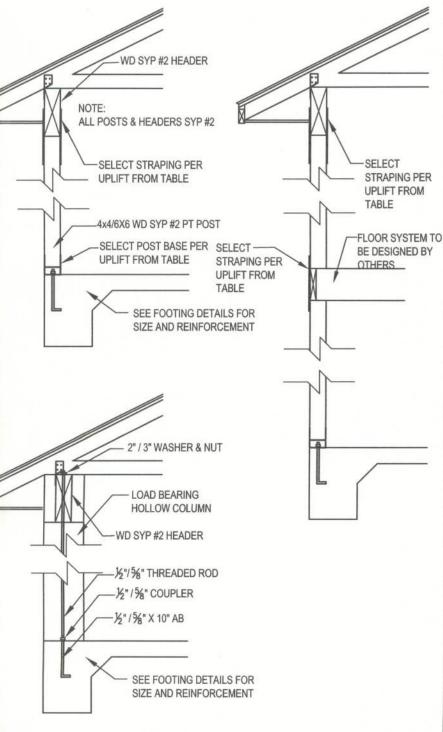


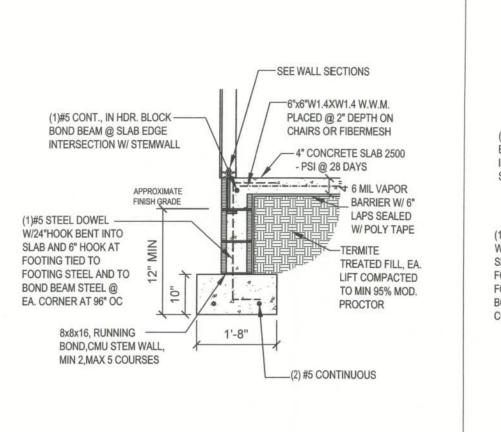
TYPICAL TRUSS UPLIFT & MAX 10'-0" WALL HIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE SPH4/SPH6 SPACING	
770 LB	48" O.C. 48" O.C.		N/A	
950 LB	48* O.C.	32" O.C.	N/A	
1270 LB	32" O.C.	16" O.C.	32" O.C.	
1500 LB	24" O.C.	16" O.C.	16" O.C.	
2200 LB	LTTI31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK	
NOTE: SP2 TOP	& SP1 BOTTOM AL	TERNATE FO	OR SP4/6	

W1 - SINGLE STORY EXT. WALL SECTION



	SYP #2 PT WD POSTS		
TYPICAL	POST BASE	BETWEEN FLOOR	HEADER
POST UPLIFT	ANCHOR	STRAPING	STRAPING
555 LB	ABA44 W/ (6)-10d &	(2) LSTA21 W/	(2) LSTA21 W
	½" AB	(6)-10d EA.	(6)-10d EA.
720 LB	ABA66 W/ (8)-16d	(2) LSTA21 W/	(2) LSTA21 W.
	& % " AB	(8)-10d EA.	(8)-10d EA.
2200 LB	ABU44 W/ (12)-16d,	(2) LSTA21 W/	(2) LSTA21 W
	(2) ½ BOLTS & 5 €"AB	(16)-10d EA.	(16)-10d EA.
2300 LB	ABU66 W/ (12)-16d,	(2) LSTA21 W/	(2) LSTA21 W/
	(2)1/2 BOLTS & 1/8"AB	(16)-10d EA.	(16)-10d EA.
	HOLLOW COLUMN		
1500 LB	½" X 10" AB ATT THREADED ROD W THRU COLUMN & I WASHER &		
2300 LB	%" X 10" AB ATT THREADED ROD W THRU COLUMN & I WASHER &		

W12 - PORCH HEADER ANCHORS



F1 - STEM WALL FOUNDATION SCALE: 1/2"=1'-0" REV-27-MAY-03

---- 4" CONCRETE SLAB 2500 - PSI

-6"x6"W1.4XW1.4 W.W.M. PLACED @ 2"

DEPTH ON CHAIRS OR FIBERMESH

W/ POLY TAPE

PROCTOR

F12 - NON - BEARING STEP FOOTING

2X4 SCAB CONT. TOP-

CHORD@ 8' FROM

4 - 10d NAILS OR 4 -

ALL CONNECTIONS

2X4 SCAB IF VERT.

CONT. 2X4X8' #2 SYP -

LATERAL BRACE @ 48"

2X4 BLOCKING @ 48" OC.

BETWEEN GABLE AND

FIRST TRUSS.

2 X 4 CONT. LATERAL BRACING

1/2 GYP. BOARD W/ 5d COOLER NAILS

5d COOLER NAILS AT7 IN. O.C.

ALTERNATIVE TO BALLOON FRAMING IS GYIPSUM CEILING

ABOVE, PROTECT GYPSUM FROM MOISTURIE TO PRESERVE

ALONG GABLE AND 29ga DIAPHRAGM WILL RESIST 200 PLF OUT OF PLANE LATIERAL LOAD AT THE GABLE TRUSS BOTTOM CHORD.

DIAPHRAGM AS SPECIFIED IN THE "WOOD FIRAME CONSTRUCTION

MANUAL" (WFCM). SEE WFCM TABLE 3.13 AIND FIGURE 3.6a. SHONE

AT 6' O.C. W/ 2-10d NAILS EA.

AT 10 IN. O.C.

STRENGTH OF DIAPHRAGM.

WEB IS NOT PRESENT

.131"x 3.25" TYPICAL AT

TO BOTTOM

GABLE

- 6 MIL VAPOR BARRIER W/ 66" LAPS SEALED

2X4 OUTRIGGER @ 48" OC .-

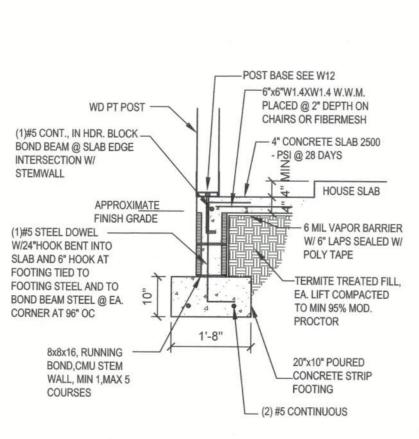
BLOCKING REQUIRED BETWEEIN OUTRIGGERS

2X4 BLOCKING @ SHEATHING JOINT 4' F'ROM GABLE END -

- TERMITE TREATED FILL, EA. LIFT

COMPACTED TO MIN 95% MOD.

@ 28 DAYS



F10 - STEM WALL PORCH FOOTING

(1) #5 CONTINUOUS

SCALE: 1/2"=1'-0" REV-10-FEB-03

3) .131 X 3 🗗

CONT 2X4 SCAB FROM TOP

TO BOTTOM CHORD @

VERTICAL IF HIGHER THAN

- 2 X 4 BLOCK NAILED TO EA. BRACE

-ENDWALL STUDS

NOTE: EXTEND RAT RUN BRACES GABLE TO GABLE

OR A LENGTH EQUAL TO 1/2 OF GABLE WIDTH FOR

ROOFS UP TO 7:12 AND 5/8 OF GABLE WIDTH OVER

-GABLE END TRUSS

W/4-10d NAILS

X-BRACING (PROVIDE

ADDITIONAL 2X4'S @

48" TO FORM AN "L"

NOTE: ALL MEMBERS SHALL BES

2X4 X-BRACE @ 6'-0" OC.

W10 - TYPICAL GABLE END (X-BRACING

W23 - GYPSUM CEILING DIAPHRAGM OPTION - GABLE END WALL

IMPORTANT NOTE: GYPSUM BOARD MAY BE REPLACED BY 29ga STEEL CEILING PANELS ATTACHED PER MANUFACTURERS PUBLISHED DATA FOR 100 PLF SHEAR LOAD. RAT RUN BRACING SHOULD BE RUN AT 45

DEGREES FROM GABLE END TO SIDE WALLS AND ADD 6 - 16d NAILS AT EACH END OF THE RAT RUN INTO THE GABLE TRUSS AND INTO THE SIDE WALL BLOCKING. THE SUM OF DIAGONAL RAT RUNS AT 6'OC

- 4" CONCRETE SLAB 2500 - PSI @

6"x6"W1.4XW1.4 W.W.M. PLACED @ 2"

DEPTH ON CHAIRS OR FIBERMESH

-TERMITE TREATED FILL

EA. LIFT COMPACTED TO

MIN 95% MOD. PROCTOR

- 6 MIL VAPOR BARRIER W/ 6"

F13 - NON - BEARING THICKENED SLAB EDGE

-7/16" STRUCTURAL ROOF

HURRICANE

-CLIP H-2.5 OR

EQUAL 48" OC.

TOP CHORD

OF GABLE END

TRUSS DROP 3

TOE NAIL TRUSS TO

COM @8" OC.

DOUBLE PLATE w/ 16d

-BOTTOM CHORD OF GABLE END TRUSS

--(2) - 2X4 TOP PLATE

SIMPSON LSTA

24 @ 48" OC.

-SEE DETAIL W1

- SIMPSON LSTA30 STRAP W/

10-8d NAILS IN 2 X 4 BRACE &

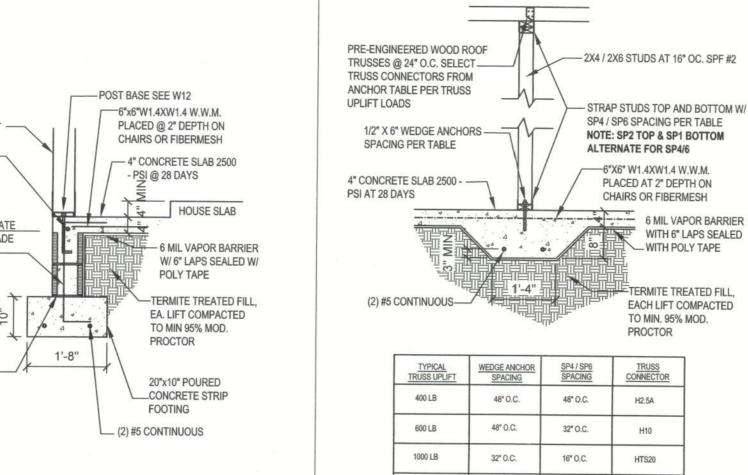
10-8 d NAILS IN ENDWALL STUD

SHEATHING

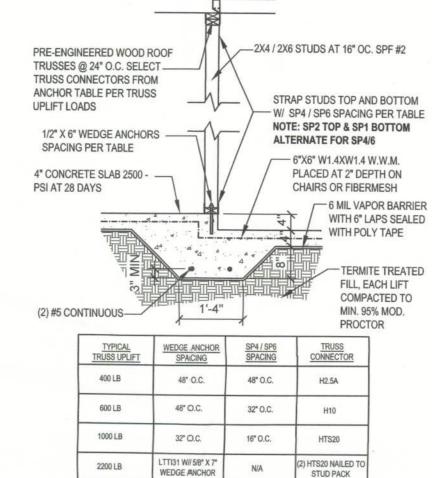
2X4 BARGE -

RAFTER CONT.

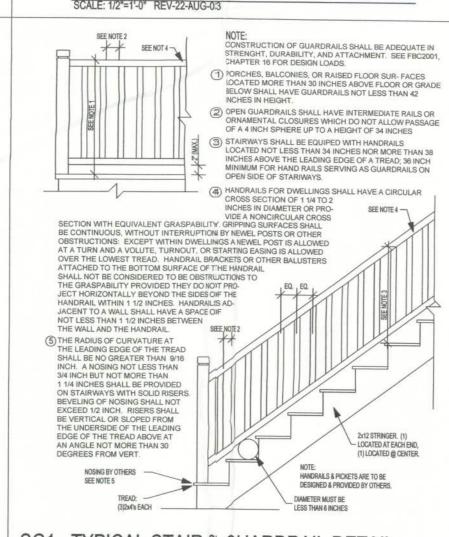
LAPS SEALED W/ POLY TAPE



2200 LB WEDGE ANCHOR F4 - INTERIOR BEARING FOOTING

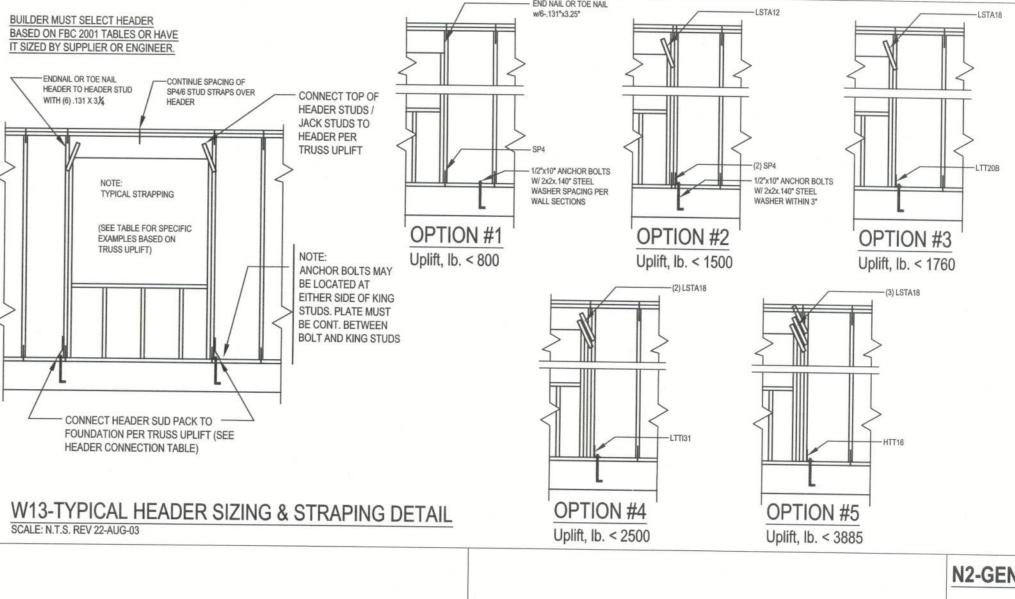


F5 - INTERIOR BEARING STEP FOOTING



SG1 - TYPICAL STAIR & GUARDRAIL DETAIL

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4I-8d	4-8d
245	350	H5A	3i-8d	3-8d
535	600	H2.5A	5-8d	5-8d
620	720	H10	6-10dx1½"	6-10dx 11/5"
850	990	LTS12	8-8dx1½"	8-8dx 1½"
1245	1450	HTS20	100-10d or 12-10dx11/2"	10-10d or 12-10dx 11/5"
1265	1470	H16, H16-2	100-10dx1 1/5"	2-10dx 11/5"
1785	2050	LGT2	14-10d Sinker	16-16d Sinker
3655	4200	MGT	%" Thd. Rod	22-10d
SPF	SYP	Strap Connector	To One Member	To Other Member
760	885	SP4	6-10dx1 1/2"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	710d	7-10d
1170	1360	SPH4	12-10tx1½"	N/A
420	455	SSP	4-100	3-10d to double plate or 1-10d to single
600	825	DSP	810d	6-10d to double plate or 2-10d to single
1420	1650	CS16	141-8d or 11-10d	14-8d or 11-10d
SPF	SYP	Column Anchor	To Foundation	To Column / Truss
1160	1350	LTT19	5⁄68"x 16" AB	8-16d Sinkers
1985	2310	LTTI31	%;"x 16" AB	18-10dx 1½"
2385	2775	HD2A	56; "x 16" AB	2-5/8" Bolts
3590	4175	HTT16	% "x 16" AB	18-16d
1975	2300	ABU66	% "x 16" AB	12-16d
Studs Supporting Trus	sses: The bu	ilder is responsible for gravit	ty loæds, but you should put an ex of the truss and top plate (SPF, F	dra 2x4 stud under truss bearing location for each



N2-GENERAL NOTES: FOUNDATION: FOR POINT LOADS GRATER THAN 5000 Ib OR REPETITIVE TRUSS LOADS GRATER THAN 2000 Ib PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 Ib OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH

- FLOOR SYSTEM TO BE

NOTE: SEAL ALL PENETRATIONS IN

FIRE STOP BLOCKING WITH CODE

APPROVED SEALANT

-5/8" TYPE "X" (FIRECODE)

- (3) 2x12 STRINGERS

NOTE:

including interior bearing walls.

is the responsibility of the builder.

RISERS AT 7 3/4" MAX TREADS AT 9" MINIMUM

(2) RISERS + (1) TREAD SHALL NOT

BE LESS THAN 24" OR MORE THAN 25"

TREADS 10" OR LESS MUST HAVE 1" NOSE

GYP. BOARD UNDER ALL INT. STAIRS

DESIGNED BY OTHERS

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR

SURFACE, ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES

SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS

REPRESENTATIVE.

Load Bearing Header Sizing Methods (BY BUILDER)

Jack Studs and King Studs (BY BUILDER)

Header Uplift Connections (BY BUILDER)

dividing by the length of the header.

FBC2001, TABLE 2308.3A

Supporting Roof+Ceiling (20psf+20psf)

NOTES: NJ = Number of jack studs

width is measured perpendicular to

the ridge. For widths between those

Spans are based on uniform loads on

shown, spans may be interpolated.

required to support each end. Building

5. Use one jack stud for every 3000 lb vertical load.

connection) and stud to foundation (bottom connection).

< 800 End nail or toe nail

w/6-.131"x3.25"

Option # Uplift, lb. Top Connector

2. Use supplier pubished data or Southern pine span tables.

3. For engineered lumber beams have suppliers engineer size beam.

4. Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.

6. Total king plus jack studs = studs needed to be there if no opening was there.

. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and

Bottom Connector

SP4, 6-10dx1 1/2"

Header Spans Building Width / Truss Span (ft)

8. Select header connections from table below or mfg. catalog to connect header to stud (top

#2 < 1500 LSTA12, 10-10d 755 (2) SP4, 6-10dx1½",½" AB 138 #3 < 1750 LSTA18, 14-10d 1055 LTT20B, 10-16d ½" AB 175 #4 < 2500 (2) LSTA18, 14-10d 2110 LTT131, 18-10d½"x10" AB 218

#5 < 3885 (3) LSTA18, 14-10d 3480 HTT16, 18-16d, 1/2 "x10" AB 4175

Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL, THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAPS SPLICES 48 * db (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30

SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS

AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; NO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

SG2 - TYPICAL STAIR DETAIL

than 1: 3.5 (plus special shear wall

17A & 3.17B with table 3.17E

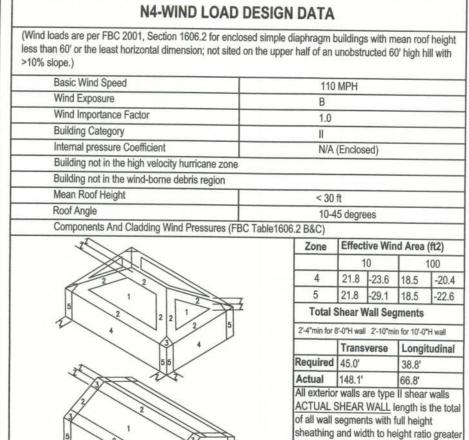
djustment for type II shear wall (or

segments if noted.) REQUIRED SHEAR

VALL length is from WFCM-2001, table

equivalent calculation) REV-27-Jun-03

2x4 PLATE



2ND. FIN. FLR.

1ST FIN. FLR.

N3-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wimd load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not

engineer of record for the trusses and did not design the trusses or delegate to the truss designer. BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work. * Confirm that tithe foundation design & site conditions meet gravity load requirements (assume 1000 PSF bearing capacity unless visual observation or soils test proves otherwise * Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.

* Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately. * Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations. * Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions;

* Size headers for gravity loads; headers sized by the builder for gravity loads will also satisfy wind loads. OCUMENT CONTROL and PRIORITY: Structural requirements on S-1 control unless the building code or rchitectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and

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Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915

3/4" T&G FLOORING-

ATTACH TO FLOOR SYSTEM

WITH .131 X 3 1/4" RS @ 6" OC

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 rind loads, to the best of my knowledge.

.IMITATION: This design is valid for one building, at specified location. is drawing is not valid for construction unless raised seal is affixed.

Sheet S-1 of 6 Sheets

Windload Engineering

Job # 502044 REV-06-OCT-03

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

REV-27-Jul-04

WINDLOAD ENGINEERING

Mark Disosway P.E. POB 868, Lake City, FL 32056 Phone: (386) 754-5419

Fax: (386) 754-6749 Email: windloadengineer@bellsouth.net Location: 231 SW Bonanza Glen Lake City, Florida 32025

Stephenson Residence

Builder: Edgley Construction, Inc.

Designer:

Approved: FLPE#53915