DATE 08/07/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027247

APPLICANT ROY LEWIS PHONE 454-1584

ADDRESS 20818 S US HIGHWAY 441 HIGH SPRINGS FL 32643

OWNER ROY LEWIS PHONE 454-1584

ADDRESS 20818 S US HIGHWAY 441 HIGH SPRINGS FL 32643

CONTRACTOR SAME AS APPLICANT PHONE

LOCATION OF PROPERTY 441S, 1/2 MILE SOUTH OF BELLAMY ROAD ON RIGHT SIDE

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 84100.00

HEATED FLOOR AREA 1230.00 TOTAL AREA 1682.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 09-7S-17-09957-112 SUBDIVISION SHERWOOD FREST

LOT 13 BLOCK PHASE UNIT TOTAL ACRES 1.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
EXISTING	08-439	BK	WR
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance

New Resident

COMMENTS: LEGAL NON-CONFORMING LOT OF RECORD, IMPACT FEE EXEMPT, EXISTING MH TO BE REMOVED 45 DAYS OF CO ISSUANCE, NOC ON FILE

Check # or Cash 3000

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by Foundation date/app. by Monolithic date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 425.00 CERTIFICATION FEE $ 8.41 SURCHARGE FEE $ 8.41

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 516.82

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0807-57 Date Received 7/23/08 By GF Permit # 27247

Zoning Official BLK Date 29.07.08 Flood Zone X FEMA Map # N/A Zoning A-3

Land Use A-3 Elevation N/A MFE 1ft above #1 River N/A Plans Examiner (WE) Date 7/29/08

Comments Legal Non-Conforming Lot of Record, Impact Fee Exempt Existing MH to be removed 45 day after issue of CO.

☑ NOC ☑ EH ☐ Deed or PA ☐ Site Plan ☑ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor existing well

☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 08-0439 Fax

Name Authorized Person Signing Permit Roy E. Lewis Phone 386-454-1584

Address 20818 S. US Hwy 441 High Springs, Fl. 32643

Owners Name Roy E. Lewis Phone 386-454-1584

911 Address 20818 S. US Hwy 441 High Springs, Fl. 32643

Contractors Name Roy E Lewis (owner-Builder) Phone 386-454-1584

Address 20818 S US Hwy 441 High Springs, Fl. 32643

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Driscoll Engineering Inc P.O. Box 357597, GNV, Fl. 3260

Mortgage Lenders Name & Address

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 09-7S-17-09957-112 Estimated Cost of Construction $65,000.

Subdivision Name Sherwood Forest Lot 13 Block Unit 3 Phase

Driving Directions ON US441 - ½ mile South OF South West Bellamy Rd (ON WEST Side OF Hwy)

Number of Existing Dwellings on Property 1

Construction of SFD Total Acreage 1 Lot Size 100.0 FT X 435.

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive) Total Building Height 15'10"

Actual Distance of Structure from Property Lines - Front 150' Side 25.25' Side 25.25' Rear 340.93'

Number of Stories 1 Heated Floor Area 1230 SF Total Floor Area 1682 SF Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

left message to Roy FDOT 8/5/08

left message 7/30/08

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

FLORIDA'S CONSTRUCTION LIEN LAW; Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Roy E. Davis 7-23-08
Owners Signature

PAIGE HALE
MY COMMISSION # DD 633396
EXPIRES: January 28, 2011
Bonded Thru Notary Public Underwriters

Paige Hale

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ **day of** _____ **20**____.

Personally known_____ **or Produced Identification**_____

SEAL:

State of Florida Notary Signature (For the Contractor)

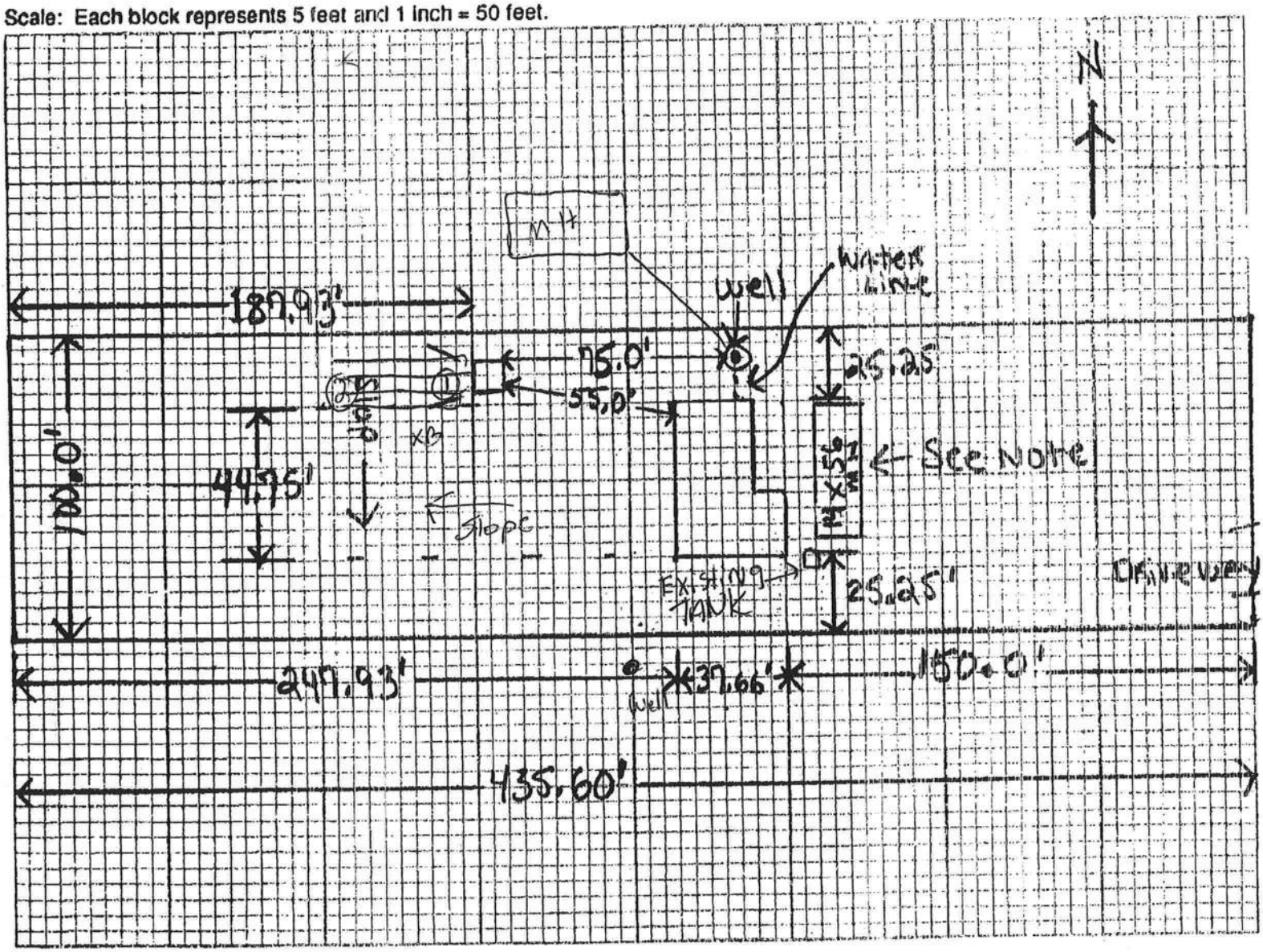
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0439

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: MOBILE HOME IS TO BE REMOVED AFTER COMPLETION OF NEW HOME CONSTRUCTION
OLD TANK TO BE ABANDON AFTER COMPLETION OF NEW HOME

Site Plan submitted by: Roy Lewis (Signature) — Owner (Title)

Plan Approved ✓ Not Approved _____ Date 6-18-08

By _____ Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave.. Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

(X) Single Family Dwelling ( ) Two-Family Residence ( ) Farm Outbuilding
( ) Other_____ ( ) Addition, Alteration, Modification or other Improvement

I Roy E. Lewis, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

Roy E. Lewis 7-23-08
Owner Builder Signature Date

FLORIDA NOTARY
The above signer is personally known to me or produced identification _____
Notary Signature Paige Hale Date 7-23-08

PAIGE HALE
MY COMMISSION # DD 633396
EXPIRES: January 28, 2011
Bonded Thru Notary Public Underwriters

FOR BUILDING DEPARTMENT USE ONLY
I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date_____ Building Official/Representative_____

Inst:200812013734 Date:7/23/2008 Time:2:55 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1155 P:267

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 09-7S-17-09957-112

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes. the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: Lot Thirteen (13) of Sherwood Forest Unit No-3
   a) Street *(job)* Address: 20818 S. US Hwy 441 - High Springs, Fl. 32643
2. General description of improvements: New Construction of Single Family Home
3. Owner Information
   a) Name and address: Roy E. Lewis Jr. 32643
   b) Name and address of fee simple titleholder (if other than owner) 20818 S. US Hwy 441 - High Springs, Fl
   c) Interest in property 100%
4. Contractor Information
   a) Name and address: ~~N/A~~ Roy E. Lewis JR 20818 S. US Hwy 441 High Springs, 32643
   b) Telephone No.: 386-454-1584 Fax No. (Opt.)
5. Surety Information
   a) Name and address: N/A
   b) Amount of Bond:
   c) Telephone No.: Fax No. (Opt.)
6. Lender
   a) Name and address: N/A
   b) Phone No.
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address:
   b) Telephone No.: Fax No. (Opt.)
8. In addition to himself. owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b). Florida Statutes:
   a) Name and address: N/A
   b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified)**:

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Roy E. Lewis JR
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Roy E. Lewis JR
Print Name

The foregoing instrument was acknowledged before me , a Florida Notary, this 23rd day of July, 2008, by: F. Voncile Dow as Notary **(type of authority, e.g. officer, trustee, attorney fact)** for Roy E Lewis JR **(name of party on behalf of whom instrument was executed).**

Personally Known ____ OR Produced Identification X Type FL D/L

Notary Signature F. Voncile Dow Notary Stamp or Seal:

F. VONCILE DOW
MY COMMISSION # DD 592031
EXPIRES: October 3, 2010
Bonded Thru Notary Public Underwriters

—AND—

11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Roy E Lewis Jr.
Signature of Natural Person Signing (in line # 10 above.)

This Special Warranty Deed Made the 15th day of February, 2008, by

LISA J. MOORE, a married person, conveying her separate, non-homestead real property,

whose post office address is
161 S. E. Robin Hood Place, High Springs, FL 32643-1385,
hereinafter called the grantor, to

ROY EUGENE LEWIS, JR.,

whose post office address is
20747 N. W. 166th Place, High Springs, FL 32643-7782,
and whose social security number is
hereinafter called the grantee:

Inst:200812003264 Date 2/18/2008 Time 11:36 AM
Doc Stamp-Deed: 175.00
DC,P.DeWitt Cason,Columbia County Page 1 of 1

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth: That the grantor, for and in consideration of the sum of $10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

Lot Thirteen (13) of SHERWOOD FOREST UNIT NO. 3, a subdivision, according to the Plat thereof as recorded in Plat Book 5, Page 8, of the Public Records of Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons claiming, by, through or under the Grantor herein; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, and except for covenants, easements and restrictions of record.

Subject to that certain Perpetual Grant of Easement recorded in Official Records Book 1088, Page 1720, of the Public Records of Columbia County, Florida.

Property Appraisers Parcel Number(s): 09-7S-17-09957-112

Grantor further warrants and states that the real property described herein is not her homestead and that she resides at 161 S. E. Robin Hood Place, High Springs, Florida.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered
in our presence as witnesses:

Diana E. Harris
Print Name Diana E. Harris

Wayne P. Castello
Print Name Wayne P. Castello

Lisa J. Moore L.S.
LISA J. MOORE

STATE OF FLORIDA
COUNTY OF ALACHUA

The foregoing instrument was acknowledged before me this 15th day of February, 2008, by LISA J. MOORE, a married person, conveying her separate, non-homestead real property, who is personally known to me or who has produced a Driver License as identification.

Diana E. Harris
Notary Public, State of Florida
My Commission expires:
My Commission number:

DIANA E. HARRIS
MY COMMISSION EXPIRES
JULY 22, 2011
#DD 684891
Bonded thru
Troy Fain-Insurance
NOTARY PUBLIC, STATE OF FLORIDA

**This Instrument Prepared by,
and return to:**
WAYNE P. CASTELLO
ATTORNEY AT LAW
2772 NW 43 STREET, STE W
GAINESVILLE, FLORIDA 32606
HUD - 2008-48

NO REVIEW OR EXAMINATION OF TITLE TO THE ABOVE-DESCRIBED PROPERTY HAS BEEN MADE BY WAYNE P. CASTELLO AND THE DESCRIPTION WAS DERIVED WITHOUT A SURVEY AND NO OPINION OR REPRESENTATION IS BEING MADE EITHER EXPRESSLY OR IMPLIEDLY BY SUCH INDIVIDUAL AS TO THE ACCURACY OF SAID DESCRIPTION.

Pg 1

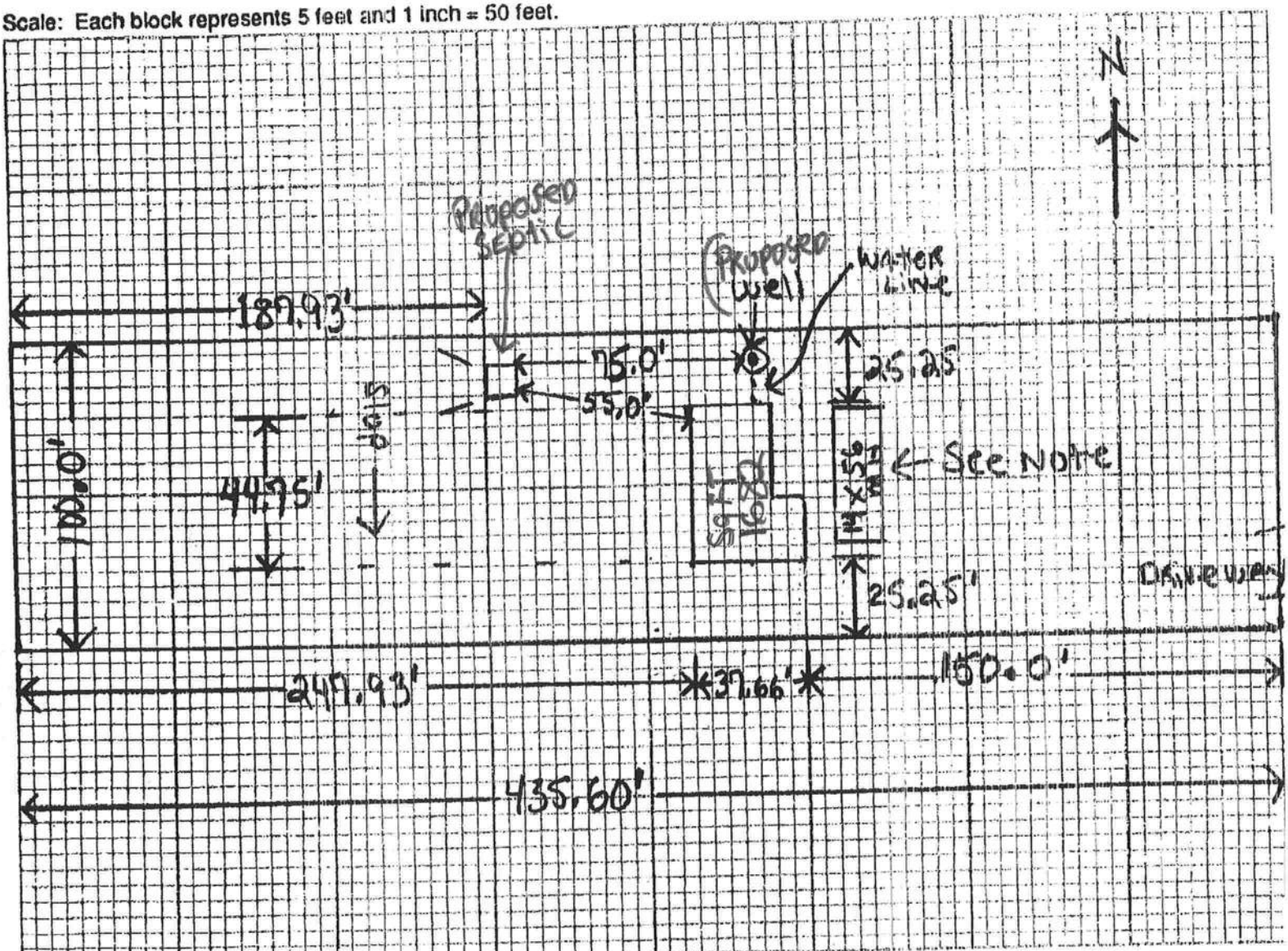
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number _____

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: Mobile Home is To Be Removed AFTER Completion of New Home Construction. Mobile Home Sq Ft. 784+-
See page 2 FOR Additional Information IF Needed

Site Plan submitted by: Roy Lewis _____ Owner
Signature Title

Plan Approved _____ Not Approved _____ Date _____

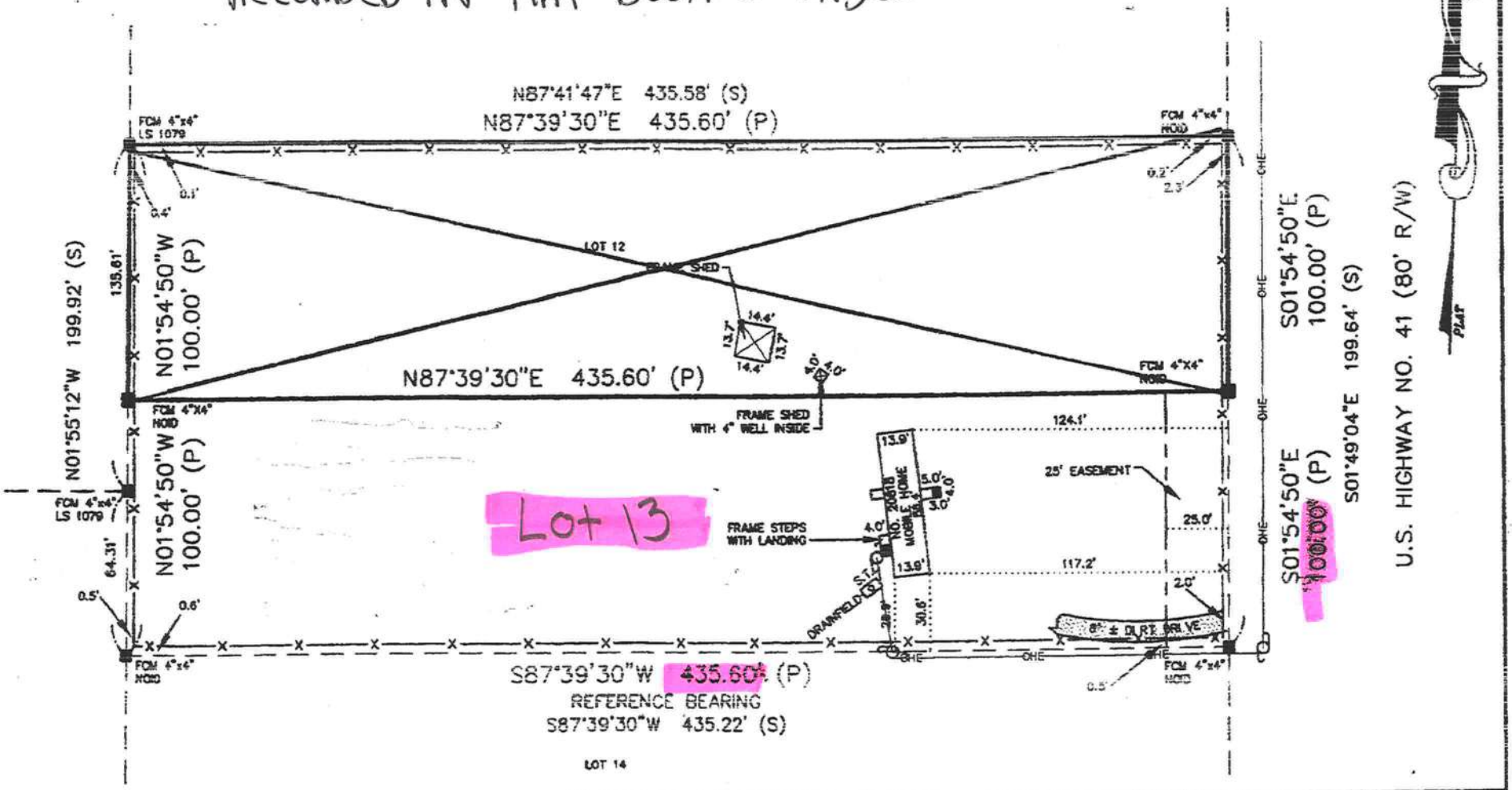
By _____ County Health Departmer

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

MAP OF BOUNDARY SURVEY

Pg 2

Legal = Lot 13 OF Sherwood Forest Unit No. 3
Recorded in Plat Book 5 Page 8



NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

REV:		
	SCALE: 1" = 60'	
	JOB NUMBER	SHEET 1 OF 1

LEGEND

(P) = PLAT
(S) = SURVEY MEASUREMENT
NOID = NO SURVEYORS IDENTIFICATION
LS = LAND SURVEYOR
FCM = FOUND CONCRETE MONUMENT
FIR = FOUND IRON ROD
PRM = PERMANENT REFERENCE MONUMENT
R/W = RIGHT OF WAY
OHE = OVER HEAD ELECTRIC
ST = SEPTIC TANK
WPP = WOOD POWER POLE
X ---- X = WIRE FENCE

OWNER IMPACT FEE OCCUPANCY AFFIDAVIT

STATE OF FLORIDA
COUNTY OF COLUMBIA

BEFORE ME, the undersigned authority, personally appeared _____ ("Owner"), who, after being duly sworn, deposes and says:

1. Except as otherwise stated herein, Affiant has personal knowledge of the facts and matters set forth in this affidavit.

2. Affiant is the owner of the following described real property located in Columbia County, Florida, (herein "the property"):

(a) Parcel No.: 09-7S-17-09957-112
(b) Legal description (may be attached): Sherwood Forest Lot 13

3. Affiant has or will apply to the Columbia County Building Department for a building permit for the replacement of a building or dwelling unit on the property where no additional square footage or dwelling units will be created and will be located on the same property.

4. Either based upon Affiant's personal knowledge or the attached signed written statement of another person, a certificate of occupancy has been issued for the replacement building or dwelling on the property within seven (7) years of the date the previous building or dwelling unit was previously occupied. The building or dwelling unit was last occupied on _____.

5. This affidavit is given for the purpose of obtaining an exemption pursuant to Article VIII, Section 8.01, Columbia County Comprehensive Impact Fee Ordinance No. 2007-40, adopted October 18, 2007, as may be amended.

Further Affiant sayeth naught.

Roy E. Lewis

Print: Roy E. Lewis JR

Address: 20818 S. US Hwy 441
High Spring, Fl
32643

SWORN TO AND SUBSCRIBED before me this 23rd day of July, 2008, by Roy Lewis who is personally known to me or who has produced DL as identification.

Gale Tedder
Notary Public, State of Florida

(NOTARIES SEAL)

My Commission Expires:

GALE TEDDER
MY COMMISSION # DD 805686
EXPIRES: July 14, 2012
Bonded Thru Notary Public Underwriters

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	Masonite	Wood Edge Steel Door	FL4904
2. Sliding	Better Built	Series 940	FL6483
3. Sectional	Overhead Door	Series 180/280/380	FL 674
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung	Better Built	Series 940 Fin	FL7085
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	James Hardie	Hardiplank	FL889-R2
2. Soffits	Alcoa	Aluminum Soffit	FL2641
3. EIFS			
4 Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10 Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	GAF Materials	Sentinal Asphalt Singles	FL183.9
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite: 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspection

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Location

Permit # (FOR STAFF USE ONLY)


ZONE X
0995 T-12

FAX MEMORANDUM

MEMORANDUM

FLORIDA DEPARTMENT OF TRANSPORTATION

To: Mr. John Kerce, Dept. Director **Columbia Co. Building Dept.** **Fax No: 904-758-2160**	**From: Neil E. Miles, FDOT Permits Coor.** **Date: 11-09-06 Fax No. 904-961-7180** **Attention: In-House Staff**

( ) Sign and return. (XX) For your files. ( ) Please call me. ( ) FYI ( ) For Review

Reason for Contact. Property Owner Now wishes to obtain Col. Co. Propt. Improvements and needs local County Building Permit Approval.

The existing driveway connection was inspected on 8-04-08 for acceptance for current access / entrance compliances and was approved for use without and improvements being required.

REF: Notice of Approval of existing Access Point / Inspected On: 8-04-08
PROJECT: Single Family Residential Home
PROPT. OWNER: Roy Eugene Lewis, Jr.
PROPOSED: Access to parcel propt. conn. on SR- 25 South or (US 41/441)
PERMITTEE's MAILING ADDRESS: 20818 South US 41, High Springs, Fl. 32643
COUNTY PARCEL Tax ID No: 09-7S-17-09957-112
Phone #: 352-281-1619 Cell #
FDOT Permit No: NONE REQUIRED FOR THIS SITE
Engineer: N/A

Mr. Kerce or Staff Member:

The above project land parcel have pre-existing legal access to the parcel mentioned above that connects directly to SR-25 South. Please accept this as our notice of same and lift any Columbia County Permit restrictions that may have applied to this described residential parcel that may be due to State FDOT Access Permit Compliances. Please thank all the staff for the continued assistance in these access matters!

If further information is required on this project please do not hesitate to contact this office for additional access permitting information details. My office number is 961-7193 or 961-7180.

Sincerely,
Neil Miles
Access Permits Coordinator
It's great to have folks like you to work with, thanks again for your assistance!

No. of pages faxed: 3

FORM 600A-2004R EnergyGauge® 4.5

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Roy Lewis**	Builder:	**Owner**
Address:		Permitting Office:	Columbia
City, State:	,	Permit Number:	27247
Owner:		Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 30.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 14.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft²)	1230 ft²	13. Heating systems	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		a. Electric Heat Pump	Cap: 30.0 kBtu/hr
a. U-factor:	Description Area		HSPF: 7.80
(or Single or Double DEFAULT)	7a. (Dble Default) 191.0 ft²	b. N/A	
b. SHGC:		c. N/A	
(or Clear or Tint DEFAULT)	7b. (Clear) 191.0 ft²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 50.0 gallons
a. Slab-On-Grade Edge Insulation	R=0.0, 160.0(p) ft		EF: 0.97
b. N/A		b. N/A	
c. N/A		c. Conservation credits	
9. Wall types		(HR-Heat recovery, Solar	
a. Frame, Wood, Exterior	R=13.0, 1000.0 ft²	DHP-Dedicated heat pump)	
b. Frame, Wood, Adjacent	R=13.0, 200.0 ft²	15. HVAC credits	PT, CF,
c. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
d. N/A		HF-Whole house fan,	
e. N/A		PT-Programmable Thermostat,	
10. Ceiling types		MZ-C-Multizone cooling,	
a. Under Attic	R=30.0, 1300.0 ft²	MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Con. AH: Garage	Sup. R=6.5, 100.0 ft		
b. N/A			

Glass/Floor Area: 0.16 Total as-built points: 18723 Total base points: 19715 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____

DATE: 5.14.08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA ★ IN GOD WE TRUST ★

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Summary Energy Code Results

Residential Whole Building Performance Method A

Project Title:
Roy Lewis

Class 3 Rating
Registration No. 0
Climate: North

5/14/2008

Building Loads			
Base		As-Built	
Summer:	**14892 points**	Summer:	**16242 points**
Winter:	**12581 points**	Winter:	**13281 points**
Hot Water:	**7273 points**	Hot Water:	**7273 points**
Total:	**34746 points**	Total:	**36796 points**

Energy Use			
Base		As-Built	
Cooling:	**4840 points**	Cooling:	**4413 points**
Heating:	**6970 points**	Heating:	**6812 points**
Hot Water:	**7905 points**	Hot Water:	**7498 points**
Total:	**19715 points**	Total:	**18723 points**

PASS
e-Ratio: 0.95

EnergyGauge®(Version: FLRCSB v4.5)

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

1.	New construction or existing	New			
2.	Single family or multi-family	Single family			
3.	Number of units, if multi-family	1			
4.	Number of Bedrooms	3			
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1230 ft²			
7.	Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area			
	(or Single or Double DEFAULT)	7a. (Dble Default) 191.0 ft²			
	b. SHGC:				
	(or Clear or Tint DEFAULT)	7b. (Clear) 191.0 ft²			
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 160.0(p) ft			
	b. N/A				
	c. N/A				
9.	Wall types				
	a. Frame, Wood, Exterior	R=13.0, 1000.0 ft²			
	b. Frame, Wood, Adjacent	R=13.0, 200.0 ft²			
	c. N/A				
	d. N/A				
	e. N/A				
10.	Ceiling types				
	a. Under Attic	R=30.0, 1300.0 ft²			
	b. N/A				
	c. N/A				
11.	Ducts				
	a. Sup: Unc. Ret: Con. AH: Garage	Sup. R=6.5, 100.0 ft			
	b. N/A				
12.	Cooling systems				
	a. Central Unit		Cap: 30.0 kBtu/hr		
			SEER: 14.00		
	b. N/A				
	c. N/A				
13.	Heating systems				
	a. Electric Heat Pump		Cap: 30.0 kBtu/hr		
			HSPF: 7.80		
	b. N/A				
	c. N/A				
14.	Hot water systems				
	a. Electric Resistance		Cap: 50.0 gallons		
			EF: 0.97		
	b. N/A				
	c. Conservation credits				
	(HR-Heat recovery, Solar DHP-Dedicated heat pump)				
15.	HVAC credits		PT, CF,		
	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)				

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1230.0	18.59	4116.0	1.Double, Clear	S	1.5	3.0	52.0	35.87	0.66	1230.0	
				2.Double, Clear	W	1.5	6.0	30.0	38.52	0.91	1055.0	
				3.Double, Clear	W	1.5	6.0	20.0	38.52	0.91	703.0	
				4.Double, Clear	E	1.5	6.0	75.0	42.06	0.91	2879.0	
				5.Double, Clear	E	1.5	3.0	9.0	42.06	0.73	274.0	
				6.Double, Clear	E	1.5	3.0	5.0	42.06	0.73	152.0	
				As-Built Total:				**191.0**			**6293.0**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	200.0	0.70	140.0	1. Frame, Wood, Exterior			13.0	1000.0	1.50		1500.0	
Exterior	1000.0	1.70	1700.0	2. Frame, Wood, Adjacent			13.0	200.0	0.60		120.0	
Base Total:	**1200.0**		**1840.0**	**As-Built Total:**				**1200.0**			**1620.0**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	20.0	2.40	48.0	1.Exterior Insulated				20.0	4.10		81.9	
Exterior	20.0	6.10	121.9	2.Adjacent Insulated				20.0	1.60		32.0	
Base Total:	**40.0**		**169.8**	**As-Built Total:**				**40.0**			**113.9**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1230.0	1.73	2127.9	1. Under Attic			30.0	1300.0	1.73 X 1.00		2249.0	
Base Total:	**1230.0**		**2127.9**	**As-Built Total:**				**1300.0**			**2249.0**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	160.0(p)	-37.0	-5920.0	1. Slab-On-Grade Edge Insulation			0.0	160.0(p	-41.20		-6592.0	
Raised	0.0	0.00	0.0									
Base Total:			**-5920.0**	**As-Built Total:**				**160.0**			**-6592.0**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1230.0	10.21	12558.3					1230.0	10.21		12558.3	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 14892.0			**Summer As-Built Points:**					**16242.2**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 30000btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Gar(AH),R6.5(INS)					
			16242	1.00	(1.08 x 1.147 x 1.00)	0.244	0.902	4413.5
14892.0	**0.3250**	**4839.9**	**16242.2**	**1.00**	**1.235**	**0.244**	**0.902**	**4413.5**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1230.0	20.17	4466.0	1.Double, Clear	S	1.5	3.0	52.0	13.30	1.64	1133.0	
				2.Double, Clear	W	1.5	6.0	30.0	20.73	1.02	636.0	
				3.Double, Clear	W	1.5	6.0	20.0	20.73	1.02	424.0	
				4.Double, Clear	E	1.5	6.0	75.0	18.79	1.04	1459.0	
				5.Double, Clear	E	1.5	3.0	9.0	18.79	1.12	189.0	
				6.Double, Clear	E	1.5	3.0	5.0	18.79	1.12	105.0	
				As-Built Total:				**191.0**			**3946.0**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	200.0	3.60	720.0	1. Frame, Wood, Exterior			13.0	1000.0	3.40		3400.0	
Exterior	1000.0	3.70	3700.0	2. Frame, Wood, Adjacent			13.0	200.0	3.30		660.0	
Base Total:	**1200.0**		**4420.0**	**As-Built Total:**				**1200.0**			**4060.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	20.0	11.50	229.8	1.Exterior Insulated				20.0	8.40		167.8	
Exterior	20.0	12.30	245.8	2.Adjacent Insulated				20.0	8.00		159.8	
Base Total:	**40.0**		**475.5**	**As-Built Total:**				**40.0**			**327.7**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1230.0	2.05	2521.5	1. Under Attic			30.0	1300.0	2.05 X	1.00	2665.0	
Base Total:	**1230.0**		**2521.5**	**As-Built Total:**				**1300.0**			**2665.0**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	160.0(p)	8.9	1424.0	1. Slab-On-Grade Edge Insulation			0.0	160.0(p	18.80		3008.0	
Raised	0.0	0.00	0.0									
Base Total:			**1424.0**	**As-Built Total:**				**160.0**			**3008.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1230.0	-0.59	-725.7					1230.0	-0.59		-725.7	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12581.3**	**Winter As-Built Points:**					**13281.0**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 30000 btuh ,EFF(7.8) Ducts:Unc(S),Con(R),Gar(AH),R6.5					
			13281.0	1.000	(1.056 x 1.169 x 1.00)	0.437	0.950	6812.4
12581.3	**0.5540**	**6970.1**	**13281.0**	**1.00**	**1.235**	**0.437**	**0.950**	**6812.4**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2635.00	7905.0	50.0	0.97	3	1.00	2499.18	1.00	7497.5	
				As-Built Total:						**7497.5**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
4840	**6970**	**7905**	**19715**	**4413**	**6812**	**7498**	**18723**

PASS

RESIDENTIAL HEATING AND COOLING REQUIREMENTS*

HVAC WORKSHEET FOR WATT-WISE LIVING

HEATING AND COOLING REQUIREMENTS DUE TO GLASS AREA

WINDOWS & GLASS DOORS	AREA SQUARE FEET	HEATING MULTIPLIER (CIRCLE ONE) DESIGN TEMPERATURE DIFFERENCE 30°	35°	40°	45°	50°	HEATING (BTUH LOSS)
Glass Doors, Infiltration less than 1.0 CFM/FT							
Single Glass		50	60	70	75	85	
Double Glass	20	40	45	(50)	55	60	1000
Other Sliding Glass Doors							
Single Glass		75	85	100	115	125	
Double Glass	33.33	60	70	(80)	90	100	2666
Windows, Infiltration less than 0.50 CFM/FT							
Single Glass		40	50	55	60	70	
Double Glass	109	25	30	(35)	40	45	3815
Windows, Infiltration less than 0.75 CFM/FT							
Single Glass		45	50	60	65	75	
Double Glass		30	35	40	45	50	
Other Windows							
Single Glass		75	90	105	115	130	
Double Glass		60	70	80	90	105	
Fixed or Picture Windows							
Single Glass		40	50	55	60	70	
Double Glass		25	30	35	40	45	
Other							
Total BTUH Loss (Enter on Line 2, Page 2)							7481

WINDOWS & GLASS DOORS	AREA SQUARE FEET	COOLING MULTIPLIER (CIRCLE) SINGLE GLASS 90° C	T	R	95° C	T	R	DOUBLE GLASS 90° C	T	R	95° C	T	R	COOLING (BTUH GAIN)
No Shading														
N		30	22	20	30	26	25	20	14	13	25	17	16	
NE & NW		60	41	36	65	45	41	50	29	24	50	32	27	
E & W		85	60	53	90	64	57	70	44	36	75	47	39	
SE & SW		75	51	45	80	55	50	60	37	30	65	40	33	
S		45	31	28	50	35	33	35	21	18	40	24	21	
Draperies or Blinds														
N	5	20	17	16	25	21	20	15	11	11	(20)	14	14	100
NE & NW		35	33	30	40	37	34	30	22	21	35	25	24	
E & W	157	55	48	43	55	52	47	45	32	30	(50)	35	33	7850
SE & SW		45	39	35	50	43	39	40	26	25	40	29	28	
S		30	26	24	30	30	28	25	17	16	25	20	19	
Roller Shades														
N		25	19	17	25	23	22	20	12	11	20	15	14	
NE & NW		45	36	32	50	40	37	40	26	22	45	29	25	
E & W		65	53	47	70	57	51	55	37	32	60	40	35	
SE & SW		56	44	39	60	48	44	50	32	27	50	36	30	
S		35	28	25	40	32	30	30	20	16	35	23	19	
Awnings, Porches, Etc.														
All Directions		25	22	20	30	26	25	15	14	13	20	17	16	
Other														
Total BTUH Gain (Line 2, Page 2)														7850

*REFERENCE A.C.C.A. MANUAL "J"

(C - Clear T - Tinted R - Reflective)

Prepared By: Harry's Htg + Air

TOTAL HEATING AND COOLING REQUIREMENTS

Page 2

352-281-1619 cell
386-454-1584

For:

Name: Roy Lewis

Address:

City:

(✓) Check Constr. Type	ITEM	AREA SQUARE FEET	DESIGN TEMPERATURE DIFFERENCE 30°	35°	40°	45°	50°	HEATING (BTUH LOSS)	DESIGN TEMP 90°	95°	COOLING MULT. (CIRCLE)	COOLING (BTUH GAIN)
	Gross Wall Area	992										
	Glass Area (From page 1)	162						7481				7950
	Partitions, Frame											
	Finished 1 side, No Insulation		17	19	22	25	28		6.5	10.0		
	Finished 2 sides, No Insulation		9	11	12	14	16		4.5	6.0		
	Finished 2 sides, R-5		4	5	5.5	6	7		2.5	3.5		
	Finished 2 sides, R-11	288	2	3	(3)	4	4	864	2.0	(2.5)		720
	Other											
	Doors (Excluding glass)											
	No weatherstripping		135	160	180	200	225		10.0	13.0		
	Weatherstripped		70	85	95	110	120		10.0	13.0		
	R-5 Insulation, No weatherstripping		123	144	164	185	205		4.3	5.5		
	R-5 Insulation, weatherstripping	20	68	79	(90)	101	113	1800	4.0	(5.0)		100
	Other											
	Net Exterior Walls											
	CBS Furred, No Insulation		9	10	12	13	14		4.5	6.0		
	CBS Furred, R-3 Insulation		5	6	7	8	8		3.0	4.2		
	CBS Furred, R-4 Insulation		4	5	6	6	7		2.7	3.8		
	CBS Furred, R-5 Insulation		4	5	5	6	6		2.5	3.5		
	Frame, No Insulation		8	9	10	11	13		5.5	7.0		
	Frame, R-11 Insulation	830	2	2	(3)	3	4	2490	2.5	(3.0)		2490
	Frame, R-14 Insulation		1.5	1.7	2	2.5	3		2	2.8		
	Other											
	Ceiling under attic (Roof DK / LT)											
	No Insulation DK LT		18	21	24	27	30		9 / 7	10 / 8.5		
	R-11 Insulation DK LT		2.4	2.8	3.2	3.5	3.9		2.5 / 2	3 / 2.5		
	R-19 Insulation DK LT		1.5	1.7	1.9	2.2	2.4		1.5 / 1.5	2 / 1.5		
	R-22 Insulation DK LT		1.2	1.5	1.7	1.9	2.1		1.5 / 1.0	1.5 / 1.5		
	R-26 Insulation DK LT		1.1	1.3	1.4	1.6	1.8		1.3 / 1	1.5 / 1.2		
	R-30 Insulation (DK) LT	1230	1	1.1	(1.3)	1.4	1.6	1599	1.1 / .9	(1.3) / 1.0		1599
	Other											
	Floor, Concrete Slab	Perimeter Ft.										
	No Edge Insulation	124	35	40	(40)	45	45	4960	0	0		
	Other											
	Subtotal							19194				12859
	People @ 300 & Appl. @ 1200											5100
	Sensible BTUH Gain											
	Duct BTUH Loss & Gain							19194				17959
	2 In. Flex. or 1 In. Rigid				.10			1919		.10		1796
	1½ In. Rigid				.075					.075		
	Total BTUH Loss							21113				
	Subtotal BTUH Gain											19755
	x 1.3 = Total BTUH Gain											25682

Calculated Heating Requirements 21113 BTUH
Size of Unit Chosen . . . . . 30,000 BTUH
% Oversized . . . . . .
% Undersized . . . . . .

Calculated Cooling Requirements 25682 BTUH
Size of Unit Chosen . . . . . 30,000 BTUH
% Oversized . . . . . .
% Undersized . . . . . .

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TGI8228Z0208100234

Truss Fabricator:	Anderson Truss Company
Job Identification:	8-109--OWNER BUILDER Roy Lewis -- , **
Truss Count:	23
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.35, 7.24.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Notes:

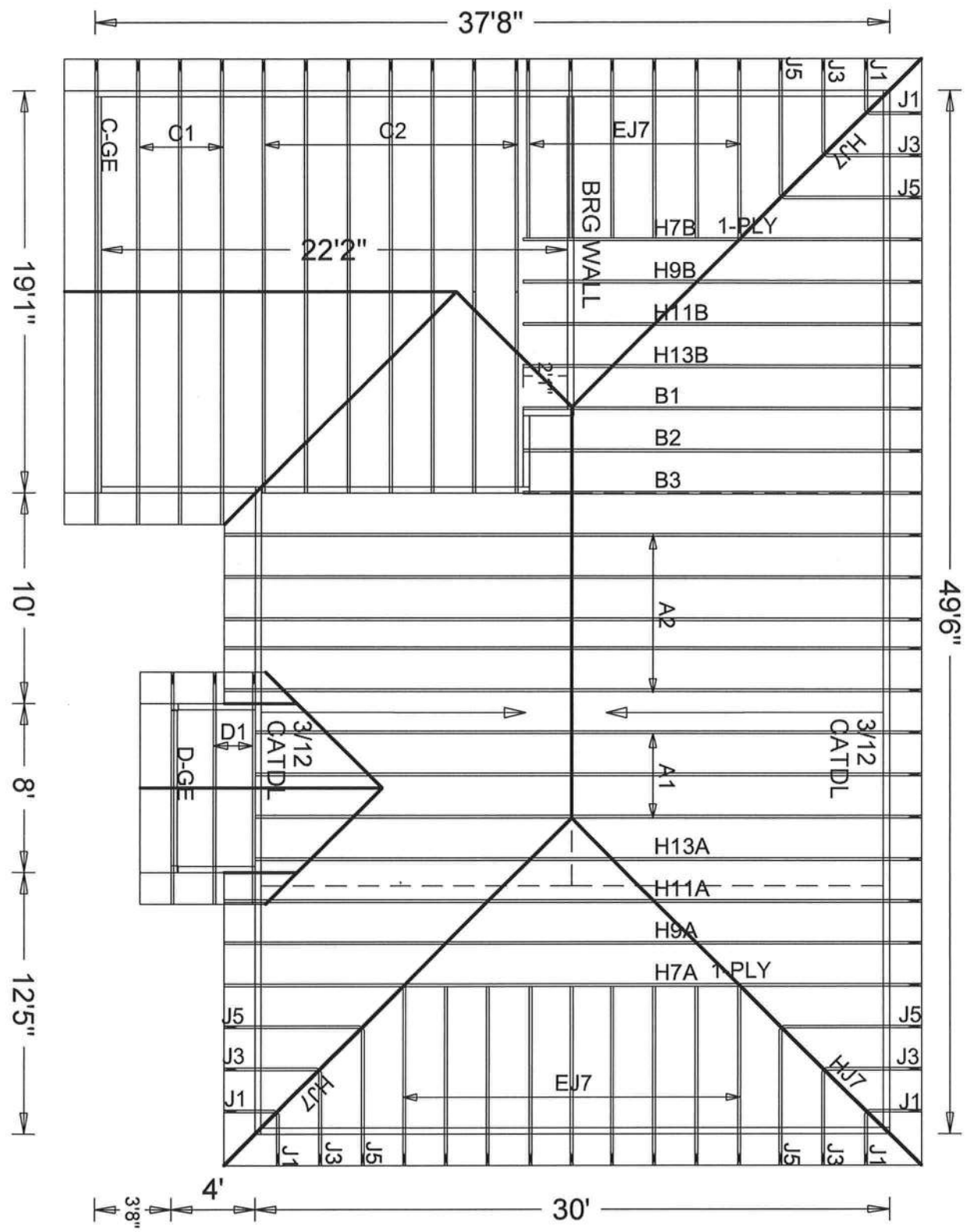
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Seal Date: 04/08/2008

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-A11015EE-GBLLETIN-A13015EE-

#	Ref	Description	Drawing#	Date
1	80470	--A1	08099023	04/08/08
2	80471	--A2	08099024	04/08/08
3	80472	--H13A	08099003	04/08/08
4	80473	--H7A	08099004	04/08/08
5	80474	--H9A	08099005	04/08/08
6	80475	--H11A	08099006	04/08/08
7	80476	--H7B	08099007	04/08/08
8	80477	--H9B	08099008	04/08/08
9	80478	--H11B	08099009	04/08/08
10	80479	--H13B	08099010	04/08/08
11	80480	--B1	08099011	04/08/08
12	80481	--B2	08099012	04/08/08
13	80482	--B3	08099013	04/08/08
14	80483	--C-GE	08099015	04/08/08
15	80484	--C1	08099014	04/08/08
16	80485	--C2	08099016	04/08/08
17	80486	--D-GE	08099017	04/08/08
18	80487	--D1	08099018	04/08/08
19	80488	--HJ7	08099019	04/08/08
20	80489	--J5	08099020	04/08/08
21	80490	--J3	08099021	04/08/08
22	80491	--J1	08099022	04/08/08
23	80492	--EJ7	08099002	04/08/08



#8-109
ROY LEWIS
04/08/08

Roof Plane Sheathing Area = 2185 sq. ft
Gable Sheathing Area = 63 sq. ft
Total Sheathing Area = 2248 sq. ft
Fascia Material = 205 linear ft
Valley Flashing Material = 45 linear ft
Ridge Cap Material = 50 linear ft
Hip Ridge Material = 82 linear ft

JOB DESCRIPTION:: OWNER BUILDER
/: Roy Lewis

JOB NO: 8-109

PAGE NO: 1 OF 1

(8-109--OWNER BUILDER Roy Lewis -- , ** - A1)

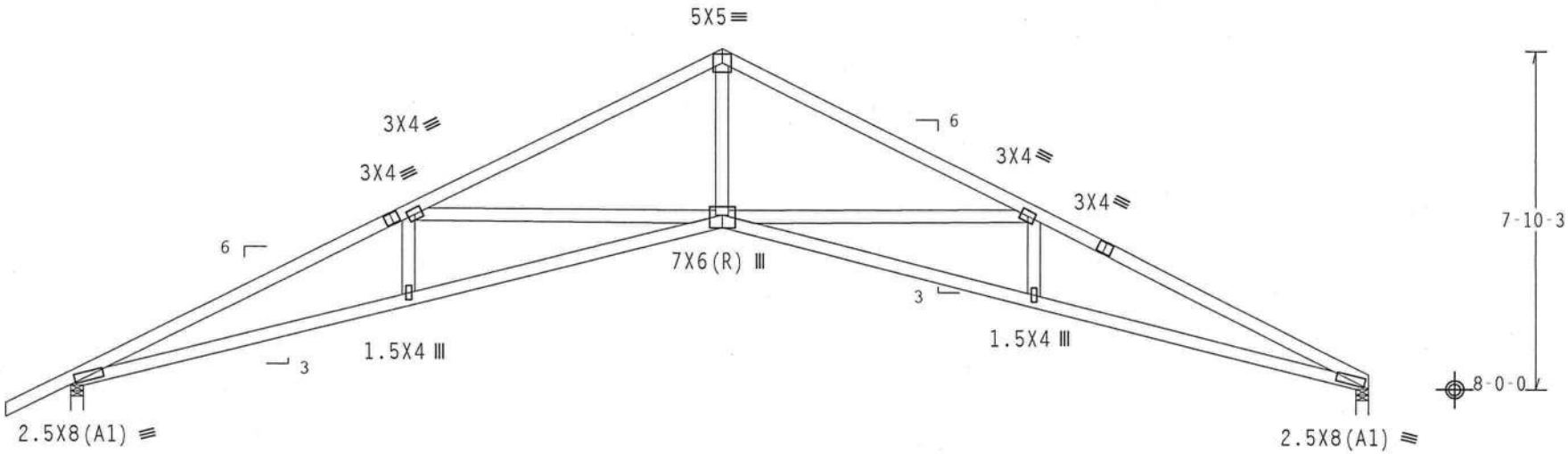
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

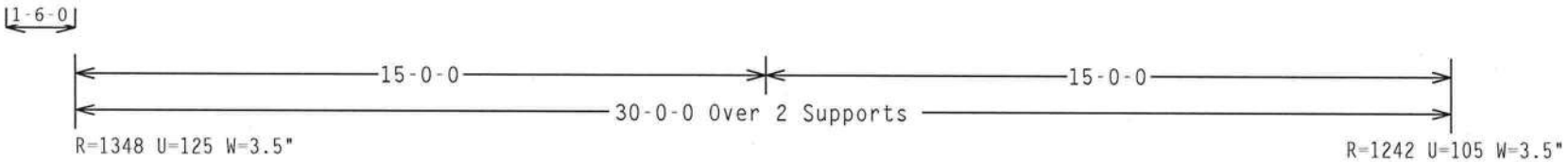
Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.22" due to live load and 0.35" due to dead load.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.35.03 QTY:3 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF	R8228- 80470
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099023
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	20686 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - A2)

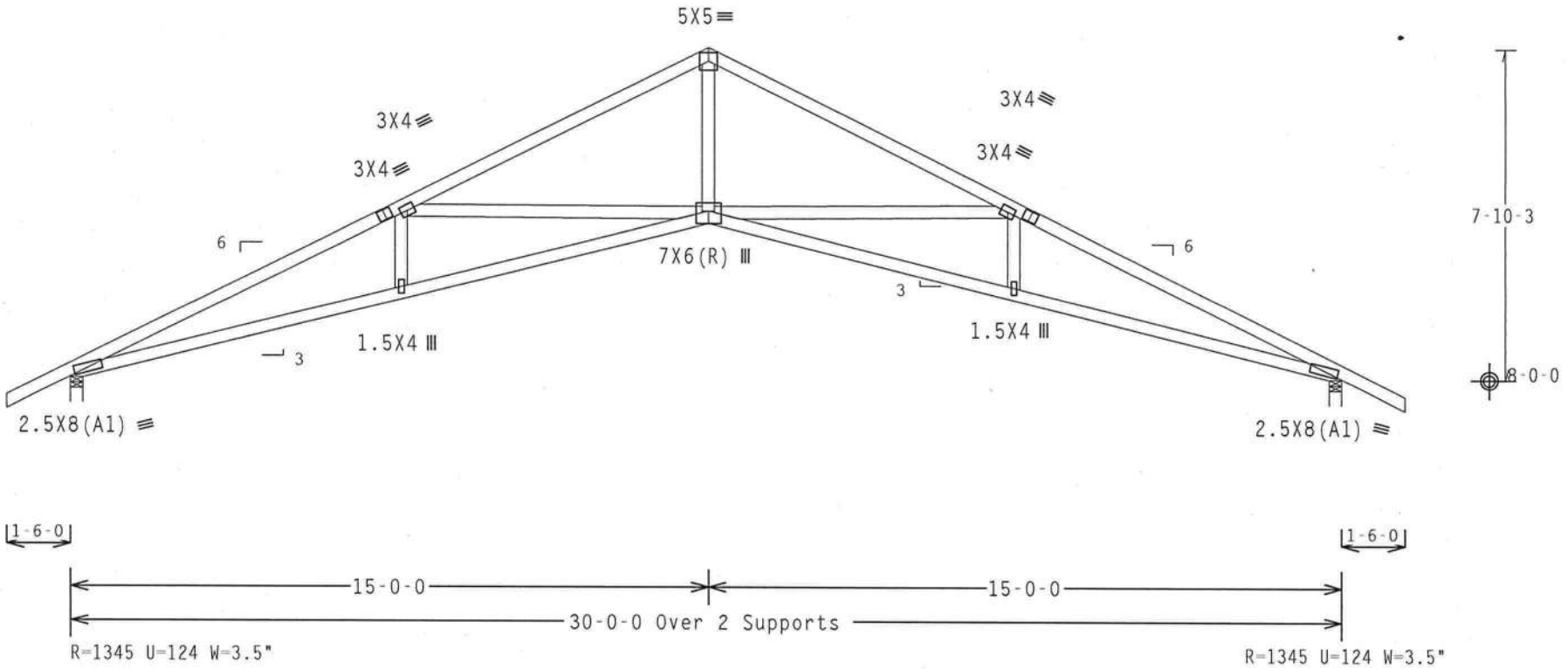
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Calculated horizontal deflection is 0.22" due to live load and 0.35" due to dead load.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.35.0316 QTY:5 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80471
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099024
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 20689 REV
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H13A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

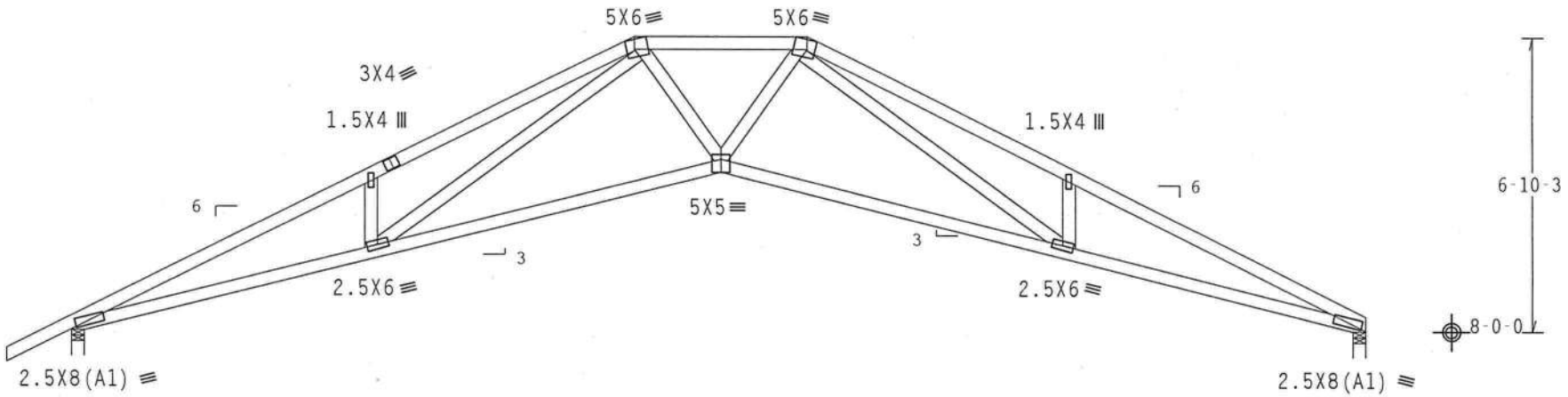
Calculated horizontal deflection is 0.22" due to live load and 0.34" due to dead load.

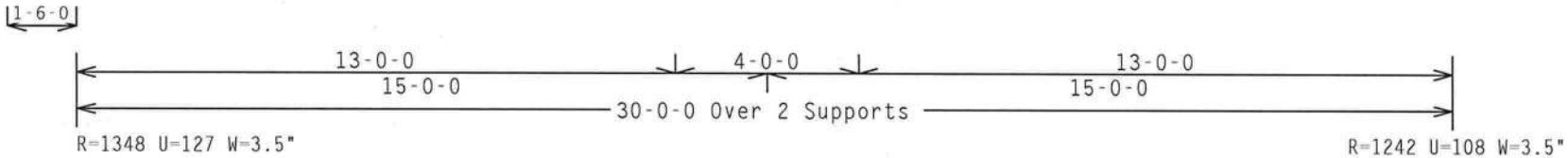
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF R8228- 80472
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099003
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 168824
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

Top chord 2x4 SP #2 Dense :T2 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

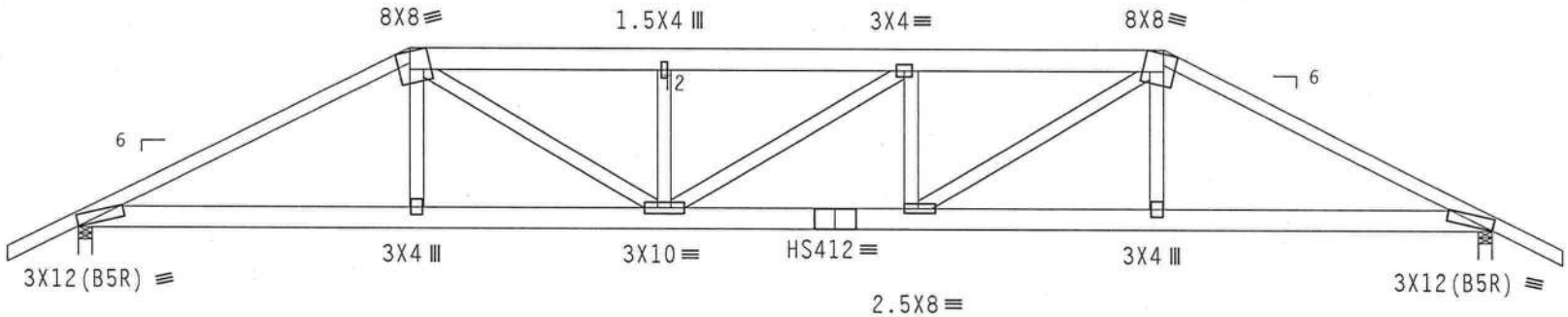
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

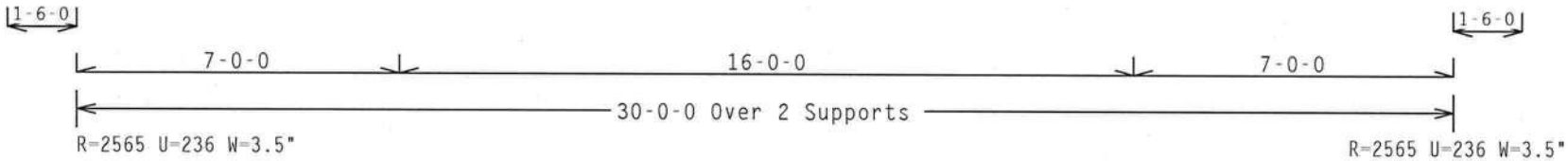
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF	R8228- 80473
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099004
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168770
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H9A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

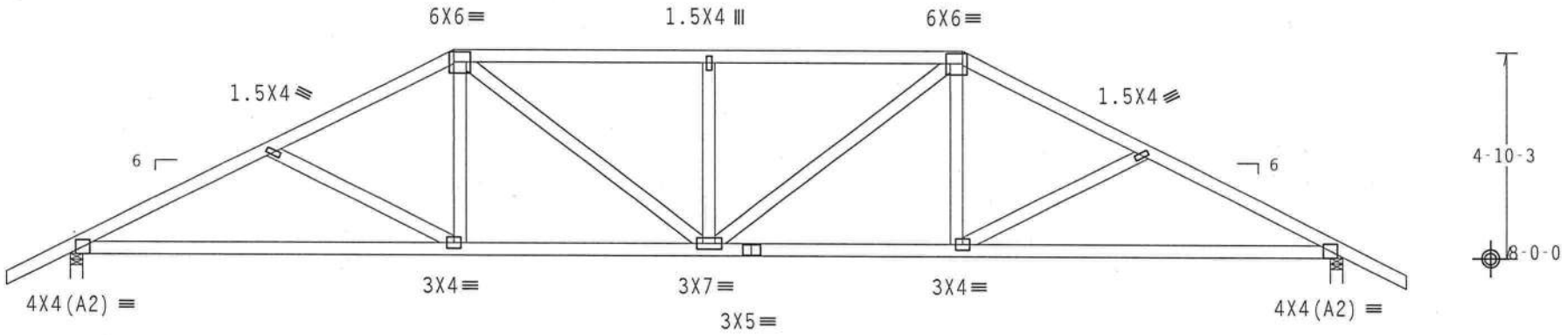
Roof overhang supports 2.00 psf soffit load.

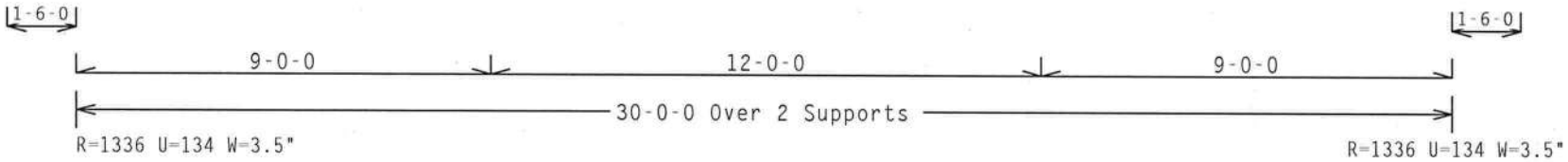
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123

QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF	R8228- 80474
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099005
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	168775
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H11A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

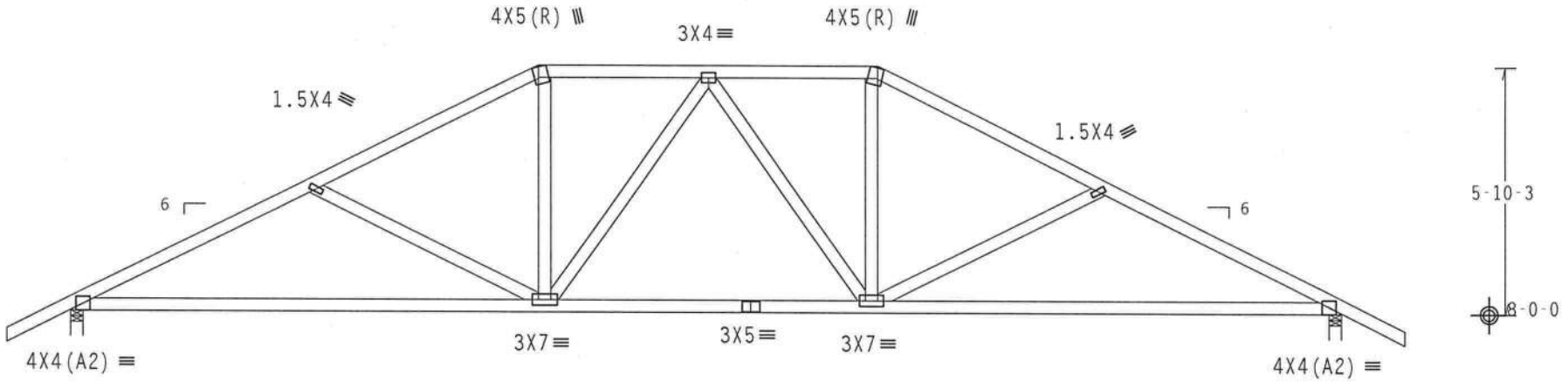
Roof overhang supports 2.00 psf soffit load.

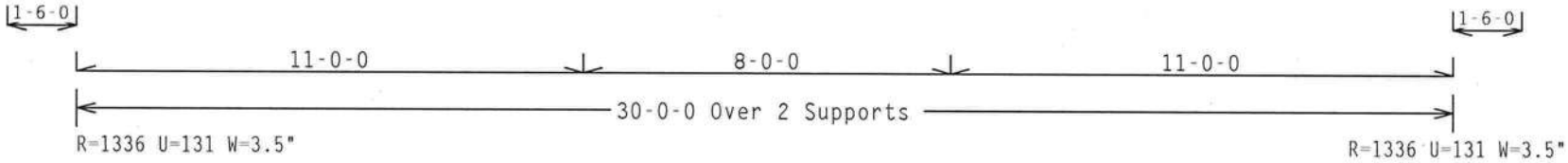
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF	R8228- 80475
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099006
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	168780
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H7B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

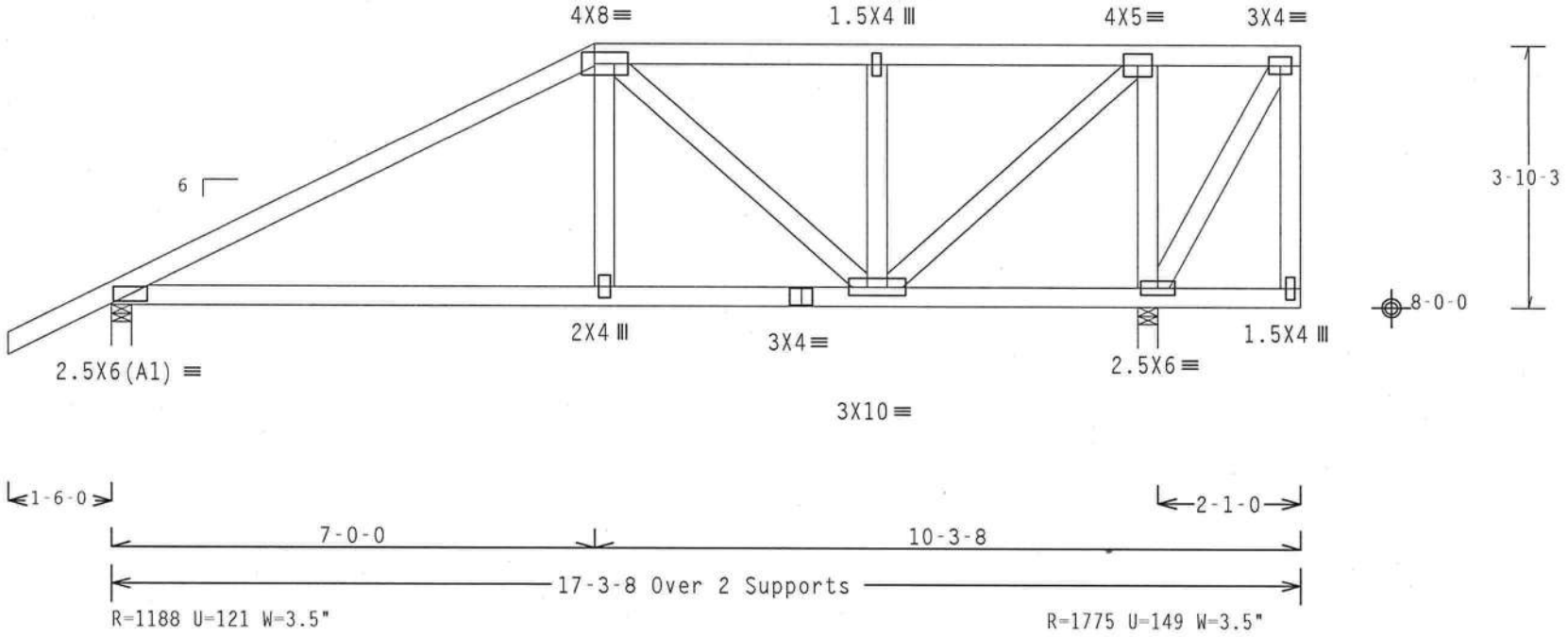
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123[illegible]

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80476
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099007
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 168787
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H9B)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

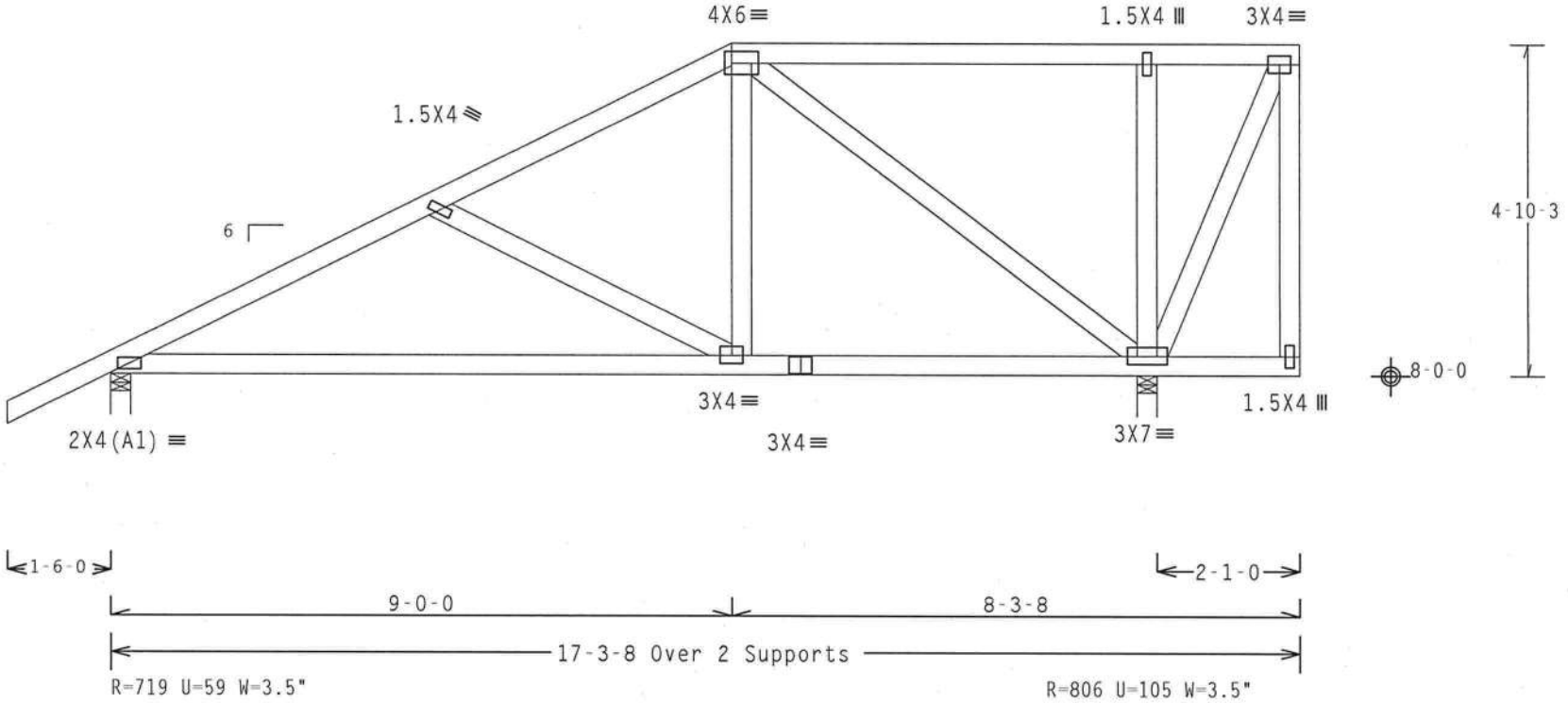
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.1230

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	20.0 PSF	REF R8228- 80477
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099008
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 168792
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H11B)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

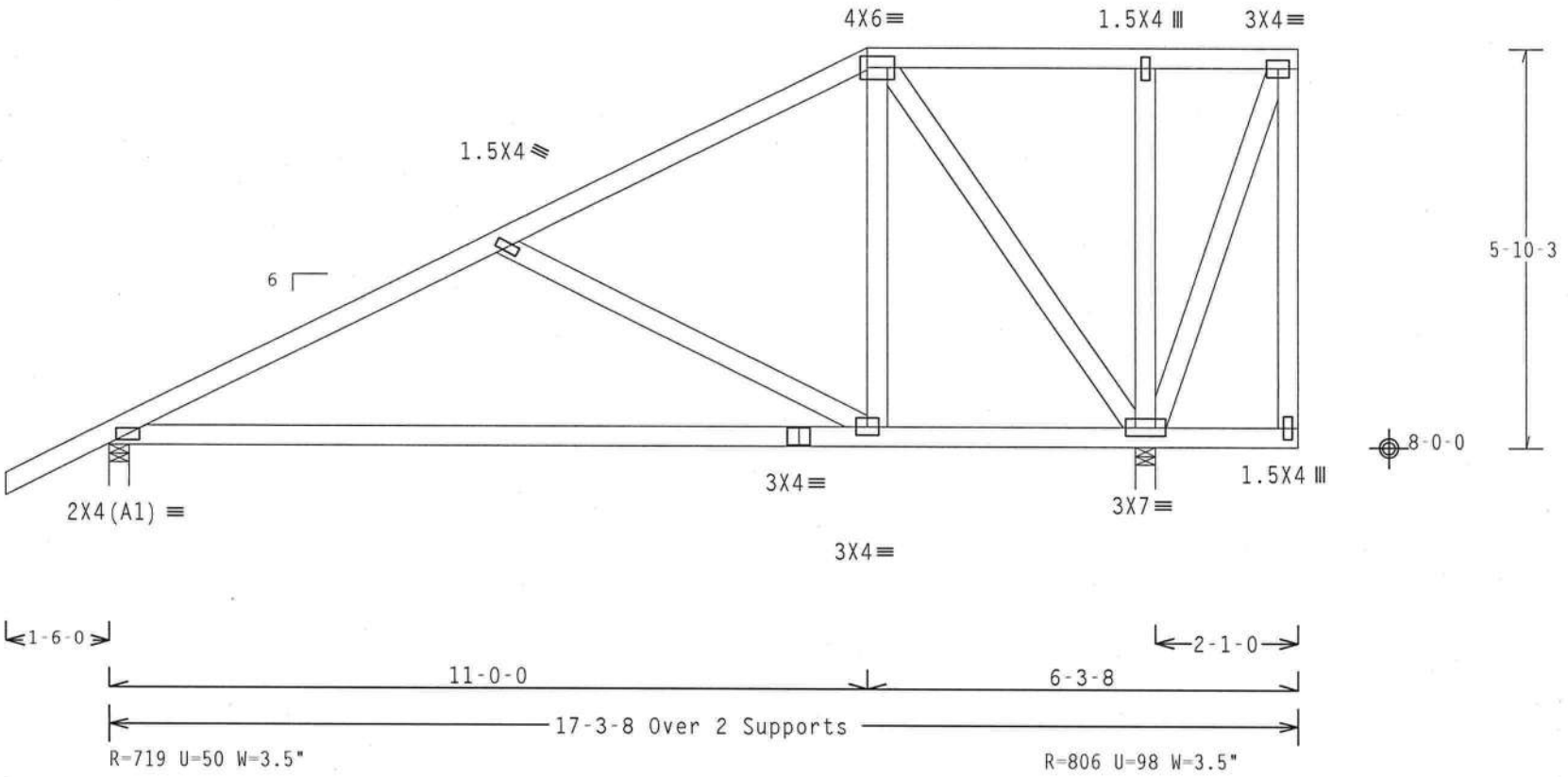
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80478
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099009
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 168797
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - H13B)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

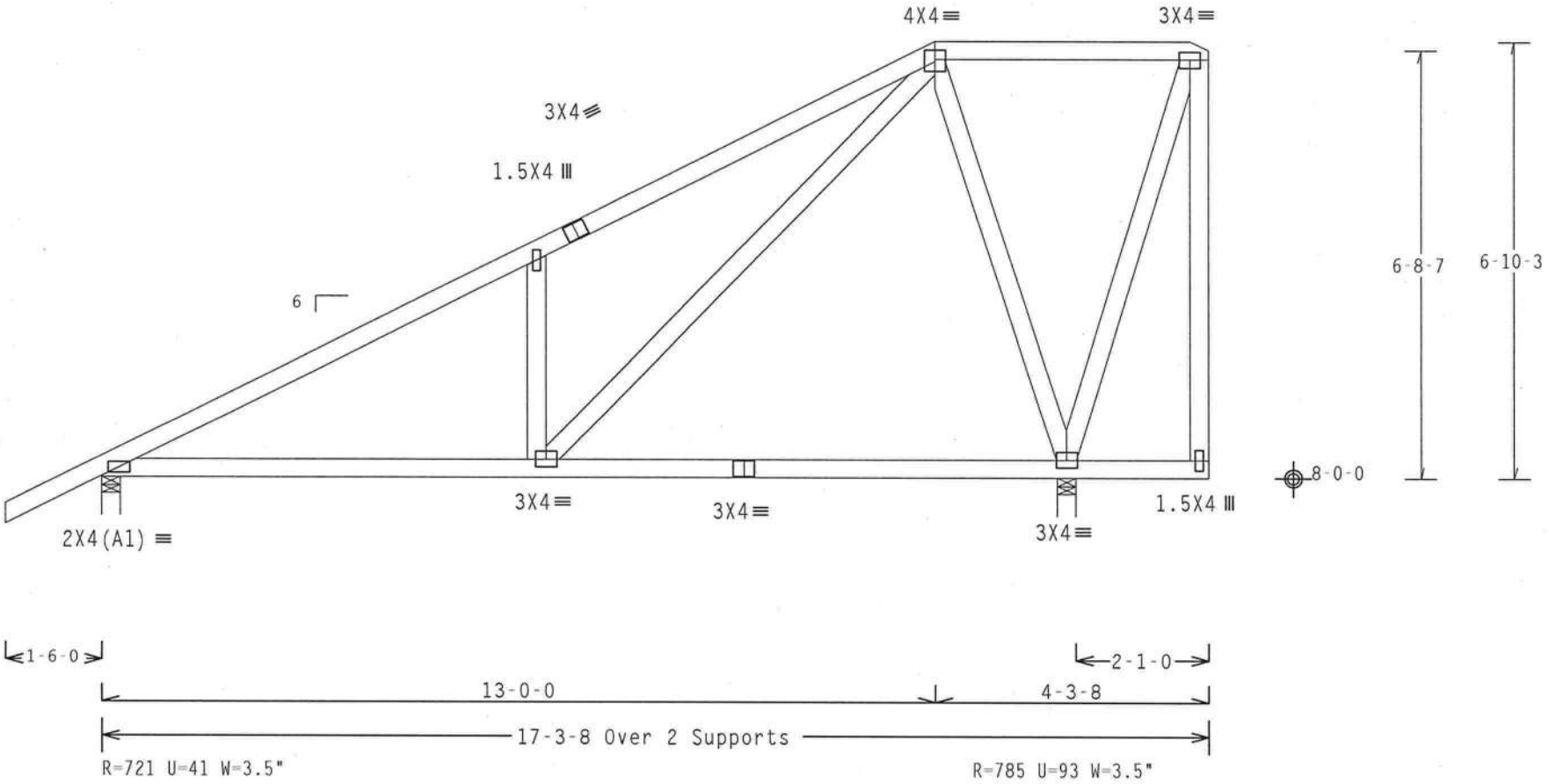
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80479
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099010
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 168802
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - B1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

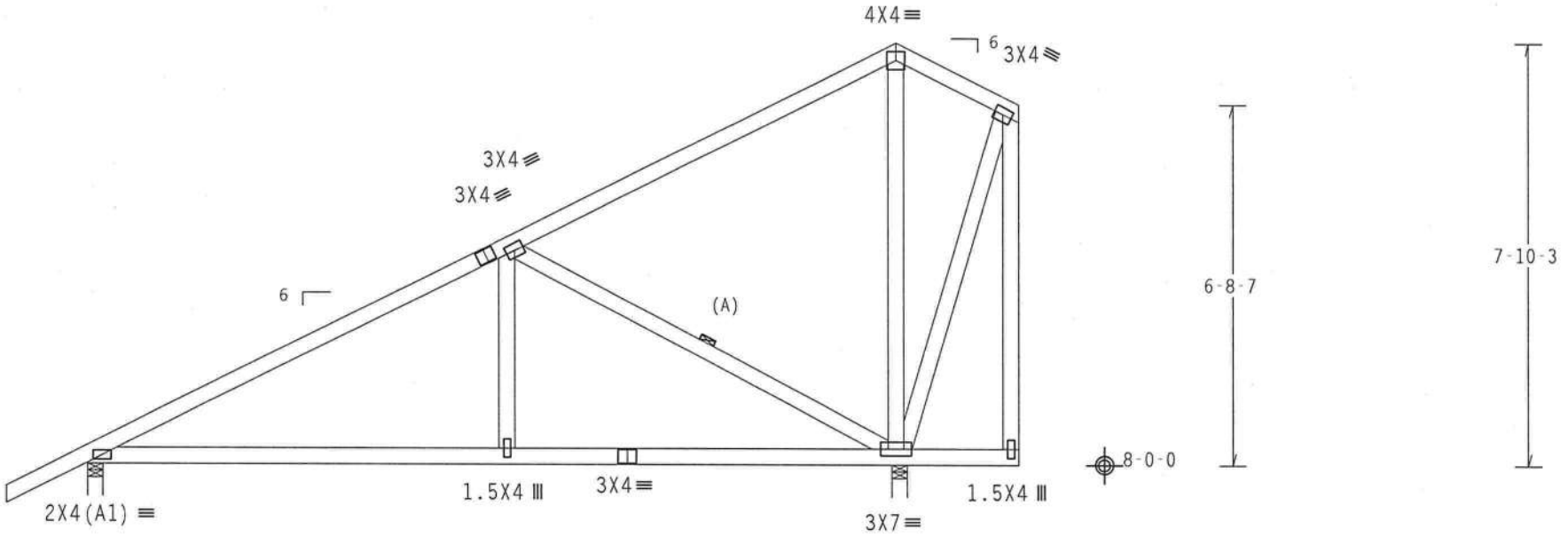
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/0(0) 7.24.123
QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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TC LL	20.0 PSF	REF	R8228- 80480
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099011
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	168809
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

08 '08

(8-109--OWNER BUILDER Roy Lewis -- , ** - B2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

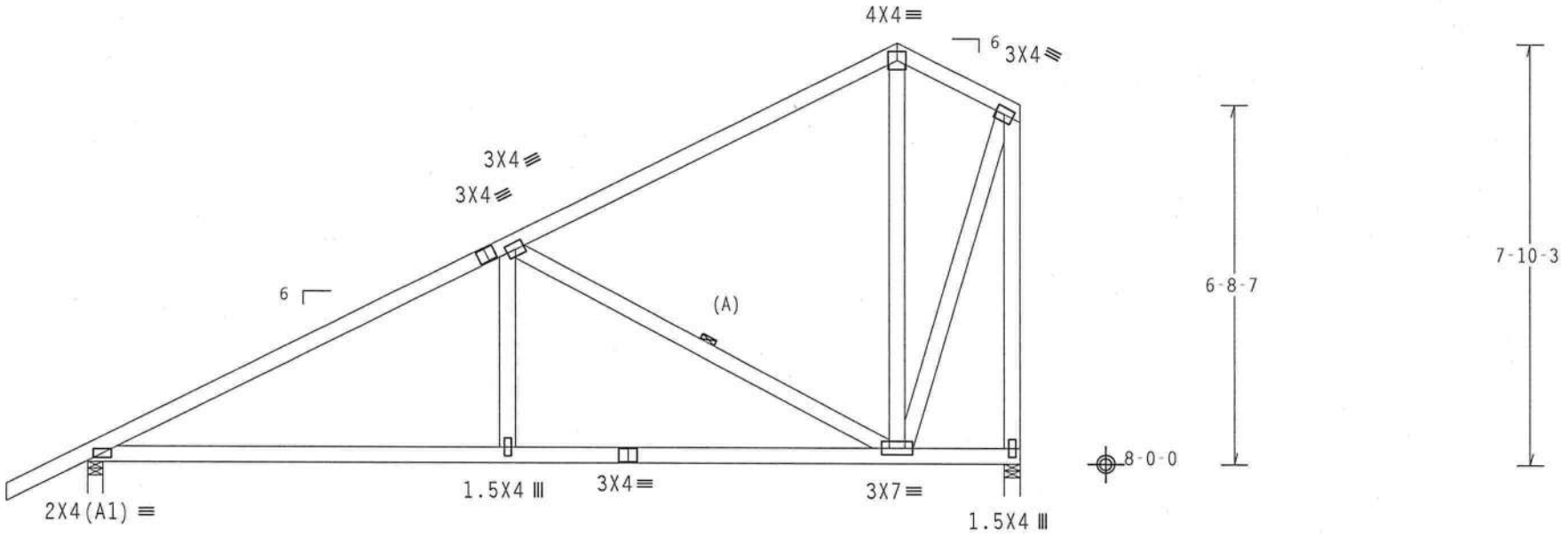
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF	R8228- 80481
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099012
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	168817
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - B3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

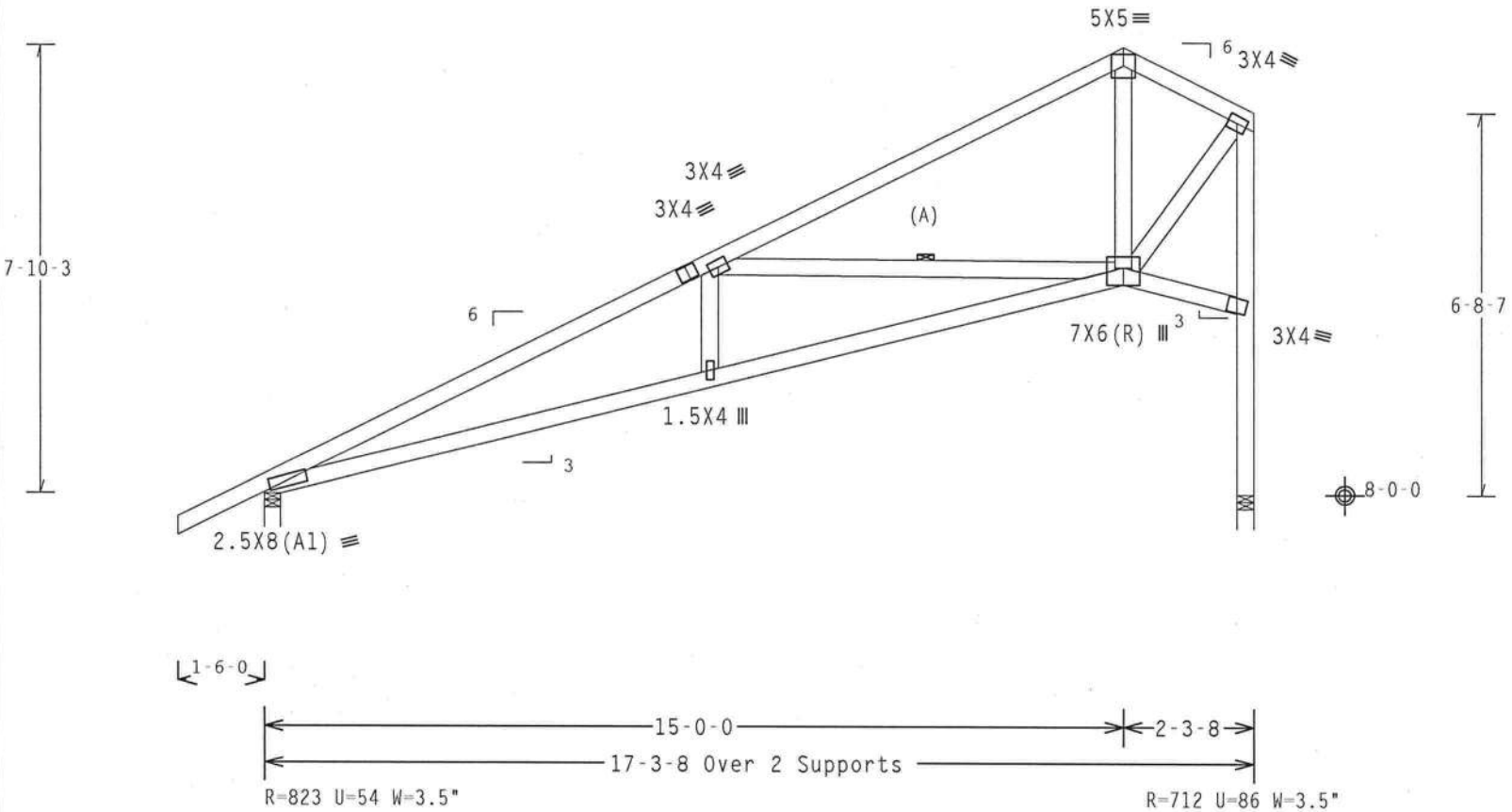
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Leg-down designed for vertical loads only.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12.80

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 08 '08

TC LL	20.0 PSF	REF	R8228- 80482
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099013
BC LL	0.0 PSF		HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	168843
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - C-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

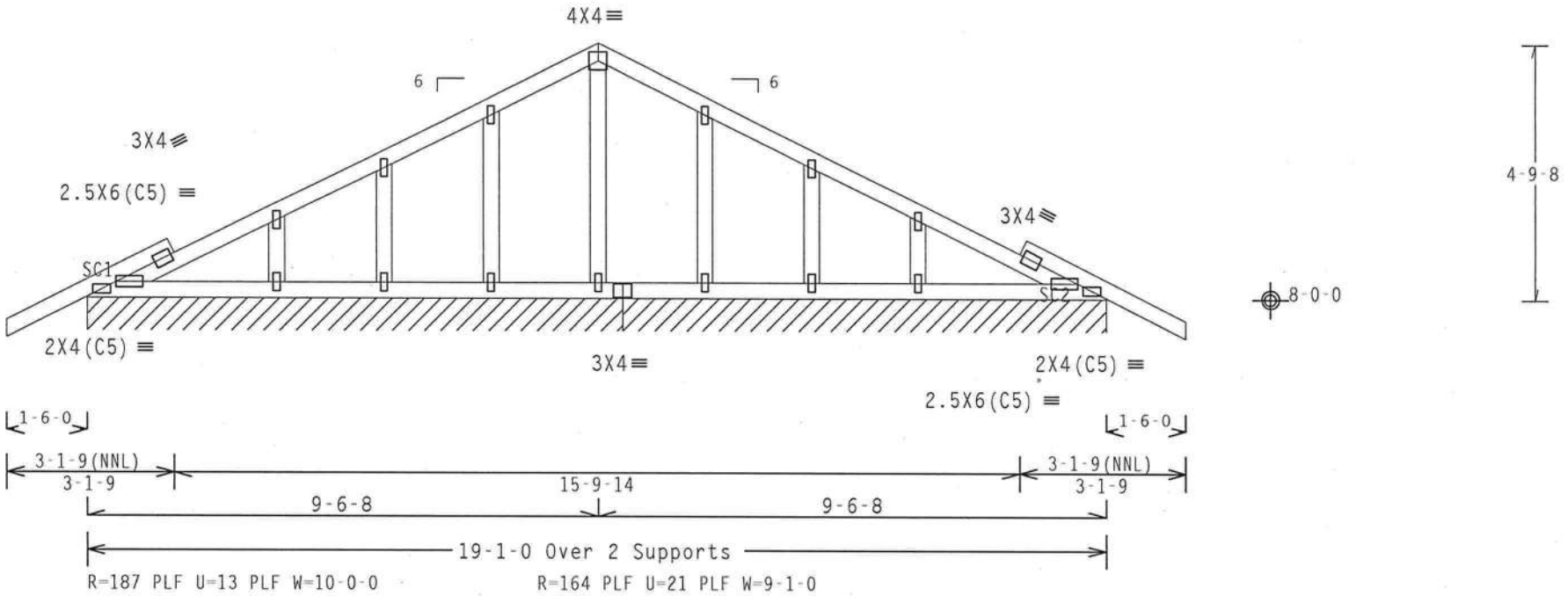
The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.1230 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80483
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099015
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 168849
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - C1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

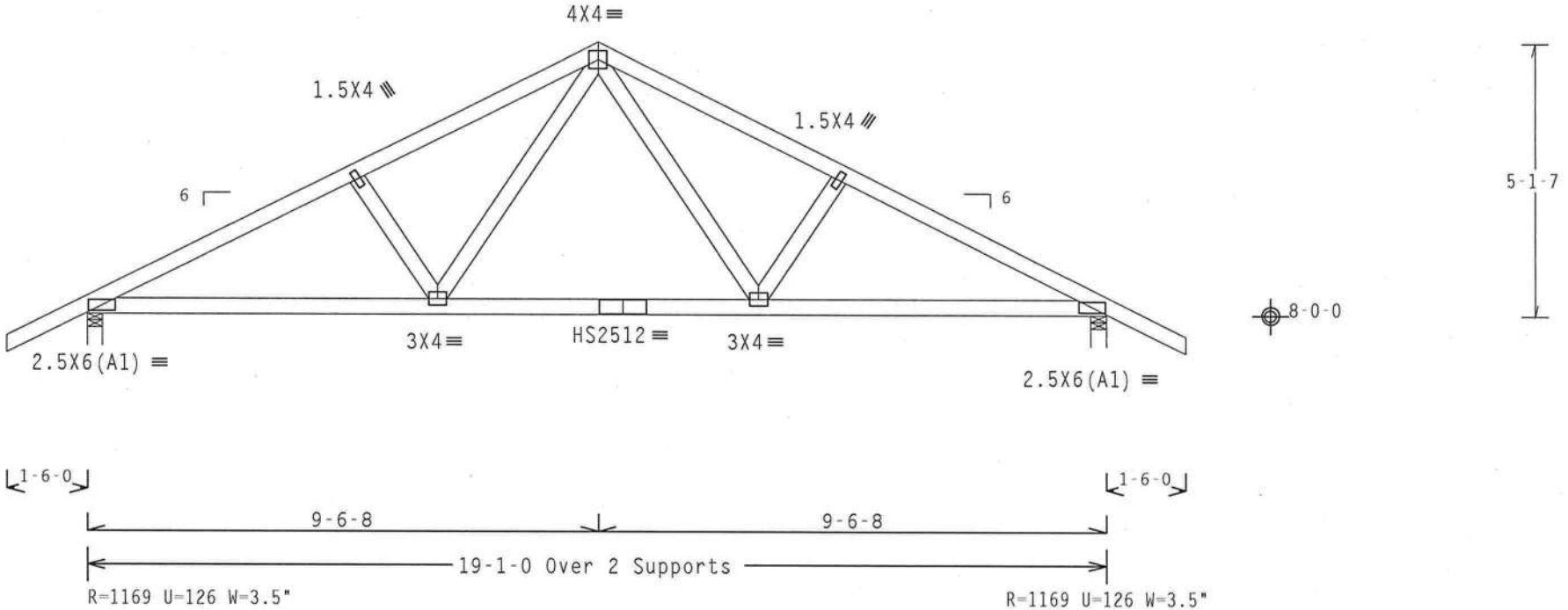
TC - From	62 PLF	at	-1.50	to	62 PLF	at 9.54
TC - From	62 PLF	at	9.54	to	62 PLF	at 20.58
BC - From	4 PLF	at	-1.50	to	4 PLF	at 0.00
BC - From	20 PLF	at	0.00	to	20 PLF	at 6.72
BC - From	120 PLF	at	6.72	to	120 PLF	at 12.37
BC - From	20 PLF	at	12.37	to	20 PLF	at 19.08
BC - From	4 PLF	at	19.08	to	4 PLF	at 20.58

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.24.123 QTY:3 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF	R8228- 80484
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099014
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168735
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - C2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

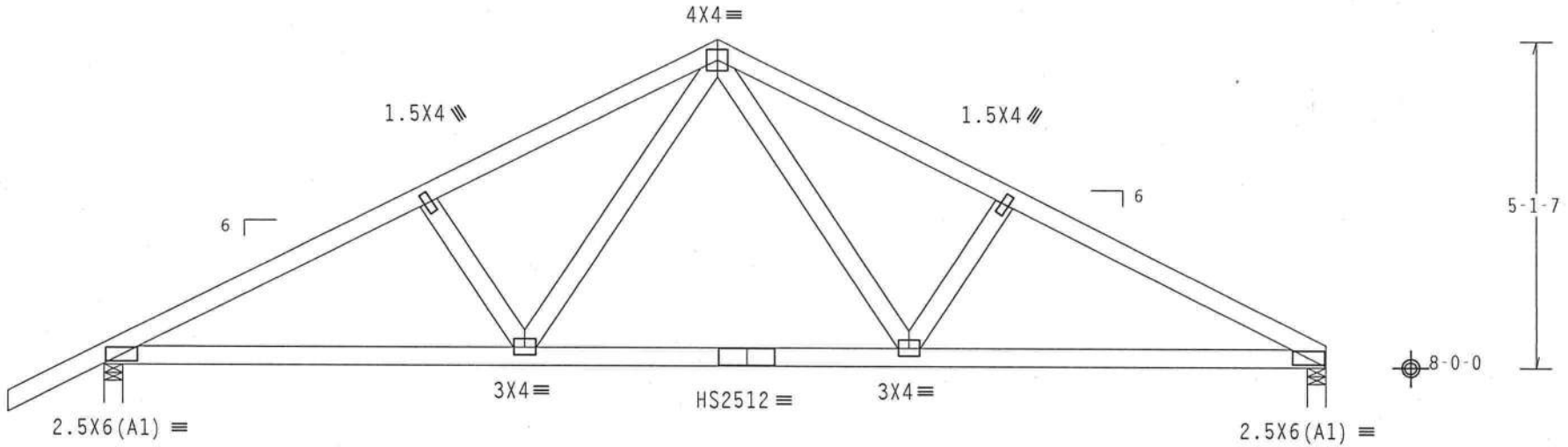
TC - From	62 PLF	at	-1.50	to	62 PLF	at 9.54
TC - From	62 PLF	at	9.54	to	62 PLF	at 19.08
BC - From	4 PLF	at	-1.50	to	4 PLF	at 0.00
BC - From	20 PLF	at	0.00	to	20 PLF	at 6.72
BC - From	120 PLF	at	6.72	to	120 PLF	at 12.37
BC - From	20 PLF	at	12.37	to	20 PLF	at 19.08

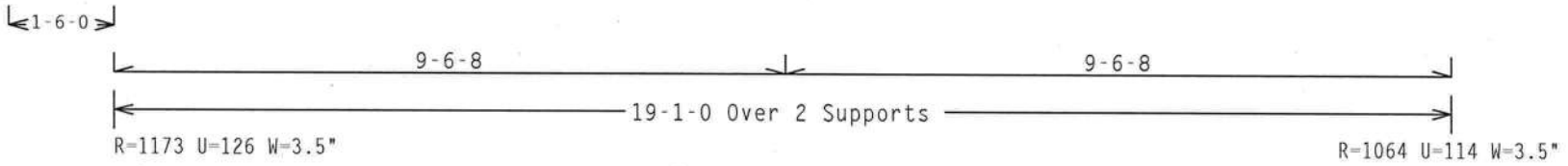
Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:7 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF	R8228- 80485
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099016
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168740
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - D-GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:
:Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

See DWGS A13015EE0207 & GBLLETIN0207 for more requirements.

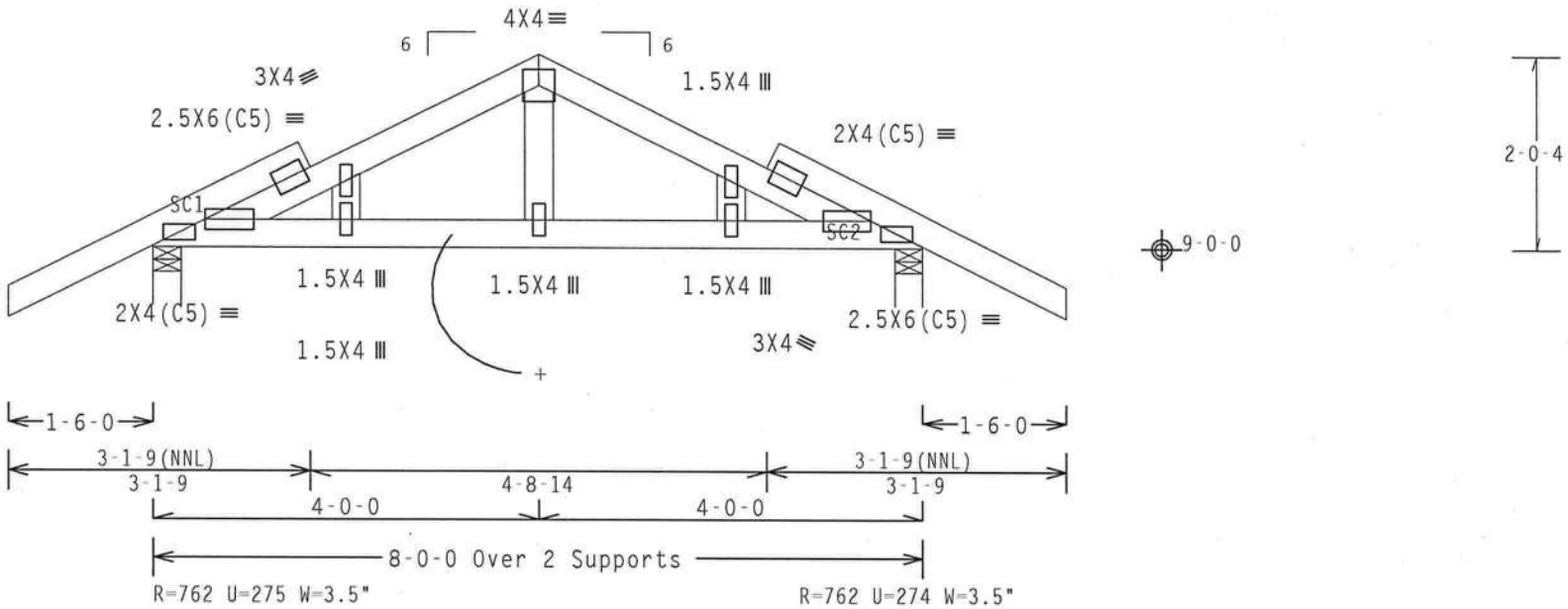
+ Member to be laterally braced for horizontal wind loads. Bracing system to be designed and furnished by others.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The Building Designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the Building Designer.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80486
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099017
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 168730
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - D1)

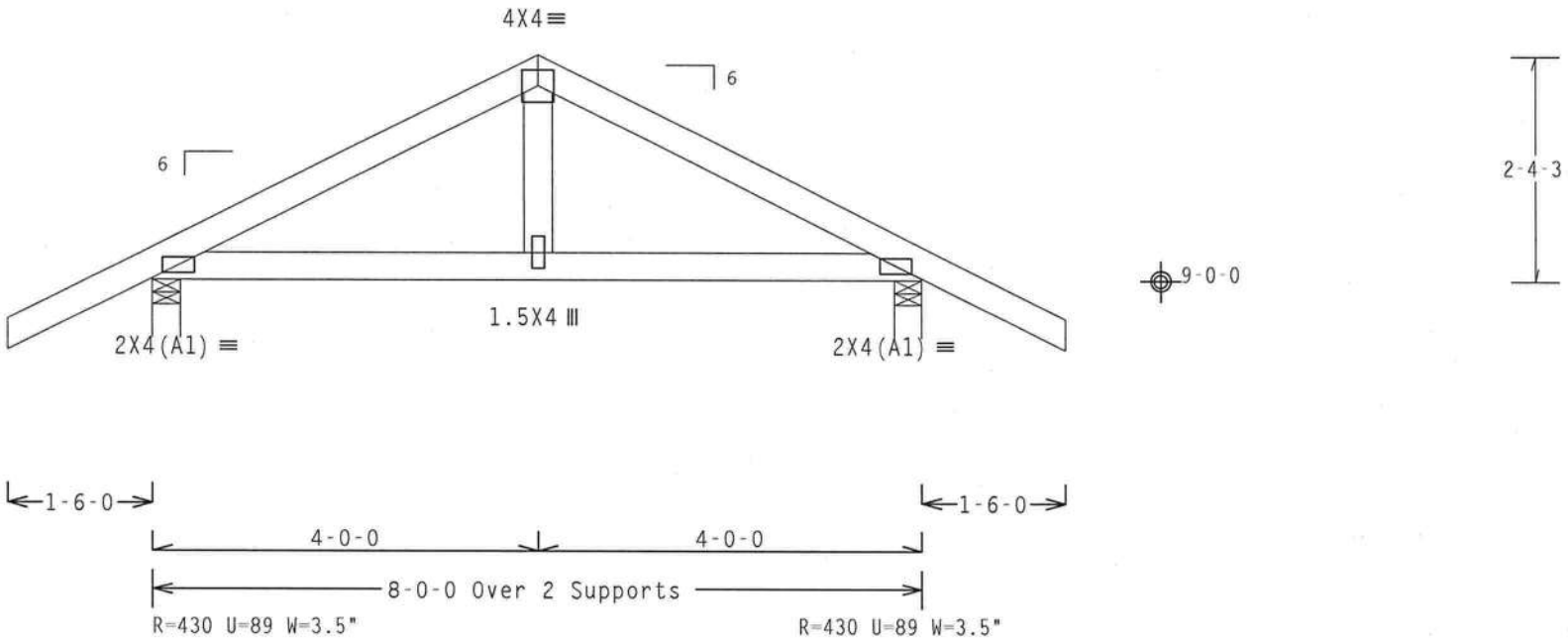
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.12

PLT TYP. Wave QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80487
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099018
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 168722
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - HJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

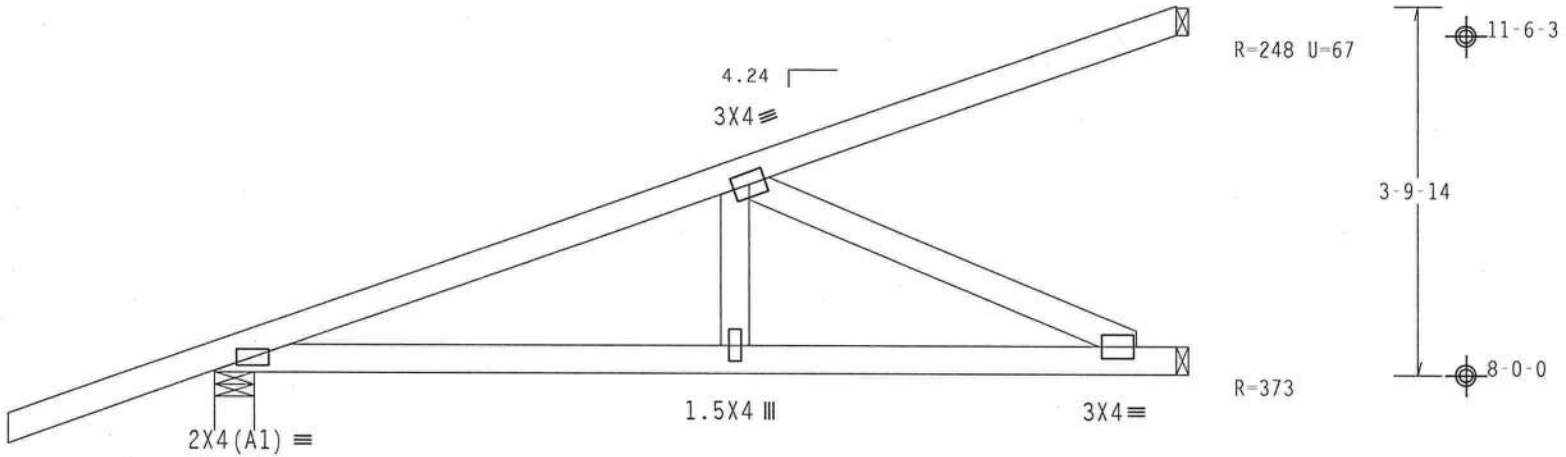
Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (3) 0.162x3.5" 16d Common toe-nails at Bottom Chord.



2-1-7

9-10-13 Over 3 Supports

R=461 U=55 W=4.95"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123

PLT TYP. Wave QTY:3 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF R8228- 80488
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099019
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 168765
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

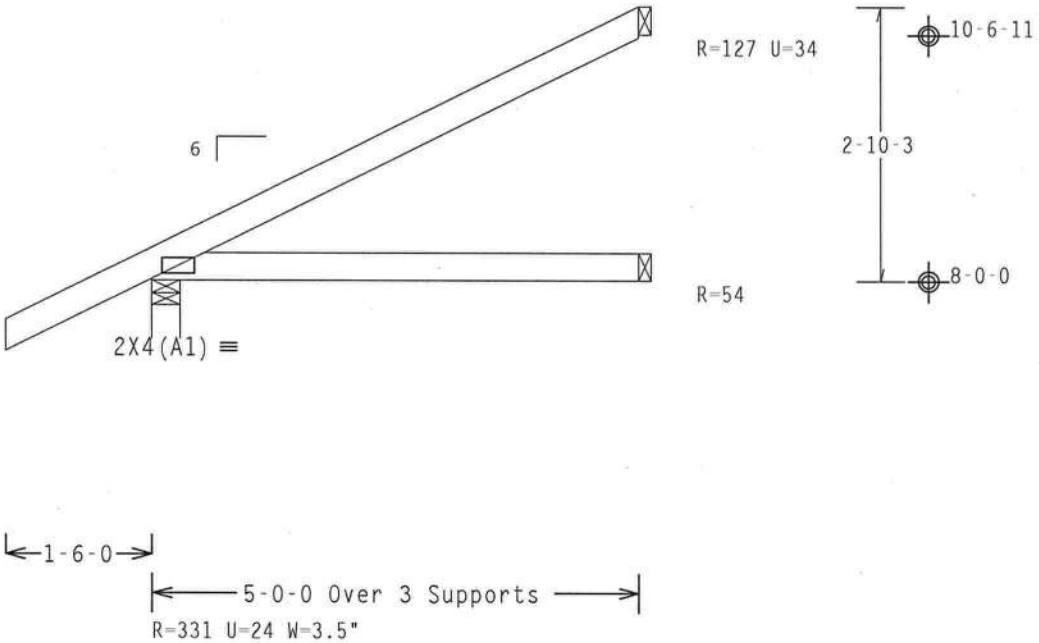
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
Provide (2) 0.162x3.5" 16d Common toe-nails at Bottom Chord.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.122 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF	R8228- 80489
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099020
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168750
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - J3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
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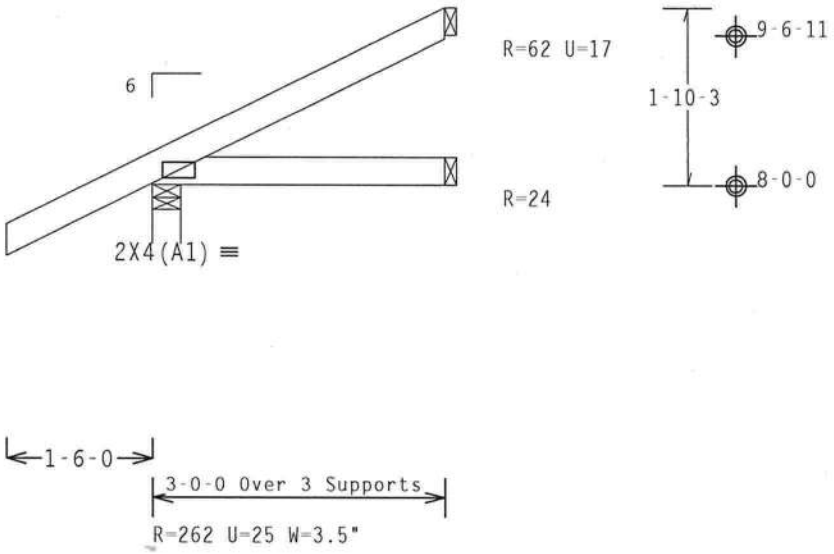

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12 QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

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ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF	R8228- 80490
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099021
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168755
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Provide (2) 0.162x3.5" 16d Common toe-nails at Top Chord.
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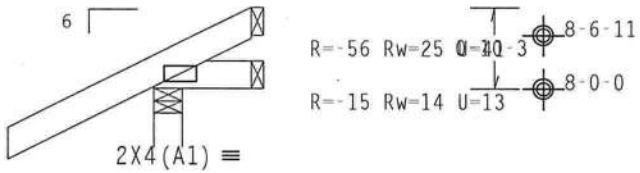

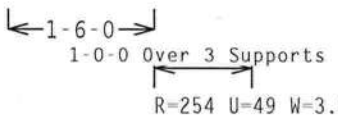

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.24.12[illegible] QTY:6 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
08 '08

TC LL	20.0 PSF	REF R8228- 80491
TC DL	10.0 PSF	DATE 04/08/08
BC DL	10.0 PSF	DRW HCUSR8228 08099022
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 168759
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGI8228Z02

(8-109--OWNER BUILDER Roy Lewis -- , ** - EJ7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

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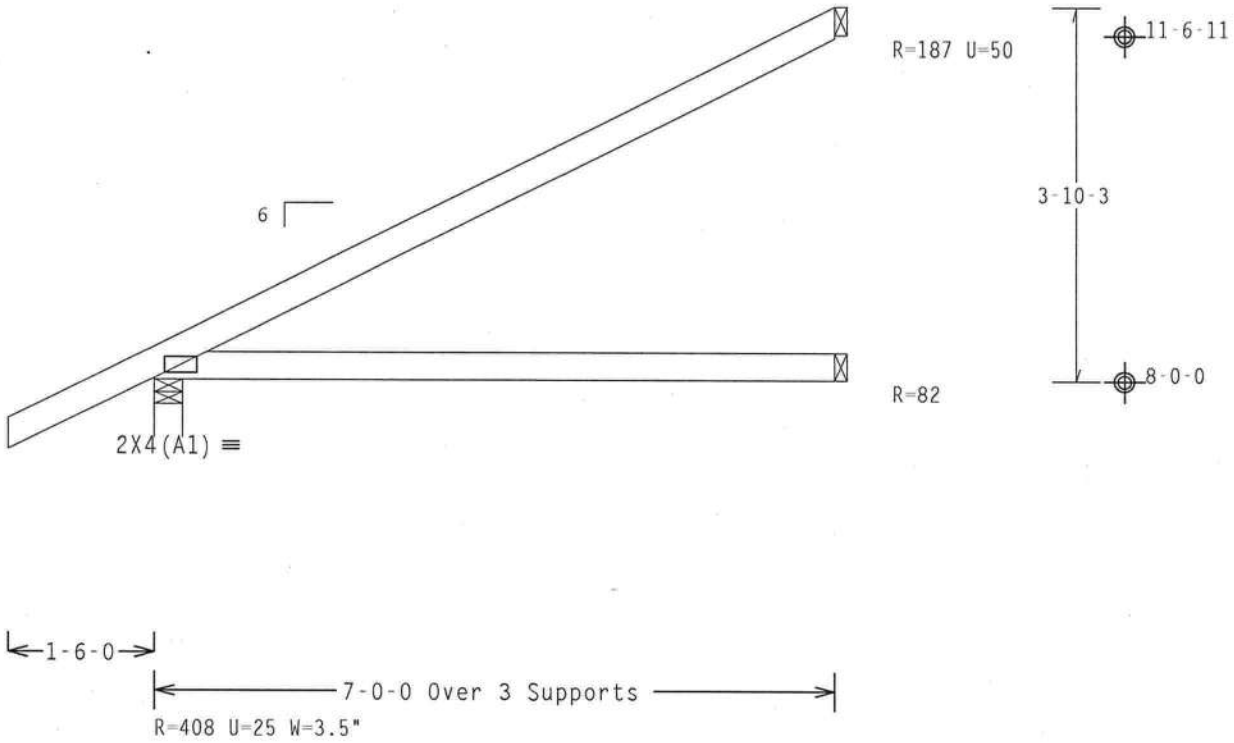
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.123[illegible]17

PLT TYP. Wave

QTY:15 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

08 '08

TC LL	20.0 PSF	REF	R8228- 80492
TC DL	10.0 PSF	DATE	04/08/08
BC DL	10.0 PSF	DRW	HCUSR8228 08099002
BC LL	0.0 PSF		HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN-	168744
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGI8228Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

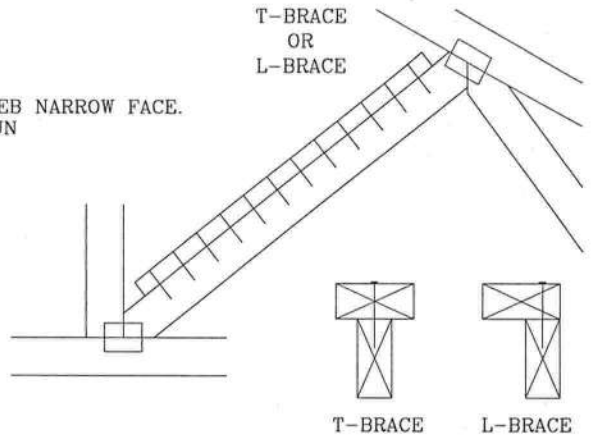
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

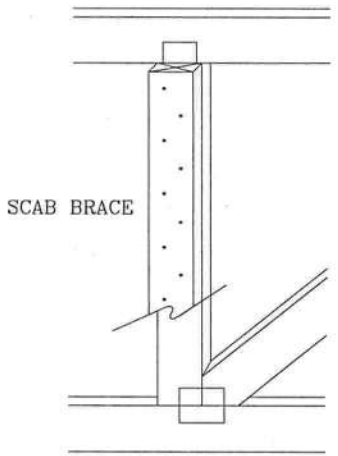
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579.640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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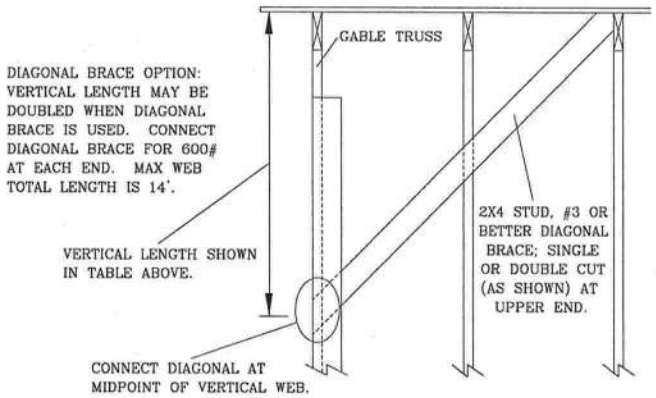
DOUGLAS FLEMING LICENSE No. 66648 '08 '08 STATE OF FLORIDA PROFESSIONAL ENGINEER

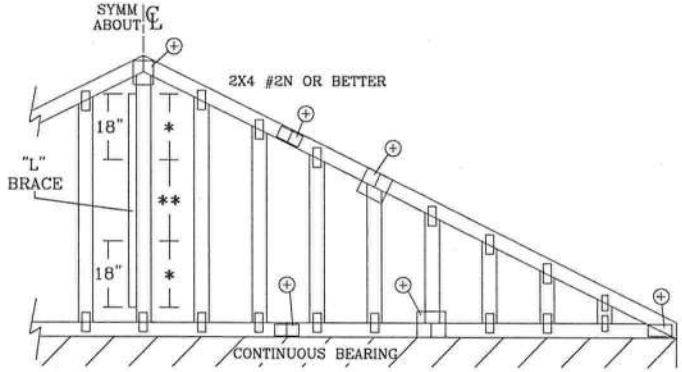
TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	GRADE / BRACE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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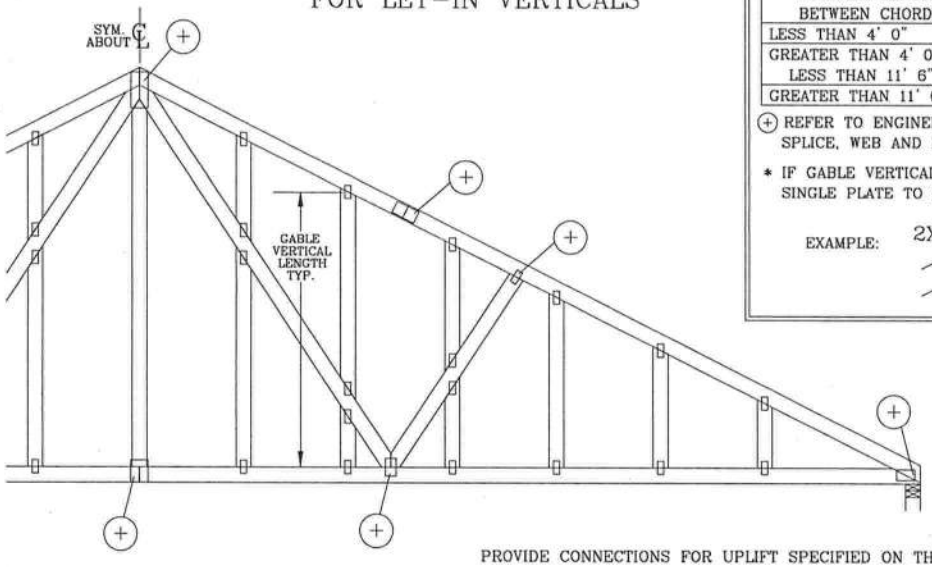
DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 08 '08

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

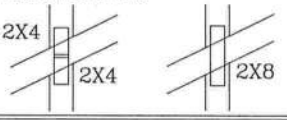


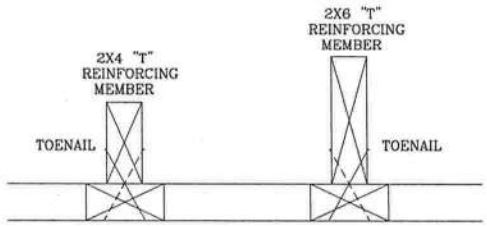
GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:





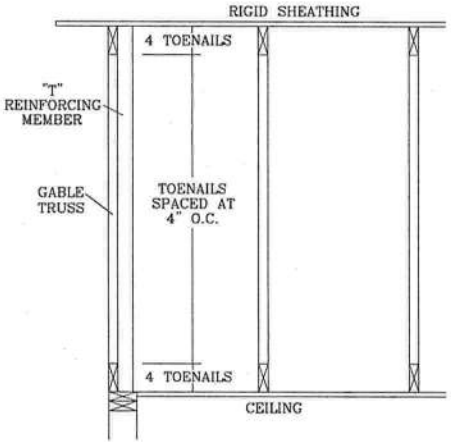
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.
GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.
ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207
ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207
ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207
ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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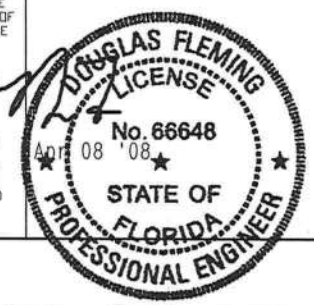


MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	5' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 11"	5' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

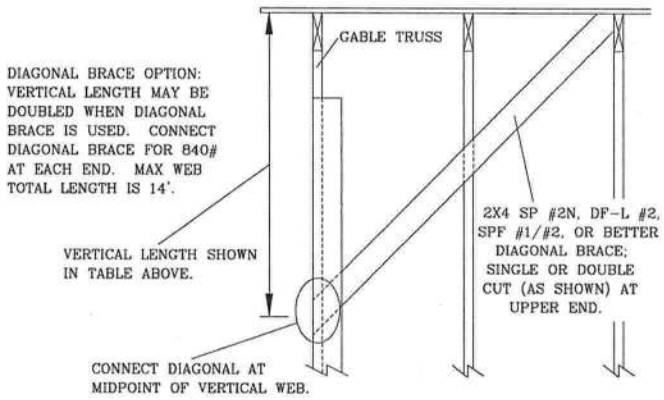
SOUTHERN PINE
#3
STUD
STANDARD

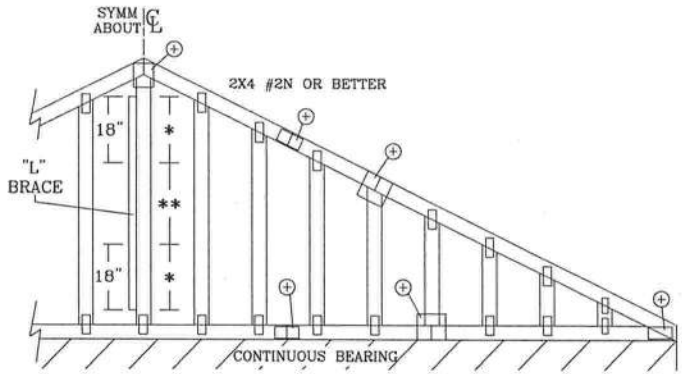
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 135 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

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GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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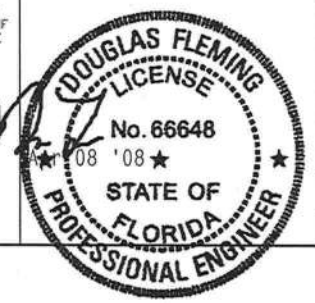
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GAL.V. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	2/23/07
DRWG	A13015EE0207
-ENG	

WINDLOAD ANALYSIS

PREPARED BY

DRISCOLL ENGINEERING,INC.
CONSULTING ENGINEERS
P.O. BOX 357577
GAINESVILLE, FL. 32606
EB 8690
PH (352) 331-1513

3-16-08

PREPARED FOR

CLIENT:

LEWIS

20818 S. US 441

HIGH SPRINGS, FL

DW08-08

INDEX

PLANS HAVE BEEN PREPARED IN COMPLIANCE WITH THE 2004 FLORIDA BUILDING CODE AND TO THE 2006 AMENDMENTS TO THE FLORIDA BUILDING CODE.

MICHAEL DRISCOLL P.E.
FL REG. #43922

3-19-08

FOR WINDLOAD ANALYSIS FOR BARN ONLY

WIND ANALYSIS SUMMARY

DRISCOLL ENGINEERING, Inc.
P.O. BOX 357577.
Gainesville, FL 32606
352-331-1513
E.B. 8690

Project No. **DW08-08**

Michael E. Driscoll, P.E.
FL Registration No. 43922

1. Name: Lewis
2. Address: 20818 S. US 441 HIGH SPRINGS , FL
3. Description: New Residence

Certification

I hereby certify that the accompanying wind load analysis for the Lewis Residence as described above demonstrates compliance with the FBC 2004 Section 1609, to the best of my knowledge.

Project Wind load Information

1. Basic wind speed for this design = 110 MPH
2. Wind importance factor use 1.0
3. Wind exposure for this design is Exposure B
4. Interior Pressure Coefficient or Gcpi = +/- 0.18
5. For design of MWFRS:
   a. Transverse on roof: end zone = 7.3 P.S.F., interior zone = 6.4 P.S.F.
   b. Longitudinal on wall: end zone = 19.2 P.S.F., interior zone = 12.7 P.S.F.
6. For design of Components & Cladding as follows:
   Zone 1 = +13.0 & -20.0 P.S.F.
   Zone 2 = +13.0 & - 35.0 P.S.F.
   Zone 3 = +13.0 & - 51.0 P.S.F.
   Zone 4 = +21.8 & - 23.6 P.S.F.
   Zone 5 = +21.8 & - 29.1 P.S.F.

Drawings

See drawings for additional details. In case of conflict, the more restrictive requirements of the drawings or these calculations govern.

Roof Structure

1. Trusses: Pre-engineered wood trusses at 24" o.c. The Truss engineering for this project was not available prior to the preparation of these wind-load calculations. A Typical Connector Schedule is provided for the convenience of the owner/builder as a selection guide only. If the truss uplift from the truss engineering exceeds the capacity of the specified connector, contact the Engineer immediately. Truss engineering shall be provided to Driscoll Engineering for review and confirmation of connector selection prior to beginning construction.

2. Truss Connection (Hurricane Clips): Use Simpson H2.5A Connectors typical with nails per mfr. recommendations for uplift capacity of 520 lbs. If the truss uplift from the truss engineering exceeds the capacity of the specified connector, contact the Engineer immediately. Also comply with note 1. above prior to beginning construction.

3. Roof Sheathing: Sheathing to be or 15/32" OSB min. to adequately resist exterior shear and uplift forces due to nailing. Panels to be facenailed w/ 8d @ 4" oc along edges and @ 8" oc along interior supports. Galv. metal edging to be nailed @ 4" oc.

Asphalt shingles Asphalt shingle roofing shall be installed per mfg. specifications to meet 110 m.p.h. wind loading & in accord with the Florida Building Code 2004 & 2006 amendments.

Exterior load bearing & shearwalls

1 Studs: Studs: 2 x 4 @ 16" o.c.
Governing load combination: dead + wind
Fv D+W = 55 psi
Fb D-W = 1900 psi
Use: SPF No. 2 grade or better

2. Shearwall Sheathing Minimum 7/16 structural sheathing, sheathing grade; attach all edges to framing with 8d common nails @ 6" o.c. attach to intermediate framing with 8d common nails @ 12" o.c. Sheathing shall be applied to outside face of **all exterior frame walls.** Use same nail pattern referenced above for non-shearwall segments also. Note that 8d common nails have a min 0.131 diameter.

Headers

1. Provide headers in accordance with Section 2308 of the *Florida Building Code, 2004.u.n.o.*
2. All wood header & beam connections to trusses shall be designed & engineered by the roof truss mfg.

Porches

1. Posts: Pressure treated 4 x 4 posts fasten to footing below w/ Simpson ABU44 base typical
2. Beam: Double 2x12 built up wd. Beam with ½" C.D.X. plywood filler. Fasten beams to post tops with connectors AC, ACE & LSTA12 to posts at ea. end of beam.

Gable End Walls

Construct a ceiling diaphragm using a minimum 5/8" thick gypsum board fastened directly to the bottom chord of each truss with 5d cooler nails or GWB-54 1 5/8" nails at 7" o.c. Install full depth blocking at 4 ft. maximum o.c. in the first four framing spaces from each end

Foundations (sizes based on wind load requirements only)

Stemwall footing: 10"T x 20"W; reinforce with (2) #5 continuous bars. Provide (2) #5x 25"x25" bars at each corner or intersection. Lap bars 25".

Monolithic slab (alternate): 20"T x 12"W; reinforce with (2) #5 continuous bars. Provide (2) #5 x 25" x 25" bars at each corner or intersection. Lap bars 25".

Anchor bolts ½" dia. x 10" (A307) @ 32" o.c., within 6" of each side of each opening and within 6" of each corner & sill plate end.

M.E. Driscoll, P.E.
Effective 1-01-07

GENERAL NOTES

Design Criteria

1. Structure to meet wind load requirements of FBC 2004 SEC. 1609 for a design wind speed of 110 mph.
2. Wood framing and fasteners to meet NDS-2005 requirements.
3. Fastener requirements: (1) All nails are Common galvanized; (2) all bolts are to be galvanized steel and include nuts and washers; and (3) all other hardware (Simpson, etc.) is to be installed according to manufacturer's specifications and recommendations. Nailing (size and number) shall satisfy Tables 2306.3.1, 2306.3.2 and 2306.4.1 FBC unless otherwise indicated. Note: fasteners exposed to the weather are to be treated for weather resistance and compatible with the type of pressure treated wood used (connectors, nails, bolts, nuts and washers).
4. Fasteners shall be driven flush with surface of sheathing.

Concrete Construction Notes

1. Concrete work shall conform to "Building Code Requirements for Reinforced Concrete" (ACI-318) and "Specifications for Structural Concrete" (ACI-301), Latest Edition.
2. Concrete Mix "A" shall be used for foundation walls, footings and interior slabs on grade. Concrete mix "B" shall be used for exterior slabs, curbs and all other exterior concrete. All concrete mixes shall contain a water-reducing admixture conforming to ASTM C-494. Air-entraining admixture shall conform to ASTM C-260.

	Mix A	Mix B
Ultimate Compressive Strength @ 28 days	3000 psi	3000 psi
Slump Range	4"+/-1"	3"+/-1"
Maximum Aggregate Size	1"	1"
Entrained Air	None	5-7%
Dry Weight per Cubic Foot	150#	150#

3. All concrete shall be cured for a minimum of 28 days. If forms for vertical surfaces are removed prior to the end of the curing period, spray surfaces with liquid membrane curing compound.
4. Reinforcing steel shall conform to ASTM A615, Grade 40 (Fy=40 ksi). Lap continuous bars for tension lap splice per ACI-318, unless otherwise noted. Provide corner bars of same size and spacing as horizontal wall reinforcement. Cover for concrete reinforcing steel shall be in accordance with ACI-318, Paragraph 7.7.
5. Welded wire fabric (WWF) shall conform to ASTM A185. Lap sheets two mesh spaces and wire tie adjacent sheets together securely. Cut alternate reinforcement at control joints.
6. All slabs on grade shall have construction or control joints not to exceed 15' – 0" spacing, unless otherwise noted.
7. Electrical conduit and other pipes to be embedded in structural concrete floor slabs or walls shall be placed in accordance with the requirements of ACI-318, Paragraph 6.3.

Masonry Construction Notes

1. Concrete masonry work shall conform to "Building Code Requirements for Masonry Structures" (ACI 530-02/ASCE5-02) and "Specifications for Masonry Structures: (ACI 530.1-02/ASCE6-02).
2. Concrete masonry units shall be Type 1 and comply with "Standard Specifications for Hollow Load-Bearing Concrete Masonry Units" (ASTM C90-90).
3. The minimum net area compressive strength of masonry (f'm), as determined by the unit strength method, shall be 1500 psi.
4. Mortar shall conform to ASTM C270. Type M Mortar shall be used unless otherwise noted. Type S Mortar shall be used with masonry in contact with earth.
5. Masonry column reinforcement shall have #2 ties in the bed joints at 8" oc, unless otherwise noted.
6. Grout for filling block cores and bond beams shall have a minimum compressive strength (f'c) of 3,000 psi at the age of 28 days.

TERMS AND CONDITIONS OF PROFESSIONAL ENGINEERING SERVICES

1.0 PLANS AND SPECIFICATIONS

The plans and specifications presented herein are applicable only for the anticipated construction at the locations shown. If construction plans change, the Design Professional should be notified so the plans and specifications can be re-evaluated. The Design Professional should be given the opportunity to review final plans and specifications to see if the intent of the plans and specifications has been followed and/or if supplemental details and recommendations are needed. The Design Professional warrants that the plans and specifications contained herein, have been prepared in accordance with generally accepted professional engineering practice. No other warranties are implied or expressed.

2.0 CORPORATE PROTECTION

It is understood and agreed that the Design Professional's Basic Services under this Agreement do not include project observation or review of the Contractor's performance or any other construction phase services, and that such services will be provided by the Client. The Client assumes all responsibility for interpretation of the contractor Documents and for construction observation and supervision and waives any claims against the Design Professional that may be in any way connected thereto.

In addition, the Client agrees, to the fullest extent permitted by law, to indemnify and hold the Design Professional harmless from any loss, claim or cost, including reasonable attorney's fees and costs of defense, arising or resulting from the performance of such services by other person or entities and from any and all claims arising from modifications, clarifications, interpretations, adjustments or changes made to Contract Documents to reflect changed field or other conditions, except for claims arising from the sole negligence or willful misconduct to the Design Professional.

3.0 OWNERSHIP OF INSTRUMENTS OF SERVICE

All reports, plans, specifications, computer files, field data, notes and other documents and instruments prepared by the Design Professional as instruments of service shall remain the property of the Design Professional. The Design Professional shall retain all common law, statutory and other reserved rights, including the copyright thereto.

4.0 DEFECTS IN SERVICE

The Client shall promptly report to the Design Professional any defects or suspected defects in the Design Professional's work or services of which the Client becomes aware, so that the Design Professional may take measures to minimize the consequences of such a defect. The Client warrants that he or she will impose a similar notification requirement on all contractors in his or her Client/Contractor contract and shall require all subcontractors at any level to contain a like requirement. Failure by the Client, and the Client's contractors or subcontractors to notify the Design Professional, shall relieve the Design Professional of the costs of remedying the defects above the sum such remedy would have cost had prompt notification been given.

5.0 VERIFICATION OF EXISTING CONDITIONS

Inasmuch as the remodeling and/or rehabilitation of an existing building requires that certain assumptions be made regarding existing conditions, and because some of these assumptions may not be verifiable without expending additional sums of money or destroying otherwise adequate or serviceable portions of the building, the Client agrees, to the fullest extent permitted by law, to indemnify and hold the Design Professional harmless from any claim, liability or cost (including reasonable attorney's fees and costs of defense) for injury or economic loss arising or allegedly arising out of the professional services provided under this Agreement, excepting only those damages, liabilities, or costs attributable to the sole negligence or willful misconduct of the Design Professional.

Driscoll Engineering Inc.
3538 NW 97 st
Gainesville FL
352 331 1513
fx 352 333 6996

JOB TITLE	LEWIS		
	20818 S US 441 HIGH SPRINGS, FL		
JOB NO.	MED	SHEET NO.	
CALCULATED BY	DEI	DATE	2/18/2008
CHECKED BY		DATE	

WINDLOAD ANALYSIS ONLY

STRUCTURAL CALCULATIONS

FOR

LEWIS

HIGH SPRINGS

Driscoll Engineering Inc.
3538 NW 97 st
Gainesville FL
352 331 1513
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Code Search

I. Code: ASCE 7 - 02

II. Occupancy:

Occupancy Group = R Residential

III. Type of Construction:

Fire Rating:

Roof = 0.0 hr
Floor = 0.0 hr

IV. Live Loads:

Roof angle (θ) 6.00 / 12 26.6 deg

Roof	0 to 200 sf:	18 psf
	200 to 600 sf:	21.6 - 0.018Area, but not less than 12 psf
	over 600 sf:	12 psf
Floor		0 psf
Stairs & Exitways		100 psf
Balcony		100 psf
Mechanical		N/A
Partitions		20 psf

V. Wind Loads : ASCE 7 - 02

Importance Factor	1.00
Basic Wind speed	110 mph
Directionality (Kd)	0.85
Mean Roof Ht (h)	12.0 ft
Parapet ht above grd	0.0 ft
Exposure Category	B
Enclosure Classif.	Enclosed Building
Internal pressure	+/-0.18
Building length (L)	49.5 ft
Least width (B)	37.7 ft
Kh case 1	0.701
Kh case 2	0.575

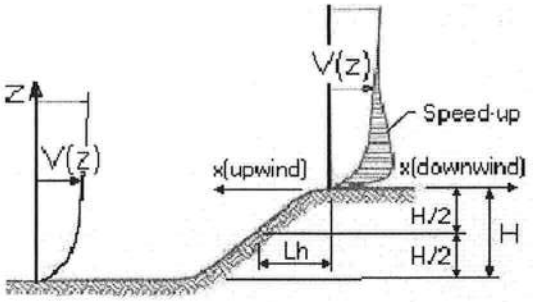
Topographic Factor (Kzt)

Topography		Flat	
Hill Height	(H)	0.0 ft	H< 60ft;exp B
Half Hill Length	(Lh)	0.0 ft	∴ Kzt=1.0
Actual H/Lh	=	0.00	
Use H/Lh	=	0.00	
Modified Lh	=	0.0 ft	
From top of crest:	x=	50.0 ft	
Bldg up/down wind?		downwind	

H/Lh= 0.00	K_1 =	0.000
x/Lh = 0.00	K_2 =	0.000
z/Lh = 0.00	K_3 =	1.000

At Mean Roof Ht:

$Kzt = (1+K_1K_2K_3)^2$ = 1.00



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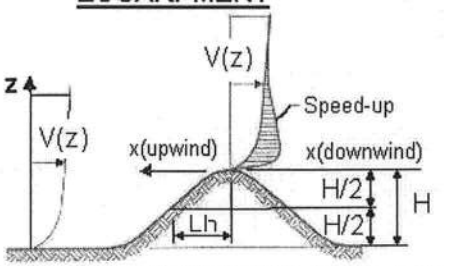

2D RIDGE or 3D AXISYMMETRICAL HILL

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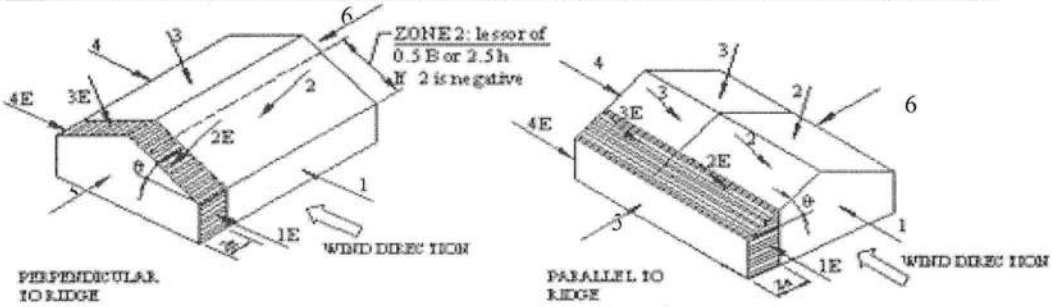
CHECKED BY	**DATE**

V. Wind Loads - MWFRS h≤60' (Low-rise Buildings) Enclosed/partially enclosed only



Torsional loads are 25% of zones 1 - 4. See code for loading diagram

Transverse Direction **Longitudinal Direction**

Kz = Kh =	0.70 (case 1)	Edge Strip	(a) 3.8 ft
Base pressure (qh) =	**18.4 psf**	End Zone	(2a) 7.5 ft
GCpi =	+/-0.18	Zone 2 length	= 18.9 ft

	Transverse Direction			Longitudinal Direction			
	Perpendicular θ = 26.6 deg			Parallel θ = 0deg			
Surface	GCpf	w/-GCpi	w/+GCpi	GCpf	w/-GCpi	w/+GCpi	
1	0.55	0.73	0.37	0.40	0.58	0.22	
2	-0.10	0.08	-0.28	-0.69	-0.51	-0.87	
3	-0.45	-0.27	-0.63	-0.37	-0.19	-0.55	
4	-0.39	-0.21	-0.57	-0.29	-0.11	-0.47	
5	-0.45	-0.27	-0.63	-0.45	-0.27	-0.63	
6	-0.45	-0.27	-0.63	-0.45	-0.27	-0.63	
1E	0.73	0.91	0.55	0.61	0.79	0.43	
2E	-0.19	-0.01	-0.37	-1.07	-0.89	-1.25	
3E	-0.58	-0.40	-0.76	-0.53	-0.35	-0.71	
4E	-0.53	-0.35	-0.71	-0.43	-0.25	-0.61	

Wind Surface pressures (psf) - use 10 psf minimum for zones 1 plus 4 and 5 plus 6

1		13.5	6.8	10.7	4.1
2		1.5	-5.1	-9.4	-16.0
3		-4.9	-11.6	-3.5	-10.1
4		-3.9	-10.5	-2.0	-8.7
5		-5.0	-11.6	-5.0	-11.6
6		-5.0	-11.6	-5.0	-11.6
1E		16.7	10.1	14.6	7.9
2E		-0.2	-6.8	-16.4	-23.1
3E		-7.5	-14.1	-6.5	-13.1
4E		-6.5	-13.2	-4.6	-11.3

MWFRS Simple Diaphragm Pressures (psf)

Transverse direction (normal to L)

Interior Zone:	Wall	17.3 psf
	Roof	6.4 psf
End Zone:	Wall	23.3 psf
	Roof	7.3 psf

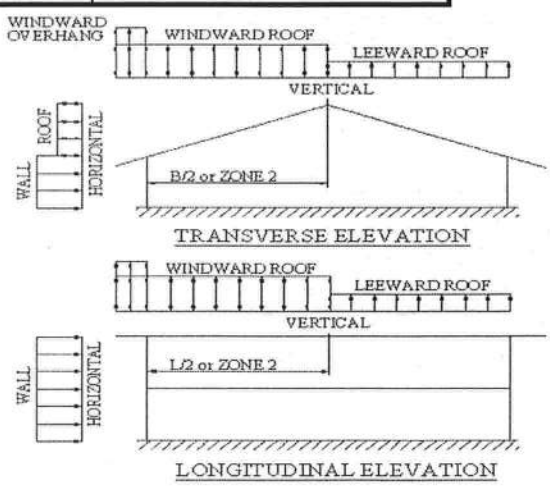
Longitudinal direction (parallel to L)

Interior Zone:	Wall	12.7 psf
End Zone:	Wall	19.2 psf

Windward roof overhangs: 12.5 psf (upward) add to windward roof pressure

Parapet

Windward parapet:	0.0 psf	(GCpn = +1.8)
Leeward parapet:	0.0 psf	(GCpn = -1.1)



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JOB TITLE LEWIS
20818 S US 441 HIGH SPRINGS, FL
JOB NO. MED SHEET NO.
CALCULATED BY DEI DATE 3/18/08
CHECKED BY DATE

V. Wind Loads - Components & Cladding: Buildings h≤60' & Alternate design 60'<h<90'

Kz = Kh (case 1) = 0.70 GCpi = +/-0.18
Base pressure (qh) = **18.4 psf** a = 3.8 ft
Minimum parapet height at building perimeter = 0.0 ft

NOTE: If tributary area is greater than 700sf, MWFRS pressure may be used.

Roof Angle = 26.6 deg
Type of roof = Gable

Roof	GCp +/- GCpi			Surface Pressure (psf)			**User input**	
Area	10 sf	50 sf	100 sf	10 sf	50 sf	100 sf	20 sf	70 sf
Negative Zone 1	-1.08	-1.01	-0.98	-20 psf	-19 psf	-18 psf	-19 psf	-18 psf
Negative Zone 2	-1.88	-1.53	-1.38	-35 psf	-28 psf	-25 psf	-32 psf	-27 psf
Negative Zone 3	-2.78	-2.36	-2.18	-51 psf	-44 psf	-40 psf	-48 psf	-42 psf
Positive All Zones	0.68	0.54	0.48	13 psf	10 psf	10 psf	11 psf	10 psf
Overhang Zone 2	-2.20	-2.20	-2.20	-41 psf	-41 psf	-41 psf	-41 psf	-41 psf
Overhang Zone 3	-3.70	-2.86	-2.50	-68 psf	-53 psf	-46 psf	-62 psf	-50 psf

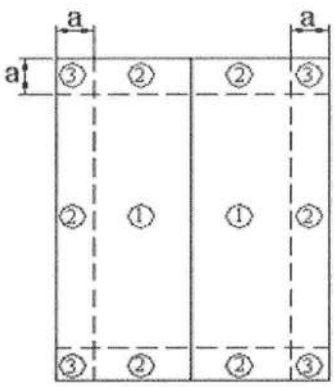
Walls	GCp +/- GCpi			Surface Pressure (psf)			**User input**	
Area	10 sf	100 sf	500 sf	10 sf	100 sf	500 sf	50 sf	200 sf
Negative Zone 4	-1.28	-1.10	-0.98	-23.6 psf	-20.4 psf	-18.1 psf	-21.3 psf	-19.4 psf
Negative Zone 5	-1.58	-1.23	-0.98	-29.1 psf	-22.6 psf	-18.1 psf	-24.6 psf	-20.7 psf
Positive Zone 4 & 5	1.18	1.00	0.88	21.8 psf	18.5 psf	16.2 psf	19.5 psf	17.5 psf

Parapet

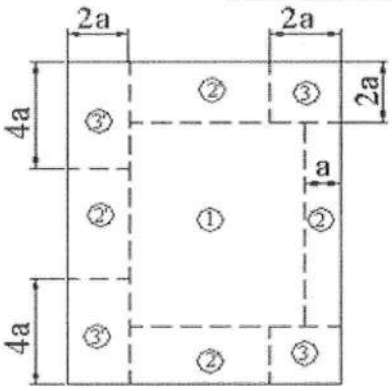
qp = 0.0 psf

CASE A = pressure towards building
CASE B = pressure away from building

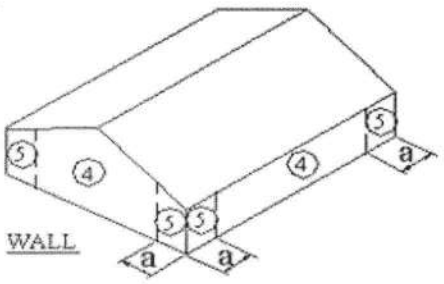
Solid Parapet Pressure	10 sf	100 sf	500 sf
CASE A : Interior zone :	0.0 psf	0.0 psf	0.0 psf
Corner zone :	0.0 psf	0.0 psf	0.0 psf
CASE B : Interior zone :	0.0 psf	0.0 psf	0.0 psf
Corner zone :	0.0 psf	0.0 psf	0.0 psf

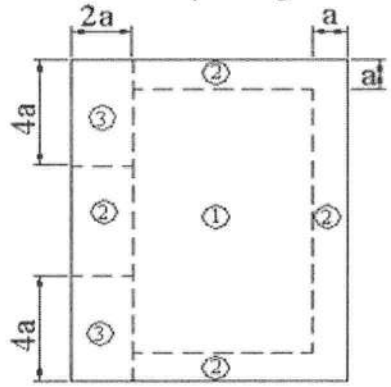


θ ≤ 7 degrees and
Monoslope ≤ 3 degrees

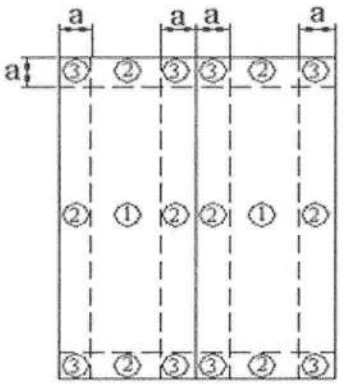


Monoslope roofs
3° < θ ≤ 10°

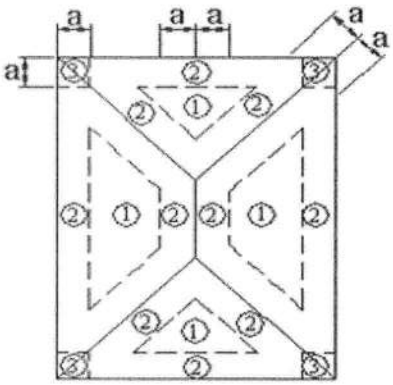




Monoslope roofs 10° < θ ≤ 30°



θ > 7 degrees



θ > 7 degrees

WoodWorks® Shearwalls 7.0

LEWIS.wsr Mar. 18, 2008 13:51:43

Project Information

COMPANY AND PROJECT INFORMATION

Company	Project
DRISCOLL ENGINEERING INC 3538 N.W. 97th BLVD GAINESVILLE, FL 352- 331-1513	LEWIS 20818 S US 441 HIGH SPRINGS, FL DW08-8

DESIGN SETTINGS

Design Code	Wind Standard	Seismic Standard
2004 FBC 4 ~~IBC 2003~~ 2006 MOMENTS	ASCE 7-02 All heights	ASCE 7-02

Wind Capacity Increase		Load Combinations	Building Code Capacity Reduction	
Shear	C&C Panels		Wind	Seismic
1.40	1.60	1.00 Wind	1.00	1.00

Nail Withdrawal Modification				Max Shearwall Offset [ft]	
Duration Factor	Temperature Range	Moisture Content		Plan (within story)	Elevation (between stories)
		Fabrication	Service		
1.60	T<=100F	Dry	Dry	0.50	-

Maximum Height-to-width Ratio					
Plywood		Fiberboard	Lumber		Gypsum
Wind	Seismic		Wind	Seismic	Blocked / Unblocked
3.5	-	-	-	-	2.0 / 1.5

Shearwall Relative Rigidity:	Designed capacity using flexible distribution
Hold-down Forces:	Based on applied shearline force
Seismic Materials:	-
Case 2 rigid diaphragm load distribution:	75% loads, torsional moment

SITE INFORMATION

IBC Occupancy	ASCE 7 Equivalent
Category II - All others	Category II - All others

Wind	Seismic
ASCE 7-02 General analytic method for all bldgs	-

Design Wind Speed	110mph	**Use Group**	-
Exposure	Exposure B	**Design Category**	-
Enclosure	Enclosed	**Site Profile Class**	-

Topographic Information [ft]			Spectral Response Acceleration		
Shape	Height	Length	S1: -	Ss: -	
-	-	-	**Fundamental Period**	**E-W**	**N-S**
Site Location: -			**T Used**	-	-
-			**Approximate Ta**	-	-
-			**Maximum T**	-	-
			Response Factor R	-	-

Structural Data

STORY INFORMATION

	Story Elev [ft]	Floor/Ceiling Depth [in]	Wall Height [ft]
Ceiling	10.83	0.0	
Level 1	2.83	10.0	8.00
Foundation	2.00		

BLOCK and ROOF INFORMATION

Block Dimensions [ft]			Roof Panels Face	Type	Slope	Overhang [ft]
Block 1	1 Story	E-W Ridge				
Location X,Y =	0.00	4.00	**North**	Side	30.0	1.00
Extent X,Y =	49.50	33.50	**South**	Side	30.0	1.00
Ridge Y Location, Offset	20.75	0.00	**East**	Gable	90.0	1.00
Ridge Elevation, Height	20.50	9.67	**West**	Gable	90.0	1.00

MATERIALS by WALL GROUP

Wall		Sheathing [in]			Fasteners		Spcg [in]		Framing Members [in]				Apply
Grp	Surf	Material	Thick	Orient	Size	Type	Edg	Fld	Blkg	Species	G	Spc	Notes
1	Ext	Struct Shthg	7/16	Vert	8d	Nail	6	12	yes	S-P-F	0.42	16	1,3
	Int	Gyp WB 1-ply	1/2	Horz	5d	Nail	7	7	yes	S-P-F	0.42	16	
2	Ext	Structural I	7/16	Vert	8d	Nail	6	12	yes	S-P-F	0.42	16	1,3
	Int	Gyp WB 1-ply	1/2	Horz	5d	Nail	7	7	yes	S-P-F	0.42	16	

Grp - Wall Design Group; Surf - Exterior or interior surface of exterior wall; Spcg - Edge or field nail spacing; Blkg - Blocked; G - Specific gravity; Spc - Wall stud spacing

Notes:

1. Capacity has been reduced according to IBC specific gravity adjustment.

3. Shear capacity for current design has been increased to the value for 15/32" sheathing with same nailing because stud spacing is 16" max. or panel orientation is horizontal.

SHEARLINE, WALL and OPENING DIMENSIONS

North-South Shearlines	Type	Wall Group(s)	Location X [ft]	Extent [ft] Start	End	Length [ft]	FHS [ft]	Height [ft]
Line 1								
Level 1								
Line 1, Level 1	Prf	1	0.00	4.00	37.50	33.50	33.50	8.00
Wall 1-1	Prf	1	0.00	4.00	37.50	33.50	33.50	
Line 2								
Level 1								
Line 2, Level 1	Prf	1	19.50	4.00	37.50	33.50	10.50	8.00
Wall 2-1	Prf	1	19.50	4.00	14.50	10.50	10.50	
Line 3								
Level 1								
Line 3, Level 1	Prf	1	49.50	4.00	37.50	33.50	21.00	8.00
Wall 3-2	Prf	1	49.50	14.50	37.50	23.00	21.00	
Opening 1				24.50	26.50	2.00		3.00
Wall 3-1	NSW		49.50	4.00	14.50	10.50	10.50	

East-West Shearlines	Type	Wall Group(s)	Location Y [ft]	Extent [ft] Start	End	Length [ft]	FHS [ft]	Height [ft]
Line A								
Level 1								
Line A, Level 1	Prf	1	4.00	0.00	49.50	49.50	16.50	8.00
Wall A-1	Prf	1	4.00	0.00	19.50	19.50	16.50	
Opening 1				8.50	11.50	3.00		5.00
Wall A-2	NSW		4.00	19.50	49.50	30.00	30.00	
Line B								
Level 1								
Line B, Level 1	Prf	1	14.50	0.00	49.50	49.50	13.50	8.00
Wall B-1	Prf	1	14.50	19.50	49.50	30.00	13.50	
Opening 1				23.00	29.00	6.00		5.00
Opening 2				33.00	36.00	3.00		6.80
Opening 3				41.50	45.50	4.00		5.00
Line C								
Level 1								
Line C, Level 1	Prf	1	37.50	0.00	49.50	49.50	24.00	8.00
Wall C-1	Prf	1	37.50	0.00	49.50	49.50	24.00	
Opening 1				3.00	5.00	2.00		3.00
Opening 2				8.50	11.50	3.00		5.00
Opening 3				15.50	18.50	3.00		3.00
Opening 4				22.50	25.50	3.00		3.00
Opening 5				30.00	35.00	5.00		6.70
Opening 6				42.00	45.00	3.00		5.00

Type - Seg = segmented, prf = perforated, NSW = non-shearwall; Location - dimension perp. to wall; FHS - length of full-height sheathing; Wall Group(s) - refer to Materials by Wall Group table, refer to Shear Results tables for each design case if more than one group

Loads

WIND SHEAR LOADS (overlapping loads combined)

Level 1 Block	F	Element	Load Case	Wnd Dir	Surf Dir	Prof	Location [ft] Start	Location [ft] End	Magnitude [lbs,plf,psf] Start	Magnitude [lbs,plf,psf] End	Trib Ht [ft]
Block 1	W		1	W->E	Wind	Line	4.00	20.75	41.2	141.7	
Block 1	W		2	W->E	Wind	Line	4.00	20.75	30.9	106.3	
Block 1	W		2	W->E	Wind	Line	20.75	37.50	106.3	30.9	
Block 1	W		1	W->E	Wind	Line	20.75	37.50	141.7	41.2	
Block 1	E		2	W->E	Lee	Line	4.00	20.75	15.8	54.0	
Block 1	E		1	W->E	Lee	Line	4.00	20.75	21.1	72.0	
Block 1	E		2	W->E	Lee	Line	20.75	37.50	54.0	15.8	
Block 1	E		1	W->E	Lee	Line	20.75	37.50	72.0	21.1	
Block 1	W		2	E->W	Lee	Line	4.00	20.75	15.8	54.0	
Block 1	W		1	E->W	Lee	Line	4.00	20.75	21.1	72.0	
Block 1	W		1	E->W	Lee	Line	20.75	37.50	72.0	21.1	
Block 1	W		2	E->W	Lee	Line	20.75	37.50	54.0	15.8	
Block 1	E		1	E->W	Wind	Line	4.00	20.75	41.2	141.7	
Block 1	E		2	E->W	Wind	Line	4.00	20.75	30.9	106.3	
Block 1	E		1	E->W	Wind	Line	20.75	37.50	141.7	41.2	
Block 1	E		2	E->W	Wind	Line	20.75	37.50	106.3	30.9	
Block 1	S		1	S->N	Wind	Line	-1.00	0.00	28.4		
Block 1	S		2	S->N	Wind	Line	-1.00	0.00	21.3		
Block 1	S		2	S->N	Wind	Line	0.00	49.50	52.2		
Block 1	S		1	S->N	Wind	Line	0.00	49.50	69.6		
Block 1	S		1	S->N	Wind	Line	49.50	50.50	28.4		
Block 1	S		2	S->N	Wind	Line	49.50	50.50	21.3		
Block 1	N		2	S->N	Lee	Line	-1.00	0.00	60.1		
Block 1	N		1	S->N	Lee	Line	-1.00	0.00	80.1		
Block 1	N		2	S->N	Lee	Line	0.00	49.50	79.6		
Block 1	N		1	S->N	Lee	Line	0.00	49.50	106.1		
Block 1	N		2	S->N	Lee	Line	49.50	50.50	60.1		
Block 1	N		1	S->N	Lee	Line	49.50	50.50	80.1		
Block 1	S		2	N->S	Lee	Line	-1.00	0.00	60.1		
Block 1	S		1	N->S	Lee	Line	-1.00	0.00	80.1		
Block 1	S		2	N->S	Lee	Line	0.00	49.50	79.6		
Block 1	S		1	N->S	Lee	Line	0.00	49.50	106.1		
Block 1	S		1	N->S	Lee	Line	49.50	50.50	80.1		
Block 1	S		2	N->S	Lee	Line	49.50	50.50	60.1		
Block 1	N		1	N->S	Wind	Line	-1.00	0.00	28.4		
Block 1	N		2	N->S	Wind	Line	-1.00	0.00	21.3		
Block 1	N		2	N->S	Wind	Line	0.00	49.50	52.2		
Block 1	N		1	N->S	Wind	Line	0.00	49.50	69.6		
Block 1	N		2	N->S	Wind	Line	49.50	50.50	21.3		
Block 1	N		1	N->S	Wind	Line	49.50	50.50	28.4		

Block - Block used in load generation, Accum. = loads from one block combined with another, Manual = user-entered loads (so no block); F - Building face (north, south, east or west); Element - building surface on which loads generated or entered; Load Case - ASCE 7 All heights Case 1 or 2, ASCE 7 low rise T = Transverse, L = Longitudinal; Surf Dir - Windward or leeward side of the building; Prof - Profile (distribution); Location - start and end points on building element; Magnitude - Start = intensity of uniform and point loads or leftmost intensity of trapezoidal load, end = right intensity of trap load; Trib Ht - Tributary height of user-applied area loads only

WIND C&C LOADS

Block	Building Face	Level	Magnitude [psf] Interior	End Zone
Block 1	West	1	23.6	29.1
Block 1	East	1	23.6	29.1
Block 1	South	1	23.6	29.1
Block 1	North	1	23.6	29.1

UPLIFT LOADS

Shear Line	Level	Profile	Tributary Width [ft]	Location [ft] Start	End	Mag [lbs,psf,psi] Start	End
A	1	Line		0.00	19.50	225.0	
B	1	Line		19.50	49.50	225.0	
C	1	Line		0.00	49.50	225.0	
1	1	Line		4.00	37.50	225.0	
2	1	Line		4.00	14.50	225.0	
3	1	Line		14.50	37.50	225.0	

Flexible Diaphragm Wind Design

SHEAR RESULTS

North-South Shearlines	W Gp	For Dir	Ld. Case	Shear Force [plf] V [lbs]	V/L	V/FHS	Allowable Shear [plf] Int	Ext	Co	C	Total	V [lbs]	Crit. Resp.
Line 1													
Level 1													
Ln1, Lev1	1	Both	1	1822	54.4	54.4	125	335	1.00	A	460	15406	0.12
Line 2													
Ln2, Lev1	1	Both	1	4349	129.8	414.2	125	335	1.00	A	460	4829	0.90
Line 3													
Ln3, Lev1	1	Both	1	2744	81.9	130.7	125	335	0.99	A	455	9554	0.29

East-West Shearlines	W Gp	For Dir	Ld. Case	Shear Force [plf] V [lbs]	V/L	V/FHS	Allowable Shear [plf] Int	Ext	Co	C	Total	V [lbs]	Crit. Resp.
Line A													
Level 1													
LnA, Lev1	1	Both	1	493	10.0	29.9	125	335	0.88	A	405	6688	0.07
Line B													
LnB, Lev1	1	Both	1	2648	53.5	196.2	125	335	0.65	A	301	4060	0.65
Line C													
LnC, Lev1	1	Both	1	1481	29.9	61.7	125	335	0.64	A	295	7088	0.21

W Grp - Wall group as listed in Materials table; For Dir - force direction, always "both" ; H/W factor - Factor due to shearwall height-to-width ratio for interior and exterior wall sheathing; V - Factored shear force applied to entire line and amount taken by each wall; V/L - Diaphragm shear force = factored shearline force divided by length of shearline;

*Following values marked with * means that value for shearline is the one for wall with critical design response on line; V/FHS* - Design shear force = factored shear force per unit full height sheathing, Int* - Unit shear capacity of interior sheathing; Ext* - Unit shear capacity of exterior sheathing Co* - Perforation factor; C - Sheathing combination rule, A = Add capacities, S = Strongest side only, X = Strongest side or twice weakest; Total* - Combined unit shear capacity inc. perforation factor and H/W factor; V - Combined shear capacity of wall or total capacity of shearline ; Crit Resp* - Critical response = Vapp/FHS/Vcap = design shear force/unit shear capacity*

DRAGSTRUT AND HOLDDOWN FORCES

Level 1 Line-Wall	Position on Wall or Opening	Location [ft] X	Y	Ld. Case	Tensile Holddown Force [lbs] Shear	Dead	Uplift	Cmb'd	Dragstrut Force [lbs]
Line 1									
1-1	Left Wall End	0.00	4.25	1	435		3769	4204	
1-1	Right Wall End	0.00	37.25	1	435		3769	4204	
Line 2									
2-1	Left Wall End	19.50	4.25	1	3313		1181	4494	
2-1	Right Wall End	19.50	14.25	1	3313		1181	4494	
2-1	Right Wall End	19.50	14.50	1					273
Line 3									
3-2	Right Wall End	49.50	37.25	1	1057		2588	3644	
3-2	Left Wall End	49.50	14.75	1	1057		2588	3644	
3-2	Left Wall End	49.50	14.50	1					860
3-2	Left Opening 1	49.50	24.50	1					270
3-2	Right Opening 1	49.50	26.50	1					434
Line A									
A-1	Right Wall End	19.25	4.00	1	271		2194	2465	
A-1	Left Wall End	0.25	4.00	1	271		2194	2465	
A-1	Right Wall End	19.50	4.00	1					1205
A-1	Left Opening 1	8.50	4.00	1					636
A-1	Right Opening 1	11.50	4.00	1					606
Line B									
B-1	Left Wall End	29.25	14.50	1	2400		2306	4706	
B-1	Right Wall End	49.25	14.50	1	2400		2306	4706	
B-1	Right Opening 1	29.00	14.50	1					1552
B-1	Left Opening 2	33.00	14.50	1					1526
B-1	Right Opening 2	36.00	14.50	1					1686
B-1	Left Opening 3	41.50	14.50	1					1651
B-1	Right Opening 3	45.50	14.50	1					1865
Line C									
C-1	Left Wall End	11.75	37.50	1	769		4275	5044	
C-1	Right Wall End	49.25	37.50	1	769		4275	5044	
C-1	Right Opening 2	11.50	37.50	1					344
C-1	Left Opening 3	15.50	37.50	1					214
C-1	Right Opening 3	18.50	37.50	1					304
C-1	Left Opening 4	22.50	37.50	1					174
C-1	Right Opening 4	25.50	37.50	1					264
C-1	Left Opening 5	30.00	37.50	1					118
C-1	Right Opening 5	35.00	37.50	1					268
C-1	Left Opening 6	42.00	37.50	1					41
C-1	Right Opening 6	45.00	37.50	1					130

Line-Wall - Shearline for forces carried by vertical elements, wall for forces at wall or opening ends; Position...- Vert. element = column or strengthened studs req'dotherwise indicates stud location at wall end or opening;. Location - Co-ordinates in Plan View; Ld. Case - Results are for critical load case: ASCE 7 All heights Case 1 or 2, ASCE 7 low rise T = Transverse, L = Longitudinal; Shear - Factored wind shear overturning component ; Dead - Unfactored dead load resisting component; Uplift - Uplift wind load component; Cmb'd - Sum of factored overturning, dead and uplift forces; Dragstrut Force - Factored drag strut force at opening headers and gaps in walls along shearline

Note:

This table includes tension holddown forces only.

Dead load contribution to combined force is factored by 0.60 load combination factor

Rigid Diaphragm Wind Design

Rigid analysis performed using Case 2 loading in ASCE 7-02 Fig 6-9, using only the eccentricity prescribed to generate a torsional moment. For analysis using Case 1 loading, change the Rigid Diaphragm Analysis Design Setting, regenerate loads, and redesign.

SHEAR RESULTS

North-South Shearlines	W Gp	For Dir	Ld. Case	Shear Force [plf] V [lbs]	V/L	V/FHS	Allowable Shear [plf] Int	Ext	Co	C	Total	V [lbs]	Crit. Resp.
Line 1													
Level 1													
Ln1, Lev1	1	Both	2	4279	127.7	127.7	125	335	1.00	A	460	15406	0.28
Line 2													
Ln2, Lev1	1	Both	2	1090	32.5	103.8	125	335	1.00	A	460	4829	0.23
Line 3													
Ln3, Lev1	1	Both	2	2959	88.3	140.9	125	335	0.99	A	455	9554	0.31

East-West Shearlines	W Gp	For Dir	Ld. Case	Shear Force [plf] V [lbs]	V/L	V/FHS	Allowable Shear [plf] Int	Ext	Co	C	Total	V [lbs]	Crit. Resp.
Line A													
Level 1													
LnA, Lev1	1	Both	2	1399	28.3	84.8	125	335	0.88	A	405	6688	0.21
Line B													
LnB, Lev1	1	Both	2	809	16.3	59.9	125	335	0.65	A	301	4060	0.20
Line C													
LnC, Lev1	1	Both	2	1497	30.2	62.4	125	335	0.64	A	295	7088	0.21

W Grp - Wall group as listed in Materials table; For Dir - force direction, always "both" ; H/W factor - Factor due to shearwall height-to-width ratio for interior and exterior wall sheathing; V - Factored shear force applied to entire line and amount taken by each wall; V/L - Diaphragm shear force = factored shearline force divided by length of shearline;
*Following values marked with * means that value for shearline is the one for wall with critical design response on line; V/FHS* - Design shear force = factored shear force per unit full height sheathing, Int* - Unit shear capacity of interior sheathing; Ext* - Unit shear capacity of exterior sheathing Co* - Perforation factor; C - Sheathing combination rule, A = Add capacities, S = Strongest side only, X = Strongest side or twice weakest; Total* - Combined unit shear capacity inc. perforation factor and H/W factor; V - Combined shear capacity of wall or total capacity of shearline ; Crit Resp* - Critical response = Vapp/FHS/Vcap = design shear force/unit shear capacity*

DRAGSTRUT AND HOLDDOWN FORCES

Level 1 Line-Wall	Position on Wall or Opening	Location [ft] X	Y	Ld. Case	Tensile Holddown Force [lbs] Shear	Dead	Uplift	Cmb'd	Dragstrut Force [lbs]
Line 1									
1-1	Right Wall End	0.00	37.25	2	1022		3769	4790	
1-1	Left Wall End	0.00	4.25	2	1022		3769	4790	
Line 2									
2-1	Right Wall End	19.50	14.25	2	830		1181	2012	
2-1	Left Wall End	19.50	4.25	2	830		1181	2012	
2-1	Right Wall End	19.50	14.50	2					748
Line 3									
3-2	Left Wall End	49.50	14.75	2	1139		2588	3727	
3-2	Right Wall End	49.50	37.25	2	1139		2588	3727	
3-2	Left Wall End	49.50	14.50	2					927
3-2	Left Opening 1	49.50	24.50	2					402
3-2	Right Opening 1	49.50	26.50	2					578
Line A									
A-1	Left Wall End	0.25	4.00	2	770		2194	2963	
A-1	Right Wall End	19.25	4.00	2	770		2194	2963	
A-1	Right Wall End	19.50	4.00	2					848
A-1	Left Opening 1	8.50	4.00	2					481
A-1	Right Opening 1	11.50	4.00	2					396
Line B									
B-1	Right Wall End	49.25	14.50	2	733		2306	3039	
B-1	Left Wall End	29.25	14.50	2	733		2306	3039	
B-1	Right Opening 1	29.00	14.50	2					474
B-1	Left Opening 2	33.00	14.50	2					300
B-1	Right Opening 2	36.00	14.50	2					349
B-1	Left Opening 3	41.50	14.50	2					109
B-1	Right Opening 3	45.50	14.50	2					174
Line C									
C-1	Right Opening 3	18.50	37.50	2					310
C-1	Left Wall End	11.75	37.50	2	777		4275	5052	
C-1	Right Wall End	49.25	37.50	2	777		4275	5052	
C-1	Right Opening 2	11.50	37.50	2					348
C-1	Left Opening 3	15.50	37.50	2					219
C-1	Left Opening 4	22.50	37.50	2					181
C-1	Right Opening 4	25.50	37.50	2					272
C-1	Left Opening 5	30.00	37.50	2					128
C-1	Right Opening 5	35.00	37.50	2					279
C-1	Left Opening 6	42.00	37.50	2					54
C-1	Right Opening 6	45.00	37.50	2					145

Line-Wall - Shearline for forces carried by vertical elements, wall for forces at wall or opening ends; Position...- Vert. element = column or strengthened studs req'dotherwise indicates stud location at wall end or opening;. Location - Co-ordinates in Plan View; Ld. Case - Results are for critical load case: ASCE 7 All heights Case 1 or 2, ASCE 7 low rise T = Transverse, L = Longitudinal; Shear - Factored wind shear overturning component ; Dead - Unfactored dead load resisting component; Uplift - Uplift wind load component; Cmb'd - Sum of factored overturning, dead and uplift forces; Dragstrut Force - Factored drag strut force at opening headers and gaps in walls along shearline

Note:

This table includes tension holddown forces only.

Dead load contribution to combined force is factored by 0.60 load combination factor

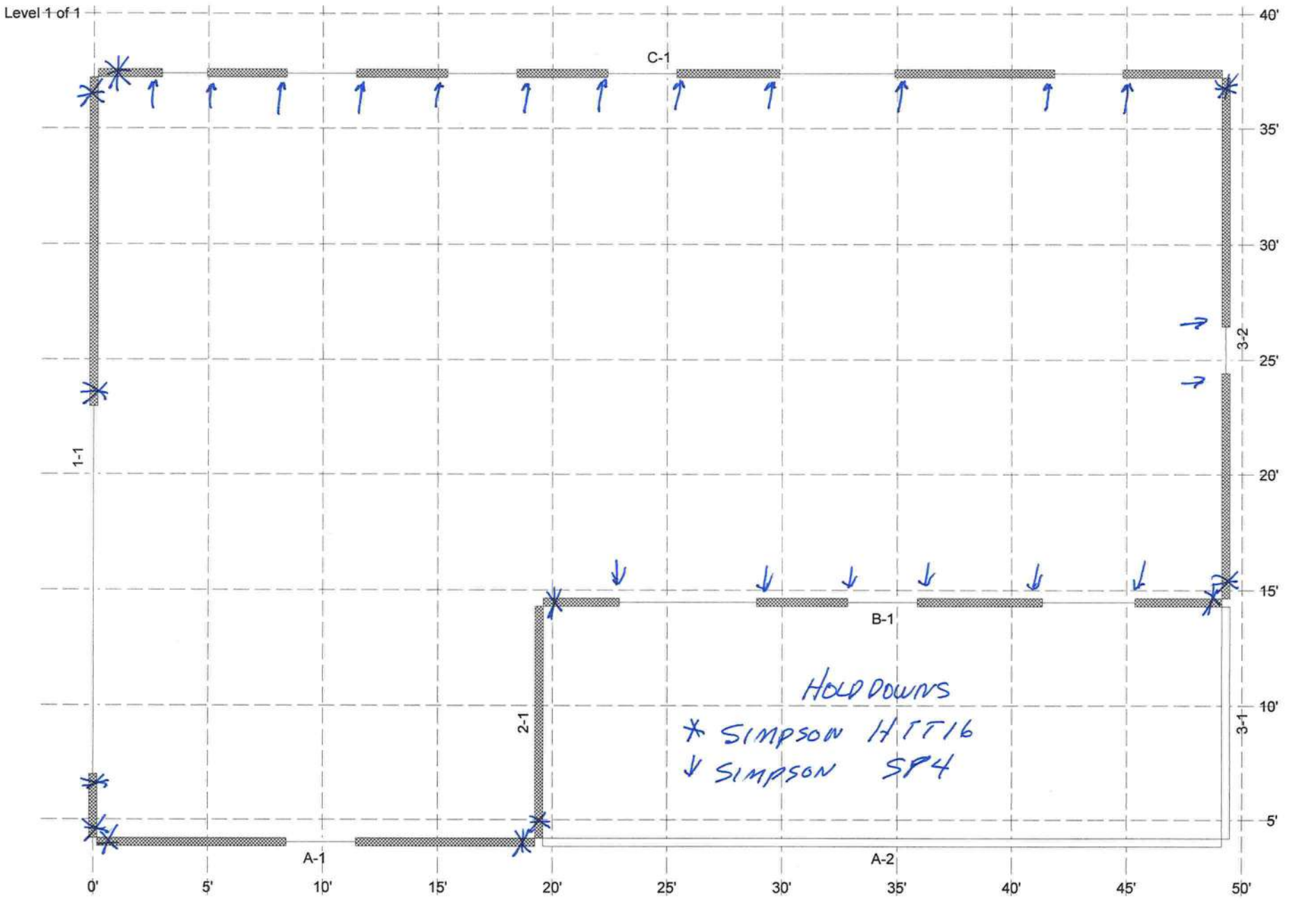
Wind Suction Design

COMPONENTS AND CLADDING by SHEARLINE

North-South Shearlines			Sheathing [psf]			Fastener Withdrawal [lbs]					Service Cond Factors	
			Force	Cap	Force/	Force		Cap	Force/Cap			
Line	Lev	Grp			Cap	End	Int		End	Int	Temp	Moist
1	1	1	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00
3	1	1	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00
		2	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00

East-West Shearlines			Sheathing [psf]			Fastener Withdrawal [lbs]					Service Cond Factors	
			Force	Cap	Force/	Force		Cap	Force/Cap			
Line	Lev	Grp			Cap	End	Int		End	Int	Temp	Moist
A	1	1	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00
		2	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00
C	1	1	29.1	48.0	0.61	39	31	68	0.57	0.46	1.00	1.00

Grp - Wall Design Group, results for design groups for rigid, flexible design listed for each wall; Sheathing: Force - Unit area end zone C&C load, Cap - Out-of-plane bending capacity of exterior sheathing assuming continuous over 3 studs; Fastener Withdrawal: Force - Force tributary to each nail in end zone and interior zone; Cap - Factored withdrawal capacity of individual fastener according to NDS 11.2-3


Level 1 of 1
C-1
B-1
A-1
A-2
1-1
2-1
3-1
3-2
0'
5'
10'
15'
20'
25'
30'
35'
40'
45'
50'
HOLDDOWNS
* SIMPSON HTT16
↓ SIMPSON SP4

TYPICAL CONNECTOR SCHEDULE

HEADERS / SILL ANCHOR

UPLIFT LOAD	CONNECTORS		RATING LBS.	
	TOP	BOTT.	TOP	BOTT.
UP TO 605 lbs.	LSTA9	H3	605	455
UP TO 755 lbs.	LSTA12	2-H3	755	910
UP TO 1,055 lbs.	LSTA18	LTT19	1,055	1,205
UP TO 1,750 lbs.	2-LSTA12	LTT20	1,510	1,750
UP TO 2,110 lbs.	2-LSTA18	HD2A-2.5	2,110	2,565
UP TO 2,865 lbs.	3-LSTA18	HD5A-2.5	3,165	3,095
UP TO 3,700 lbs.	3-LSTA24	HD5A-3	3,885	3,705

REMARKS

1. REFER TO TRUSS ENGINEERING FOR GRAVITY, LATERAL AND UPLIFT LOADS AT BEARING POINTS.
2. ALL REFERENCED CONNECTORS ARE SIMPSON BRAND. INSTALLER MAY SUBSTITUTE EQUIVALENT CONNECTORS BY OTHER MANUFACTURERS
3. ALL HEADERS SHALL BE CONNECTED BY A MIN. (2 EACH) CS16 STRAPS AT EACH END TO THE DOUBLE TOP PLATE BY RUNNING STRAPS UP ONE SIDE AND ATTACHED TO THE OPPOSITE SIDE.
4. INSTALL REQUIRED ANCHORS NOT MORE THAN 6" FROM PLATE SPLICES, OPENINGS AND END OF SHEARWALL

TRUSSES / GIRDERS / RAFTERS / SILL ANCHORS

UPLIFT LOAD	WD. CONNECTORS		CONNECTORS MASONRY
	TOP	BOTT.	
UP TO 415 lbs.	H2.5	SP1	HM9
415 TO 780 lbs.	H10	SPH4	MTSM20
UP TO 1,200 lbs.	TS22	LTT21	H16 OR HETA40
UP TO 1,750 lbs.	2-TS22	LTT21	H16 OR HETA16
UP TO 2,310 lbs.	2-TS22	HD2A-3	HHETA16
UP TO 3,645 lbs.	3-TS22	HD5A-3	MGT
UP TO 5,250 lbs.	2-MST48	HTT22	HGT-2
UP TO 8,310 lbs.	2-MST48	HD10A-3	HGT-4

REMARKS

1. INSTALL H10 CONNECTORS AT EACH TRUSS BEARING POINT AS A MINIMUM OR HIGHER CAPACITY CONNECTOR AS REQUIRED BY TRUSS ENGINEERING.
2. ALL REFERENCED CONNECTORS ARE SIMPSON BRAND. INSTALLER MAY SUBSTITUTE EQUIVALENT CONNECTORS BY OTHER MANUFACTURERS
3. CONTRACTOR SHALL MAINTAIN A CONTINUOUS LOAD PATH FROM TRUSS/RAFTER/RIDGE BEAM AND GIRDERS TO FOUNDATION.
4. STRAP TRUSSES/RAFTERS AND GIRDERS @ EA. BEARING POINT TO RESIST UPLIFT LOADING PER TRUSS ENGINEERING.
5. STRAP RIDGE BEAM @ EA. BEARING POINT TO RESIST UPLIFT LOADING PER TRUSS ENGINEERING.

POSTS/BEAM SEATS

ITEM	CONNECTORS		CONNECTORS	
	TIE	LBS.	TIE	LBS.
BEAM SEATS	LSTA21	1,200	LTT19	1,250
POSTS	2-LSTA18	2,100	ABU44	2,200
	2-LSTA18	2,100	ABU66	2,300
	2-LSTA24	2,400	ABU66	2,300

REMARKS

1. REFER TO TRUSS ENGINEERING FOR GRAVITY, LATERAL AND UPLIFT LOADS AT BEARING POINTS.
2. ALL REFERENCED CONNECTORS ARE SIMPSON BRAND. INSTALLER MAY SUBSTITUTE EQUIVALENT CONNECTORS BY OTHER MANUFACTURERS
3. POST SPACING 8'-0" O.C. MAX. U.N.O.
3. REFER TO TRUSS ENGINEERING WHERE APPLICABLE.

STUDS

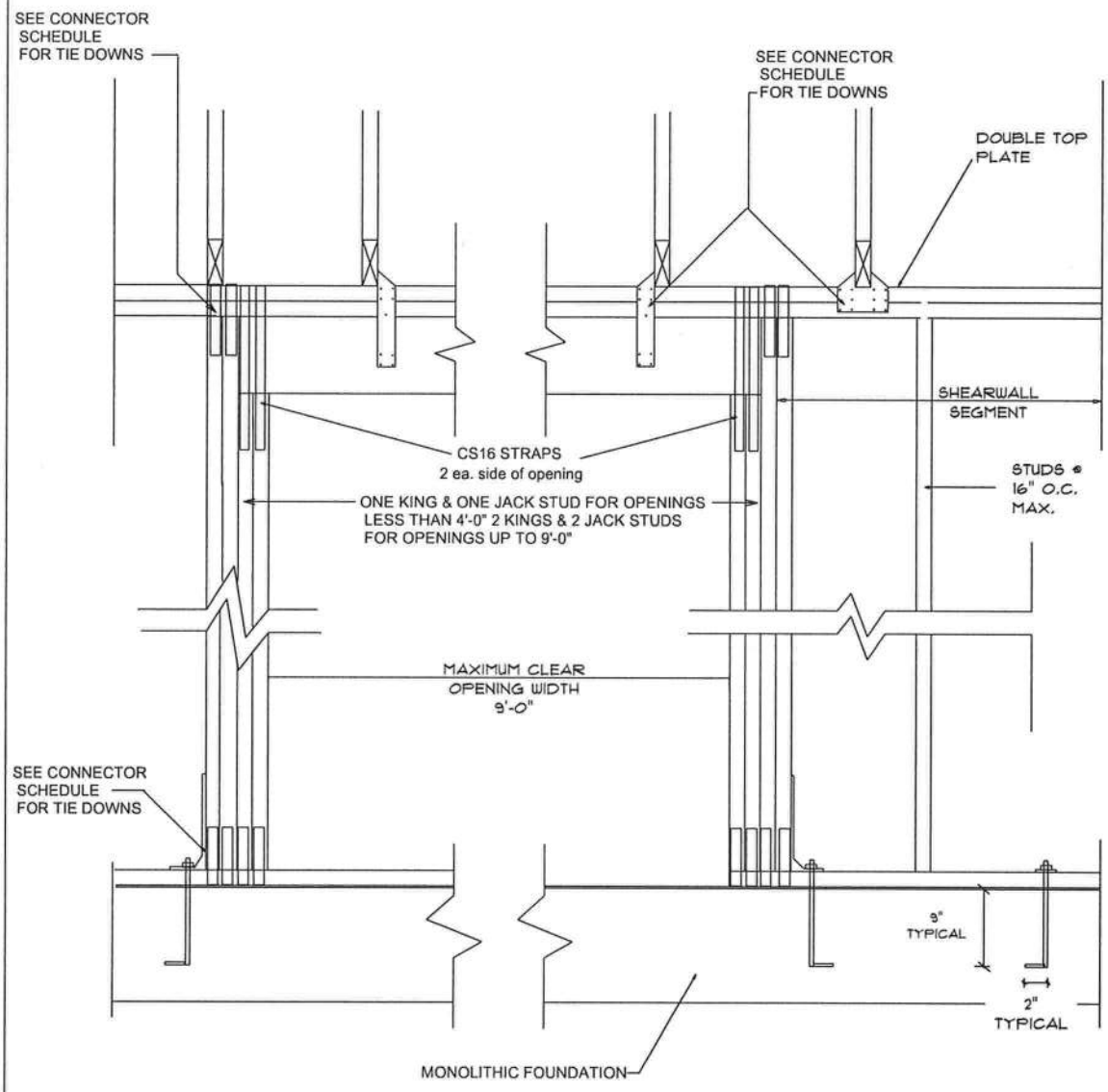
ITEM	CONNECTORS	
	TOP	BOTT.
EXT. SHEARWALLS	SP2 EA.	SP1 EA.
EXT. SHEARWALLS	CS16 EA.	CS16 EA.
INT SHEARWALLS	SP2 EA.	SP1 EA.
INT SHEARWALLS	CS16 EA.	CS16 EA.
EXT. SHEARWALLS	SP4 EA.	SP4 EA.
EXT. SHEARWALLS	SP6 EA.	SP6 EA.

REMARKS

1. FASTEN SHEATHING W/ 0.131" DIAMETER COMMON NAILS @ 4" O.C. AT EDGES AND 8" O.C. IN THE FIELD
2. ALL REFERENCED CONNECTORS ARE SIMPSON BRAND. INSTALLER MAY SUBSTITUTE EQUIVALENT CONNECTORS BY OTHER MANUFACTURERS
3. INSTALL REQUIRED SILL ANCHORS NOT MORE THAN 6" FROM SPLICES, OPENINGS AND ENDS OF WALLS.
4. CONTRACTOR SHALL PROVIDE A CONTINUOUS LOAD PATH FROM TRUSS/RAFTER/RIDGE BEAM TO FOUNDATION.
5. ANCHOR BOLTS SHALL BE 1/2" x 10" (A307) OR 5/8" x 10" WITH 8" MIN. EMBEDMENT. PROVIDE 1/8" X 2" WASHERS @ EA. BOLT

NOTE: WHEN USING SPF LUMBER IN LIEU OF SYP, USE 0.86 MULTIPLIER FOR TABLE VALUES.

** FOR LOADS LARGER THAN THOSE SPECIFIED SPECIAL ENGINEERING/CONNECTORS SHALL BE REQUIRED



LOAD BEARING WALL OPENING FRAMING DETAIL

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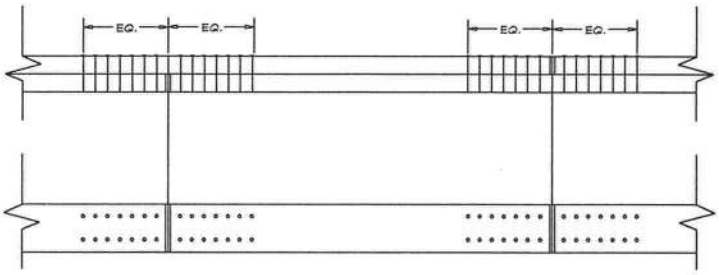
GAINESVILLE, FL. 32606
EB 8690
PH (352) 332-1513

OWNERS:
LEWIS
20818 S US 441
HIGH SPRINGS, FL DS08-8

DATE:
DESIGNER:
CHECKED:
DRAWN:
REVISED:
JOB NO.
SHEET
OF

2 ROWS OF 7-10d @ 3" O.C.
EACH SIDE OF SPLICE (TYP.)

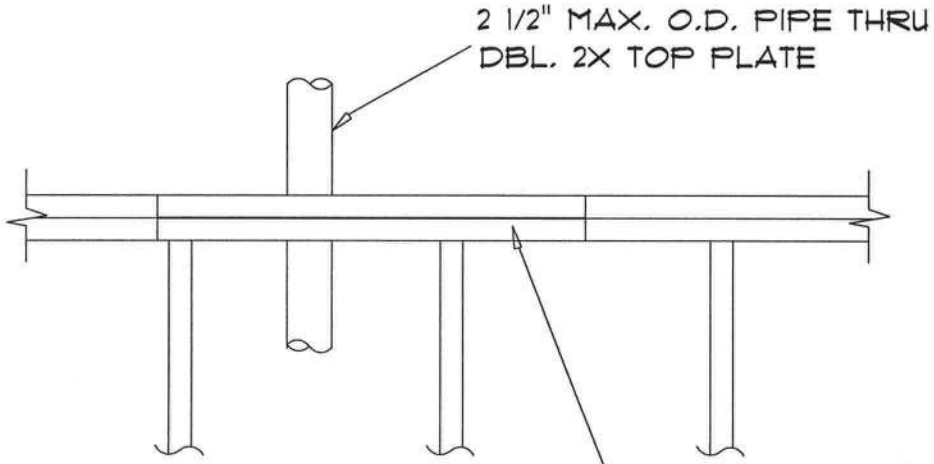
DOUBLE 2X
TOP PLATE



PLAN VIEW

NOTE:
10d NAILS @ 16" O.C
ELSEWHERE

TOP PLATE SPLICE



TOP PLATE/BRG. WALL PENETRATION DETAIL

NOT TO SCALE

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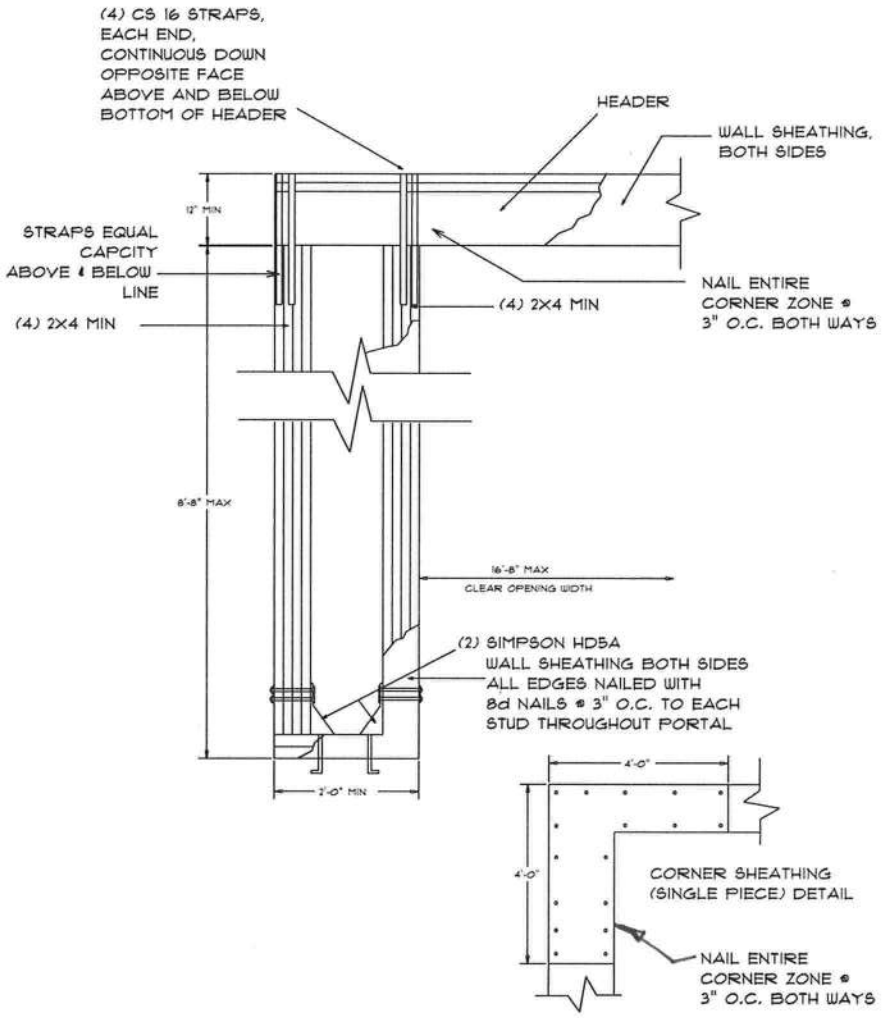

GARAGE DOOR OPENING ENDWALL FRAME

NTS

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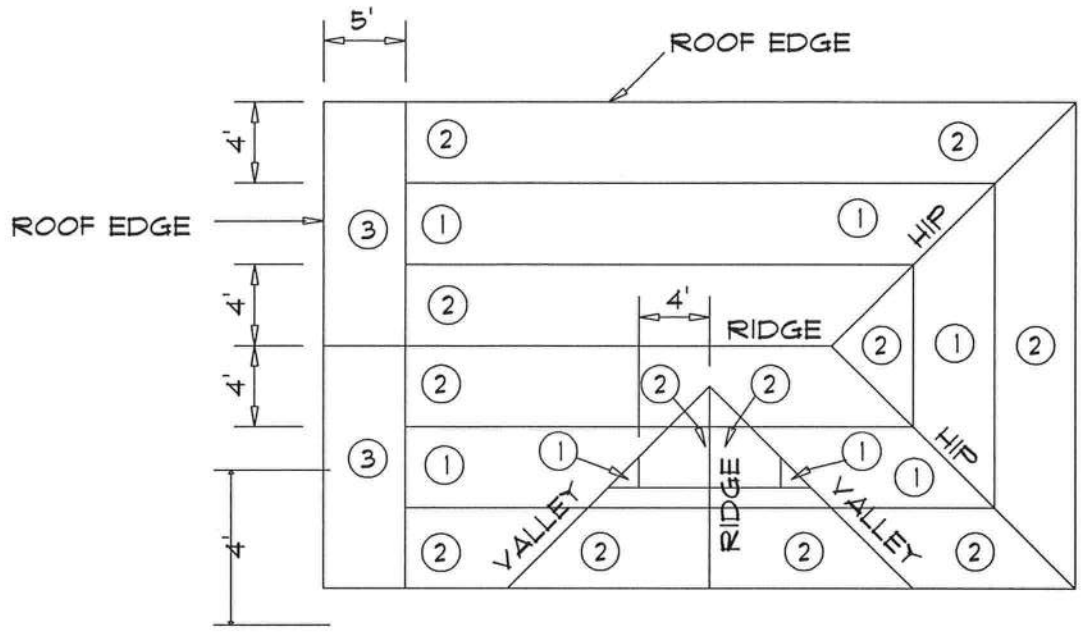

ROOF ATTACHMENT PLAN

ZONE 1 EDGES AT TRUSSES 6" O.C. AND INTERMEDIATE TRUSSES 10" O.C.
ZONE 2 EDGES AT TRUSSES 6" O.C. AND INTERMEDIATE TRUSSES 6" O.C.
ZONE 3 EDGES AT TRUSSES 4" O.C. AND INTERMEDIATE TRUSSES 6" O.C.

NOTES:

1. ALL NAILS TO BE RING SHANK NAILS MIN.

2. IF BUILDING WIDTH EXCEEDS 40 FT OR HEIGHT IS MORE THAN 2 STORIES USE 10d INSTEAD OF 8d FOR ATTACHMENT OF ROOF SHEATHING

3. ALL STRUCTURAL SHEATHING PANELS TO BE 15/32" PLYWOOD MIN. THICKNESS.

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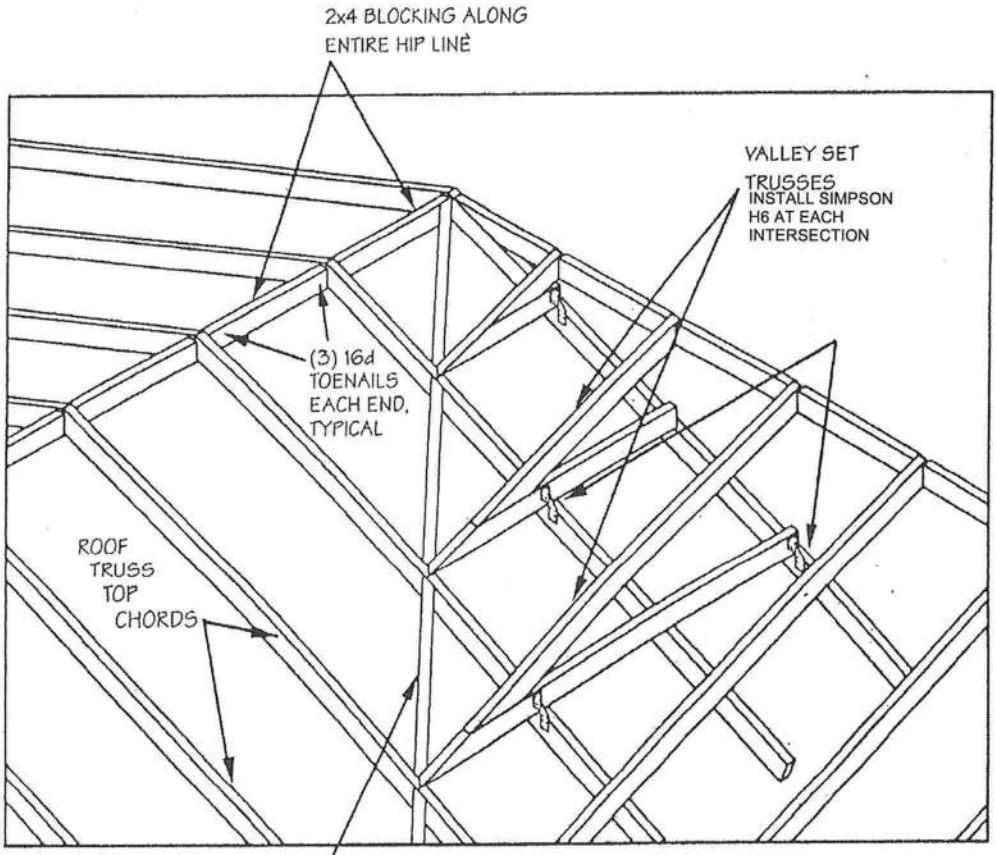

NOTE: SHEATHING MAY BE PROVIDED BETWEEN MAIN ROOF TRUSSES AND VALLEY SET TRUSSES.

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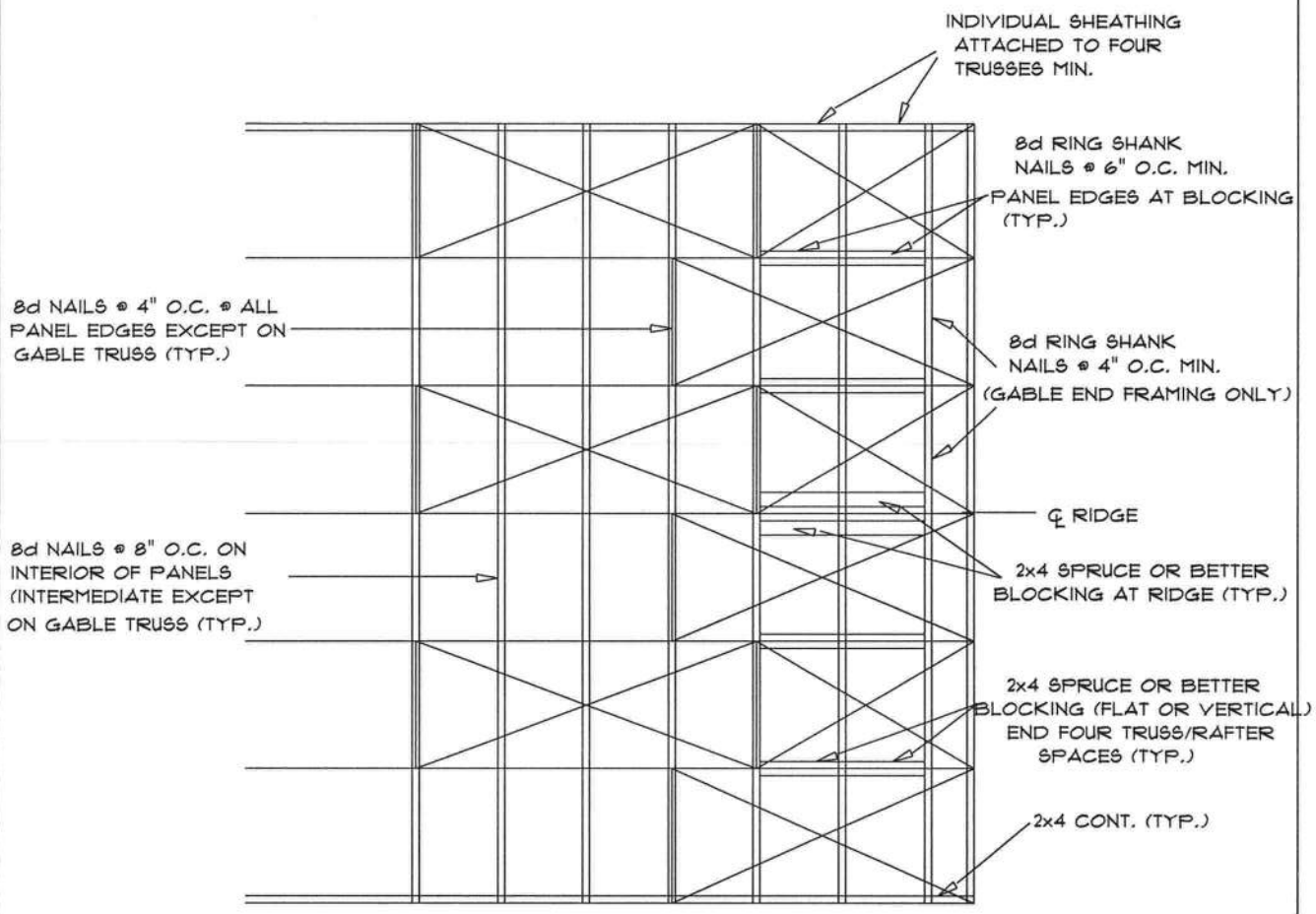

NOTES:

1. ALL NAILS TO BE RING SHANK NAILS MIN.
2. IF BUILDING WIDTH EXCEEDS 40 FT OR HEIGHT IS MORE THAN 2 STORIES USE 10d INSTEAD OF 8d FOR ATTACHMENT OF ROOF SHEATHING.
3. ALL STRUCTURAL SHEATHING PANELS TO BE 15/32 OSB MIN.

ROOF SHEATHING ATTACHMENT PLAN

N.T.S.

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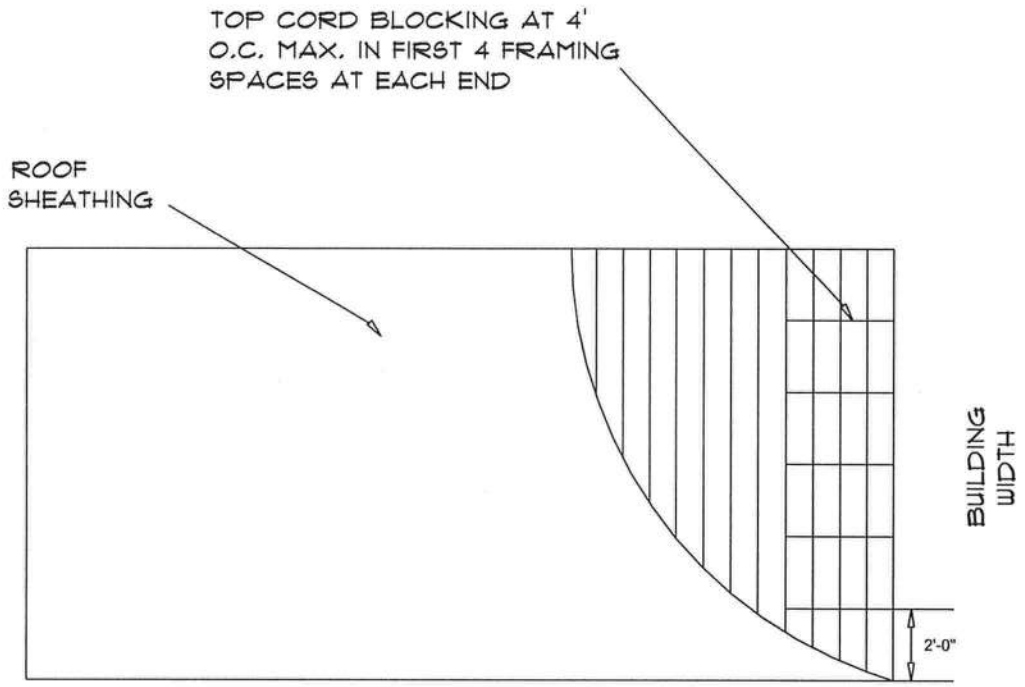

ROOF SHEATHING LAYOUT AND DIAPHRAGM BLOCKING

NOT TO SCALE

NOTE: FOR GABLE END FRAMING ONLY

NOTES:

1. ALL NAILS TO BE RING SHANK NAILS MIN.
2. IF BUILDING WIDTH EXCEEDS 40 FT OR HEIGHT IS MORE THAN 2 STORIES USE 10d INSTEAD OF 8d FOR ATTACHMENT OF ROOF SHEATHING.
3. ALL STRUCTURAL SHEATHING PANELS TO BE 15/32" OSB MIN.

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SHEET
OF

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 09-7S-17-09957-112 **Building permit No.** 000027247

Use Classification SFD,UTILITY **Fire:** 0.00

Permit Holder SAME AS APPLICANT **Waste:** 0.00

Owner of Building ROY LEWIS **Total:** 0.00

Location: 20818 S HIGHWAY 441, HIGH SPRINGS, FL

Date: 02/16/2010

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)