

GENERAL NOTES

FOUNDATIONS:

THE SOIL BEARING VALUE SHALL NOT BE LESS THAN 2000 PSF. THE BEARING VALUE SHALL BE VERIFIED BY THE G.C. FOOTINGS SHALL BEAR IN UNDISTURBED SOIL. ALL 7' FOOT. ELEVATION ARE TO BE THE SAME UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE NOT THE SAME C&S CANOPY MUST BE INSTRUCTED OF ELEV.'S IN WRITING WHEN CANOPY ORDER IS PLACED. FOOTING DESIGN COMPLIES W/ACI-318.

REINFORCING STEEL:

ALL DEFORMED BARS SHALL COMPLY W/ASTM A615 FY=60 AND SHALL BE WIRE TIED AT ALL JOINTS. RUSTY, OILY, OR DIRTY STEEL SHALL NOT BE USED.

ANCHOR BOLTS:

ANCHOR BOLTS MUST BE INSTALLED WITH A TEMPLATE AND WITHIN 1/8-INCH OF MEASUREMENTS OF THE BASE PLATE OR COLUMN WILL NOT FIT. CONCRETE CONTRACTOR IS RESPONSIBLE FOR RECESSING FOOTINGS 18 - INCHES BELOW FINISH GRADE AND FOR EXTENDING ANCHOR BOLTS 6 - INCHES ABOVE FOOTINGS IN ORDER FOR CANOPY TO ERECT PROPERLY. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36.

CONCRETE:

ALL CONCRETE SHALL BE 3000 PSI IN 28 DAYS. ALL CONC. SHALL BE PLACED IN ACCORDANCE WITH ACI-318.

STRUCTURAL STEEL:

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OTHER CONCRETE ITEMS:

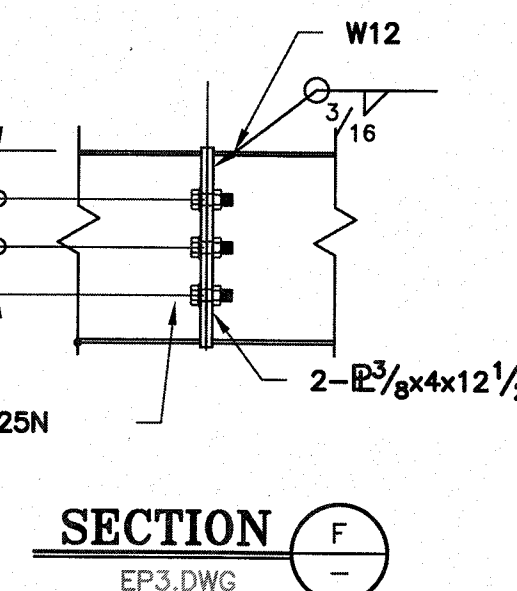
OTHER CONCRETE ITEMS SUCH AS DRIVE THRU SLAB, BUILDING SLAB, PERIMETER FOOTING AND LOAD BEARING FOOTINGS NOT USED FOR THE CANOPY ARE TO BE AT THE SAME ELEVATION UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE TO VARY, C & S CANOPY MUST BE INSTRUCTED OF ELEVATIONS IN WRITING WHEN ORDER IS PLACED.

FOUNDATION NOTE:

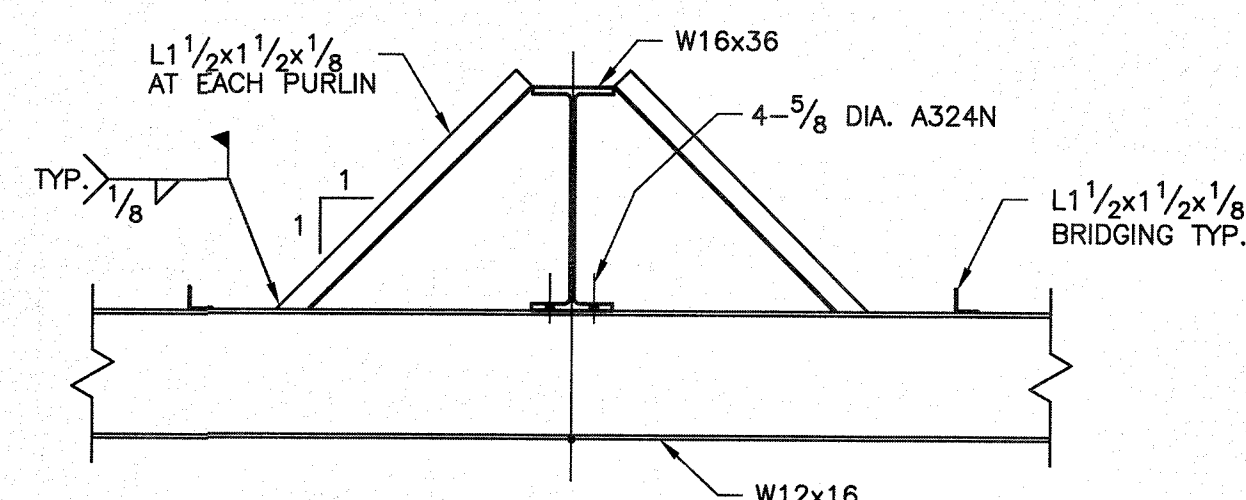
FOOTINGS PROVIDED BY CUSTOMER. C & S CAN. IS NOT RESPONSIBLE OR LIABLE FOR THE USE OF EXIST. FOOTINGS.

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"



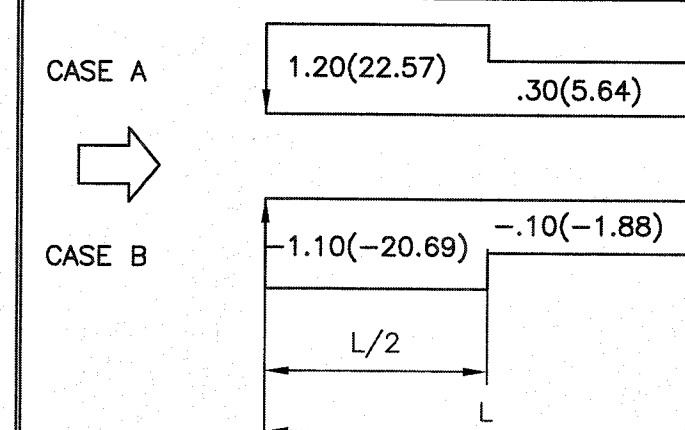
SECTION E-E



SECTION F-F

EP3.DWG

NOTE: THIS CANOPY IS DESIGNED PER ASCE 7-16 SEE FIG. 27.3-4, UNBALANCED WIND LOAD



* CLEAR WIND FLOW

NOTES:

- THESE LOADS HAVE BEEN APPLIED TO STRUCTURE IN ACCORDANCE WITH ASCE 7-16, CHAPTER 2, 2.4.1 BASIC COMBINATIONS FOR ALLOWABLE STRESS
 - THESE STEEL MEMBERS HAVE BEEN SIZED BASED ON ASD, AISC 14th EDITION.
 - ANALYSIS OF THIS STRUCTURE HAS BEEN ACCOMPLISHED USING THE LATEST GENERATION OF MATRIX BASED SOFTWARE.
 - COLUMN SLENDERNESS FACTORS ARE BASED ON CHAPTER C, DIRECT ANALYSIS METHOD.
- Kx = 1.0
Kz = 1.0
5. BASES ARE FIXED.

STRUCTURE LOADS

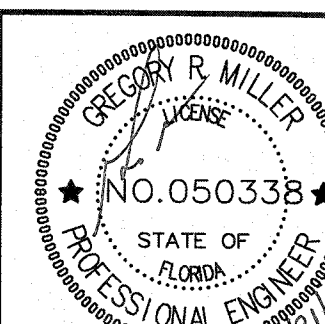
PARAMETER

CODE REFERENCE
FLORIDA CODE 2020
2

DEAD LOAD	4.0 PSF	1606.1
LIVE LOAD	20.0 PSF (w/ APP. RED.)	1607.12.2.1
SNOW LOAD	1.0 PSF + DRIFTS	ASCE 7-16, PART 7.0
WIND SPEED	117.0 MPH 3 SEC. GUST CATEGORY II lw 1.0 EXP. B	ASCE 7, PART 26-29
VERT. ROOF PRESSURE	Case A Cnw Cnl 1.20(22.57) .30(5.64) -1.10(-20.69) -.10(-1.88)	ASCE 7, FIG. 27.3-4
HORIZ. FASCIA PRESSURE	Case A & B Cfx 1.93(40.25) 1.87(39.05)	ASCE 7, FIG. 29.3-1
SEISMIC DATA	2 SEC. SPECTRUM RESPONSE, Ss 1 SEC. SPECTRUM RESPONSE, S1 LONG PERIOD TRANSITION PERIOD, Tl RISK CATEGORY SEISMIC FACTOR, Is SITE COEFFICIENT, Fa SITE COEFFICIENT, Fv SITE CLASSIFICATION SITE ADJUSTMENT COEFFICIENT, Sms SITE ADJUSTMENT COEFFICIENT, Sm1 DESIGN SPECTRAL RESPONSE, SDS DESIGN SPECTRAL RESPONSE, SD1 W, Kips SEISMIC RESPONSE COEFFICIENT, Cs BASIC STRUCTURAL SYSTEM - SEISMIC RESISTING SYSTEM Non-Detailed Sys./Ord. Cant. Col. RESPONSE MODIFICATION FACTOR, R METHOD OF ANALYSIS - EQUIVALENT LATERAL FORCE BASE SHEAR, Kips	ASCE 7-16 FIG. 22.1 FIG. 22-2 FIG. 22-14 TAB. 1.5-1 TAB. 1.5-2 TAB. 11.4-1 TAB. 11.4-2 TAB. 20.3-1 EQ. 11.4-1 EQ. 11.4-2 EQ. 11.4-3 EQ. 11.4-4 12.8 12.8, 1.1 12.8-1 12.8 EQ. 12.8-1

PREPARED BY: CARTER-MILLER-SANSING, LTD., P.O.B 4324, MERIDIAN, MS 39304. 601-483-0601

GREGORY R. MILLER
FLORIDA (CA 31480)
FL REG. 50338



MANUFACTURER:

C & S CANOPY, INC.

P.O. BOX 526 / CLANTON, AL 35045
PHONE: 1-(800) 932-4549

CUSTOMER:

A. PHELPS PETROLEUM

SCALE: NOTED

DATE:

LOCATION:

A. PHELPS PETROLEUM
4772 NW US HWY 41
LAKE CITY, FL

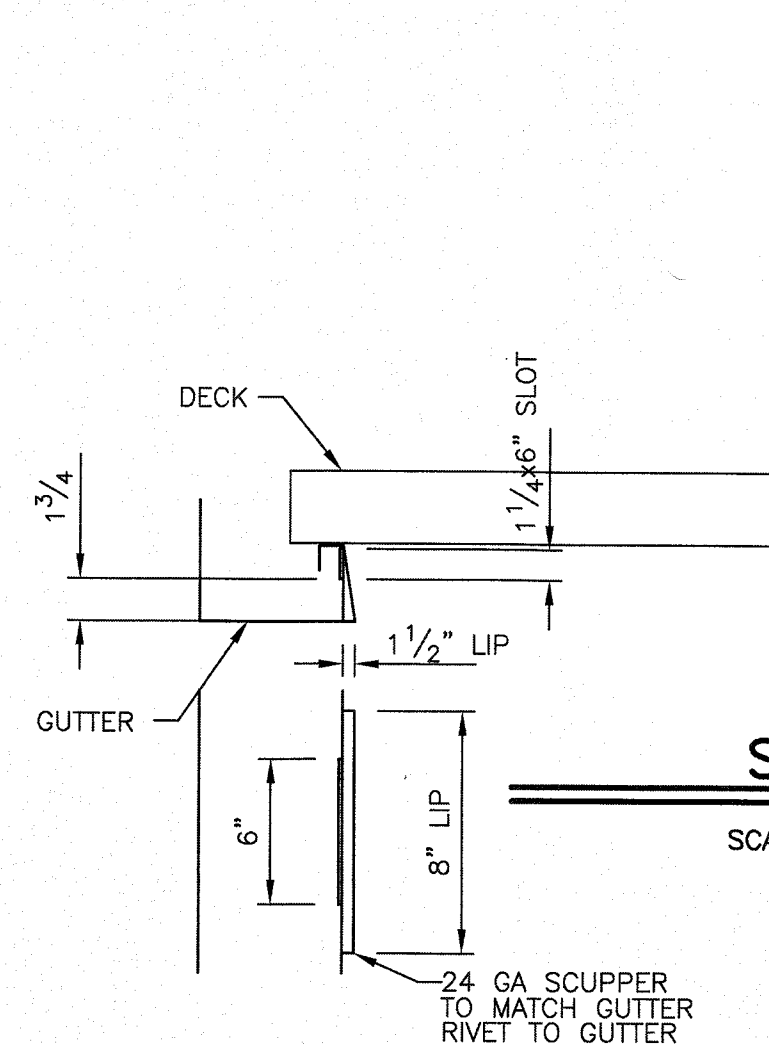
SHT. 1 OF 2

DRAWING NO:
20096

NO.	DATE:	BY:
1	10/14/2020	
2	2/21/2021	
3		

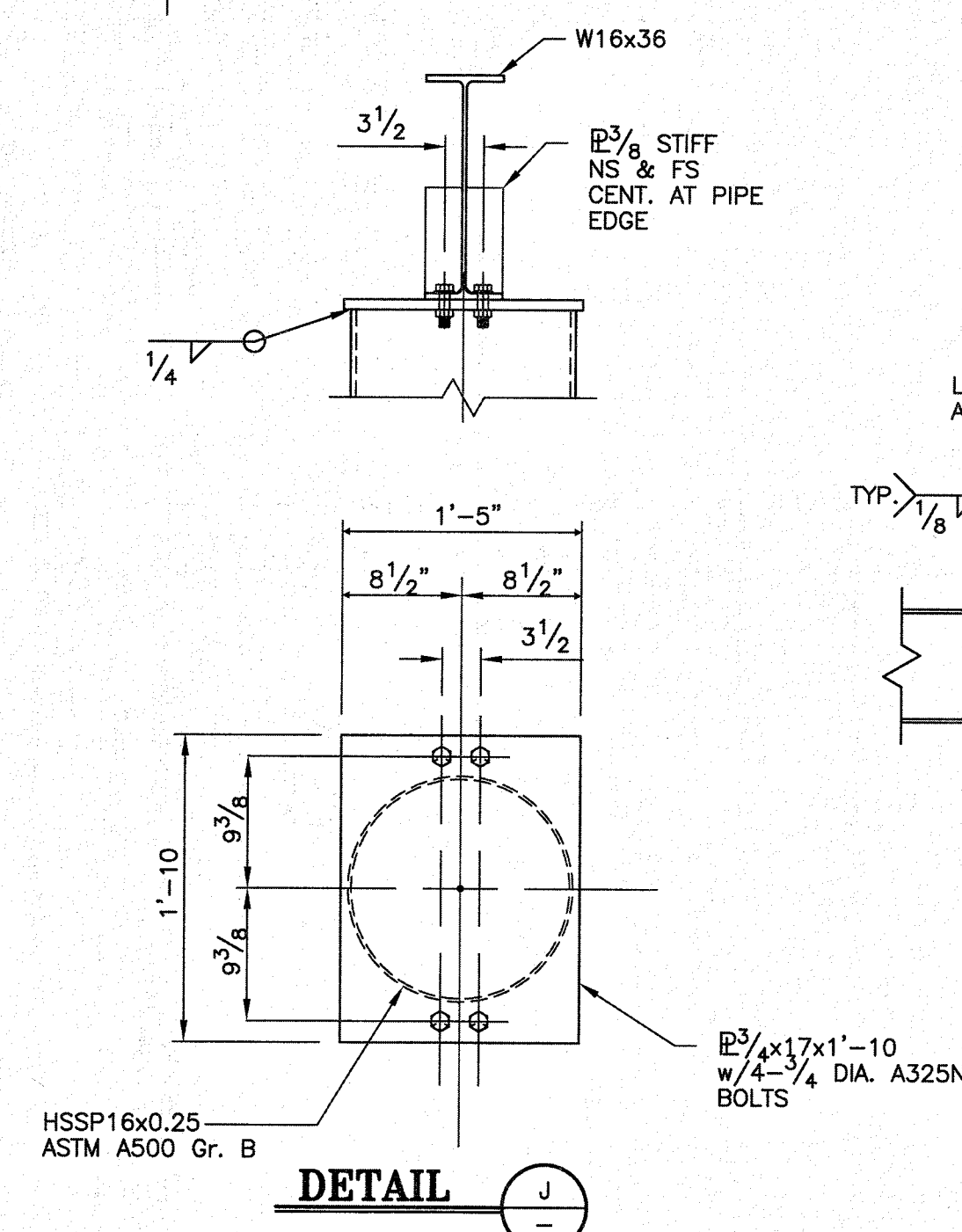
CARTER MILLER SANSING: 20-345
DATE: 8/28/2020

CERTIFICATION

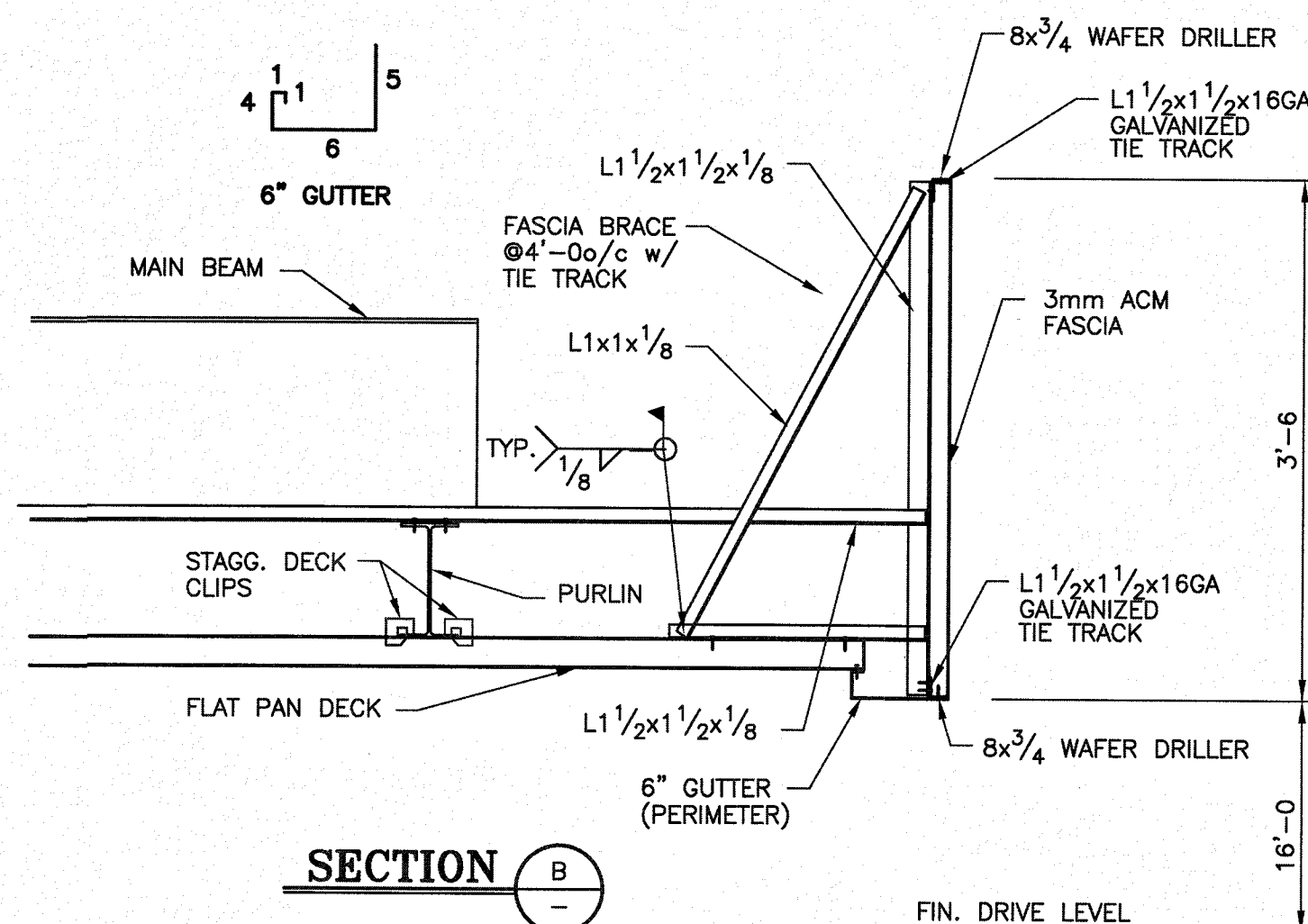


SCUPPER

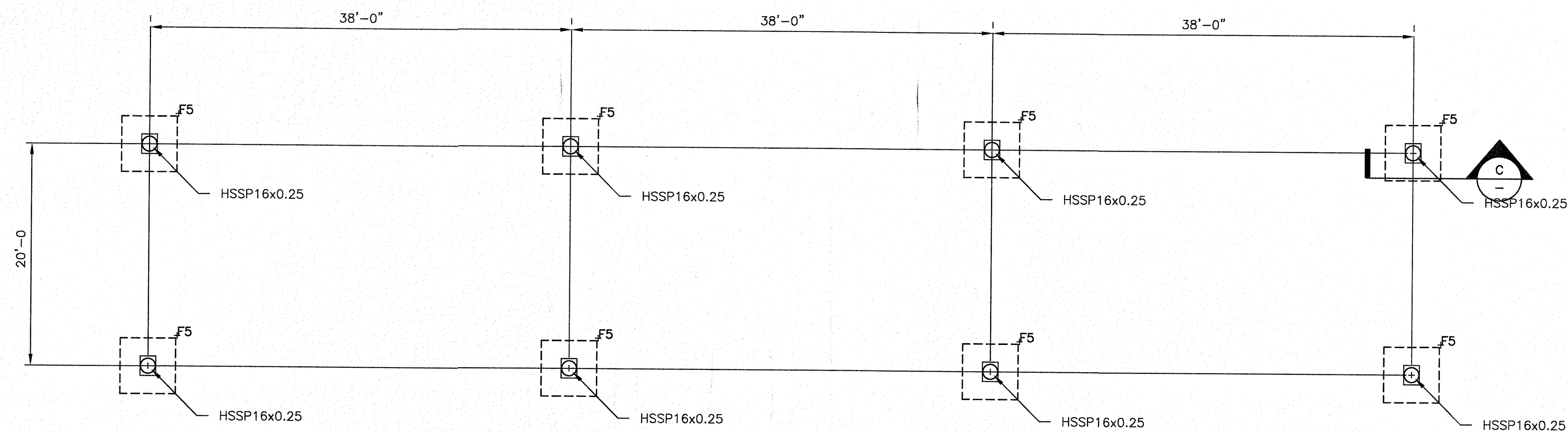
SCALE: 1 1/2" = 1'-0"



DETAIL J-J

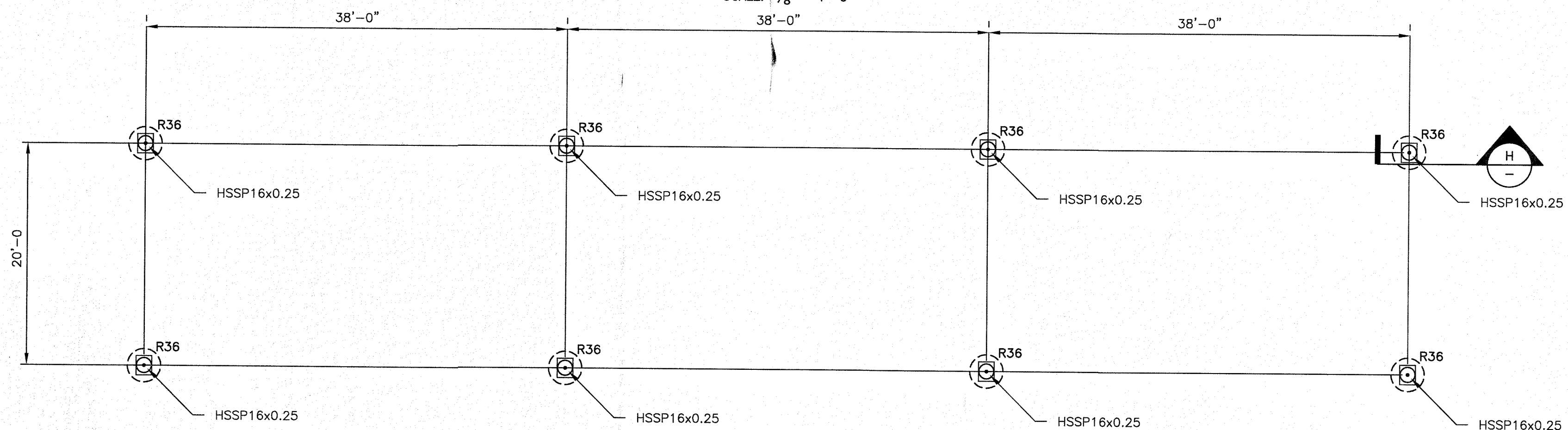


SECTION B-B



FOUNDATION PLAN

SCALE: $\frac{1}{8} = 1'-0$

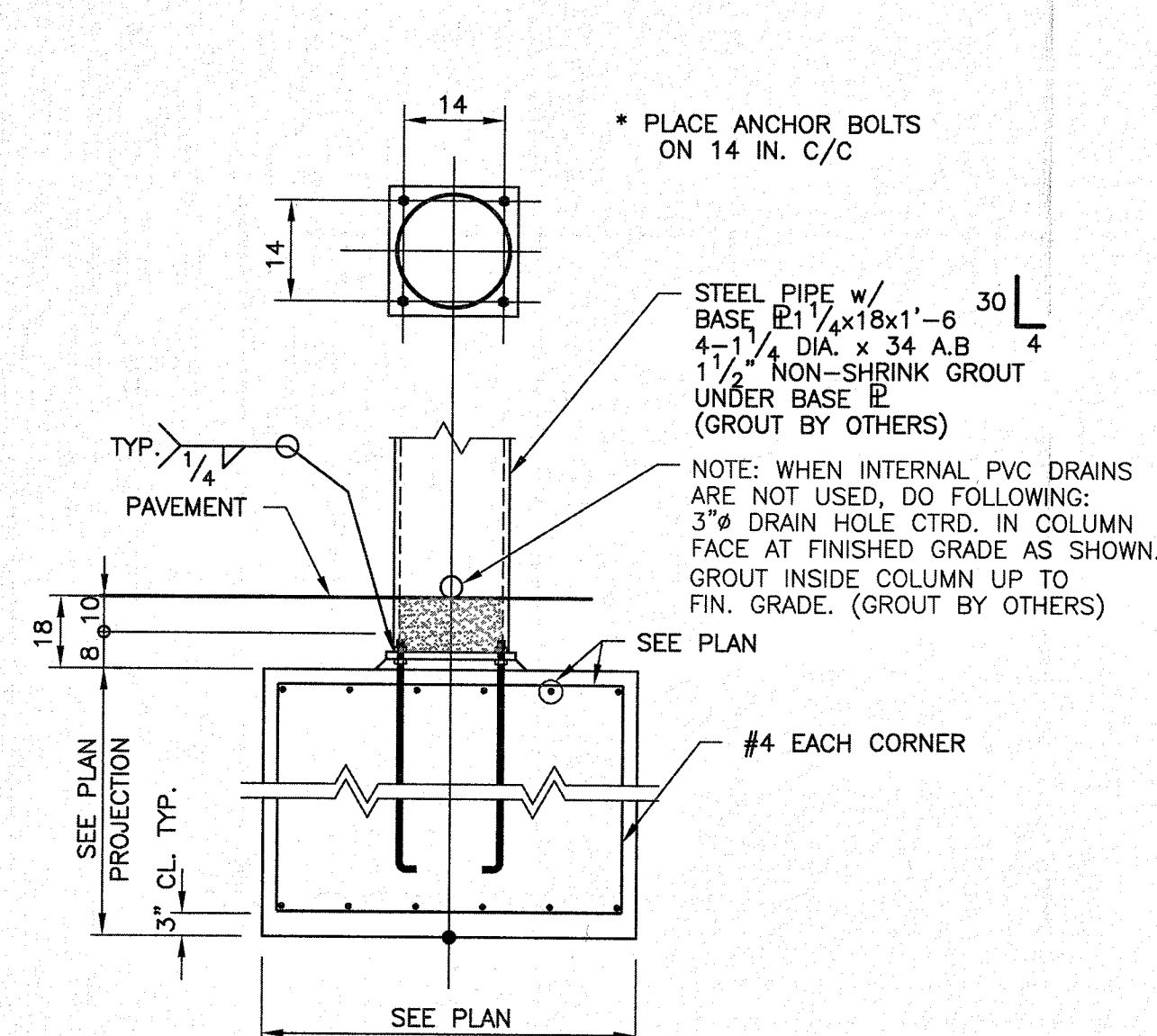


FOUNDATION PLAN (ALTERNATE)

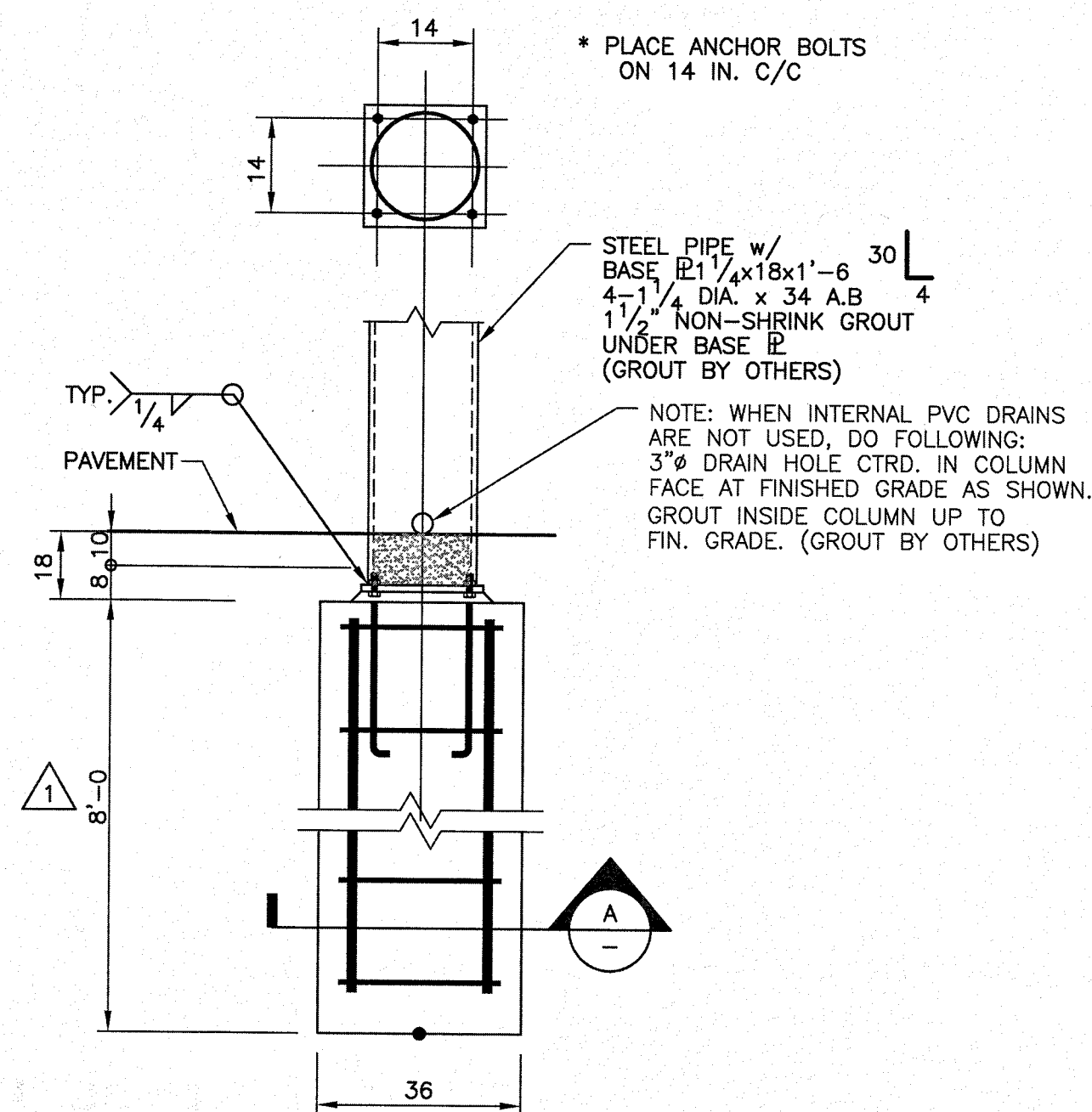
SCALE: $\frac{1}{8} = 1'-0$

FOOTING SCHEDULE

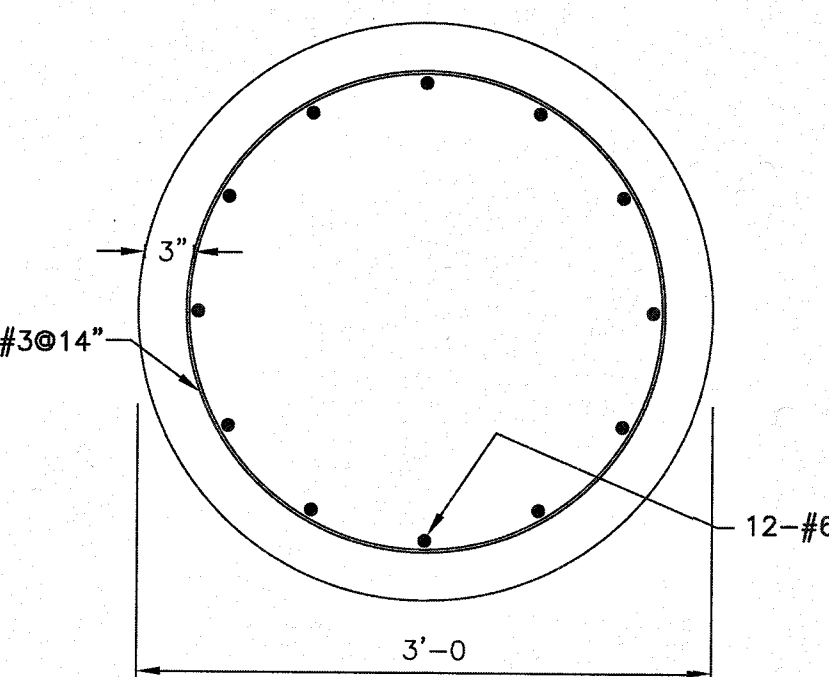
MARK	DIMENSIONS: LxWxD	#4 STEEL MAT TOP AND BOTTOM	#5 STEEL MAT TOP AND BOTTOM	#6 STEEL MAT TOP AND BOTTOM	REMARKS
F5	5'-0x5'-0x5'-0	#4@8o/c E.W	#5@10o/c E.W	#6@12o/c E.W	#4, #5, AND #6 OPTIONS ALL STEEL SHOWN IS TO BE PLACED EACH WAY.



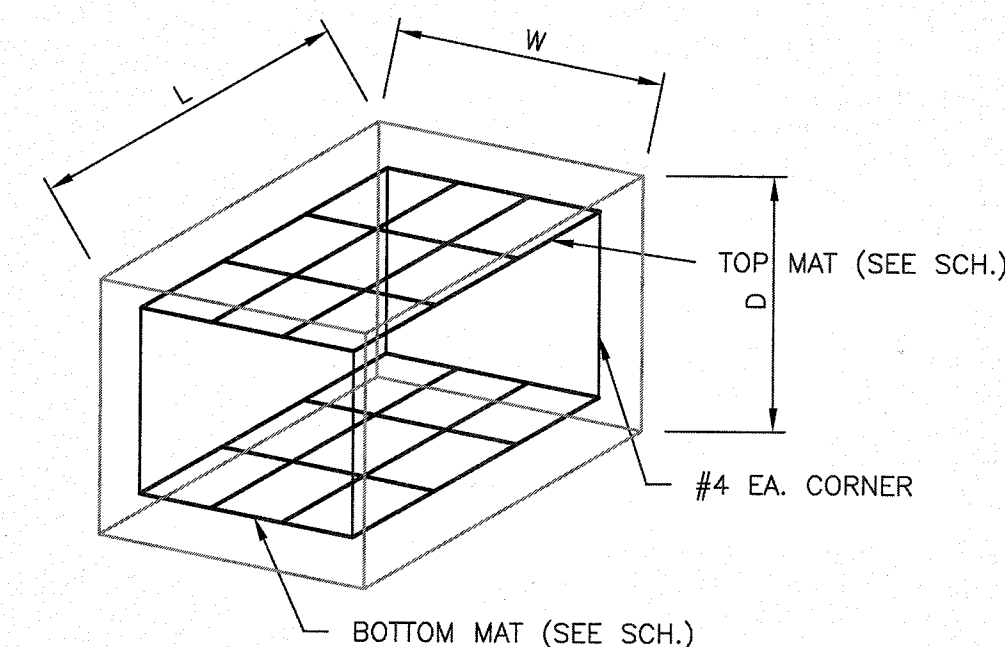
DETAIL C



DETAIL G (ALTERNATE)



SECTION A



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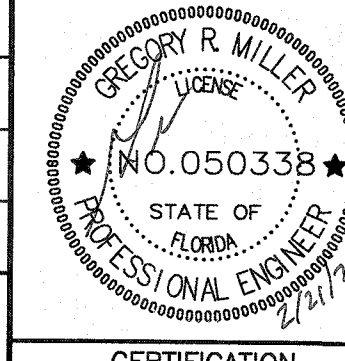
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VERT. ROOF PRESSURE		
Case A Cnw Cnl	Case B Cnw Cnl	ASCE 7, FIG. 27.3-4
1.20(22.57) .30(5.64)	-1.10(-20.69) -.10(-1.88)	
HORIZ. FASCIA PRESSURE	EXP. C	
Case A & B Cfx Cfy		ASCE 7, FIG. 29.3-1
1.93(40.25) 1.87(39.05)		
SEISMIC DATA		ASCE 7-16
2 SEC. SPECTRUM RESPONSE, Ss	0.0870	FIG. 22.1
1 SEC. SPECTRUM RESPONSE, S1	0.0510	FIG. 22.2
LONG PERIOD TRANSITION PERIOD, Tl	8	FIG. 22-14
RISK CATEGORY	II	TAB. 1.5-1
SEISMIC FACTOR, Ie	1.00	TAB. 1.5-2
SITE COEFFICIENT, Fa	1.60	TAB. 11.4-1
SITE COEFFICIENT, Fv	2.40	TAB. 11.4-2
SITE CLASSIFICATION	D	TAB. 20.3-1
SITE ADJUSTMENT COEFFICIENT, Sm1	0.1392	EQ. 11.4-1
SITE ADJUSTMENT COEFFICIENT, Sm2	0.1224	EQ. 11.4-2
DESIGN SPECTRAL RESPONSE, SDS	0.0928	EQ. 11.4-3
DESIGN SPECTRAL RESPONSE, SD1	0.0816	EQ. 11.4-4
W, Kips	33.00	12.8
SEISMIC RESPONSE COEFFICIENT, Cs	Z X	12.8.1.1
0.0743 0.0743		
BASIC STRUCTURAL SYSTEM - SEISMIC RESISTING SYSTEM		
Non-Detailed Sys./Orid. Cant. Col.		
RESPONSE MODIFICATION FACTOR, R	1.25 1.25	TAB. 12.2-1
METHOD OF ANALYSIS - EQUIVALENT LATERAL FORCE	V=C&W	12.8
BASE SHEAR, Kips	3.06 3.06	EQ. 12.8-1

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DATE:

LOCATION:

APPROVED BY:

A. PHELPS PETROLEUM

4772 NW US HWY 41

LAKE CITY, FL

SHT. 2 OF 2

DRAWING NO:

20096