

Julius Lee



RE: 475006 - GIEBEIG - Lot 22 Unit 3 Mayfair

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: GIEBEIG HOMES Project Name: 475006 Model: ST. JOHNS 3 BDRM
Lot/Block: 22 Subdivision: MAYFAIR
Address:
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: BRIAN TRENT GIEBEIG License #: RR282811523
Address: 462 SW FAIRLINGTON CT
City: LAKE CITY State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

FBC 2010/TPI 2007 Design Program: MiTek 20/20 7.3
ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 28 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

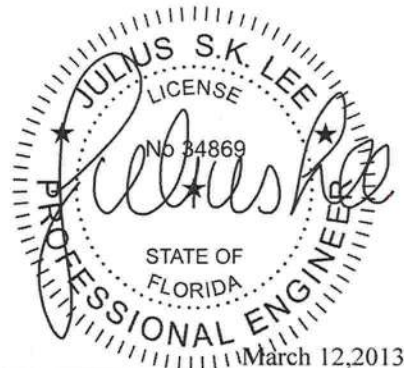
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I6492787	CJ1	3/12/013	18	I6492804	T11	3/12/013
2	I6492788	CJ3	3/12/013	19	I6492805	T12	3/12/013
3	I6492789	CJ5	3/12/013	20	I6492806	T13	3/12/013
4	I6492790	EJ01	3/12/013	21	I6492807	T14	3/12/013
5	I6492791	EJ7	3/12/013	22	I6492808	T15	3/12/013
6	I6492792	HJ01	3/12/013	23	I6492809	T16	3/12/013
7	I6492793	HJ9	3/12/013	24	I6492810	T17	3/12/013
8	I6492794	T01	3/12/013	25	I6492811	T18	3/12/013
9	I6492795	T02	3/12/013	26	I6492812	T19	3/12/013
10	I6492796	T03	3/12/013	27	I6492813	T20	3/12/013
11	I6492797	T04	3/12/013	28	I6492814	T21	3/12/013
12	I6492798	T05	3/12/013				
13	I6492799	T06	3/12/013				
14	I6492800	T07	3/12/013				
15	I6492801	T08	3/12/013				
16	I6492802	T09	3/12/013				
17	I6492803	T10	3/12/013				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Job 475006	Truss CJ1	Truss Type JACK	Qty 18	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:20 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-8twpoPRntODoaGluGAirJlJgNWfsqnDtBQI8vuzbf3T	I6492787
Builders FirstSource, Lake City, FL 32055						

Scale = 1/8" = 1'-0"

Plate Offsets (X, Y): [2-0-6-0, 0-0-14]								
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL) -0.00	7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL) -0.00	7	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)						
						Weight: 7 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=182/0-3-8 (min. 0-1-8), 4=-27/Mechanical, 3=-14/Mechanical
 Max Horz 2=52(LC 12)
 Max Uplift 2=-102(LC 12), 4=-34(LC 2), 3=-18(LC 2)
 Max Grav 2=223(LC 2), 4=21(LC 12), 3=13(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-9)

- 1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint 2, 34 lb uplift at joint 4 and 18 lb uplift at joint 3.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



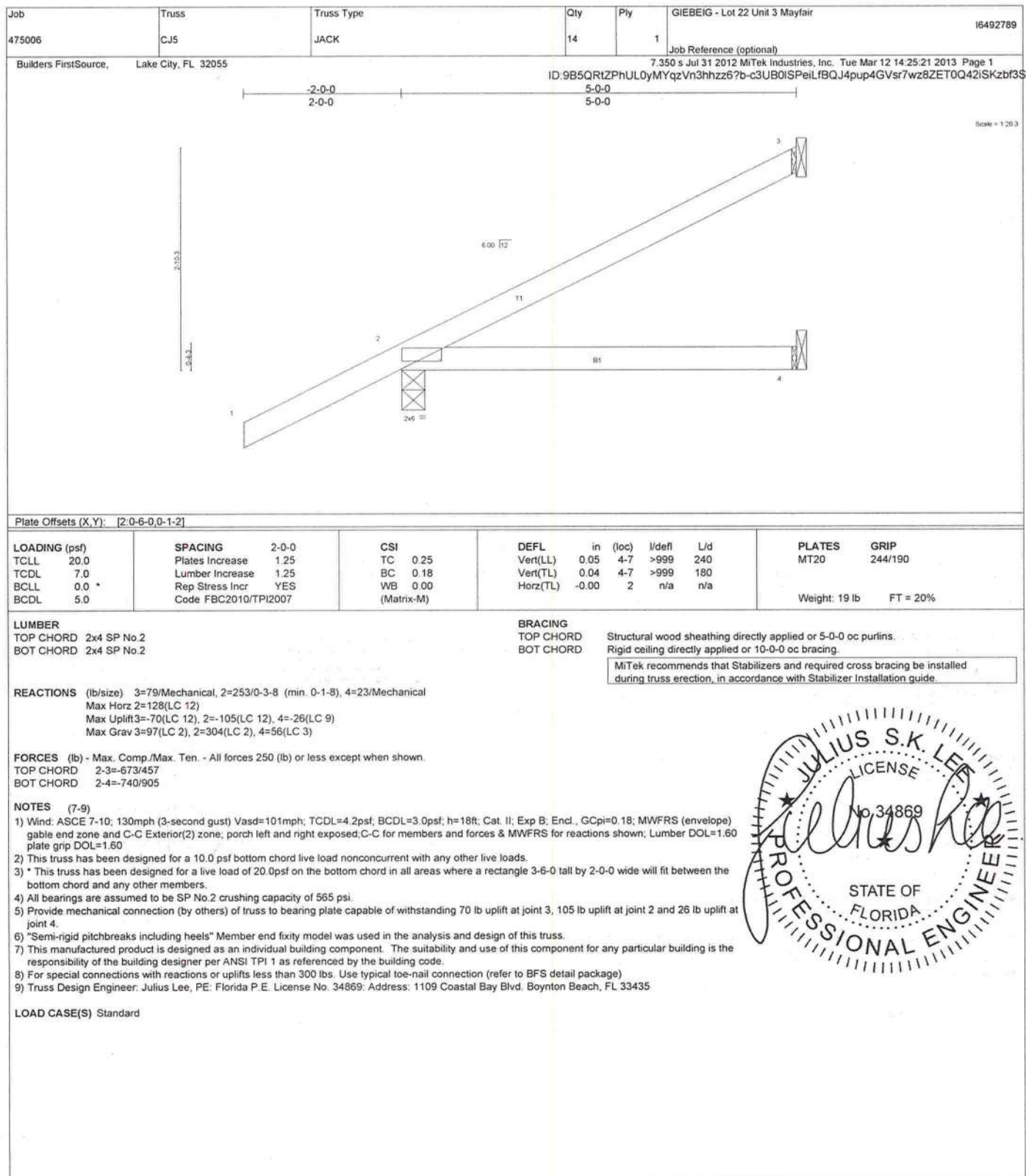
March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M11-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



March 12, 2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information**, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss EJ7	Truss Type MONO TRUSS	Qty 26	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:22 2013 Page 1 ID: 9B5QRIZPhULdyMYqzVn3hhzz6?b-4G2ZD5T1P?TWpauGNbKJojPyXJFahjAfkF_nzbF3R
Builders FirstSource, Lake City, FL 32055					

Scale = 1/2"=1'-0"

Plate Offsets (X,Y): [2'-0"-5'-0", 0'-0"-4"]									
LOADING (psf)	SPACING	2'-0"-0"	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.46	Vert(LL)	0.19	4-7	>436	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.36	Vert(TL)	0.16	4-7	>527		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.01	2	n/a		
BCDL 5.0	Code FBC2010/TP12007		(Matrix-M)						
								Weight: 26 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

REACTIONS (lb/size) 3=113/Mechanical, 2=318/0-3-8 (min. 0-1-8), 4=32/Mechanical

Max Horz 2=115(LC 12)

Max Uplift 3=64(LC 12), 2=-96(LC 9), 4=-34(LC 9)

Max Grav 3=139(LC 2), 2=380(LC 2), 4=79(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1048/1161

BOT CHORD 2-4=-1754/1508

NOTES (7-9)

- 1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 3, 96 lb uplift at joint 2 and 34 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-9 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss HJ9	Truss Type MONO TRUSS	Qty 7	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:24 2013 Page 1 ID: 9B5QRtZPhUL0yMYqzVn3hhzz67b-0eAJenUlxdkD212fV0Nnu8UHb7uFmXyT62GM2fzb3P	I6492793
Builders FirstSource, Lake City, FL 32055						

Scale = 1/4\"

Plate Offsets (X,Y): [2-0-3-7,0-0-13]				
LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TPI2007	CSI TC 0.56 BC 0.54 WB 0.27 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.09 6-7 >999 240 Vert(TL) -0.12 6-7 >998 180 Horz(TL) -0.01 5 n/a n/a	PLATES GRIP MT20 244/190 Weight: 44 lb FT = 20%

LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 REACTIONS (lb/size) 4=132/Mechanical, 2=339/0-5-11 (min. 0-1-8), 5=188/Mechanical Max Horz 2=186(LC 4) Max Uplift 4=124(LC 10), 2=377(LC 4), 5=232(LC 10) Max Grav 4=132(LC 1), 2=486(LC 2), 5=239(LC 3)	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing. <div style="border: 1px solid black; padding: 5px; margin-top: 5px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-11=-536/556, 11-12=-472/549, 3-12=-472/543

BOT CHORD 2-14=-621/476, 14-15=-621/476, 7-15=-621/476, 7-16=-621/476, 6-16=-621/476

WEBS 3-6=-504/658

NOTES (10-12)

- Wind: ASCE 7-10, 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18, MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- Concentrated loads from layout are not present in Load Case(s): #2 Regular Only; #6 MWFRS Wind Left Positive; #7 MWFRS Wind Right Positive; #12 MWFRS 1st Wind Parallel Positive; #13 MWFRS 2nd Wind Parallel Positive; #14 Live Only; #18 MWFRS Wind Left Positive + Regular; #19 MWFRS Wind Right Positive + Regular; #20 MWFRS 1st Wind Parallel Positive + Regular; #21 MWFRS 2nd Wind Parallel Positive + Regular.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 4, 377 lb uplift at joint 2 and 232 lb uplift at joint 5.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb up at 1-5-12, 37 lb up at 1-5-12, 10 lb down and 23 lb up at 4-3-11, 10 lb down and 23 lb up at 4-3-11, and 49 lb down and 75 lb up at 7-1-10, and 49 lb down and 75 lb up at 7-1-10 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, 12 lb down and 33 lb up at 4-3-11, 12 lb down and 33 lb up at 4-3-11, and 42 lb down and 52 lb up at 7-1-10, and 42 lb down and 52 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2



March 12, 2013



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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 475006	Truss T01	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7 350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:25 2013 Page 1 ID:9B5QR1ZPhUL0yMYqzVn3hhzz67b-Urkir7Vvwws4g1dr2ju0QL1QEXDjV0Wcl0vb6zbf3C	I6492794
Builders FirstSource, Lake City, FL 32055						

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.69	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.58	Vert(LL) 0.17 7-9 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.19	Vert(TL) -0.19 7-9 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix-M)	Horz(TL) 0.06 5 n/a n/a		
	Code FBC2010/TPI2007			Weight: 88 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except*
T2: 2x4 SYP No.1

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-8-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-5-1 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1040/0-3-8 (min. 0-1-8), 5=1040/0-3-8 (min. 0-1-8)

Max Horz 2=45(LC 8)

Max Uplift 2=654(LC 8), 5=647(LC 9)

Max Grav 2=1231(LC 2), 5=1231(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2020/1294, 3-16=-1744/1185, 16-17=-1744/1185, 4-17=-1744/1185, 4-5=-2021/1282

BOT CHORD 2-9=-1114/1730, 9-18=-1126/1742, 8-18=-1126/1742, 8-19=-1126/1742, 7-19=-1126/1742, 5-7=-1089/1731

WEBS 3-9=-285/494, 4-7=-298/495

NOTES (11-13)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10, 130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 654 lb uplift at joint 2 and 647 lb uplift at joint 5.

8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 163 lb down and 204 lb up at 7-0-0, 85 lb down and 72 lb up at 9-0-12, and 85 lb down and 72 lb up at 10-11-4, and 203 lb down and 204 lb up at 13-0-0 on top chord, and 258 lb down and 286 lb up at 7-0-0, 49 lb down and 44 lb up at 9-0-12, and 49 lb down and 44 lb up at 10-11-4, and 258 lb down and 286 lb up at 12-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.

13) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44, 3-4=-44, 4-6=-44, 10-13=-10



March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T02	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:26 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-z1I43TWYTE_xlBC2cRPFzZZisxbQETRIaMIT7Yzbf3N	I6492795
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [5-0-6-0,0-2-8], [7-0-2-10,0-1-8]	
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(LL) -0.11 9-14 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Vert(TL) -0.21 9-14 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.03 7 n/a n/a		
				Weight: 103 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD

BOT CHORD

REACTIONS (lb/size) 7=628/0-3-8 (min. 0-1-8), 2=628/0-3-8 (min. 0-1-8)

Max Horz 2=-55(LC 13)

Max Uplift 7=-117(LC 13), 2=-117(LC 12)

Max Grav 7=748(LC 2), 2=748(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1076/497, 3-4=-820/384, 4-5=-707/381, 5-6=-819/383, 6-7=-1075/496

BOT CHORD 2-11=-339/1030, 10-11=-151/707, 9-10=-151/707, 7-9=-345/1093

WEBS 3-11=-299/221, 6-9=-299/221

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 7 and 117 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

March 12,2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T04	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-vPPqU8Yo?rEfXVMQksRj2_fwZkEziBc21gEZCQzbf3L	I6492797																																				
Builders FirstSource, Lake City, FL 32055			7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:28 2013 Page 1																																							
<div style="display: flex; justify-content: space-between;"> <div style="width: 60%;"> </div> <div style="width: 35%; text-align: right;"> <p>Scale = 1/8" = 1'-0"</p> </div> </div>																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:25%;">LOADING (psf)</td> <td style="width:25%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:15%;">DEFL</td> <td style="width:15%;">PLATES</td> <td style="width:20%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 1.25</td> <td>TC 0.75</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1.25</td> <td>BC 0.68</td> <td>Vert(LL) 0.30 9-10 >999 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Stress Incr NO</td> <td>WB 0.87</td> <td>Vert(TL) -0.46 10-11 >781 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TP12007</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.12 7 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td colspan="4"></td> <td colspan="2" style="text-align: right;">Weight: 164 lb FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	Plates Increase 1.25	TC 0.75	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Lumber Increase 1.25	BC 0.68	Vert(LL) 0.30 9-10 >999 240			BCLL 0.0 *	Rep Stress Incr NO	WB 0.87	Vert(TL) -0.46 10-11 >781 180			BCDL 5.0	Code FBC2010/TP12007	(Matrix-M)	Horz(TL) 0.12 7 n/a n/a							Weight: 164 lb FT = 20%	
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REACTIONS (lb/size) 2=1539/0-3-8 (min. 0-2-2), 7=1540/0-3-8 (min. 0-2-2) Max Horz 2=45(LC 8) Max Uplift 2=908(LC 8), 7=908(LC 9) Max Grav 2=1823(LC 2), 7=1823(LC 2)																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3460/1916, 3-16=-3067/1765, 16-17=-3067/1765, 4-17=-3067/1765, 4-18=-4044/2211, 18-19=-4044/2211, 19-20=-4044/2211, 5-20=-4044/2211, 5-21=-3067/1765, 21-22=-3067/1765, 6-22=-3067/1765, 6-7=-3460/1915 BOT CHORD 2-11=-1670/3024, 11-23=-2115/3980, 23-24=-2115/3980, 24-25=-2115/3980, 10-25=-2115/3980, 10-26=-2097/3983, 26-27=-2097/3983, 27-28=-2097/3983, 9-28=-2097/3983, 7-9=-1640/3024 WEBS 3-11=-605/1019, 4-11=-1187/561, 4-10=-100/286, 5-10=-99/285, 5-9=-1189/562, 6-9=-604/1018																																										
NOTES (11-13) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; h=18ft; Cat. II; Exp B; End., GCPI=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 908 lb uplift at joint 2 and 908 lb uplift at joint 7. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 163 lb down and 204 lb up at 7-0-0, 85 lb down and 72 lb up at 9-0-12, 85 lb down and 72 lb up at 11-0-12, 85 lb down and 72 lb up at 13-0-12, 85 lb down and 72 lb up at 15-0-8, 85 lb down and 72 lb up at 17-0-4, 85 lb down and 72 lb up at 19-0-4, and 85 lb down and 72 lb up at 21-0-4, and 163 lb down and 204 lb up at 23-1-0 on top chord, and 258 lb down and 286 lb up at 7-0-0, 49 lb down and 44 lb up at 9-0-12, 49 lb down and 44 lb up at 11-0-12, 49 lb down and 44 lb up at 13-0-12, 49 lb down and 44 lb up at 15-0-8, 49 lb down and 44 lb up at 17-0-4, 49 lb down and 44 lb up at 19-0-4, and 49 lb down and 44 lb up at 21-0-4, and 258 lb down and 286 lb up at 23-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code. 12) Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB. 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																										
LOAD CASE(S) Standard Continued on page 2																																										



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T05	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:30 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-roXbuqZ2WTUNmoVprHUB7PkLTywwABeLU_jgGJzbf3.	I6492798
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [8-0-2-10,0-1-8], [11-0-3-0,0-3-0]				
LOADING (psf) TCCL 20.0 TCCL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.40 BC 0.58 WB 0.45 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.12 10-15 >999 240 Vert(TL) -0.24 10-15 >999 180 Horz(TL) 0.09 8 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 154 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 8=900/0-3-8 (min. 0-1-8), 2=900/0-3-8 (min. 0-1-8)

Max Horz 2=55(LC 12)

Max Uplift 8=138(LC 13), 2=138(LC 12)

Max Grav 8=1071(LC 2), 2=1071(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1748/765, 3-4=-1512/661, 4-5=-1337/633, 5-6=-1337/633, 6-7=-1511/660, 7-8=-1747/765

BOT CHORD 2-12=-575/1548, 11-12=-522/1601, 10-11=-522/1601, 8-10=-582/1560

WEBS 3-12=-266/207, 4-12=-137/399, 5-12=-433/153, 5-10=-433/153, 6-10=-137/399, 7-10=-265/206

NOTES (9-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10, 130mph (3-second gust) Vasd=101mph, TCCL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 8 and 138 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-11 oc purflins.

BOT CHORD Rigid ceiling directly applied or 7-8-11 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013

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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 475006	Truss T07	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:32 2013 Page 1 ID: 9B5QRtZPhUL0yMYqzVn3hhzz6?b-nBfLJWbJ24k506fBzhWfCqpeVLc2e4jeYlCnLCzbf3H	I6492800
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [4:0-6:0,0-2-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	0.19	11	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.60	Vert(TL)	-0.37	10-11	>825
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.54	Horz(TL)	0.16	7	n/a
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)				
						Weight: 131 lb FT = 20%	

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-4-14 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 5-7-5 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 2-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=685/0-3-8 (min. 0-1-8), 7=680/0-3-8 (min. 0-1-8)
 Max Horz 1=105(LC 12)
 Max Uplift 1=118(LC 12), 7=95(LC 13)
 Max Grav 1=812(LC 2), 7=805(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-2668/1184, 2-3=-1232/558, 3-4=-1064/555, 4-5=-1072/508
 BOT CHORD 1-11=-1116/2422, 10-11=-1060/2294, 9-10=-311/896, 8-9=-311/896, 7-8=-408/940
 WEBS 2-11=-231/632, 2-10=-1326/706, 3-10=-67/273, 4-10=-112/280, 5-7=-1035/478

NOTES (10-12)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No 2 crushing capacity of 565 psi.
 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 1 and 95 lb uplift at joint 7.
 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 12) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T09	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair i6492802																																				
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:34 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-kZm5kCcZah_pFQpa46Y8IFuyi9HZ6vExPchuP4zbf3F																																						
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LUMBER TOP CHORD 2x4 SYP No.1 BOT CHORD 2x6 SYP No.2 WEBS 2x4 SP No.3 OTHERS 2x6 SYP No.2			BRACING TOP CHORD Structural wood sheathing directly applied or 2-11-2 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 5-6-5 oc bracing. WEBS T-Brace: 2 X 6 SYP No.2 - 6-8 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.																																						
REACTIONS (lb/size) 8=1517/0-3-8 (min. 0-2-2), 2=1507/0-3-8 (min. 0-2-2) Max Horz 2=116(LC 8) Max Uplift 8=885(LC 5), 2=846(LC 8) Max Grav 8=1796(LC 2), 2=1786(LC 2)																																									
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-3377/1804, 3-14=-2991/1654, 14-15=-2991/1654, 4-15=-2991/1654, 4-16=-3883/2009, 16-17=-3883/2009, 17-18=-3883/2009, 5-18=-3883/2009, 5-19=-2911/1462, 19-20=-2911/1462, 20-21=-2911/1462, 6-21=-2911/1462 BOT CHORD 2-11=-1631/2950, 11-25=-2014/3874, 25-26=-2014/3874, 26-27=-2014/3874, 10-27=-2014/3874, 10-28=-1878/3730, 28-29=-1878/3730, 29-30=-1878/3730, 30-31=-1878/3730, 9-31=-1878/3730, 9-32=-1123/2278, 32-33=-1123/2278, 33-34=-1123/2278, 34-35=-1123/2278, 8-35=-1123/2278 WEBS 3-11=-550/984, 4-11=-1146/468, 5-10=-194/342, 5-9=-1108/562, 6-9=-597/1113, 6-8=-2700/1331																																									
NOTES (12-14) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10, 130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 885 lb uplift at joint 8 and 846 lb uplift at joint 2. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 163 lb down and 204 lb up at 7-0-0, 85 lb down and 72 lb up at 9-0-12, 85 lb down and 72 lb up at 11-0-12, 85 lb down and 72 lb up at 13-0-12, 85 lb down and 72 lb up at 15-0-12, 85 lb down and 72 lb up at 17-0-12, 85 lb down and 72 lb up at 19-0-12, 85 lb down and 72 lb up at 21-0-12, 85 lb down and 72 lb up at 23-0-12, 85 lb down and 72 lb up at 25-0-12, and 85 lb down and 72 lb up at 27-0-12, and 85 lb down and 72 lb up at 29-0-12 on top chord, and 258 lb down and 286 lb up at 7-0-0, 49 lb down and 44 lb up at 9-0-12, 49 lb down and 44 lb up at 11-0-12, 49 lb down and 44 lb up at 13-0-12, 49 lb down and 44 lb up at 15-0-12, 49 lb down and 44 lb up at 17-0-12, 49 lb down and 44 lb up at 19-0-12, 49 lb down and 44 lb up at 21-0-12, 49 lb down and 44 lb up at 23-0-12, 49 lb down and 44 lb up at 25-0-12, and 49 lb down and 44 lb up at 27-0-12, and 49 lb down and 44 lb up at 29-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. Continued on page 2																																									



March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T10	Truss Type MONO HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:35 2013 Page 1 ID.9B5QRtZPHUL0yMYqzVn3hhzz67b-CmKUyYdBL?6gtaOmeq3NqSRAoZZWrs24eGRRxXzbf3E	I6492803
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [6-0-4-0,0-3-0]				
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.49 BC 0.88 WB 0.51 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.29 8-9 >999 240 Vert(TL) -0.53 8-9 >674 180 Horz(TL) 0.08 8 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 155 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-6 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-8-6 oc bracing.

WEBS 1 Row at midpt 5-11, 6-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=802/0-3-8 (min. 0-1-8), 2=903/0-3-8 (min. 0-1-8)
 Max Horz 2=143(LC 12)
 Max Uplift 8=183(LC 9), 2=134(LC 9)
 Max Grav 8=950(LC 2), 2=1074(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1753/721, 3-4=-1523/620, 4-5=-1342/597, 5-6=-1411/571
 BOT CHORD 2-11=-772/1557, 10-11=-697/1587, 9-10=-697/1587, 8-9=-501/1153
 WEBS 3-11=-256/201, 4-11=-101/387, 5-11=-407/126, 5-9=-290/205, 6-9=-115/465, 6-8=-1349/593

NOTES (9-11)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 183 lb uplift at joint 8 and 134 lb uplift at joint 2.
 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T14	Truss Type SPECIAL	Qty 2	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:41 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b=0wlcbyxrtbVrw_4An4jhCO_efF7Nz0Bu9Azb38	I6492807
Builders FirstSource, Lake City, FL 32055						

Scale = 1/621

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.49	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.76	Vert(LL) 0.25 14-15 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.64	Vert(TL) -0.50 14-15 >724 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.27 9 n/a n/a		
				Weight: 176 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

WEBS 2x4 SP No.3 *Except*

W10: 2x4 SP No.2

REACTIONS (lb/size) 2=943/0-3-8 (min. 0-1-8), 9=859/0-3-8 (min. 0-1-8)

Max Horz 2=130(LC 12)

Max Uplift 2=171(LC 12), 9=127(LC 13)

Max Grav 2=1074(LC 2), 9=957(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3385/1354, 3-4=-3374/1460, 4-5=-1706/752, 5-6=-1713/778, 6-7=-1703/728

BOT CHORD 2-15=-1224/3057, 14-15=-733/1920, 14-20=-327/1119, 20-21=-327/1119, 13-21=-327/1119, 12-13=-545/1501, 10-12=-341/883, 9-10=-395/990

WEBS 4-15=-635/1446, 4-14=-748/429, 5-14=-290/745, 5-13=-248/556, 6-13=-284/227, 7-12=-585/1524, 7-10=-1175/520, 7-9=-1278/519

NOTES (9-11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCPI=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2 and 127 lb uplift at joint 9.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing. Except: 9-5-0 oc bracing: 10-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T16	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	I6492809
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Builders FirstSource, Lake City, FL 32055 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:43 2013 Page 1
 ID:9B5QRiZPhUL0yMYqzVn3hhzz67b-zlpVdHjCSS7Xqo?J6VDF98mYJnKdj2JGUVNsD3zbf36

Scale = 1/8" = 1'-0"

Plate Offsets (X,Y):	[3 0-3-0,0-3-0], [4 0-6-0,0-2-8], [6 0-3-0,0-3-0], [7 0-2-10,0-1-8]				
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.46	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(LL) -0.29 12-13 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.61	Vert(TL) -0.63 12-13 >571 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.21 7 n/a n/a		
				Weight: 158 lb FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-8 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-5-4 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=807/0-3-8 (min. 0-1-8), 7=905/0-3-8 (min. 0-1-8)

Max Horz 1=89(LC 12)

Max Uplift 1=141(LC 12), 7=153(LC 13)

Max Grav 1=957(LC 2), 7=1077(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3238/1333, 2-3=-3240/1448, 3-4=-1395/644, 4-5=-1175/613, 5-6=-1354/628, 6-7=-1758/741

BOT CHORD 1-13=-1167/2975, 12-13=-683/1851, 11-12=-324/1204, 10-11=-324/1204, 9-10=-549/1556, 7-9=-548/1570

WEBS 3-13=-635/1445, 3-12=-794/438, 4-12=-194/529, 5-10=-136/345, 6-10=-455/265

NOTES (10-12)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 1 and 153 lb uplift at joint 7.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T18	Truss Type Hip Truss	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:45 2013 Page 1 ID: 9B5QRIZPHUL0yMYqzVn3hhzz6?b-vhxG2yIT_4NF469hDwFjEZsxmbA5B4xYxpszlxbzbf34	I6492811
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [3-0-6-0-0-2-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(LL) 0.01 7-8 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.03	Vert(TL) -0.01 7-8 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.00 5 n/a n/a		
				Weight: 40 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 2=311/0-3-8 (min. 0-1-8), 5=311/0-3-8 (min. 0-1-8)

Max Horz 2=25(LC 12)

Max Uplift 2=190(LC 8), 5=205(LC 9)

Max Grav 2=371(LC 2), 5=371(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-326/296, 3-15=-262/302, 4-15=-262/302, 4-5=-327/326

BOT CHORD 2-8=-236/260, 8-16=-241/262, 7-16=-241/262, 5-7=-255/261

NOTES (11-13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10, 130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf, h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 2 and 205 lb uplift at joint 5.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3 lb down and 46 lb up at 3-0-0, and 3 lb down and 46 lb up at 4-2-0, and 8 lb down and 116 lb up at 5-4-0 on top chord, and 13 lb down and 62 lb up at 3-0-0, and 3 lb down and 25 lb up at 4-2-0, and 13 lb down and 62 lb up at 5-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB
- 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44, 3-4=-44, 4-6=-44, 9-12=-10

Concentrated Loads (lb)

Vert: 3=2(B) 4=1(B) 8=0(B) 7=0(B) 15=2(B) 16=-1(B)

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T20	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:46 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-NtVeFIm5INV6hGkunemynnO1d?RYwSkiATbWqOzbf33	I6492813
Builders FirstSource, Lake City, FL 32055						

Plate Offsets (X,Y): [2-0-3-0,0-1-1], [4-0-3-0,0-1-1]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.58	Vert(LL) -0.05 6-8 >999 240	MT20 244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.40	Vert(TL) -0.09 6-8 >999 180		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.38	Horz(TL) 0.02 4 n/a n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)			Weight: 68 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SYP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=813/0-3-8 (min. 0-1-8), 4=813/0-3-8 (min. 0-1-8)

Max Horz 2=45(LC 12)

Max Uplift 2=273(LC 5), 4=273(LC 4)

Max Grav 2=967(LC 2), 4=967(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1604/463, 3-4=-1604/463

BOT CHORD 2-6=-366/1421, 4-6=-366/1421

WEBS 3-6=-361/1159

NOTES (11-13)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope); porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2 and 273 lb uplift at joint 4.
- Girder carries hip end with 7-0-0 end setback.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1017 lb down and 247 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44, 3-5=-44, 2-4=-10

Concentrated Loads (lb)

Vert: 6=-693(F)



March 12, 2013



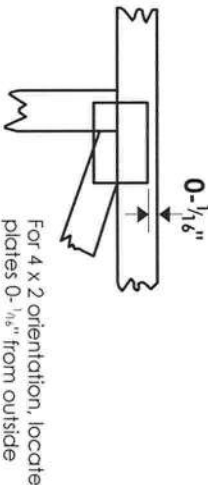
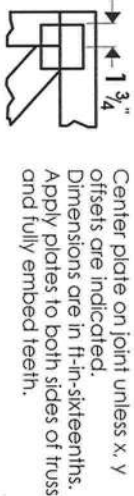
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-87 and 8CSI1 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

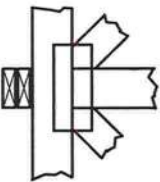
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



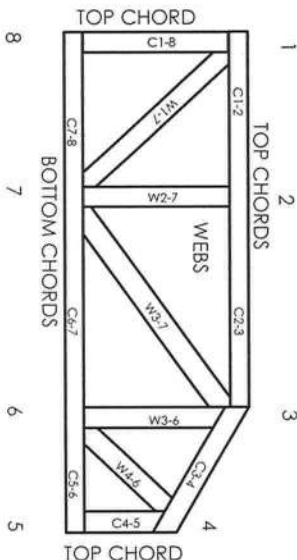
BEARING



Industry Standards:

ANSI/TP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

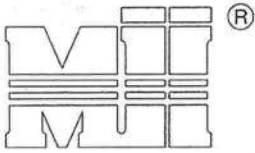
Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TP11.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP11.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP11 Quality Criteria.

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



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Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.

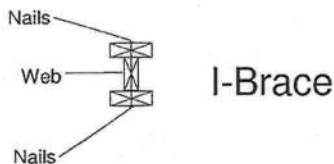
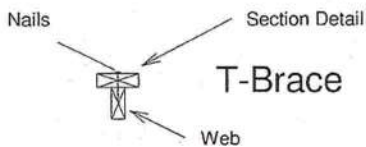
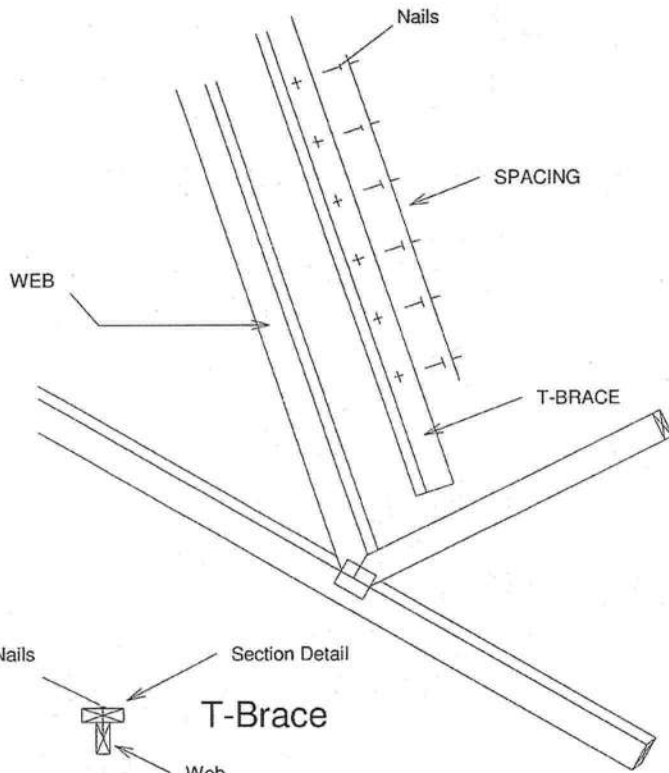
Note: Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

Brace Size for One-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

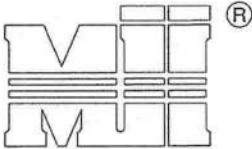
Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size for Two-Ply Truss		
Specified Continuous Rows of Lateral Bracing		
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.



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NOTES:

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

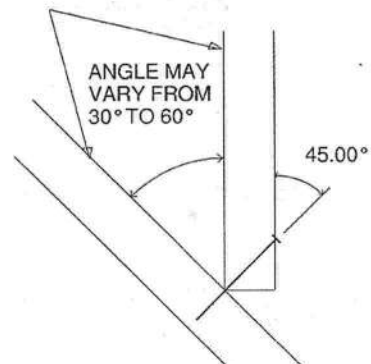
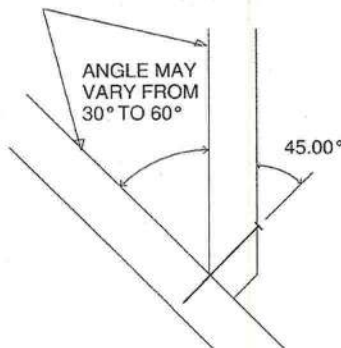
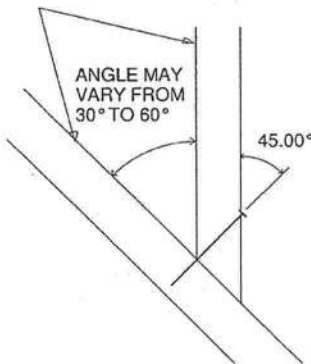
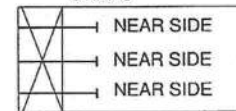
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

THIS DETAIL APPLICABLE TO THE
 THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR
 ILLUSTRATION PURPOSES ONLY

SIDE VIEW

3 NAILS

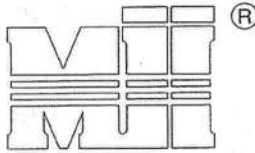


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FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10



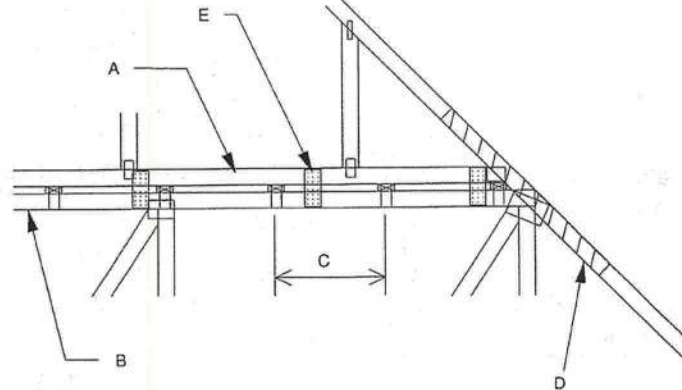
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MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE : 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

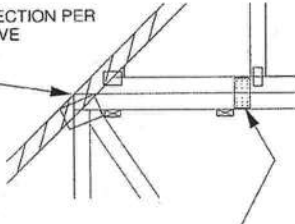
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



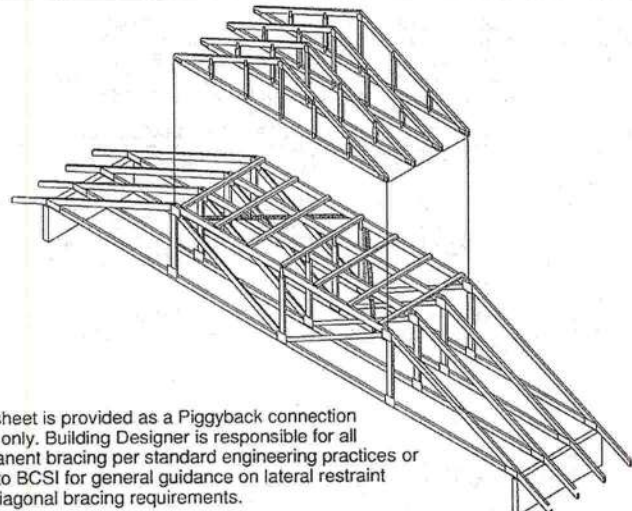
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER
NOTE D ABOVE

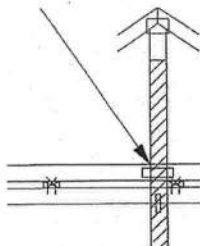


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK

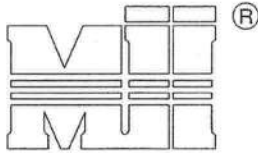


FOR LARGE CONCENTRATED LOADS APPLIED
TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) ATTACH 2 X 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



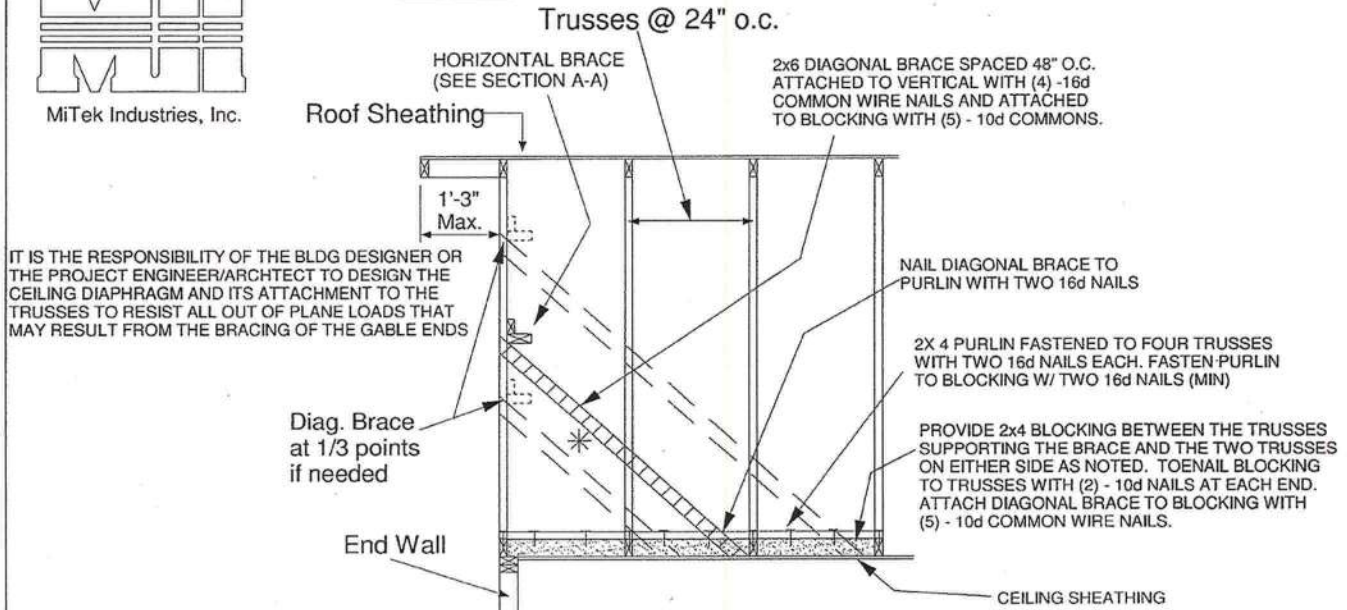
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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

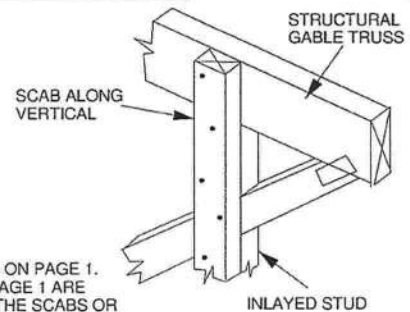
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

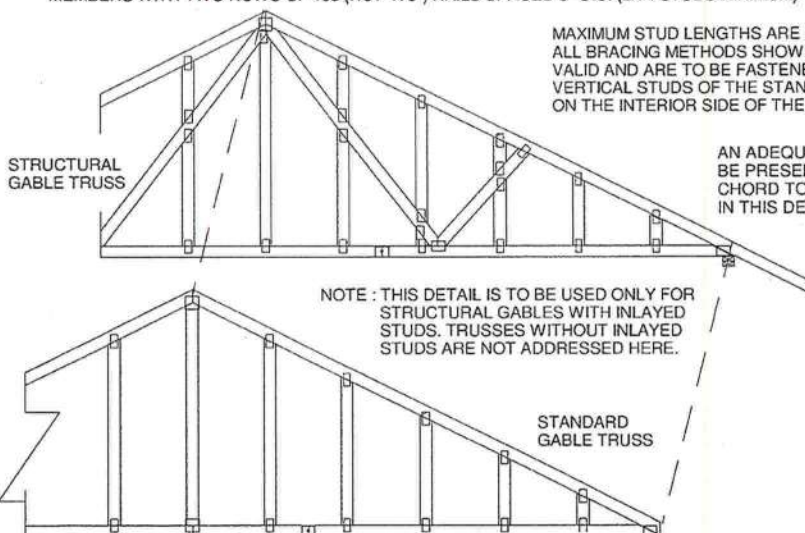
METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (.131" X 3") NAILS SPACED 6" O.C.
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)



MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.



AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.



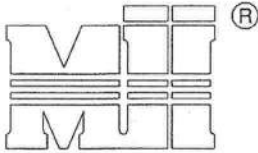
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FEBRUARY 14, 2012

Standard Gable End Detail

ST-GE130-001

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MiTek Industries, Inc.

Typical 2x4 L-Brace Nailed To
2x Verticals w/10d Nails, 6" o.c.

Vertical Stud

SECTION B-B

TRUSS GEOMETRY AND CONDITIONS
SHOWN ARE FOR ILLUSTRATION ONLY.

Varies to Common Truss

SEE INDIVIDUAL MITTEK ENGINEERING
DRAWINGS FOR DESIGN CRITERIAPROVIDE 2x4 BLOCKING BETWEEN THE FIRST
TWO TRUSSES AS NOTED. TOENAIL BLOCKING
TO TRUSSES WITH (2) - 10d NAILS AT EACH END.
ATTACH DIAGONAL BRACE TO BLOCKING WITH
(5) - 10d COMMON WIRE NAILS.(4) - 8d NAILS MINIMUM, PLYWOOD
SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

Diag. Brace
at 1/3 points
if needed

End Wall

HORIZONTAL BRACE
(SEE SECTION A-A)

NOTE:

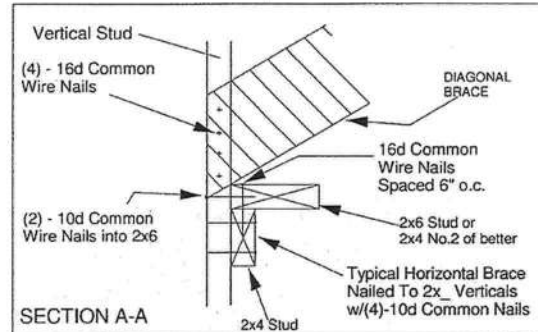
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS $L/240$.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-0-7	4-3-2	6-0-4	8-0-15	12-1-6
2x4 SPF Std/Stud	16" O.C.	3-7-0	3-8-4	5-2-10	7-1-15	10-8-15
2x4 SPF Std/Stud	24" O.C.	2-11-1	3-0-2	4-3-2	5-10-3	8-9-4

- * Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 l-braces attached to both edges. Fasten T and l braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE : 1.60

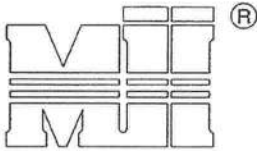
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



SECTION A-A



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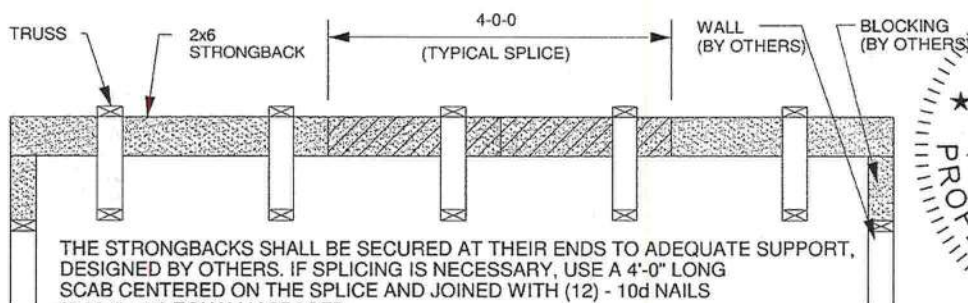
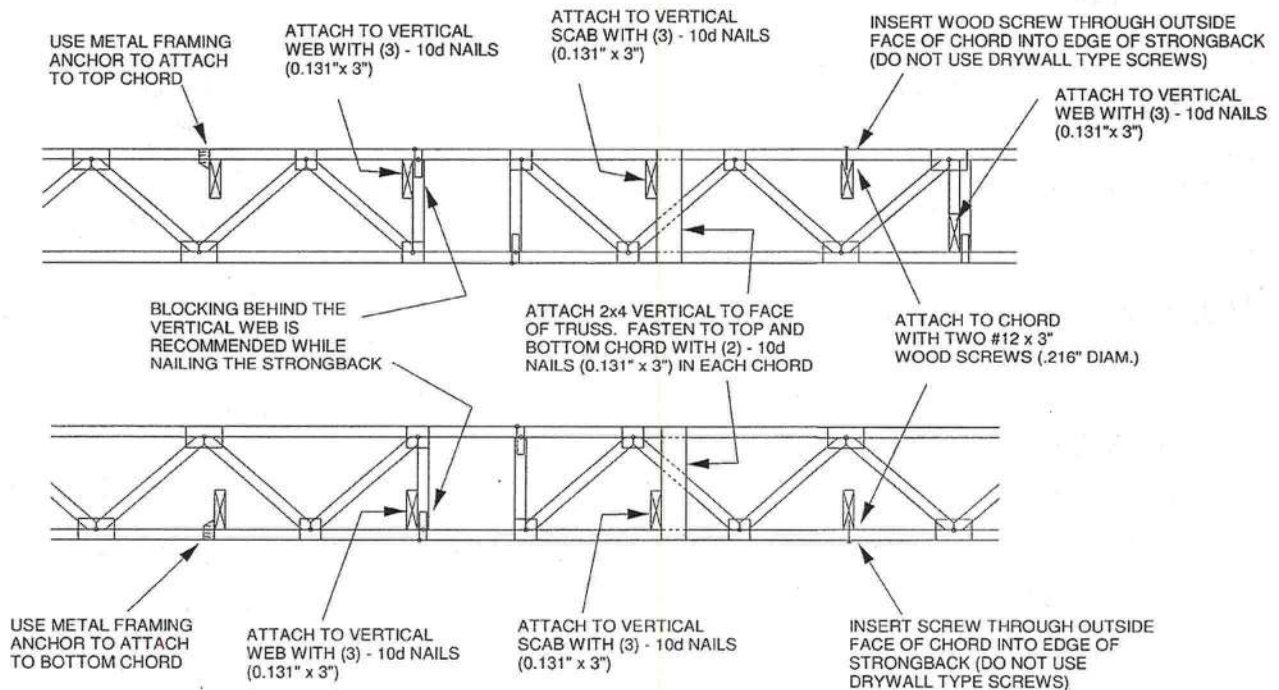
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TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.



ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

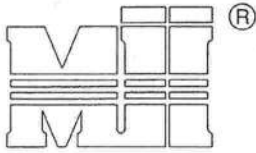


1109 COASTAL BAY
BOYNTON BC, FL 33435

FEBRUARY 14, 2012

TRUSSED VALLEY SET DETAIL

ST-VALLEY HIGH WIND1

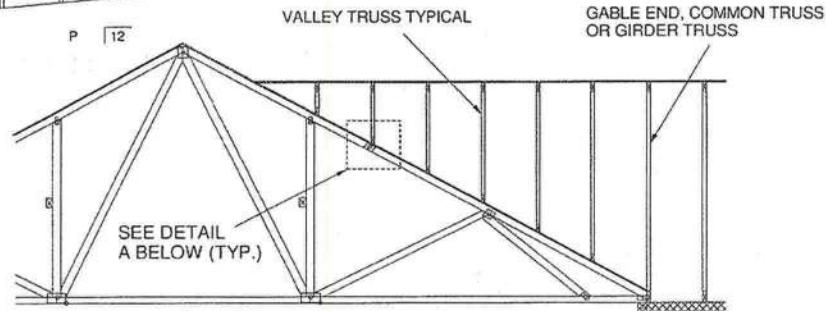
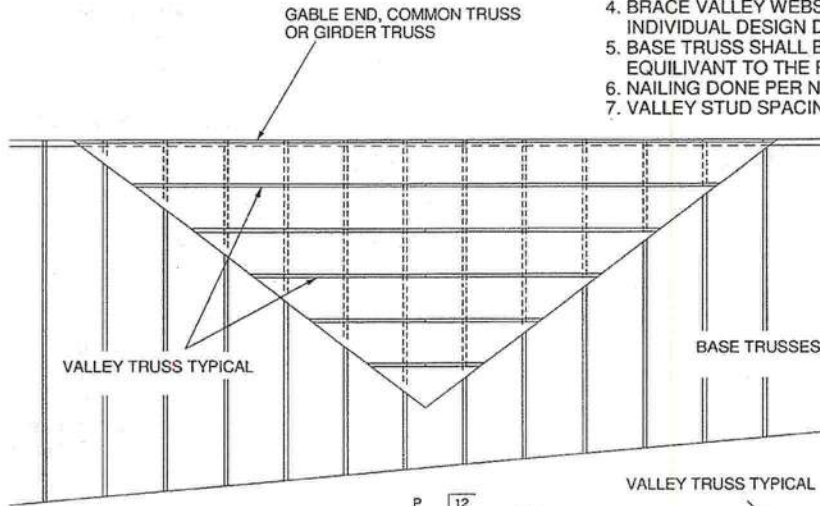


MiTek Industries, Inc.

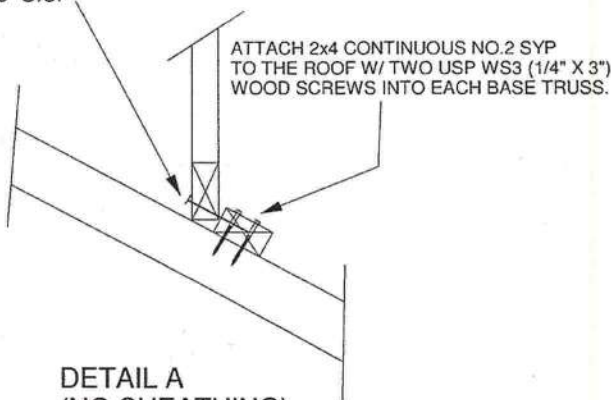
MiTek Industries, Chesterfield, MO Page 1 of 1

GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUIVARIANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



SECURE VALLEY TRUSS
W/ ONE ROW OF 10d
NAILS 6" O.C.



DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH
WIND DESIGN PER ASCE 7-10 160 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 8 PSF
ON THE TRUSSES



1109 COASTAL BAY
BOYNTON BC, FL 33435

Job 475006	Truss T21	Truss Type COMMON	Qty 2	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	I6492814
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Builders FirstSource, Lake City, FL 32055 7350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:47 2013 Page 1
 ID: 9B5QRIZPhUL0yMYqzVn3hhzz6?b-r330TemjWhdzJQJ4LLHBJ_xEzOpLfzdrP7L4Nqzb32

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [4-0-2-10,0-1-8]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	in (loc) l/defl L/d	MT20 244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.30	Vert(LL) 0.11 6-12 >999 240	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.08	Vert(TL) 0.09 6-12 >999 180	
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)	Horz(TL) 0.01 4 n/a n/a	
Weight: 56 lb FT = 20%					

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 2=466/0-3-8 (min. 0-1-8), 4=466/0-3-8 (min. 0-1-8)

Max Horz 2=45(LC 16)

Max Uplift 2=156(LC 9), 4=156(LC 8)

Max Grav 2=556(LC 2), 4=556(LC 2)

FORCES (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-964/914, 3-4=-972/918

BOT CHORD 2-6=-1157/1271, 4-6=-1177/1314

WEBS 3-6=-317/209

NOTES (8-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft, Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 2 and 156 lb uplift at joint 4.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE

LICENSE

No. 34869

PROFESSIONAL ENGINEER

STATE OF FLORIDA

March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T19	Truss Type COMMON	Qty 2	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	16492812
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Builders FirstSource, Lake City, FL 32055 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:46 2013 Page 1
 ID: 9B5QRtZPhUL0yMYqzVn3hhzz6?b=NtVeFIm5INV6hGkunemynnO7p?VlwXxiATbWqOzbf33

Plate Offsets (X,Y): [2'-0"-6'-0",0'-0"-14'] [4'-0"-6'-0",0'-0"-14']	
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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2'-0"-0" Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.25 BC 0.13 WB 0.05 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.02 5-8 >999 240 Vert(TL) 0.02 5-8 >999 180 Horz(TL) -0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 33 lb FT = 20%
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LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0"-0" oc purlins.

BOT CHORD Rigid ceiling directly applied or 10'-0"-0" oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=221/0-3-8 (min. 0-1-8), 2=317/0-3-8 (min. 0-1-8)

Max Horz 2=68(LC 16)

Max Uplift 4=-100(LC 8), 2=-135(LC 12)

Max Grav 4=262(LC 2), 2=379(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-558/406, 3-4=-352/476

BOT CHORD 2-5=-321/677, 4-5=-589/442

NOTES (8-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint 4 and 135 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



March 12, 2013

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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

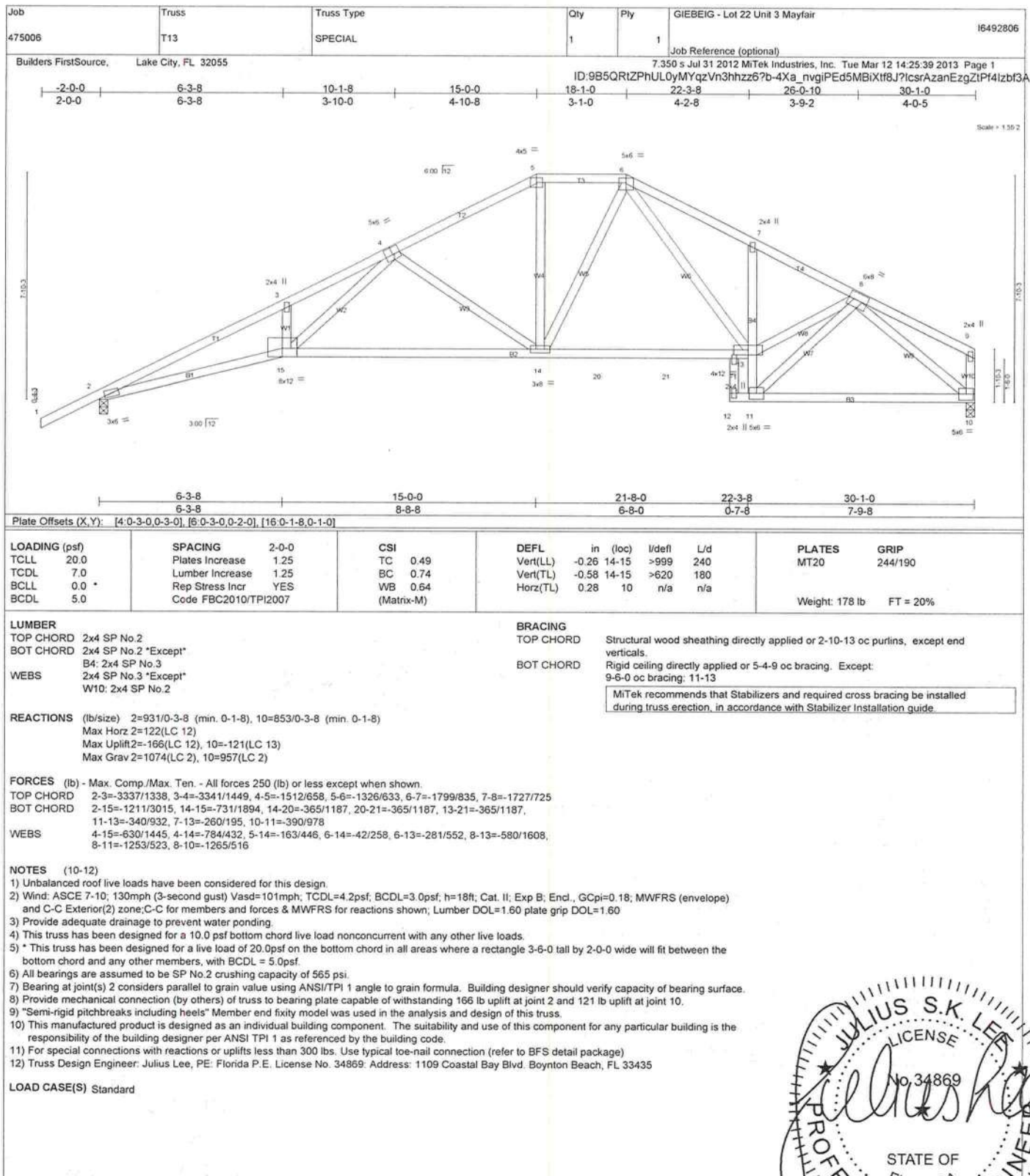
Job 475006	Truss T17	Truss Type SPECIAL	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) ID:9B5QRIZPhULQyMYqzVn3nhzz6?b-RUNtrdkqDmFOSyaVgDkUhmJ3BgZSWMPi96QmVzbf35	I6492810																																																						
Builders FirstSource, Lake City, FL 32055		7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:44 2013 Page 1																																																										
<div style="display: flex; justify-content: space-between;"> <div style="width: 60%;"> </div> <div style="width: 35%; text-align: right;"> <p>Scale = 1/8" = 1'-0"</p> </div> </div>																																																												
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">6-3-8</td> <td style="width:15%;">10-0-11</td> <td style="width:15%;">15-0-8</td> <td style="width:15%;">16-10-8</td> <td style="width:15%;">19-10-8</td> <td style="width:15%;">24-8-8</td> <td style="width:15%;">30-1-0</td> <td style="width:15%;">32-1-0</td> </tr> <tr> <td>6-3-8</td> <td>3-9-3</td> <td>4-11-13</td> <td>1-10-0</td> <td>3-0-0</td> <td>4-10-0</td> <td>5-4-8</td> <td>2-0-0</td> </tr> </table>							6-3-8	10-0-11	15-0-8	16-10-8	19-10-8	24-8-8	30-1-0	32-1-0	6-3-8	3-9-3	4-11-13	1-10-0	3-0-0	4-10-0	5-4-8	2-0-0																																						
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Plate Offsets (X,Y): [3-0-3-0,0-3-0], [8-0-1-8,Edge]																																																												
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REACTIONS (lb/size) 1=806/0-3-8 (min. 0-1-8), 8=906/0-3-8 (min. 0-1-8) Max Horz 1=88(LC 12) Max Uplift 1=142(LC 12), 8=167(LC 13) Max Grav 1=956(LC 2), 8=1077(LC 2)																																																												
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=3235/1341, 2-3=3236/1455, 3-4=1425/666, 4-5=1371/679, 5-6=1289/641, 6-7=1467/665, 7-8=1748/791 BOT CHORD 1-13=1175/2972, 12-13=699/1870, 11-12=433/1391, 10-11=433/1391, 8-10=599/1632 WEBS 3-13=628/1427, 3-12=782/429, 4-12=452/992, 5-12=499/263, 6-10=152/402, 7-10=320/246																																																												
NOTES (10-12) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph, TCDL=4.2psf, BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 1 and 167 lb uplift at joint 8. 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																																												
LOAD CASE(S) Standard																																																												



March 12, 2013

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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435



March 12, 2013

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Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

March 12, 2013

Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 22 Unit 3 Mayfair
475006	T09	MONO HIP	1	1	I6492802

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:34 2013 Page 2
ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-kZm5kCcZah_pFQpa46Y8IFuyi9HZ6vExPchuP4zb3F

NOTES (12-14)

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

13) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-44, 3-7=-44, 2-8=-10

Concentrated Loads (lb)

Vert: 3=-135(B) 11=-187(B) 14=-69(B) 15=-69(B) 16=-69(B) 17=-69(B) 18=-69(B) 19=-69(B) 20=-69(B) 21=-69(B) 22=-69(B) 23=-69(B) 24=-69(B) 25=-22(B) 26=-22(B) 27=-22(B) 28=-22(B) 29=-22(B) 30=-22(B) 31=-22(B) 32=-22(B) 33=-22(B) 34=-22(B) 35=-22(B)



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T08	Truss Type SPECIAL	Qty 4	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	16492801
Builders FirstSource, Lake City, FL 32055			7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:33 2013 Page 1			
			ID:9B5QRIZPhUL0yMYqzVn3hhzz67b-FNDjXsbxpOsyeGEOwP1v11MrWvaNUXnByyKtezb3G			

Job 475006	Truss T06	Truss Type HIP	Qty 1	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	I6492799																																				
Builders FirstSource, Lake City, FL 32055		7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:31 2013 Page 1 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-J_5z6AahHmcEOy4?P_?QgcGXhylvhhVjeTEolzbf3i																																								
<div style="display: flex; justify-content: space-between;"> <div style="width: 45%;"> <p>2-0-0 5-7-11 11-0-0 15-0-8 19-1-0 24-5-5 30-1-0 32-1-0</p> <p>2-0-0 5-7-11 5-4-5 4-0-8 4-0-8 5-4-5 5-7-11 2-0-0</p> </div> <div style="width: 50%; text-align: right;"> <p>Scale = 1/50</p> </div> </div>																																										
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>2-0-0</td> <td>TC 0.31</td> <td>in (loc) l/defl L/d</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Plates Increase 1.25</td> <td>BC 0.49</td> <td>Vert(LL) -0.14 11-13 >999 240</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Lumber Increase 1.25</td> <td>WB 0.27</td> <td>Vert(TL) -0.28 11-13 >999 180</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Rep Stress Incr YES</td> <td>(Matrix-M)</td> <td>Horz(TL) 0.08 8 n/a n/a</td> <td></td> <td></td> </tr> <tr> <td></td> <td>Code FBC2010/TPI2007</td> <td></td> <td></td> <td>Weight: 160 lb</td> <td>FT = 20%</td> </tr> </table>							LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP	TCLL 20.0	2-0-0	TC 0.31	in (loc) l/defl L/d	MT20	244/190	TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(LL) -0.14 11-13 >999 240			BCLL 0.0 *	Lumber Increase 1.25	WB 0.27	Vert(TL) -0.28 11-13 >999 180			BCDL 5.0	Rep Stress Incr YES	(Matrix-M)	Horz(TL) 0.08 8 n/a n/a				Code FBC2010/TPI2007			Weight: 160 lb	FT = 20%
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REACTIONS (lb/size) 2=933/0-3-8 (min. 0-1-8), 8=933/0-3-8 (min. 0-1-8) Max Horz 2=64(LC 12) Max Uplift 2=150(LC 12), 8=150(LC 13) Max Grav 2=1071(LC 2), 8=1071(LC 2)																																										
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1812/732, 3-4=-1482/628, 4-5=-1297/608, 5-6=-1297/608, 6-7=-1482/627, 7-8=-1812/732 BOT CHORD 2-14=-537/1597, 13-14=-537/1597, 13-21=-377/1360, 12-21=-377/1360, 12-22=-377/1360, 11-22=-377/1360, 10-11=-544/1610, 8-10=-544/1610 WEBS 3-13=-378/240, 4-13=-130/398, 6-11=-130/398, 7-11=-377/240																																										
NOTES (9-11) 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf. 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 2 and 150 lb uplift at joint 8. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package) 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																										
LOAD CASE(S) Standard																																										



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 22 Unit 3 Mayfair
475006	T04	HIP	1	1	16492797

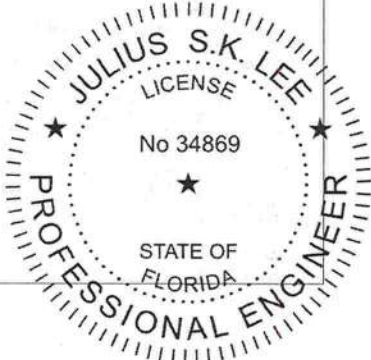
Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:28 2013 Page 2
ID:9B5QRtZPhUL0yMYqzVn3hhzz67b-vPPqU8Yo?rEfXVMQksRj2_fwZkEziBc21gEzCQzb3L

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-44, 3-6=-44, 6-8=-44, 2-7=-10
Concentrated Loads (lb)
Vert: 3=-135(F) 6=-135(F) 11=-187(F) 10=-22(F) 9=-187(F) 16=-69(F) 17=-69(F) 18=-69(F) 19=-69(F) 20=-69(F) 21=-69(F) 22=-69(F) 23=-22(F) 24=-22(F) 25=-22(F) 26=-22(F) 27=-22(F) 28=-22(F)

Julius Lee



JULIUS S.K. LEE
LICENSE
No 34869
★
STATE OF FLORIDA
PROFESSIONAL ENGINEER

March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss T03	Truss Type COMMON	Qty 5	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	16492796
Builders FirstSource, Lake City, FL 32055		7,350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:27 2013 Page 1 ID:9B5QRIZPhUL0yMYqzVn3hhzz67b-RDrSGpXAEY6ovLnEA8wUVm6qzKrhzuTvo0V0f_zbf3M				

LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2010/TPI2007	CSI TC 0.42 BC 0.87 WB 0.22 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) 0.20 8-10 >999 240 Vert(TL) -0.38 8-10 >629 180 Horz(TL) 0.04 6 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 96 lb FT = 20%
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LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

REACTIONS (lb/size) 6=801/0-3-8 (min. 0-1-8), 2=801/0-3-8 (min. 0-1-8)
Max Horz 2=59(LC 16)
Max Uplift 6=161(LC 13), 2=161(LC 12)
Max Grav 6=953(LC 2), 2=953(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1637/719, 3-4=-1501/695, 4-5=-1500/695, 5-6=-1637/719
BOT CHORD 2-10=-538/1453, 9-10=-270/963, 8-9=-270/963, 6-8=-545/1464
WEBS 4-8=-264/609, 5-8=-253/200, 4-10=-264/609, 3-10=-253/200

NOTES (9-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 6 and 161 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-44, 4-7=-44, 10-14=-10, 8-10=-61(F=-51), 8-11=-10

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-3 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

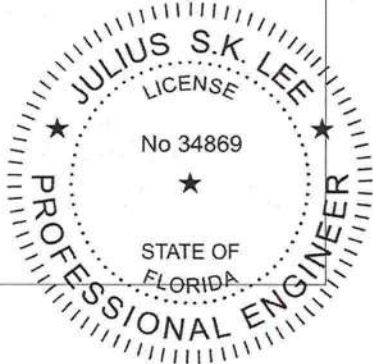
Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 22 Unit 3 Mayfair
475006	T01	HIP	1	1	16492794

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:25 2013 Page 2
 ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-Urkir7Vwiws4g1dr2ju0QL1QEXDJv0WcLi0vb6zb3C

LOAD CASE(S) Standard
 Concentrated Loads (lb)
 Vert: 3=-135(B) 4=-135(B) 9=-187(B) 7=-187(B) 16=-69(B) 17=-69(B) 18=-22(B) 19=-22(B)

Julius Lee



March 12, 2013



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 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D88-89 and BCS11 Building Component**
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
 1109 Coastal Bay Blvd.
 Boynton, FL 33435


Job	Truss	Truss Type	Qty	Ply	GIEBEIG - Lot 22 Unit 3 Mayfair
475006	HJ9	MONO TRUSS	7	1	16492793

Builders FirstSource, Lake City, FL 32055

7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:24 2013 Page 2
ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-0eAJenUlxdkD2t2fV0Nnu8UHb7uFmXyT62GM2fzf3P

LOAD CASE(S) Standard
Uniform Loads (plf)
Vert: 1-4=-44, 5-8=-10
Concentrated Loads (lb)
Vert: 11=75(F=37, B=37) 12=46(F=23, B=23) 13=-99(F=-49, B=-49) 14=10(F=5, B=5) 15=-8(F=-4, B=-4) 16=-28(F=-14, B=-14)

Julius Lee



March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Professional Engineer Seal for Julius S.K. Lee, License No. 34869, State of Florida.



Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truck designer. String shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss EJ01	Truss Type Jack-Partial Truss	Qty 3	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional) 7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:22 2013 Page 1 ID:9B5QRIZPhULQyMYqzVn3hhzz6?b-4G2ZD5T1P?TWpauGNbKJojP0tJbIhJAknF_nzbf3R
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16492790

Builders FirstSource, Lake City, FL 32055

Scale = 1/16"

Plate Offsets (X,Y): [2-0-0,0-0-0-14]									
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.25	Vert(LL)	-0.01	4-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.10	Vert(TL)	-0.01	4-7	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL)	0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)							
								Weight: 13 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

REACTIONS (lb/size) 3=42/Mechanical, 2=193/0-3-8 (min. 0-1-8), 4=11/Mechanical

Max Horz 2=90(LC 12)

Max Uplift 3=-38(LC 12), 2=-87(LC 12), 4=-15(LC 9)

Max Grav 3=51(LC 2), 2=233(LC 2), 4=33(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-402/109

BOT CHORD 2-4=-106/509

NOTES (7-9)

- 1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf, BCDL=3.0psf; h=18ft, Cat. II; Exp B; End., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 87 lb uplift at joint 2 and 15 lb uplift at joint 4.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013



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Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 475006	Truss CJ3	Truss Type JACK	Qty 14	Ply 1	GIEBEIG - Lot 22 Unit 3 Mayfair Job Reference (optional)	I6492788
Builders FirstSource, Lake City, FL 32055			7.350 s Jul 31 2012 MiTek Industries, Inc. Tue Mar 12 14:25:20 2013 Page 1			
			ID:9B5QRtZPhUL0yMYqzVn3hhzz6?b-8twp0PRntODoaGluGAlqJgNW7qnDIBQI8vuzbf3T			

Plate Offsets (X, Y): [2-0-0, 0-0-14]				
LOADING (psf) TCCL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.25 BC 0.10 WB 0.00 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 4-7 >999 240 Vert(TL) -0.01 4-7 >999 180 Horz(TL) 0.00 2 n/a n/a	PLATES GRIP MT20 244/190 Weight: 13 lb FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2

REACTIONS (lb/size) 3=42/Mechanical, 2=193/0-3-8 (min. 0-1-8), 4=11/Mechanical
 Max Horz 2=90(LC 12)
 Max Uplift 3=-38(LC 12), 2=-87(LC 12), 4=-15(LC 9)
 Max Grav 3=52(LC 2), 2=233(LC 2), 4=33(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-402/109
 BOT CHORD 2-4=-106/509

NOTES (7-9)
 1) Wind: ASCE 7-10; 130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp B; End.; GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 87 lb uplift at joint 2 and 15 lb uplift at joint 4.
 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 8) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

March 12, 2013



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult - **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

