

SHEET NUMBER 2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances. Contractor shall verify all dimensions prior to commencing construction.

FIBERGLASS SHINGLES ON 30-LB BUILDER'S FELT OVER 1/16" ORIENTED STRAND BOARD ROOF SHEATHING PRE-ENGINEERED WOOD ROOF TRUSSES AT 24" O.C. BLOWN-IN INSULATION EQUAL TO R-30 (2) 2X4 SYP *2 DOUBLE TOP PLATE 18" OVERHANG (TYPICAL) NOTE: SEAL ALL PENETRATIONS IN TOP PLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT 2x6 #2 SPF SUB-FASCIA ALUMINUM DRIP EDGE MOLDING, FASCIA, AND VENTED SOFFIT - INTERIOR FINISH - 1/2" GYPSUM WALLBOARD - 2X4 *2 SPF PRECUT STUDS AT 16" O.C. WITH FULL-THICK FIBERGLASS INSULATION EQUAL TO R-11 EXTERIOR FINISH: VINYL SIDING _ 7/16" O.S.B. WALL SHEATHING 4" CONCRETE FLOOR SLAB REINFORCED WITH 6X6-1.4/1.4 WELDED WIRE MESH EMBEDDED 2" IN SLAB OR FIBER MESH ON 6 MIL POLY VAPOR BARRIER (6" LAPS SEALED WITH POLY TAPE) OVER COMPACTED FILL TREATED WITH TERMITICIDE 2 x 4 P.T. SYP *2 SOLE PLATE 8" CONCRETE BLOCK STEMWALL, FULL GROUT (MIN 2/MAX 5 COURSES) ______ -----APPROXIMATE FINISH GRADE POURED CONCRETE STRIP FOOTING

TYPICAL WALL SECTION

SCALE: 1" = 1'0" REFER TO STRUCTURAL PAGE FOR STRUCTURAL SPECIFICATIONS

55'-8" 3'-6" x 5'-0" (2) 2'-6" × 6'-0" (2) 3'-0" × 5'-0" (2) 2'-6" × 6'-0" EGRESS BEDROOM #2 8' CLG MASTER SUITE 8' CLG GREAT ROOM 10' CLG KITCHEN 10' CLG 5'-0" 10' CLG DINING ROOM 10' CLG BEDROOM #3 (2) 1068 SIDELIGHTS 8' CLG 11'-715" ENTRY r - - - -10' CLG ATTIC ACCESS (2) 3'-0" × 5'-0" (2) 3'-0" X 2'-0" TRANS L __ _ _ _ 3'-6" x 5'-0" 2 CAR GARAGE 8' CLG AREA SUMMARY LIVING AREA GARAGE AREA PORCH AREA 92 SF TOTAL AREA 16'-0"

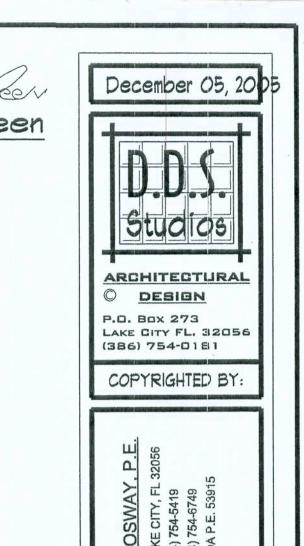
11'-4"

9'-0"

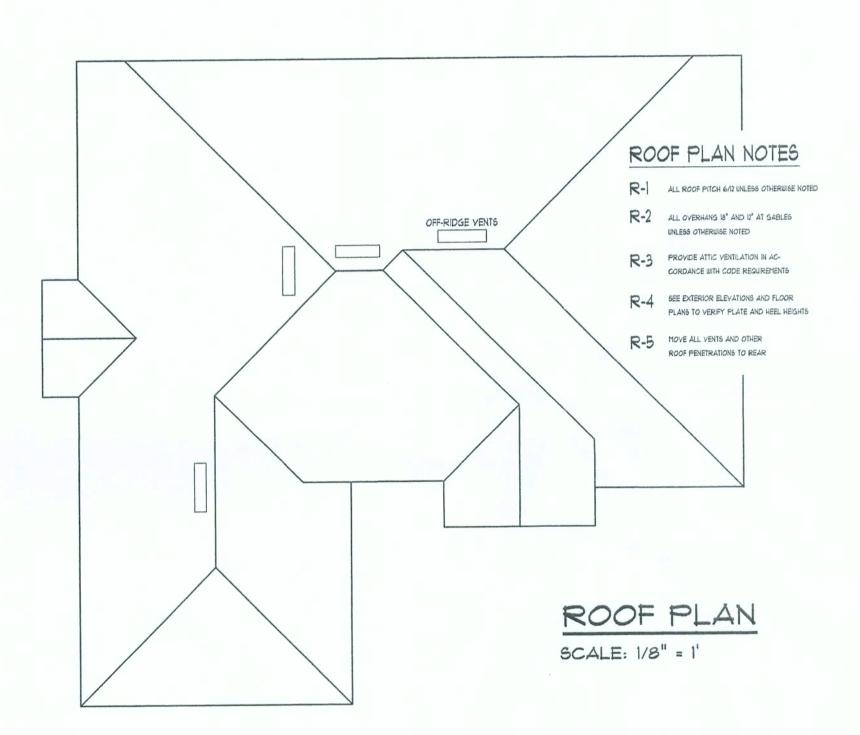
12'-8"

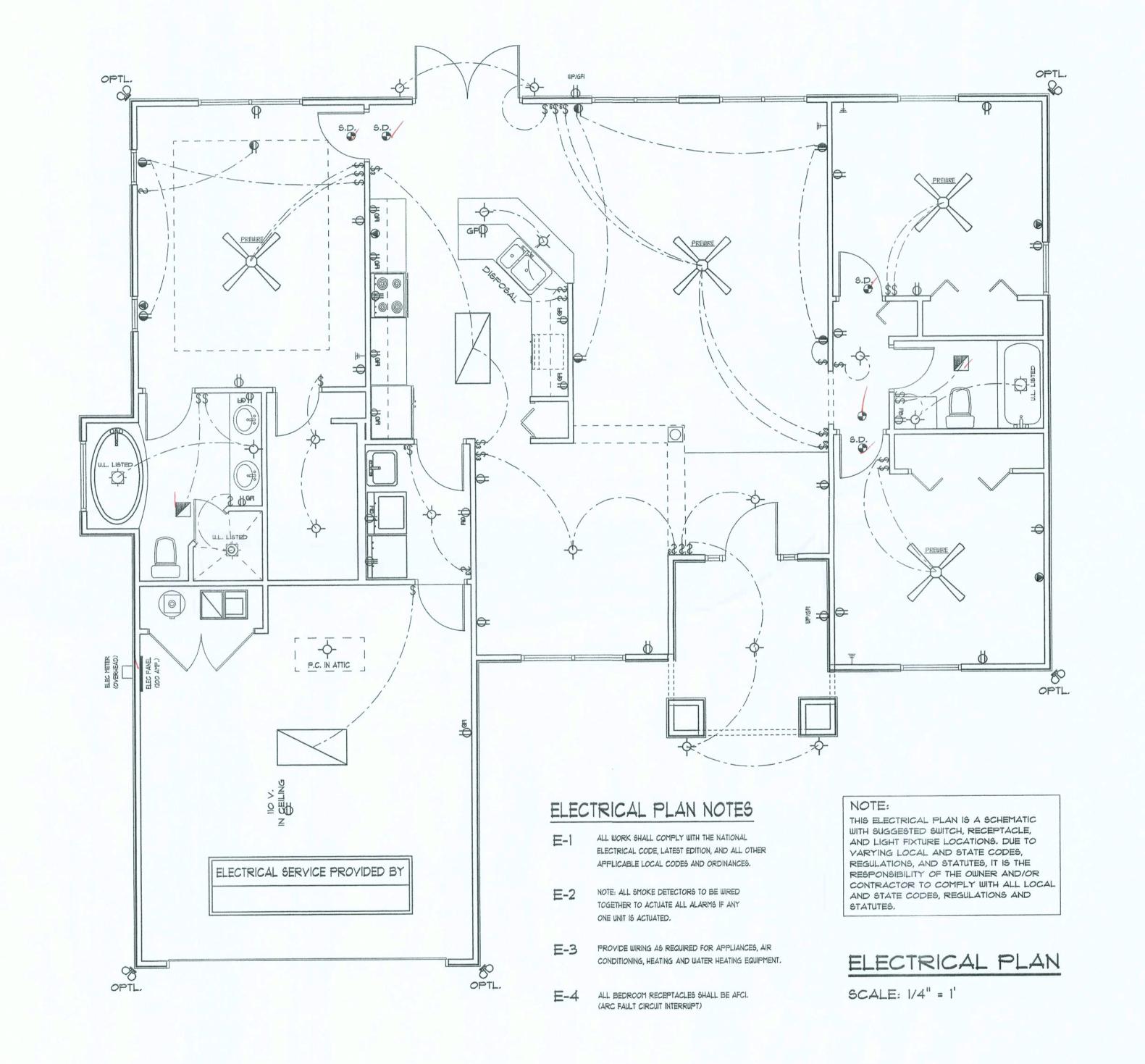
FLOOR PLAN

SCALE: 1/4" = 1'



ENGINEERED BY:





ELECTRICAL PLAN ROOF PLAN

PROJECT INFO:

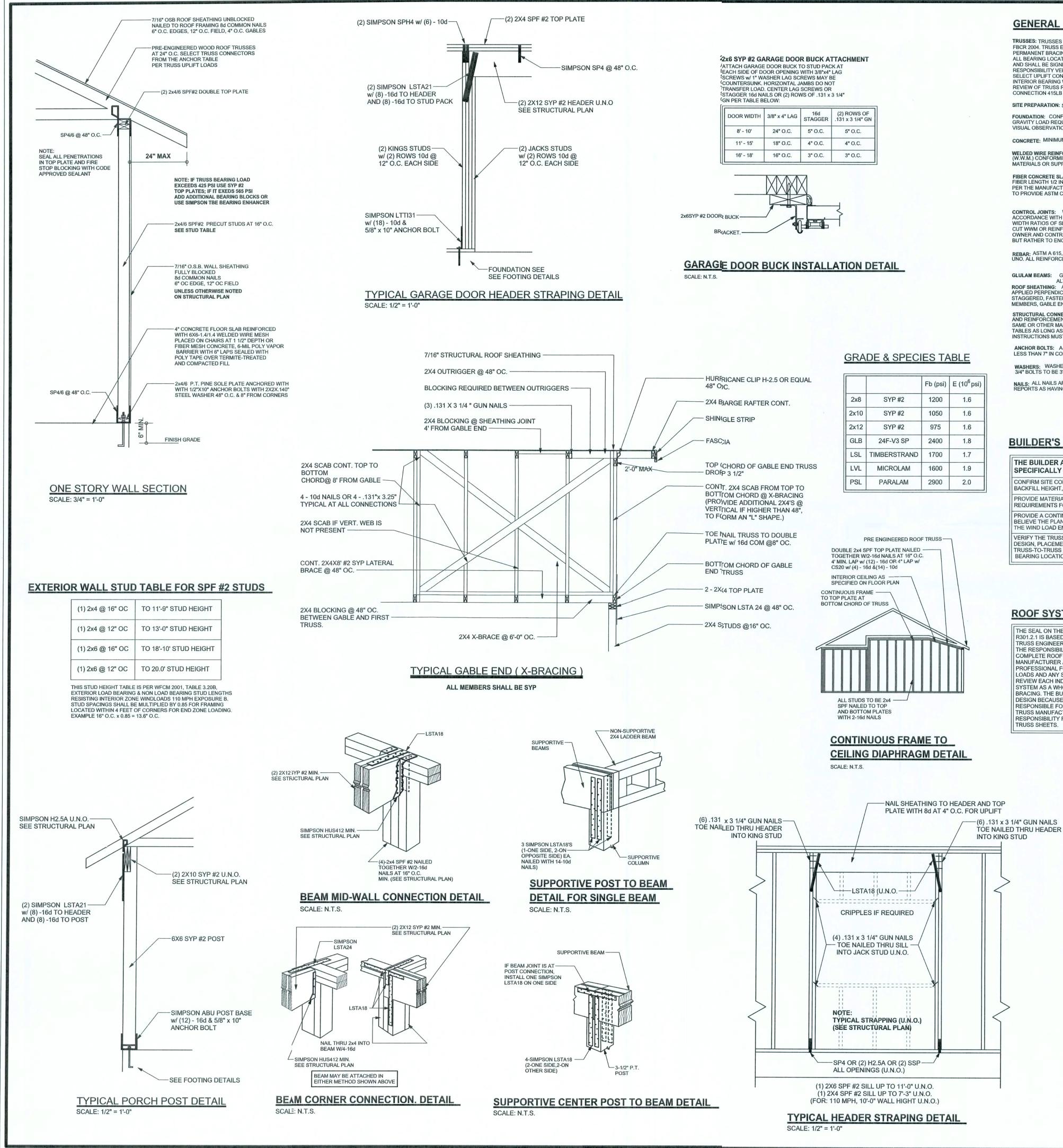
SHEET NUMBER

3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances.

Contractor shall verify all dimensions prior to commencing construction.

DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMEN



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	DITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.	
PROVIDE MATERIAL REQUIREMENTS FO	S AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.	CR 2004
BELIEVE THE PLAN	JOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU DMITS A CONTINUOUS LOAD PATH CONNECTION, CALL SINEER IMMEDIATELY.	
DESIGN, PLACEMEN	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS T PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, ONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL IS.	

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

CMU standard

3.3.E.7 | Movement joints

Clay brick standard

Reinforcing bars, #3 - #11

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow,

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

bond and 12"x12" or 16"x16" column

ASTM C 270, Type N, UNO

5.5"x2.75"x11.5"

Coating for corrosion protection Anchors, sheet metal ties completely

Coating for corrosion protection | Joint reinforcement in walls exposed to

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

or 304SS

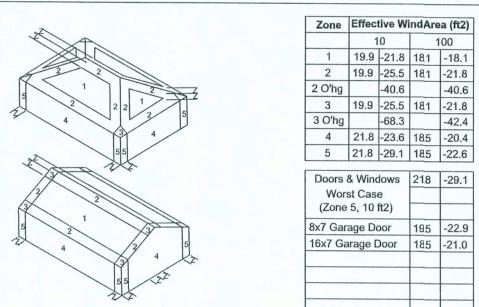
ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	-	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			T) FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8' THREADED ROD 1:" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8' THREADED ROI 1:" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8' THREADED ROD 1:" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8' THREADED ROD 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		To FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
< 4175	< 3695	HTT16	18 - 16d		5/8" AB	
< 1400	< 1400	PAHD42	16-16d			
< 3335	< 3335	HPAHD22	16-16d			
		ABU44	12-16d		1/2" AB	
< 2200	< 2200	ADO-14				
	< 2200	ABU66	12-16d		1/2" AB	

DESIGN DATA

	ID LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
ME/ ON	ICLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GAB_E ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. CAND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = B
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



5	2 3	8x7 Garage Door	195	-22.9
2	4 /3/ 5	16x7 Garage Door	185	-21.0
	4 4	-	-	
	55		-	-
	7.8.5			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN

NDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserv its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express writter permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have xamined this plan, and that the applicable ortions of the plan, relating to wind engine comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one

building, at specified location.

MARK DISOSWAY P.E. 53915

Ewpl, Inc.

The Nicolas Model Lot 25 Rolling Meadows S/D

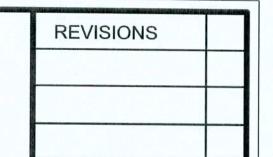
ADDRESS: Lot 25 Rolling Meadows S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

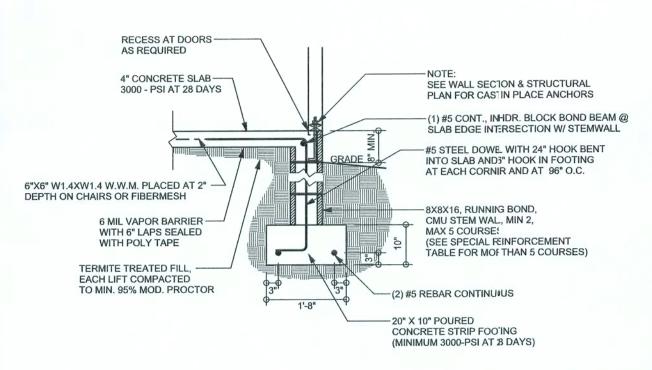
PRINTED DATE: December 26, 2005 CHECKED BY DRAWN BY: David Disosway

FINALS DATE: 26 / Dec / 05 JOB NUMBER: 512091 DRAWING NUMBER **S-1**

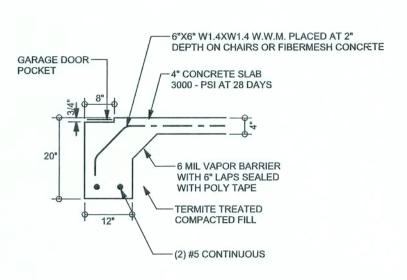
OF 3 SHEETS



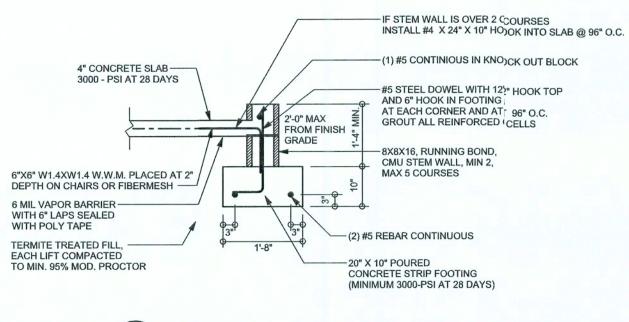




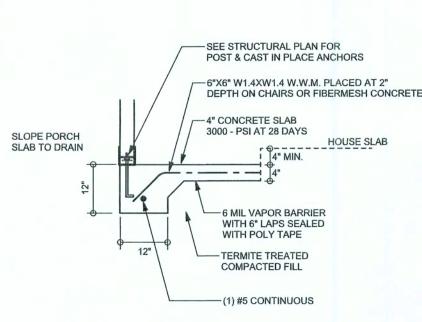
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"

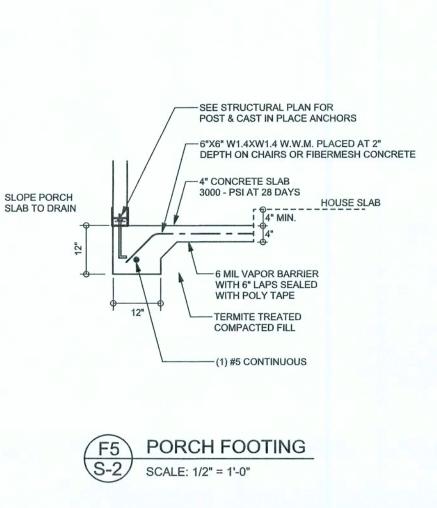


F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"



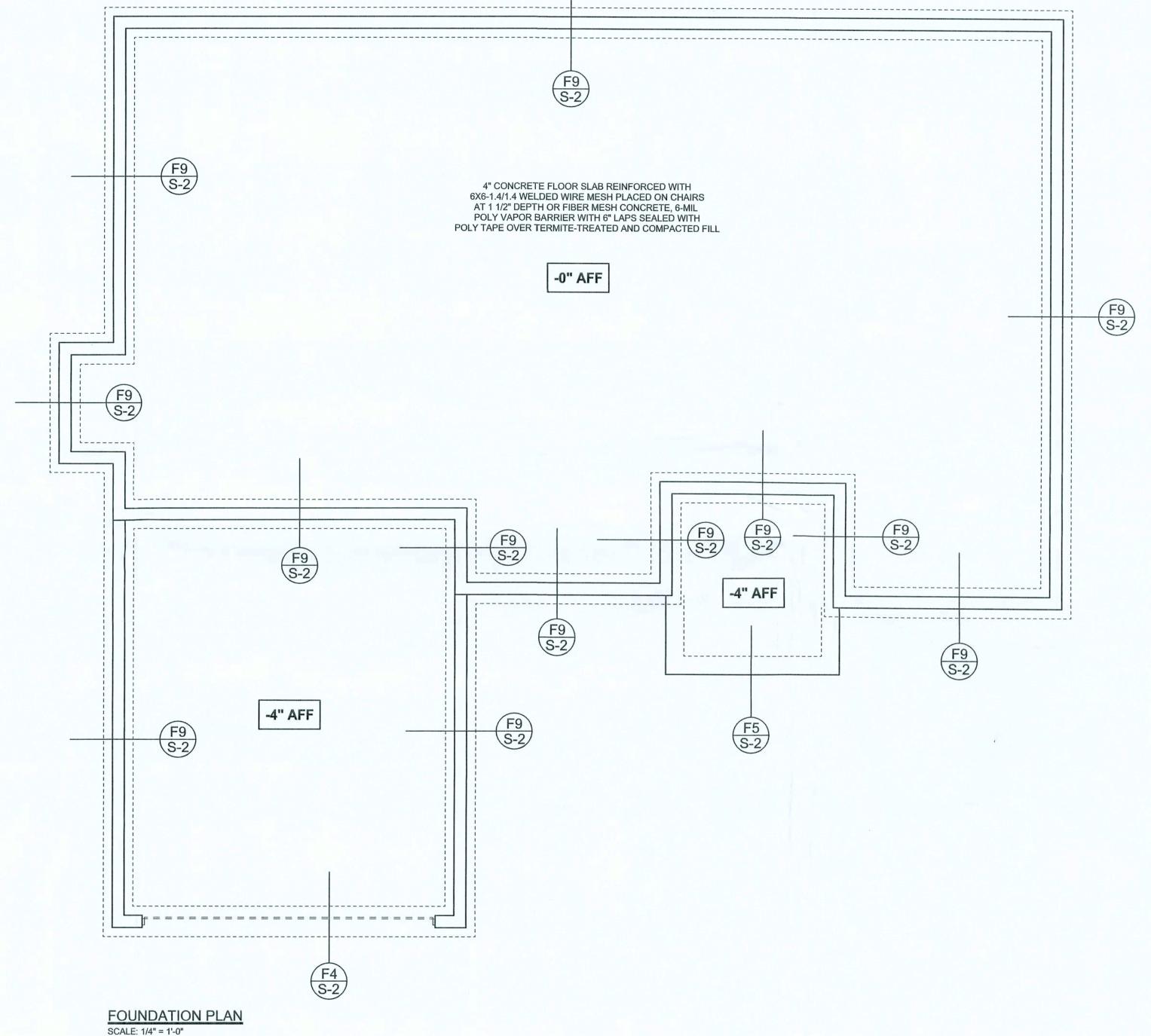
F18\ STEM WALL CURB FOOTING SCALE: 1/2" = 1'-0"





TALL STEM WALL TABLE The table assume:60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, adcDurowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 cortinuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcementas shown in the table below.

STEMWALL HEIGHT (FEET)	UNIALANCED B\CKFILL IEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			B\CKFILL FOR 8" CMU STEMWALL FOR 12" CMU STEMV			MWALL
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserved its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

Ewpl, Inc.

The Nicolas Model Lot 25 Rolling Meadows S/D

ADDRESS: Lot 25 Rolling Meadows S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

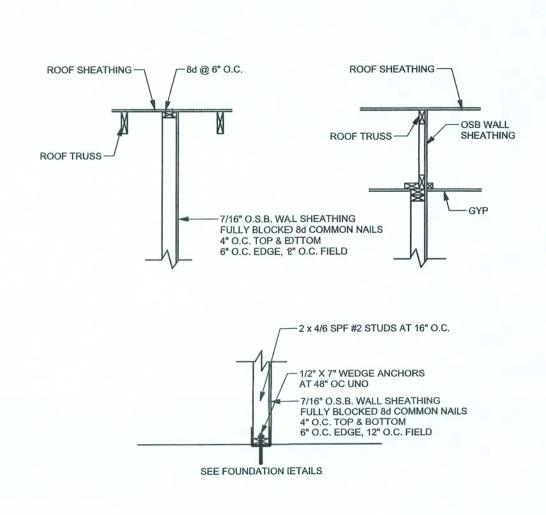
PRINTED DATE: December 26, 2005 DRAWN BY: CHECKED BY

FINALS DATE:

David Disosway

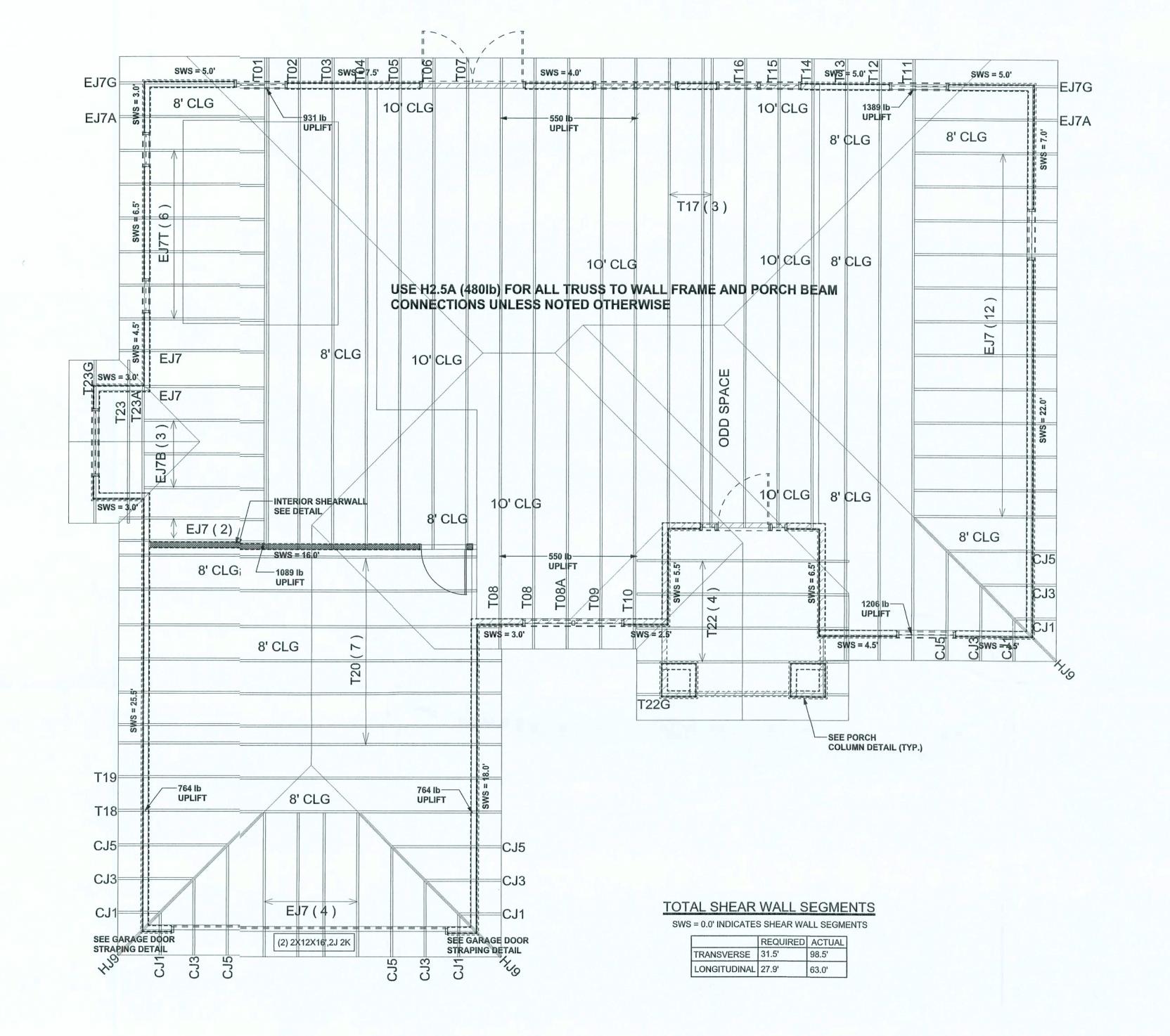
26 / Dec / 05 JOB NUMBER: 512091 DRAWING NUMBER

> **S-2** OF 3 SHEETS



INTERIOR SHEAR WALL DETAIL

SCALE: 1/2" = 1'-0"



STRUCTURAL FPLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL FOLAN NOTES

SN-1 ALL LOAD BE:ARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BE: ARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3

DIMENSIONS; ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

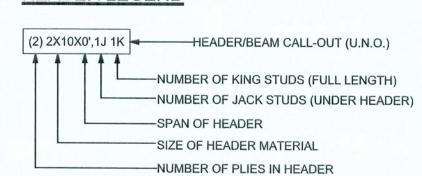
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACK/AGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO. INC. JOB #ELIXSON SOFTPIAN

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

Ewpl, Inc.

The Nicolas Model Lot 25 Rolling Meadows S/D

ADDRESS: Lot 25 Rolling Meadows S/D Columbia County, Florida

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PRINTED DATE:
December 26, 2005

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 26 / Dec / 05

JOB NUMBER: 512091 DRAWING NUMBER

S-3 OF 3 SHEETS