DATE 04/06/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000024360

APPLICANT JOHN NORRIS PHONE 758-3663

ADDRESS 351 NW CORWIN GLEN LAKE CITY FL 32055

OWNER IMAGE DEVELOPMENT GROUP PHONE 352 538-9697

ADDRESS 122 SW THISTLEWOOD LANE FT. WHITE FL 32038

CONTRACTOR JOHN NORRIS PHONE 758-3663

LOCATION OF PROPERTY 47S, TL ON SR 27, TL ON CR 18, TL ON GREENWOOD,
TL ON THISTLEWOOD, 1ST LOT ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 73650.00

HEATED FLOOR AREA 1473.00 TOTAL AREA 2245.00 HEIGHT ____ STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING FW MAX. HEIGHT 8

Minimum Set Back Requirments: STREET-FRONT ____ REAR ____ SIDE ____

NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO. ____

PARCEL ID 34-6S-16-04056-128 SUBDIVISION THORNWOOD

LOT 28 BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES ____

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RG0066597	

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	06-0278-N	BK	JH	Y

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 3636

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by Foundation ____ date/app. by Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by Slab ____ date/app. by Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by Heat & Air Duct ____ date/app. by Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by C.O. Final ____ date/app. by Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by Pool ____ date/app. by

Reconnection ____ date/app. by Pump pole ____ date/app. by Utility Pole ____ date/app. by

M/H Pole ____ date/app. by Travel Trailer ____ date/app. by Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 370.00 CERTIFICATION FEE $ 11.22 SURCHARGE FEE $ 11.22

MISC. FEES $ 0.00 ZONING CERT. FEE $ ____ FIRE FEE $ 0.00 WASTE FEE $ ____

FLOOD DEVELOPMENT FEE $ ____ FLOOD ZONE FEE $ ____ CULVERT FEE $ ____ **TOTAL FEE** 392.44

INSPECTORS OFFICE ____ CLERKS OFFICE ____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
1089 SW MAIN BLVD.
LAKE CITY, FLORIDA 32025

Inst:2006005979 Date:03/10/2006 Time:14:43
DC,P.DeWitt Cason,Columbia County B:1076 P:2378

Parcel I.D. #: ~~04056-122~~ & *04056-128*

SPACE ABOVE THIS LINE FOR PROCESSING DATA — SPACE ABOVE THIS LINE FOR RECORDING DATA

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713.13, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

Lots ~~27~~ & 28, THORNWOOD, according to the map or plat thereof as recorded in Plat Book 7, Page 202, 204, of the Public Records of Columbia County, FLORIDA.

2. General description of improvement: **construction of single family dwelling**

3. Owner information:
   a. Name and address:
   IMAGE DEVELOPMENT GROUP, LLC
   20074 NW 258TH DRIVE, HIGH SPRINGS, FL 32643
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address)
   JOHN NORRIS CONSTRUCTION, INC.
   351 NW CORWIN GLN, LAKE CITY, FL 32055
   Telephone Number: **386-758-3663 and 386-961-4549**

5. Surety (if any):
   a. Name and Address:
   Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
   CAPITAL CITY BANK
   1301 METROPOLITAN BLVD., TALLAHASSEE, FLORIDA 32308
   Telephone Number: **850-402-2223**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: (Name and Address)
   N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address)
   CAPITAL CITY BANK
   1301 METROPOLITAN BLVD., TALLAHASSEE, FLORIDA 32308
   Telephone Number: **850-402-2223**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

IMAGE DEVELOPMENT GROUP, LLC

By: _____ {SEAL}
RICHARD C. PARKER

_____ {SEAL}

Sworn to and subscribed before me this **10th** day of **March, 2006**, by **RICHARD C. PARKER** as **MANAGER** of **IMAGE DEVELOPMENT GROUP, LLC**, who is personally known to me or who has produced drivers license as identification

_____
Notary Public
My Commission Expires: _____

NOTARY PUBLIC STATE OF FLORIDA
BRENDA STYONS
MY COMMISSION # DD 287986
EXPIRES: February 5, 2008
Bonded Thru Budget Notary Services

392.44 ~~[illegible]~~

Revised 9-23-04

Columbia County Building Permit Application

For Office Use Only Application # 0603-79 Date Received 3/21 By JW Permit # ~~[illegible]~~ 24360

Plans Examiner OK JTH Date 4-6-06

Application Approved by - Zoning Official ______ Date ______ Land Use Plan Map Category ______

Flood Zone ______ Development Permit ______ Zoning ______

Comments ~~[illegible]~~ ; ~~[illegible]~~ [illegible] Ft. White.

Applicants Name John Norris Phone 758-3663

Address 351 NW Corwin Gln

Owners Name Image Development Group LLC Phone 352-538-9697

911 Address 122 SW Thistlewood Ln Ft White 32038

Contractors Name John Norris Phone 758-3663

Address 351 NW Corwin Gln

Fee Simple Owner Name & Address Image Development Group

Bonding Co. Name & Address ______

Architect/Engineer Name & Address Bill Freeman

Mortgage Lenders Name & Address Capital City Bank

Circle the correct power company - Fl Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 34-6S-16-04056-128 Estimated Cost of Construction 90,000.

Subdivision Name Thornwood Lot 28 Block ____ Unit ____ Phase ____ CRB

Driving Directions take SR 47 South to Ft White - Left on SR 27 - 1/4 mile - left on 1/2 mile - left on Greenwood - left on Thistlewood Ln - lot 28

Type of Construction New Home Number of Existing Dwellings on Property 0

Total Acreage 1 Lot Size ______ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 25' Side 25' Side 25' Rear 25'

Total Building Height 8'8" Number of Stories 1 Heated Floor Area 1473 Roof Pitch 6/12

Porch 314 Garage 458 Total 2245

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor) Contractor Signature

Contractors License Number RG0066597

Competency Card Number ______

NOTARY STAMP/SEAL

GALE TEDDER MY COMMISSION # DD 333686 ... June 28, 2008 ... Public Underwriters

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 6th day of April 2006.

Personally known ✓ or Produced Identification ______

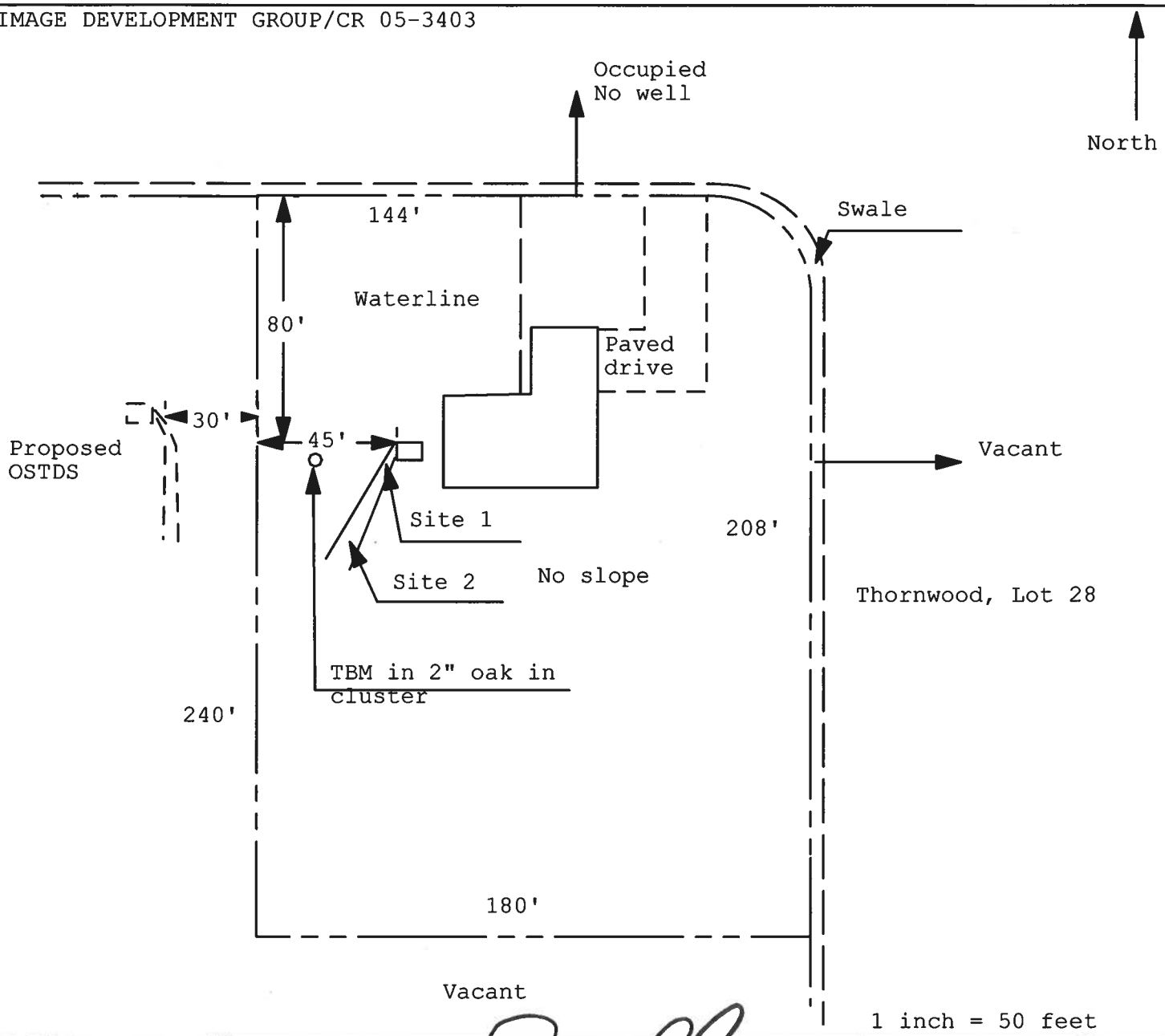
Notary Signature

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0278N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

IMAGE DEVELOPMENT GROUP/CR 05-3403



Site Plan Submitted By Paul Lloyd **Date** 3/6/06

Plan Approved X **Not Approved** ____ **Date** 3/28/6

By APPROVED Columbia CHD **CPHU**

Notes: ____

0603-79

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@ CAM112M01    S  CamaUSA Appraisal System                    Columbia Count
 3/21/2006 16:30  Legal Description Maintenance      40500  Land   001
Year T Property                          Sel                 AG     000
2006 R 34-6S-16-04056-128 ..... ... ..                       Bldg   000
                                                             Xfea   000
       IMAGE DEVELOPMENT GROUP LLC                   40500  TOTAL      I

  1  LOT 28 THORNWOOD S/D ..... WD 1030-2804 ..................    2
  3  ..... ..... ..... ..... .....    4
  5  ..... ..... ..... ..... .....    6
  7  ..... ..... ..... ..... .....    8
  9  ..... ..... ..... ..... .....   10
 11  ..... ..... ..... ..... .....   12
 13  ..... ..... ..... ..... .....   14
 15  ..... ..... ..... ..... .....   16
 17  ..... ..... ..... ..... .....   18
 19  ..... ..... ..... ..... .....   20
 21  ..... ..... ..... ..... .....   22
 23  ..... ..... ..... ..... .....   24
 25  ..... ..... ..... ..... .....   26
 27  ..... ..... ..... ..... .....   28
                                  Mnt 12/03/2004 LARRY
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
```

Town of Fort White

Post Office Box 129 ◆ Fort White, Florida 32038-0129 ◆ 386-497-2321 ◆ FAX 386-497-4946

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes

OWNER'S NAME: Image Development Group

ADDRESS: P.O. Box 305 Newberry, FL 32669

PROPERTY DESCRIPTION: parcel #4056-128 Lot #28
(parcel number if possible) 122 SW Thistlewood Lane / Thornwood

DEVELOPMENT Single Family Dwelling

You are hereby authorized to issue the appropriate building permits.

3/2/06
DATE

James S. Revels KA
LAND DEVELOPMENT REGULATION
ADMINISTRATOR
TOWN OF FORT WHITE

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Thronwood Lot 28**	Builder:	John Norris
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	24360
Owner:	**John Norris**	Jurisdiction Number:	221000
Climate Zone:	**South**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1473 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	130.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 210.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1680.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1620.3 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Con. Ret: Con. AH: Interior		Sup. R=6.0, 67.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 24.0 kBtu/hr	
	SEER: 10.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 24.0 kBtu/hr	
	HSPF: 7.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

Glass/Floor Area: 0.09

Total as-built points: 19853
Total base points: 25509

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: William H Free

DATE: 2/16/06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE

GLASS TYPES

.18 X	Conditioned Floor Area	X BSPM	= Points
.18	1473.0	32.50	8617.1

WALL TYPES	Area	X BSPM	= Points
Adjacent	0.0	0.00	0.0
Exterior	1680.0	2.70	4536.0
Base Total:	**1680.0**		**4536.0**

DOOR TYPES	Area	X BSPM	= Points
Adjacent	0.0	0.00	0.0
Exterior	59.8	6.40	383.0
Base Total:	**59.8**		**383.0**

CEILING TYPES	Area	X BSPM	= Points
Under Attic	1473.0	2.80	4124.4
Base Total:	**1473.0**		**4124.4**

FLOOR TYPES	Area	X BSPM	= Points
Slab	210.0(p)	-20.0	-4200.0
Raised	0.0	0.00	0.0
Base Total:			**-4200.0**

INFILTRATION	Area	X BSPM	= Points
	1473.0	18.79	27677.7

AS-BUILT

Type/SC	Overhang Ornt	Len	Hgt	Area	X SPM	X SOF	= Points
Double, Clear	W	1.5	4.0	9.0	61.59	0.83	460.0
Double, Clear	W	1.5	6.0	15.0	61.59	0.92	848.1
Double, Clear	W	1.5	6.0	26.0	61.59	0.92	1470.1
Double, Clear	E	1.5	6.0	30.0	68.60	0.92	1888.1
Double, Clear	E	1.5	6.0	50.0	68.60	0.92	3146.9
As-Built Total:				**130.0**			**7813.2**

Type	R-Value	Area	X SPM	= Points
Frame, Wood, Exterior	13.0	1680.0	2.40	4032.0
As-Built Total:		**1680.0**		**4032.0**

Type	Area	X SPM	= Points
Exterior Insulated	40.8	6.40	261.1
Exterior Insulated	19.0	6.40	121.9
As-Built Total:	**59.8**		**383.0**

Type	R-Value	Area	X SPM X SCM	= Points
Under Attic	30.0	1620.3	2.77 X 1.00	4488.2
As-Built Total:		**1620.3**		**4488.2**

Type	R-Value	Area	X SPM	= Points
Slab-On-Grade Edge Insulation	0.0	210.0(p	-20.00	-4200.0
As-Built Total:		**210.0**		**-4200.0**

	Area	X SPM	= Points
	1473.0	18.79	27677.7

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT						
Summer Base Points:			**41138.1**	**Summer As-Built Points:**						**40194.1**
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
41138.1	**0.4266**		**17549.5**	40194.1 **40194.1**	1.000 **1.00**	(1.000 x 1.165 x 0.90) **1.048**	0.341 **0.341**	0.857 **0.857**		12321.3 **12321.3**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1473.0	2.36	625.7	Double, Clear	W	1.5	4.0	9.0	3.98	1.00	35.8
				Double, Clear	W	1.5	6.0	15.0	3.98	1.00	59.6
				Double, Clear	W	1.5	6.0	26.0	3.98	1.00	103.3
				Double, Clear	E	1.5	6.0	30.0	3.30	1.02	101.1
				Double, Clear	E	1.5	6.0	50.0	3.30	1.02	168.5
				As-Built Total:				**130.0**			**468.3**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1680.0	0.60		1008.0
Exterior	1680.0	0.60	1008.0								
Base Total:	**1680.0**		**1008.0**	**As-Built Total:**				**1680.0**			**1008.0**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	0.0	0.00	0.0	Exterior Insulated				40.8	1.80		73.4
Exterior	59.8	1.80	107.7	Exterior Insulated				19.0	1.80		34.3
Base Total:	**59.8**		**107.7**	**As-Built Total:**				**59.8**			**107.7**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1473.0	0.10	147.3	Under Attic			30.0	1620.3	0.10 X	1.00	162.0
Base Total:	**1473.0**		**147.3**	**As-Built Total:**				**1620.3**			**162.0**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	210.0(p)	-2.1	-441.0	Slab-On-Grade Edge Insulation			0.0	210.0(p	-2.10		-441.0
Raised	0.0	0.00	0.0								
Base Total:			**-441.0**	**As-Built Total:**				**210.0**			**-441.0**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1473.0	-0.06	-88.4					1473.0	-0.06		-88.4

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**1359.4**	**Winter As-Built Points:**					**1216.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
1359.4	0.6274	852.9	1216.6 **1216.6**	1.000 **1.00**	(1.000 x 1.137 x 0.91) **1.035**	0.487 **0.487**	0.950 **0.950**	582.6 **582.6**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2369.00	7107.0	50.0	0.90	3	1.00	2316.36	1.00	6949.1	
				As-Built Total:							**6949.1**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
17550	**853**	**7107**	**25509**	**12321**	**583**	**6949**	**19853**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 87.3

The higher the score, the more efficient the home.

John Norris, , , ,

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1473 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear - single pane	0.0 ft²	130.0 ft²	
b. Clear - double pane	0.0 ft²	0.0 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	0.0 ft²	
d. Tint/other SHGC - double pane			
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 210.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1680.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1620.3 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Con. Ret: Con. AH: Interior		Sup. R=6.0, 67.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 24.0 kBtu/hr	
	SEER: 10.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 24.0 kBtu/hr	
	HSPF: 7.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-1824.*

Residential System Sizing Calculation

Summary

John Norris

Project Title:
Thronwood Lot 28

Code Only
Professional Version
Climate: South

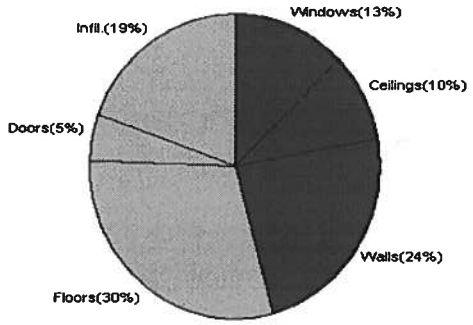
2/15/2006

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	98	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	23	F
Total heating load calculation	**22063**	**Btuh**	**Total cooling load calculation**	**19534**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	108.8	24000	Sensible (SHR = 0.5)	79.1	12000
Heat Pump + Auxiliary(0.0kW)	108.8	24000	Latent	274.9	12000
			Total (Electric Heat Pump)	122.9	24000

WINTER CALCULATIONS

Winter Heating Load (for 1473 sqft)

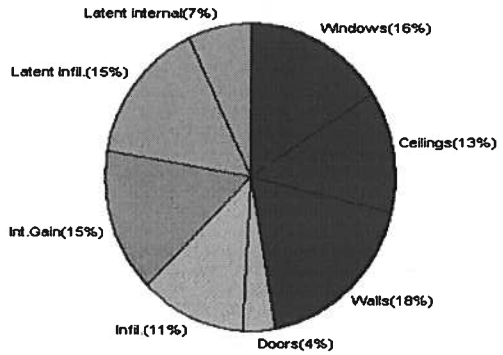
Load component			Load	
Window total	130	sqft	2795	Btuh
Wall total	1680	sqft	5208	Btuh
Door total	60	sqft	1097	Btuh
Ceiling total	1620	sqft	2106	Btuh
Floor total	210	ft	6636	Btuh
Infiltration	98	cfm	4221	Btuh
Subtotal			**22063**	**Btuh**
Duct loss			0	Btuh
TOTAL HEAT LOSS			**22063**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1473 sqft)

Load component			Load	
Window total	130	sqft	3120	Btuh
Wall total	1680	sqft	3595	Btuh
Door total	60	sqft	747	Btuh
Ceiling total	1620	sqft	2528	Btuh
Floor total			0	Btuh
Infiltration	86	cfm	2178	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**15168**	**Btuh**
Duct gain			0	Btuh
Total sensible gain			**15168**	**Btuh**
Latent gain(infiltration)			2986	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4366**	**Btuh**
TOTAL HEAT GAIN			**19534**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: Willie N Tice
DATE: 2/16/06

System Sizing Calculations - Winter

Residential Load - Component Details

John Norris

Project Title:
Thronwood Lot 28

Code Only
Professional Version
Climate: South

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

2/15/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	9.0	21.5	194 Btuh
2	2, Clear, Wood, DEF	N	15.0	21.5	322 Btuh
3	2, Clear, Wood, DEF	N	26.0	21.5	559 Btuh
4	2, Clear, Wood, DEF	S	30.0	21.5	645 Btuh
5	2, Clear, Wood, DEF	S	50.0	21.5	1075 Btuh
	Window Total		130		2795 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1680	3.1	5208 Btuh
	Wall Total		1680		5208 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Exter		41	18.3	748 Btuh
2	Insulated - Exter		19	18.3	349 Btuh
	Door Total		60		1097Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1620	1.3	2106 Btuh
	Ceiling Total		1620		2106Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	210.0 ft(p)	31.6	6636 Btuh
	Floor Total		210		6636 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	14730(sqft)	98	4221 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			98	4221 Btuh

Totals for Heating	**Subtotal**	**22063 Btuh**
	Duct Loss(using duct multiplier of 0.00)	**0 Btuh**
	Total Btuh Loss	**22063 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

John Norris

Project Title:
Thronwood Lot 28

Code Only
Professional Version
Climate: South

Reference City: Gainesville (User customized) Summer Temperature Difference: 23.0 F 2/15/2006

Window	Type Panes/SHGC/U/InSh/ExSh Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, DEF, N, N N	1.5	4	9.0	0.0	9.0	24	24	216 Btuh
2	2, Clear, DEF, N, N N	1.5	6	15.0	0.0	15.0	24	24	360 Btuh
3	2, Clear, DEF, N, N N	1.5	6	26.0	0.0	26.0	24	24	624 Btuh
4	2, Clear, DEF, N, N S	1.5	6	30.0	30.0	0.0	24	39	720 Btuh
5	2, Clear, DEF, N, N S	1.5	6	50.0	50.0	0.0	24	39	1200 Btuh
	Window Total			130					3120 Btuh

Walls	Type	R-Value	Area	HTM	Load
1	Frame - Exterior	13.0	1680.0	2.1	3595 Btuh
	Wall Total		1680.0		3595 Btuh

Doors	Type	Area	HTM	Load
1	Insulated - Exter	40.8	12.5	509 Btuh
2	Insulated - Exter	19.0	12.5	238 Btuh
	Door Total	59.8		747 Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load
1	Under Attic/Dark	30.0	1620.3	1.6	2528 Btuh
	Ceiling Total		1620.3		2528 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab-On-Grade Edge Insulation	0.0	210.0 ft(p)	0.0	0 Btuh
	Floor Total		210.0		0 Btuh

Infiltration	Type	ACH	Volume	CFM=	Load
	Natural	0.35	14730	86.1	2178 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			86	2178 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 300 +	1200	3000 Btuh

Totals for Cooling		
	Subtotal	**15168 Btuh**
	Duct gain(using duct multiplier of 0.00)	**0 Btuh**
	Total sensible gain	**15168 Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**2986 Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380 Btuh**
	Latent other gain	**0 Btuh**
	TOTAL GAIN	**19534 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

Project Information for:	L157751		
Builder:	JOHN NORRIS	Date:	4/3/2006
Lot:	LOT 28 THORNWOOD	Start Number:	**1184**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	32		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **99**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0403061184	4/3/2006				
2	CJ3	0403061185	4/3/2006				
3	CJ3T	0403061186	4/3/2006				
4	CJ5	0403061187	4/3/2006				
5	CJ5T	0403061188	4/3/2006				
6	EJ4	0403061189	4/3/2006				
7	EJ7	0403061190	4/3/2006				
8	EJ7T	0403061191	4/3/2006				
9	HJ5	0403061192	4/3/2006				
10	HJ9	0403061193	4/3/2006				
11	HJ9T	0403061194	4/3/2006				
12	T01	0403061195	4/3/2006				
13	T02	0403061196	4/3/2006				
14	T03	0403061197	4/3/2006				
15	T04	0403061198	4/3/2006				
16	T05	0403061199	4/3/2006				
17	T06	0403061200	4/3/2006				
18	T07	0403061201	4/3/2006				
19	T08	0403061202	4/3/2006				
20	T09	0403061203	4/3/2006				
21	T10	0403061204	4/3/2006				
22	T10A	0403061205	4/3/2006				
23	T11	0403061206	4/3/2006				
24	T12	0403061207	4/3/2006				
25	T13	0403061208	4/3/2006				
26	T14	0403061209	4/3/2006				
27	T15	0403061210	4/3/2006				
28	T15H	0403061211	4/3/2006				
29	T15T	0403061212	4/3/2006				
30	T16	0403061213	4/3/2006				
31	T17	0403061214	4/3/2006				
32	T18	0403061215	4/3/2006				

APR 03 2006

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Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID** (Primary Name) **INDIVIDUAL** (Alternate Name)
Main Address:	**351 NW CORWIN GLN** **LAKE CITY, Florida 32055**
Lic. Location:	**WOODGLEN DRIVE** **LAKE CITY, FL 32055** **Columbia**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current, Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
No Qualified Business License Required	02/20/2004

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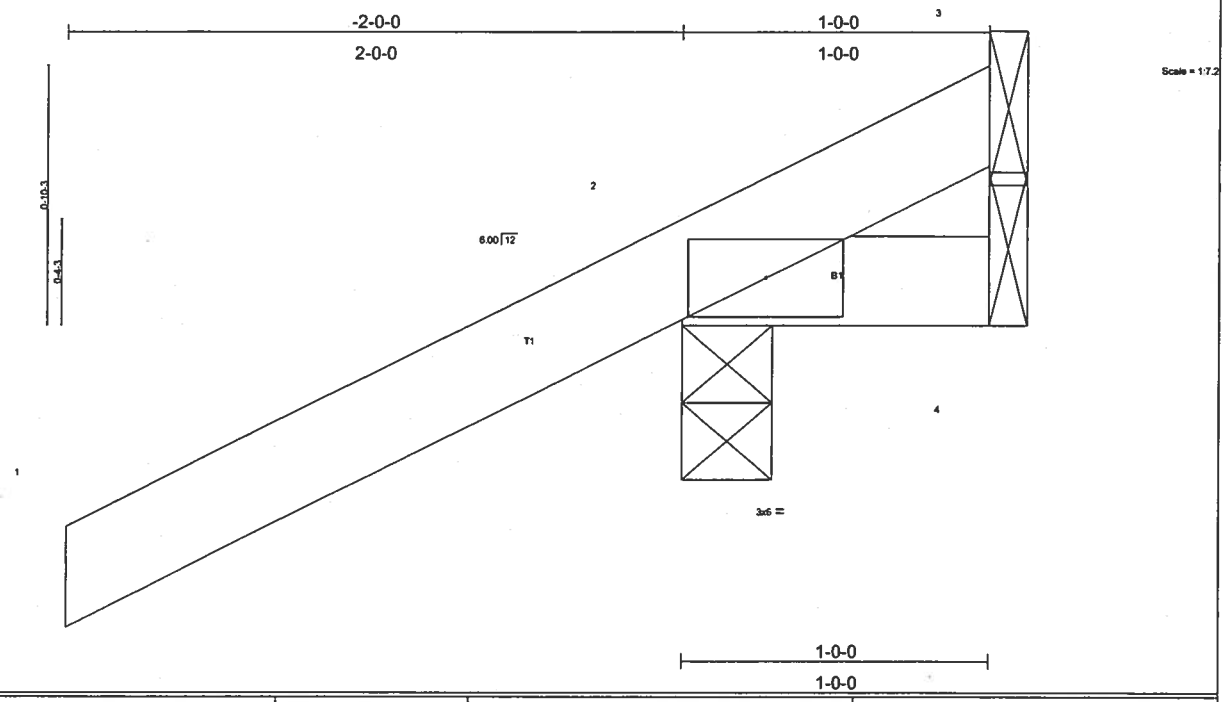
APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061184

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	CJ1	MONO TRUSS	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:45 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061185

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	CJ3	MONO TRUSS	14	1	Job Reference (optional)

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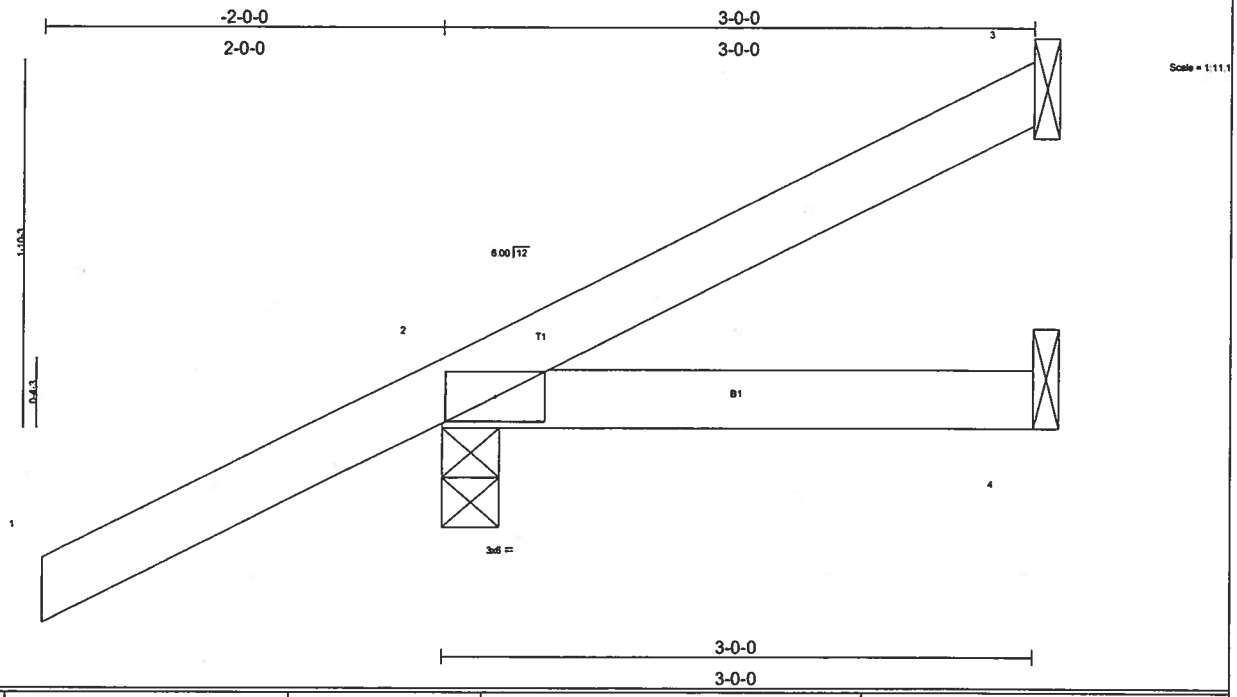

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061186

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	CJ3T	SPECIAL	4	1	Job Reference (optional)

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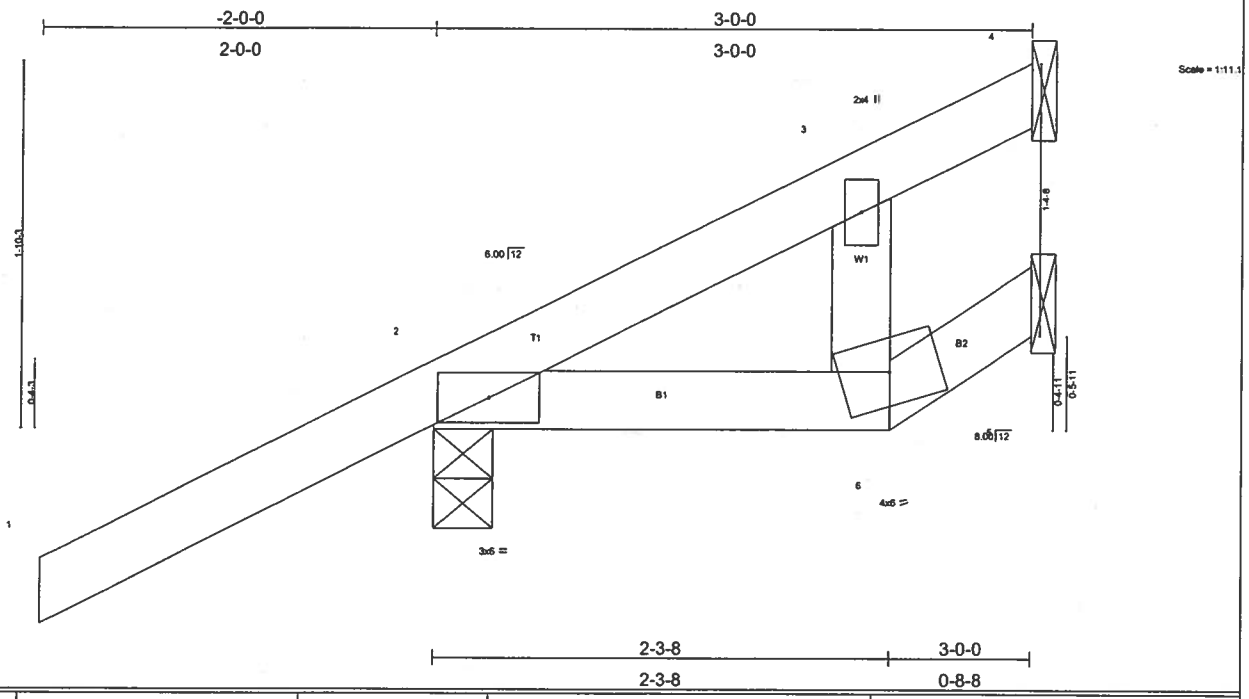


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=63/Mechanical, 2=278/0-3-8, 5=10/Mechanical
Max Horz 2=132(load case 5)
Max Uplift4=-21(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/2, 3-4=-18/26
BOT CHORD 2-6=-6/0, 5-6=-4/4
WEBS 3-6=0/42

JOINT STRESS INDEX
2 = 0.13, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 203 lb uplift at joint 2.

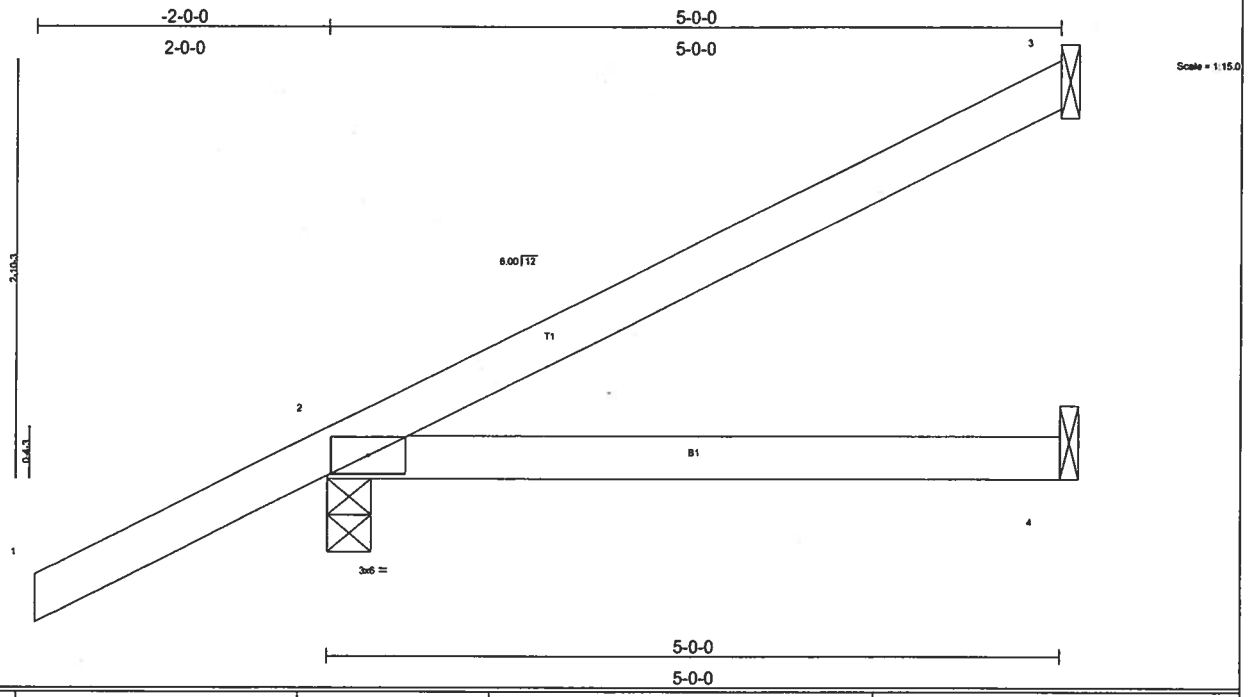
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061187

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	CJ5	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

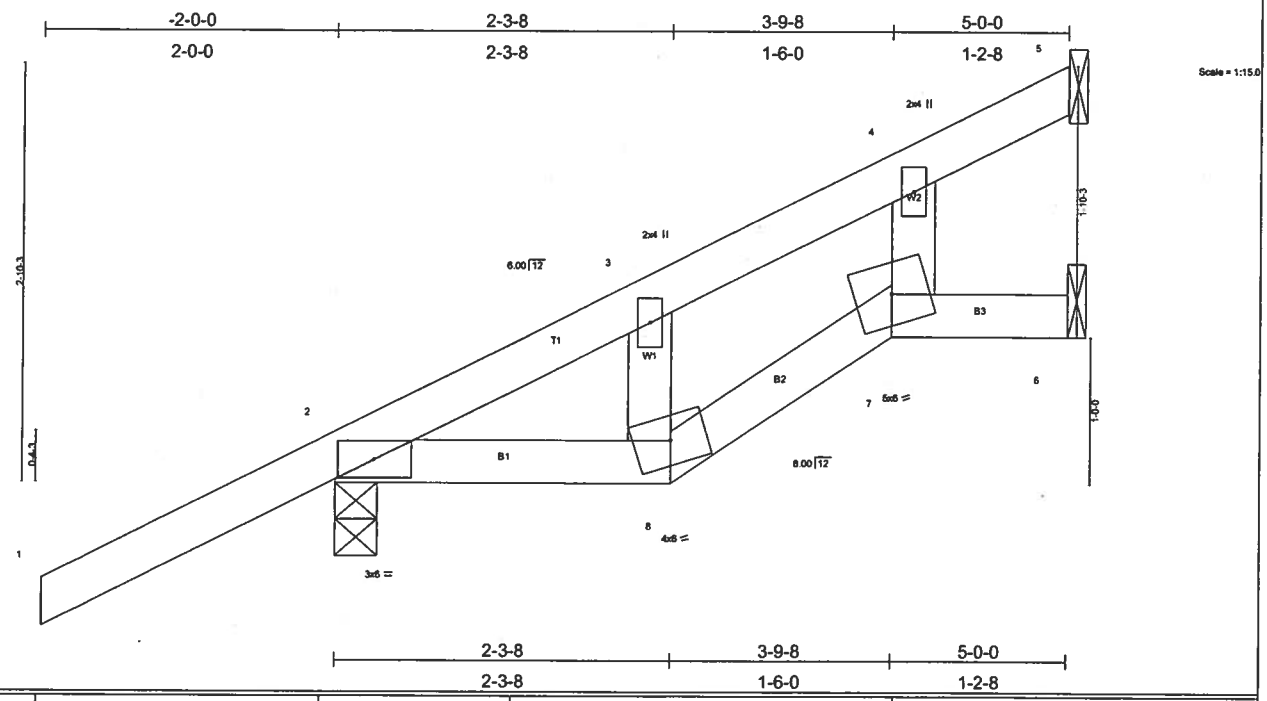
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061188

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	CJ5T	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	0.06	8	>886	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.10	8	>599	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=158/Mechanical, 2=343/0-3-8, 6=17/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-76(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-94/0, 3-4=-77/17, 4-5=-65/68
BOT CHORD 2-8=-0/0, 7-8=-2/18, 6-7=-0/0
WEBS 3-8=0/51, 4-7=0/44

JOINT STRESS INDEX
2 = 0.15, 3 = 0.04, 4 = 0.03, 7 = 0.01 and 8 = 0.03

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 5 and 199 lb uplift at joint 2.

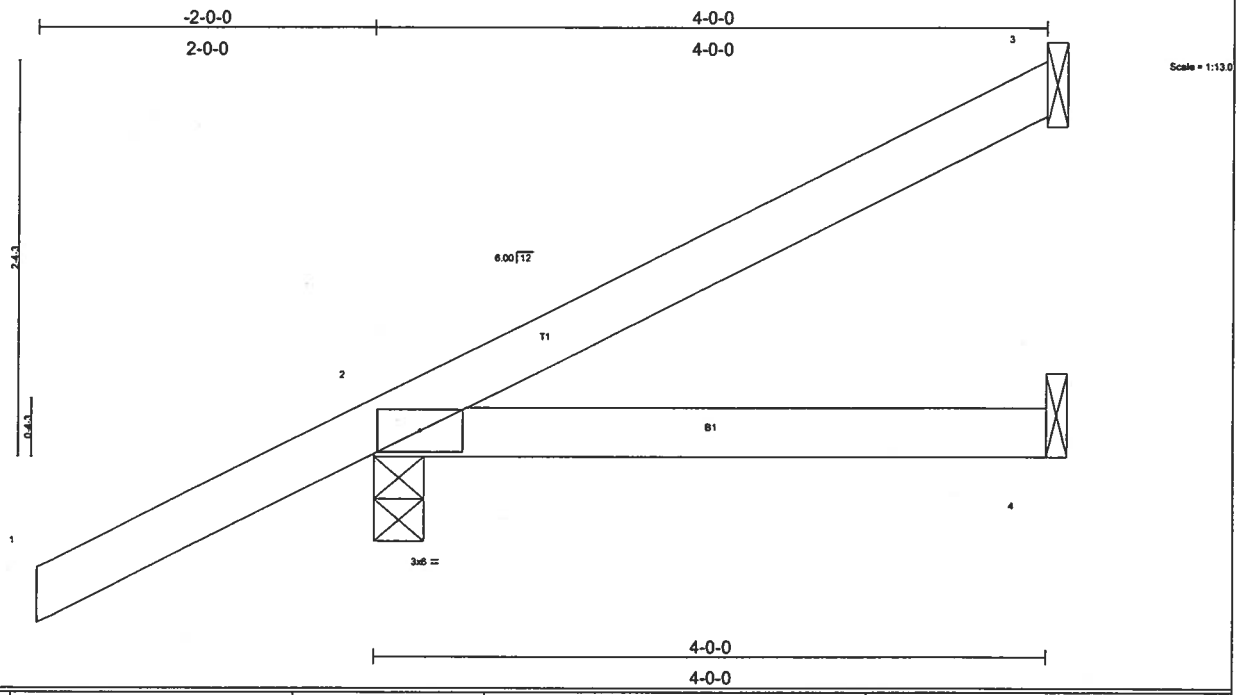
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061189

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	EJ4	JACK	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:48 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	0.03	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=70/Mechanical, 2=308/0-3-8, 4=57/Mechanical
Max Horz 2=155(load case 5)
Max Uplift3=-54(load case 5), 2=-246(load case 5), 4=-36(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-63/23
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 3, 246 lb uplift at joint 2 and 36 lb uplift at joint 4.

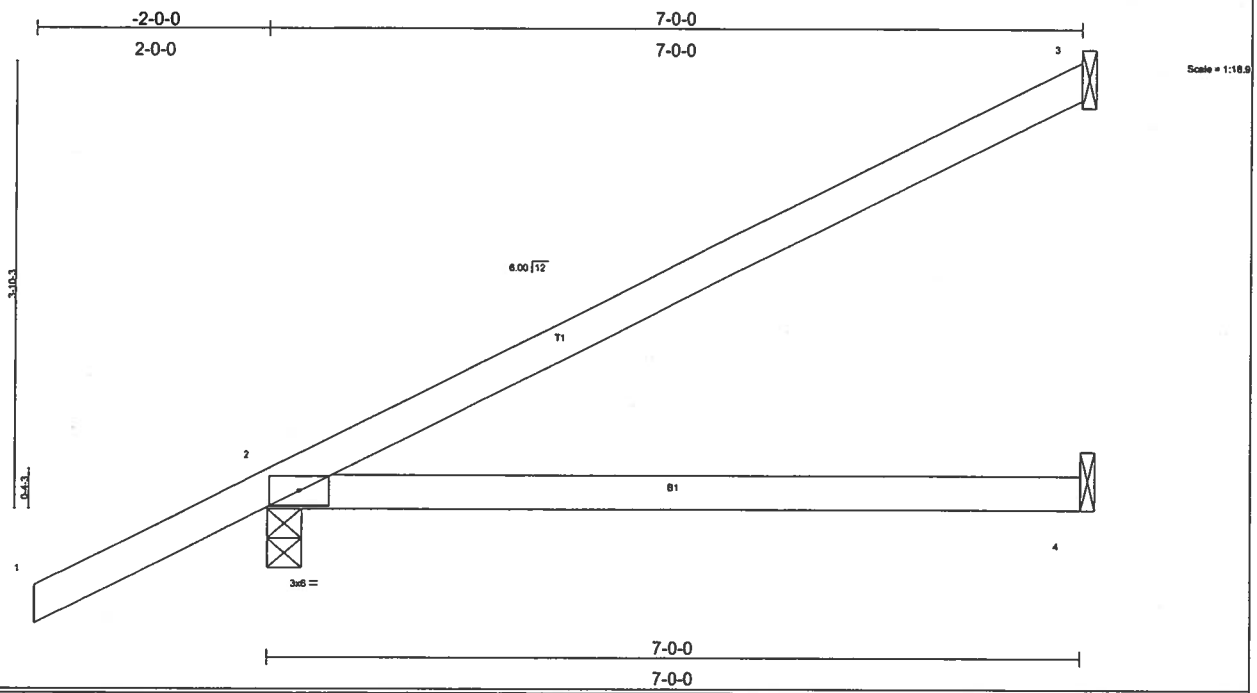
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061190

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	EJ7	MONO TRUSS	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:48 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.33	2-4	>250	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.28	2-4	>295	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-295(load case 5), 4=-68(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-131/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 295 lb uplift at joint 2 and 68 lb uplift at joint 4.

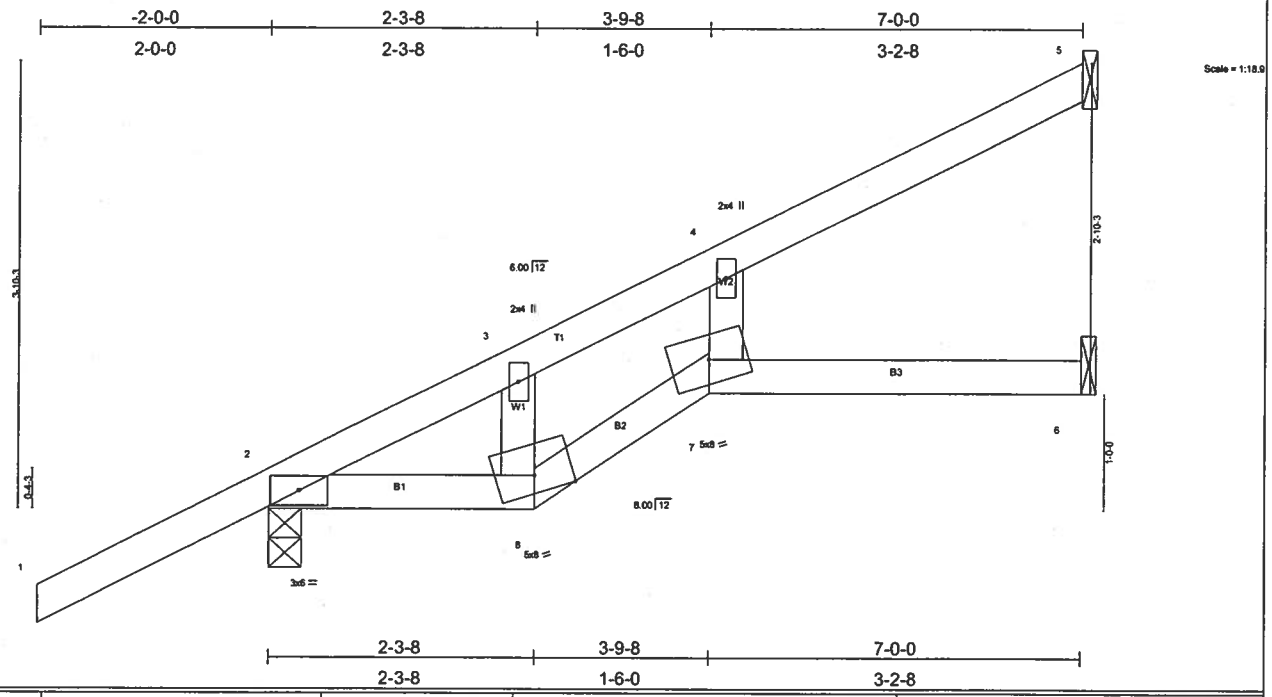
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061191

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	EJ7T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:49 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.19	7-8	>435	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	-0.24	7-8	>335	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 29 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=157/Mechanical, 2=419/0-3-8, 6=110/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-101(load case 5), 2=-210(load case 5), 6=-25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-184/0, 3-4=-145/0, 4-5=-80/63
BOT CHORD 2-8=0/11, 7-8=-7/32, 6-7=-0/0
WEBS 3-8=-80/92, 4-7=0/101

JOINT STRESS INDEX
2 = 0.59, 3 = 0.05, 4 = 0.05, 7 = 0.84 and 8 = 0.69

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 5, 210 lb uplift at joint 2 and 25 lb uplift at joint 6.

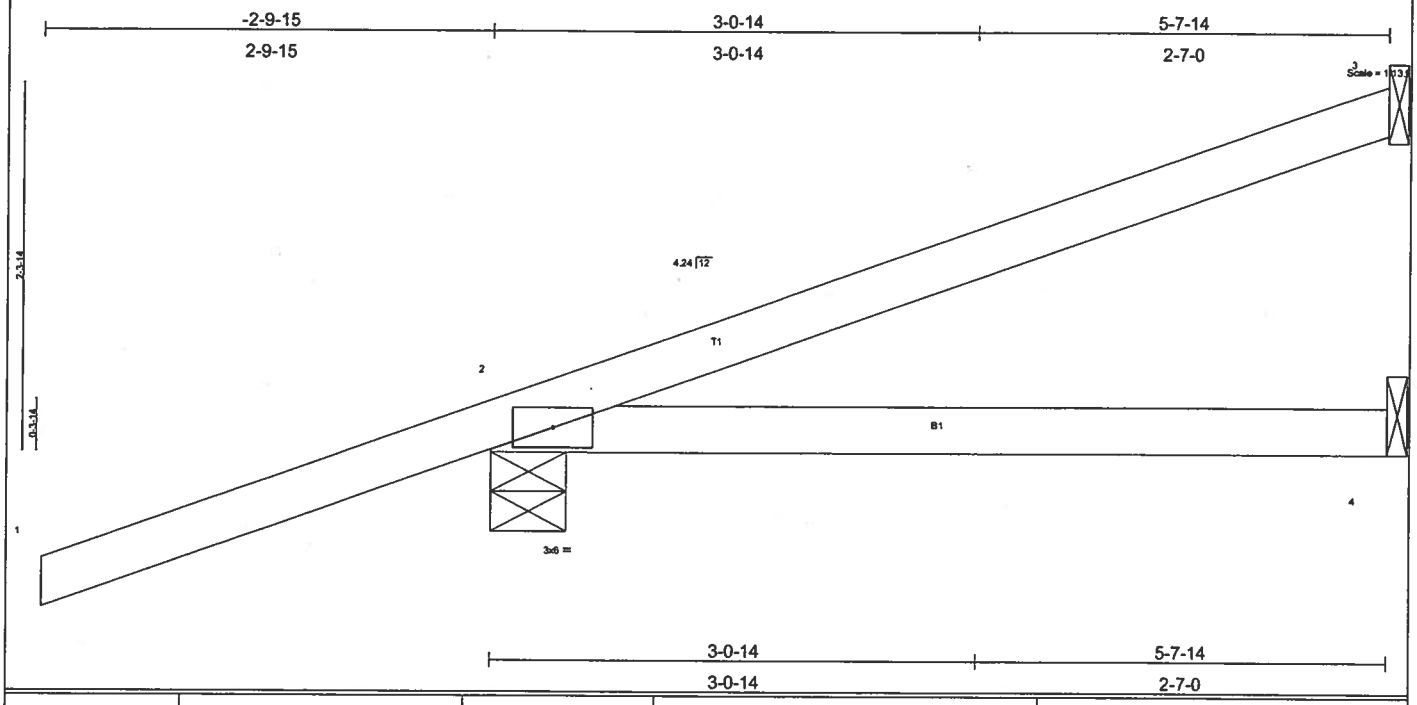
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061192

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	HJ5	JACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:50 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.05	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=92/Mechanical, 2=325/0-5-11, 4=76/Mechanical
Max Horz 2=128(load case 2)
Max Uplift3=-58(load case 2), 2=-314(load case 2), 4=-52(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-42/19
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 58 lb uplift at joint 3, 314 lb uplift at joint 2 and 52 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

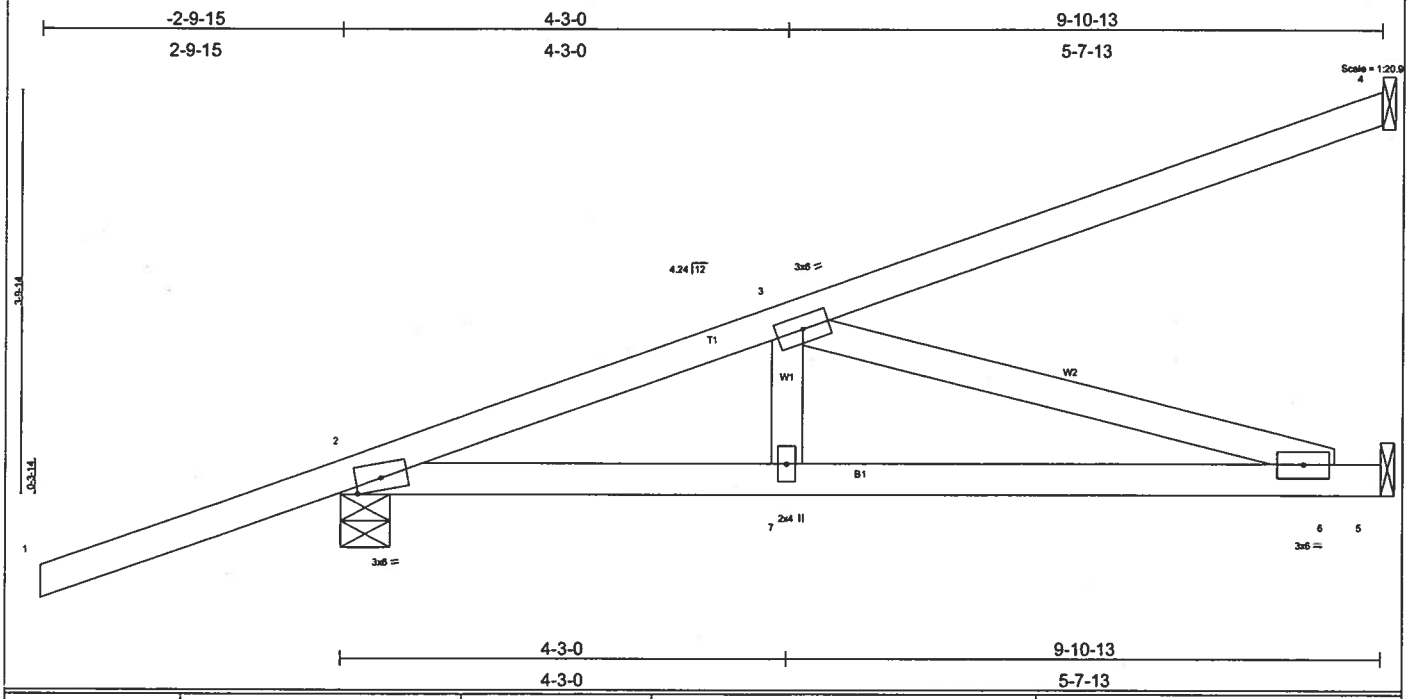
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-76(F=-11, B=-11), 2=-0(F=15, B=15)-to-4=-42(F=-6, B=-6)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061193

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	HJ9	MONO TRUSS	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:51 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.19	6-7	>623	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-9 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-233(load case 2), 2=-401(load case 2), 5=-181(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-883/363, 3-4=-105/66
BOT CHORD 2-7=-535/817, 6-7=-535/817, 5-6=0/0
WEBS 3-7=-94/188, 3-6=-851/557

JOINT STRESS INDEX
2 = 0.76, 3 = 0.23, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 401 lb uplift at joint 2 and 181 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061194

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	HJ9T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:51 2006 Page 1

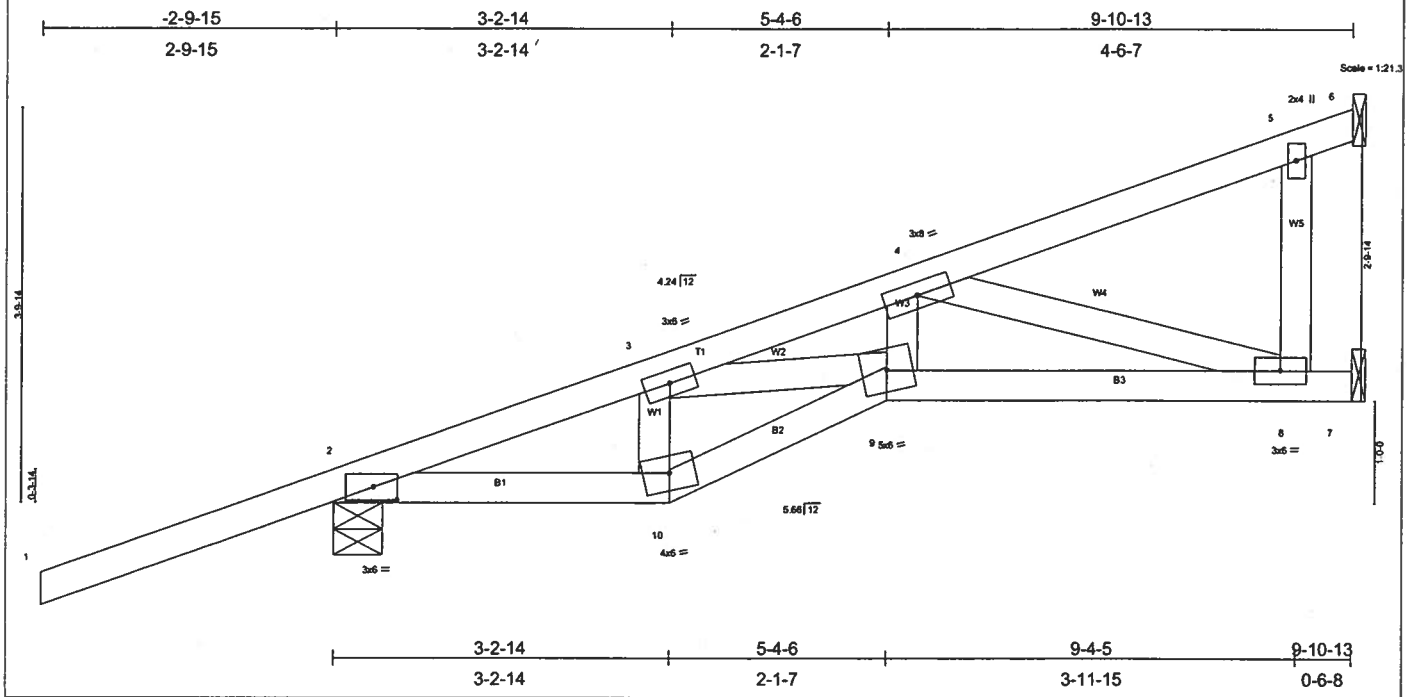


Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.08	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.13	8-9	>883	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.03	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
8-2-7 oc bracing: 8-9.

REACTIONS (lb/size) 6=289/Mechanical, 2=535/0-5-11, 7=355/Mechanical
Max Horz 2=268(load case 2)
Max Uplift6=-47(load case 2), 2=-281(load case 2), 7=-246(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-635/7, 3-4=-1466/410, 4-5=-90/31, 5-6=-19/94
BOT CHORD 2-10=-155/563, 9-10=-162/618, 8-9=-543/1228, 7-8=0/0
WEBS 3-10=-307/150, 3-9=-449/860, 4-9=-64/484, 4-8=-1277/565, 5-8=0/155

JOINT STRESS INDEX
2 = 0.73, 3 = 0.43, 4 = 0.75, 5 = 0.09, 8 = 0.36, 9 = 0.61 and 10 = 0.36

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 6, 281 lb uplift at joint 2 and 246 lb uplift at joint 7.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-6=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-10=-23(F=3, B=3), 10=-23(F=3, B=3)-to-9=-40(F=-5, B=-5), 9=-40(F=-5, B=-5)-to-7=-74(F=-22, B=-22)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061195

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T01	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:52 2006 Page 1

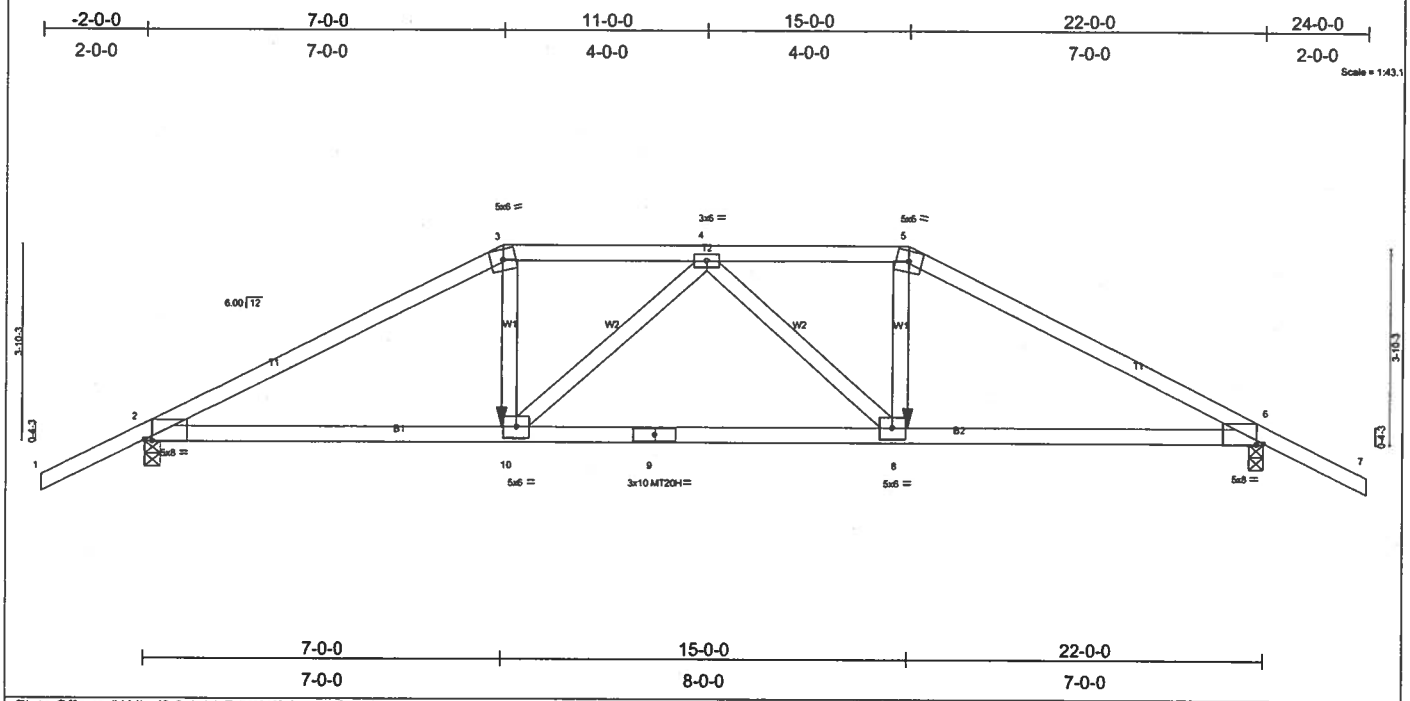


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.28	8-10	>935	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.46	8-10	>561	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.38	Horz(TL)	0.10	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 99 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-11-7 oc bracing.

REACTIONS (lb/size) 2=1926/0-3-8, 6=1926/0-3-8
Max Horz 2=-87(load case 5)
Max Uplift2=-822(load case 4), 6=-822(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3486/1289, 3-4=-3070/1226, 4-5=-3070/1226, 5-6=-3486/1290, 6-7=0/47
BOT CHORD 2-10=-1099/3024, 9-10=-1277/3280, 8-9=-1277/3280, 6-8=-1058/3024
WEBS 3-10=-384/1183, 4-10=-399/316, 4-8=-399/316, 5-8=-384/1183

JOINT STRESS INDEX
2 = 0.79, 3 = 0.76, 4 = 0.36, 5 = 0.76, 6 = 0.79, 8 = 0.42, 9 = 0.97 and 10 = 0.42

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 822 lb uplift at joint 2 and 822 lb uplift at joint 6.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 255 lb up at 15-0-0, and 539 lb down and 255 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-113(F=-59), 5-7=-54, 2-10=-30, 8-10=-62(F=-33), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061196

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T02	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:52 2006 Page 1

-2-0-0	4-9-4	9-0-0	13-0-0	17-2-12	22-0-0	24-0-0
2-0-0	4-9-4	4-2-12	4-0-0	4-2-12	4-9-4	2-0-0

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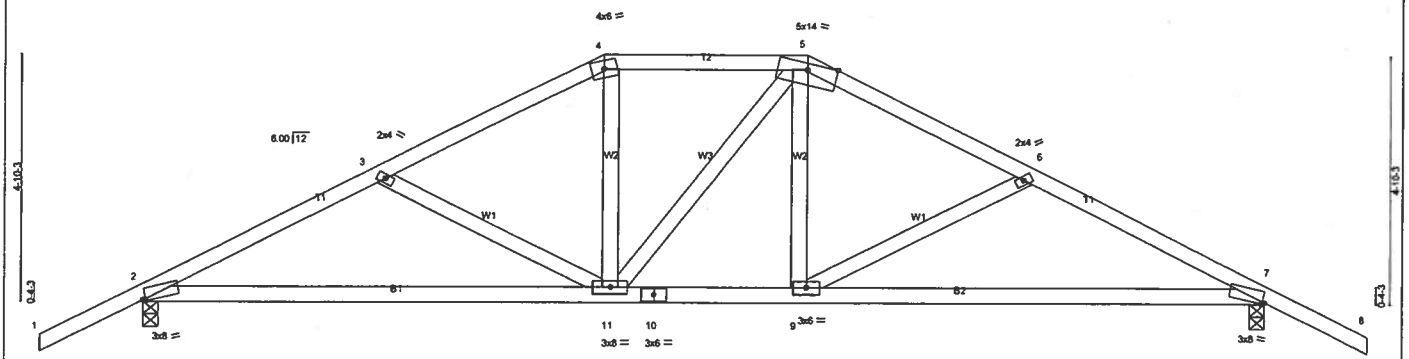

9-0-0	13-0-0	22-0-0
9-0-0	4-0-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.17	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.30	7-9	>873	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 111 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-0 oc bracing.

REACTIONS (lb/size) 2=1028/0-3-8, 7=1028/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-410(load case 5), 7=-410(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1505/635, 3-4=-1246/523, 4-5=-1074/519, 5-6=-1245/523, 6-7=-1505/635, 7-8=0/47
BOT CHORD 2-11=-404/1305, 10-11=-210/1073, 9-10=-210/1073, 7-9=-404/1305
WEBS 3-11=-273/221, 4-11=-59/313, 5-11=-100/103, 5-9=-58/314, 6-9=-274/221

JOINT STRESS INDEX
2 = 0.80, 3 = 0.34, 4 = 0.40, 5 = 0.38, 6 = 0.34, 7 = 0.80, 9 = 0.35, 10 = 0.58 and 11 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 2 and 410 lb uplift at joint 7.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061197

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T03	ATTIC	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:56:01 2006 Page 1

-2-0-0	4-0-8	7-4-4	9-1-15	11-0-0	12-10-1	14-7-12	17-11-8	22-0-0	24-0-0
2-0-0	4-0-8	3-3-12	1-9-11	1-10-1	1-10-1	1-9-11	3-3-12	4-0-8	2-0-0

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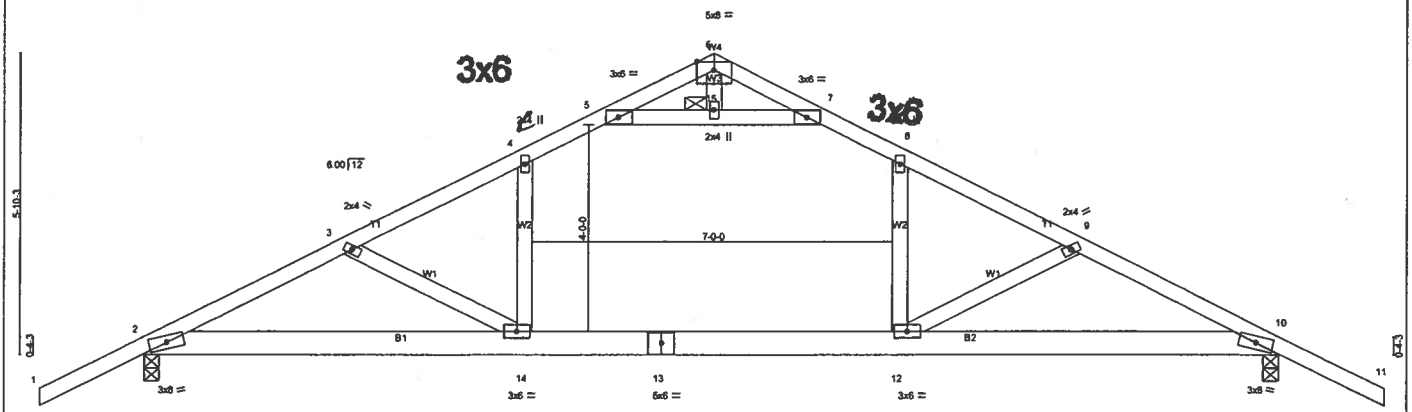


0-1-12	4-0-8	7-4-4	14-7-12	17-11-8	22-0-0
0-1-12	3-10-11	3-3-12	7-3-8	3-3-12	4-0-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.27	12-14	>955	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.45	12-14	>575	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.11	12-14	>999	240	Weight: 123 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 2=1394/0-3-8, 10=1394/0-3-8
Max Horz 2=117(load case 5)
Max Uplift2=-319(load case 5), 10=-319(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-2504/442, 3-4=-2167/326, 4-5=-1739/357, 5-6=0/465, 6-7=0/465, 7-8=-1739/357, 8-9=-2167/326, 9-10=-2504/442, 10-11=0/51
BOT CHORD 2-14=-249/2205, 13-14=-52/1826, 12-13=-52/1826, 10-12=-249/2205
WEBS 5-15=-2364/375, 7-15=-2364/375, 4-14=0/734, 8-12=0/734, 3-14=-452/224, 9-12=-452/224, 6-15=0/77

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.59, 5 = 0.69, 6 = 0.60, 7 = 0.69, 8 = 0.59, 9 = 0.34, 10 = 0.78, 12 = 0.53, 13 = 0.86, 14 = 0.53 and 15 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-15, 7-15; Wall dead load (5.0psf) on member(s).4-14, 8-12
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 2 and 319 lb uplift at joint 10.

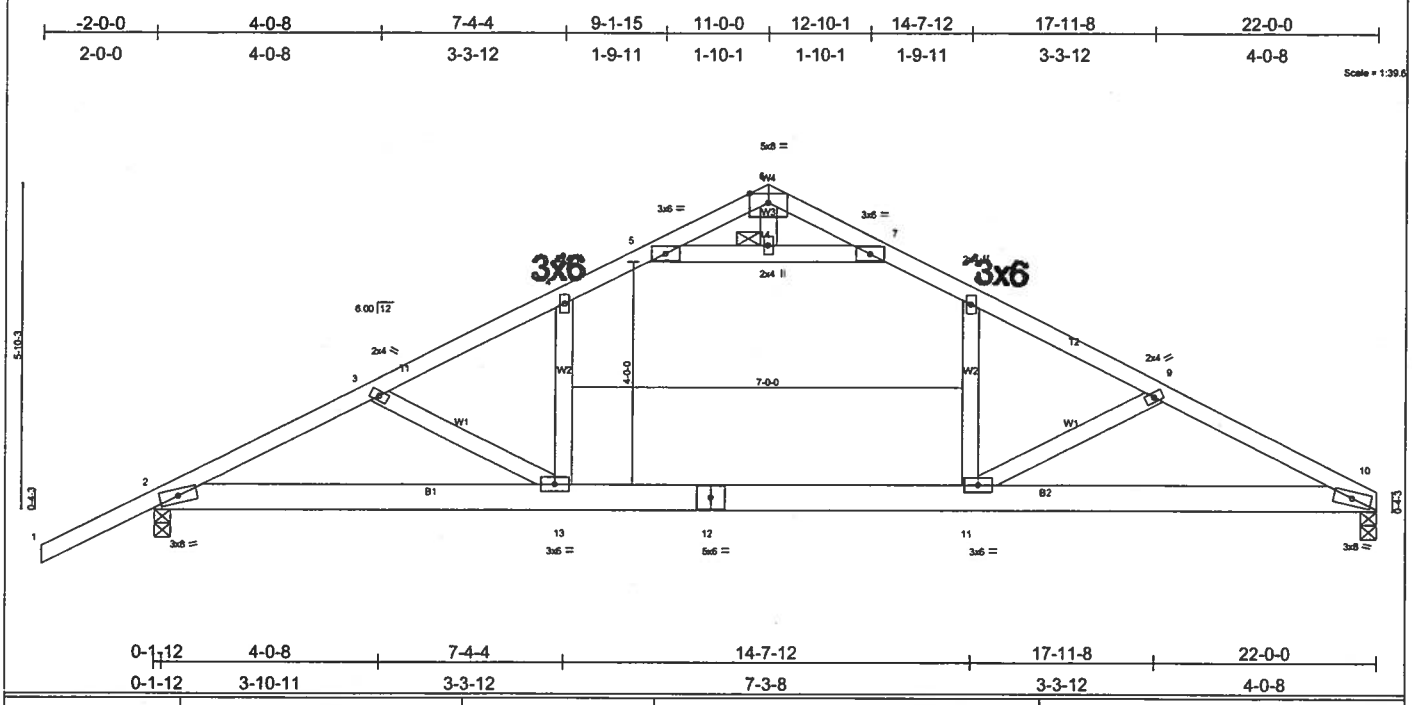
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061198

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T04	ATTIC	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:56:29 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.27	11-13	>953	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.45	11-13	>573	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Wind(LL)	0.12	11-13	>999	240	Weight: 120 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 14

REACTIONS (lb/size) 2=1400/0-3-8, 10=1272/0-3-8
Max Horz 2=144(load case 5)
Max Uplift2=-322(load case 5), 10=-191(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-2520/470, 3-4=-2186/357, 4-5=-1755/385, 5-6=0/469, 6-7=0/472, 7-8=-1753/381, 8-9=-2190/365, 9-10=-2533/506
BOT CHORD 2-13=-357/2219, 12-13=-166/1844, 11-12=-166/1844, 10-11=-402/2245
WEBS 5-14=-2388/416, 7-14=-2388/416, 4-13=0/738, 8-11=-27/754, 3-13=-451/218, 9-11=-478/269, 6-14=0/78

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.60, 5 = 0.70, 6 = 0.61, 7 = 0.70, 8 = 0.60, 9 = 0.34, 10 = 0.79, 11 = 0.54, 12 = 0.86, 13 = 0.54 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-14, 7-14; Wall dead load (5.0psf) on member(s).4-13, 8-11
4) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 322 lb uplift at joint 2 and 191 lb uplift at joint 10.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061199

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T05	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:55 2006 Page 1

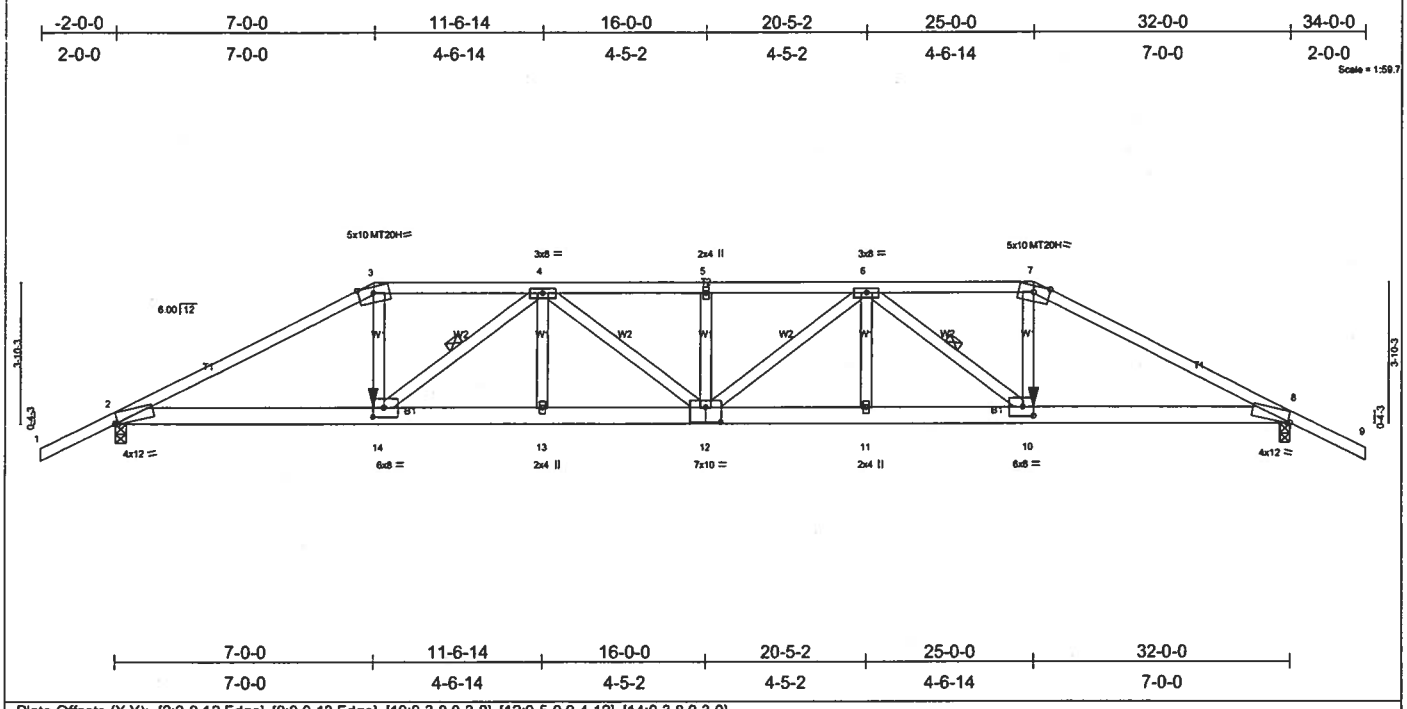


Plate Offsets (X,Y): [2:0-0-13,Edge], [8:0-0-13,Edge], [10:0-3-8,0-3-0], [12:0-5-0,0-4-12], [14:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.46	12	>833	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.73	12	>521	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.17	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-4 oc bracing.
WEBS 1 Row at midpt 4-14, 6-10

REACTIONS (lb/size) 2=2801/0-3-8, 8=2801/0-3-8
Max Horz 2=-89(load case 5)
Max Uplift2=-1131(load case 4), 8=-1131(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5417/2142, 3-4=-4845/1981, 4-5=-6781/2785, 5-6=-6781/2785, 6-7=-4845/1981, 7-8=-5417/2142, 8-9=0/51
BOT CHORD 2-14=-1870/4762, 13-14=-2539/6325, 12-13=-2539/6325, 11-12=-2507/6325, 10-11=-2507/6325, 8-10=-1831/4762
WEBS 3-14=-718/1986, 4-14=-1976/892, 4-13=0/311, 4-12=-249/611, 5-12=-486/367, 6-12=-250/611, 6-11=0/311, 6-10=-1976/892, 7-10=-718/1986

JOINT STRESS INDEX
2 = 0.81, 3 = 0.89, 4 = 0.96, 5 = 0.34, 6 = 0.96, 7 = 0.89, 8 = 0.81, 10 = 0.55, 11 = 0.34, 12 = 0.99, 13 = 0.34 and 14 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1131 lb uplift at joint 2 and 1131 lb uplift at joint 8.
7) Girder carries hip end with 7-0-0 end setback.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 255 lb up at 25-0-0, and 539 lb down and 255 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-113(F=-59), 7-9=-54, 2-14=-30, 10-14=-62(F=-33), 8-10=-30
Concentrated Loads (lb)
Vert: 14=-539(F) 10=-539(F)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061200

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T06	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:55 2006 Page 1

-2-0-0	4-9-4	9-0-0	16-0-0	23-0-0	27-2-12	32-0-0	34-0-0
2-0-0	4-9-4	4-2-12	7-0-0	7-0-0	4-2-12	4-9-4	2-0-0

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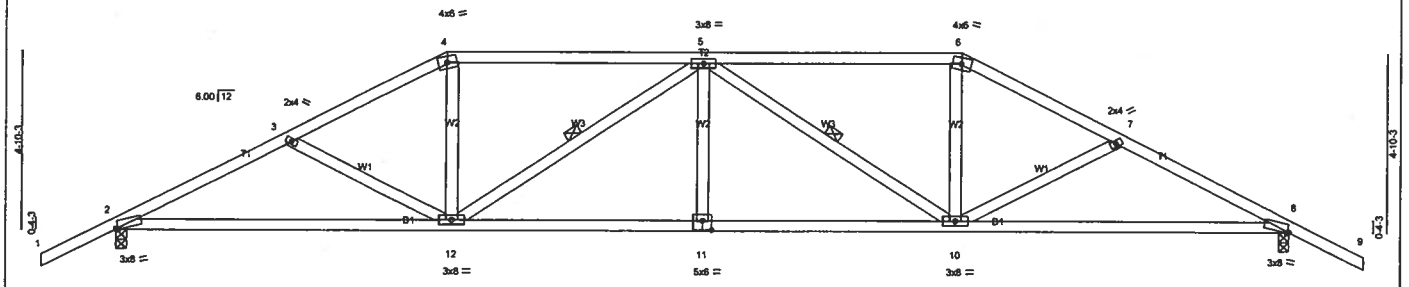


9-0-0	16-0-0	23-0-0	32-0-0
9-0-0	7-0-0	7-0-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.21	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.35	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.20	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-15 oc bracing.
WEBS 1 Row at midpt 5-12, 5-10

REACTIONS (lb/size) 2=1448/0-3-8, 8=1448/0-3-8
Max Horz 2=101(load case 5)
Max Uplift2=-507(load case 5), 8=-507(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2399/978, 3-4=-2180/886, 4-5=-1930/854, 5-6=-1930/854, 6-7=-2180/886, 7-8=-2399/978, 8-9=0/47
BOT CHORD 2-12=-706/2092, 11-12=-731/2357, 10-11=-731/2357, 8-10=-706/2092
WEBS 3-12=-209/188, 4-12=-150/636, 5-12=-605/270, 5-11=0/174, 5-10=-605/269, 6-10=-150/636, 7-10=-209/189

JOINT STRESS INDEX
2 = 0.84, 3 = 0.34, 4 = 0.77, 5 = 0.57, 6 = 0.77, 7 = 0.34, 8 = 0.84, 10 = 0.57, 11 = 0.59 and 12 = 0.57

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 507 lb uplift at joint 2 and 507 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061201

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T07	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:56 2006 Page 1

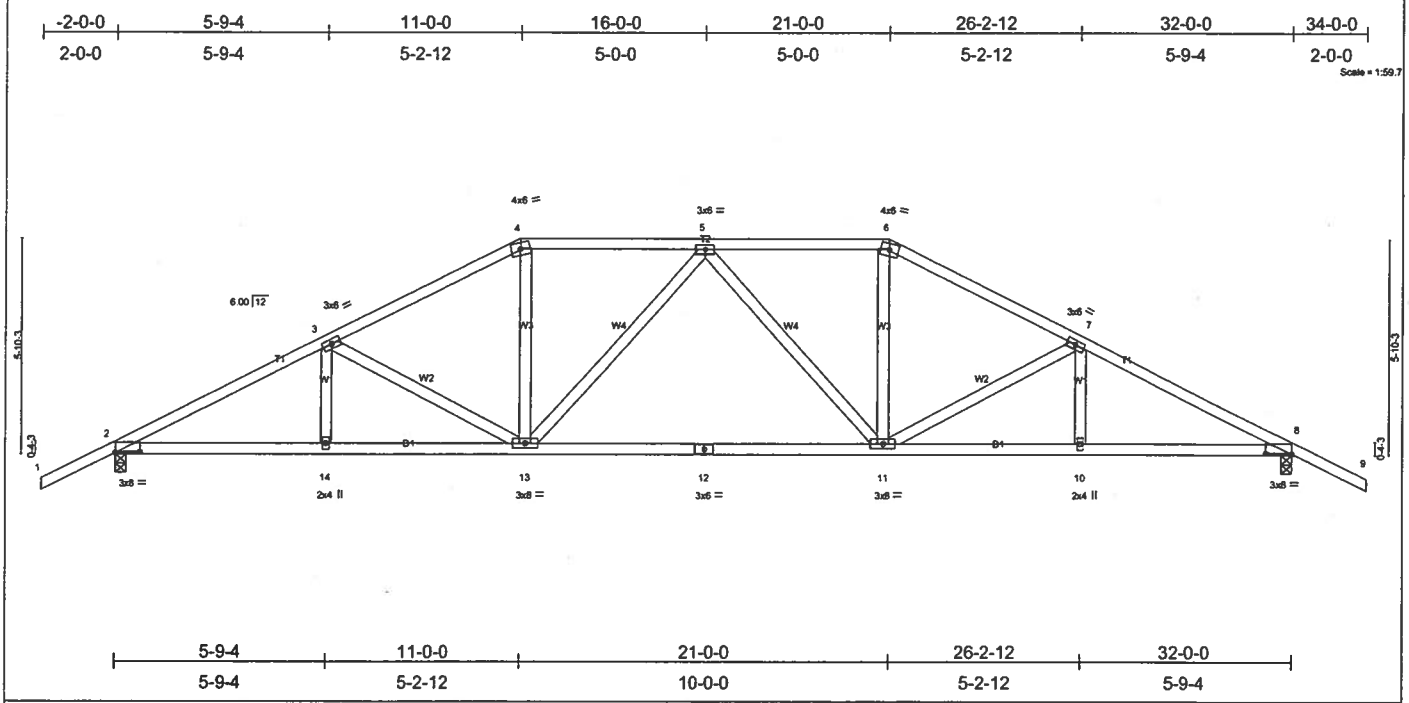


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-0-0,0-0-0], [4:0-0-0,0-0-0], [5:0-0-0,0-0-0], [6:0-0-0,0-0-0], [7:0-0-0,0-0-0], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.31	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.51	11-13	>740	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.11	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 168 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-6-15 oc bracing.

REACTIONS (lb/size) 2=1448/0-3-8, 8=1448/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-524(load case 5), 8=-524(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2451/965, 3-4=-2009/857, 4-5=-1755/828, 5-6=-1755/828, 6-7=-2009/857, 7-8=-2451/965, 8-9=0/47
BOT CHORD 2-14=-689/2115, 13-14=-689/2115, 12-13=-542/1866, 11-12=-542/1866, 10-11=-689/2115, 8-10=-689/2115
WEBS 3-14=0/134, 3-13=-433/258, 4-13=-163/589, 5-13=-281/180, 5-11=-281/180, 6-11=-163/589, 7-11=-433/258, 7-10=0/134

JOINT STRESS INDEX
2 = 0.78, 3 = 0.40, 4 = 0.61, 5 = 0.38, 6 = 0.61, 7 = 0.40, 8 = 0.78, 10 = 0.34, 11 = 0.57, 12 = 0.96, 13 = 0.57 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 2 and 524 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

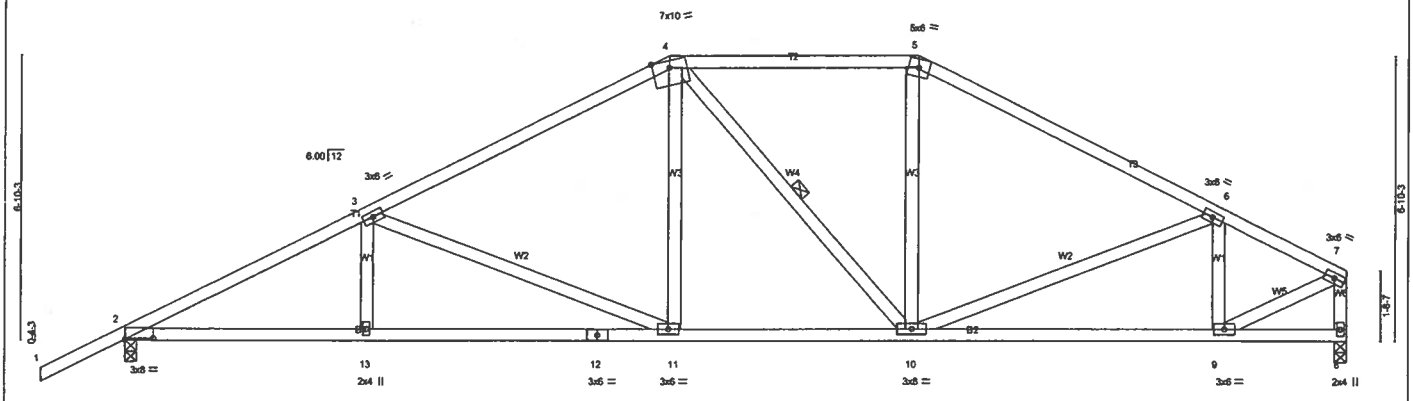
Dwg.#0403061202

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T08	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:57 2006 Page 1

-2-0-0 | 5-9-4 | 13-0-0 | 19-0-0 | 26-2-12 | 29-3-8
2-0-0 | 5-9-4 | 7-2-12 | 6-0-0 | 7-2-12 | 3-0-12

Scale = 1:52.8



5-9-4 | 13-0-0 | 19-0-0 | 26-2-12 | 29-3-8
5-9-4 | 7-2-12 | 6-0-0 | 7-2-12 | 3-0-12

Plate Offsets (X,Y): [2:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.16	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.26	11-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-3 oc bracing.
WEBS 1 Row at midpt 4-10

REACTIONS (lb/size) 2=1338/0-3-8, 8=1214/0-3-8
Max Horz 2=201(load case 5)
Max Uplift2=-511(load case 5), 8=-364(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2264/902, 3-4=-1609/726, 4-5=-1274/690, 5-6=-1504/683, 6-7=-1325/571, 7-8=-1202/507
BOT CHORD 2-13=-794/1955, 12-13=-794/1955, 11-12=-794/1955, 10-11=-471/1371, 9-10=-499/1185, 8-9=-7/12
WEBS 3-13=0/211, 3-11=-639/350, 4-11=-96/433, 4-10=-271/108, 5-10=-49/324, 6-10=-81/205, 6-9=-410/300, 7-9=-566/1325

JOINT STRESS INDEX
2 = 0.72, 3 = 0.41, 4 = 0.72, 5 = 0.66, 6 = 0.41, 7 = 0.79, 8 = 0.44, 9 = 0.76, 10 = 0.57, 11 = 0.35, 12 = 0.74 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 511 lb uplift at joint 2 and 364 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

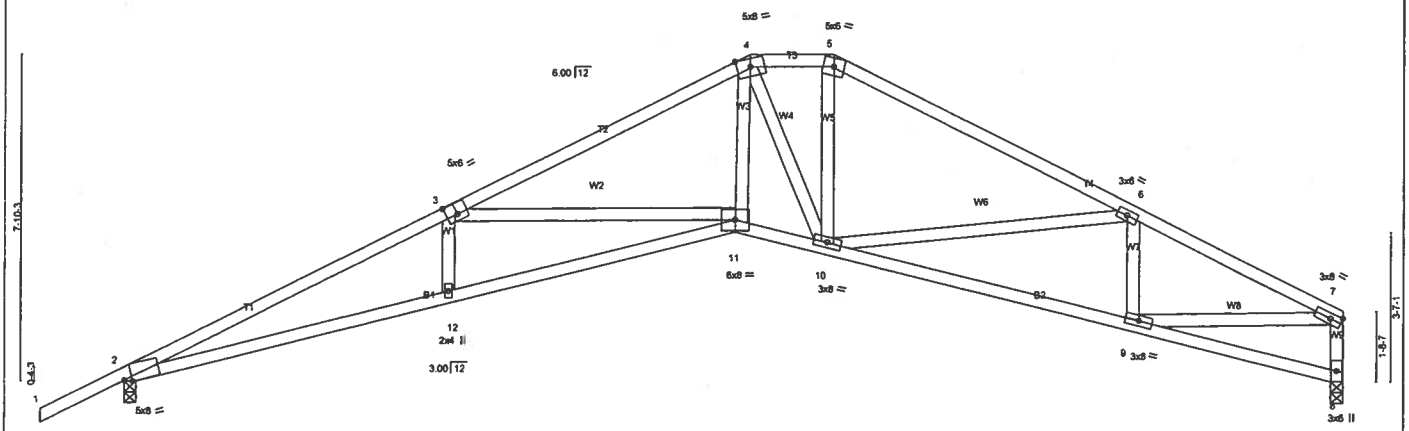
Dwg.#0403061203

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:58 2006 Page 1

-2-0-0	7-9-4	15-0-0	17-0-0	24-2-12	29-3-8
2-0-0	7-9-4	7-2-12	2-0-0	7-2-12	5-0-12

Scale = 1:52.8



7-9-4	14-7-12	17-0-0	24-2-12	29-3-8
7-9-4	6-10-8	2-4-4	7-2-12	5-0-12

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.39	11-12	>888	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.63	11-12	>551	180		
BCLL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(TL)	0.38	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-7 oc bracing.

REACTIONS (lb/size) 2=1338/0-3-8, 8=1214/0-3-8
Max Horz 2=214(load case 5)
Max Uplift2=-522(load case 5), 8=-393(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-3996/1631, 3-4=-2729/1125, 4-5=-1972/946, 5-6=-2266/961, 6-7=-2189/938, 7-8=-1169/536
BOT CHORD 2-12=-1458/3607, 11-12=-1460/3599, 10-11=-746/2282, 9-10=-810/2000, 8-9=-52/120
WEBS 3-12=0/244, 3-11=-1134/640, 4-11=-544/1577, 5-10=-215/701, 6-10=-96/189, 6-9=-369/274, 7-9=-730/1818, 4-10=-707/271

JOINT STRESS INDEX
2 = 0.77, 3 = 0.76, 4 = 0.61, 5 = 0.68, 6 = 0.41, 7 = 0.83, 8 = 0.29, 9 = 0.78, 10 = 0.64, 11 = 0.85 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 522 lb uplift at joint 2 and 393 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

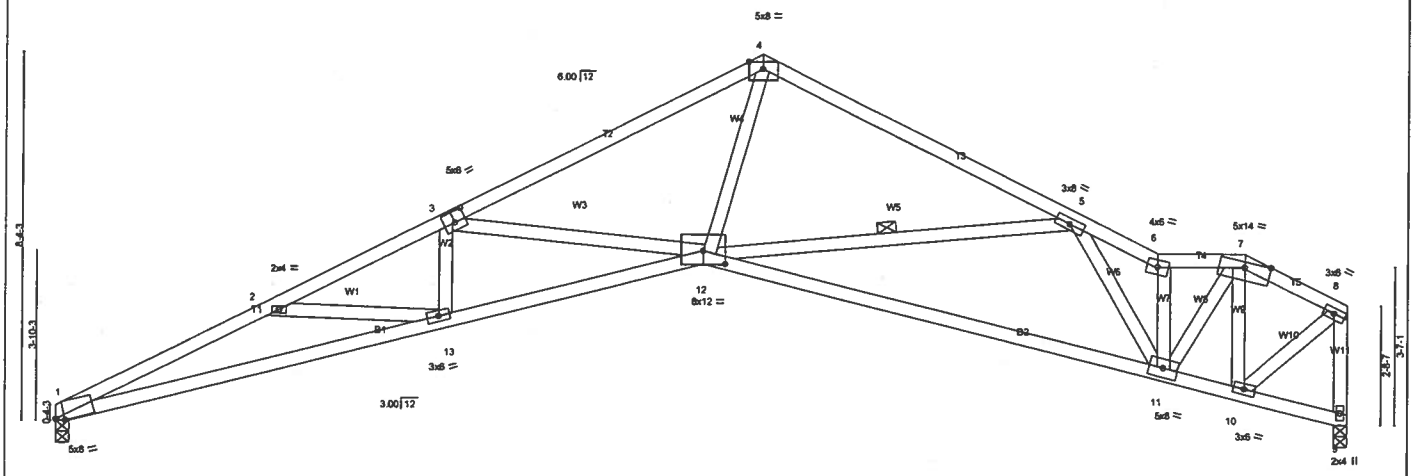
Dwg.#0403061204

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:58 2006 Page 1

5-0-0	8-9-4	16-0-0	23-2-12	25-0-0	27-0-0	29-3-8
5-0-0	3-9-4	7-2-12	7-2-12	1-9-4	2-0-0	2-3-8

Scale = 1:50.0



8-9-4	14-7-12	25-0-0	27-0-0	29-3-8
8-9-4	5-10-8	10-4-4	2-0-0	2-3-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [12:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.46	11-12	>755	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.77	11-12	>453	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.38	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 155 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-12

REACTIONS (lb/size) 1=1218/0-3-8, 9=1218/0-3-8
Max Horz 1=193(load case 5)
Max Uplift1=-399(load case 5), 9=-388(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4010/1865, 2-3=-3740/1671, 3-4=-2767/1196, 4-5=-2179/982, 5-6=-1917/795, 6-7=-1699/711, 7-8=-962/443, 8-9=-1156/539
BOT CHORD 1-13=-1760/3649, 12-13=-1502/3441, 11-12=-900/2081, 10-11=-352/860, 9-10=-9/27
WEBS 3-13=-40/270, 3-12=-967/588, 4-12=-663/1800, 5-12=-289/253, 5-11=-475/382, 6-11=-798/306, 7-11=-524/1560, 7-10=-1016/355, 8-10=-470/1060, 2-13=-208/248

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.64, 4 = 0.84, 5 = 0.40, 6 = 0.79, 7 = 0.73, 8 = 0.77, 9 = 0.57, 10 = 0.72, 11 = 0.81, 12 = 0.64 and 13 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 1 and 388 lb uplift at joint 9.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

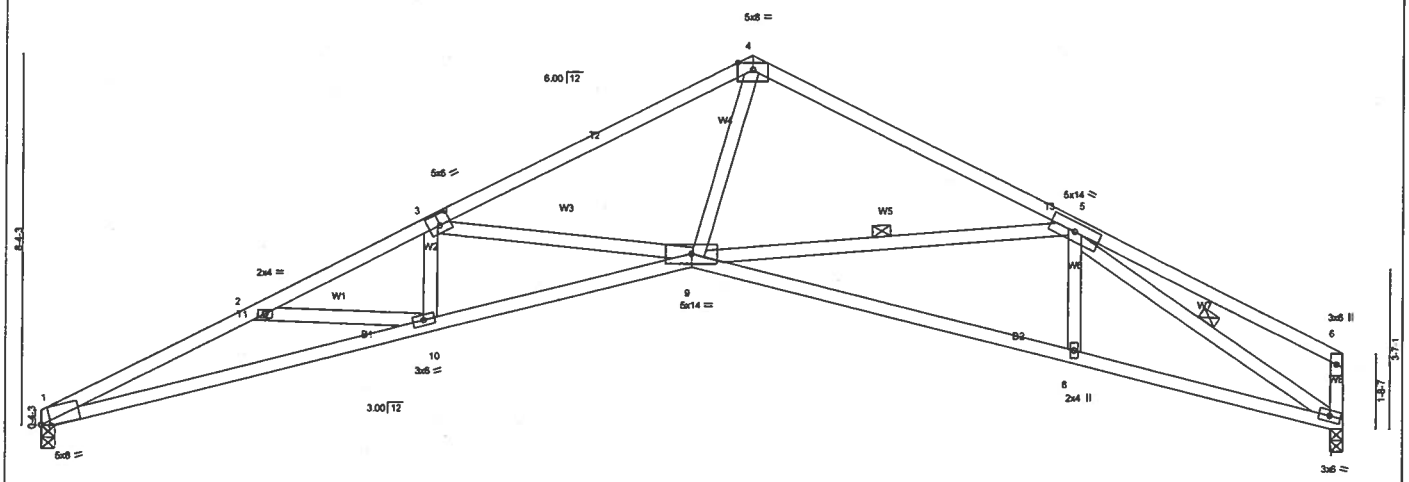
Dwg.#0403061205

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T10A	SCISSOR	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:53:59 2006 Page 1

8-9-4 | 16-0-0 | 23-2-12 | 29-3-8
8-9-4 | 7-2-12 | 7-2-12 | 6-0-12

Scale = 1:49.5



7-8-13 | 14-7-12 | 23-2-12 | 29-3-8
7-8-13 | 6-10-15 | 8-7-0 | 6-0-12

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.40	1-10	>878	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.65	8-9	>537	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.40	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-9-5 oc bracing.
WEBS 1 Row at midpt 5-9, 5-7

REACTIONS (lb/size) 1=1218/0-3-8, 7=1218/0-3-8
Max Horz 1=161(load case 5)
Max Uplift1=-401(load case 5), 7=-385(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4014/1834, 2-3=-3735/1634, 3-4=-2751/1165, 4-5=-2171/959, 5-6=-291/211, 6-7=-260/207
BOT CHORD 1-10=-1677/3652, 9-10=-1417/3443, 8-9=-845/2115, 7-8=-838/2105
WEBS 3-9=-987/583, 4-9=-638/1783, 5-9=-337/267, 5-8=0/226, 5-7=-2286/883, 3-10=-38/283, 2-10=-206/251

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.64, 4 = 0.86, 5 = 0.57, 6 = 0.54, 7 = 0.70, 8 = 0.34, 9 = 1.00 and 10 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 401 lb uplift at joint 1 and 385 lb uplift at joint 7.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

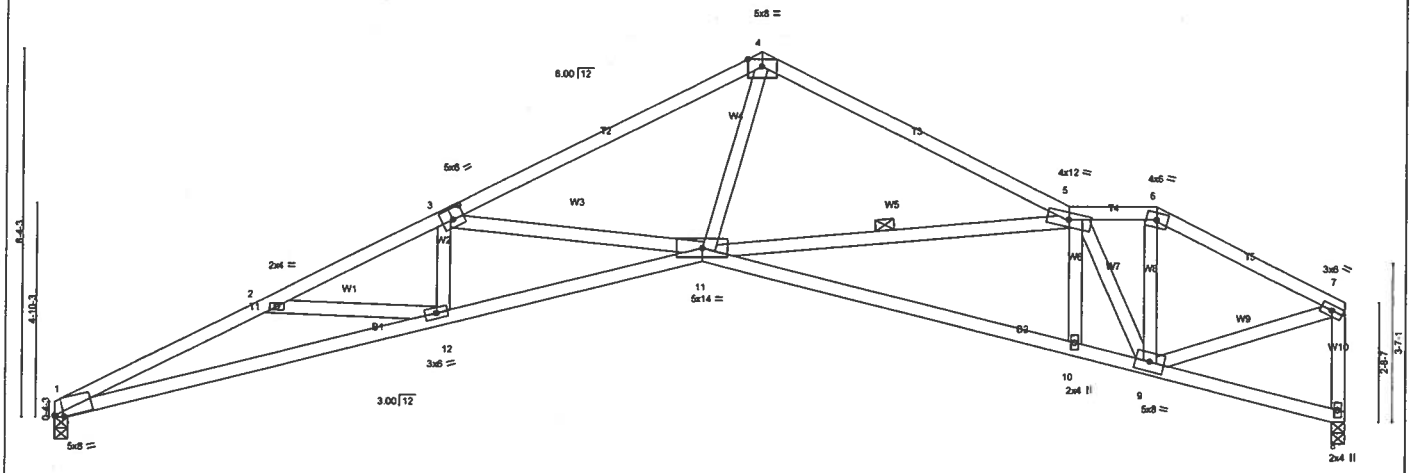
Dwg.#0403061206

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:00 2006 Page 1

5-0-0	8-9-4	16-0-0	23-0-0	25-0-0	29-3-8
5-0-0	3-9-4	7-2-12	7-0-0	2-0-0	4-3-8

Scale = 1:50.0



8-9-4	14-7-12	23-0-0	25-0-0	29-3-8
8-9-4	5-10-8	8-4-4	2-0-0	4-3-8

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.39	1-12	>895	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.64	1-12	>548	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.37	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-0 oc bracing.
WEBS 1 Row at midpt 5-11

REACTIONS (lb/size) 1=1218/0-3-8, 8=1218/0-3-8
Max Horz 1=193(load case 5)
Max Uplift1=-399(load case 5), 8=-388(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4015/1862, 2-3=-3736/1667, 3-4=-2748/1198, 4-5=-2163/983, 5-6=-1287/637, 6-7=-1469/653, 7-8=-1155/544
BOT CHORD 1-12=-1757/3653, 11-12=-1500/3444, 10-11=-875/2113, 9-10=-862/2119, 8-9=-32/68
WEBS 3-12=-37/285, 3-11=-989/583, 4-11=-664/1779, 5-11=-337/232, 5-10=0/239, 5-9=-1593/661, 6-9=-216/512, 7-9=-519/1277, 2-12=-206/248

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.64, 4 = 0.85, 5 = 0.95, 6 = 0.30, 7 = 0.80, 8 = 0.78, 9 = 0.55, 10 = 0.34, 11 = 1.00 and 12 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 1 and 388 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061207

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:00 2006 Page 1

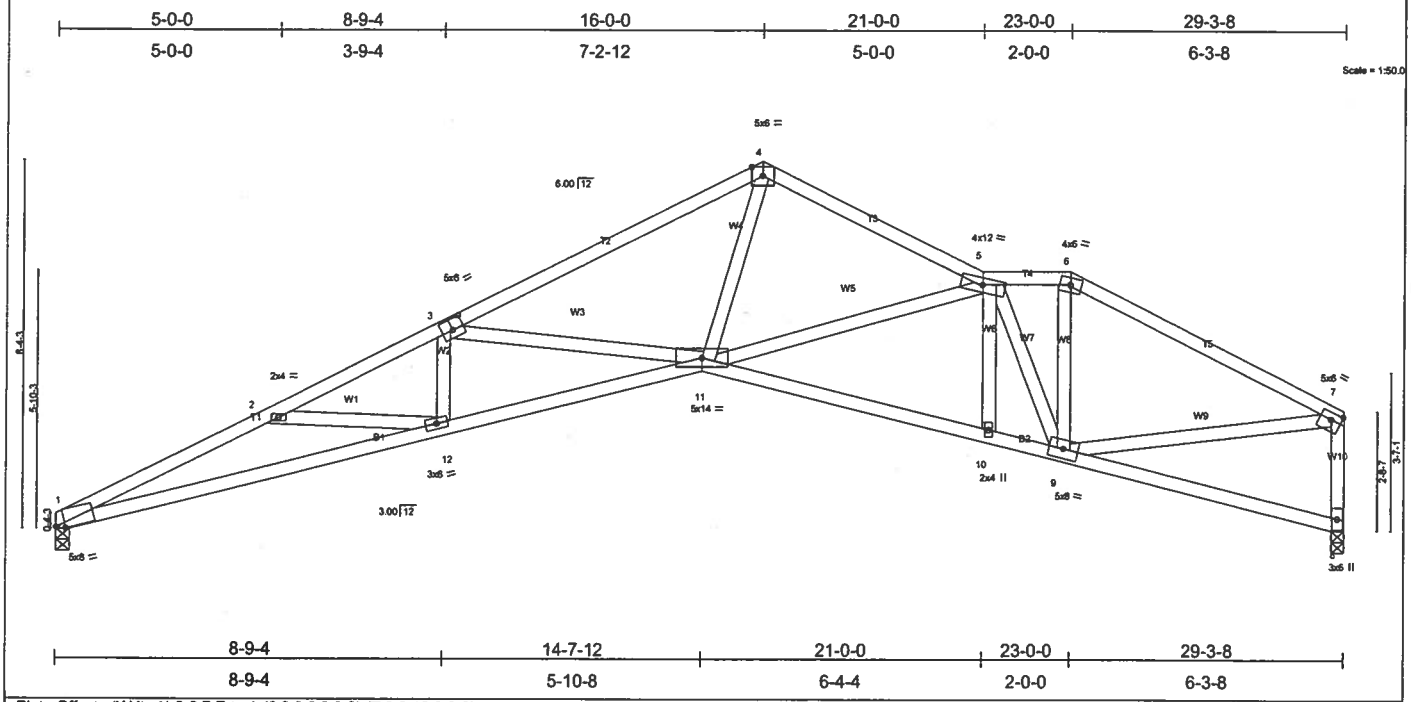


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0], [7:0-2-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.38	1-12	>905	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.63	1-12	>555	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.37	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 156 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-1 oc bracing.

REACTIONS (lb/size) 1=1218/0-3-8, 8=1218/0-3-8
Max Horz 1=193(load case 5)
Max Uplift1=-399(load case 5), 8=-388(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4015/1857, 2-3=-3747/1669, 3-4=-2727/1192, 4-5=-2087/978, 5-6=-1544/774, 6-7=-1783/784, 7-8=-1142/556
BOT CHORD 1-12=-1752/3653, 11-12=-1504/3452, 10-11=-837/2113, 9-10=-833/2101, 8-9=-64/131
WEBS 3-12=-32/297, 3-11=-1005/595, 4-11=-691/1791, 5-11=-338/203, 5-10=0/125, 5-9=-1160/451, 6-9=-107/473, 7-9=-559/1435, 2-12=-205/240

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.68, 4 = 0.67, 5 = 0.78, 6 = 0.78, 7 = 0.77, 8 = 0.41, 9 = 0.62, 10 = 0.34, 11 = 1.00 and 12 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 1 and 388 lb uplift at joint 8.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061208

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:01 2006 Page 1

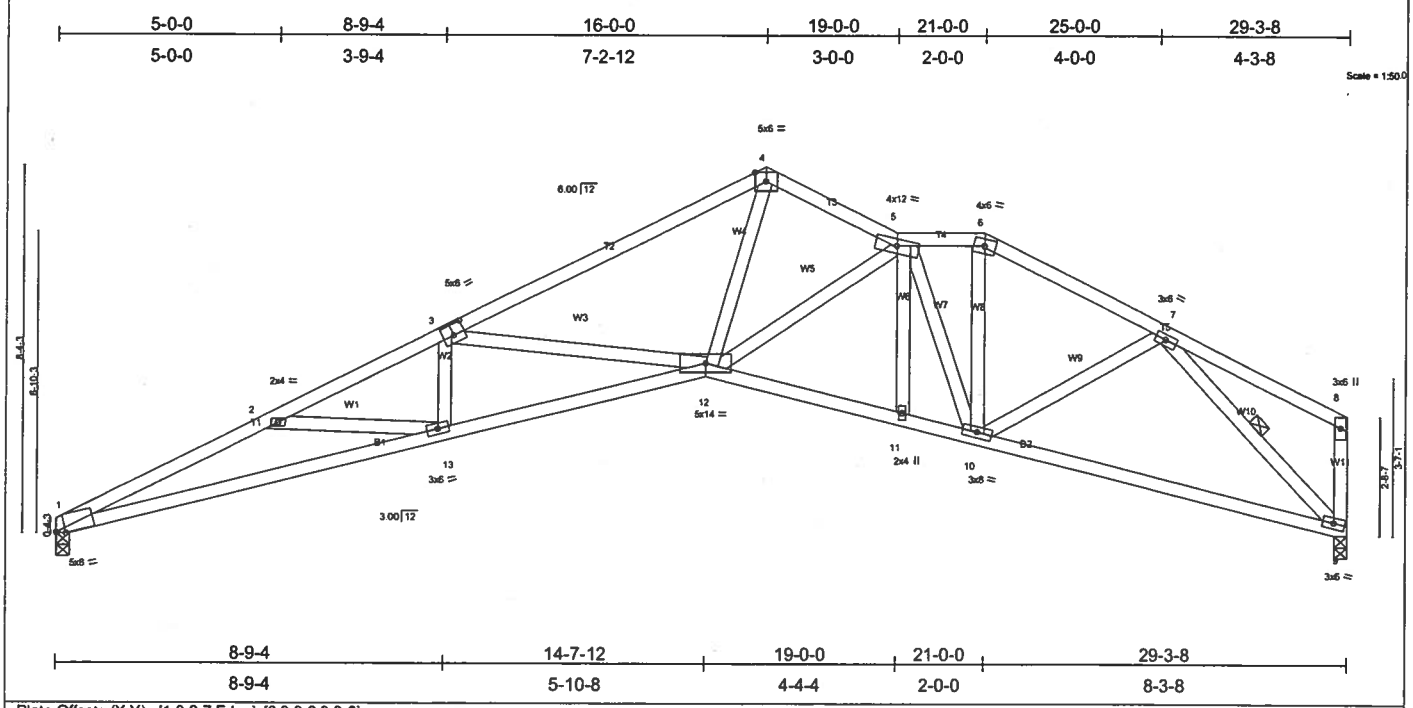


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.38	1-13	>918	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.62	1-13	>563	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.37	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-2 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 1=1218/0-3-8, 9=1218/0-3-8
Max Horz 1=193(load case 5)
Max Uplift1=-399(load case 5), 9=-388(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4017/1854, 2-3=-3750/1667, 3-4=-2717/1190, 4-5=-2042/977, 5-6=-1648/814, 6-7=-1869/853, 7-8=-171/77, 8-9=-174/111
BOT CHORD 1-13=-1749/3654, 12-13=-1503/3455, 11-12=-773/2044, 10-11=-775/2009, 9-10=-543/1248
WEBS 3-13=-31/305, 3-12=-1007/596, 4-12=-696/1782, 5-12=-295/159, 5-11=-4/2, 5-10=-831/367, 6-10=-189/554, 7-10=-91/514, 7-9=-1623/764, 2-13=-208/238

JOINT STRESS INDEX
1 = 0.77, 2 = 0.34, 3 = 0.69, 4 = 0.64, 5 = 0.64, 6 = 0.49, 7 = 0.52, 8 = 0.31, 9 = 0.75, 10 = 0.65, 11 = 0.34, 12 = 1.00 and 13 = 0.38

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 1 and 388 lb uplift at joint 9.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061209

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T14	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:02 2006 Page 1

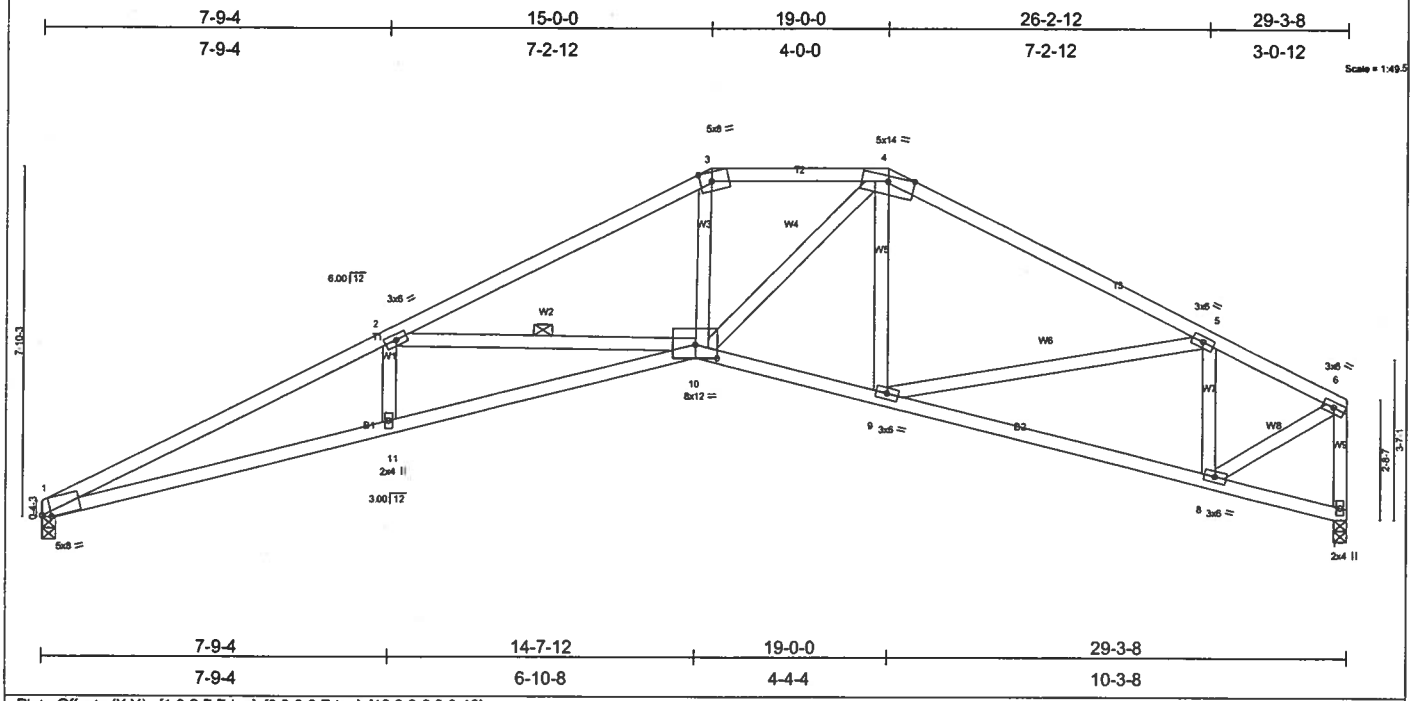


Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-3,Edge], [10:0-6-0,0-3-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.36	10-11	>957	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.59	10-11	>593	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.35	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 150 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 2-10

REACTIONS (lb/size) 1=1218/0-3-8, 7=1218/0-3-8
Max Horz 1=186(load case 5)
Max Uplift1=-394(load case 5), 7=-365(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4040/1762, 2-3=-2752/1178, 3-4=-2295/1098, 4-5=-1937/860, 5-6=-1214/534, 6-7=-1212/522
BOT CHORD 1-11=-1642/3672, 10-11=-1638/3660, 9-10=-606/1714, 8-9=-487/1146, 7-8=-9/19
WEBS 2-11=0/254, 2-10=-1159/718, 3-10=-315/914, 4-10=-342/923, 4-9=-67/91, 5-9=-133/582, 5-8=-741/449, 6-8=-554/1279

JOINT STRESS INDEX
1 = 0.78, 2 = 0.41, 3 = 0.62, 4 = 0.94, 5 = 0.41, 6 = 0.83, 7 = 0.45, 8 = 0.75, 9 = 0.38, 10 = 0.68 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 394 lb uplift at joint 1 and 365 lb uplift at joint 7.

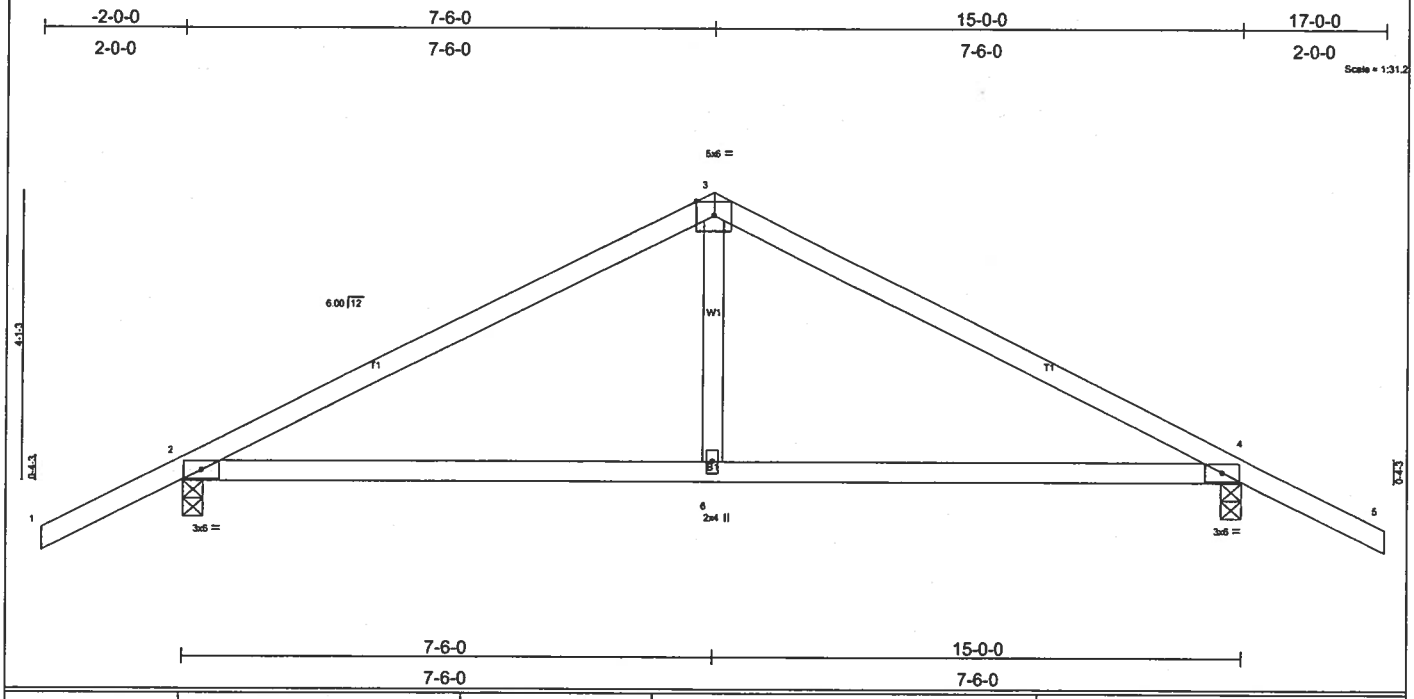
LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061210

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T15	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:02 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.10	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.15	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 59 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=734/0-3-8, 4=734/0-3-8
Max Horz 2=-91(load case 6)
Max Uplift2=-328(load case 5), 4=-328(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-862/338, 3-4=-862/338, 4-5=0/47
BOT CHORD 2-6=-124/695, 4-6=-124/695
WEBS 3-6=0/272

JOINT STRESS INDEX
2 = 0.57, 3 = 0.75, 4 = 0.57 and 6 = 0.20

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 2 and 328 lb uplift at joint 4.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

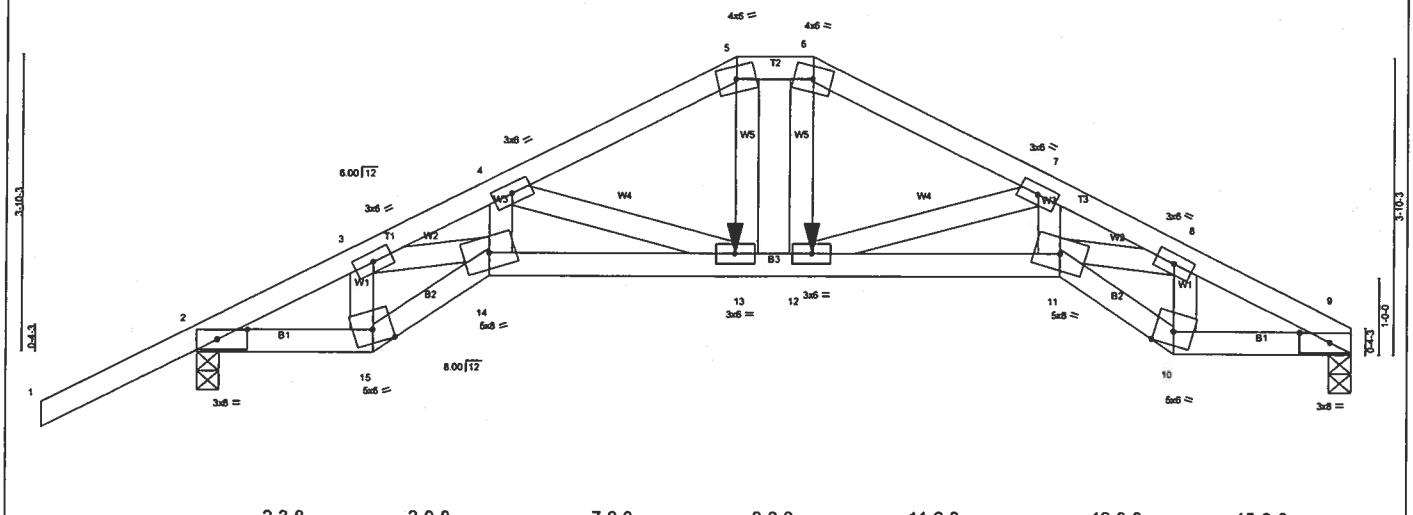
Dwg.#0403061211

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T15H	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:03 2006 Page 1

-2-0-0	2-3-8	3-9-8	7-0-0	8-0-0	11-2-8	12-8-8	15-0-0
2-0-0	2-3-8	1-6-0	3-2-8	1-0-0	3-2-8	1-6-0	2-3-8

Scale = 1:26.6



2-3-8	3-9-8	7-0-0	8-0-0	11-2-8	12-8-8	15-0-0
2-3-8	1-6-0	3-2-8	1-0-0	3-2-8	1-6-0	2-3-8

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [9:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.16	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.84	Vert(TL)	-0.25	11-12	>710	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 77 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-14 oc bracing.

REACTIONS (lb/size) 9=1053/0-3-8, 2=1186/0-3-8
Max Horz 2=112(load case 4)
Max Uplift9=-362(load case 5), 2=-497(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1936/598, 3-4=-3712/1193, 4-5=-2342/793, 5-6=-2134/750, 6-7=-2341/791, 7-8=-3785/1261, 8-9=-2036/694
BOT CHORD 2-15=-480/1647, 14-15=-518/1807, 13-14=-1015/3167, 12-13=-634/2134, 11-12=-1014/3220, 10-11=-624/1921, 9-10=-575/1750
WEBS 3-15=-916/326, 3-14=-611/1773, 4-14=-220/887, 4-13=-1134/414, 5-13=-281/859, 6-12=-283/863, 7-12=-1191/463, 7-11=-274/945, 8-11=-522/1734, 8-10=-933/327

JOINT STRESS INDEX
2 = 0.73, 3 = 0.86, 4 = 0.70, 5 = 0.47, 6 = 0.47, 7 = 0.70, 8 = 0.86, 9 = 0.71, 10 = 0.53, 11 = 0.97, 12 = 0.56, 13 = 0.56, 14 = 0.97 and 15 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 9 and 497 lb uplift at joint 2.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 7-0-0, and 400 lb down and 151 lb up at 8-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-6=-117(F=-63), 6-9=-54, 2-15=-30, 14-15=-30, 13-14=-30, 12-13=-65(F=-35), 11-12=-30, 10-11=-30, 9-10=-30
Concentrated Loads (lb)
Vert: 13=-400(F) 12=-400(F)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

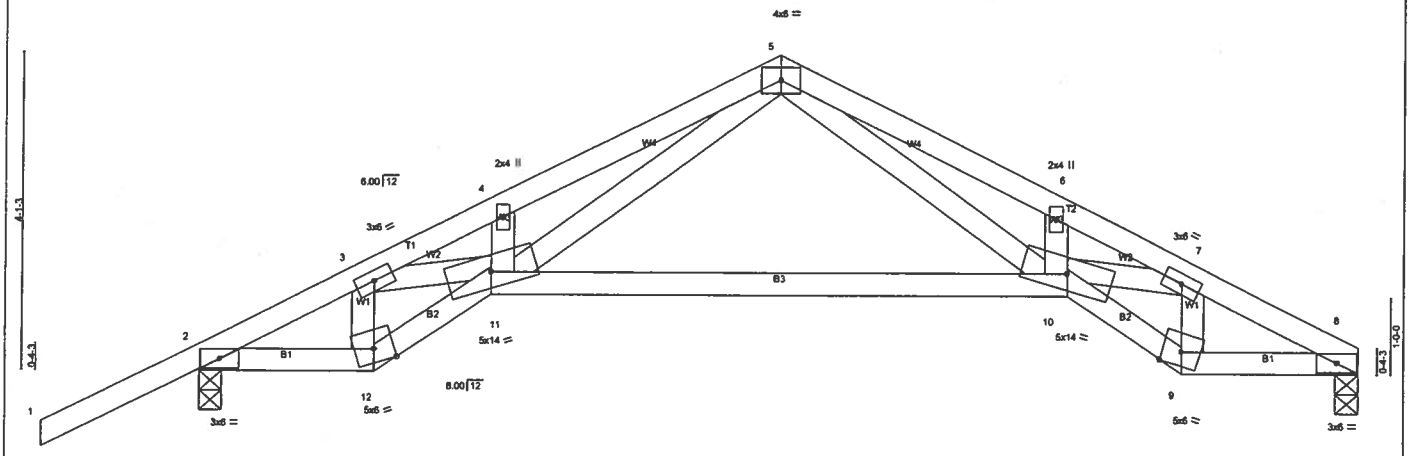
Dwg.#0403061212

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T15T	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:04 2006 Page 1

-2-0-0	2-3-8	3-9-8	7-6-0	11-2-8	12-8-8	15-0-0
2-0-0	2-3-8	1-6-0	3-8-8	3-8-8	1-6-0	2-3-8

Scale = 1:28.4



2-3-8	3-9-8	11-2-8	12-8-8	15-0-0
2-3-8	1-6-0	7-5-0	1-6-0	2-3-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.16	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.26	10-11	>676	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-6-11 oc bracing.

REACTIONS (lb/size) 8=609/0-3-8, 2=742/0-3-8
Max Horz 2=115(load case 5)
Max Uplift8=-197(load case 6), 2=-332(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1001/335, 3-4=-1883/673, 4-5=-1865/751, 5-6=-1920/842, 6-7=-1948/772, 7-8=-1096/500
BOT CHORD 2-12=-232/839, 11-12=-243/947, 10-11=-238/797, 9-10=-428/1059, 8-9=-403/942
WEBS 3-12=-523/186, 3-11=-314/890, 4-11=-96/146, 5-11=-360/1094, 5-10=-464/1157, 6-10=-82/127, 7-10=-228/837, 7-9=-535/204

JOINT STRESS INDEX
2 = 0.64, 3 = 0.43, 4 = 0.08, 5 = 0.88, 6 = 0.08, 7 = 0.43, 8 = 0.64, 9 = 0.32, 10 = 0.45, 11 = 0.45 and 12 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 197 lb uplift at joint 8 and 332 lb uplift at joint 2.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061213

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T16	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:05 2006 Page 1

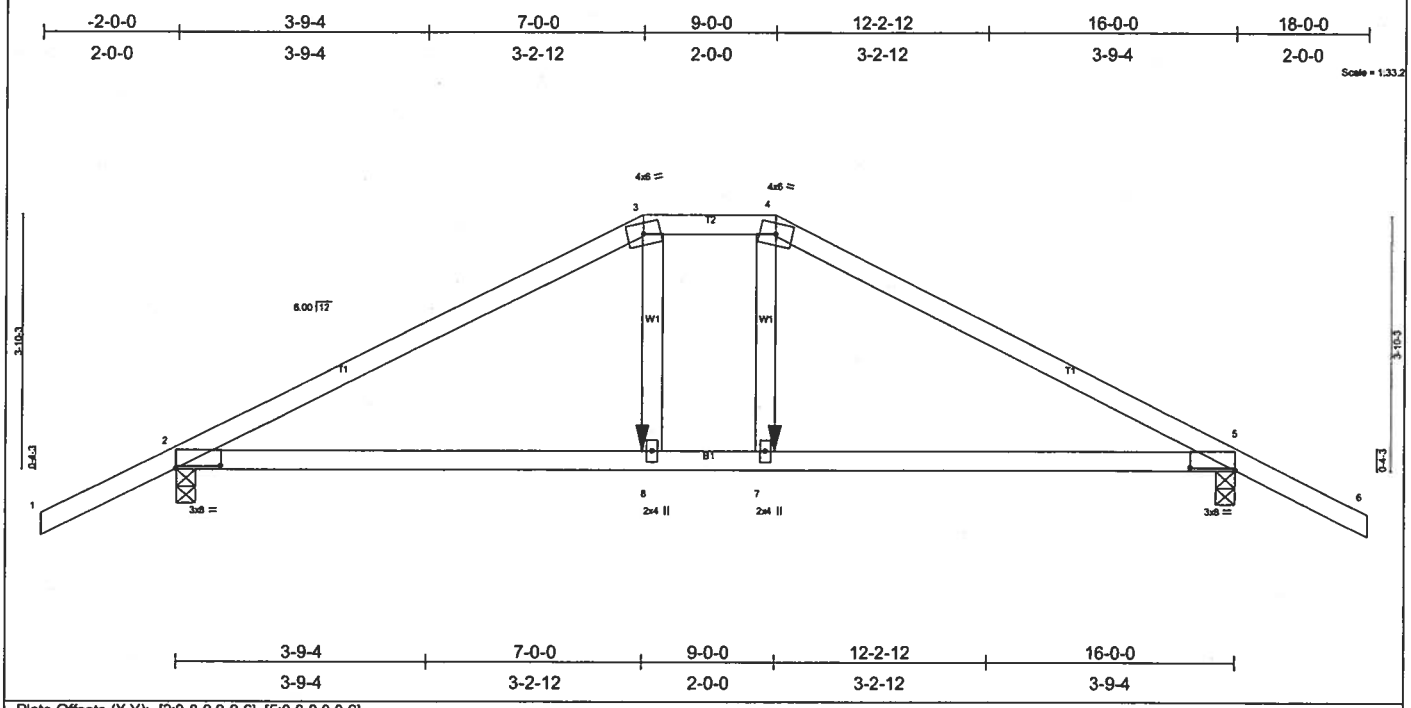


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.17	5-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.25	5-7	>760	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-12 oc bracing.

REACTIONS (lb/size) 2=1408/0-3-8, 5=1408/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-862(load case 4), 5=-862(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2314/1232, 3-4=-2013/1160, 4-5=-2314/1231, 5-6=0/47
BOT CHORD 2-8=-1007/1984, 7-8=-1021/2013, 5-7=-1005/1984
WEBS 3-8=-399/792, 4-7=-399/792

JOINT STRESS INDEX
2 = 0.73, 3 = 0.83, 4 = 0.83, 5 = 0.73, 7 = 0.58 and 8 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 862 lb uplift at joint 2 and 862 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

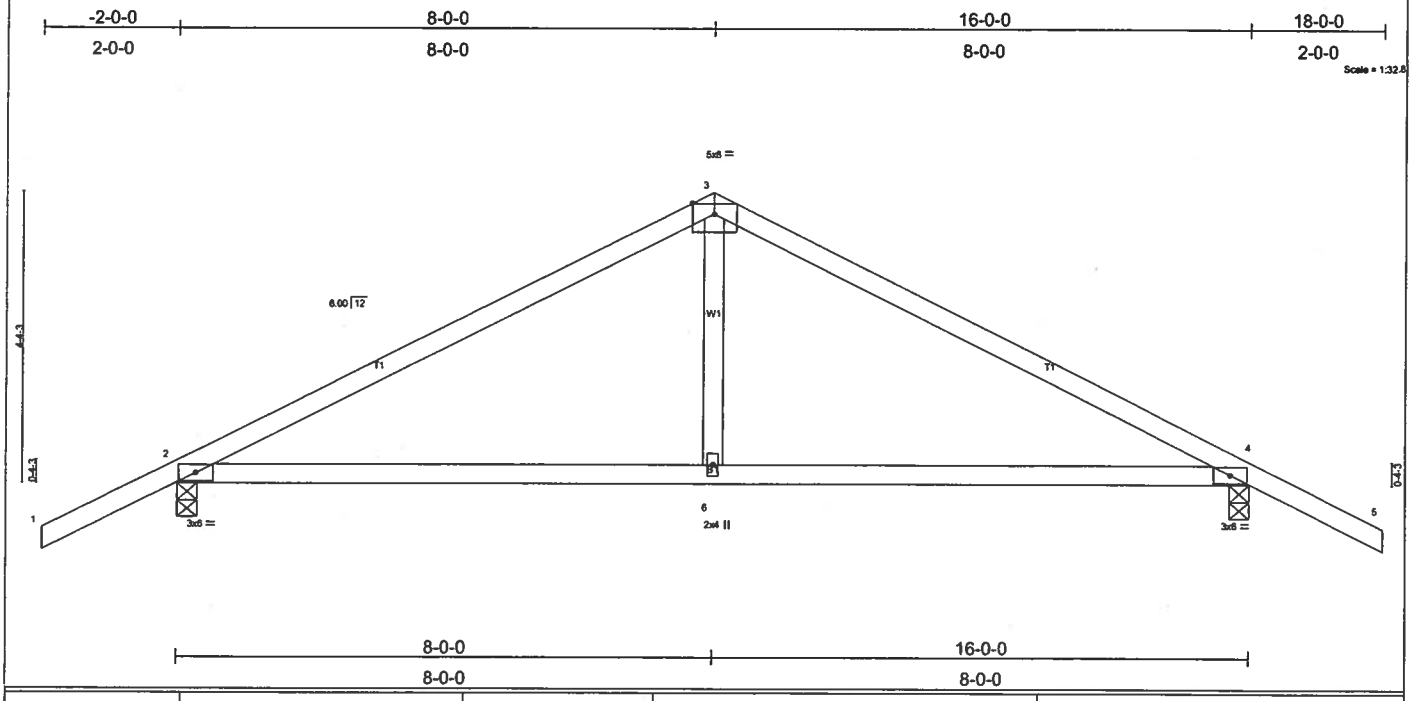
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061214

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T17	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:05 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.24	4-6	>779	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	4-6	>925	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-0 oc bracing.

REACTIONS (lb/size) 2=776/0-3-8, 4=776/0-3-8
Max Horz 2=94(load case 5)
Max Uplift2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-930/981, 3-4=-930/981, 4-5=0/47
BOT CHORD 2-6=-689/752, 4-6=-689/752
WEBS 3-6=-489/295

JOINT STRESS INDEX
2 = 0.62, 3 = 1.00, 4 = 0.62 and 6 = 0.22

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

LOAD CASE(S) Standard

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0403061215

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS-SIERRA
L157750	T18	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Apr 03 10:54:06 2006 Page 1

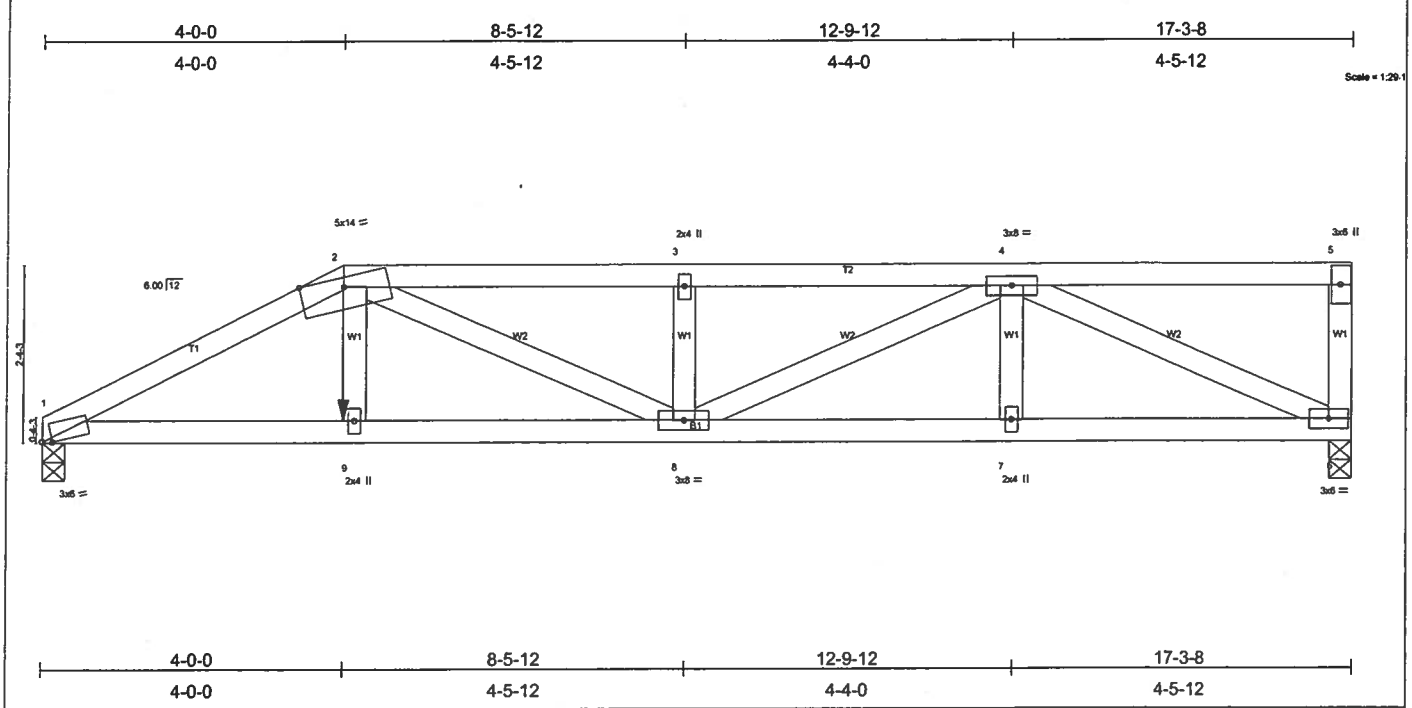


Plate Offsets (X,Y): [1:0-1-9,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.11	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.16	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.72	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-14 oc bracing.

REACTIONS (lb/size) 1=998/0-3-8, 6=1029/0-3-8
Max Horz 1=95(load case 4)
Max Uplift1=-565(load case 3), 6=-641(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1884/1089, 2-3=-2237/1382, 3-4=-2236/1382, 4-5=-99/62, 5-6=-169/127
BOT CHORD 1-9=-993/1640, 8-9=-1004/1662, 7-8=-1052/1693, 6-7=-1052/1693
WEBS 2-9=-160/306, 2-8=-447/632, 3-8=-334/263, 4-8=-365/601, 4-7=-67/171, 4-6=-1764/1096

JOINT STRESS INDEX
1 = 0.79, 2 = 0.48, 3 = 0.34, 4 = 0.82, 5 = 0.35, 6 = 0.53, 7 = 0.34, 8 = 0.59 and 9 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 565 lb uplift at joint 1 and 641 lb uplift at joint 6.
5) Girder carries hip end with 0-0-0 right side setback, 4-0-0 left side setback, and 4-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 140 lb down and 72 lb up at 4-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-5=-77(F=-23), 1-9=-30, 6-9=-43(F=-13)
Concentrated Loads (lb)
Vert: 9=-140(F)

APRIL 03, 2006 TRUSS DESIGN ENGINEER:
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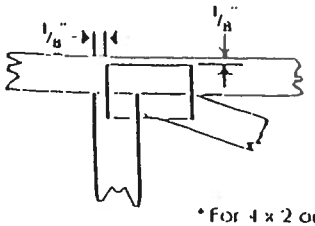
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

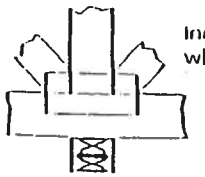
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

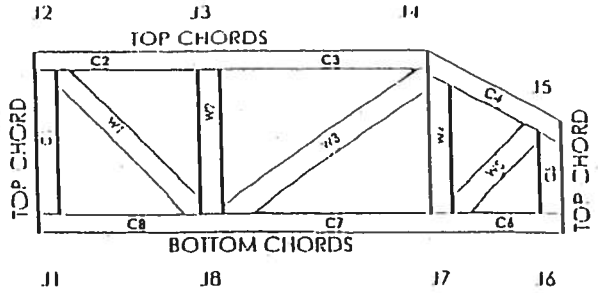
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transfering connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results	
Title	**Test Specimen #1**	**Test Specimen #2**
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft^2	0.09 cfm/ft^2
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03
Test Dates: 10/07/03
Through: 10/08/03
And: 12/01/03
And: 12/15/03
And: 03/17/04
Report Date: 04/16/04
Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

Description	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.75"	See Note #2
	30.0 psf (negative)	0.71"	See Note #2

***Note #2**: The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.13"	0.26" max.
	45.0 psf (negative)	<0.01"	0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
Test Specimen #2: HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.62"	See Note #2
	30.0 psf (negative)	0.51"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.03"	0.21" max.
	45.0 psf (negative)	0.04"	0.21" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC

Digitally Signed by: Eric Westphal

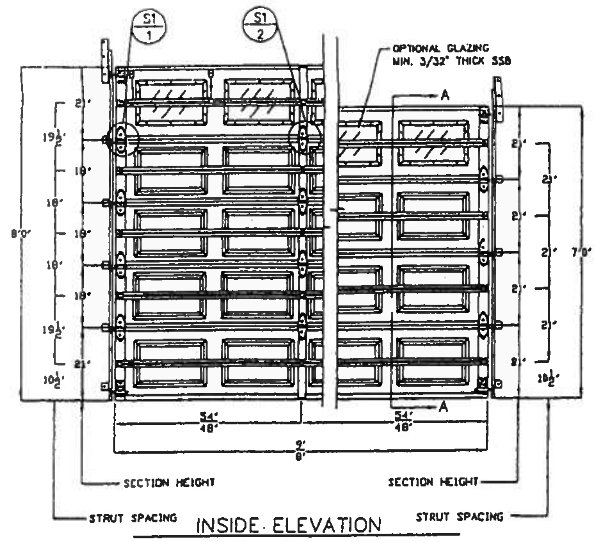
Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

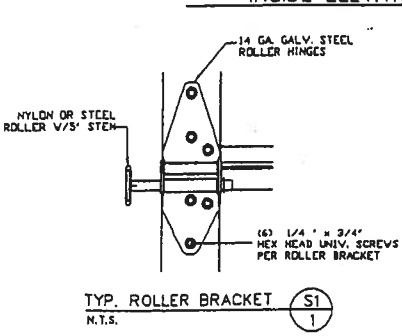
Steven M. Urich, P. E.
Senior Project Engineer

EW:dme
01-47320.03

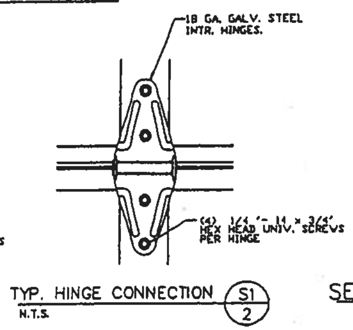
APRIL 20, 2004



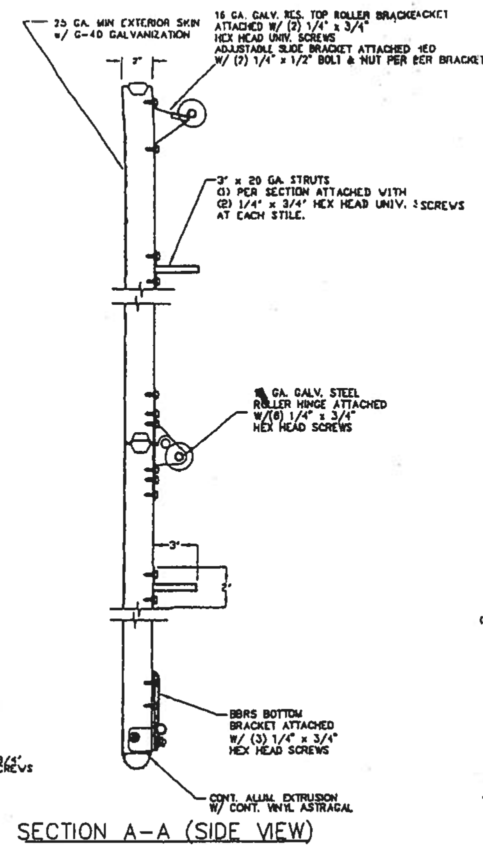
INSIDE ELEVATION



TYP. ROLLER BRACKET (S1 1)
N.T.S.



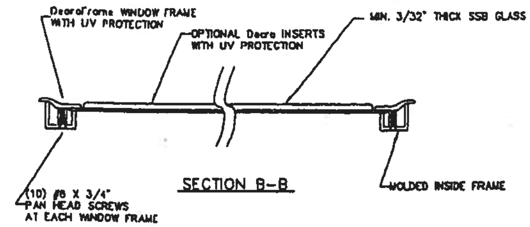
TYP. HINGE CONNECTION (S1 2)
N.T.S.



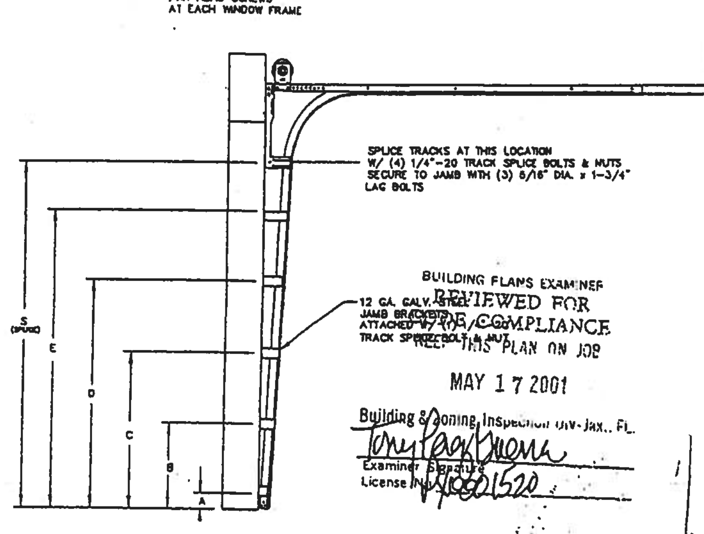
SECTION A-A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-011 ON OCTOBER 12, 1995 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.4 PSF AND -54.7 PSF. BY COMPARISON, FOUR (4) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 9' X 7' AND 8' X 8' MODEL 600 AND 850 DOORS.



SECTION B-B



BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div.-Jax., FL.
Examiner Signature
License No.

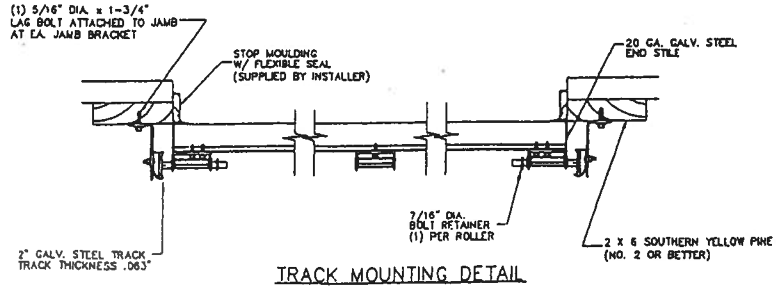
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.019") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY. G-40 GALVANIZATION.
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   - A. BASIC WIND SPEED OF 110 MPH
   - B. DOOR CAN BE INSTALLED WITH 6 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   - C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   - D. USE FACTOR OF 1.0
   - E. EXPOSURE RATING OF C



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

4/19/01

MAX SIZE 8' x 8'	
DESIGN LOADS +28.7 PSF -31.3 PSF	
TEST LOADS +43.0 PSF -46.0 PSF	

Amarr
MODEL #600 STRATFORD
MODEL #950 HERITAGE III
SIZE 8
DRAWING NUMBER SBC-580-007-J
SCALE: NOT TO SCALE
SHEET 1 OF 1

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358 outswing

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3^0-7^0 Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. **RD0087**, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00,prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

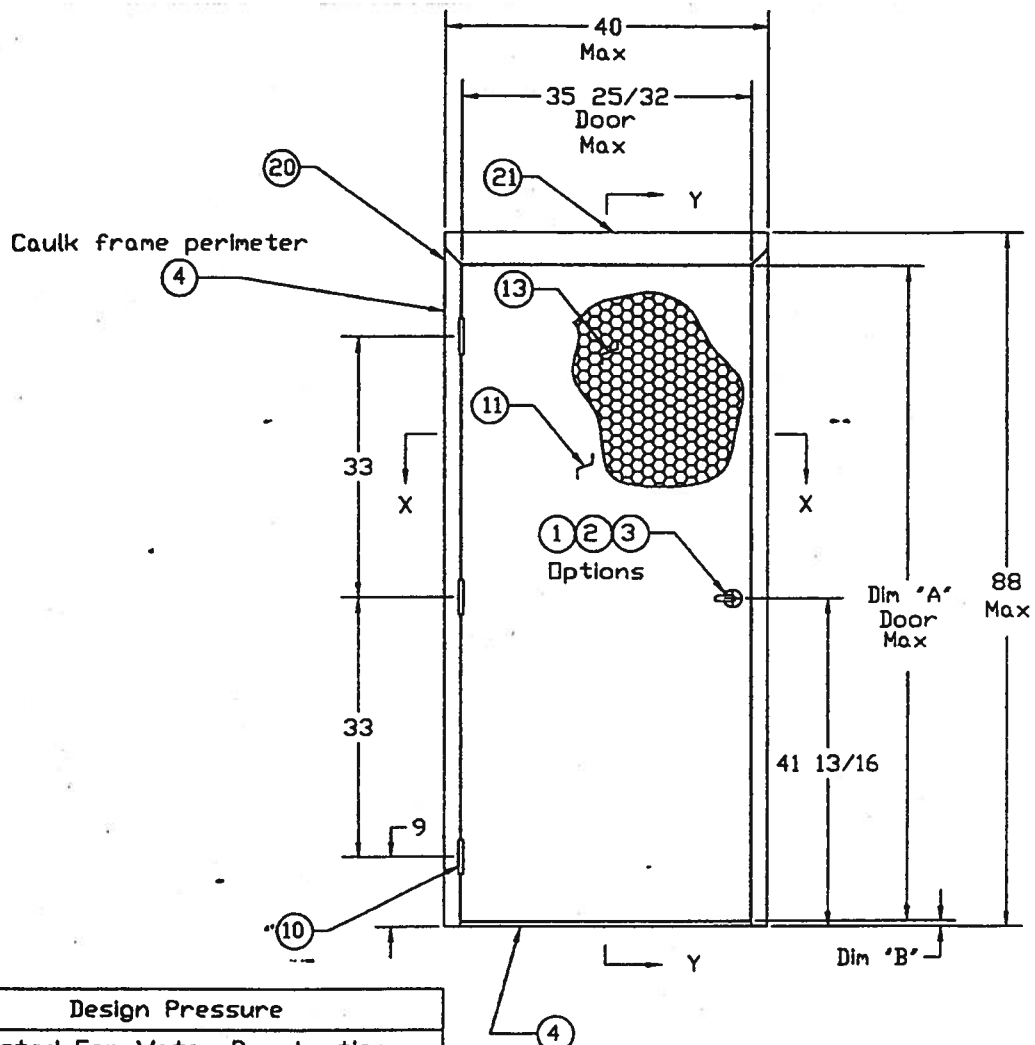
LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA **renews NOA # 00-0315.03** and consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P.E.**



Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim 'A'	Dim 'B'
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 18 2008
By Manuel Perez
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2009
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Deleted (2) Drawings Revised Sheet Numbers

MATERIAL SPECIFICATIONS:

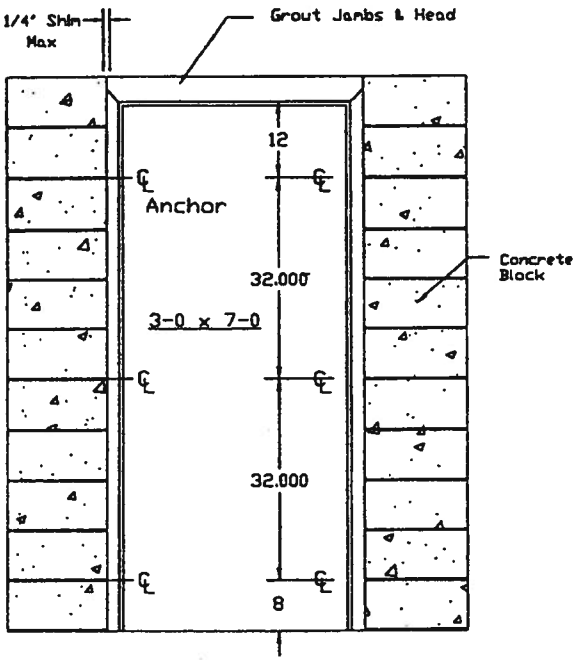
Finish: Rust Inhibitive Primer

3-0 x 7-0 Series Elevation Drawing

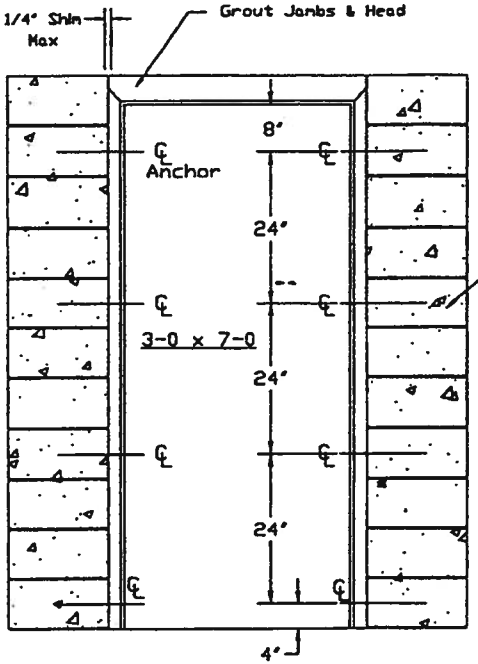
CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

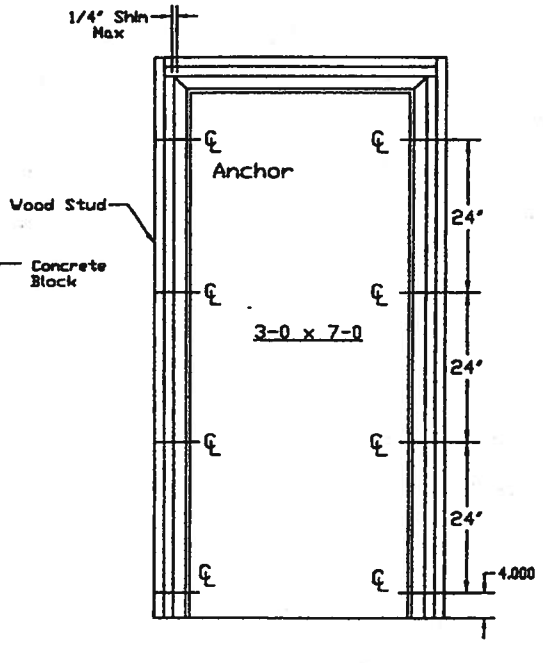
DRAWING NUMBER:
RD0087
Sheet 1 Of 7



Masonry "T" Anchor

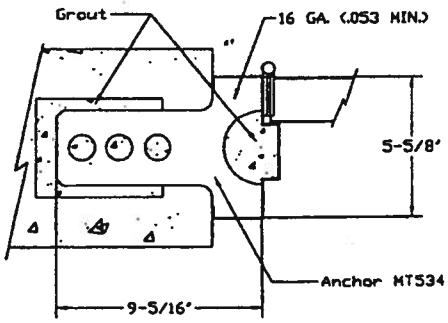


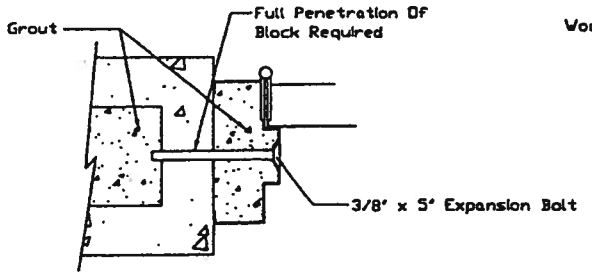
Existing Opening Anchor Into Block

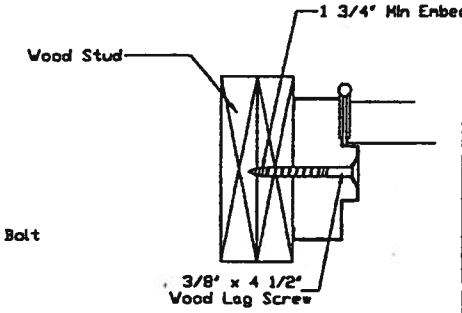


Existing Opening Anchor Into Wood Stud

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division







APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS

MATERIAL SPECIFICATIONS:

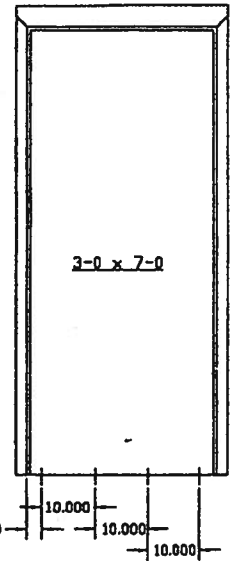
Frame Anchor Installation Details

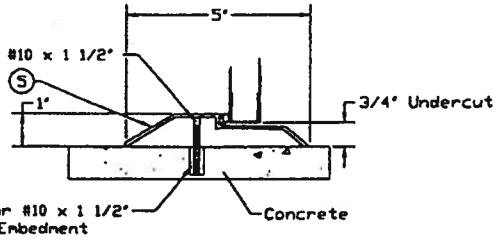
CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

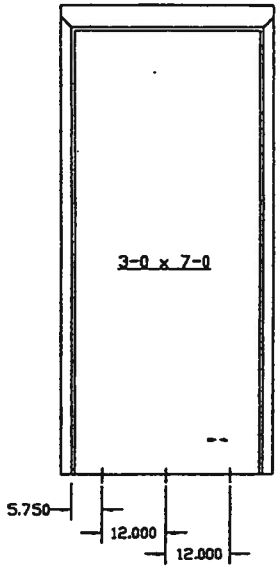
DRAWN BY: GWS | DATE: 5/30/97

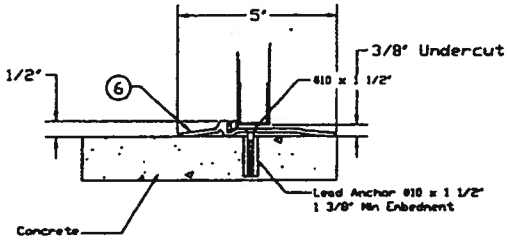
DRAWING NUMBER: RD0087



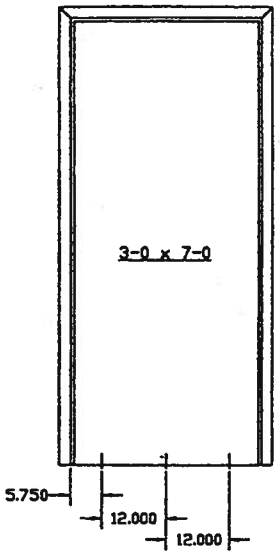


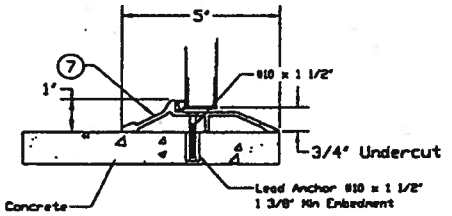
Threshold: National Guard 803S





Threshold: Penko 2005AV

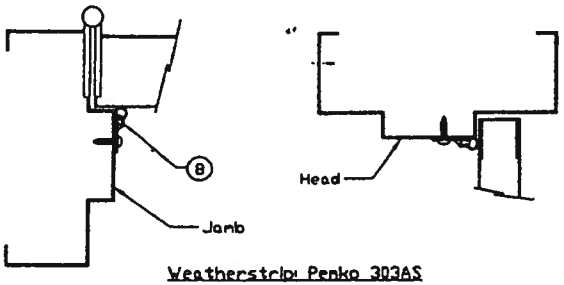




Threshold: Penko 181AV

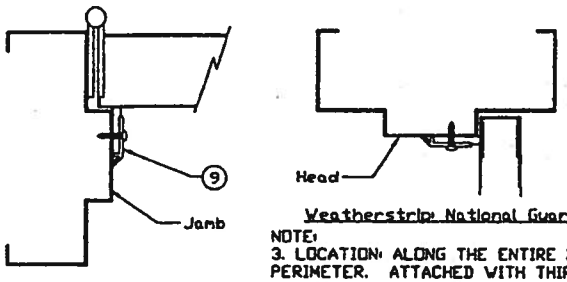
NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 16, 2008
By Manuel Perez
Miami-Dade Product Control Division



Weatherstrip: Penko 303AS

NOTE:
2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



Weatherstrip: National Guard 130NA

NOTE:
3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.

NOTE: 4. See Sheet 7 For Bill of Material

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: June 08/2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

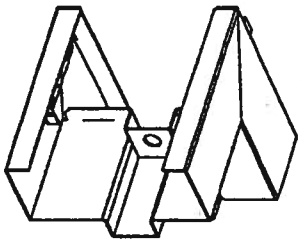
MATERIAL SPECIFICATIONS:

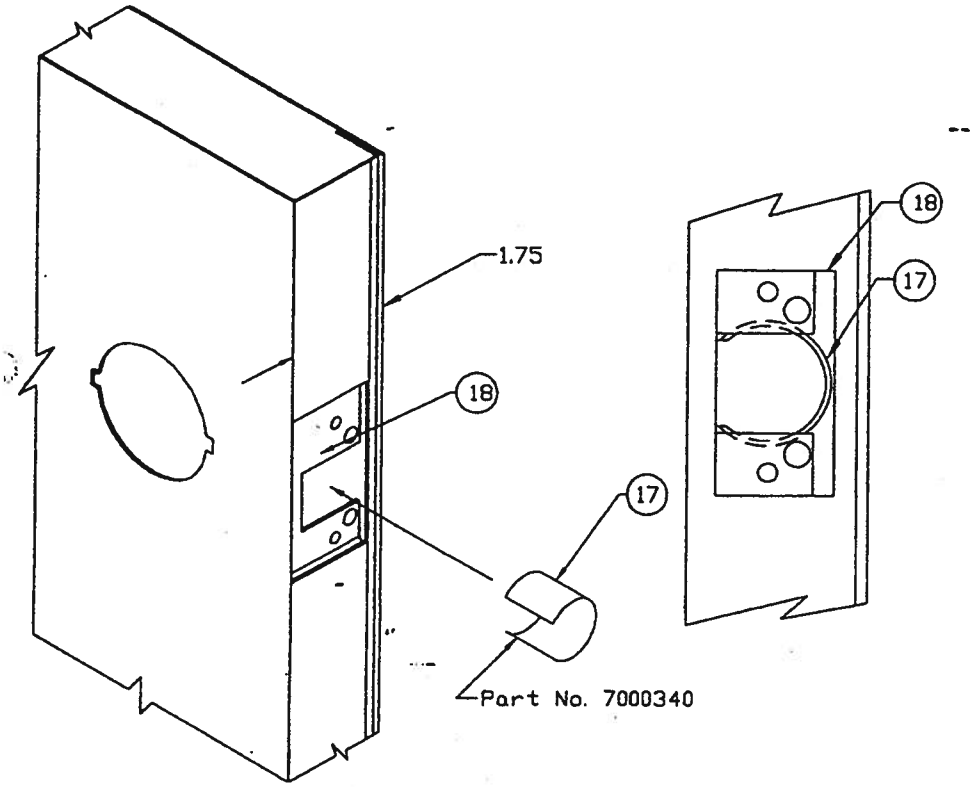
Threshold & Weatherstrip Installation details

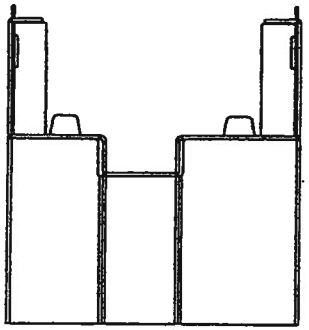
CECO DOOR PRODUCTS
Milan, Tennessee 38358

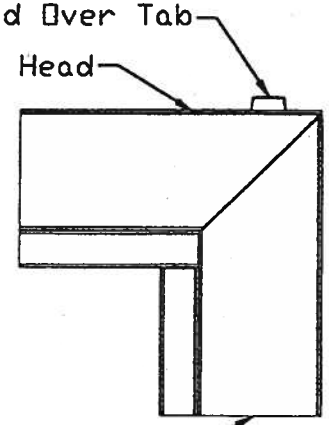
DRAWN BY: GWS
DATE: 5/30/97

RD0087
Sheet 3 Of 7









PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By: Manuel Perez
Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

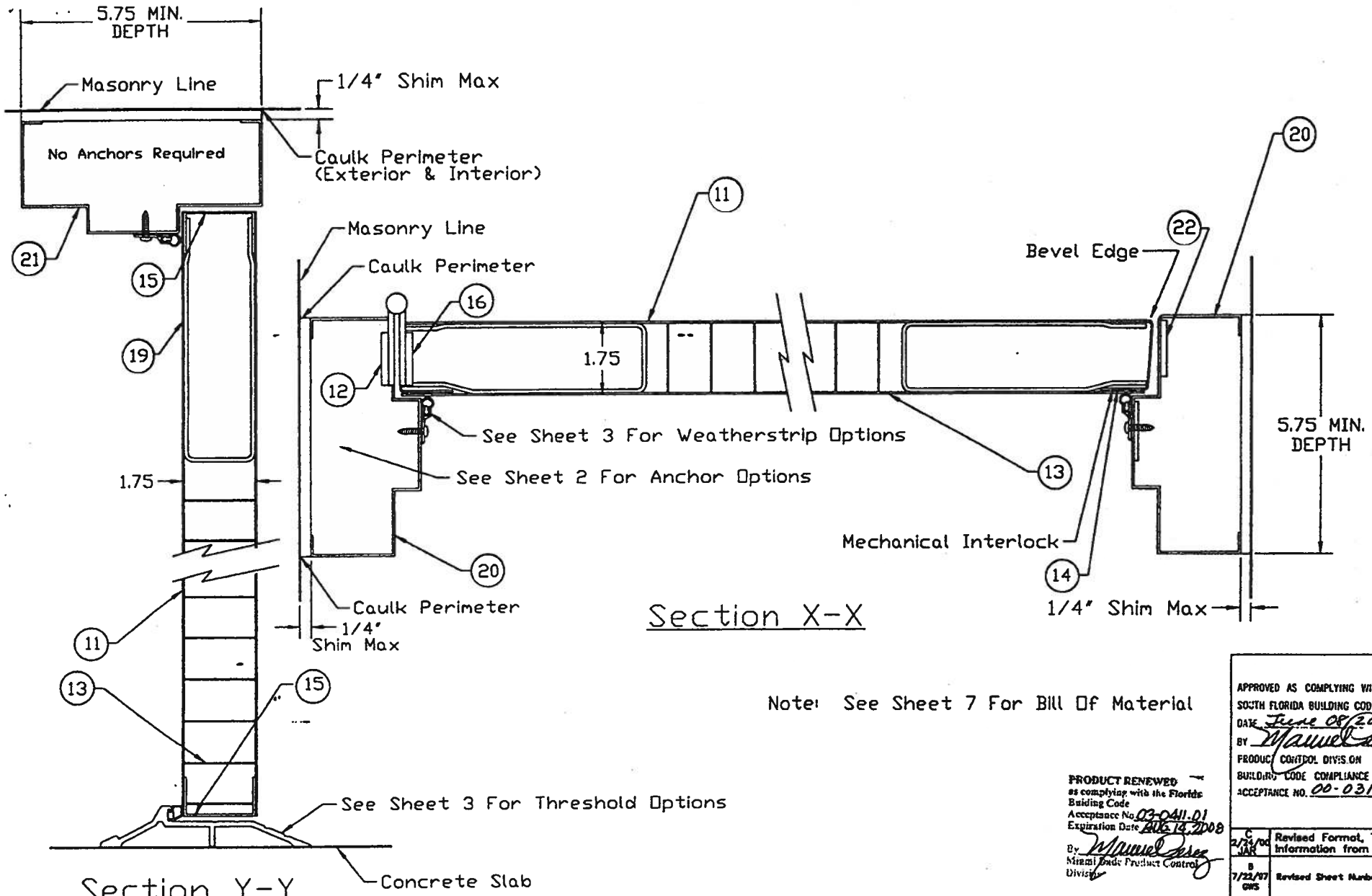
MATERIAL SPECIFICATIONS:

Cylindrical Lock Reinforcement and "SF" Seriers Frame Corner Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS | DATE: 6/06/97

DRAWING NUMBER:
RD0087
Sheet 4 of 7



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug 14, 2008
By Miami-Dade Product Control Division

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DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

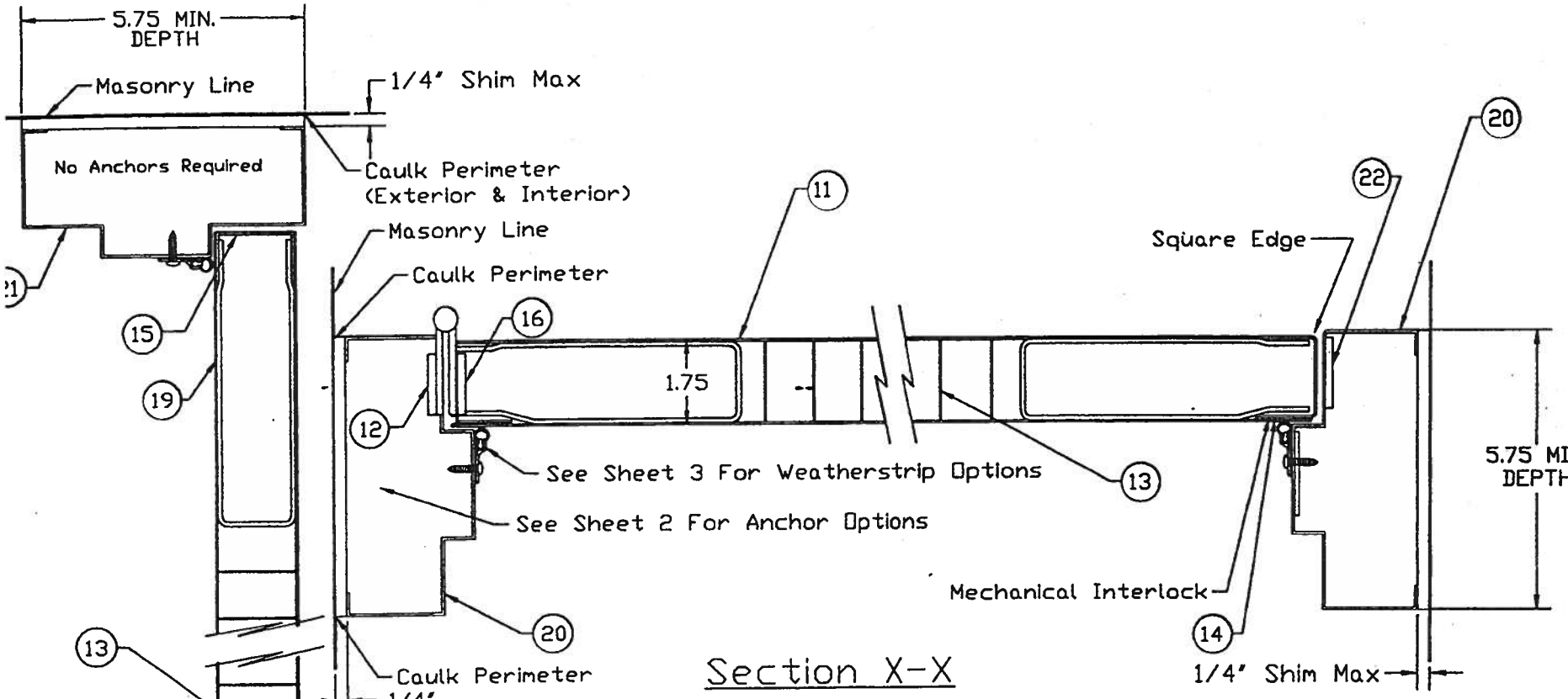
ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

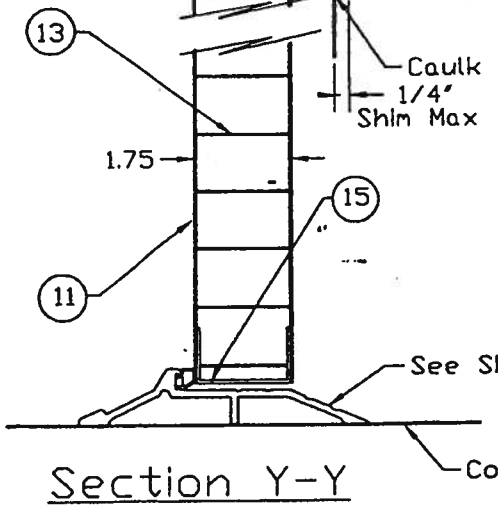
Cross Section View
Regent Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97
DRAWING NUMBER: RD0087
Sheet 5 of 7



Section X-X



Section Y-Y

Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: June 08, 2000
BY: Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Omega Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS DATE: 5/30/97

DRAWING NUMBER: RD0087

Sheet 6 of 7

ITEM	QTY	DESCRIPTION	MATERIAL	SIZE
1	1	SCHLAGE SERIES AL53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	1	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	1	YALE SERIES AU53070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	1	CAULK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	1	NATIONAL GUARD #803S		
6	1	PEMKO #2005AV		
7	1	PEMKO #181AV		
8	1 ROW	PEMKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TM) BULB INSERT		
9	1 ROW	NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3	HAGAR BB1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psD	18 GAUGE (.042" MIN. THICK)
12	3	HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA.
13	1	CORE: FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA. (.053" MIN)
16	3	DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA.
17	1	DOOR LATCH REINFORCEMENT, STEEL "C" RING	28 GA. GALV.	.015" THICK X 1.313 INSIDE DIAMETER
18	1	DOOR LOCK REINFORCEMENT	STEEL	16 GA.
19	1	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	12 GA. (.093")
20	2	SERIES "SF", FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psD	2" FACE, 5-3/4" DEPTH MIN.
21	1	SERIES "SF", FRAME HEAD, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psD	2" FACE, 5-3/4" DEPTH MIN.
22	1	JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 14, 2008
By: Manuel Perez
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY: Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered information from NOA
A 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series
Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS | DATE: 6/02/97

DRAWING NUMBER: RD0087

Sheet 7 of 7

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

In Swing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors –Impact

APPROVAL DOCUMENT: Drawing No RD0728, titled "3-0 x 7-0 , Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

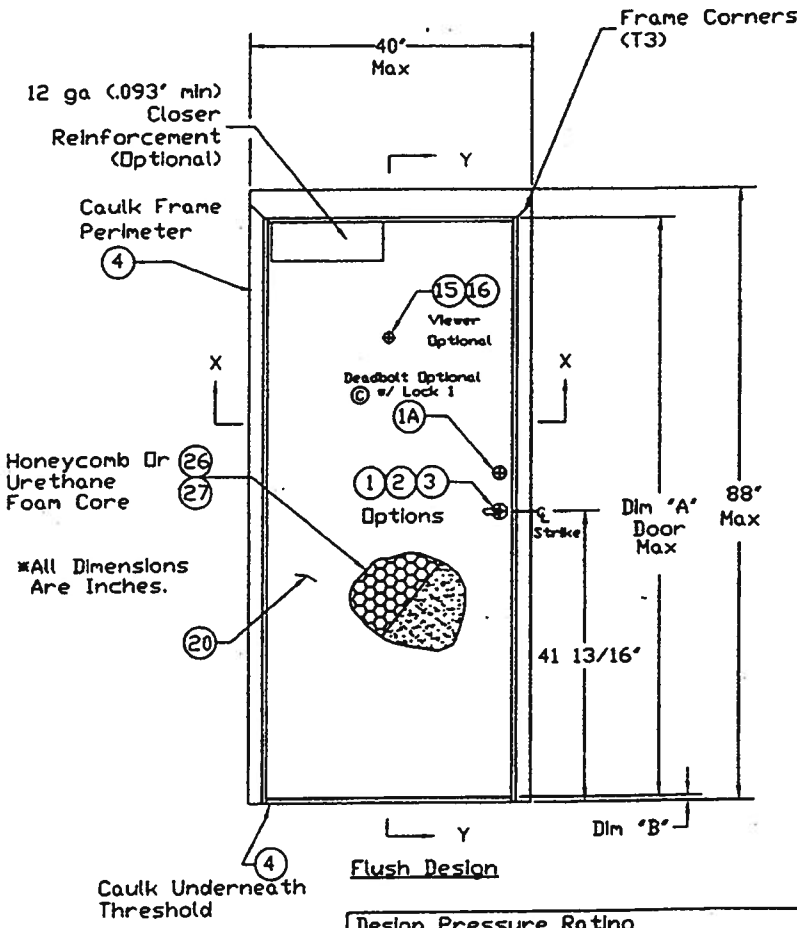
RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

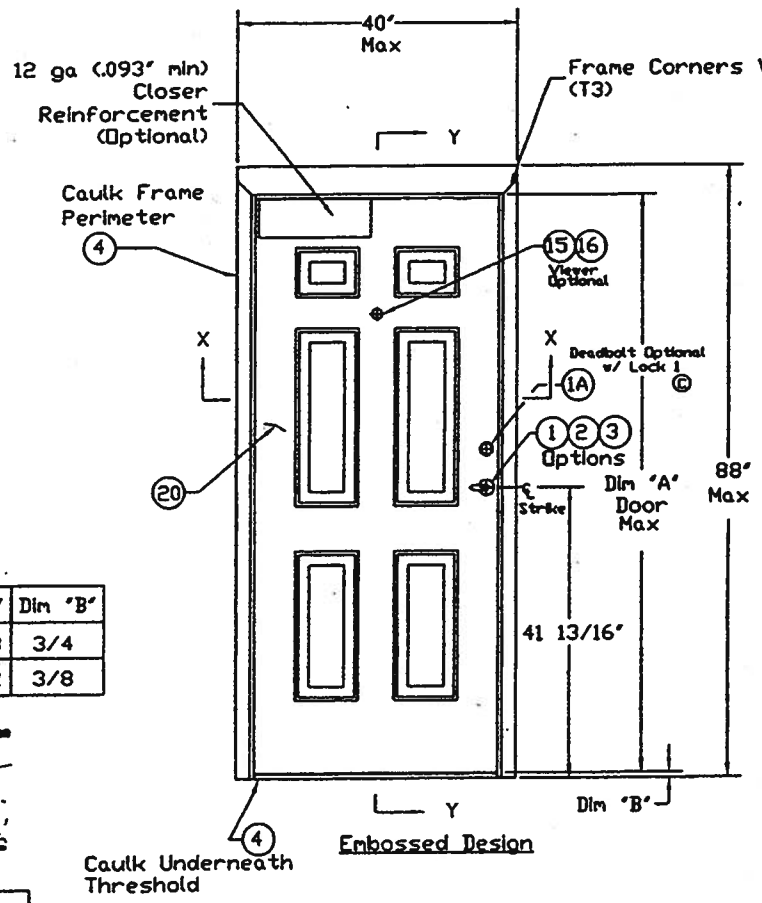
TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Ishaq I. Chanda, P.E.**

PRODUCT CONTROL ACCEPTED





In-Swing Door
(Exterior View)

	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Approved as complying with the Florida Building Code
Date OCTOBER 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

Design Pressure Rating		
	Where Water Infiltration Requirement Is Needed	Where Water Infiltration Requirement Is Not Needed
Positive	Not Approved	+70 PSF
Negative	Not Approved	-70 PSF

Notes:
1) In-swing Not Approved For Water Infiltration
2) This Door Does Not Need A Hurricane Protection Systen
3) Hinge Spacing Is 33" O.C., 13" From Top Of Frame & 9" From The Botton.

MATERIAL SPECIFICATIONS:
Finish: Rust Inhibitive Primer

3-0 x 7-0 Series
Regent, Omega, Imperial, & Versadoor
In-Swing Elevation Drawing

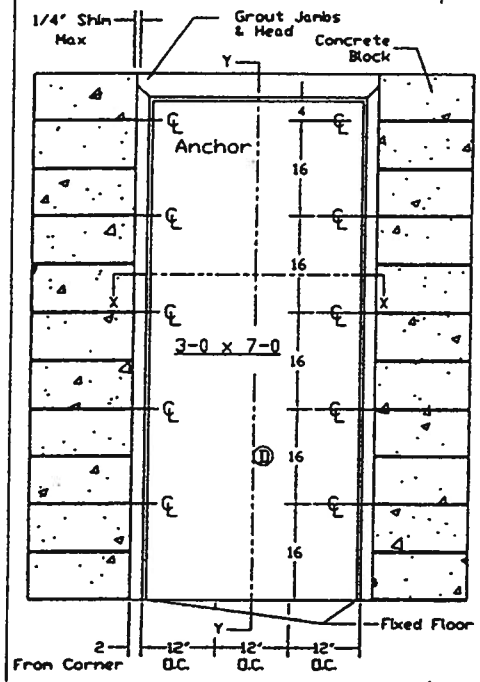
CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A 9/20/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

DRAWN BY: LT
DATE: 5/22/02
DRAWING NUMBER: RD0728
Sheet 1 of 9

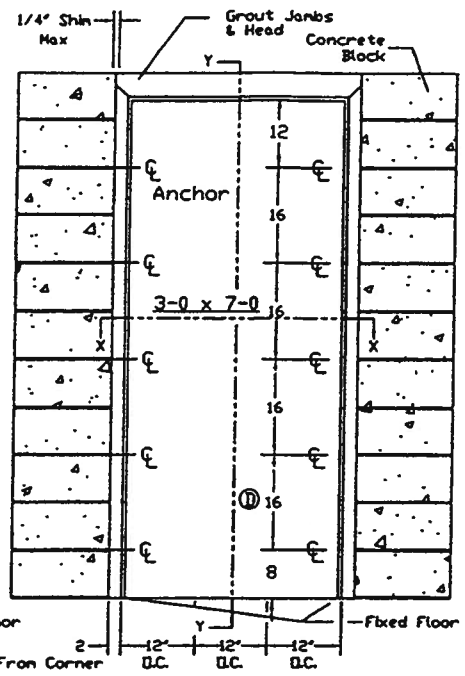
Masonry "T" Anchor

Min. 3500 PSI



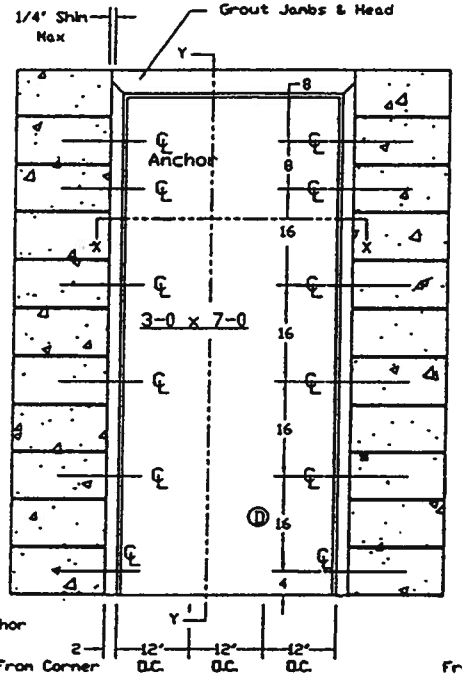
Masonry Wire Anchor

Min. 3500 PSI

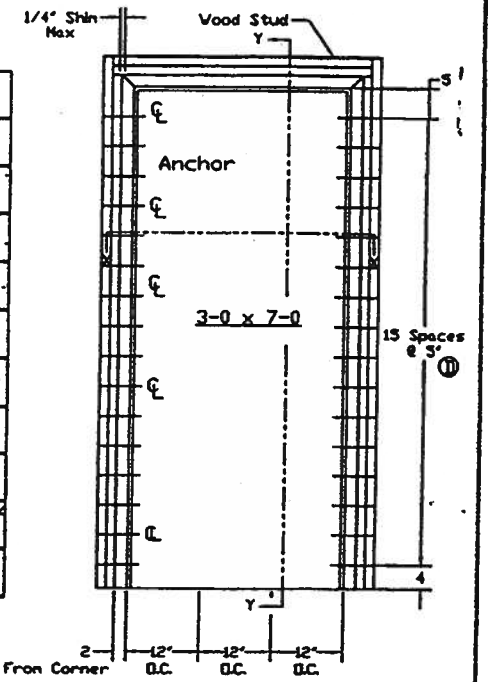


Existing Opening W/Lockbolt or Sleeve Anchor Into Block

Min. 3500 PSI

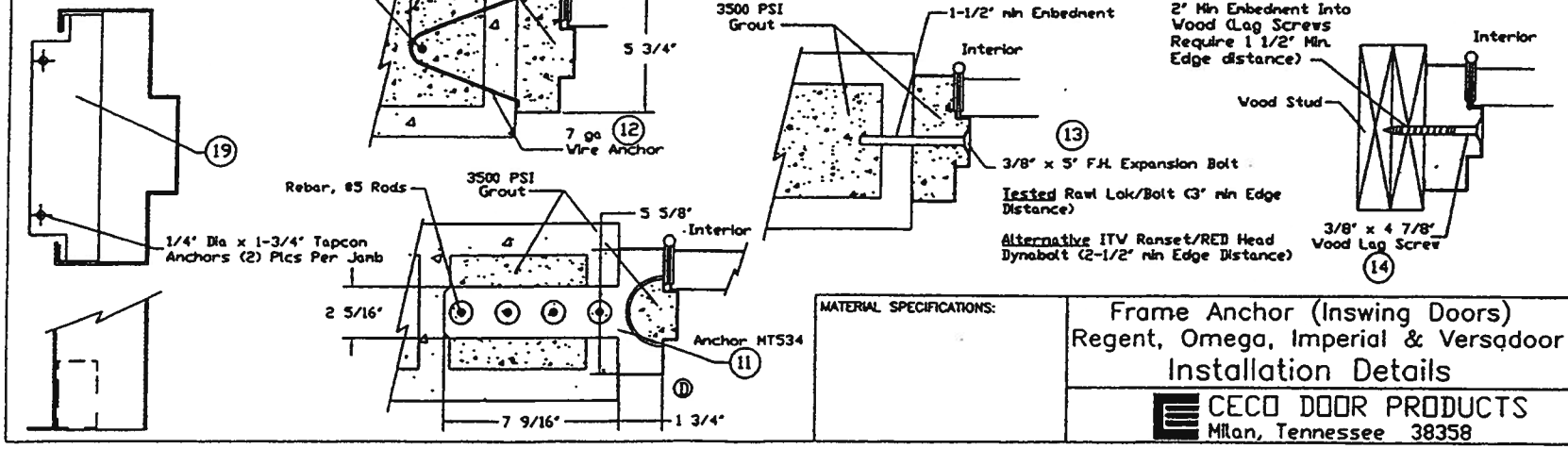


Existing Opening Anchor Into Wood Stud



*All Dimensions Are Inches

Installation Details Hinge Jambs / Lock Jambs



Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

ISSUE	REVISIONS
A 6/28/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

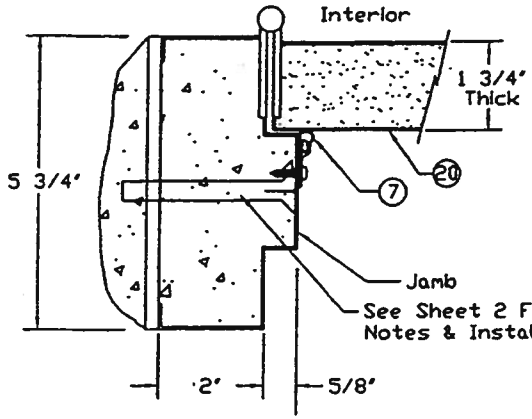
Frame Anchor (Inswing Doors)
Regent, Omega, Imperial & Versadoor
Installation Details

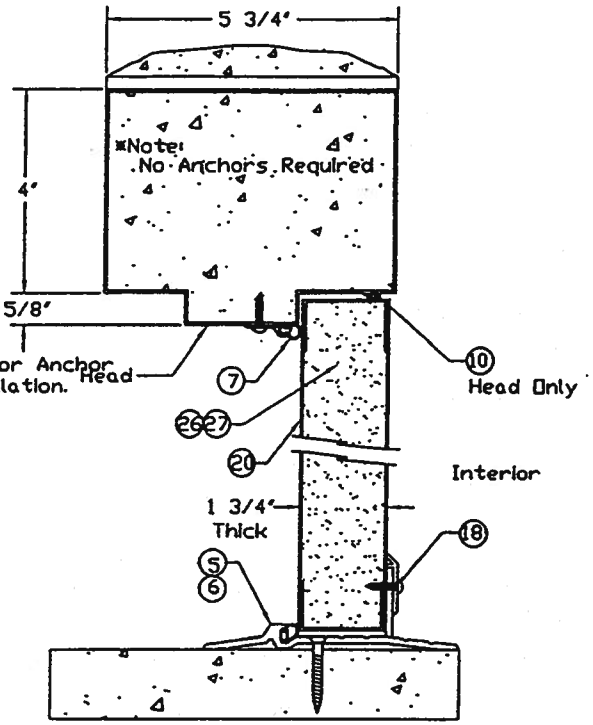
CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: LT
DATE: 5/22/02

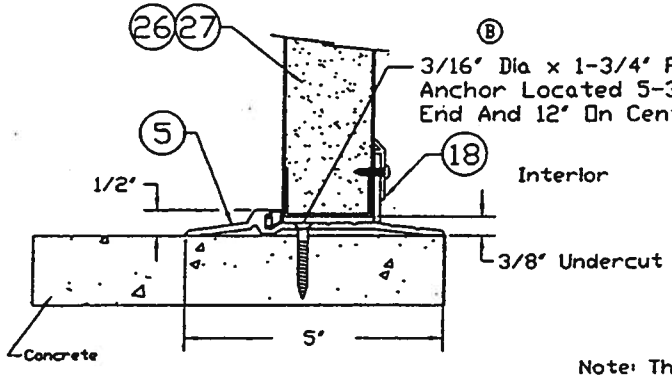
DRAWING NUMBER: RD0728

*Note: Structural Member At Header Must Be Designed To carry 58.3#/ft. load Imposed And Must Be Reviewed By Building Official.





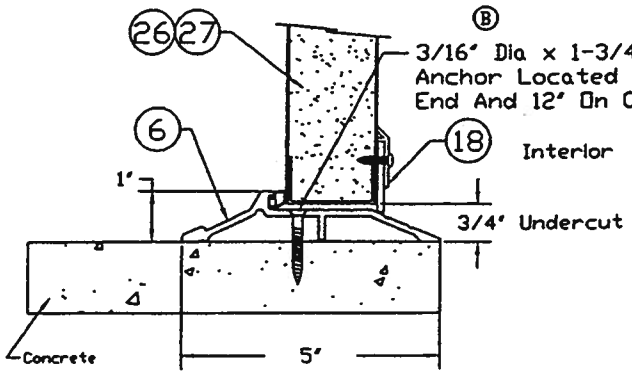
Section Y-Y



Threshold: Pemko 2005AV

Inswing
(Not Approved For Water)

Note: Thresholds Not Approved For Water.



Threshold: Pemko 181AV

Approved as complying with the Florida Building Code
Date: OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By: Ishaq I. Chanda

D	5/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda
C	5/23/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda

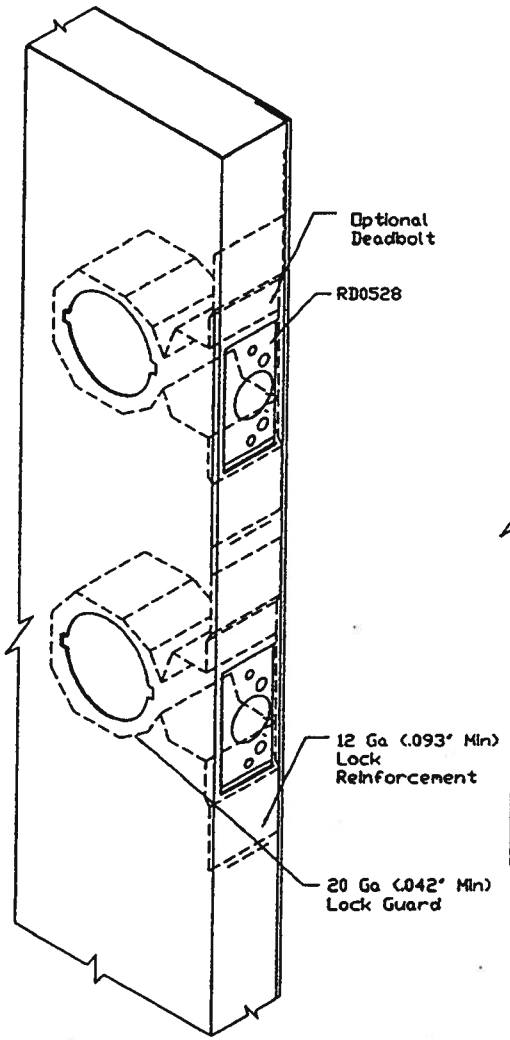
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/22/02

MATERIAL SPECIFICATIONS:

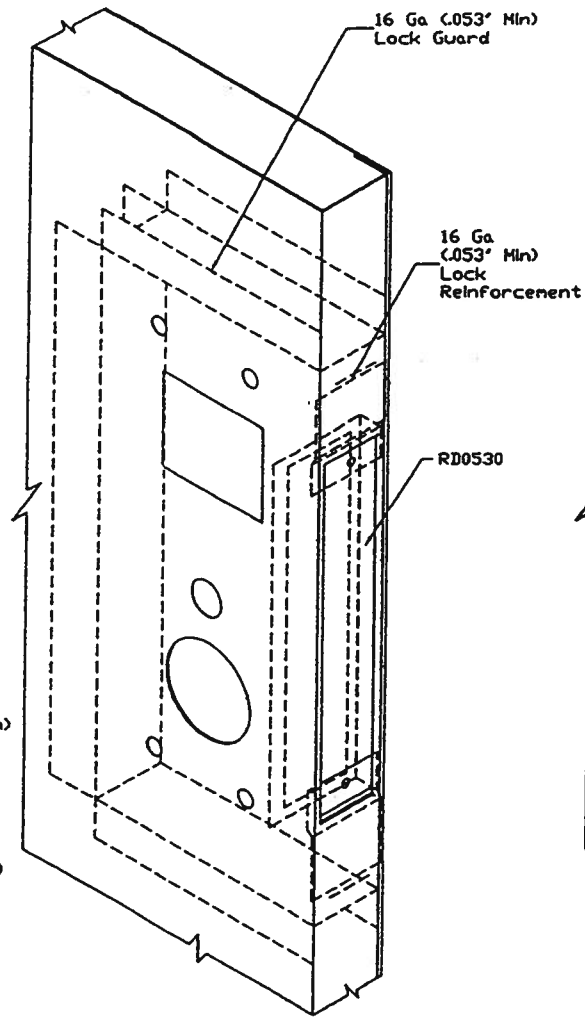
Threshold & Weatherstrip (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

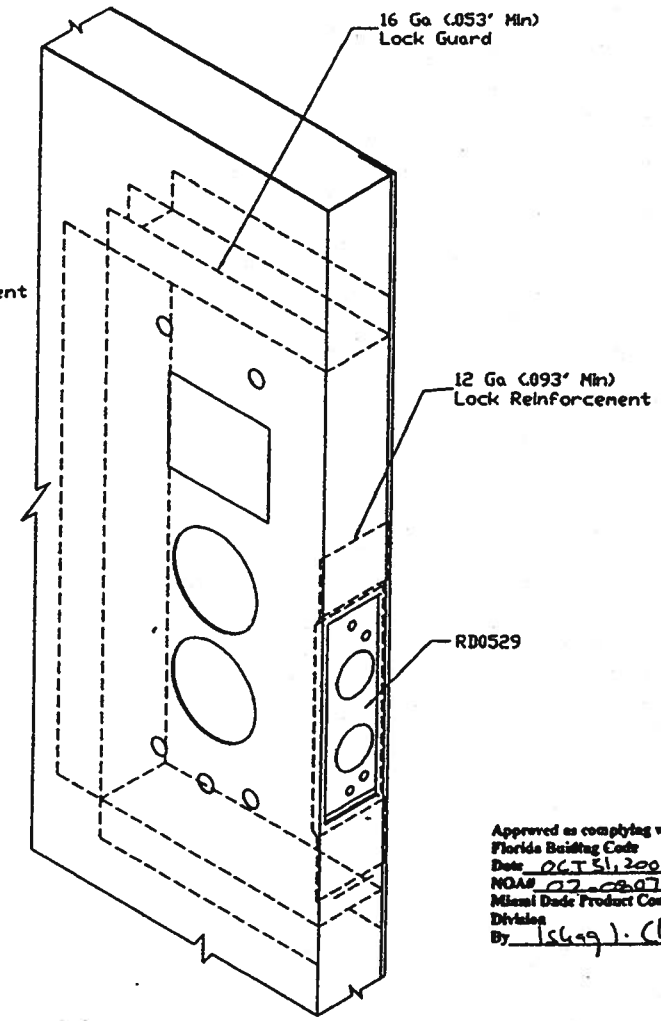
RD0728
Sheet 3 Of 9



Schlage AL53PD



Saflok MT



Saflok Premier SL2500

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By [signature]

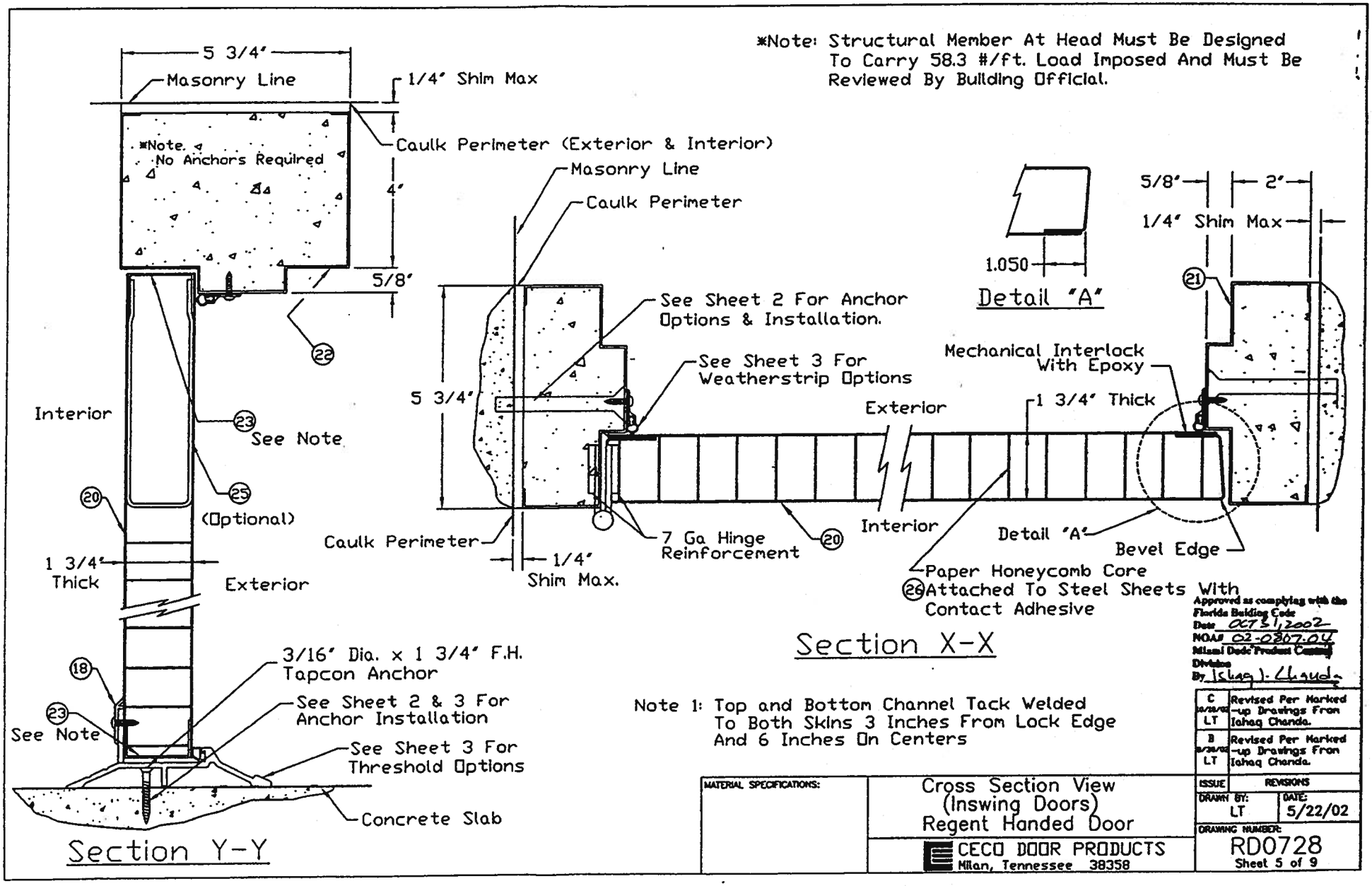
A 5/28/02 LT	Added RD0528, RD0529 & RD0530.
ISSUE	REVISIONS

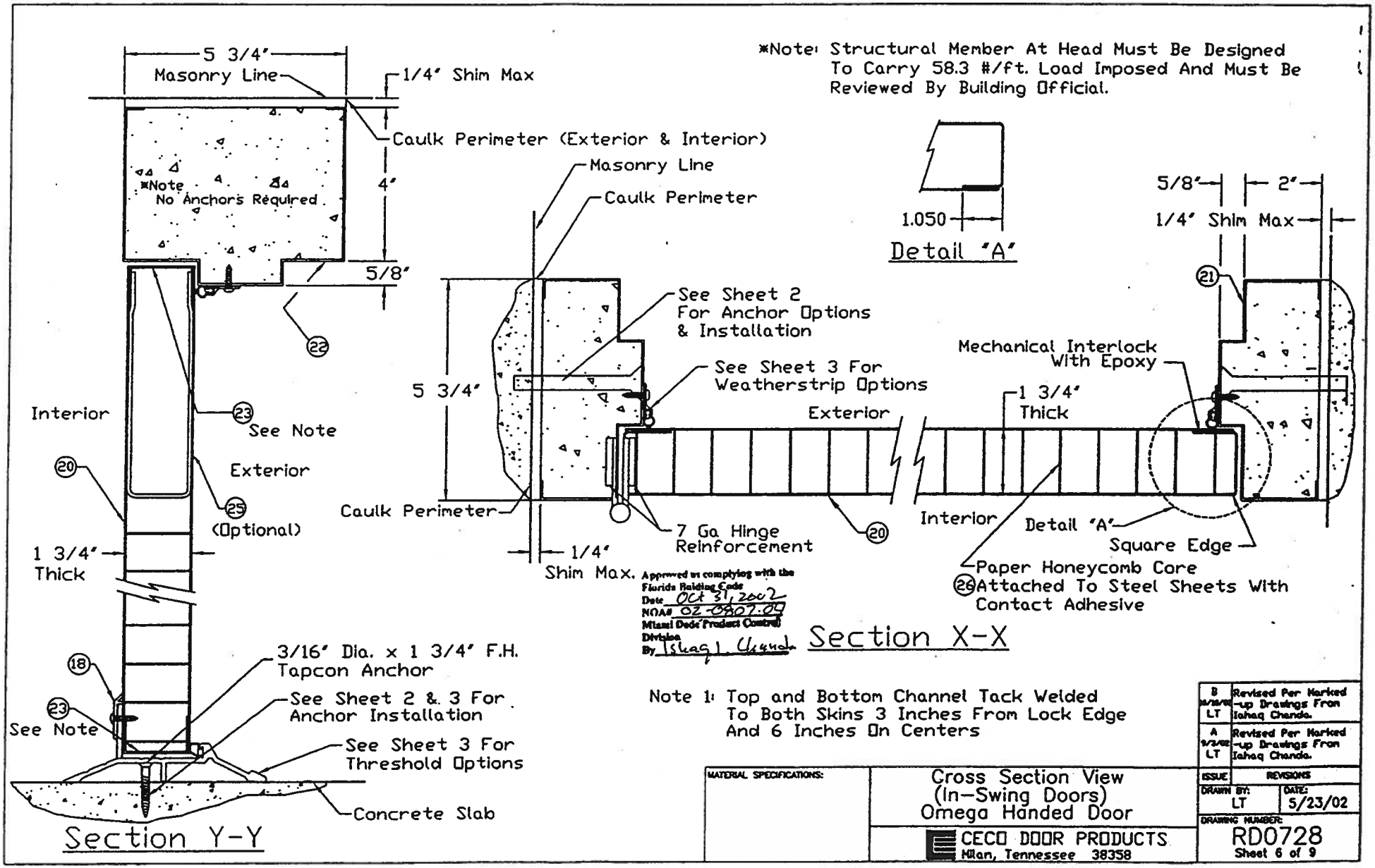
MATERIAL SPECIFICATIONS:

Lock Reinforcement (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Reinforcement Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	
Sheet 4 of 9	


*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.
5 3/4"
Masonry Line
1/4" Shim Max
Caulk Perimeter (Exterior & Interior)
*Note. No Anchors Required
4"
5/8"
22
Interior
23
See Note
25
(Optional)
20
1 3/4" Thick
Exterior
18
3/16" Dia. x 1 3/4" F.H. Tapcon Anchor
See Sheet 2 & 3 For Anchor Installation
See Sheet 3 For Threshold Options
Concrete Slab
Section Y-Y
Masonry Line
Caulk Perimeter
See Sheet 2 For Anchor Options & Installation.
See Sheet 3 For Weatherstrip Options
5 3/4"
Caulk Perimeter
1/4" Shim Max.
7 Ga Hinge Reinforcement
20
Interior
Exterior
1 3/4" Thick
1.050
Detail "A"
5/8"
2"
1/4" Shim Max
21
Mechanical Interlock With Epoxy
Detail 'A'
Bevel Edge
Paper Honeycomb Core
26
Attached To Steel Sheets With Contact Adhesive
Section X-X
Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers
Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0307.04
Miami Dade Product Control Division
By Ishaq Chanda
C 10/28/02 LT Revised Per Marked -up Drawings From Ishaq Chanda.
B 9/26/02 LT Revised Per Marked -up Drawings From Ishaq Chanda.
ISSUE REVISIONS
DRAWN BY: LT
DATE: 5/22/02
MATERIAL SPECIFICATIONS:
Cross Section View (Inswing Doors) Regent Handed Door
CECO DOOR PRODUCTS
Milan, Tennessee 38358
DRAWING NUMBER: RD0728
Sheet 5 of 9



Approved as complying with the Florida Building Code
Date: Oct 31, 2002
NOA# 02-0807.09
Miami Dade Product Control Division
By: Ishaq I. Chanda

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

B 9/25/02 LT	Revised Per Marked-up Drawings From Ishaq Chanda.
A 9/3/02 LT	Revised Per Marked-up Drawings From Ishaq Chanda.
ISSUE	REVISIONS

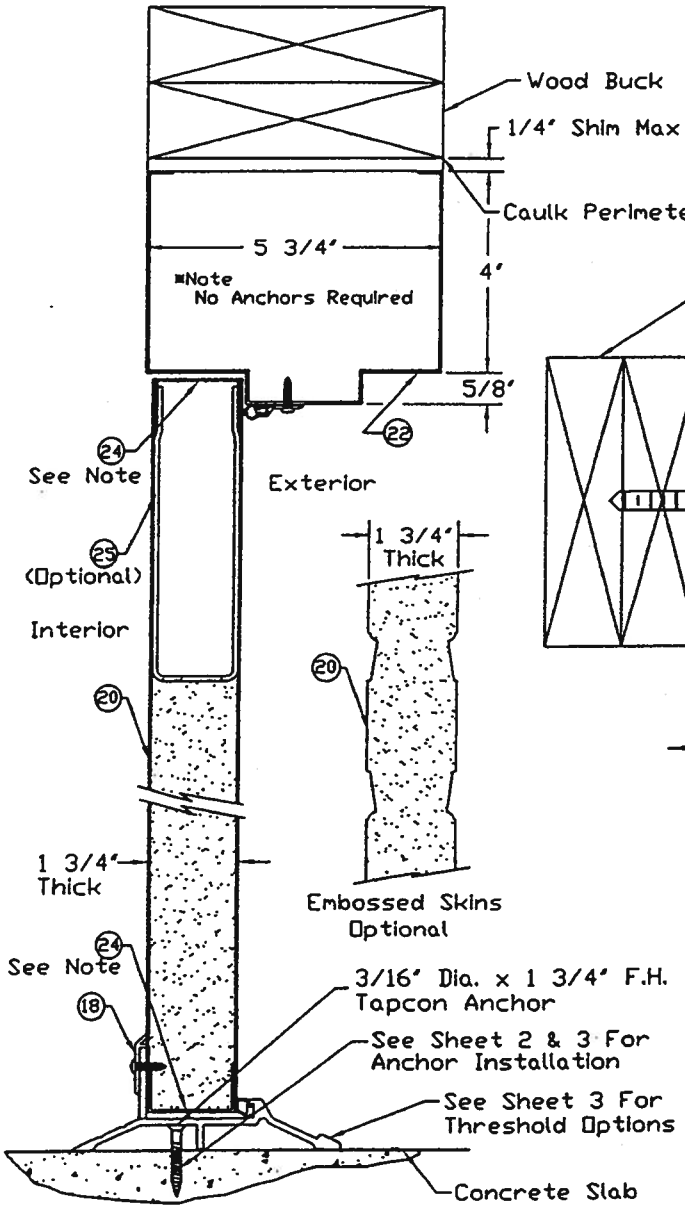
MATERIAL SPECIFICATIONS:

Cross Section View
(In-Swing Doors)
Omega Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

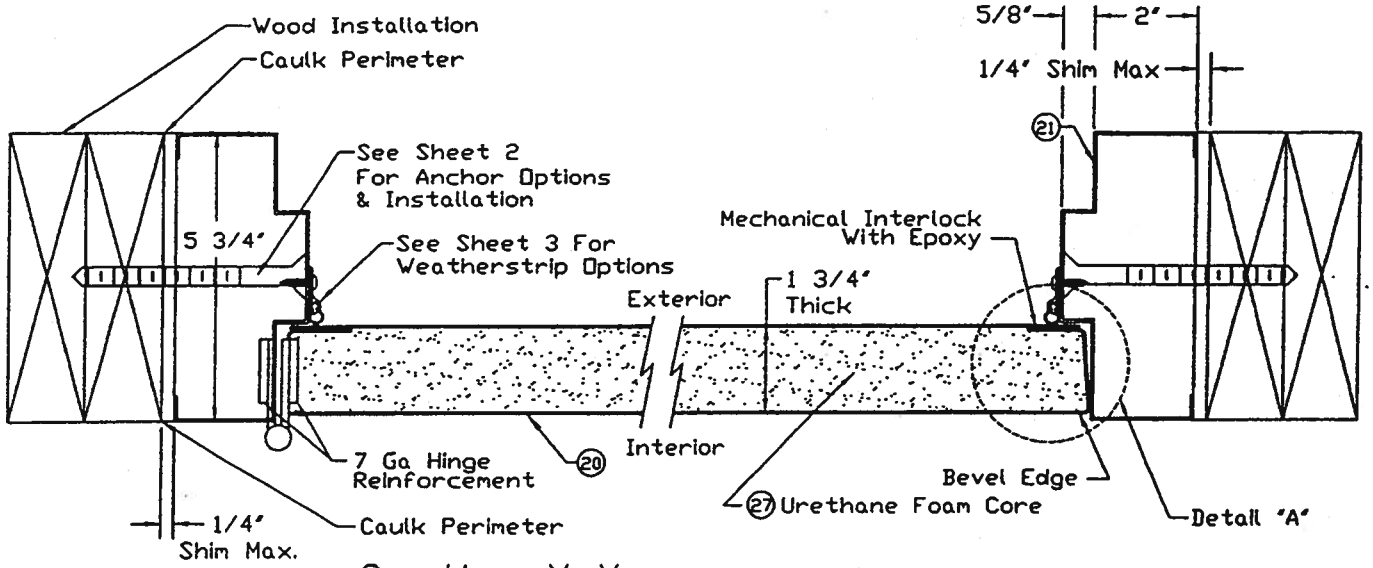
DRAWN BY: LT
DATE: 5/23/02

DRAWING NUMBER: RD0728
Sheet 6 of 9



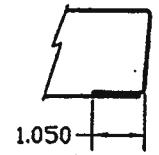
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Detail "A"

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02.0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

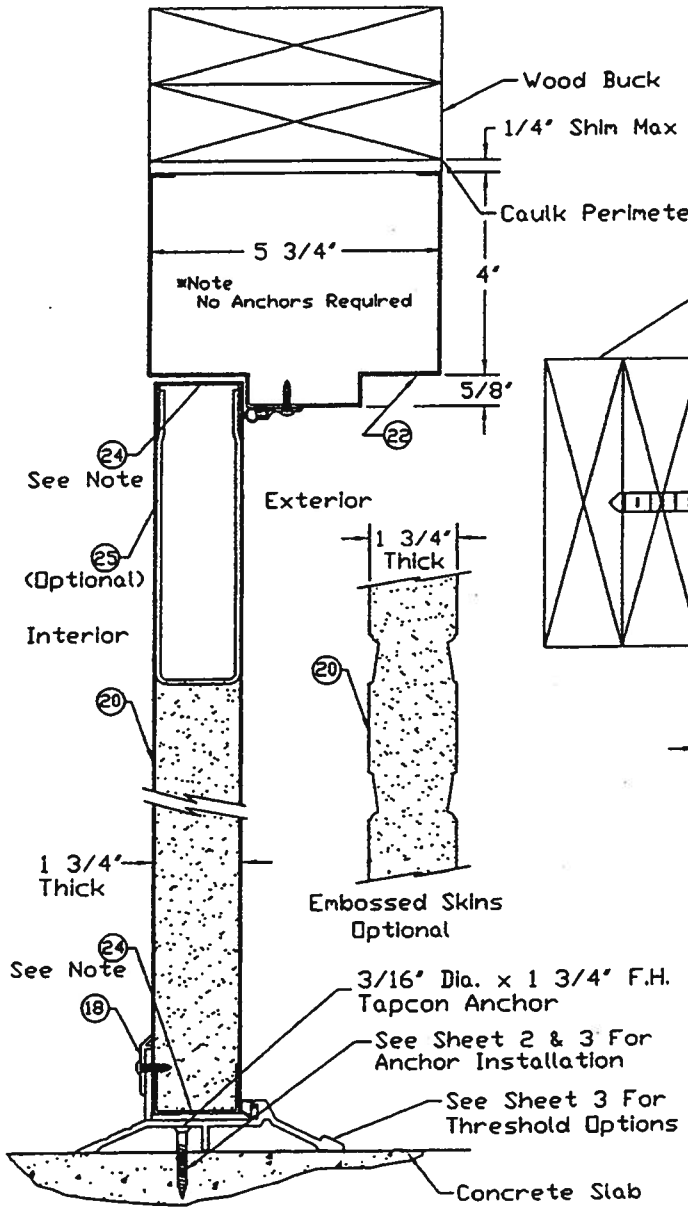
B w/ver LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
ISSUE	REVISIONS

DRAWN BY: LT
DATE: 5/23/02
DRAWING NUMBER: RD0728
Sheet 7 of 9

MATERIAL SPECIFICATIONS:

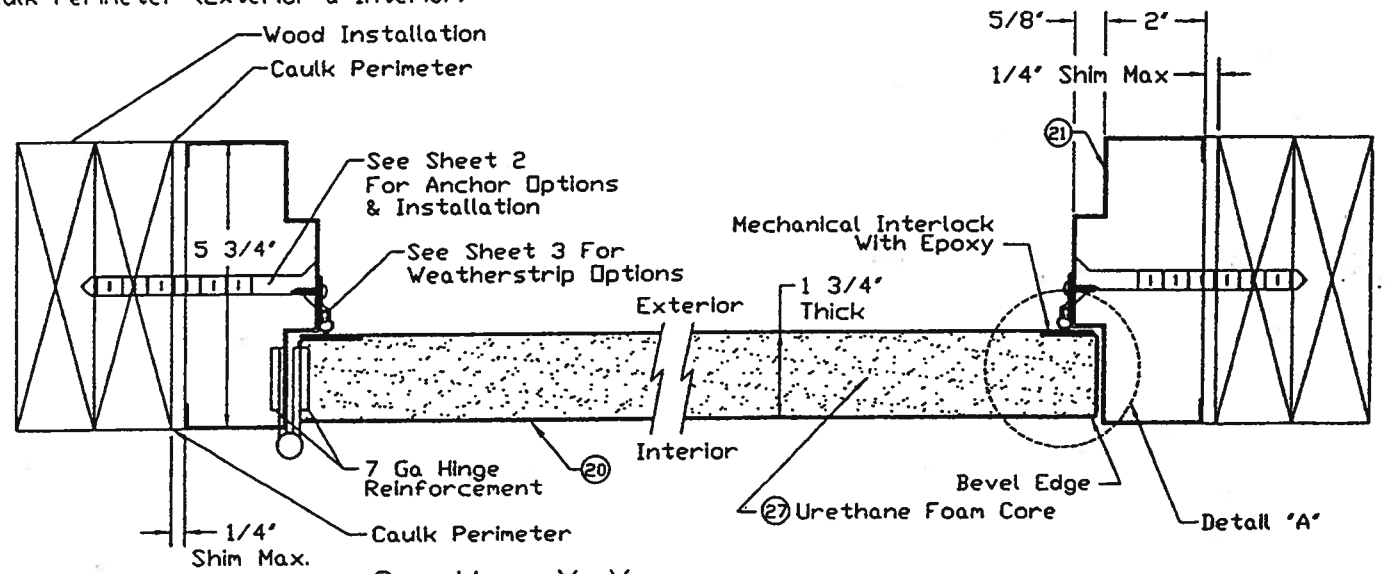
Cross Section View
(In-Swing Doors)
Imperial Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358



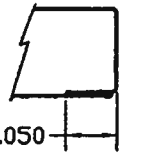
Section Y-Y

Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section X-X

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.



Detail "A"

Approved as complying with the Florida Building Code
Date OCT 21, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

ISSUE	REVISIONS
B 5/23/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

DRAWN BY: LT
DATE: 5/23/02

DRAWING NUMBER: RD0728
Sheet 8 of 9

MATERIAL SPECIFICATIONS:

Cross Section View
(In-Swing Doors)
Versadoor Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

1	Cylindrical Lock & Lock Reinforcement (RD0528)	Schlage	AL53PD
1A	Deadbolt (Optional) Ⓓ	Schlage	B100
2	Or Cylindrical Lock & Lock Reinforcement	Saflok	Premier SL2500
3	Or Mortise Lock	Saflok	MT
4	Caulk	Dow Corning	899 Silicone Glazing Sealant
5	Threshold	Pemko	2005AV36
6	Or	Pemko	181AV36
7	Weatherstrip	Pemko	303AV3684
8	Hinge (Ball Bearing)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
9	Or (Spring)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
10	Weatherstrip	Pemko	S88
11	Frame Anchor	Masonary Tee (RD0057)	16 ga (.053" min) Galv Steel Fymin = 30ksi
12	Or	Wire, Relaxed Dimension 9" x 8"	#7 (.167 " min) Galv Steel Wire (70,000 - 90,000 psi Tensile Strength)
13	Or	Expansion Bolt	3/8" x 5" F.H. Rawl Lok/Bolt Or 3/8" x 5" F.H. Ramset/RED Head
14	Or	Wood Lag Screw	3/8" x 4-5/8"
15	Viewer	Hager	1755
16	Or	MAG Security	8724-C
17	Drip Cap; Top	Pemko	346
18	Sweep	Pemko	315 N
19	Floor Anchor	Fixed Floor Anchor	16 ga (.053" min) galvanized Steel
20	Face Sheet A60 Galv Conforming To ASTM A653	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 Ga (.053" min)
21	Series SF, Frame Jamb, Double Rabbet Profile, A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Commercial Steel Type B (Minimum Yield Strength 30,000psi)	2" Face, 5-3/4" Depth Min. (RD0033)
22	Series SF, Frame Head, Double Rabbet, Profile A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Commercial Steel Type B (Minimum Yield Strength 30,000psi)	4" Face, 5-3/4" Depth Min. (RD0033)
23	Door Channels; Spot Welded To Bottom Skin Glued To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min) x 1" x 1-3/4" x 1"
24	Door Channels; Spot Welded To Bottom Skin Taped To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Commercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min x 1" x 1-3/4" x 1"
25	Closer Reinforcement (Optional)	12 Ga (.093" min) CS Type B	12 ga (.093" min) x 5-3/8" x 16"
26	Honeycomb Core	Non-Impregnated Kraft Paper Ⓔ	1.2" Nominal Cell Size
27	Urethane Core	Foam Enterprises	2 lb/ft^3 Density

Approved as complying with the Florida Building Code
Date Oct 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

B	10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A	9/4/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series In-Swing Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	
Sheet 9 Of 9	

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-46

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-6S-16-04056-128 **Building permit No.** 000024360

Use Classification SFD,UTILITY **Fire:** 11.84

Permit Holder JOHN NORRIS **Waste:** 0.00

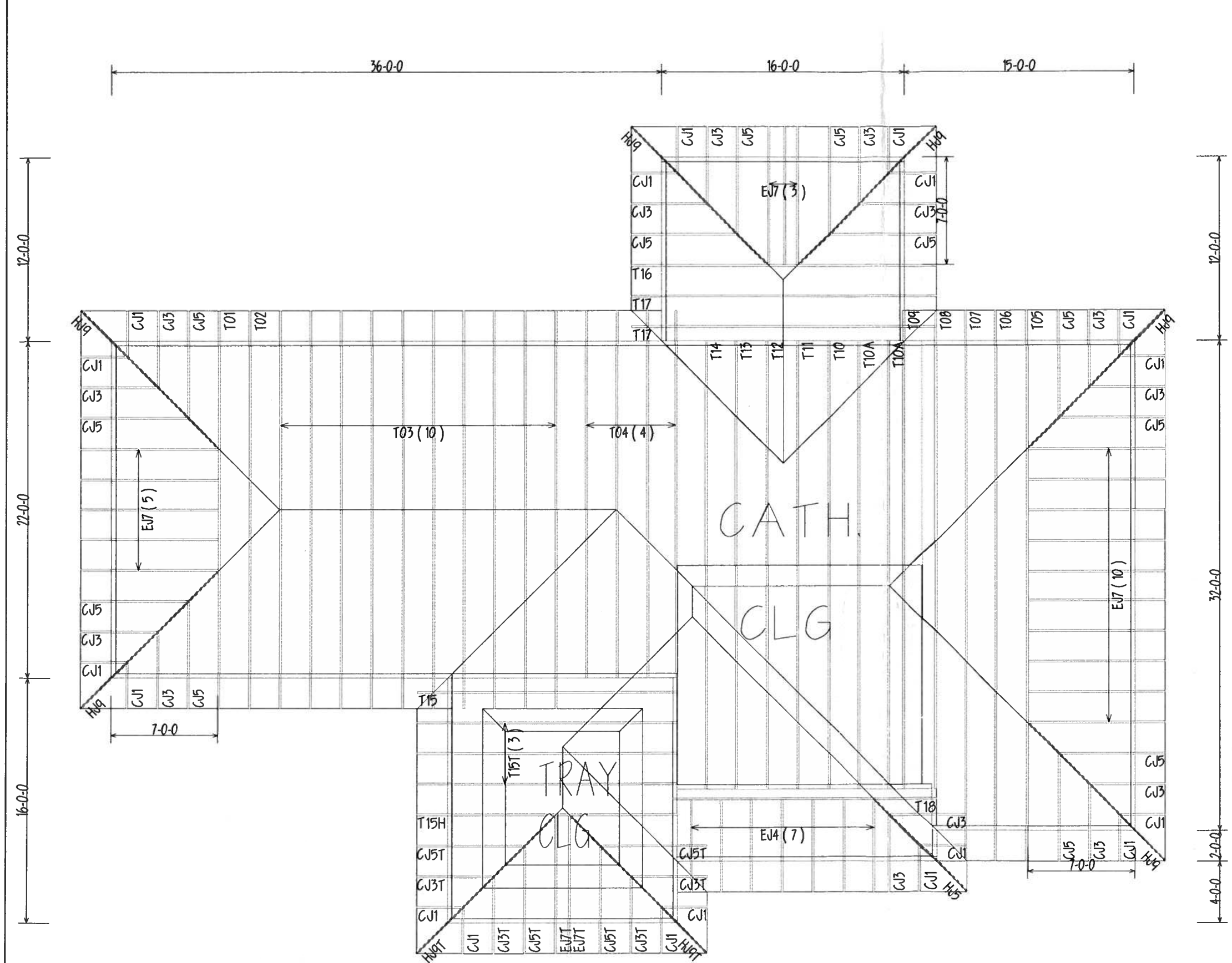
Owner of Building IMAGE DEVELOPMENT GROUP **Total:** 11.84

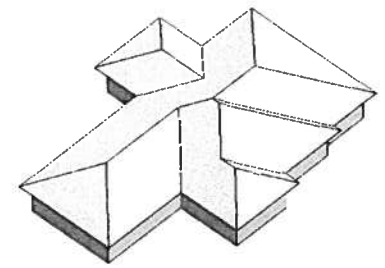
Location: 156 SW TISTLEWOOD LANE(THORNWOOD, LOT 27)

Date: 08/07/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)





BEARING HEIGHT SCHEDULE

8'-0"

6/12 PITCH
2' OH

28

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437 3349 FAX: 904-437 3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904 755-7973

Sanford
PHONE: 407-322-0059 FAX: 407 322-5553

BUILDER: JOHN NORRIS			
LEGAL ADDRESS:			
MODEL: SIERRA		REVISION:	
		SCALE: NTS	
DATE: 4/3/06	DRAWN BY: JRD	JOB #: L157751	