO BE EXTERIOR TO THE BUILDING ENVELOPE SHALL BE HOT DIP GALVANIZED. ALL SHEAR STUDS SHALL BE TYPE B. ASTM A108 (Fu = 65 ksi)

ALL CABLES SHALL BE STAINLESS STEEL 6x19 WIRE ROPE - TYPE 304, IN COMPLIANCE WITH ASCE19-10 PROVISIONS.

DO NOT CUT THE TOP STEEL PLATES IN ORDER TO ALLOW CONCRETE FROM THE TIE BEAM TO FILL THE STEEL COLUMN. IF CONCRETE FILLED STEEL COLUMNS ARE REQUIRED BY THE STEEL COLUMN SCHEDULE OR THE ARCHITECT, THEY SHALL BE FILLED IN THE FABRICATING SHOP PRIOR TO PLATE WELDING. ALL SHOP AND FIELD WELDING SHALL BE PERFORMED BY WELDERS QUALIFIED, AS

DESCRIBED IN "AMERICAN WELDING SOCIETY'S STANDARD QUALIFICATION PROCEDURE" (ASW D1.1), TO PERFORM THE TYPE OF WORK REQUIRED. ALL STEEL WELDING RODS SHALL BE E70XX ELECTRODES. ALL STEEL BEAM CONNECTIONS ARE INTENDED TO SIT DIRECTLY ON TOP OF TOP

PLATES OF HOLLOW STEEL TUBE COLUMNS (NOT TO THE SIDE OF STEEL TUBE COLUMNS). DO NOT CUT TOP PLATES OF STEEL COLUMNS TO BE FILLED WITH ALL STEEL BEAMS CONNECTIONS TO EMBED PLATES ON CONCRETE SHALL BE DONE

AFTER CONCRETE HAS REACHED 70% OF THE SPECIFIED COMPRESSIVE STRENGTH (VERIFY WITH THE 7 DAY CONCRETE SPECIMEN BREAK RESULTS). DO NOT CUT THE HEADS OR NUTS OF ANY BOLTS THAT PROTRUDE PAST STEEL PLATES OF STEEL BUCKETS WITHOUT PRIOR CONSULTATION WITH THIS OFFICE.

PERMIT ISSUE: REVISIONS: DATE: **AMERICAN** BUILDING **ENGINEERS** 1842 East Oakland Park Blvd. Fort Lauderdale, Florida 33306 305.681.5563 Fort Lauderdale: 954.739.5099 Palm Beach: 561.243.3939 Certificate of Authorization # 4732 2021 THESE PLANS AND THE INFORMATION SHOWN HEREIN ARE THE PROPERTY OF A.B.E.INC ANY REPRODUCTION, DISCLOSURES OR USE THERE OF IN WHOLE OR PART IS EXPRESSLY PROHIBITED S ABE BORUJERDI. P.E. RJE1138553 PHONE: (954)739-5099 PROVED SEAL SIGNATURE No 38533 STATE OF CORIDA WINSONAL EN "Hilling COUNT 9 4 Œ <u>М</u>  $\leq \mathbb{Z}$  $\supset$   $\circ$ **—** 19.020

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