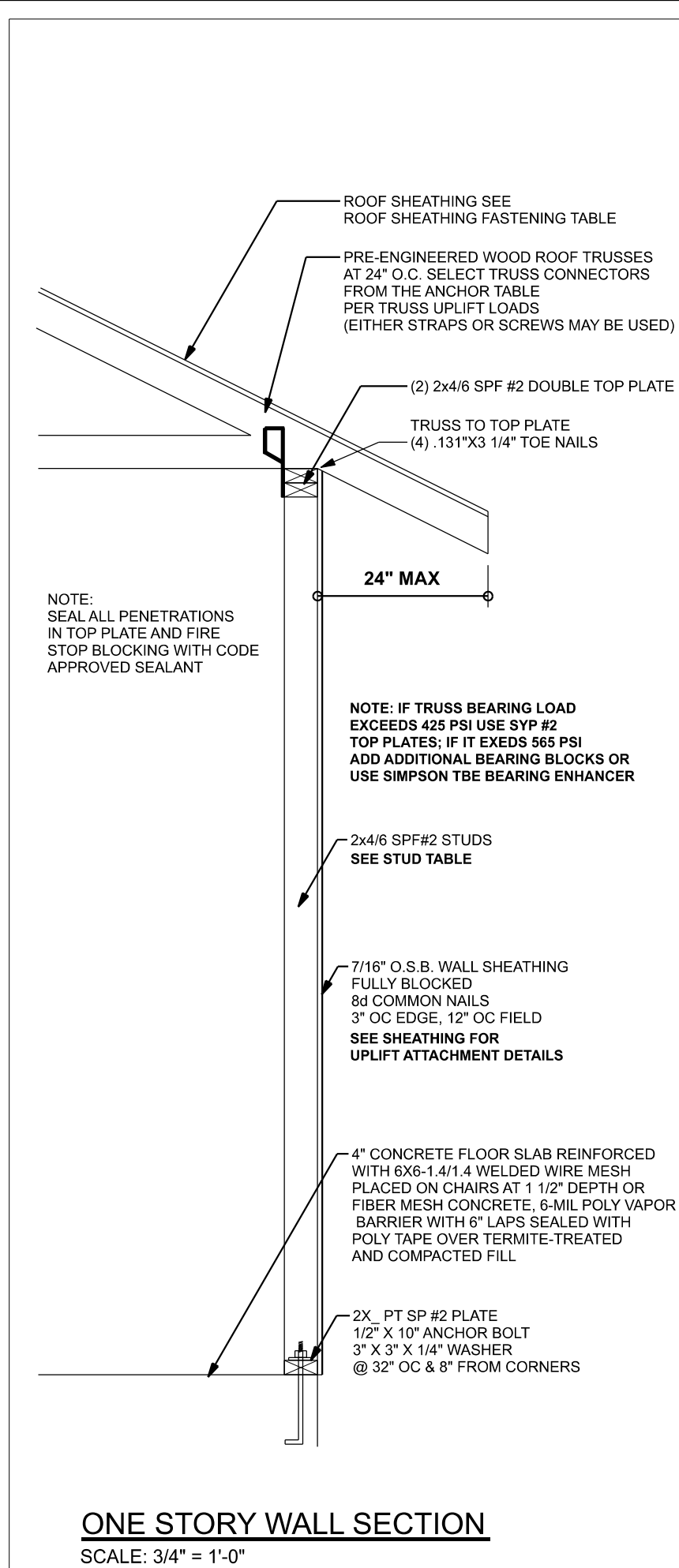
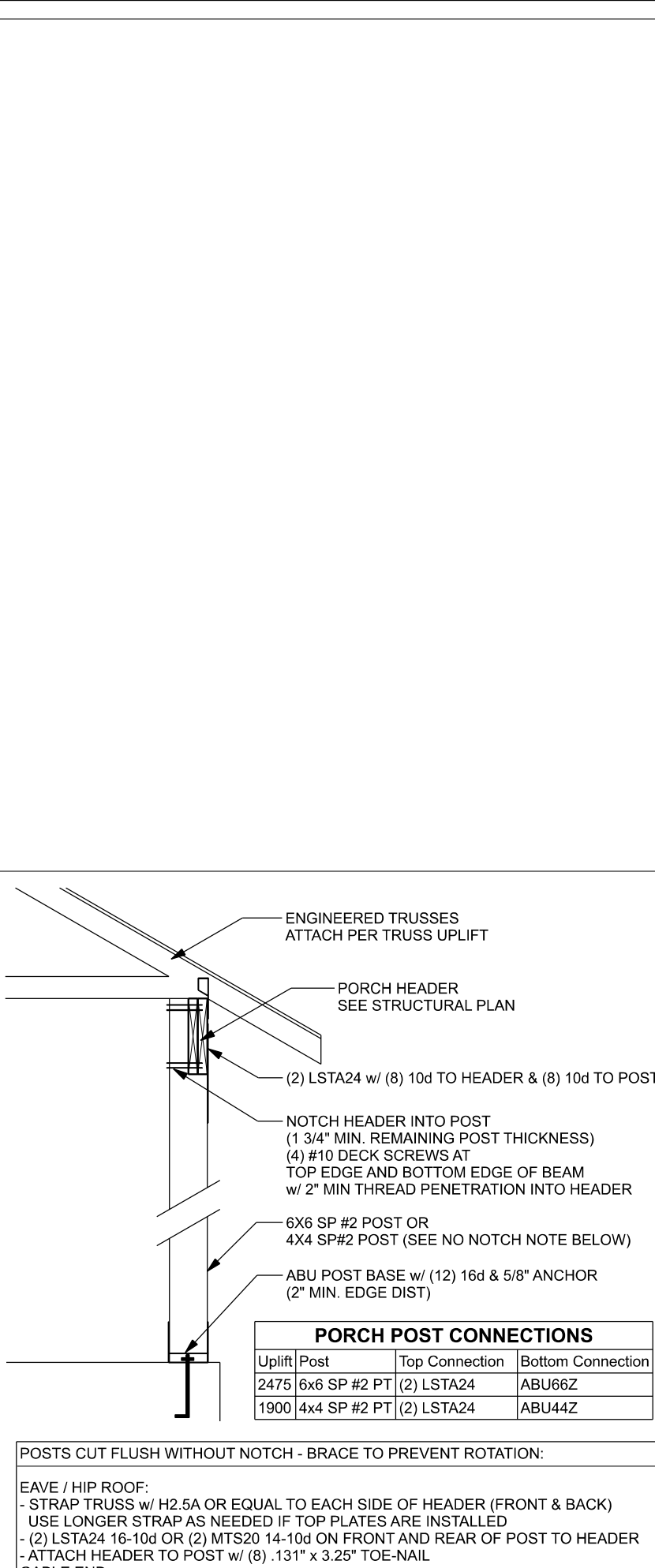
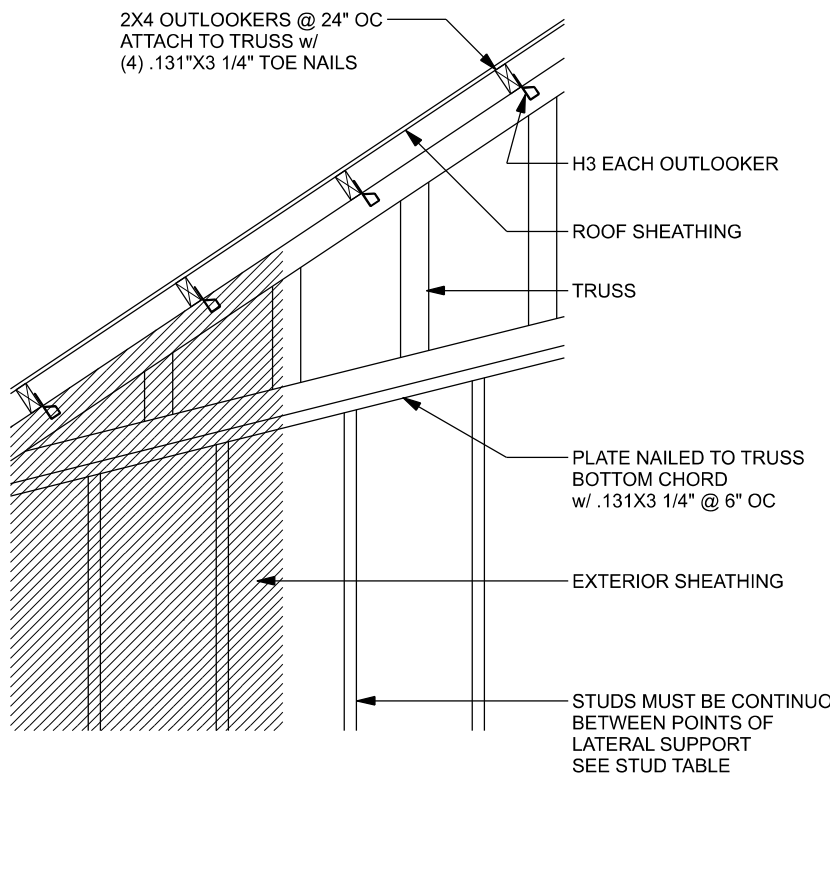
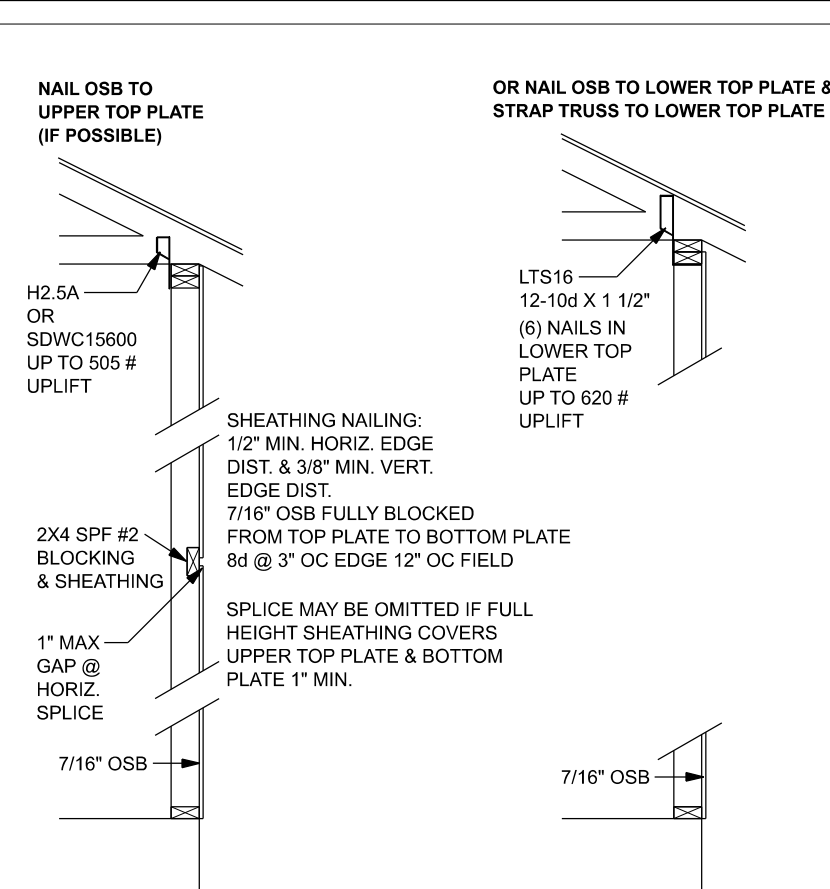
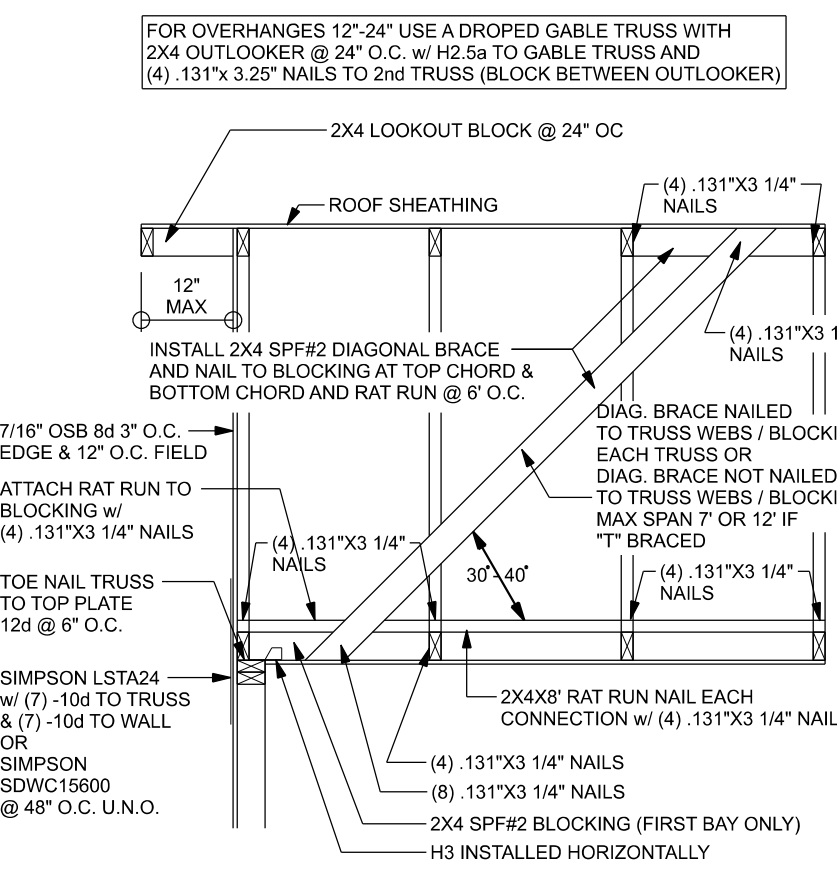




ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS GC = 0.49)				
Wind Speed	Sheathing Thickness and Wind or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. A	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.13")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2.38" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RRSR-03 (2.12" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



CONNECTOR TABLE					
Uplift Sp	Uplift SP1	SP1	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600		-	-
415	290	H3		4-8x1 1/2"	4-8x1 1/2"
1615	640	SP1	SP1	8-8x1 1/2"	8-8x1 1/2"
1320	1015	H14		9-10x1 1/2"	9-10x1 1/2"
720	620	LTS12-20		6-10x1 1/2"	6-10x1 1/2"
1000	880	MTS12-30		7-10x1 1/2"	7-10x1 1/2"
1000	1245	HTS25-30		12-10x1 1/2"	12-10x1 1/2"
Uplift SP1	Uplift SP1	SP1	Truss Tee	To Member	To Other Member
1235	1235	LSTA21		8-10d	8-10d
1640	1455	MTA524		9-10d	9-10d
Uplift SP1	Uplift SP1	CS20	Stud Plate Ties	To Stud	To Stud
1030	1030	CS20		7-10d	7-10d
Uplift SP1	Uplift SP1	SP1	SP1	To Stud	To Plate
585	585	SP1	SP1	4-10d	4-10d
1065	605	SP2		6-10d	6-10d
771	771	LSTA24		10-10d	wrap under or over plate
1235	1235	LSTA24		14-10d	wrap under or over plate
Uplift SP1	Uplift SP1	SP1	SP1	At Node	At Node
1525	1800	D1T22	SP1	8-SDS 14x1 1/2"	12x12" 18" I-tien HD
1525	1800	D1T22		8-SDS 14x1 1/2"	12x12" 18" I-tien HD
1425	3640	HTT4		18-16x25 1/2"	12x12" 18" I-tien HD
Uplift SP1	Uplift SP1	HD10x80s @ Mono	HD10x80s @ Mono	To Stud / Post	To Stud / Post
1525	1800	D1T22		8-SDS 14x1 1/2"	12x12" 18" I-tien HD
4035	3640	HTT4		18-16x25 1/2"	12x12" 18" I-tien HD
Uplift SP1	Uplift SP1	Post Bases @ Stewmalco	Post Bases @ Stewmalco	To Stud	To Stud
1900	1900	ABU442		12-16s	5/8"x17" Drill & Epoxy
2475	2475	ABU682		12-16s	5/8"x17" Drill & Epoxy
Uplift SP1	Uplift SP1	Post Bases @ Mono	Post Bases @ Mono	To Post	To Post
470	470	ABU412		12-16s	5/8"x17" Drill & Epoxy
2475	2475	ABU682		12-16s	5/8"x17" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF 22 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20.05, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB BEARING AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDOW/LAS, 130 MPH, EXPOSURE C STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISHES). STUDS SHALL BE NAILING TO STUDS FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING (END ZONE EXAMPLE 16" O.C. X 8" S = 12.8" O.C.)

1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
2) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
3) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
4) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES. TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, BRACING CONNECTIONS, AND CONNECTIONS TO UPLIFT AND DOWN LIFT AT ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S DESIGN, THE FABRICATOR'S FABRICATION, AND THE TRUSS MANUFACTURER'S BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING BUILDS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 26X RAFTERS WITH MIN. UPLIFT CONNECTION #15LB EACH END; 2X8 RAFTERS 70LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVE OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 8" x 6" W14 x W1.4, F_y = 60KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMANCE TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

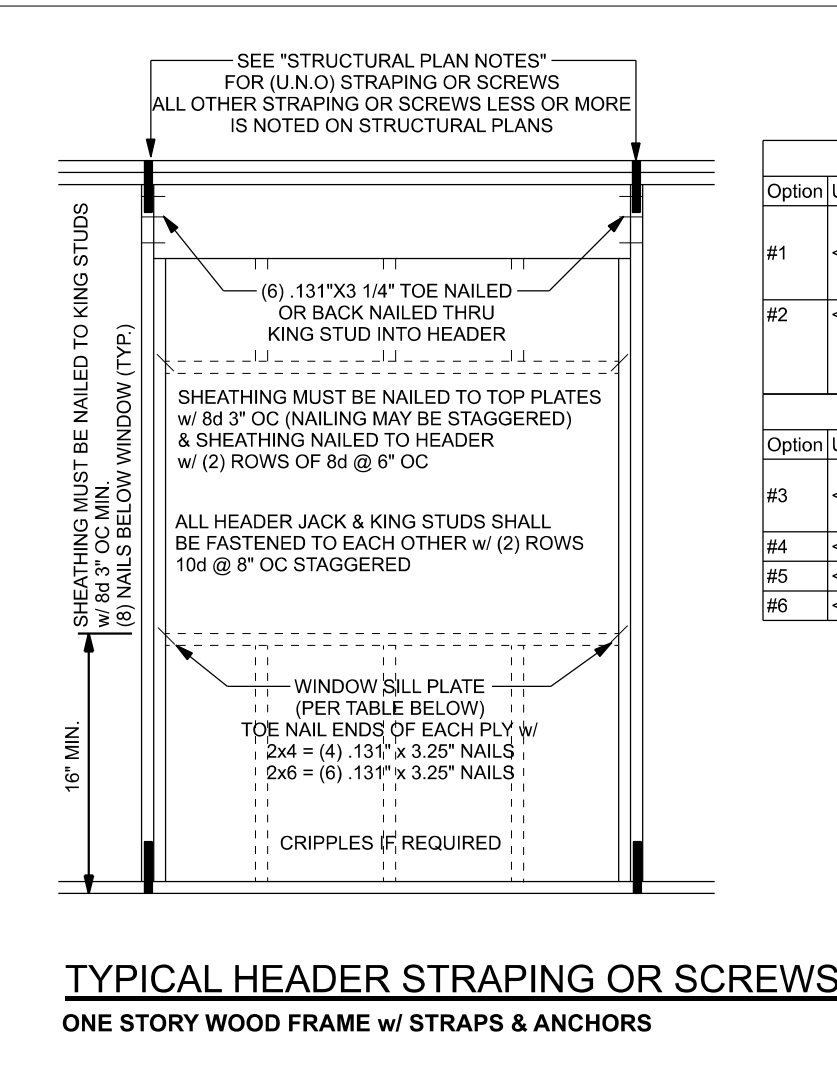
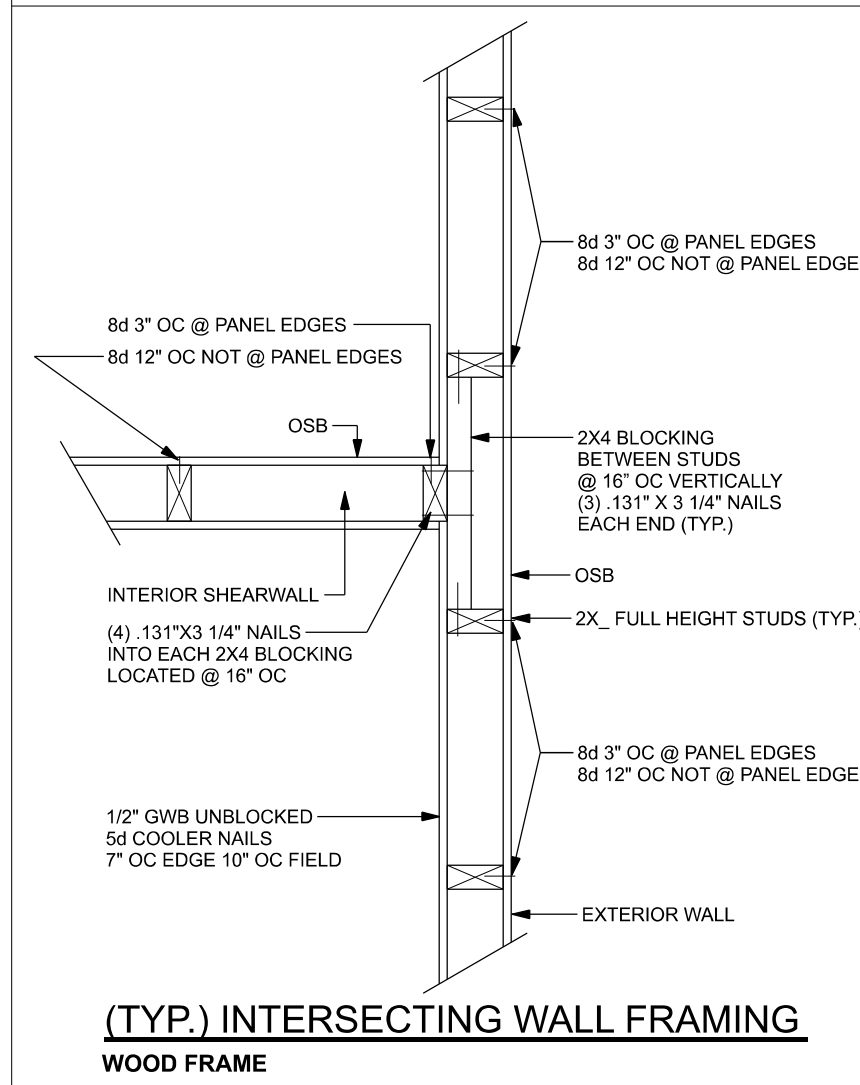
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, AND TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

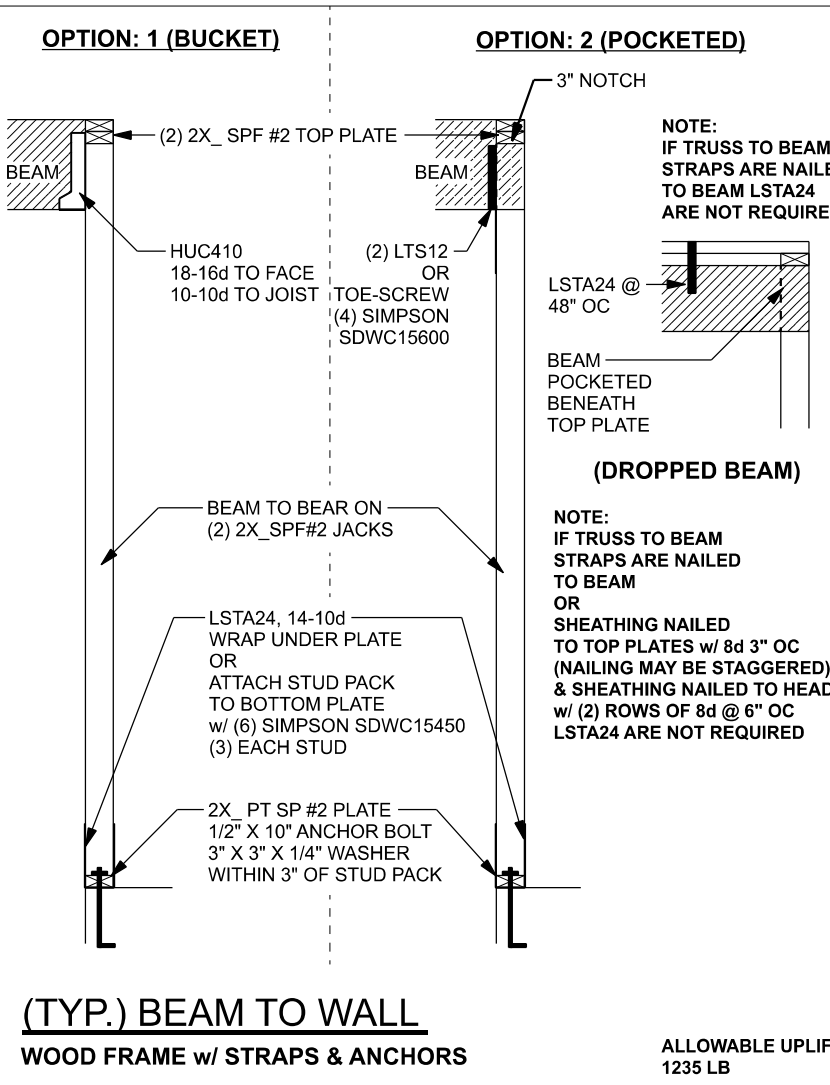
ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOADS ON TRUSS ENGINEERING. THE TRUSS DESIGNER IS RESPONSIBLE FOR IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. THE TRUSS DESIGNER IS RESPONSIBLE FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE TRUSS IS AN ENGINEERED MEMBER. THE BUILDER IS RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



HEADER SCREWS TABLE	
Top Connection	Bottom Connection
Attach king stud to top plate w/ (1) Simpson DSU-5450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations	Attach king stud to bottom plate w/ (2) Simpson DSU-5450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
Attach king stud to top plate w/ (2) Simpson DSU-5450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations	Attach king stud to bottom plate w/ (2) Simpson DSU-5450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
HEADER STRAP TABLE	
Top Connection	Bottom Connection
LSTA24, 14-1od waver over plate	LSTA24, 14-1od waver under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
M2STA24, 18-1od header to joists	DT12Z
(2) M2STA24, 18-1od header to joists	DT12Z
M2STA24, 18-1od header to joists	HT14

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.236
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS, (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

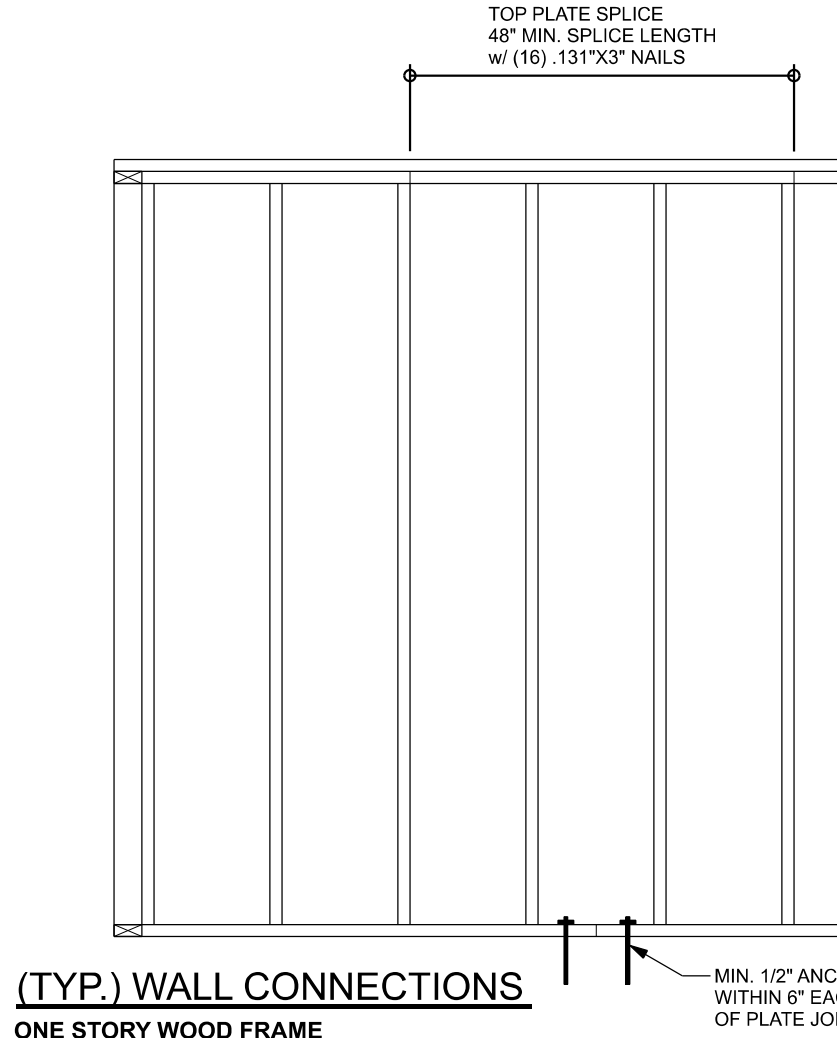
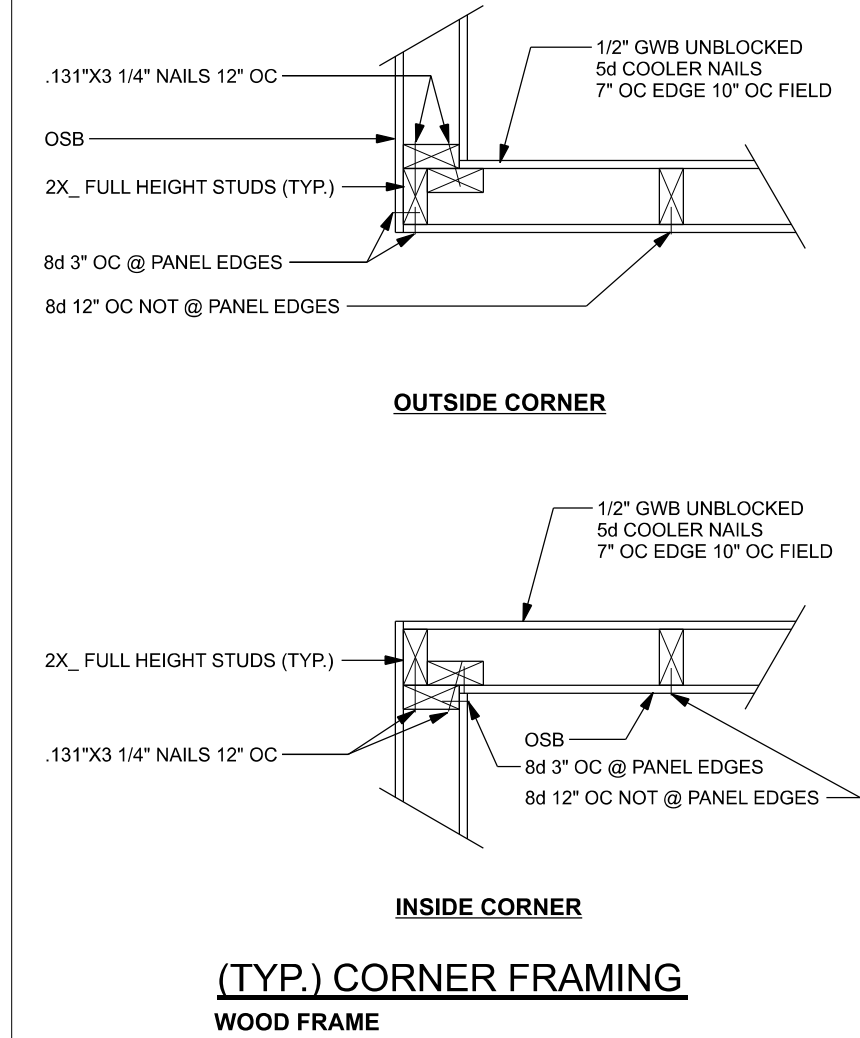
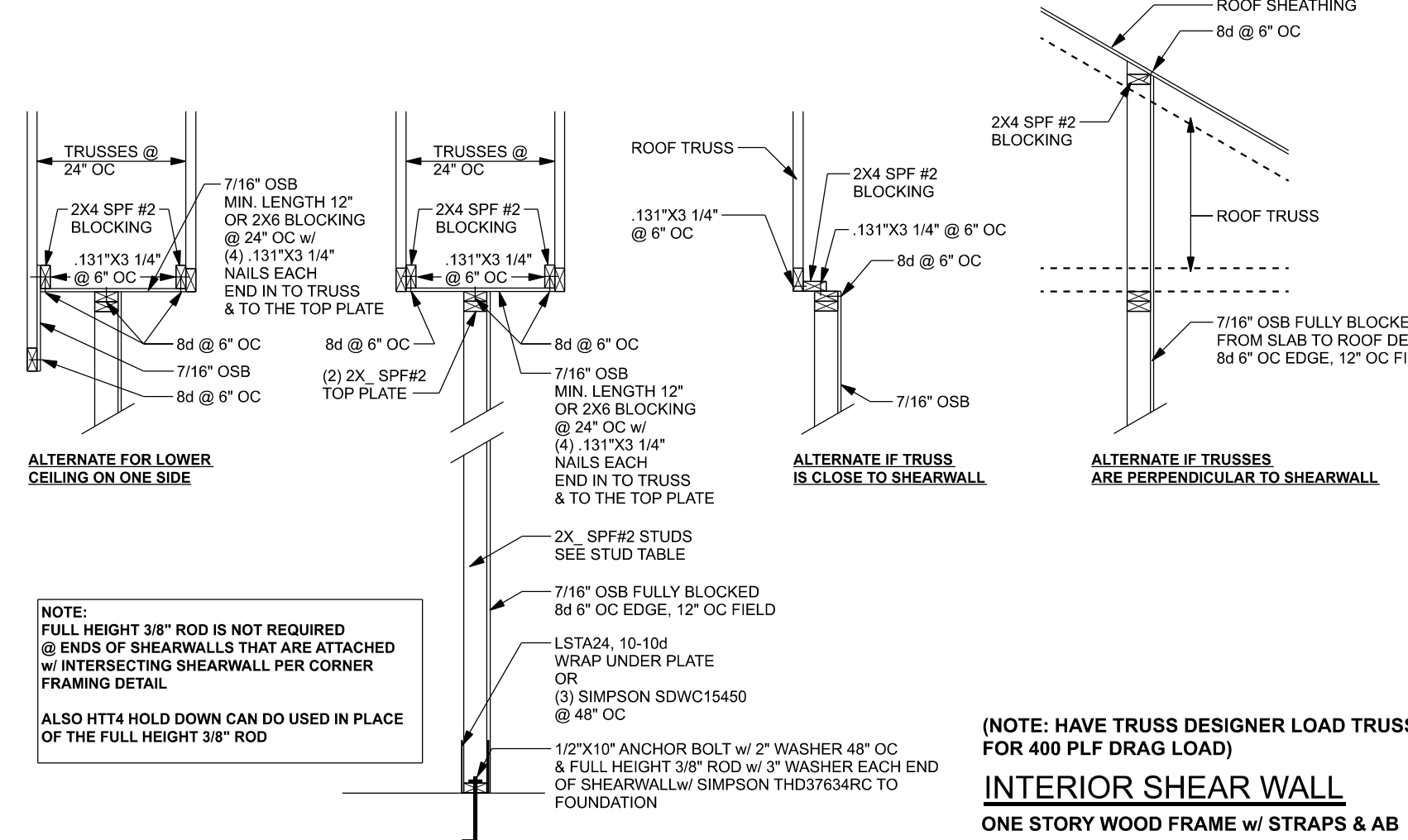


**ALTERNATE IF TRUSS
IS CLOSE TO SHEARWALL**

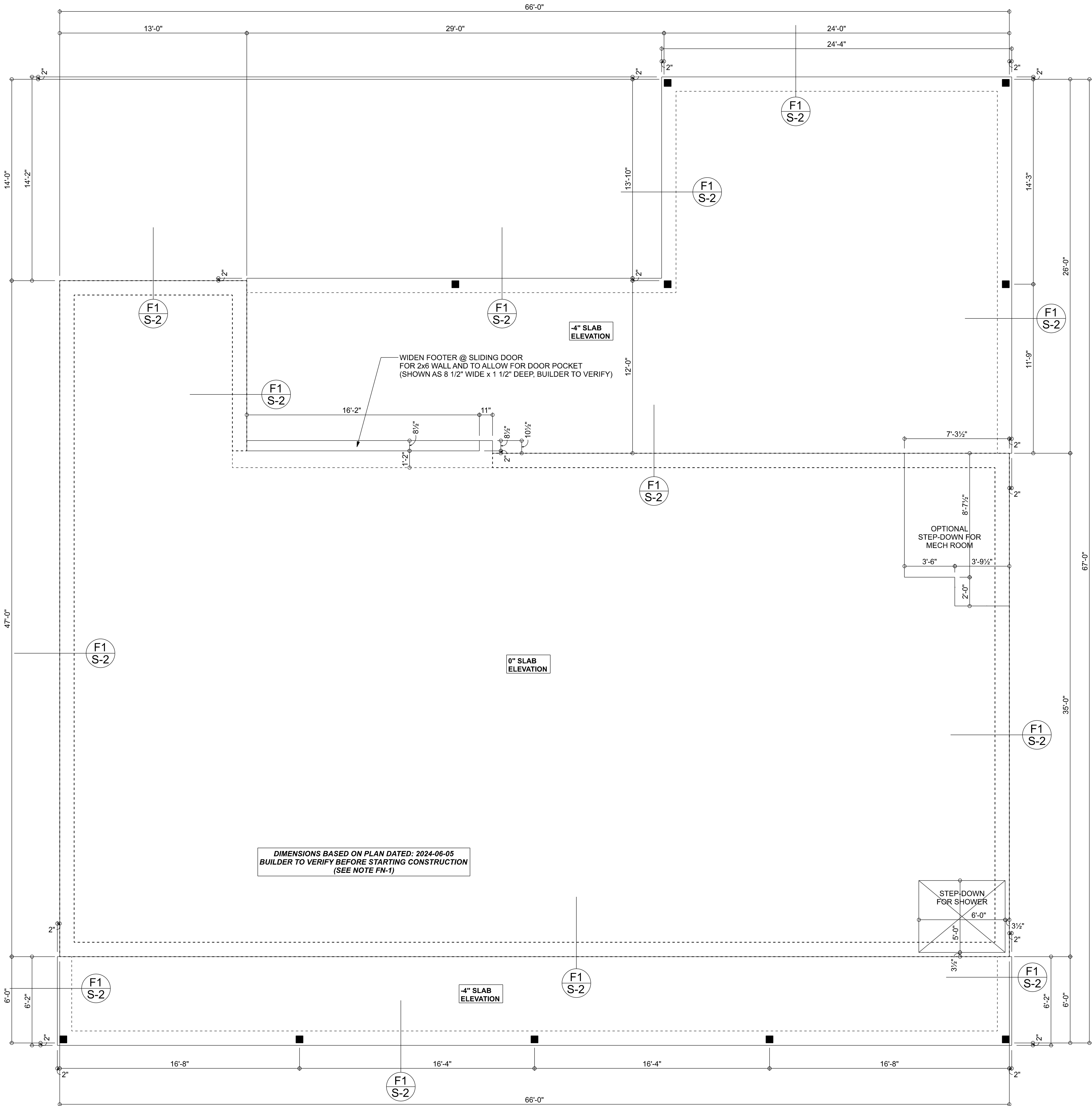
**ALTERNATE IF TRUSSES
ARE PERPENDICULAR TO SHEARWALL**

(NOTE: HAVE TRUSS DESIGNER LOAD TRUSS FOR 400 PLF DRAG LOAD)

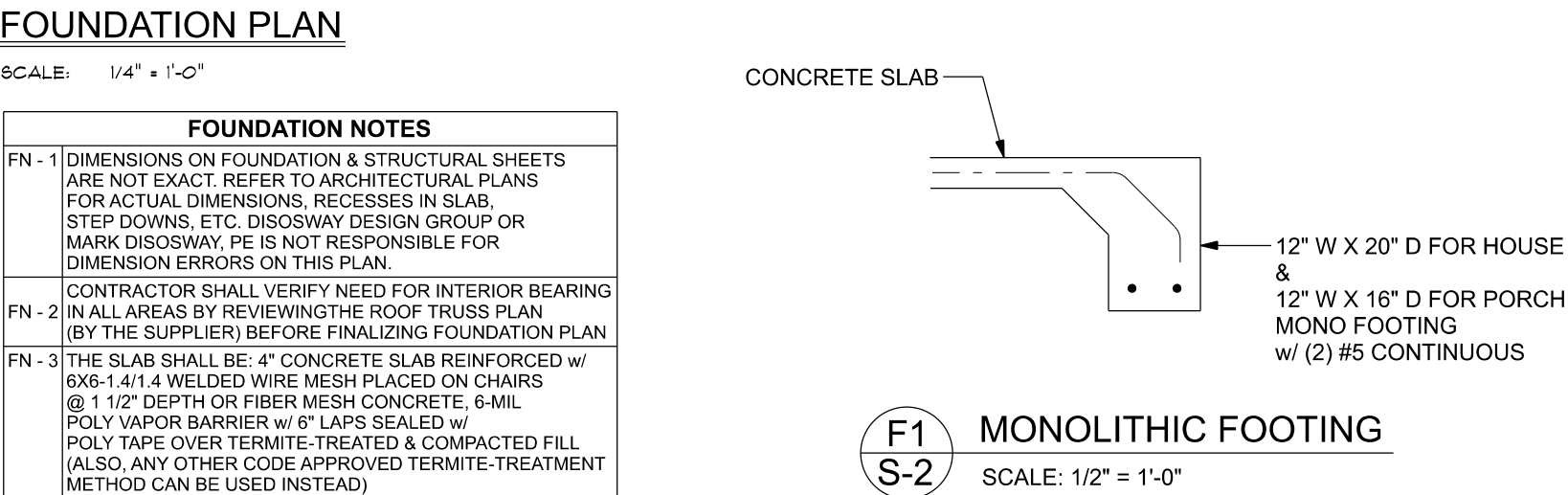
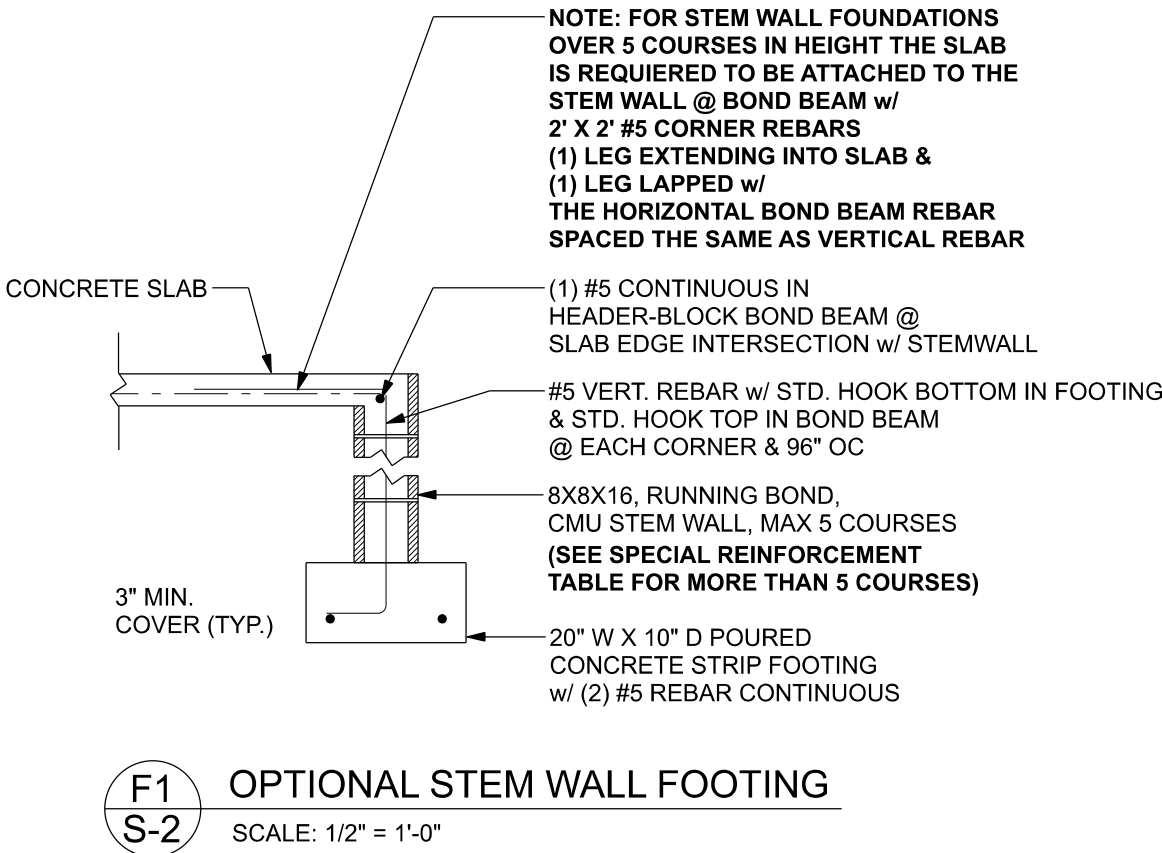
COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vult)	-46.2(Vult)	+42.6(Vult) -57(Vult)



MASONRY NOTE: CONTRACTOR CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/MS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THE REQUIREMENTS OF THIS PROJECT. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.		
	ACI530.1-02 Section	Specifications Requirements
1.6A	Compressive strength	If block bearing value $F_m = 1500$ psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, rebar surface finish, 16" x 16" x 16" minimum, 12" x 12" x 16" or 16" x 16" x column blocks
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x7.5"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM #615, Grade 60, $F_y = 40$ ksi, Lap splice min 40 dia, 25' for #6's
2.4F	Coating for corrosion protection	ASTM #615, sheet metal lites completely embedded in mortar or grout, ASTM #615, CMU, 0.000 0.000 0.0005
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wetness, all casters, sheet metal lites not completely embedded in mortar or grout, ASTM #1155, Class B2, 1.5 oz/sf or 3M55
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



STEM WALL HEIGHT (FEET)	UNBALANCED HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



**BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF
12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL**

Bryan Zecher Homes, Inc

Austin & Kelsey Barber Res.

PROJECT ADDRESS:
SW RACETRACK LANE
LAKE CITY, FLORIDA

Mark Disosway FL PE 53915
This item has been digitally signed and sealed by
Mark Disosway PE on digital signature date.
Printed copies of this document are not considered
signed and sealed and the signature must be
verified on any electronic copies.


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DIMENSIONS:
Stated dimensions supercede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering, comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
240742

S-2
3 SHEETS

