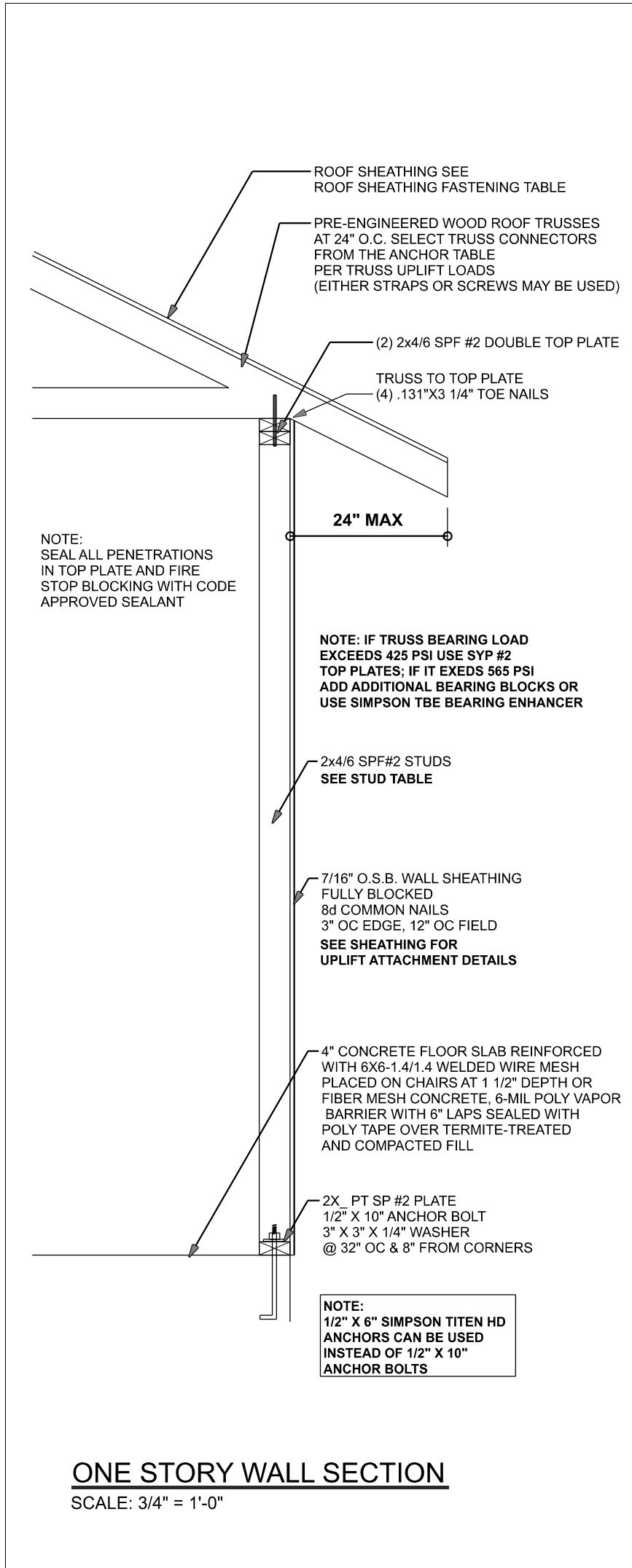
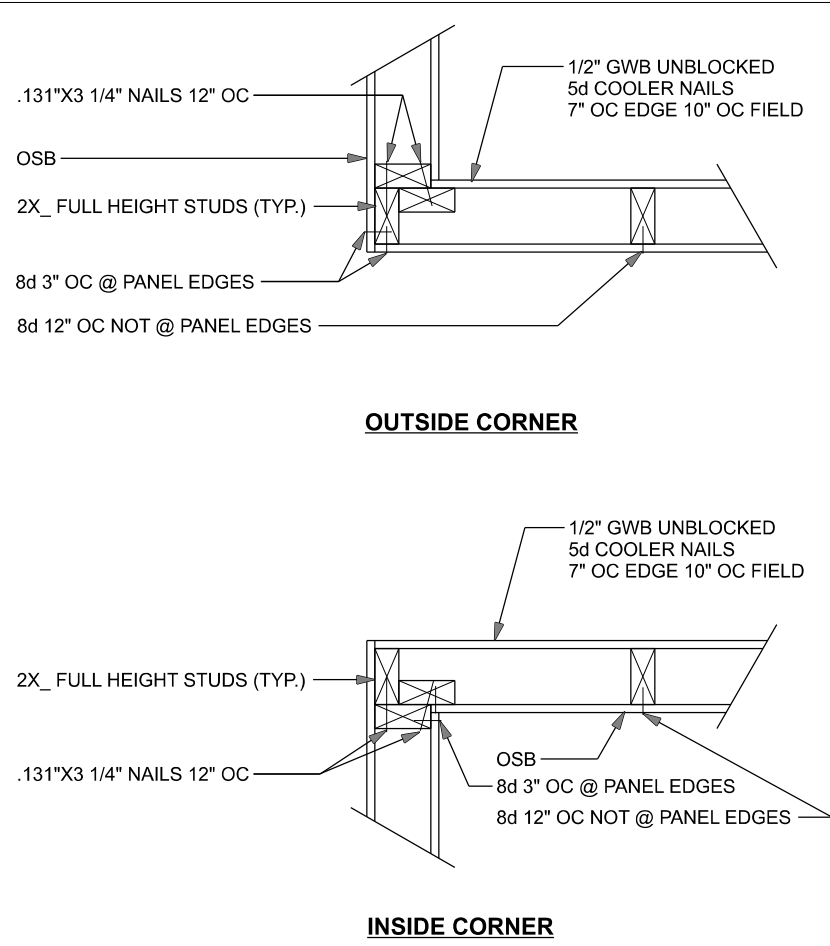


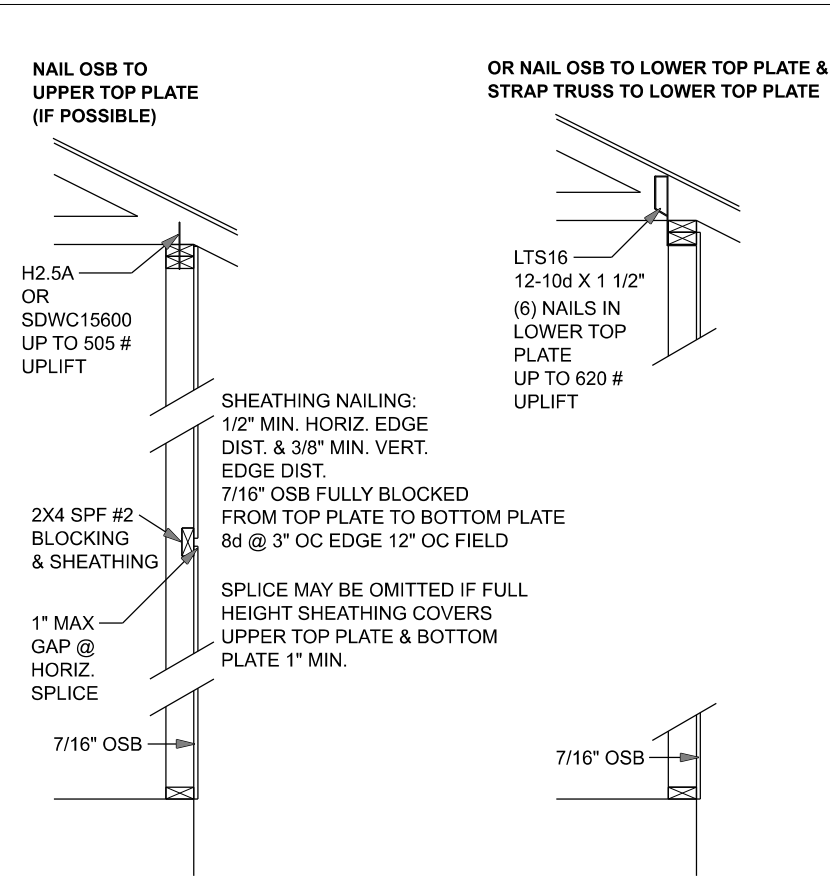


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

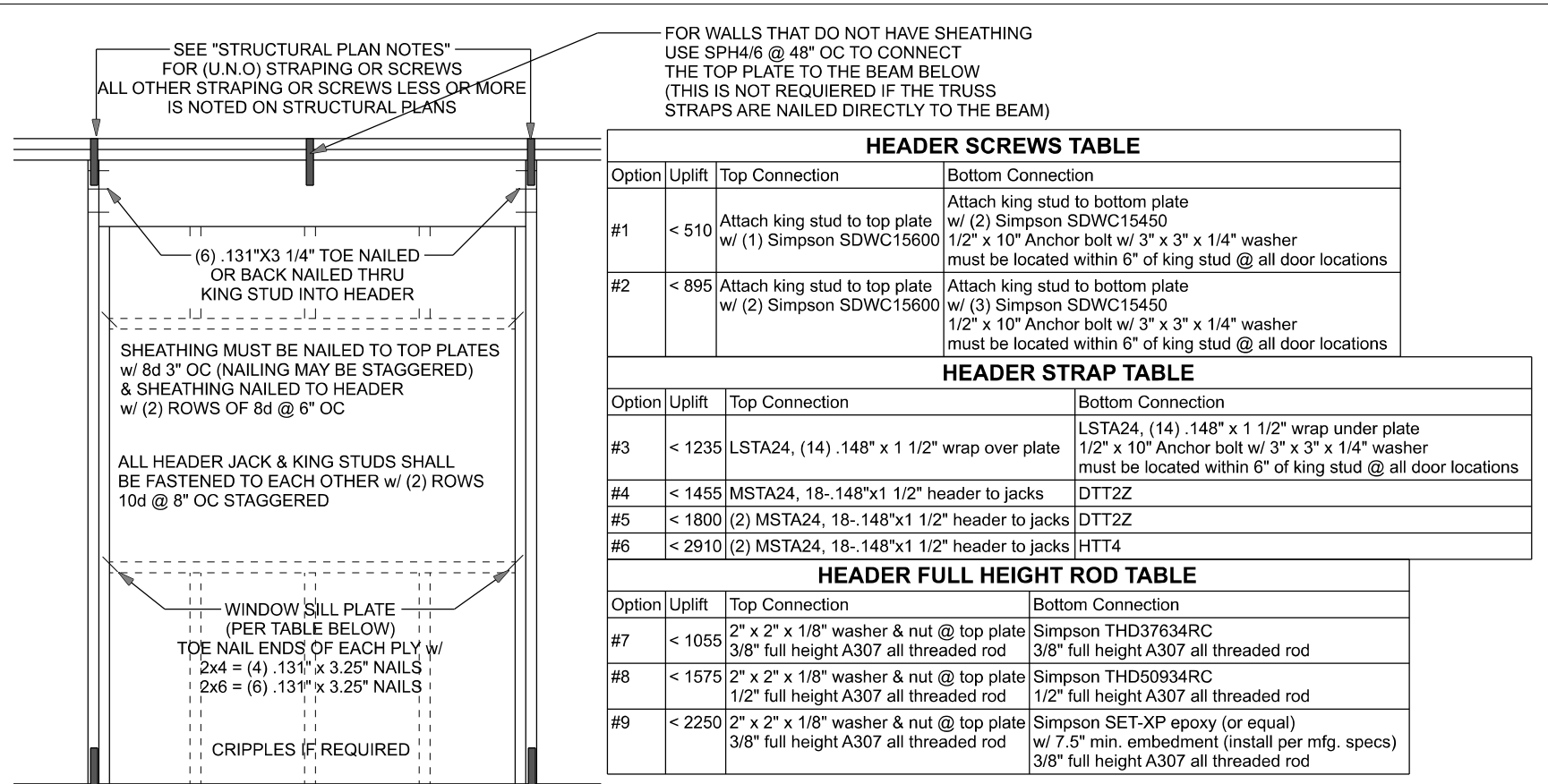
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood OR OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RSRs-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RSRs-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	6" oc	6" oc	
160 mph Exp. C	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	6" oc	6" oc	
160 mph Exp. D	19/32"	ASTM F1667 RSRs-03 (2 1/2" x 0.131")	4" oc	4" oc	



(TYP.) CORNER FRAMING
WOOD FRAME



SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



TYPICAL HEADER STRAPPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	4-131"x1 1/2"	4-131"x1 1/2"
400	280	H2	4-131"x1 1/2"	280
625	540	H2.5A	5-131"x1 1/2"	5-131"x1 1/2"
1040	1015	H10A	9-148"x1 1/2"	9-148"x1 1/2"
645	515	LTS12-20	6-148"x1 1/2"	6-148"x1 1/2"
960	850	MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"
1415	1215	MTS16-30	8-148"x1 1/2"	8-148"x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-148"x1 1/2"	8-148"x1 1/2"
1640	1460	MSTA24	9-148"x1 1/2"	9-148"x1 1/2"
1030	1030	CS20	7-148"x1 1/2"	7-148"x1 1/2"
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4-148"x3"	4-148"x3"
1010	805	SP2	6-148"x3"	6-148"x3"
1280	1100	SPH46	12-148"x1 1/2"	wrap under or over plate
771	771	LSTA24	10-148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-148"x1 1/2"	wrap under or over plate
Uplift SP	Uplift SPF	Headerdowns @ Stenwall	To Stud / Post	Anchor
2145	1835	DT122	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HT14	18-162"x2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPF	Headerdowns @ Mono	To Stud / Post	Anchor
2145	1835	DT122	8-SDS 1/4"x1 1/2"	12"x12" Titen HD
4235	3640	HT14	18-162"x2 1/2"	12"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
2475	1900	ABU642	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900	1400	ABU642	12-162"x3 1/2"	5/8"x12" Drill & Epoxy
2475	1900	ABU642	12-162"x3 1/2"	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:				
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/400 (NOT OK FOR BUTTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)				
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT			
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT			
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT			
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT			

GRADE & SPECIES TABLE			
	SP #2	Fd	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FIBER TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. THE BUILDER'S RESPONSIBILITY IS TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS CONNECTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 4x15x8 EACH END, 2X6 RAFTERS 70 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDE OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 8" W1 x 4 W1.4 F8 = 69KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 7'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB RAFTERS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI, ALL LAP SPLICES 40" DB OR 1.33 L, MIN. 6" LAP. UNDO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-06, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL GORES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMINER'S TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

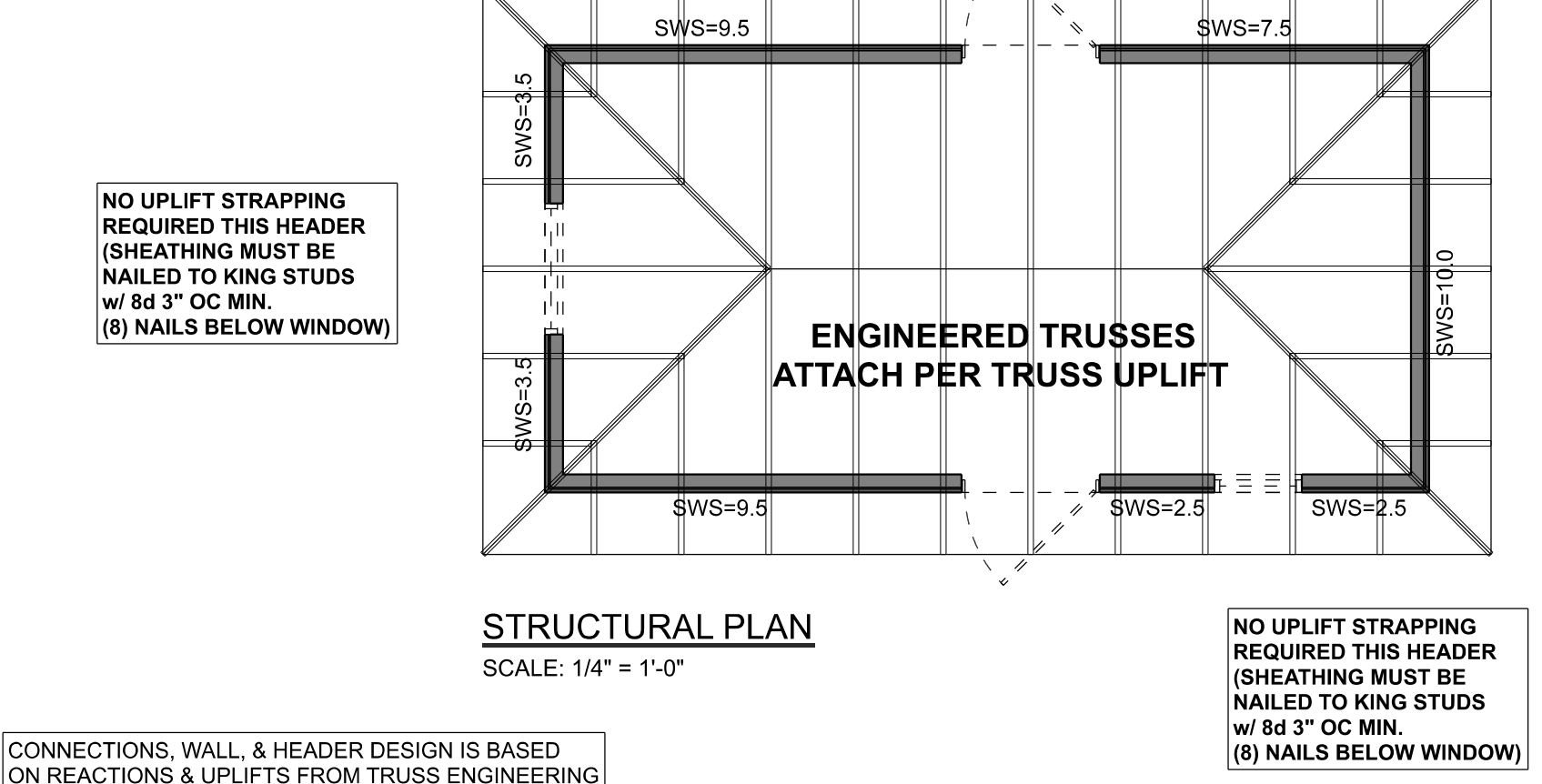
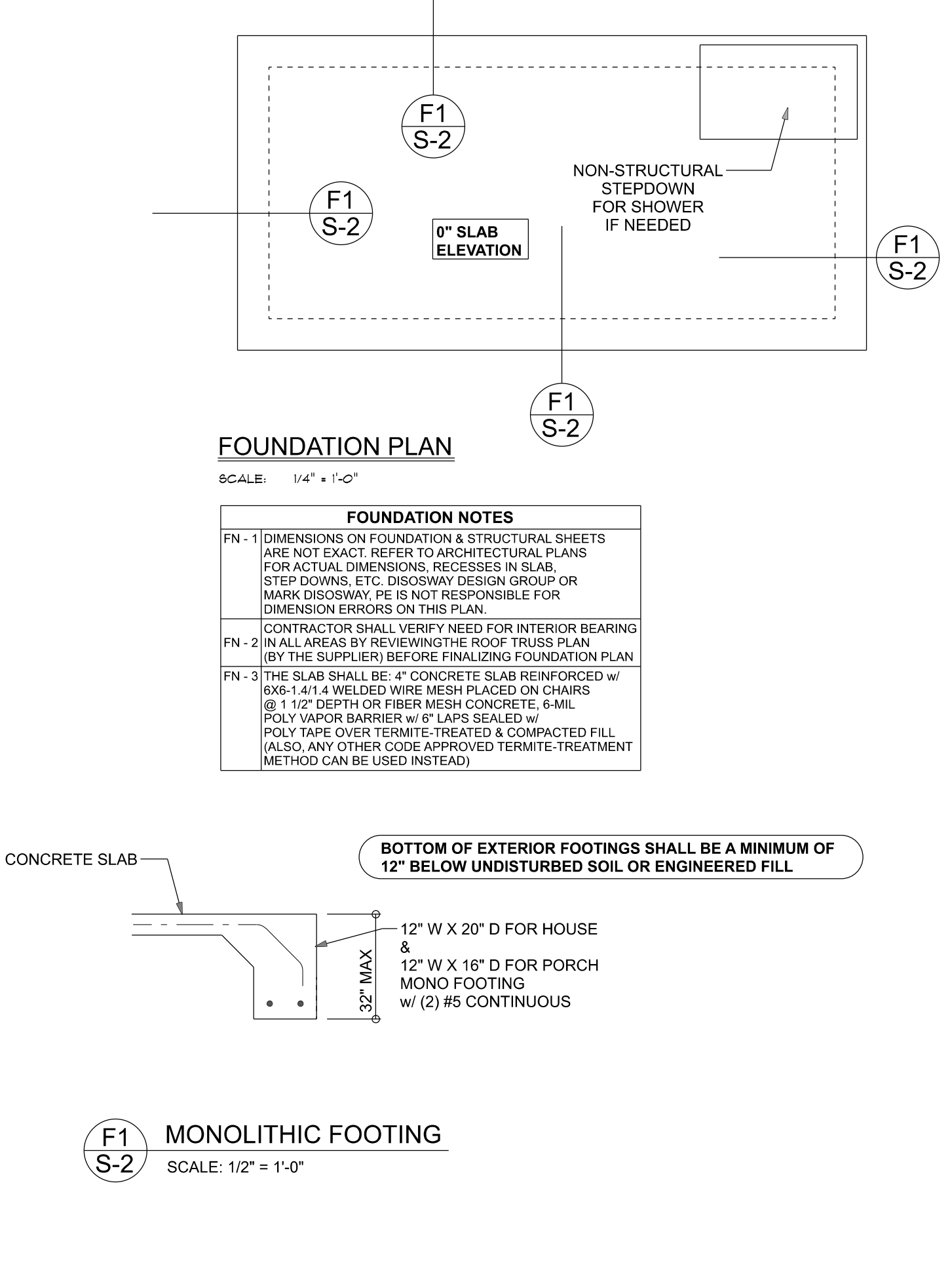
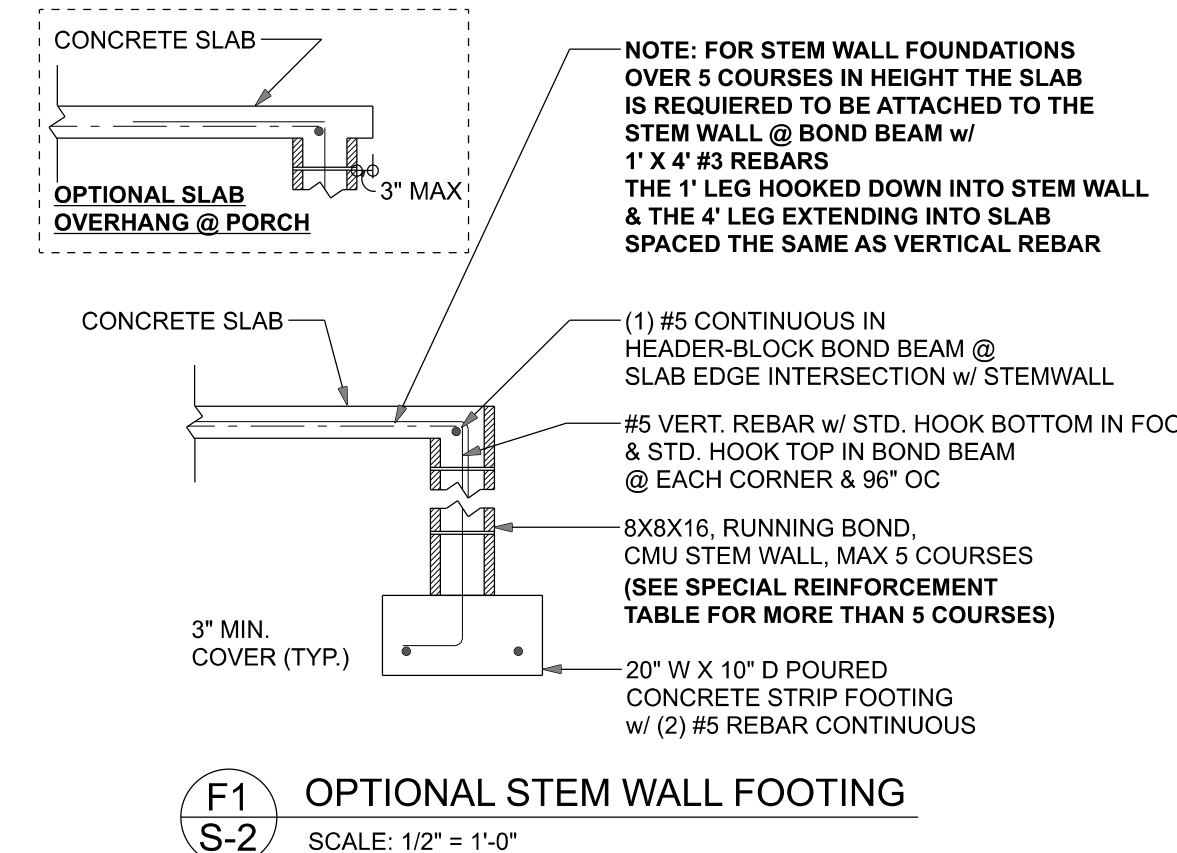
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND BEARING LOCATIONS. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. THE BUILDER IS RESPONSIBLE FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

TALL STEM WALL TABLE:							
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall).							
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96

THIS FOUNDATION DESIGN IS FOR RELATIVELY FLAT GRADE ONLY. IF FOUNDATION IS ON A STEEP SLOPE THAT EXCEEDS 1' IN 12', CONTACT ENGINEER BEFORE CONSTRUCTION FOR ADDITIONAL ENGINEERING



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	6732 LBF	12474 LBF
REQUIRED	2595 LBF	1364 LBF

STRUCTURAL PLAN NOTES

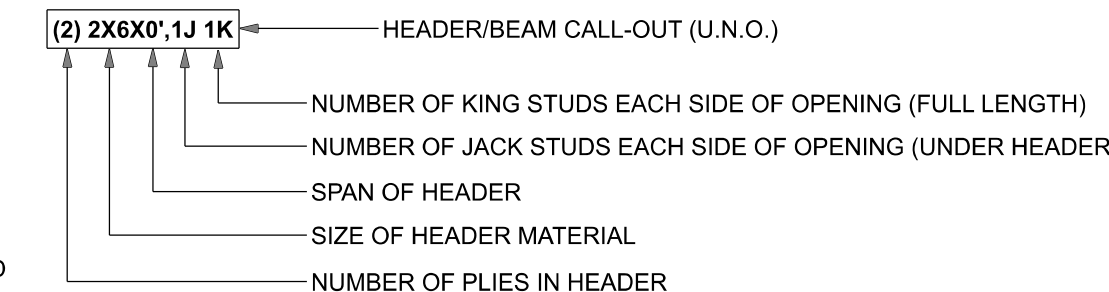
SN-1

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-2

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03, BCSH-B1, BCSH-B2, & BCSH-B3. BCSH-B1, BCSH-B2, & BCSH-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



UNLESS NOTED OTHERWISE (MINIMUM REQUIERMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" x 10" ANCHOR BOLT w/ 3" x 3" x 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, F _y = 40 ksi, Lap splices min 40 bar dia. (25' for #5)
2.4F Coating for corrosion protection	Anchors, shear metal ties completely embedded in mortar or grout, ASTM A525, Class 250, 0.60 oz/lb or 30/AS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, shear metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 30/AS
3.3.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.2.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DESIGN CRITERIA & LOADS:	
BUILDING CODE	19th EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE	C
BUILDER MUST FIELD VERIFY	1
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	ENCLOSED
ENCLOSURE CLASSIFICATION	0.18
INTERNAL PRESSURE COEFFICIENT	7-45 DEGREES
ROOF ANGLE	30 FT
MEAN ROOF HEIGHT	SEE TABLE
C&S DESIGN PRESSURES	
FLOOR LOADING	40 PSF LIVE LOAD
ROOMS OTHER THAN SLEEPING ROOMS	30 PSF LIVE LOAD
SLEEPING ROOMS	20 PSF LIVE LOAD
ROOF LOADING	16 PSF LIVE LOAD
FLAT OR < 4:12	12 PSF LIVE LOAD
4:12 TO < 12:12	12 PSF LIVE LOAD
12:12 & GREATER	1500 PSF
SOIL BEARING CAPACITY	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)			
EFFECTIVE WIND AREA (F _{T2})	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER	END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasdt)	-27.8(Vasdt)	+25.6(Vasdt)
0 - 20	+42.8(Vult)	-46.2(Vult)	+42.8(Vult)
20 - 40	+22.6(Vasdt)	-25.5(Vasdt)	+22.6(Vasdt)
16x7 GARAGE DOOR	+21.7(Vasdt)	-24.1(Vasdt)	+21.7(Vasdt)

Bryan Zecher Homes, Inc

POOL HOUSE FOR STEVEN & KAREN BEALS

PROJECT ADDRESS: 386754 5419 LAKE CITY, FL 32026

FL PE 53915
This item has been digitally signed and sealed by Mark Disoway, P.E. on digital signature date. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 18th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
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