MiTe

Lymber design values are in accordance with ANSI/TPI 1 section 6.3 hese truss designs rely on lumber values established by others.

RE: 3702918 - GIEBEIG CONST. - LOT 2 CW

MiTek, Inc.

16023 Swingley Ridge Rd. Chesterfield, MO 63017 314.434.1200

Site Information:

Customer Info: Gioebeig Const. Project Name: Spec Hse Model: 1676 Lot/Block: 2

Subdivision: Crosswinds

Address: TBD, TBD

City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Roof Load: 37.0 psf

Wind Code: ASCE 7-16

Design Program: MiTek 20/20 8.6

Wind Speed: 130 mph Floor Load: N/A psf

Truss Name

T08 T09

Date 10/6/23

10/6/23 10/6/23 10/6/23 10/6/23 10/6/23

This package includes 20 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	
1	T31766134	CJ01	10/6/23	15	T31766148	
2	T31766135 T31766136	CJ03 CJ05	10/6/23 10/6/23	16 17	T31766149 T31766150	
4	T31766137	EJ01	10/6/23	18	T31766151	
5	T31766138	EJ02	10/6/23	19	T31766152	
7	T31766139 T31766140	HJ08 HJ10	10/6/23 10/6/23	20	T31766153	
8	T31766141	T01	10/6/23			
9	T31766142 T31766143	T01G T02	10/6/23			
11	T31766144	T03	10/6/23 10/6/23			
12	T31766145	T04	10/6/23			
123456789101234	T31766146 T31766147	T05 T06	10/6/23 10/6/23			



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The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

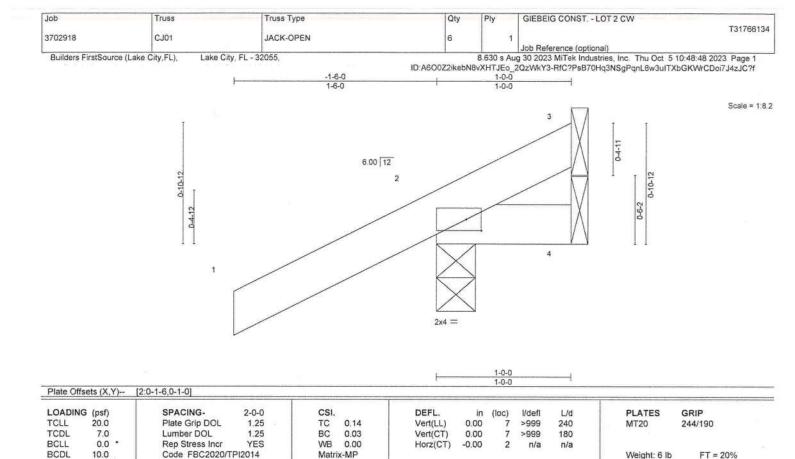
Truss Design Engineer's Name: ORegan, Philip My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=39(LC 12)

Max Uplift 3=-6(LC 1), 2=-66(LC 12), 4=-19(LC 1) Max Grav 3=7(LC 16), 2=179(LC 1), 4=18(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES.

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

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Structural wood sheathing directly applied or 1-0-0 oc purlins.

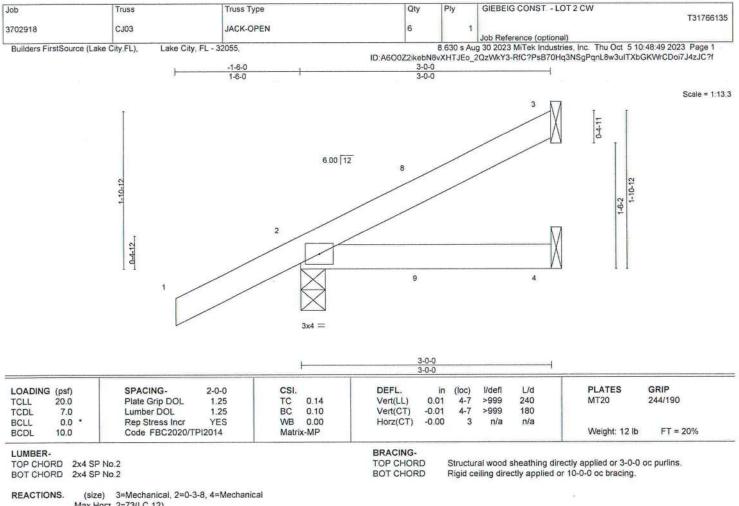
Rigid ceiling directly applied or 10-0-0 oc bracing.

Philip J. O'Regna PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16025 Swingley Ridge Rd. Chenterfield, MO 63017

October 6,2023

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Max Horz 2=73(LC 12)

Max Uplift 3=-36(LC 12), 2=-57(LC 12), 4=-16(LC 9) Max Grav 3=60(LC 1), 2=210(LC 1), 4=50(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

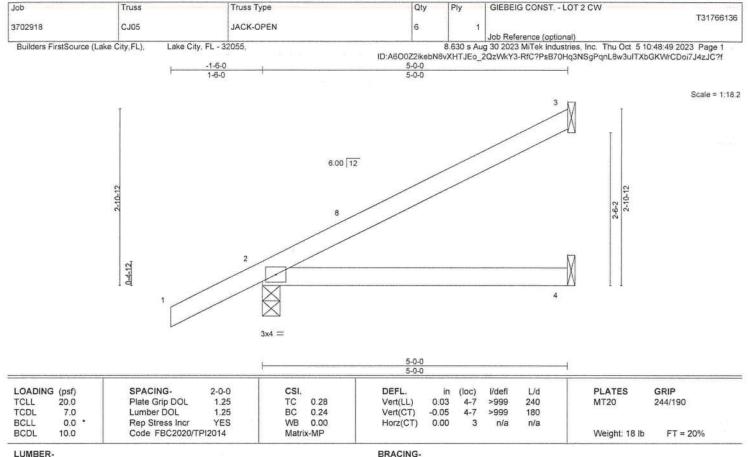
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Phills J. O'Rogon PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 5634 16025 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

3=Mechanical, 2=0-3-8, 4=Mechanical (size)

Max Horz 2=107(LC 12)

Max Uplift 3=-67(LC 12), 2=-64(LC 12)

Max Grav 3=113(LC 1), 2=276(LC 1), 4=88(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-11-4 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.

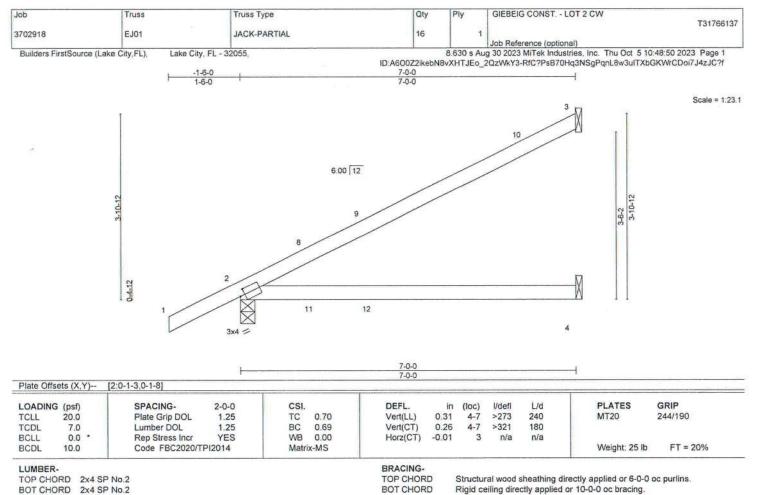
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Philip J. O'Regsa PE No.58126 MiTek Inc. DBA MITek USA H. Ceet 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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REACTIONS.

(size) 3=Mechanical 2=0-3-8 4=Mechanical

Max Horz 2=137(LC 12)

Max Uplift 3=-86(LC 12), 2=-76(LC 12), 4=-40(LC 9) Max Grav 3=164(LC 1), 2=346(LC 1), 4=126(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

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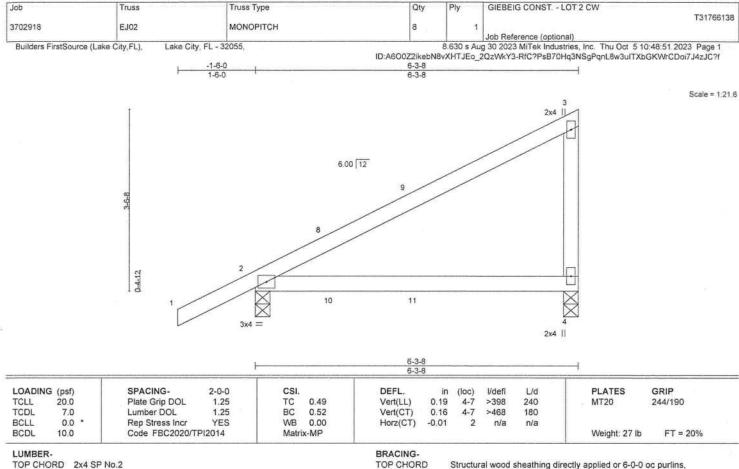
Philip J. O'Regen PE No. 58126 MITek Inc. DBA MITek USA FL. Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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BOT CHORD WEBS

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3

Structural wood sheathing directly applied or 6-0-0 oc purlins,

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 4=0-3-8, 2=0-3-8

Max Horz 2=128(LC 12)

Max Uplift 4=-86(LC 9), 2=-70(LC 12) Max Grav 4=218(LC 1), 2=318(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-1-12 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

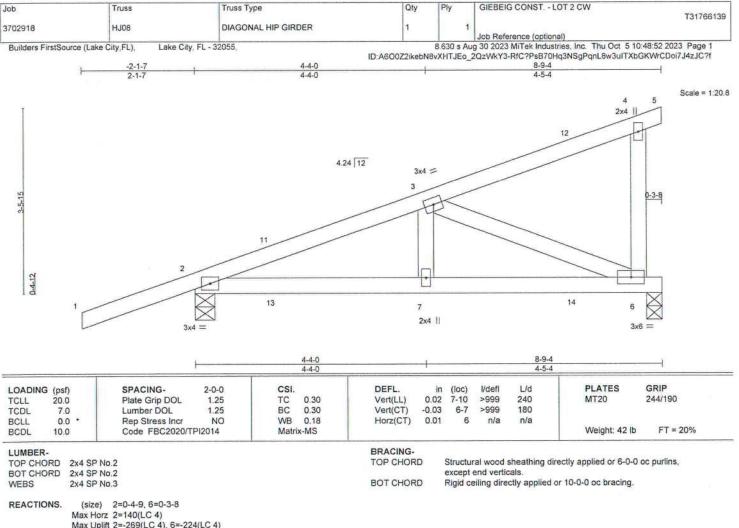
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Max Uplift 2=-269(LC 4), 6=-224(LC 4) Max Grav 2=458(LC 1), 6=434(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-538/228

BOT CHORD 2-7=-278/478, 6-7=-278/478

WEBS 3-6=-486/285

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=269, 6=224.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 60 lb down and 73 lb up at 1-6-1, 60 lb down and 73 lb up at 1-6-1, 22 lb down and 39 lb up at 4-4-0, 22 lb down and 39 lb up at 4-4-0, and 42 lb down and 78 lb up at 7-1-15, and 42 lb down and 78 lb up at 7-1-15 on top chord, and 40 lb down and 44 lb up at 1-6-1, 40 lb down and 44 lb up at 1-6-1, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, and 62 lb down at 7-1-15, and 62 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 6-8=-20

Vert: 7=-5(F=-3, B=-3) 12=-73(F=-37, B=-37) 14=-58(F=-29, B=-29)

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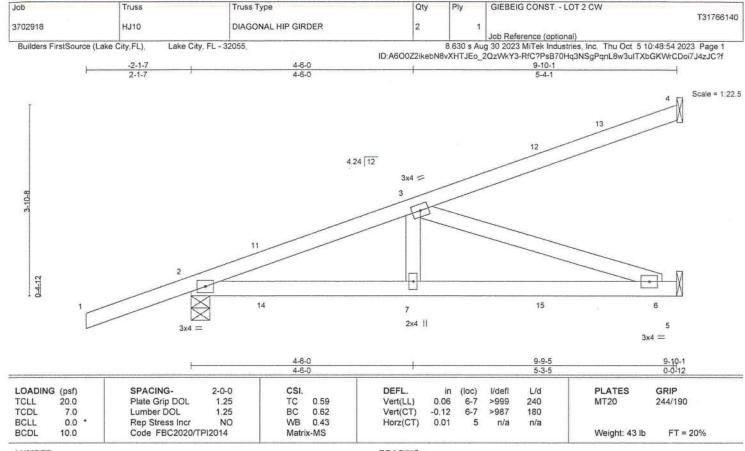
Phillp J. O'Regan PE No.58126 MITek Inc. DBA MITek USA FL Cert 8634 16913 Swingley Ridge Rd. Chosterfield, MO 63917

October 6,2023

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LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-3-0 oc bracing.

REACTIONS. (size

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical

Max Horz 2=148(LC 4)

Max Uplift 4=-78(LC 4), 2=-301(LC 4), 5=-144(LC 4) Max Grav 4=150(LC 1), 2=527(LC 1), 5=298(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3

2-3=-787/340 2-7=-394/716, 6-7=-394/716

BOT CHORD 2-7=-394/716, 6-7=-394/716 WEBS 3-7=-60/279, 3-6=-755/415

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=301, 5=144.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 60 lb down and 73 lb up at 1-6-1, 60 lb down and 73 lb up at 1-6-1, 22 lb down and 39 lb up at 4-4-0, 22 lb down and 39 lb up at 4-4-0, and 43 lb down and 43 lb down and 78 lb up at 7-1-15, and 43 lb down and 78 lb up at 7-1-15 on top chord, and 40 lb down and 44 lb up at 1-6-1, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, 19 lb down and 24 lb up at 4-4-0, and 64 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-5(F=-3, B=-3) 12=-73(F=-37, B=-37) 15=-58(F=-29, B=-29)

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Philip J, O'Regen PE No.58126 MITck Inc. DBA MITck USA FL Cert 6634 16015 Swingley Ridge Rd. Chesterfield, MO 63017 Bares

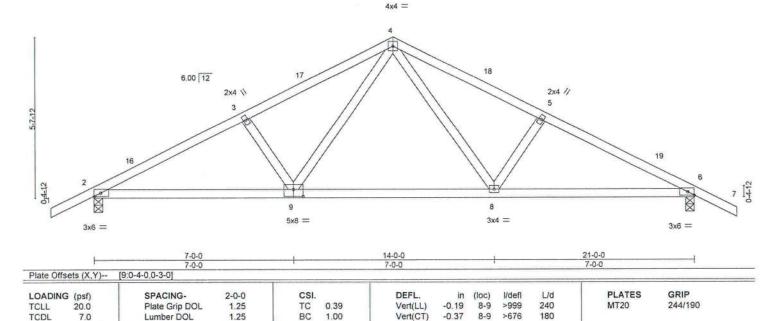
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Scale = 1:38.8



Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.05

6

n/a

n/a

Rigid ceiling directly applied or 9-4-14 oc bracing.

Structural wood sheathing directly applied or 4-1-14 oc purlins.

Weight: 99 lb

FT = 20%

LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

0.0

10.0

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 6=0-3-8

Max Horz 2=-89(LC 13)

Max Uplift 2=-250(LC 12), 6=-250(LC 13)

Rep Stress Incr

Code FBC2020/TPI2014

Max Grav 2=1068(LC 1), 6=1068(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1825/509, 3-4=-1683/512, 4-5=-1683/512, 5-6=-1825/509

BOT CHORD 2-9=-374/1579, 8-9=-190/1063, 6-8=-382/1579

WEBS 4-8=-197/713, 5-8=-255/167, 4-9=-197/713, 3-9=-255/167

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 10-6-0, Exterior(2R) 10-6-0 to 13-6-0, Interior(1) 13-6-0 to 22-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-MS

0.27

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

NO

- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=250. 6=250.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 9-10=-20, 8-9=-80(F=-60), 8-13=-20

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Phillp J. O'Regau PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chosterfield, MO 63917

October 6,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



GIEBEIG CONST. - LOT 2 CW Truss Truss Type Qty Job T31766142 COMMON SUPPORTED GAB T01G 3702918 Job Reference (optional) 8.630 s Aug 30 2023 MiTek Industries, Inc. Thu Oct 5 10:48:56 2023 Page 1 Builders FirstSource (Lake City,FL), Lake City, FL - 32055, ID:A600Z2ikebN8vXHTJEo_2QzWkY3-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 21-0-0 10-6-0 22-6-0 1-6-0

Scale = 1:39.7

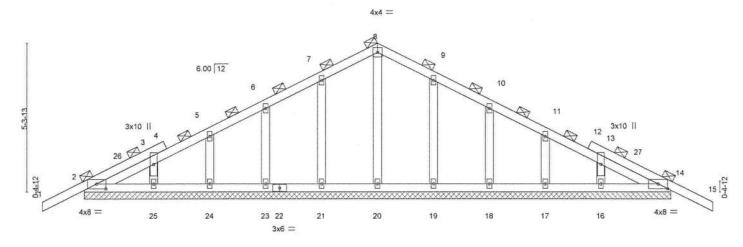


Plate Offs	sets (X,Y) [2:0-4-0,0-2-1], [14:0-4-0	0-2-1]										
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.13	Vert(LL)	-0.01	15	n/r	120	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.04	Vert(CT)	-0.01	15	n/r	120	NO DECRETO		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	14	n/a	n/a			
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-S						Weight: 114 lb	FT = 20%	

LUMBER-

OTHERS

TOP CHORD 2x4 SP No.2 BOT CHORD

2x4 SP No.2 2x4 SP No 3 BRACING-

TOP CHORD **BOT CHORD**

2-0-0 oc purlins (6-0-0 max.).

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 21-0-0.

Max Horz 2=-84(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 23, 24, 25, 19, 18, 17, 16 Max Grav All reactions 250 lb or less at joint(s) 2, 14, 20, 21, 23, 24, 25, 19, 18, 17, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

1) Unbalanced roof live loads have been considered for this design

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 10-6-0, Corner(3R) 10-6-0 to 13-6-0, Exterior(2N) 13-6-0 to 22-6-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 23, 24, 25 19 18 17 16
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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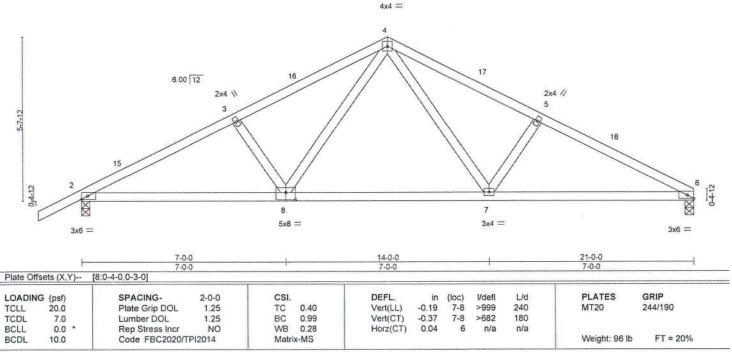
Philip J. O'Regen PE No. 58126 Mil'ek Inc. DBA Mil'ek USA FI, Cert 6634 16623 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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Job	Truss	Truss Type		Qty	Ply	GIEBEIG CONST LOT 2 CW	T31766143
3702918	T02	COMMON		4	1	Job Reference (optional)	131700143
Builders FirstSource	e (Lake City,FL), Lake	City, FL - 32055,		ID: A6O072ikebN		ug 30 2023 MiTek Industries, Inc. Thu Oct 5 1/2 2QzWkY3-RfC?PsB70Hq3NSgPqnL8w3uITXb	
, -1-6-0	5-4-0		10-6-0	1	15-8-		1
1-6-0	5-4-0		5-2-0		5-2-0	5-4-0	1



BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

6=0-3-8, 2=0-3-8 (size) Max Horz 2=100(LC 16)

Max Uplift 6=-217(LC 13), 2=-251(LC 12) Max Grav 6=984(LC 1), 2=1071(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1831/517, 3-4=-1688/520, 4-5=-1700/530, 5-6=-1844/527

BOT CHORD 2-8=-413/1584, 7-8=-223/1069, 6-7=-415/1599

4-7=-205/729, 5-7=-264/171, 4-8=-196/712, 3-8=-255/167 WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl. GCpi=0.18: MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 10-6-0, Exterior(2R) 10-6-0 to 13-6-0, Interior(1) 13-6-0 to 21-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=217, 2=251,
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 8-12=-20, 7-8=-80(F=-60), 7-9=-20

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Structural wood sheathing directly applied or 4-1-11 oc purlins.

Rigid ceiling directly applied or 9-0-9 oc bracing.

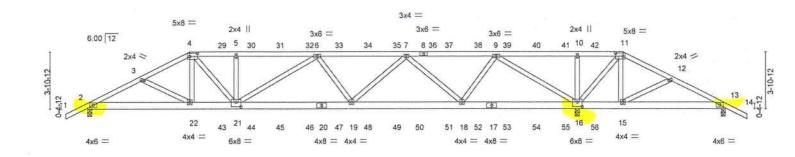
Philip J. O'Regna PE No. 58126 MITek Inc. DBA MITek USA FL Cert 6634 16623 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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Job			Truss		,		Truss Type				Qty	Ply	GIEBEIG CO	NST LOT 2	CW			T047004
3702918			T03				HIP GIRDER				1	2	Job Reference	e (optional)				T3176614
Builders First	Source (La	ake C	ity,FL),		Lake City,	FL - 3	2055,	- 1		ID:A600	72ikebN	8.630 s A	ug 30 2023 MiTe 20zWkY3-RfC	ek Industries, Ir				
,-1-6-0,	3-9-11		7-0-0	1	10-1-12		15-8-6	1	21-10-0		7-11-10		33-6-4	36-8-0	39-10-5	1	43-8-0	45-2-0
1-6-0	3-9-11		3-2-5	1	3-1-12		5-6-10	1	6-1-10	M	6-1-10	1	5-6-10	3-1-12	3-2-5	1,8	3-9-11	1-6-0



		7-0-0 10-1-12		0-11		25-7-6	-		3-6-4	-	36-8-0	43-8-0	
	8	7-0-0 3-1-12	7-1	0-15		7-6-11	N.	7-	10-15		3-1-12	7-0-0	
Plate Offse	ets (X,Y)	[4:0-6-0,0-2-8], [11:0-6-0	.0-2-8], [16:0-4-	0,0-3-12], [2	21:0-4-0,0-3-	12]		mieste in	-				
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d		PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.75	Vert(LL)	0.18	19-21	>999	240		MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.53	Vert(CT)	-0.29	19-21	>999	180			
BCLL	0.0	Rep Stress Incr	NO	WB	0.94	Horz(CT)	0.05	16	n/a	n/a			
BCDL	10.0	Code FBC2020/T	P12014	Matr	ix-MS							Weight: 523 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 2x6 SP No.2 **BOT CHORD** 2x4 SP No.3 WEBS

BRACING-

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-7-6 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

6-0-0 oc bracing: 15-16,13-15.

REACTIONS.

(size) 2=0-3-8, 16=0-3-8, 13=0-3-8

Max Horz 2=-63(LC 28) Max Uplift 2=-900(LC 8), 16=-2444(LC 4), 13=-943(LC 19) Max Grav 2=2120(LC 19), 16=5397(LC 1), 13=451(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-4156/1843, 3-4=-3969/1792, 4-5=-4424/2036, 5-6=-4424/2036, 6-7=-4420/2011,

7-9=-1746/814, 9-10=-1637/3712, 10-11=-1637/3712, 11-12=-1054/2415,

12-13=-1037/2323

BOT CHORD 2-22=-1642/3683, 21-22=-1554/3538, 19-21=-2114/4729, 18-19=-1554/3461,

16-18=-193/427, 15-16=-2141/996, 13-15=-2049/941

4-22=-210/522, 4-21=-656/1372, 5-21=-493/254, 6-21=-412/220, 6-19=-562/308,

7-19=-588/1317, 7-18=-2351/1090, 9-18=-1038/2374, 9-16=-4864/2217, 10-16=-557/284,

11-16=-2402/1097, 11-15=-224/480, 12-15=-278/150

NOTES-

WEBS

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

- 3) Unbalanced roof live loads have been considered for this design.
 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=900, 16=2444, 13=943.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regnu PE No.58126 MITek Inc. DBA MITek USA FL Cort 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG CONST LOT 2 CW	Special D
3702918	Т03	HIP GIRDER	1	_		T31766144
3702310	100	THE GIRDER		2	Job Reference (optional)	

Builders FirstSource (Lake City,FL),

Lake City, FL - 32055,

8.630 s Aug 30 2023 MiTek Industries, Inc. Thu Oct 5 10:49:03 2023 Page 2 ID:A6O0Z2ikebN8vXHTJEo_2QzWkY3-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

NOTES-

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 91 lb up at 7-0-0, 110 lb down and 91 lb up at 9-0-12, 110 lb down and 91 lb up at 11-0-12, 110 lb down and 91 lb up at 13-0-12, 110 lb down and 91 lb up at 15-0-12, 110 lb down and 91 lb up at 19-0-12, 110 lb down and 89 lb up at 21-0-12, 110 lb down and 89 lb up at 22-7-4, 110 lb down and 91 lb up at 24-7-4, 110 lb down and 91 lb up at 28-7-4, 110 lb down and 91 lb up at 32-7-4, 110 lb down and 91 lb up at 32-7-4, and 110 lb down and 91 lb up at 34-7-4, and 230 lb down and 174 lb up at 36-8-0 on top chord, and 334 lb down and 237 lb up at 7-0-0, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 13-0-12, 86 lb down and 60 lb up at 22-7-4, 86 lb down and 60 lb up at 28-7-4, 86 lb down and 60 lb up at 28-7-4, 86 lb down and 60 lb up at 28-7-4, 86 lb down and 60 lb up at 32-7-4, 86 lb down and 80 lb up at 32-7-4, 86 lb down and 80 lb up at 32-7-4, 86 lb down and 80 lb up at 32-7-4, 86 lb down and 80 lb up at 32-7-4, 8

LOAD CASE(S) Standard

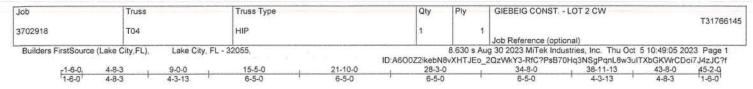
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

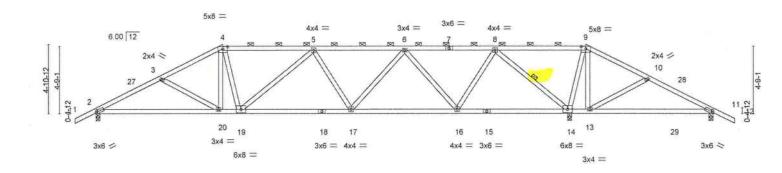
Uniform Loads (plf)

Vert: 1-4=-54, 4-11=-54, 11-14=-54, 23-26=-20

Concentrated Loads (lb)

Vert: 4=-110(B) 11=-183(B) 22=-334(B) 15=-334(B) 29=-110(B) 30=-110(B) 31=-110(B) 32=-110(B) 33=-110(B) 34=-110(B) 35=-110(B) 35=-110(B) 36=-110(B) 36=-11





	1	9-0-0	10-1-12	18-1-1	- 1	25-6-15	1		33-6-4	34-8-0	43-8-0	
		9-0-0	1-1-12	7-11-5		7-5-15	- 1		7-11-5	1-1-12	9-0-0	
Plate Offse	ets (X,Y)	[2:0-1-5,0-1-8], [4:0-4-0	,0-1-15], [9:0-	4-0,0-1-15], [1	1:0-1-5,0-1-8	1		97.				
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.70	Vert(LL)	0.22 1	3-26	>555	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.72	Vert(CT)	-0.29 2	0-23	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz(CT)	0.07	14	n/a	n/a		
BCDL	10.0	Code FBC2020	TPI2014	Matr	ix-MS	**************************************					Weight: 229 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

2x4 SP No.3

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-0-15 oc purlins,

except

2-0-0 oc purlins (4-2-0 max.): 4-9.

BOT CHORD Rigid ceiling directly applied or 5-3-5 oc bracing.

WEBS

1

1 Row at midpt 8-14

REACTIONS.

(size) 2=0-3-8, 14=0-3-8, 11=0-3-8

Max Horz 2=76(LC 12)

Max Uplift 2=-295(LC 12), 14=-593(LC 8), 11=-366(LC 23) Max Grav 2=1150(LC 23), 14=2379(LC 1), 11=88(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1952/502, 3-4=-1716/432, 4-5=-1555/424, 5-6=-1610/420, 6-8=-693/213,

8-9=-293/1326, 9-10=-254/1199, 10-11=-238/1051

BOT CHORD 2-20=-451/1706, 19-20=-319/1490, 17-19=-418/1744, 16-17=-319/1290, 13-14=-1047/298, 11-13=-908/229

WEBS 4-20=-32/285, 4-19=-98/304, 5-19=-335/172, 5-17=-270/169, 6-17=-127/507,

6-16=-940/277, 8-16=-192/1015, 8-14=-1906/528, 9-14=-1096/737, 9-13=-620/335,

10-13=-354/257

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 9-0-0, Exterior(2R) 9-0-0 to 15-5-0, Interior(1) 15-5-0 to 34-8-0, Exterior(2R) 34-8-0 to 40-10-2, Interior(1) 40-10-2 to 45-2-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=295, 14=593, 11=366.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regsu PE No.58126 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

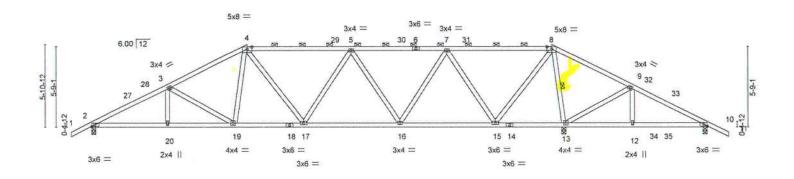
October 6,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the flabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



Job	Truss	T	russ Type	Q	Oty	Ply	GIEBEIG C	CONST LOT 2 CW		9365A4153A6655,760
3702918	T05	l _H	IP	1		1				T31766146
							Job Referer	nce (optional)		
Builders FirstSource	e (Lake City,FL),	Lake City, FL - 320	55,					Tek Industries, Inc. Thu		
				ID:A600Z2i	ikebN	8vXHTJEo 3	2QzWkY3-Rf	C?PsB70Hq3NSgPqnL8v	v3ulTXbGKWrCE	Doi7J4zJC?f
-1-6-0,	5-3-13	11-0-0	18-5-3	25-2-13		32	-8-0	38-4-3	43-8-0	45-2-0
F1-6-0 1-6-0	5-3-13	5-8-3	7-5-3	6-9-11		7-	-5-3	5-8-3	5-3-13	1-6-0



	10	5-8-3	10-1-12		15-0-5	1	21-10-0		28-7-11		33-		37-11-13	43-8-0
		5-8-3	4-5-9		4-10-9		6-9-11	1.	6-9-11		4-1	0-9	4-5-9	5-8-3
Plate Offse	ets (X,Y)	[4:0-4-0,0-1-1	5], [8:0-4-0,0)-1-15]										
LOADING	(psf)	SPAC	ING-	2-0-0		CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate 0	Grip DOL	1.25		TC	0.72	Vert(LL)	-0.10	17	>999	240	MT20	244/190
TCDL	7.0	Lumbe	er DOL	1.25		BC	0.54	Vert(CT)	-0.21	16-17	>999	180		
BCLL	0.0	Rep St	tress Incr	YES		WB	0.96	Horz(CT)	0.06	13	n/a	n/a		
BCDL	10.0	Code	FBC2020/TP	12014		Matri	x-MS						Weight: 2	34 lb FT = 20%

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.2 2x4 SP No 2

2x4 SP No.3 WEBS

BRACING-TOP CHORD

WEBS

Structural wood sheathing directly applied or 3-11-14 oc purlins,

except 2-0-0 oc purlins (3-7-2 max.): 4-8. **BOT CHORD**

Rigid ceiling directly applied or 6-0-0 oc bracing. 8-13 1 Row at midpt

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 13=0-3-8, 10=0-3-8 Max Horz 2=-91(LC 13)

Max Uplift 2=-302(LC 12), 13=-508(LC 8), 10=-263(LC 23) Max Grav 2=1185(LC 23), 13=2244(LC 1), 10=61(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-2048/498, 3-4=-1676/437, 4-5=-1467/398, 5-7=-1176/317, 8-9=-238/1131,

9-10=-177/793

2-20=-460/1782, 19-20=-460/1782, 17-19=-302/1400, 16-17=-331/1449, 15-16=-190/814, 13-15=-659/281, 12-13=-689/186, 10-12=-689/186

3-19=-388/171, 4-19=-60/326, 5-16=-527/218, 7-16=-167/698, 7-15=-1174/341, WEBS

8-15=-308/1374, 8-13=-1927/439, 9-13=-502/497, 9-12=-268/223

NOTES-

Unbalanced roof live loads have been considered for this design.

- 3) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 11-0-0, Exterior(2R) 11-0-0 to 17-2-2, Interior(1) 17-2-2 to 32-8-0, Exterior(2R) 32-8-0 to 38-10-2, Interior(1) 38-10-2 to 45-2-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=302, 13=508, 10=263. 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

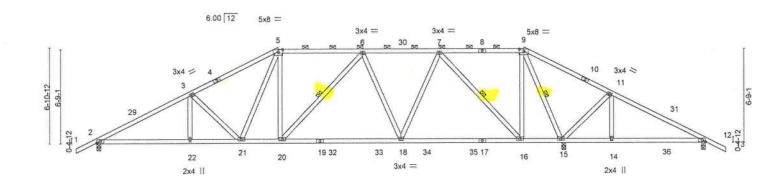
Philip J. O'Regnu PE No.58126 MiTek Inc. DBA MiTek USA FL Cert 6634 16625 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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Job	Truss		Truss Type			Qt	y Ply	GIEB	EIG CONST LOT 2 CW	OT 2 CW			
3702918	T06		HIP			1		1			T317661		
3/02310	100								eference (optional)				
Builders FirstSource	e (Lake City,FL),	Lake City, FL - 3	32055,			ID:A600Z2ik			23 MiTek Industries, Inc. 7 Y3-RfC?PsB70Hq3NSgPqr				
-1-6-0	6-8-3	, 13-	-0-0	19-0-14	1	24-7-2	30	8-0	36-11-13	43-8-0	45-2-0		
1-6-0	6-8-3	6-3	1-13	6-0-14	10	5-6-5	6-0	-14	6-3-13	6-8-3	1-6-0		



		6-8-3	10-1-12	13-0-0		21-10-0	1	30-	8-0		33-6-4	36-11-13	43-8	-0
		6-8-3	3-5-9	2-10-4		8-10-0		8-1	0-0		2-10-4	3-5-9	6-8-	3
Plate Offse	ets (X,Y)	[5:0-4-0,0-1-15], [9:0-4	1-0,0-1-15											
LOADING	(psf)	SPACING-	2-0-	0	CSI.		DEFL.	in	(loc)	l/defl	L/d		PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.2	5	TC	0.63	Vert(LL)	-0.21	18-20	>999	240		MT20	244/190
TCDL	7.0	Lumber DOL	1.2	5	BC	0.95	Vert(CT)	-0.37	18-20	>999	180			
BCLL	0.0	Rep Stress Inci	YES	S	WB	0.62	Horz(CT)	0.07	15	n/a	n/a			
BCDL	10.0	Code FBC2020	D/TPI2014		Matrix-	MS							Weight: 247 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

(size) 2=0-3-8, 15=0-3-8, 12=0-3-8

Max Horz 2=106(LC 12) Max Uplift 2=-305(LC 12), 15=-420(LC 8), 12=-137(LC 23) Max Grav 2=1301(LC 25), 15=2292(LC 2), 12=178(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2235/494, 3-5=-1854/461, 5-6=-1475/396, 6-7=-1266/284, 9-11=-140/921,

11-12=-128/584

BOT CHORD 2-22=-457/1950, 21-22=-457/1950, 20-21=-261/1469, 18-20=-254/1412, 16-18=-169/951,

14-15=-494/147, 12-14=-494/147

3-21=-487/198, 5-21=-138/360, 5-20=-37/310, 6-18=-430/204, 7-18=-144/803, WEBS

7-16=-1366/317, 9-16=-186/1274, 9-15=-2047/344, 11-15=-563/495, 11-14=-289/263

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl.. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 13-0-0, Exterior(2R) 13-0-0 to 19-0-14, Interior(1) 19-0-14 to 30-8-0, Exterior(2R) 30-8-0 to 36-11-13, Interior(1) 36-11-13 to 45-2-0 zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
- All plates are 3x6 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=305 15=420 12=137
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Structural wood sheathing directly applied or 3-7-2 oc purlins, except

6-20, 7-16, 9-15

2-0-0 oc purlins (4-5-7 max.): 5-9.

1 Row at midpt

Rigid ceiling directly applied or 2-2-0 oc bracing.

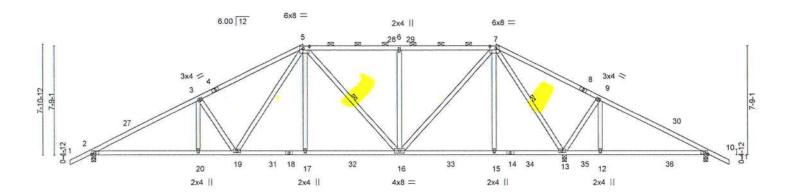
Philip J. O'Regau PE No. 58126 MiTek lac, DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd, Chesterfield, MO 63017

October 6,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters and receive the specific parameters and receive the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property demange. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)



Job	Truss		Truss Type	1			Qty	Ply	GIEBEIG CONST LOT 2 C	W	Exercises (Constitution of the Constitution of
											T3176614
3702918	T07		HIP				1	1	Job Reference (optional)		
Builders FirstSource (Lake City EL)	Lake City, F	1 - 32055				3		g 30 2023 MiTek Industries, Inc	Thu Oct 5 10:49:10	2023 Page 1
bullders Firstgodice (Lake Gity, I L),	Lake City, I	L - J2000,			ID:A600Z			2QzWkY3-RfC?PsB70Hq3NSq		
r1-6-0,	7-7-2	1	15-0-0	1	21-10-0	4	28-8-0		36-0-14	43-8-0	45-2-Q
1-6-0	7-7-2		7-4-14		6-10-0	,	6-10-0		7-4-14	7-7-2	1-6-0



	1	7-7-2	10-1-12	15-0-0		21-10-0		28-8-0		33-	6-4	36-0-14	43-8-0	
		7-7-2	2-6-10	4-10-4		6-10-0		6-10-0		4-1	0-4	2-6-10	7-7-2	
Plate Offse	ets (X,Y)	[5:0-5-4,0-2-0], [7:0-5-	4.0-2-0]											-
LOADING	(psf)	SPACING-	2-0-0		CSI.		DEFL.	in	(loc)	l/defl	L/d		PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25		TC	0.73	Vert(LL)	0.16 1	2-26	>751	240	1	MT20	244/190
TCDL	7.0	Lumber DOL	1.25		BC	0.77	Vert(CT)	-0.25 2	0-23	>999	180			
BCLL	0.0	Rep Stress Incr	YES		WB	0.84	Horz(CT)	0.07	13	n/a	n/a	1		
BCDL	10.0	Code FBC2020	D/TPI2014		Matri	x-MS							Weight: 251 lb	FT = 20%

LUMBER-

TOP CHORD BOT CHORD 2x4 SP No.2 2x4 SP No.2

WEBS

2x4 SP No.3

BRACING-

TOP CHORD

BOT CHORD

WEBS

Structural wood sheathing directly applied or 3-3-10 oc purlins,

except

2-0-0 oc purlins (4-9-9 max.): 5-7.

Rigid ceiling directly applied or 6-0-0 oc bracing 1 Row at midpt 5-16, 7-13

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 13=0-3-8, 10=0-3-8

Max Horz 2=-121(LC 13)

Max Uplift 2=-307(LC 12), 13=-349(LC 13), 10=-111(LC 13) Max Grav 2=1360(LC 2), 13=2198(LC 2), 10=280(LC 24)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES. 2-3=-2322/492, 3-5=-2009/496, 5-6=-1234/327, 6-7=-1234/327, 7-9=-75/684,

TOP CHORD 9-10=-100/408

2-20=-462/2023, 19-20=-462/2023, 17-19=-226/1416, 16-17=-226/1422, 15-16=-26/486, 13-15=-27/481, 12-13=-334/125, 10-12=-334/125

WEBS 3-20=0/256, 3-19=-526/230, 5-19=-198/594, 5-17=0/344, 5-16=-332/122, 6-16=-436/210,

7-16=-255/1132, 7-15=0/338, 7-13=-1942/302, 9-13=-594/535, 9-12=-334/276

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 15-0-0, Exterior(2R) 15-0-0 to 21-2-2, Interior(1) 21-2-2 to 28-8-0, Exterior(2R) 28-8-0 to 34-10-2, Interior(1) 34-10-2 to 45-2-0 zone; porch right
- exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=307, 13=349, 10=111.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

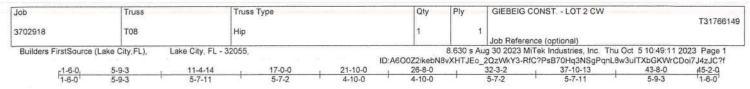
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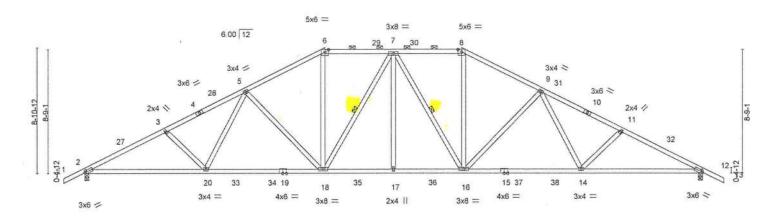
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	1	8-6-13		17-0-0	1	21-10-0	26-8	-0	1	35-1	-3	43-8-0	
		8-6-13		8-5-3		4-10-0	4-10	1-0	1	8-5	-3	8-6-13	
Plate Offs	sets (X,Y)	[2:0-1-5,0-1-8], [12:0-1-5	,0-1-8]										
LOADING	G (psf)	SPACING-	2-0-0	CSI			DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38		Vert(LL)	-0.33	18-20	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.94		Vert(CT)	-0.56	18-20	>940	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.69		Horz(CT)	0.17	12	n/a	n/a	QUIDO - VALUES 60-00-1	
BCDL	10.0	Code FBC2020/T	PI2014	Mat	rix-MS							Weight: 259 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

except

1 Row at midpt

2-0-0 oc purlins (3-9-5 max.): 6-8.

Rigid ceiling directly applied or 2-2-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.1 *Except*

15-19: 2x4 SP No.2

2x4 SP No.3 WEBS

REACTIONS. (size) 2=0-3-8, 12=0-3-8

Max Horz 2=136(LC 16)

Max Uplift 2=-376(LC 12), 12=-376(LC 13) Max Grav 2=1869(LC 2), 12=1869(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3463/661, 3-5=-3290/627, 5-6=-2583/548, 6-7=-2254/520, 7-8=-2254/520,

8-9=-2583/548, 9-11=-3290/627, 11-12=-3463/661

BOT CHORD 2-20=-644/3054, 18-20=-485/2657, 17-18=-295/2384, 16-17=-295/2384, 14-16=-392/2657,

12-14=-509/3054

WEBS 3-20=-275/175, 5-20=-79/545, 5-18=-572/243, 6-18=-141/918, 7-18=-382/136,

7-16=-382/136, 8-16=-141/918, 9-16=-572/243, 9-14=-80/545, 11-14=-275/175

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 17-0-0, Exterior(2R) 17-0-0 to 23-2-2, Interior(1) 23-2-2 to 26-8-0, Exterior(2R) 26-8-0 to 32-10-2, Interior(1) 32-10-2 to 45-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=376, 12=376
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Structural wood sheathing directly applied or 2-10-15 oc purlins,

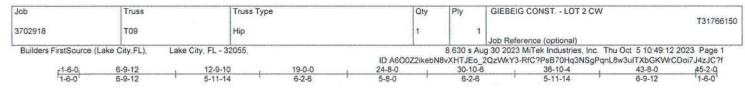
7-18, 7-16

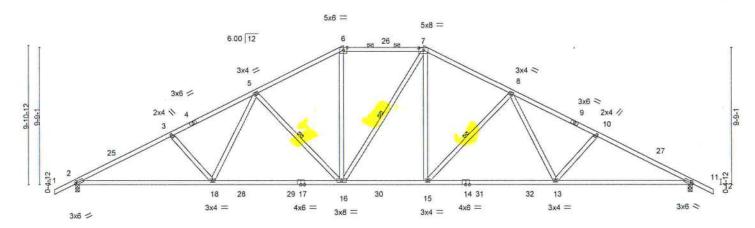
Philip J, O'Regan PE No.38126 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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		9-8-5	- 1	19-0-0		24-8-0		3	3-11-11	-	43-8-0	1
		9-8-5	1	9-3-11		5-8-0			9-3-11		9-8-5	
Plate Offs	ets (X,Y)	[2:0-1-5,0-1-8], [7:0-4-0,0	-1-15], [11:0-1	-5,0-1-8]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	-0.39	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.95	Vert(CT)	-0.65	13-15	>801	180	1000	
BCLL	0.0	Rep Stress Incr	YES	WB	0.30	Horz(CT)	0.16	11	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MS						Weight: 244 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.1 *Except* BOT CHORD

14-17: 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 11=0-3-8

Max Horz 2=-151(LC 13)

Max Uplift 2=-373(LC 12), 11=-373(LC 13) Max Grav 2=1869(LC 2), 11=1872(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-3412/640, 3-5=-3224/615, 5-6=-2406/529, 6-7=-2092/510, 7-8=-2412/529,

8-10=-3231/615, 10-11=-3419/641

BOT CHORD 2-18=-631/3003, 16-18=-457/2550, 15-16=-222/2097, 13-15=-363/2556, 11-13=-480/3008

WEBS 3-18=-318/197, 5-18=-103/653, 5-16=-659/273, 6-16=-118/777, 7-15=-155/792,

8-15=-660/273, 8-13=-104/653, 10-13=-318/197

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 19-0-0, Exterior(2E) 19-0-0 to 24-8-0, Exterior(2R) 24-8-0 to 30-10-6, Interior(1) 30-10-6 to 45-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=373, 11=373, 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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Structural wood sheathing directly applied or 2-7-0 oc purlins, except

5-16, 7-16, 8-15

2-0-0 oc purlins (3-7-7 max.): 6-7

1 Row at midpt

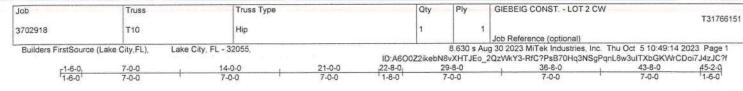
Rigid ceiling directly applied or 2-2-0 oc bracing.

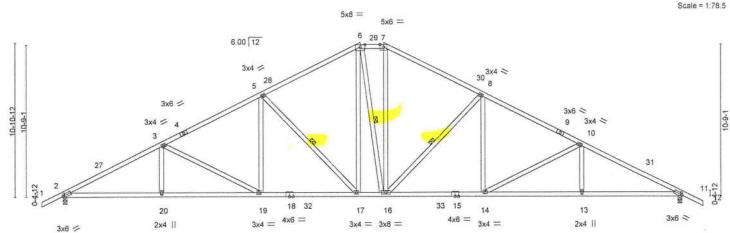
Phillp J. O'Regos PE No.38126 MiTek Inc. DBA MITek USA: FL Cert 6634 16025 Sningley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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P. Contraction	7-0-0	14-0-0	- I	21-0-0	122-8-01	29-0-0			30-0-0	43-0-0	1	
	7-0-0	7-0-0		7-0-0	1-8-0	7-0-0			7-0-0	7-0-0	1	
sets (X,Y)	[2:0-1-5,0-1-8], [6:0-4-0,0)-1-15], [11:0-1-	-5,0-1-8]									
G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (oc) I/d	lefl	L/d	PLATES	GRIP	
20.0	Plate Grip DOL	1.25	TC	0.61	Vert(LL)	-0.30 17	-19 >9	99	240	MT20	244/190	
7.0	Lumber DOL	1.25	BC	0.95	Vert(CT)	-0.53 17	-19 >9	98	180			
0.0 *	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.19	11	n/a	n/a			
10.0	Code FBC2020/T	PI2014	Matri	k-MS	000000000000000000000000000000000000000					Weight: 269 lb	FT = 20%	
	G (psf) 20.0 7.0 0.0	7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0 G (psf) SPACING- 20.0 Plate Grip DOL 1.0 Lumber DOL 1.0 Rep Stress Incr	7-0-0 7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0-1-15], [11:0-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	7-0-0 7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0-1-15], [11:0-1-5,0-1-8] G (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.25 TC 7.0 Lumber DOL 1.25 BC 0.0 Rep Stress Incr YES WB	7-0-0 7-0-0 7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0-1-15], [11:0-1-5,0-1-8] G (psf) SPACING- 2-0-0 CSI. 20.0 Plate Grip DOL 1.25 TC 0.61 7.0 Lumber DOL 1.25 BC 0.95 0.0 * Rep Stress Incr YES WB 0.70	7-0-0 7-0-0 7-0-0 1-8-0	7-0-0 7-0-0	7-0-0 7-0-0	7-0-0 7-0-0 7-0-0 1-8-0 7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0-1-15], [11:0-1-5,0-1-8] G (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl 20.0 Plate Grip DOL 1.25 TC 0.61 Vert(LL) -0.30 17-19 >999 7.0 Lumber DOL 1.25 BC 0.95 Vert(CT) -0.53 17-19 >998 0.0 * Rep Stress Incr YES WB 0.70 Horz(CT) 0.19 11 n/a	7-0-0 7-0-0 7-0-0 7-0-0 7-0-0 7-0-0 7-0-0 sets (X,Y) [2:0-1-5,0-1-8], [6:0-4-0,0-1-15], [11:0-1-5,0-1-8] G (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d 20.0 Plate Grip DOL 1.25 TC 0.61 Vert(LL) -0.30 17-19 >999 240 7.0 Lumber DOL 1.25 BC 0.95 Vert(CT) -0.53 17-19 >998 180 0.0 * Rep Stress Incr YES WB 0.70 Horz(CT) 0.19 11 n/a n/a	7-0-0 7-0 7	7-0-0 7-0-0

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING-

TOP CHORD Structural wood sheathing directly applied or 2-8-14 oc purlins,

except

2-0-0 oc purlins (4-1-8 max.): 6-7.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing. WEBS 1 Row at midpt 5-17, 6-16, 8-16

REACTIONS.

TOP CHORD

(size) 2=0-3-8, 11=0-3-8 Max Horz 2=-166(LC 13)

Max Uplift 2=-369(LC 12), 11=-369(LC 13) Max Grav 2=1838(LC 2), 11=1838(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3391/625, 3-5=-2810/537, 5-6=-2169/492, 6-7=-1873/475, 7-8=-2172/492, 8-10=-2808/536, 10-11=-3392/625

2-20=-631/2978, 19-20=-631/2978, 17-19=-430/2455, 16-17=-221/1869, 14-16=-312/2453, **BOT CHORD** 13-14=-465/2979, 11-13=-465/2979

3-20=0/274, 3-19=-596/226, 5-19=-59/568, 5-17=-831/296, 6-17=-183/738 6-16=-228/255, 7-16=-162/740, 8-16=-825/295, 8-14=-58/564, 10-14=-598/227,

10-13=0/275

NOTES-

WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 21-0-0, Exterior(2E) 21-0-0 to 22-8-0, Exterior(2R) 22-8-0 to 28-10-2, Interior(1) 28-10-2 to 45-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=369, 11=369,
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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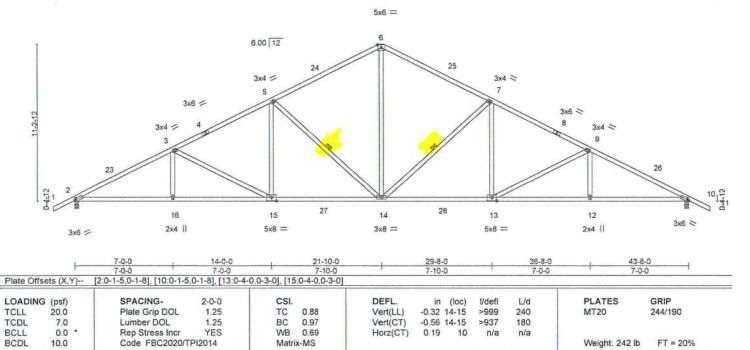
Philip J. O'Regau PE No.38126 MITek Inc. DBA MITek USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017

October 6,2023

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Job	Truss	Truss Type		Qty	Ply	GIEBEIG CONST LOT 2 CW	1	2279029030000000000000000000000000000000
- Constitution of the Cons	1			1000		. 13		T31766152
3702918	T11	Common		11		1		
						Job Reference (optional)		
Builders FirstSource (La	ske City,FL),	Lake City, FL - 32055,			8.630 s A	Aug 30 2023 MiTek Industries, Inc.	Thu Oct 5 10:49:15 2	2023 Page 1
				ID:A600Z2ikebN	18vXHTJEo	2QzWkY3-RfC?PsB70Hq3NSgPc	gnL8w3uITXbGKWrCi	Doi7J4zJC?f
r1-6-0 _i	7-0-0	14-0-0	21-10-0	1	29-8-0	36-8-0	43-8-0	45-2-0
1-6-0	7-0-0	7-0-0	7-10-0		7-10-0	7-0-0	7-0-0	1-6-0



LUMBER-

2x4 SP No.2 TOP CHORD BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-TOP CHORD

BOT CHORD WEBS

Structural wood sheathing directly applied Rigid ceiling directly applied or 2-2-0 oc bracing. 1 Row at midpt 7-14, 5-14

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 10=0-3-8 Max Horz 2=-173(LC 17) Max Uplift 2=-368(LC 12), 10=-368(LC 13)

Max Grav 2=1845(LC 2), 10=1845(LC 2)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3404/620, 3-5=-2828/548, 5-6=-2117/492, 6-7=-2117/492, 7-9=-2828/548, TOP CHORD

9-10=-3404/621

2-16=-632/2989, 15-16=-632/2989, 14-15=-437/2473, 13-14=-315/2473, 12-13=-460/2989, 10-12=-460/2989

6-14=-246/1484, 7-14=-887/319, 7-13=-54/572, 9-13=-586/221, 9-12=0/272, WEBS

5-14=-887/319, 5-15=-54/572, 3-15=-586/220, 3-16=0/272

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-10-6, Interior(1) 2-10-6 to 21-10-0, Exterior(2R) 21-10-0 to 26-2-6, Interior(1) 26-2-6 to 45-2-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=368, 10=368.

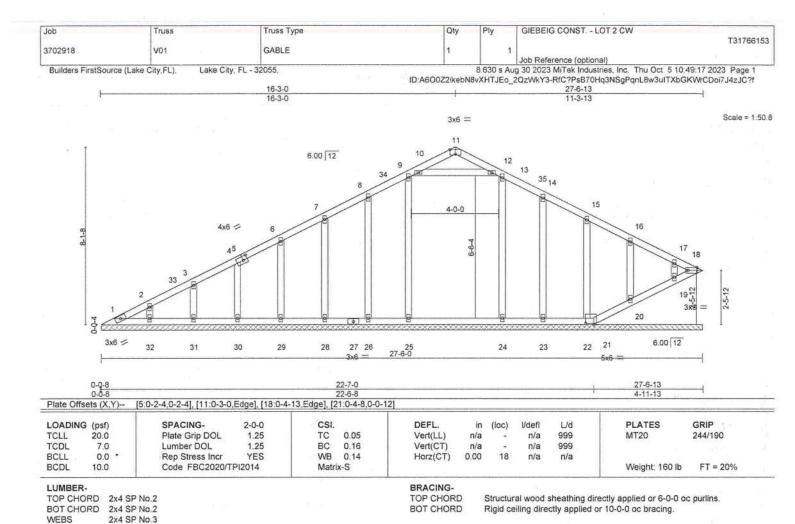
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Philip J. O'Regna PE No.58126 MITch Inc. DBA MITch USA FL Cert 6634 16025 Swingley Ridge Rd. Chesterfield, MO 63017

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REACTIONS.

2x4 SP No.3

All bearings 27-6-0. Max Horz 1=162(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 18, 21, 25, 24, 26, 28, 29, 30, 31, 32, 23, 22, 20, 19 All reactions 250 lb or less at joint(s) 18, 1, 21, 26, 28, 29, 30, 31, 32, 23, 22, 20, 19 except 25=311(LC 19), 24=305(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

OTHERS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-7-7 to 3-7-7, Interior(1) 3-7-7 to 16-3-0, Exterior(2R) 16-3-0 to 19-3-0, Interior(1) 19-3-0 to 27-2-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 21, 25, 24, 26, 28, 29, 30, 31, 32, 23, 22, 20, 19,

9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18, 20, 19.

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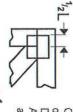
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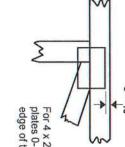


Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss offsets are indicated. Center plate on joint unless x, y and fully embed teeth. Dimensions are in ft-in-sixteenths.



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

the length parallel to slots. to slots. Second dimension is width measured perpendicular The first dimension is the plate

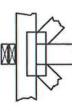
4 × 4

LATERAL BRACING LOCATION



if indicated. output. Use T or I bracing Indicated by symbol shown and/or by text in the bracing section of the

BEARING



number/letter where bearings occur reaction section indicates joint (supports) occur. Icons vary but Min size shown is for crushing only Indicates location where bearings

Industry Standards:

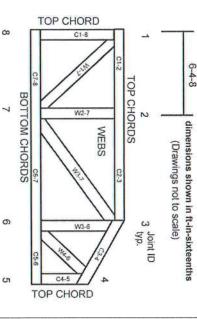
National Design Specification for Metal Plate Connected Wood Truss Construction

Building Component Safety Information, Design Standard for Bracing.

Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses

DSB-22 ANSI/TPI1:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values established by others. Lumber design values are in accordance with ANSI/TPI 1

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MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves bracing should be considered. may require bracing, or alternative Tor I
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

ω

Provide copies of this truss design to the building designer, erection supervisor, properly owner and all other interested parties.

4

Cut members to bear tightly against each other.

5

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.

9

- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Boltom chords require lateral bracing at 10 ft, spacing. or less, if no ceiling is installed, unless otherwise noted
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use. environmental, health or performance risks. Consult with
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2718981 - DETAILS

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: DETAILS Project Name: N/A Model: N/A

Lot/Block: N/A

Subdivision: N/A

Address: N/A, N/A

City: N/A

State: N/A

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16

Wind Speed: 130 mph

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 20 individual, General Truss Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Detail Name	Date	No.	Seal#	Detail Name	Date
1 2	T23399806 T23399807	MII-REP10 MII-T-BRACE 2	4/2/21	15 16	T23399820 T23399821	MII-VALLEY HIGH WIND2 MII-VALLEY SP	4/2/21
3	T23399808 T23399809	MII-SCAB-BRACE MII-REP05	4/2/21	17 18	T23399822 T23399823	MII-VALLEY SP MII-GE146-001	4/2/21
5 6	T23399810 T23399811	MII-GE130-D-SP MII-GE130-SP	4/2/21	19 20	T23399824 T23399825	MII-REP13B MII-STRGBCK	4/2/21 4/2/21
7	T23399812 T23399813	MII-GE140-001 MII-GE170-D-SP	4/2/21	ST656			25,000,000
8 9 10 11	T23399814 T23399815	MII-GE180-D-SP MII-GE180-D-SP	4/2/21				
11	T23399816 T23399817	MII-PIGGY-ALT-7-16 MII-REP01A1	4/2/21				
12 13 14	T23399818 T23399819	MII-TOENAIL SP MII-VALLEY FIIGH WIND1	4/2/21				
14	123333013	WIII-VALLE I TIIOTI WIND I	412121				

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature.

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The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: ORegan, Philip My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



RE: \$JOBNAME - \$JOBDESC

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: \$SI_CUSTOMER Project Name: \$SI_JOBNAME Model: \$SI_MODEL

ot/Block: \$SI_LOTNUM Subdivision: \$SI_SUBDIV

Lot/Block: \$SI_LOTNUM Address: \$SI_SITEADDR City: \$SI_SITECITY

ity: \$SI_SITECITY State: \$SI_SITESTATE

STANDARD REPAIR FOR ADDING MII-REP10 OCTOBER 28, 2016 A FALSE BOTTOM CHORD T23399806 (R) MiTek USA, Inc. Page 1 of 1 MAIN TRUSS MANUFACTURED WITHOUT FALSE BOTTOM CHORD. MAIN TRUSS (SPACING = 24" O.C.) MiTek USA, Inc. REFER TO THE BOTTOM CHORD BRACING SECTION OF THE INDIVIDUAL TRUSS DESIGN FOR MAXIMUM SPACING OF CONTINUOUS LATERAL BRACING WHENEVER RIGID CEILING MATERIAL IS NOT DIRECTLY ATTACHED TO THE VERTICAL STUDS @ 48" O.C.. ATTACHED WITH (3) - 10d (0.131" X 3") NAILS AT EACH END OF VERTICAL (TYP.). BOTTOM CHORD. VERTICAL STUDS TO BE 2 x 4 STUD GRADE (OR BETTER) SPF, HF, DF OR SP (BOARD SIZÉ SPECIFIED IS MINIMUM, LARGER SIZE MAY BE USED) 2 x 4 NO. 2 (OR BETTER) SPF, HF, DF OR SP FALSE BOTTOM CHORD

TRUSS SPAN

NOTES:

 LOADING: TOP CHORD: (REFER TO THE MAIN TRUSS DESIGN FOR TOP CHORD LOADING). BOTTOM CHORD: LL = 0 PSF, DL = 10 PSF.

(BOARD SIZE SPECIFIED IS MINIMUM, LARGER SIZE MAY BE USED) FALSE BOTTOM

- 2. REFER TO THE MAIN TRUSS DESIGN FOR LUMBER AND PLATING REQUIREMENTS.
- 3. MAXIMUM BOTTOM CHORD PITCH = 6/12.
- 4. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 5. FALSE BOTTOM CHORD ONLY DESIGNED TO CARRY VERTICAL LOAD. NO LATERAL (SHEAR) LOAD ALLOWED.
- 6. FILLER MAY EXTEND FOR FULL LENGTH OF TRUSS.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126

MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and fruss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



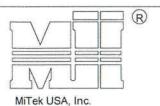
AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2 T23399807

MiTek USA, Inc.

Inc. Page 1 of 1



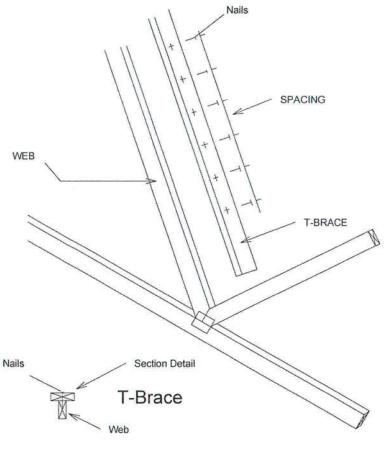
Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

1	Nailing Pattern	
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

	100000000000000000000000000000000000000	e Size -Ply Truss
		Continuous iteral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace



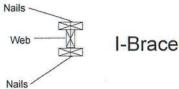
	AV	ce Size -Ply Truss
		Continuous ateral Bracing
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-99 and BCSI Building Component Safety Information

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501



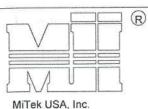
AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE T23399808

MiTek USA, Inc.

Page 1 of 1



Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x SCAB TO ONE FACE OF WEB WITH
2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
SCAB MUST BE THE SAME GRADE, SIZE AND
SPECIES (OR BETTER) AS THE WEB.

MAXIMUM WEB AXIAL FORCE = 2500 lbs
MAXIMUM WEB LENGTH = 12'-0"
2x4 MINIMUM WEB SIZE
MINIMUM WEB GRADE OF #3

Nails Section Detail

Scab-Brace
Web

Scab-Brace must be same species grade (or better) as web member.

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Philip J. O'Regan PE No.58128 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33619 Date:

April 2,2021

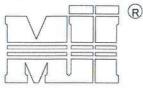


AUGUST 1, 2016

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05 T23399809

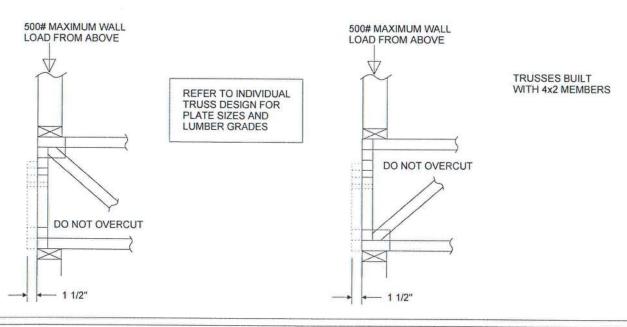
MiTek USA, Inc. Page 1 of 1

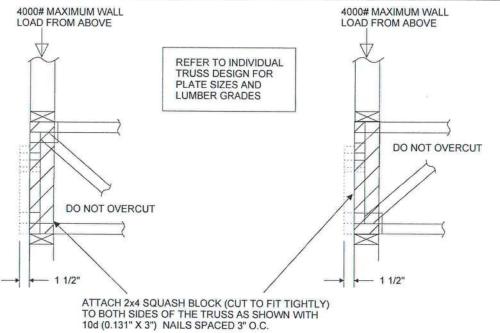


MiTek USA, Inc.

- THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
- THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.
- CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





TRUSSES BUILT WITH 4x2 MEMBERS

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd, Tampa FL 33610

April 2,2021

MRNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

Ansirtpi1 Quality Criteria, DSB-39 and BCSI Building Component Safety Information available from Truss Plate Institute, 2870 Crain Highway, Suite 203 Waldorf, MD 20601



MII-GE130-D-SP Standard Gable End Detail APRIL 12, 2019 T23399810 R MiTek USA, Inc. Typical _x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud Vertical Stud (4) - 16d Nails MiTek USA, Inc. SECTION B-B (2) - 10d Nails into 2x6 DIAGONAL BRACE 4'-0" O.C. MAX TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A PROVIDE 2x4 BLOCKING BETWEEN THE FIRST Varies to Common Truss TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. SEE INDIVIDUAL MITEK ENGINEERING ATTACH DIAGONAL BRACE TO BLOCKING WITH DRAWINGS FOR DESIGN CRITERIA (5) - 10d NAILS ** (4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK 3x4 = Roof Sheathing - Diagonal Bracing L-Bracing Refer to Section B-B Refer to Section A-A 24" Max

1'-3"

Max.

Diag. Brace at 1/3 points

End Wall

if needed

- 10d

NAILS

NOTE

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
- ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.
- (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximu	m Stud Lei	ngth	
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D

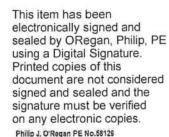
ASCE 7-98 ASCE 7-02 ASCE 7-09 ASCE 7-10, ASCE 7-16 160 MPH

ASCE 7-02 ASCE 7-05 130 MPH

ASCE 7-10 ASCE 7-16 160 MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DURANCONTONE, OARD/INCREASE In 8 OF READ NOTES CONNECTION OF THE ACTION OF T



MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

Page 1 of 2

DIAGONAL

BRACE

-2x6 Stud or 2x4 No.2 of better

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

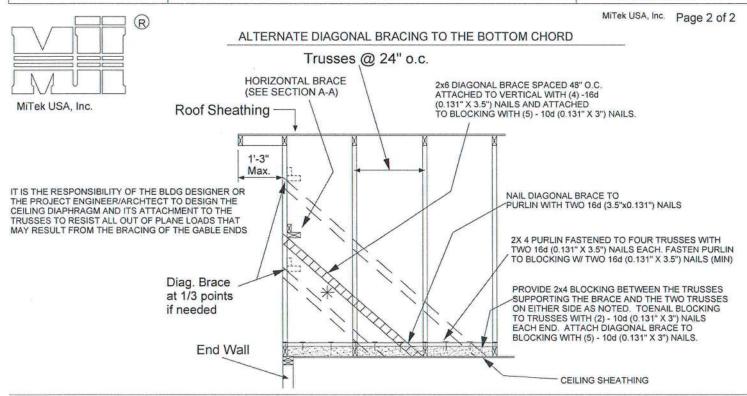
Spaced 6" o.c.



APRIL 12, 2019

Standard Gable End Detail

MII-SHEET 2 T23399810



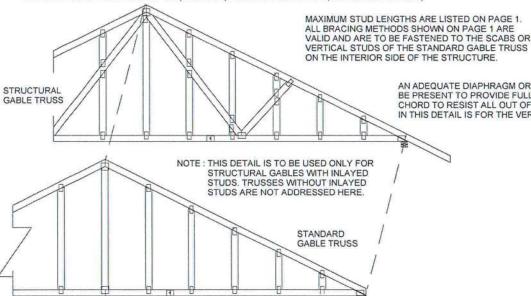
BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

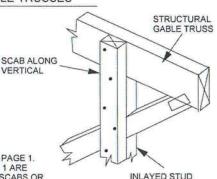
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE. METHOD 2 : ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING

NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

LING SCREDULE.
FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL
MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY

VERTICAL

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2570 Crain Highway, Suite 203 Waldorf, MD 20601



MII-GE130-SP APRIL 12, 2019 Standard Gable End Detail T23399811 R MiTek USA, Inc. Typical _x4 L-Brace Nailed To Verticals W/10d Nails spaced 6" o.c. Vertical Stud Vertical Stud (4) - 16d Nails MiTek USA, Inc. SECTION B-B (2) - 10d Nails into 2x6 DIAGONAL BRACE TRUSS GEOMETRY AND CONDITIONS 4'-0" O.C. MAX SHOWN ARE FOR ILLUSTRATION ONLY. Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A PROVIDE 2x4 BLOCKING BETWEEN THE FIRST Varies to Common Truss TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. SEE INDIVIDUAL MITEK ENGINEERING ATTACH DIAGONAL BRACE TO BLOCKING WITH DRAWINGS FOR DESIGN CRITERIA (5) - 10d NAILS ** (4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD 3x4 = SHEATHING TO 2x4 STD SPF BLOCK Roof Sheathing

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

1'-3"

Max

(2) - 10d

NAILS

NOTE

Diagonal Bracing

Refer to Section A-A

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

**

- L-Bracing Refer

to Section B-B

- CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
- BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
- GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
 THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES
- 10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

 11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND
- NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6			
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1			
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C

EXPOSURE B or C

ASCE 7-02 ASCE 7-05 130 MPH

ASCE 7-10, ASCE 7-05 16 160 MPH

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DUR MONFORE OAD/INCREASE test 60 READ NO

ESONNECTION OF BRACTING'S BASED ON HIMPERSY. 5/19/2020 BEFORE USE.

Design valid for use only with MITEKS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-99 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801

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MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd, Tampa FL 33610

April 2,2021

Page 1 of 2

DIAGONAL

2x4 No.2 of better

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

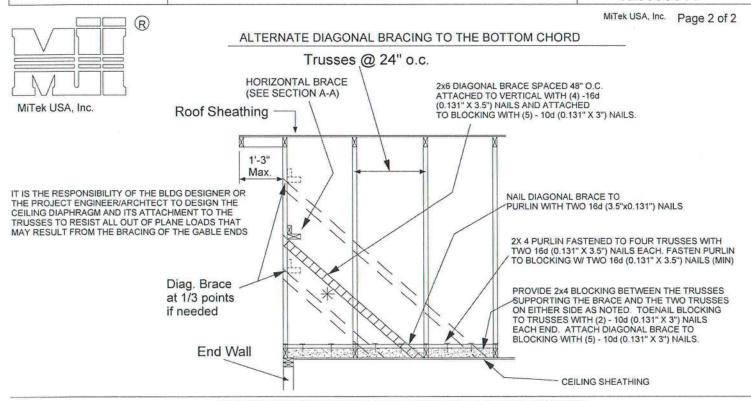
16d Nails Spaced 6" o.c.



APRIL 12, 2019

Standard Gable End Detail

MII-SHEET 2 T23399811

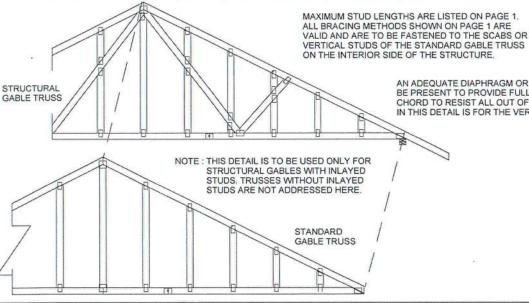


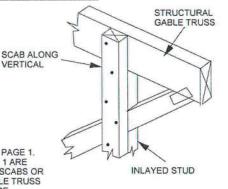
BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE

AND SPECIES AS THE TRUSS VERTICALS NAILING SCHEDULE: - FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL

MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY

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Philip J. O'Regan PE No.58126 MiTak USA, Inc. FL Cert 6834 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2870 Crain Highway. Suite 203 Waldorf, MD 20801



MII-GE140-001 Standard Gable End Detail **JANUARY 6, 2017** T23399812 (R) MiTek USA, Inc. Page 1 of 2 Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud Vertical Stud (4) - 16d Nails MiTek USA, Inc. 16d Nails Spaced 6" o.c. SECTION B-B (2) - 10d Nails into 2x6 2x6 Stud or DIAGONAL BRACE 4'-0" O.C. MAX 2x4 No.2 of better TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING Varies to Common Truss TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** (4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD DF/SPF BLOCK 3x4 = Roof Sheathing Diagonal Bracing - L-Bracing Refer Refer to Section A-A to Section B-B 24" Max 1'-3' 10g NOTE 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT Max. NAILS (2) - 10d NAILS BRACING OF ROOF SYSTEM. Trusses @ 24" o.c. 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x6 DIAGONAL BRACE SPACED 48" O.C. 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. Diag. Brace ATTACHED TO VERTICAL WITH (4) -16d

at 1/3 points

End Wall

if needed

Minimum Stud Size	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Species and Grade			Maximu	m Stud Lei	ngth	
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum

end distance. Brace must cover 90% of diagonal length.

ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05

TYPE TRUSSES.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

DIAGONAL BRACE

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DUR MONFORE OAD MORREASCHIPT. 80 READ COMPLET TRANSPIREMENTS BASED ON COMPONENTS AND CLADDING.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITP11 Quality Criteria, DSB-89 and BCSI Building Compositions

Safety Information**

available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



NAILS AND ATTACHED

TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE

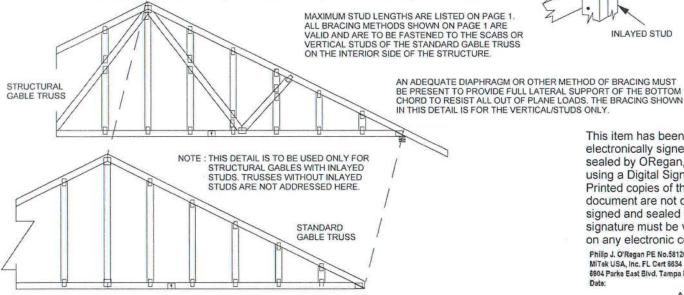
(SEE SECTION A-A)

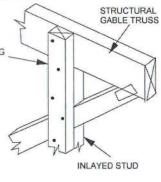
MII-SHEET 2 Standard Gable End Detail APRIL 12, 2019 T23399812 MiTek USA, Inc. Page 2 of 2 (R) ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD Trusses @ 24" o.c. HORIZONTAL BRACE 2x6 DIAGONAL BRACE SPACED 48" O.C. (SEE SECTION A-A) ATTACHED TO VERTICAL WITH (4) -16d (0.131" X 3.5") NAILS AND ATTACHED MiTek USA, Inc. Roof Sheathing TO BLOCKING WITH (5) - 10d (0.131" X 3") NAILS. 1'-3' Max IT IS THE RESPONSIBILITY OF THE BLDG DESIGNER OR NAIL DIAGONAL BRACE TO THE PROJECT ENGINEER/ARCHTECT TO DESIGN THE CEILING DIAPHRAGM AND ITS ATTACHMENT TO THE TRUSSES TO RESIST ALL OUT OF PLANE LOADS THAT PURLIN WITH TWO 16d (3.5"x0.131") NAILS MAY RESULT FROM THE BRACING OF THE GABLE ENDS 2X 4 PURLIN FASTENED TO FOUR TRUSSES WITH TWO 16d (0.131" X 3.5") NAILS EACH. FASTEN PURLIN TO BLOCKING W/ TWO 16d (0.131" X 3.5") NAILS (MIN) Diag. Brace PROVIDE 2x4 BLOCKING BETWEEN THE TRUSSES at 1/3 points SUPPORTING THE BRACE AND THE TWO TRUSSES ON EITHER SIDE AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d (0.131" X 3") NAILS EACH END. ATTACH DIAGONAL BRACE TO if needed BLOCKING WITH (5) - 10d (0.131" X 3") NAILS. End Wall **CEILING SHEATHING** BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES STRUCTURAL GABLE TRUSS STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE SCAB ALONG

FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS NAILING SCHEDULE

ING SCREDULE:
FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL
MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





VERTICAL

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd, Tampa FL 33610

April 2,2021

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ANS/ITPI1 Quality Criteria, DSB-39 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801

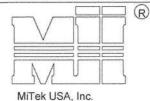


Standard Gable End Detail

MII-GE170-D-SP T23399813

MiTek USA, Inc.

Page 1 of 2



DIAGONAL BRACE

4'-0" O.C. MAX

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA 3x4 = - Diagonal Bracing Refer to Section A-A L-Bracing Refer to Section B-B

Vertical Stud 2X6 SP OR SPF No. 2 (4) - 16d Nails DIAGONAL BRACE 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2X6 SP OR SPF No. 2 Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10g/

NAILS

Roof Sheathing

1'-0"

Max.

d Wall

NOTE

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS

CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4.

(REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

ľ	
	Diag. Brace at 1/3 points if needed
	End Wa

24" Max

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D

DURATION OF LOAD INCREASE: 1.60 READ NOTE CONNECTION OF BRACING IS BASED ON MATERS 5/19/2020 BEFORE USE.

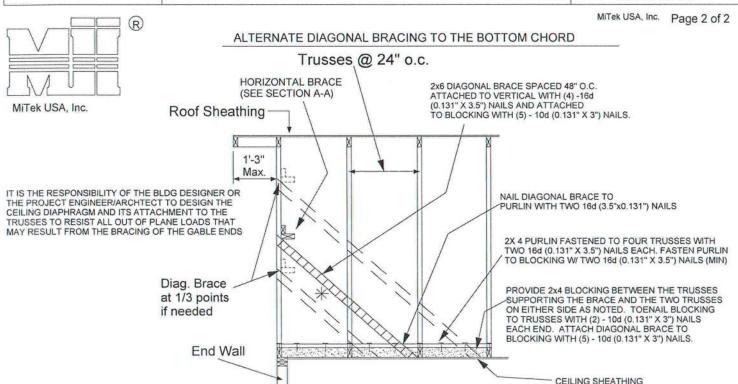
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

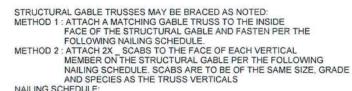


Standard Gable End Detail

MII-SHEET 2 T23399813

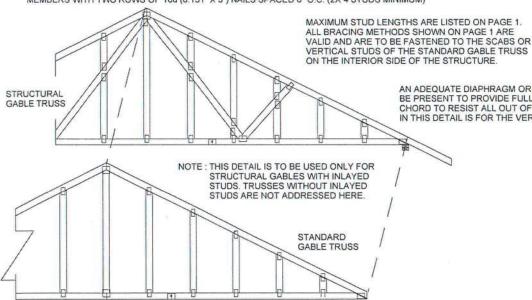


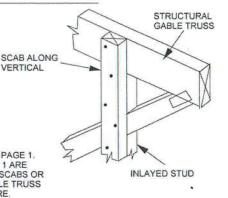
BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES



 FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
 FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

 FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6834 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



APRIL 12, 2019 R MiTek USA, Inc. DIAGONAL BRACE 4'-0" O.C. MAX

Standard Gable End Detail

MII-GE180-D-SP T23399814

MiTek USA, Inc.

Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

2X6 SP OR SPF No. 2

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED

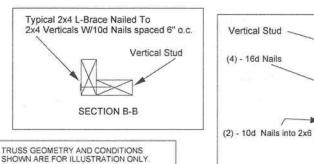
48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

HORIZONTAL BRACE

(SEE SECTION A-A)

Typical Horizontal Brace Nailed To 2x4 Verticals w/(4)-10d Nails

16d Nails Spaced 6" o.c.



24" Max

Diag. Brace at 1/3 points

End Wall

if needed

SECTION A-A Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 =

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

- 10d

NAILS

Roof Sheathing

1'-0"

Max.

- Diagonal Bracing Refer to Section A-A - L-Bracing Refer to Section B-B

NOTE

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT

 BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

Minimum	Stud	Without	2x4	DIAGONAL	2 DIAG
NAILS DESIGN	IATED 16d	ARE (0.131" X	3.5")		
11. NAILS DESIGN					
06-01-13 BY S					
10. SOUTHERN P	NE LUMBE	ER DESIGN VAL	UES ARE THO	SE EFFECTIVE	
TYPE TRUSSE	S.				
9. DO NOT USE F	LAT BOTT	OM CHORD GA	BLES NEXT TO	SCISSOR	
8. THIS DETAIL D	OES NOT	APPLY TO STR	UCTURAL GAE	BLES.	
7. GABLE STUD I		AD STEWNISH DATE OF SUPPOSE SOCIETY ST	XCEEDS L/24	0.	
(REFER TO SE			()		•
				ILS THROUGH 2x4	
				NGEST GABLE ST	ID.
2x4 AS SHOWN					
6. CONSTRUCT H		I BRACE CON	NECTING A 2V	S AND A	
DIAPHRAM AT	4'-0" O C				

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D

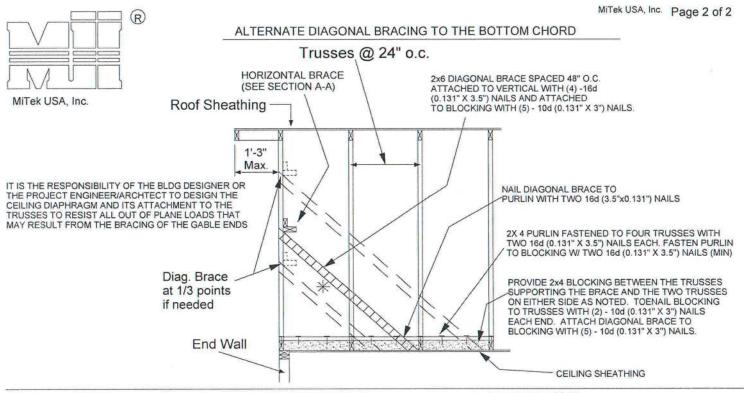
DURATION ORD DAR INCREASE 11 SA READ NOTES CONNECTION LO BERACING IS BASED ON MAIGRS. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING

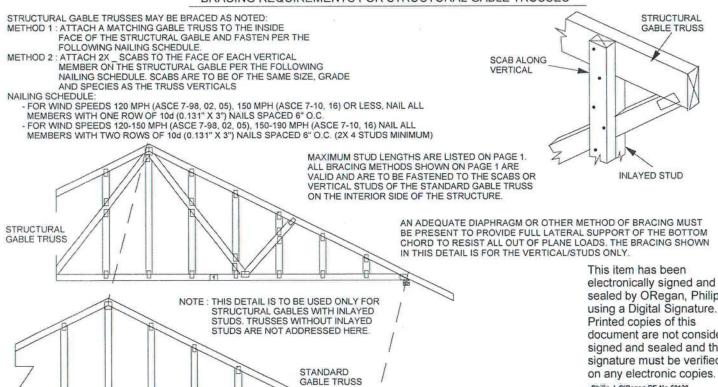


Standard Gable End Detail

MII-SHEET 2 T23399814



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES



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April 2,2021

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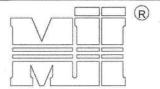
January 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-16 T23399815

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C CATEGORY II BUILDING EXPOSURE B or C **ENCLOSED BUILDING** LOADING = 5 PSF TCDL ASCE 7-10, ASCE 7-16 DURATION OF LOAD INCREASE : 1.60

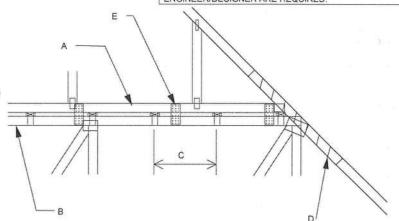
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) (0.131" X 3.5") TOE-NAILED.
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
 D 2 X __ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED
 ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
 IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
 DIRECTIONS AND: DIRECTIONS AND:
- DIRECTIONS AND:

 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
 PIGGYBACK SPAN OF 12 ft.

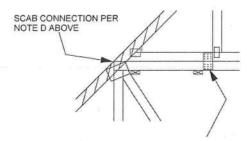
 -FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH
 MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT
 72"O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS
 FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE.



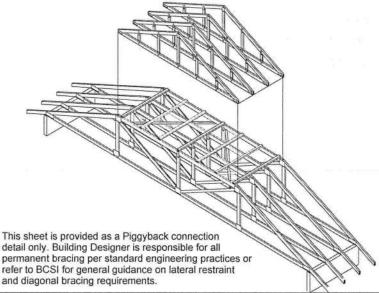
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

(MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

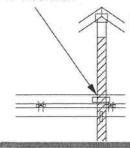
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- AS SHOWN IN DETAIL.

 ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF
 TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
 SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.

5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801

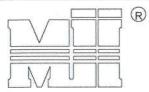


JANUARY 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT-7-16 T23399816

Page 1 of 1 MiTek USA, Inc.

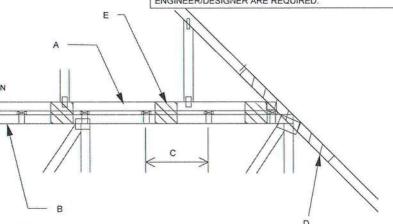


MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ENCLOSED BUILDING LOADING = 5 PSF TCDL MINIMUM ASCE 7-10, ASCE 7-16 DURATION OF LOAD INCREASE: 1.60

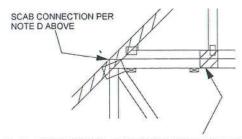
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) 0(0.131" X 3.5") TOE-NAILED.
 BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
- ONLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0. 131" X 3.5") NAILS EACH.
 2 X _ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
 INTERSECTION, WITH (2) ROWS OF (0. 131" X 3") NAILS @ 4" O C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING SCAB WAY BE OWN TEE PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND: 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E FOR WIND SPEED IN THE RANGE 116 MPH 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL 12 NAILS)

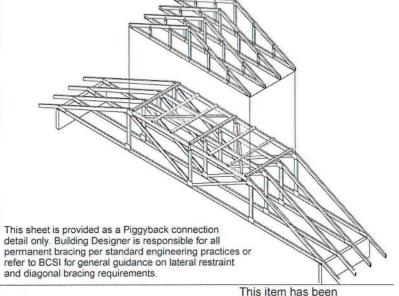


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

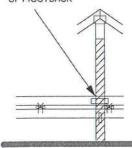
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
- IGGYBACK AND THE BASE TRUSS DESIGN

NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH

on any electronic copies. Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8834 6904 Parke East Blvd. Tampa FL 33610

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1 T23399817

MiTek USA, Inc.

Page 1 of 1

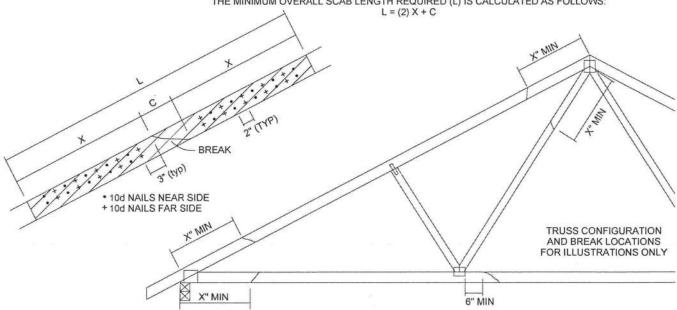


TOTAL NUMBER OF NAILS EACH SIDE OF BREAK * X		MAXIMUM FORCE (lbs) 15% LOAD DURATION								
	X INCHES	SP		DF		SPF		HF		
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED. THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ ORIENTATION ONLY.

6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

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April 2,2021

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MAY 7, 2019

LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP T23399818

MiTek USA, Inc.

Page 1 of 1

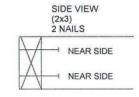


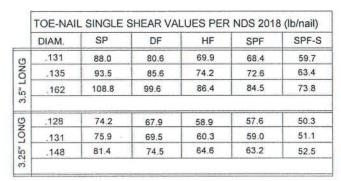
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES.
- FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY





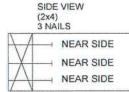
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

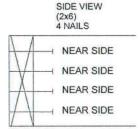
EXAMPLE:

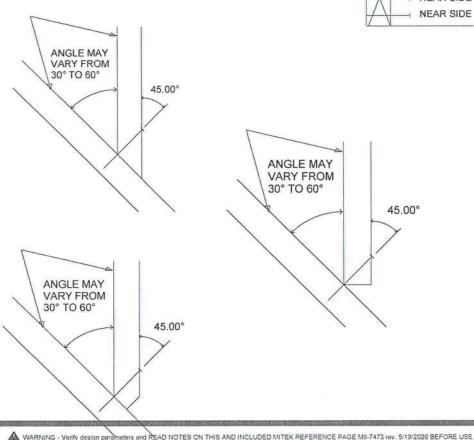
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







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April 2,2021

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ANSI/TP11 Quality Criteria, DSB-39 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501

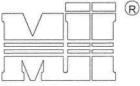


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1 T23399819

MiTek USA, Inc.

Page 1 of 1

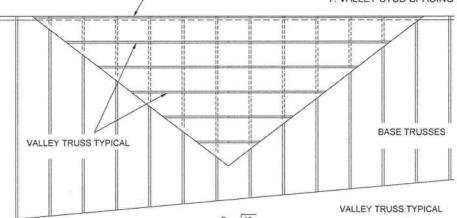


MiTek USA, Inc.

GENERAL SPECIFICATIONS

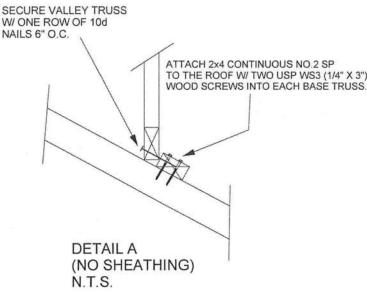
1. NAIL SIZE 10d (0.131" X 3")

- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
- INDIVIDUAL DESIGN DRAWINGS.
 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING. 6. NAILING DONE PER NDS - 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS 112 P SEE DETAIL A BELOW (TYP.)



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF

MAX SPACING = 24" O.C. (BASE AND VALLEY) MAX SPACING = 24" U.U. (BASE AND A SPACING AND A SPACING AND AND A SPACING AND A SPACI

electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies. Philip J. O'Regan PE No.58126

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2570 Crain Highway, Suite 203 Waldorf, MD 20801



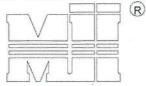
MiTek USA, Inc.

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2 T23399820

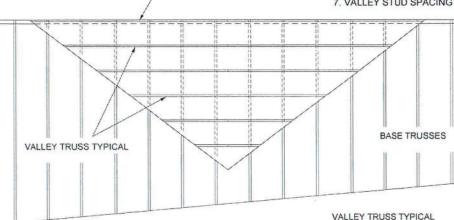
MiTek USA, Inc.

Page 1 of 1



GENERAL SPECIFICATIONS

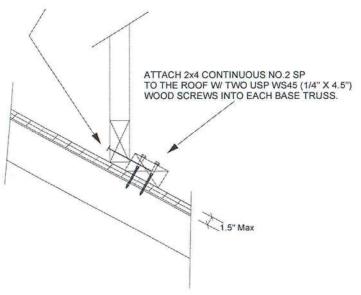
- 1. NAIL SIZE 10d (0.131" X 3")
 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE TO BASE TRUSSES AS PER DETAIL A
- 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR ĢIRDER TRUSS

GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL OR GIRDER TRUSS P 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES
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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610

April 2,2021

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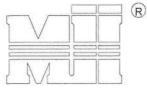


TRUSSED VALLEY SET DETAIL

MII-VALLEY SP T23399821

MiTek USA, Inc.

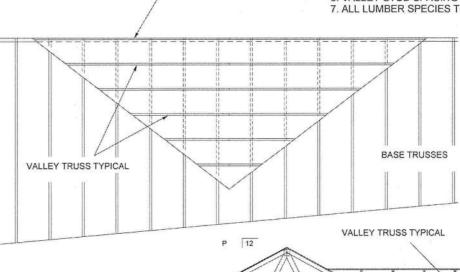
Page 1 of 1



MiTek USA, Inc.

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.

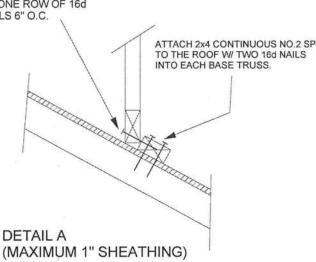


GABLE END, COMMON TRUSS

OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL A BELOW (TYP.) SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF

ON THE TRUSSES

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April 2,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2870 Crain Highway, Suite 203 Waldorf, MD 20801



TRUSSED VALLEY SET DETAIL

MII-VALLEY SP T23399822

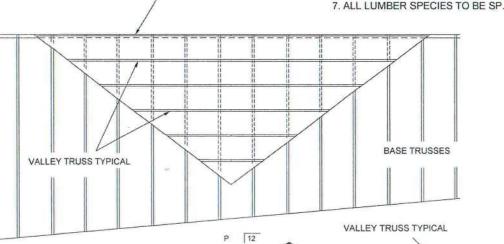
MiTek USA, Inc.

Page 1 of 1



GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5")
- 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS

GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO 16d NAILS INTO EACH BASE TRUSS. THE REAL PROPERTY OF THE PARTY **DETAIL** A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

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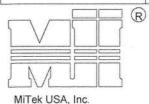
AUGUST 1, 2016

Standard Gable End Detail

MII-GE146-001 T23399823

Page 1 of 2

MiTek USA, Inc.



Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

SEE INDIVIDUAL MITEK ENGINEERING

3x4 =

Varies to Common Truss

**

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer ** to Section B-B

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND
- WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT
- BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF
- DIAPHRAM AT 4'-0" O.C.
- CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

DIAGONAL BRACE

4'-0" O.C. MAX

- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
 DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES.
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
	4	Maximum Stud Length					
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DRAWINGS FOR DESIGN CRITERIA (4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK Roof Sheathing 24" Max 1'-3" (2) - 10d Max NAILS (2) - 10d NAILS Trusses @ 24" o.c. 2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d Diag. Brace at 1/3 points NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS. if needed End Wall HORIZONTAL BRACE (SEE SECTION A-A)

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April 2,2021

EXPOSURE B or C

ASCE 7-98, ASCE 7-02, ASCE 7-05

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.

DURANGENERASE of 60 READ GONNECTION/OF BRACENGTIS BASED ON TOMPONENTS AND CLADDING.

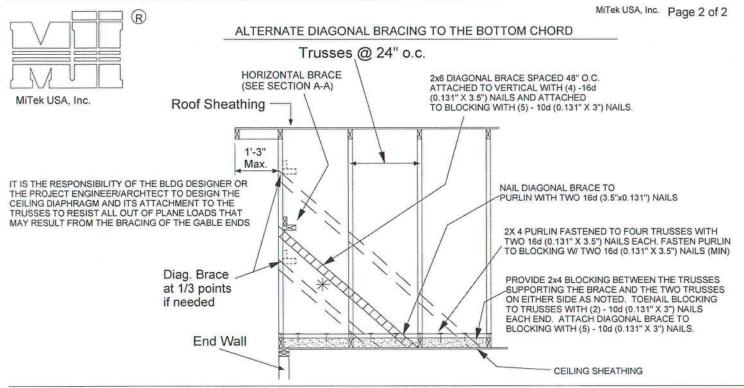
Design valid for use only with MTeks connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss we bandlor chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/PTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



Standard Gable End Detail

MII-SHEET 2 T23399823



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL

MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE. GRADE AND SPECIES AS THE TRUSS VERTICALS

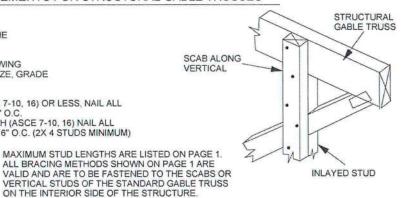
NAILING SCHEDULE:

STRUCTURAL

GABLE TRUSS

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.



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April 2,2021

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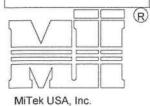
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B T23399824

MiTek USA, Inc.

Page 1 of 1



TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

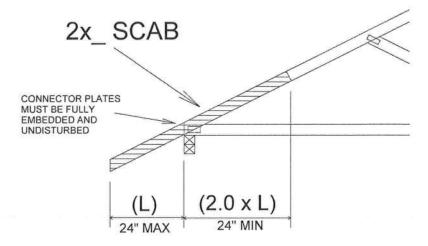
END BEARING CONDITION

NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 8634 6904 Parke East Blvd. Tampa FL 33610 Date:

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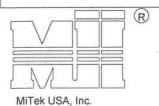


LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK T23399825

MiTek USA, Inc.

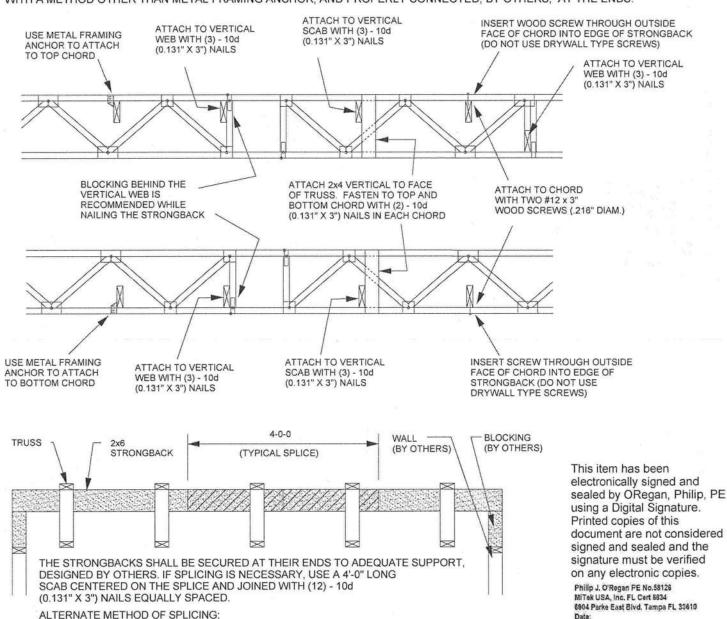
Page 1 of 1



TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



April 2,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2029 BEFORE USE.

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OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d

O BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL

(0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED.

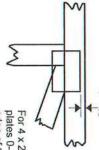


Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss offsets are indicated. and fully embed teeth. Dimensions are in ft-in-sixteenths Center plate on joint unless x, y



plates 0- 1/16" from outside For 4 x 2 orientation, locate edge of truss.

8

0

C

6 5

required direction of slots in connector plates This symbol indicates the

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

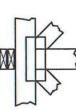
the length parallel to slots. to slots. Second dimension is width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



if indicated. Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing

BEARING



Min size shown is for crushing only reaction section indicates joint number where bearings occur. (supports) occur. Icons vary but Indicates location where bearings

Industry Standards:

National Design Specification for Metal

DSB-89

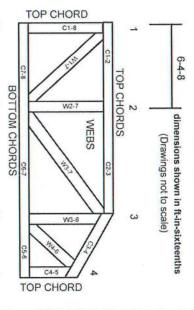
Guide to Good Practice for Handling, **Building Component Safety Information** Design Standard for Bracing

Installing & Bracing of Metal Plate Connected Wood Trusses.

Plate Connected Wood Truss Construction.

ANSI/TPI1:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

truss unless otherwise shown. Trusses are designed for wind loads in the plane of the

section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1 established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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Provide copies of this truss design to the building all other interested parties. designer, erection supervisor, property owner and

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- Cut members to bear tightly against each other
- joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1. Place plates on each face of truss at each
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.

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Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection. responsibility of truss fabricator. General practice is to

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- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted

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- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words is not sufficient and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

43-08-00 33-08-00 10-00-00 C103 C103 CJ01 CJ01 E)01 (16) 7-00-00 CJ03 CJ05 12 PITCH T04 T05 T06 T07 64-00-00 10-08 CJ05 102 CJ03 EJ02 (8) CJ01 CI03 CJ01 22-08-00 13-06-00 T01G 21-00-00 Per ANSI/TP[1-2002 all *Truss to Wall* connections are the responsibility of the Building Designer, not the Truss Manufacturer.

Use Manufacturer's specifications for all hanger connections unless neted otherwise.

Trusses are to be 24° o.e. U.N.O.

All hangers are to be 58 majoon or equivalent U.N.O.

Use 10d x 1 1/2* Nails in hanger connections to single ply tripler trusses. THE ARROW HEAD AT THE END OF THE TRUSS ON THE TRUSS ON THE TRUSS PLACEMENT PLAN (LAYOUT)
CORRESPONDS WITH THE LEFT SIDE OF THE MOPUDDAL TRUSS INDIVIDUAL TRUSS DRAWING, USE THIS AS AN DRIENTATION GUIDE WHEN SETTING THE TRUSSES ON THE TRUSSES ON THE STRUCTURE. This truss placement plan was not created by an engineer, but rather by the Builders FirstSource staff and is solely to be used as an installation guide and does not require a seal. Complete truss engineering and analysis can be found on the truss design drawings which may be sealed by the truss design engineer. All common framed roof or floor systems must be designed as to AVT impose any loads on the floor trusses below. The floor trusses have not been designed to carry any additional loads from above. It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbing drops, can lights, eet..., so the trusses do not interfere with these Refer to BCSI-B1 Summary Sheet-Tauide for handling. Installing and Bracing of Metal Plate Connected Wood Truss prior to and during truss installation. No back charges will be accepted by Builders FirstSource unless approved in writing first. 850-885-4541 Although all attempts have been made to do so, trusses may not be designed symmetrically. Please refer to the individual truss drawings and truss placement plans for proper orientation and placement. ACQ lumber is corrisive to truss plates. Any ACQ lumb that comes in contact with truss plates (i.e. scabbed on tails) must have an approved barrier applied first. Cable end trusses require continuous bottom chord bearing. Refer to local codes for wall framing It is the responsibility of the Contractor to ensure of the proper orientation of the truss placement plans as to the construction documents and field conditions of the structure orientation. If a reversed or flipped inyour is required, it will be supplied at no extra cost by Builders Trusses are not designed to support brick U.N.O.

Dimensions are Feet Inches: Sixteenths

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TEK PLATE APPROVAL # S 2 97 N 2 97 4 80 SE **EWP PRODUCT** # S FL 644 R2 BC O STS 근 392 R2

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bor 1 Job#

KLH
Floor 2 Job#:
N/A

Original Ref#: 3702918 Roof Jab#: 3702918

1676

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FIRSTSOURCE

Builders

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Lot 2 Crosswinds

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