ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1UW8215-Z0414165158

05/14/2013

Truss Fabricator:	**W.B. Howland**
Job Identification:	**8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL**
Truss Count:	**18**
Model Code:	**Florida Building Code 2010**
Truss Criteria:	**FBC2010Res/TPI-2007(STD)**
Engineering Software:	**Alpine Software,Version 12.03.**
Structural Engineer of Record:	**The identity of the structural EOR did not exist as of**
Address:	**the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads:	**Roof - 40.0 PSF @ 1.25 Duration**
	Floor - N/A
	Wind - 130 MPH ASCE 7-10 -Closed

Notes:

1. **Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1**
2. **The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.**
3. **As shown on attached drawings; the drawing number is preceded by: HCUSR215**

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-14015EC1-GBLLETIN-14030EC1-

#	Ref Description	Drawing#	Date
1	51066--A	13134049	05/14/13
2	51067--A1	13134050	05/14/13
3	51068--A2	13134051	05/14/13
4	51069--A3	13134052	05/14/13
5	51070--A4	13134058	05/14/13
6	51071--B	13134004	05/14/13
7	51072--B1	13134056	05/14/13
8	51073--B2	13134057	05/14/13
9	51074--B3	13134001	05/14/13
10	51075--C	13134008	05/14/13
11	51076--C1	13134053	05/14/13
12	51077--C2	13134005	05/14/13
13	51078--D	13134003	05/14/13
14	51079--D1	13134002	05/14/13
15	51080--G	13134054	05/14/13
16	51081--G1	13134055	05/14/13
17	51082--P	13134006	05/14/13
18	51083--P-SR	13134007	05/14/13

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#2_N__12A :T2 2x6 SP_#2_N_12A:
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

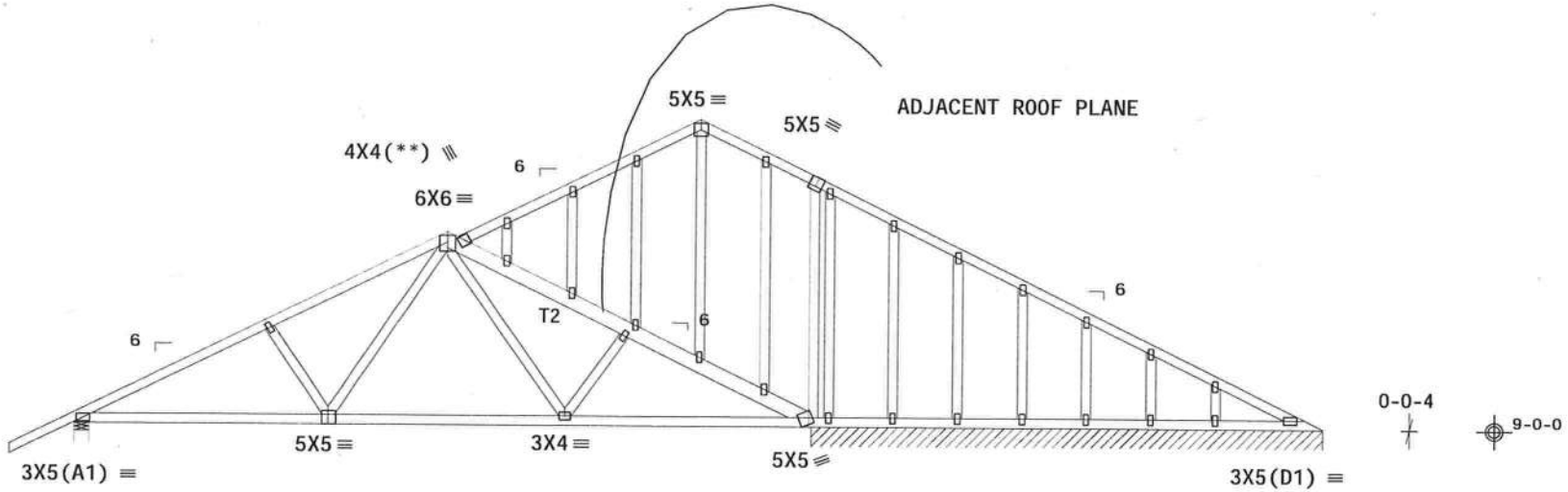
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

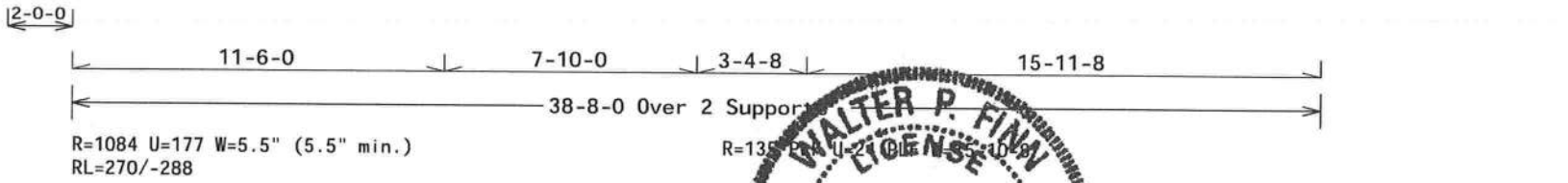
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

The overall height of this truss excluding overhang is 9-8-4.





R=1084 U=177 W=5.5" (5.5" min.)
RL=270/-288

R=135 U=21 W=5.5" (5.5" min.)

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0) 12.03.05.04.16.17 QTY 1 FL/-/1/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web members shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	20.0 PSF	REF	R215-- 51066
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134049
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	206692
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - A1)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A :B3 2x4 SP_SS_12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

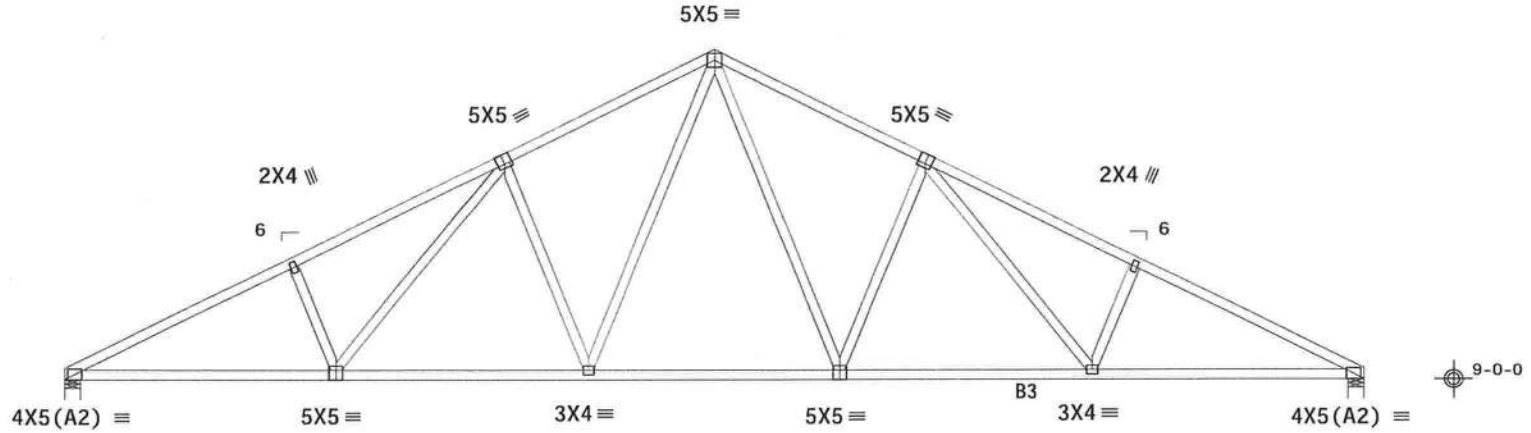
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-0-3.



R=1547 U=272 W=5.5" (5.5" min.)
RL=259/-259

R=1547 U=272 W=5.5" (5.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0)

12.03.05.0416.17 QTY:

FL/-/1/-/-/R/-

Scale =.1875"/Ft.

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05/14/2013

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R215-- 51067
DATE 05/14/13
DRW HCUSR215 13134050
HC-ENG DR/WPF
SEQN- 206648
FROM CDM
JREF- 1UW8215_Z04

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Haines City, FL 33844
FL COA #0 278

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(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - A2)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#2_N__12A :T1 2x4 SP_SS_12A:
Bot chord 2x6 SP_#2_N_12A :B2 2x6 SP_SS_12A:
Webs 2x4 SP_#2_N__12A :W5 2x4 SP_SS_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.16" due to live load and 0.23" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

The overall height of this truss excluding overhang is 10-0-3.

(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

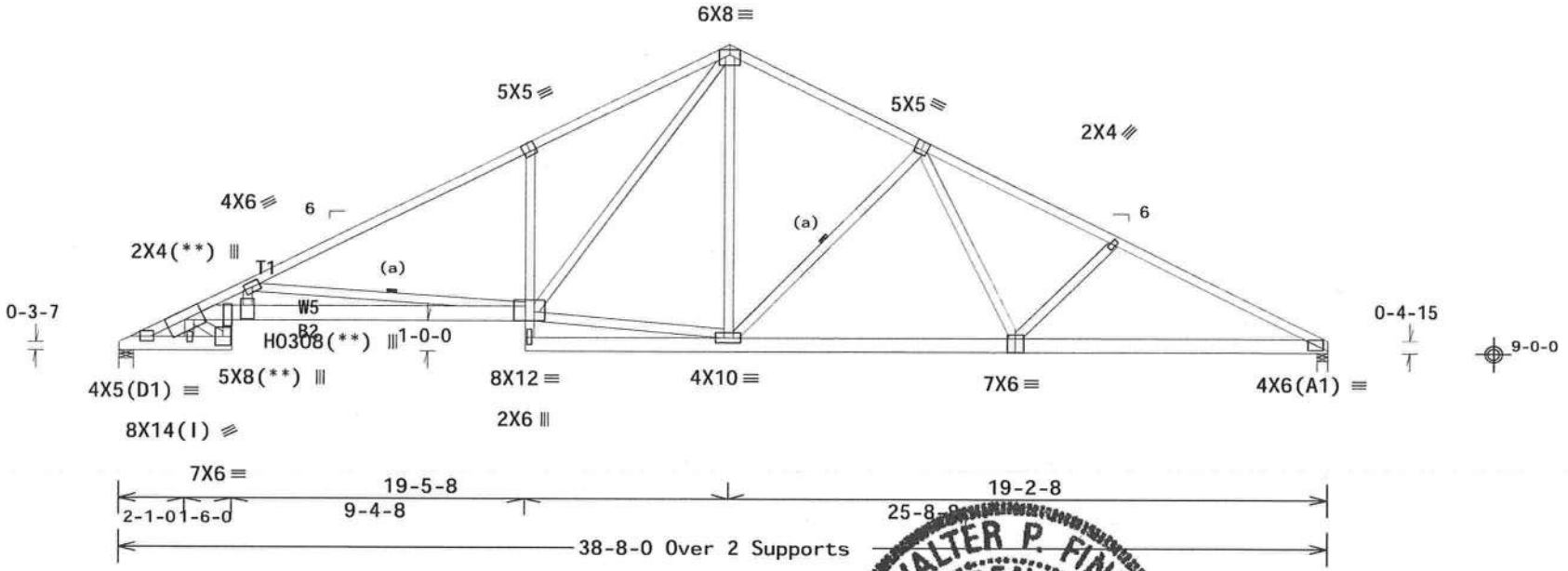
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/10(0) 12.03.05.0416.77 QTY: FL/-/1/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51068
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134051
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	206646
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - A3)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#2_N__12A :T1 2x4 SP_SS_12A:
Bot chord 2x6 SP_#2_N_12A :B2 2x6 SP_SS_12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Calculated horizontal deflection is 0.16" due to live load and 0.22" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

The overall height of this truss excluding overhang is 10-0-3.

(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

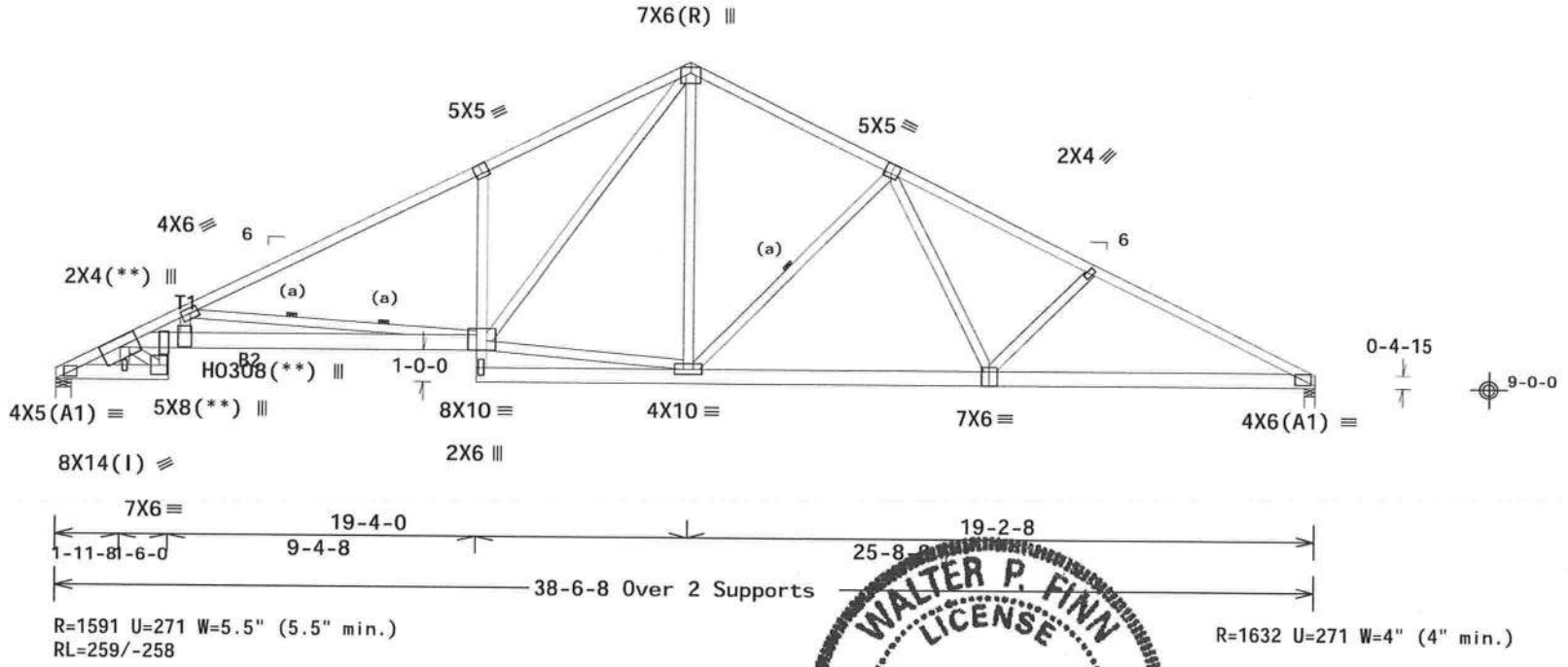
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD

PLT TYP. 20 Gauge HS,Wave

FT/RT=20%(0%)/10(0)

12.03.02.0446.17

QTY.

FL/-/1/-/-/R/-

Scale =.1875"/Ft.

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05/14/2013

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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51069
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134052
BC LL	0.0 PSF		HC-ENG DR/WPF
TOT.LD.	40.0 PSF	SEQN-	206638
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - A4)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#2_N__12A :T1 2x4 SP_SS_12A:
Bot chord 2x6 SP_SS_12A :B2 2x6 SP_#2_N_12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at 0.00 to 60 plf at 19.33
TC- From 60 plf at 19.33 to 60 plf at 25.29
BC- From 20 plf at 0.00 to 20 plf at 2.60
BC- From 10 plf at 2.60 to 10 plf at 8.60
BC- From 20 plf at 8.60 to 20 plf at 25.29
BC- 1298.76 lb Conc. Load at 2.60, 4.60, 6.60, 8.60

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The overall height of this truss excluding overhang is 10-0-3.

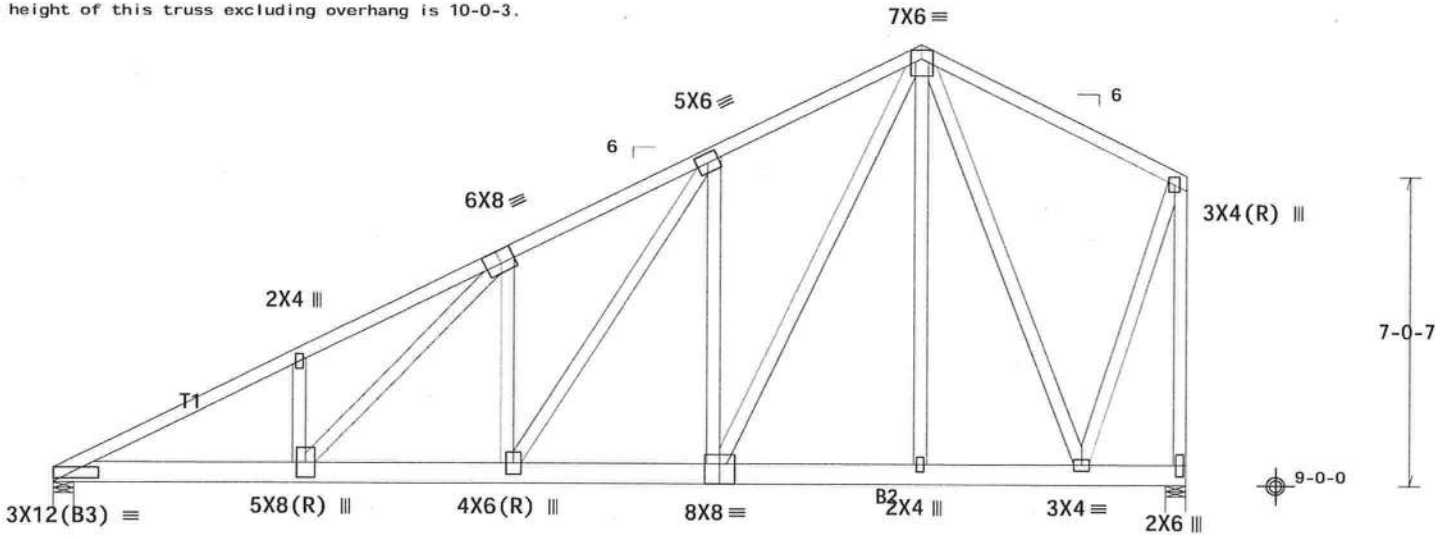
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 7.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.



19-4-0

25-3-8 Over 2 Supports

R=5038 U=313 W=5.5" (5.5" min.)

R=2[illegible]226 W=5.5" (5.5" min.)

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05/14/2013

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

★2.03.05.0446.77 QTY:

FL/-/1/-/-/R/-

Scale =.25"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R215-- 51070
DATE	05/14/13
DRW	HCUSR215 13134058
HC-ENG	DR/WPF
SEQN-	206627
FROM	CDM
JREF-	1UW8215_Z04

ALPINE
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FL COA #0 278

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(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - B)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Gable end supports 8" max rake overhang.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

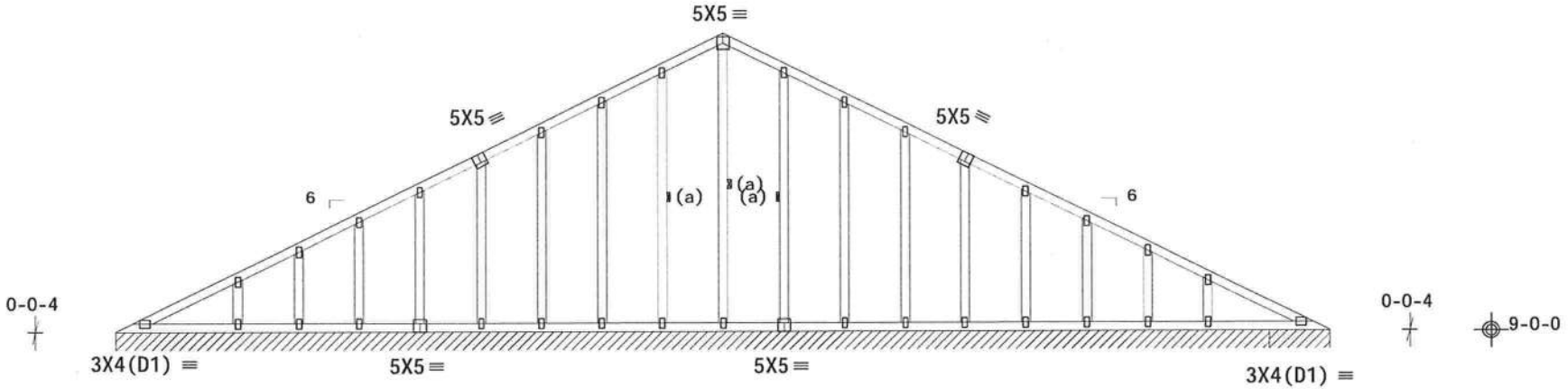
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

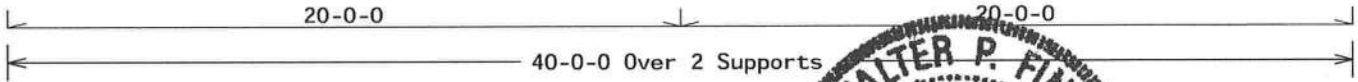
Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-0-4.





R=78 PLF U=15 PLF W=38-0-0
RL=7/-7 PLF

R=94 PLF U=0 PLF W=2-0-0

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) ★12.03.01.0103.11 QTY FL/-/1/-/-/R/- Scale =.1875"/Ft.

ALPINE
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Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

TC LL	20.0 PSF	REF	R215-- 51071
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134004
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	374348
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - B1)

Top chord 2x4 SP_SS_12A :T3 2x4 SP_#2_N__12A:
Bot chord 2x4 SP_#2_N__12A :B1 2x6 SP_SS_12A:
Webs 2x4 SP_#2_N__12A :W3, W5 2x4 SP_SS_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Truss designed for sleeping room only. No waterbeds permitted. Provide information to contractor, architect, and bldg owner. Trusses to be visibly stamped to indicate 30.00 psf MAX LL.

The overall height of this truss excluding overhang is 10-4-3.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

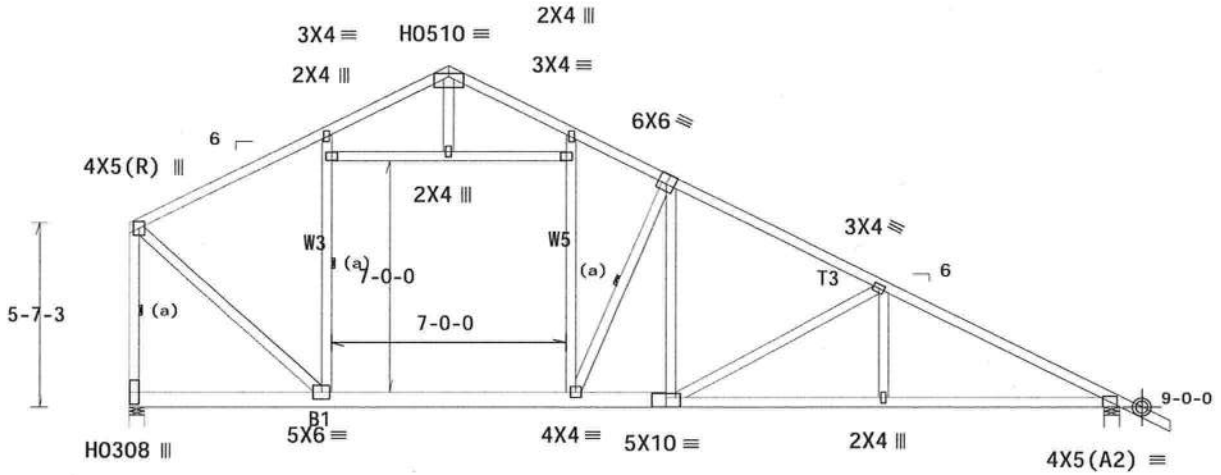
Calculated horizontal deflection is 0.22" due to live load and 0.39" due to dead load.

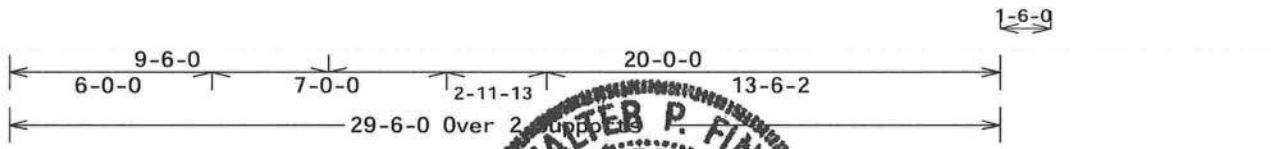
Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 30.00 psf; DL = 5.00 psf; from 6-0-0 to 13-0-0.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.57" due to live load and 1.00" due to dead load at X = 13-3-8.





R=1645 U=219 W=5.5" (5.5" min.)
RL=178/-235

R=1512 U=222 W=5.5" (5.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/10(0) ★2.03.01.0103.11 QTY: FL/-/1/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for practices prior to performing these functions. Installers shall provide temporary bracing per B Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom ch shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this desi any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	20.0 PSF	REF	R215-- 51072
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134056
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	375145
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - B2)

Top chord 2x4 SP_SS_12A :T3 2x4 SP_#2_N__12A:
Bot chord 2x4 SP_#2_N__12A :B1 2x6 SP_SS_12A:
Webs 2x4 SP_#2_N__12A :W3, W7 2x4 SP_SS_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Truss designed for sleeping room only. No waterbeds permitted. Provide information to contractor, architect, and bldg owner. Trusses to be visibly stamped to indicate 30.00 psf MAX LL.

The overall height of this truss excluding overhang is 10-4-3.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

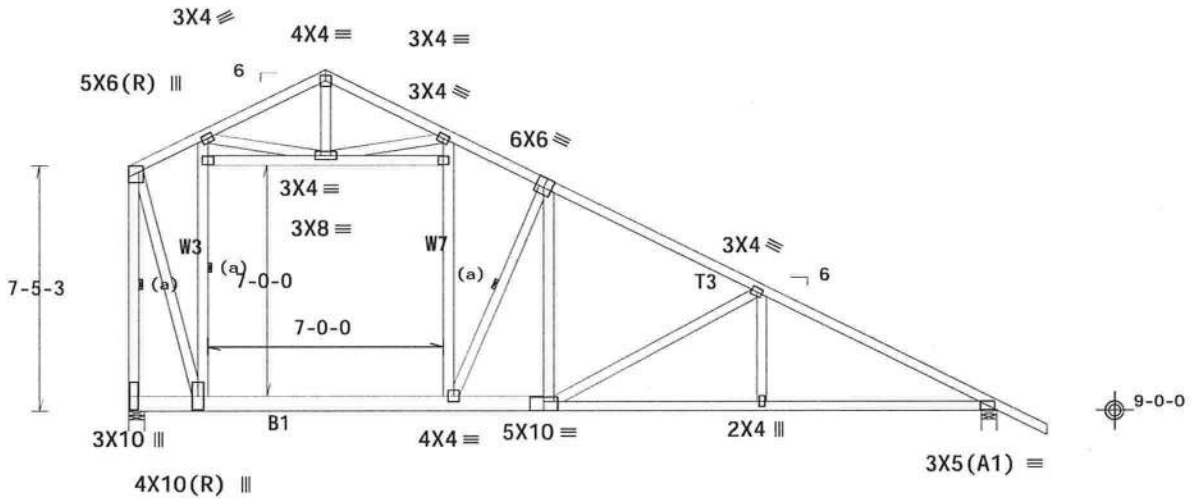
Calculated horizontal deflection is 0.30" due to live load and 0.42" due to dead load.

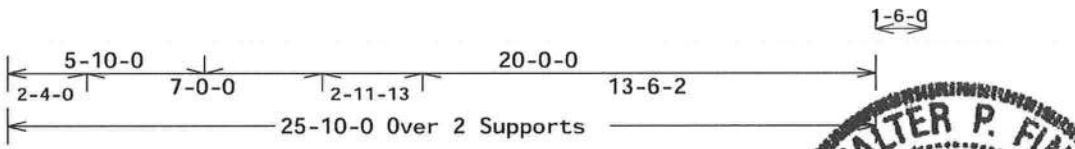
Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 30.00 psf; DL = 5.00 psf; from 6-0-0 to 13-0-0.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.58" due to live load and 0.82" due to dead load at X = 13-1-1.





R=1565 U=207 W=5.5" (5.5" min.)
RL=143/-225

R=1565 U=207 W=5.5" (5.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.01.0103.11 QTY 3 FL/-/1/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

TC LL	20.0 PSF	REF	R215-- 51073
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134057
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	375158
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - B3)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_SS_12A :B2 2x4 SP_#2_N__12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(a) Continuous lateral bracing equally spaced on member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

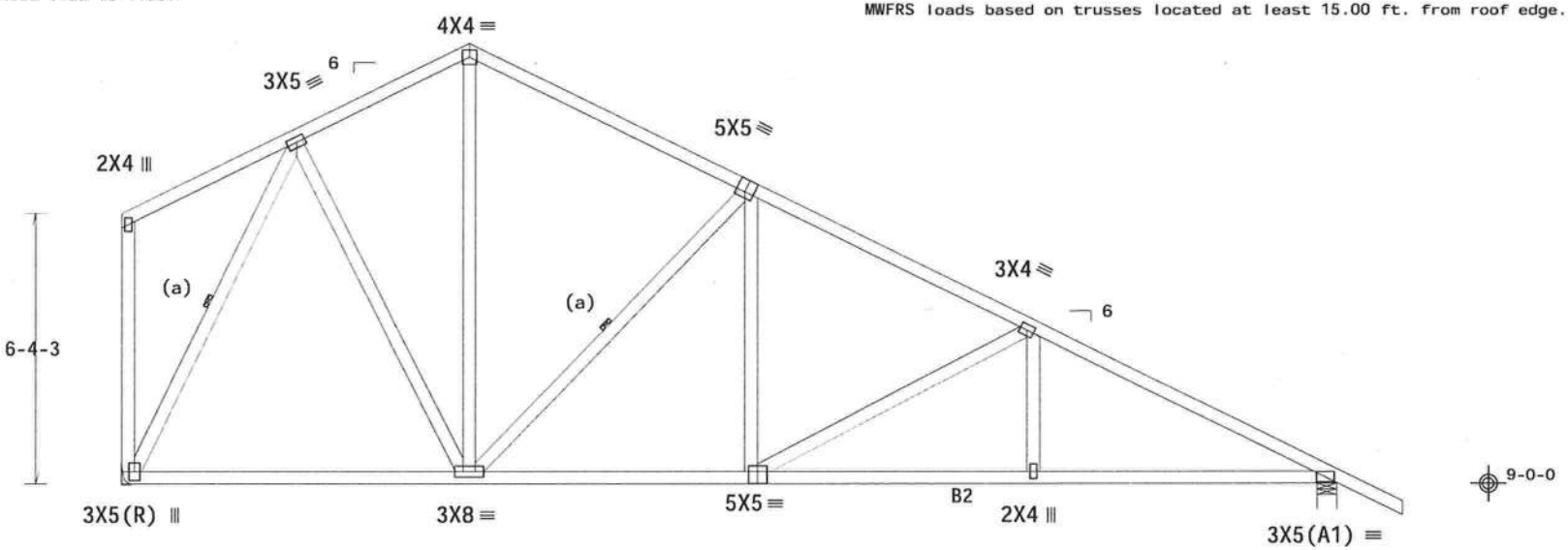
Wind loads and reactions based on MWFRS with additional C&C member design.

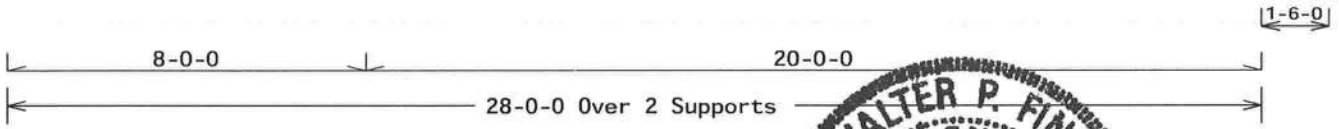
These support conditions used at bearings indicated
(H1) = ITW (KC) ATH29 w/ (2)2x8 SP_SS_12A supporting member.
Supported Member Face: (4) 0.131"x1.5" nails
Supporting Member Face: (4) 0.128"x3" nails
Supporting Member Top: (4) 0.128"x3" nails

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 10-4-3.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





R=1299 U=30
RL=163/-230
H=H1

R=1298 U=39 W=5.5"

Design Crit: FBC2010Res/TPI-2007(STD)
PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.01.0103.11 DTK: FL/-/1/-/-/R/- Scale =.25"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for general practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web members shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

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TC LL	20.0 PSF	REF	R215-- 51074
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134001
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	374354
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - C)

Top chord 2x4 SP_#2_N__12A :T2 2x6 SP_#2_N_12A:
Bot chord 2x4 SP_#2_N__12A :B2 2x4 SP_SS_12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Left end vertical not exposed to wind pressure.

Gable end supports 8" max rake overhang.

See DWGS A14030ENC100212 & GBLLETIN0212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 11-6-4.

Plates extending outside the truss perimeter shall be positioned within the tolerance specified on the plate placement polygon only, without use of TPI 1-2007 section 3.7.2.2 alternate positioning. Steel extending above the top chord or below the bottom chord may be trimmed or folded along the outer edge of that chord. Steel extending elsewhere beyond outermost truss members may be folded.

130 mph wind, 15.91 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

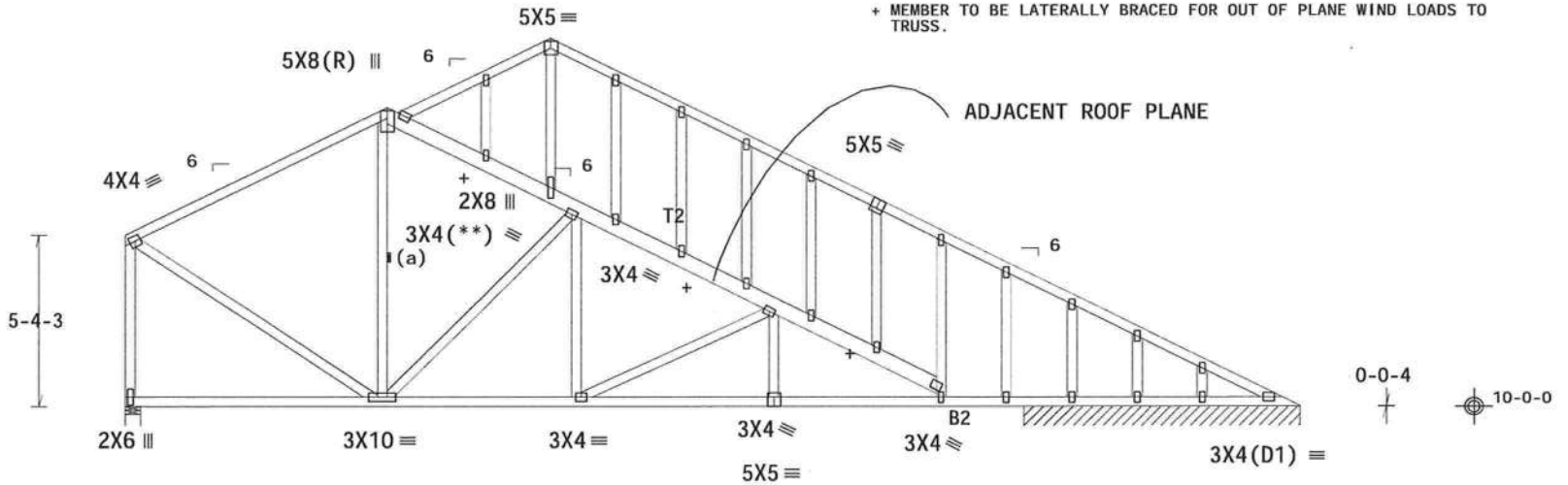
(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 31.81 ft. from roof edge.

+LATERALLY BRACE TOP CHORD BELOW FILLER AT 24" O.C. OR RIGID SHEATING, INCLUDING A LATERAL BRACE AT CHORD ENDS.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS.



8-0-0 | 5-0-0 | 11-10-4 | 11-1-12

36-0-0 Over 2 Supports

R=1243 U=0 W=5.5" (5.5" min.)
RL=199/-263

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.01.0103.11 QTY 1 FL/-/1/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51075
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134008
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	374259
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - C1)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

(I) - plates so marked were sized using a Fabrication Tolerance of 0% and a Rotational Tolerance of 0 degrees.

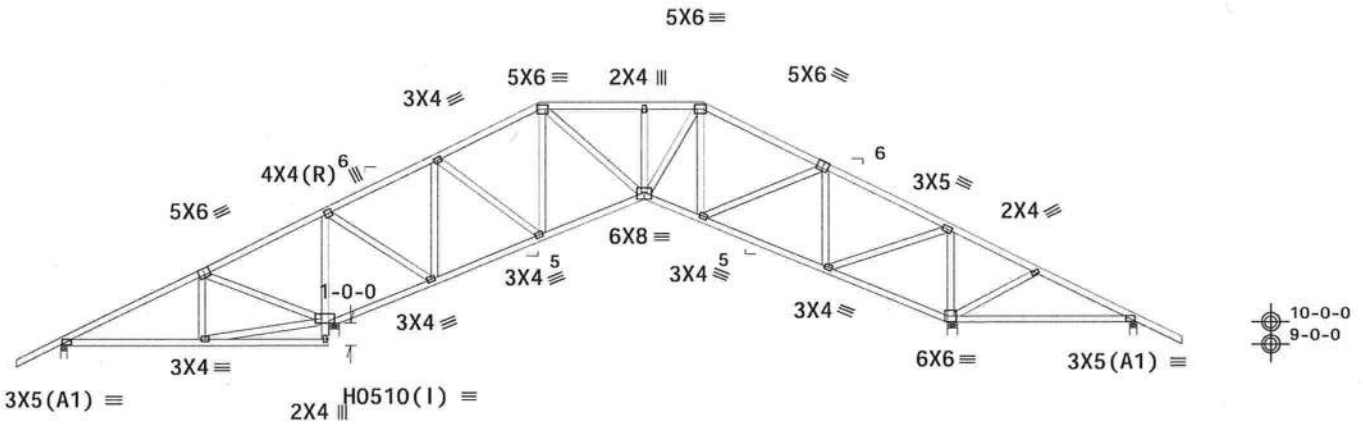
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

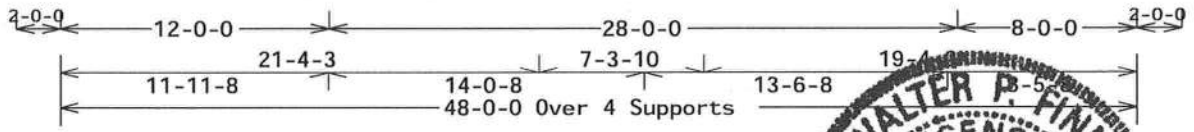
Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The overall height of this truss excluding overhang is 11-0-5.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.





R=436 U=65 W=3.5" (3.5" min.)
RL=342/-331

R=1905 U=51 W=5.5" (5.5" min.)

R=1779 U=12 W=5.5" (5.5" min.)

R=292 U=63 W=3.5" (3.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=20%(0%)/10(0)

12.03.01.0163.11

QTY: 17

FL/-/1/-/-/R/-

Scale =.125"/Ft.

WALTER P. FINA LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51076
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134053
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	375097
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - C2)

Top chord 2x4 SP_#2_N__12A :T4 2x4 SP_#2_N__12A:
T3 2x6 SP_#2_N_12A:
Bot chord 2x4 SP_SS_12A :B4 2x4 SP_#2_N__12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Calculated horizontal deflection is 0.09" due to live load and 0.18" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The overall height of this truss excluding overhang is 11-0-4.

+LATERALLY BRACE TOP CHORD BELOW FILLER AT 24" O.C, OR RIGID SHEATING, INCLUDING A LATERAL BRACE AT CHORD ENDS.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS.

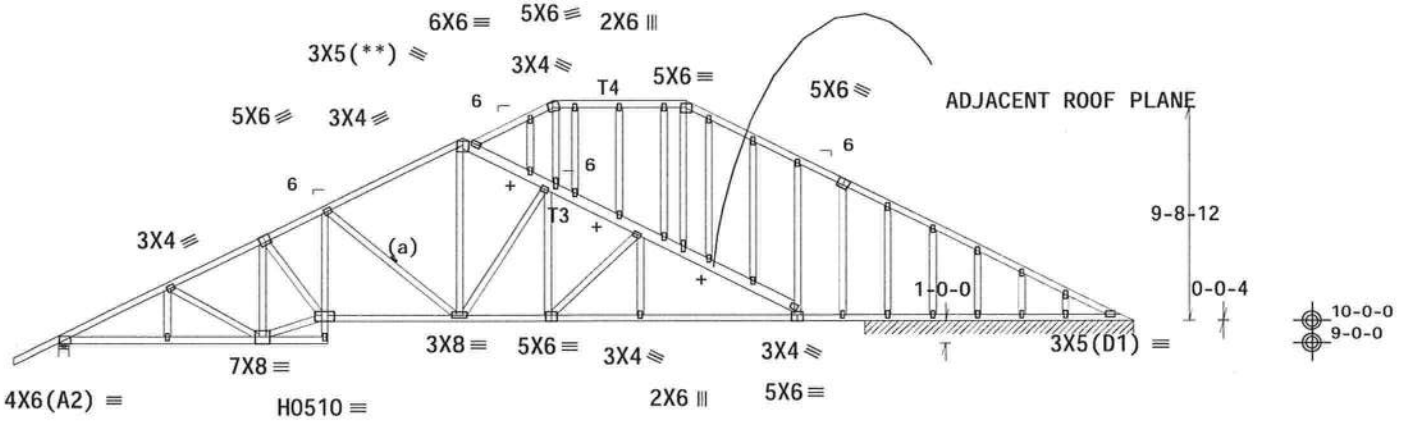
Plates extending outside the truss perimeter shall be positioned within the tolerance specified on the plate placement polygon only, without use of TPI 1-2007 section 3.7.2.2 alternate positioning. Steel extending above the top chord or below the bottom chord may be trimmed or folded along the outer edge of that chord. Steel extending elsewhere beyond outermost truss members may be folded.

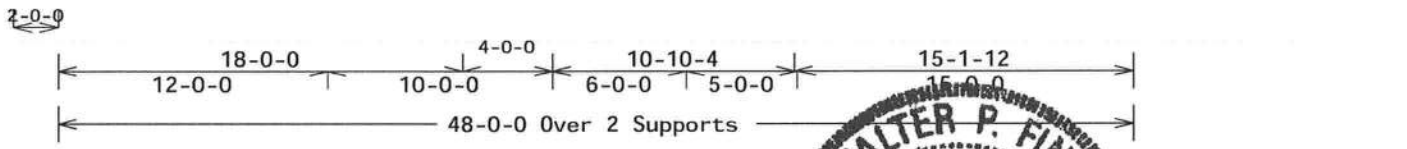
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.





R=1839 U=322 W=5.5" (5.5" min.)
RL=307/-316

R=1[illegible] U=32[illegible] W=12.0"

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave | FT/RT=20%(0%)/10(0) | 12.03.01.0103.11 | QTY 1 | FL/-/1/-/-/R/- | Scale =.125"/Ft.

WALTER P. FINAN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF R215-- 51077
TC DL	10.0 PSF	DATE 05/14/13
BC DL	10.0 PSF	DRW HCUSR215 13134005
BC LL	0.0 PSF	HC-ENG DR/DF
TOT.LD.	40.0 PSF	SEQN- 374414
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - D)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_SS_12A :B1 2x4 SP_#2_N__12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Gable end supports 8" max rake overhang.

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

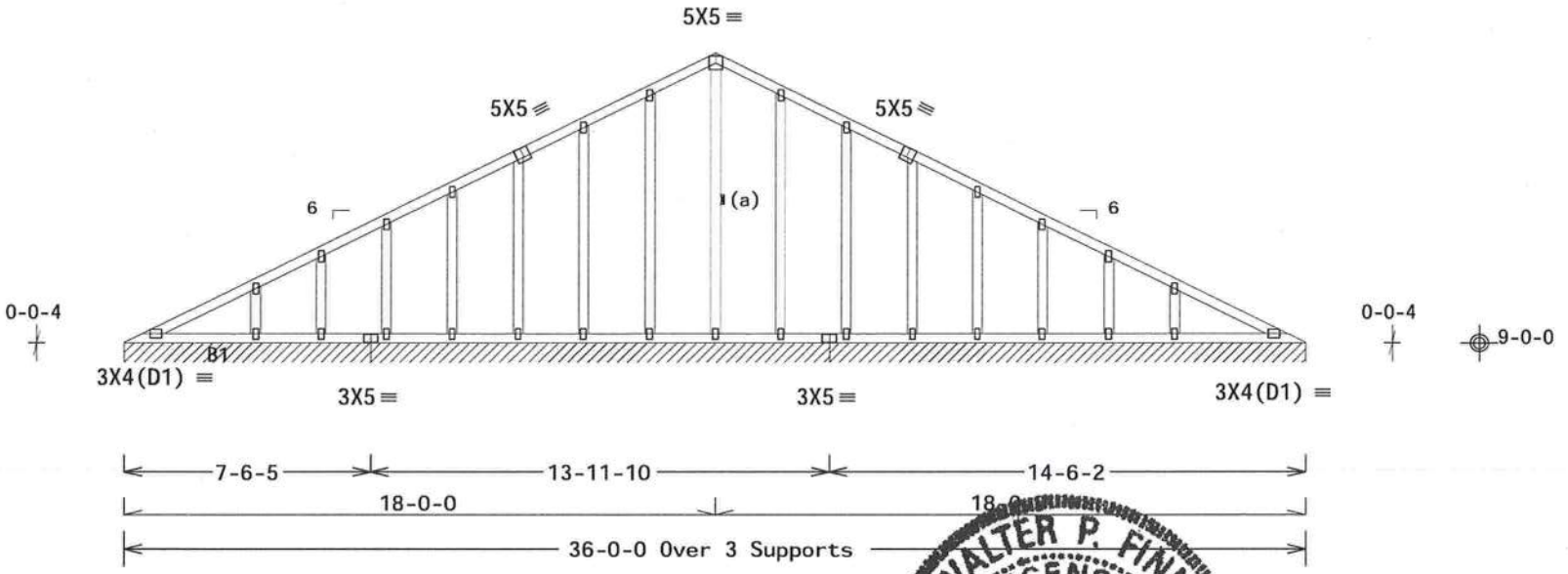
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 9-0-4.



R=73 PLF U=11 PLF W=7-6-5
RL=31/-31 PLF
R=83 PLF U=11 PLF W=13-11-10
R=81 PLF U=18 PLF W=14-6-2

Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD

PLT TYP. Wave FT/RT=20%(0%)/10(0) 12:03.01.0103.11 QTY: FL/-/1/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51078
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134003
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	374346
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - D1)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_SS_12A :B1 2x4 SP_#2_N__12A:
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

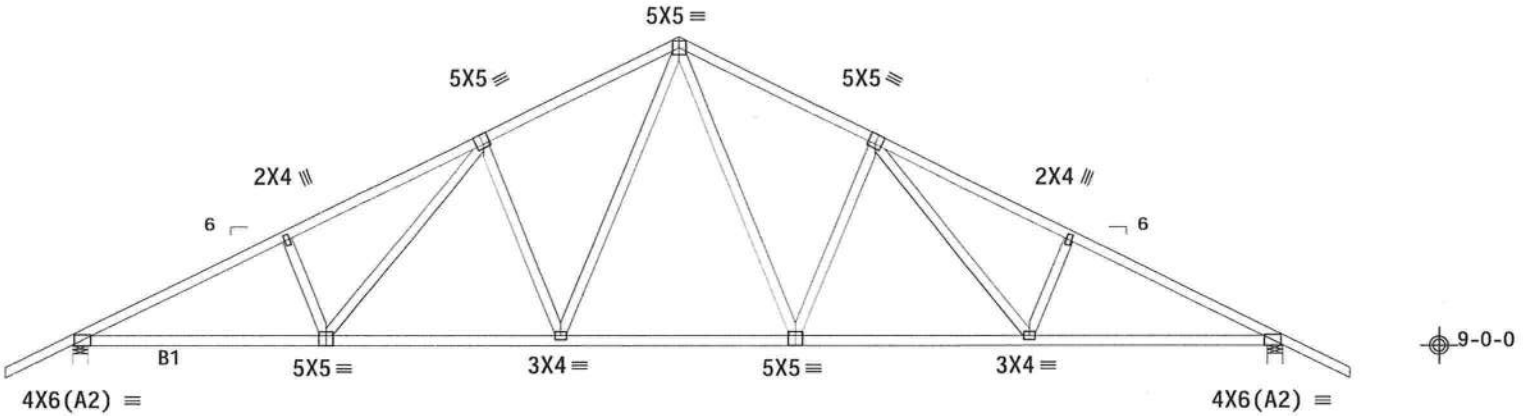
The overall height of this truss excluding overhang is 9-4-3.

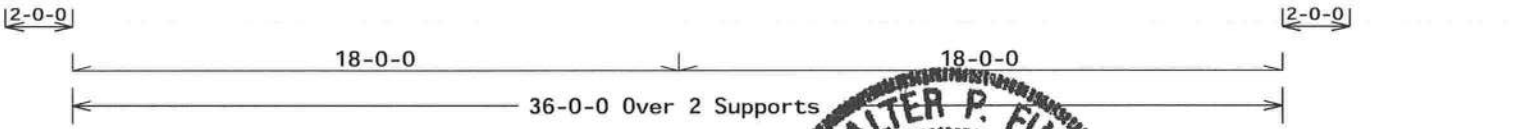
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1744 U=289 W=5.5" (5.5" min.)
RL=290/-290

R=1744 U=289 W=5.5" (5.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=20%(0%)/10(0) | 12.03.01.0103.11 | QTY 6 | FL/-/1/-/-/R/- | Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51079
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134002
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	373185
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - G)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

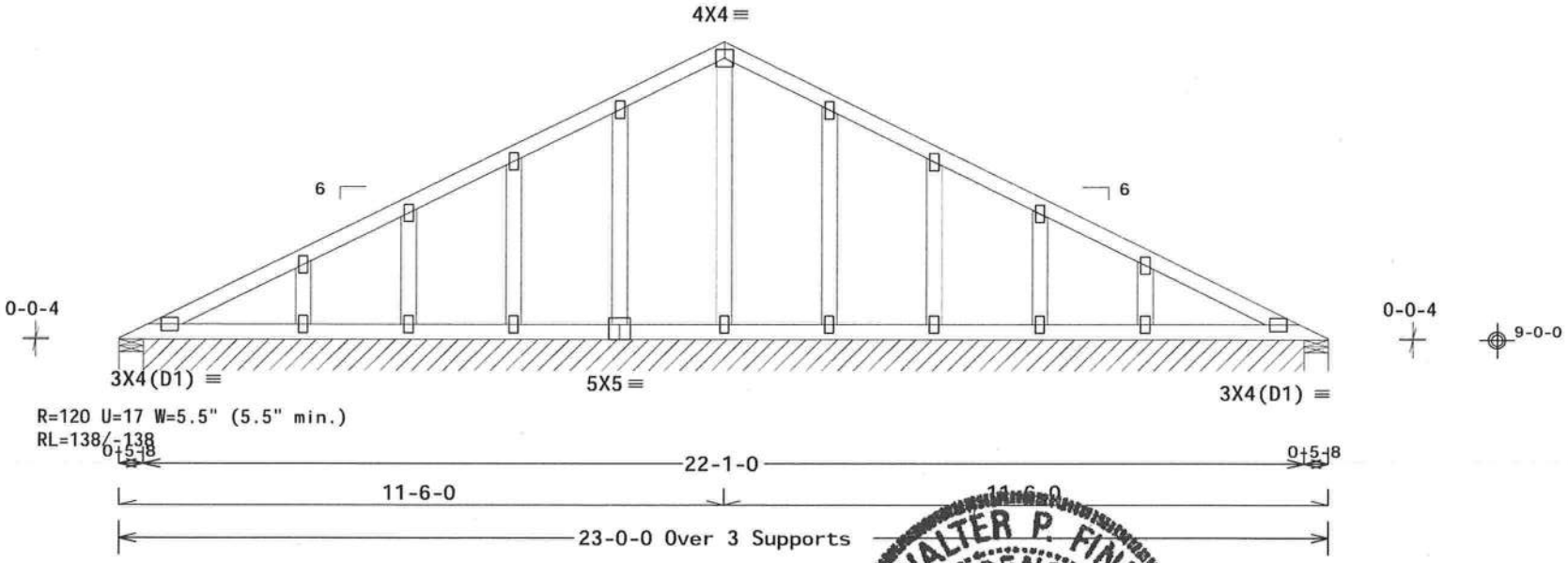
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 5-9-4.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0) ★2.03.05.0★6.77 QTY:

FL/-/1/-/-/R/- Scale =.3125"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51080
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134054
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	206652
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

(8146C-/WILSON RESIDENCE /Contractor -- LAKE CITY, FL - G1)

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

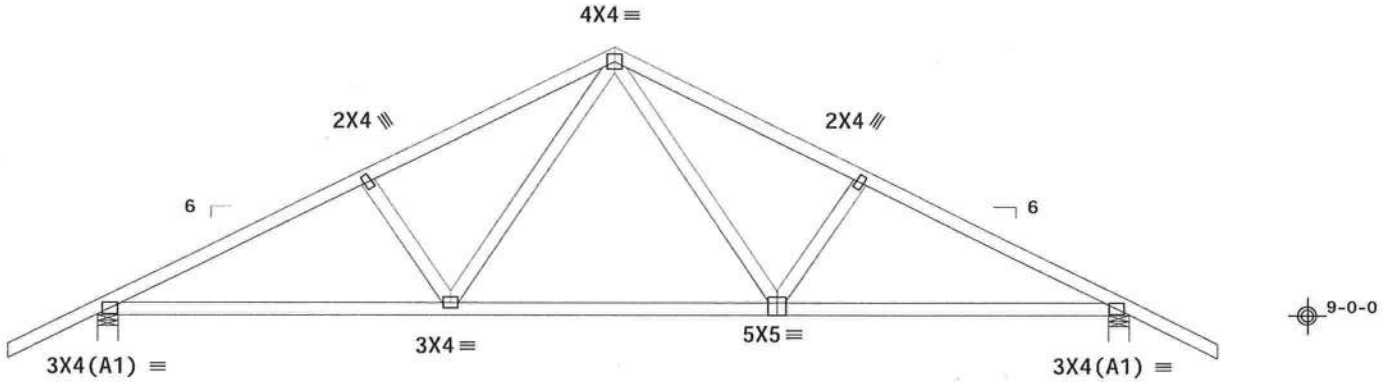
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

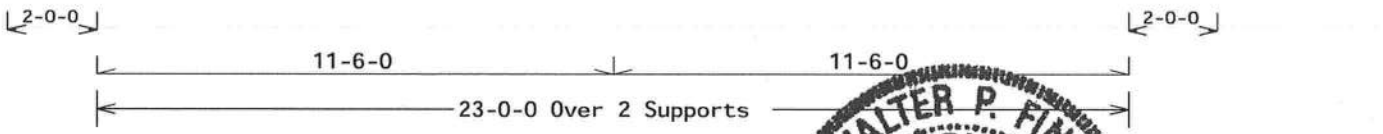
130 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

The overall height of this truss excluding overhang is 6-1-3.





R=1049 U=196 W=5.5" (5.5" min.)
RL=197/-197

R=1049 U=196 W=5.5" (5.5" min.)

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0) 12.03.05.0446.17 QTY 1

FL/-/1/-/-/R/- Scale =.25"/Ft.

WALTER P. FINA LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
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FL COA #0 278

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TC LL	20.0 PSF	REF	R215-- 51081
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134055
BC LL	0.0 PSF	HC-ENG	DR/WPF
TOT.LD.	40.0 PSF	SEQN-	206650
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

The overall height of this truss excluding overhang is 1-9-15.

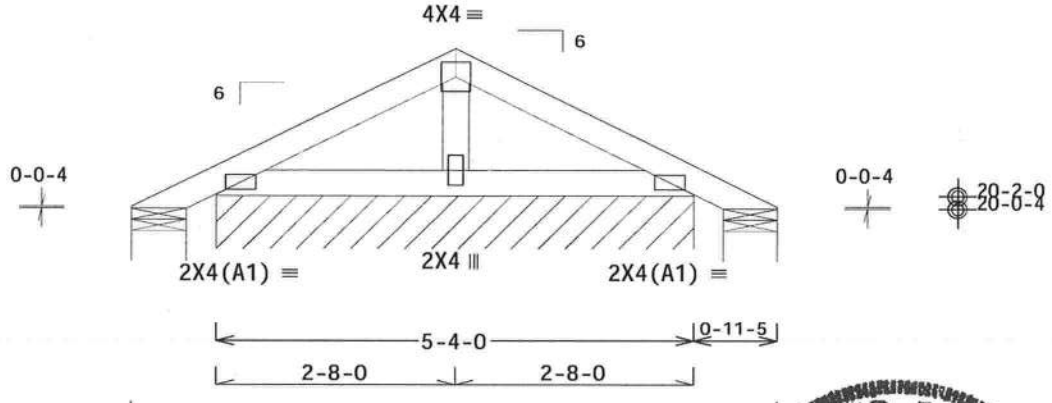
REFER TO PB160100212 FOR PIGGYBACK DETAILS.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 60 plf at -0.94 to 60 plf at 2.67
TC- From 60 plf at 2.67 to 60 plf at 6.28
BC- From 4 plf at -0.94 to 4 plf at 6.28

130 mph wind, 20.94 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 10.47 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)
PLT TYP. Wave FT/RT=20%(0%)/10(0) 12.03.01.0103.11 QTY 17 FL/-/1/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
05/14/2013

ALPINE
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TC LL	20.0 PSF	REF R215-- 51082
TC DL	10.0 PSF	DATE 05/14/13
BC DL	10.0 PSF	DRW HCUSR215 13134006
BC LL	0.0 PSF	HC-ENG DR/DF
TOT.LD.	40.0 PSF	SEQN- 373325
DUR.FAC.	1.25	FROM CDM
SPACING	24.0"	JREF- 1UW8215_Z04

Top chord 2x4 SP_#2_N__12A
Bot chord 2x4 SP_#2_N__12A
Webs 2x4 SP_#2_N__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

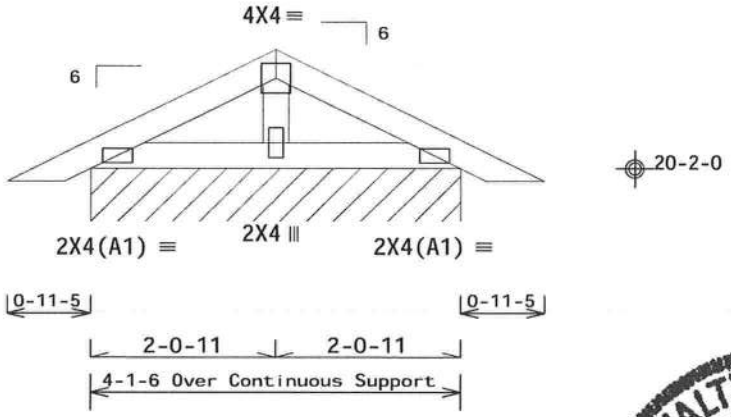
The overall height of this truss excluding overhang is 12-6-4.

REFER TO PB160100212 FOR PIGGYBACK DETAILS.

130 mph wind, 20.77 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=99 PLF U=29 PLF W=4-1-6
RL=11/-11 PLF

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/10(0) 12.03.01.0103.11 QTY 1 FL/-/1/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

ALPINE
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TC LL	20.0 PSF	REF	R215-- 51083
TC DL	10.0 PSF	DATE	05/14/13
BC DL	10.0 PSF	DRW	HCUSR215 13134007
BC LL	0.0 PSF	HC-ENG	DR/DF
TOT.LD.	40.0 PSF	SEQN-	374409
DUR.FAC.	1.25	FROM	CDM
SPACING	24.0"	JREF-	1UW8215_Z04

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

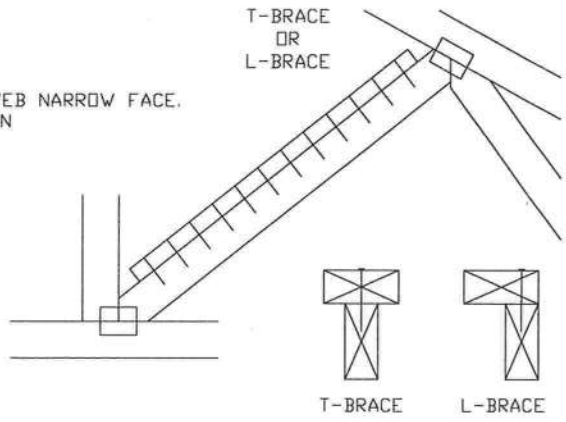
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

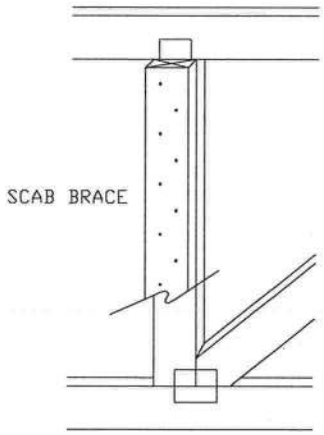
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



WALTER P. FINN
LICENSE
No. 22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
05/14/2013

May 14 '13

ITW
Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and botton chord shall have properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more info.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	7' 2"	7' 5"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	6' 11"	7' 5"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
	SP DFL	#1	4' 4"	7' 4"	7' 7"	8' 8"	9' 0"	10' 3"	10' 8"	13' 7"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	5' 11"	6' 4"	7' 11"	8' 5"	10' 1"	10' 6"	12' 5"	13' 3"	14' 0"	14' 0"
		Stud	4' 1"	5' 11"	6' 4"	7' 11"	8' 5"	10' 1"	10' 6"	12' 5"	13' 3"	14' 0"	14' 0"
		Standard	3' 11"	5' 2"	5' 5"	6' 10"	7' 4"	9' 3"	9' 11"	10' 9"	11' 6"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 2"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	8' 2"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 0"	8' 5"	8' 8"	9' 11"	10' 3"	11' 9"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	7' 3"	7' 9"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	7' 3"	7' 9"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 3"	6' 8"	8' 5"	8' 11"	11' 4"	12' 1"	13' 2"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	9' 3"	9' 7"	10' 11"	11' 4"	12' 11"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	8' 4"	8' 11"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	8' 4"	8' 11"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 3"	7' 9"	9' 8"	10' 4"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

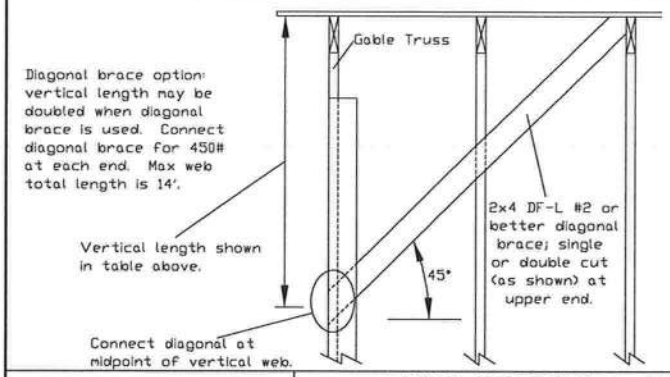
** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

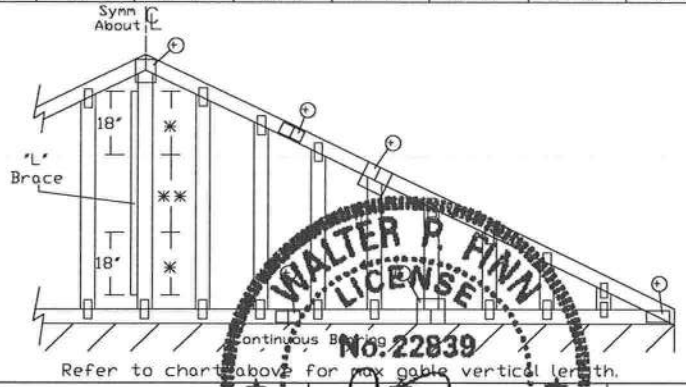
'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2.5X4
Greather than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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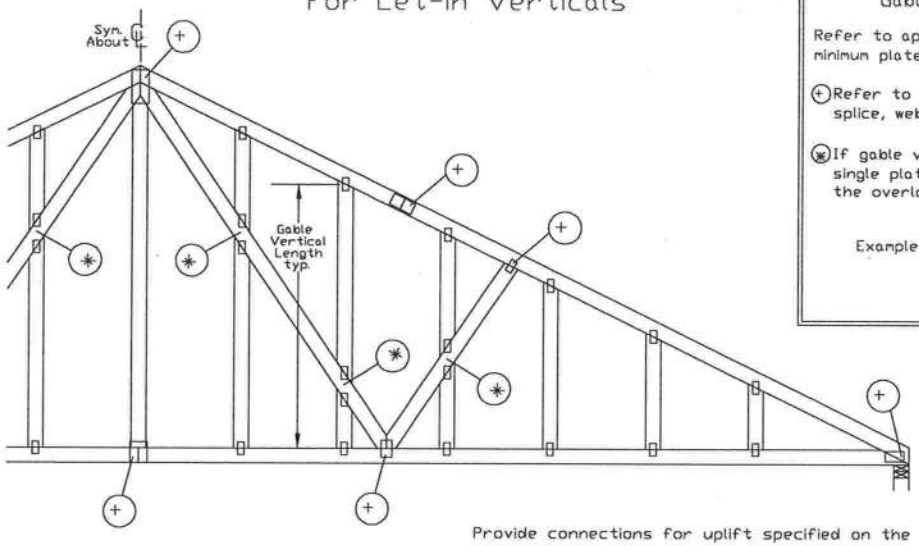
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB14015
DATE	2/14/12
DRWG	A14015ENC100212

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

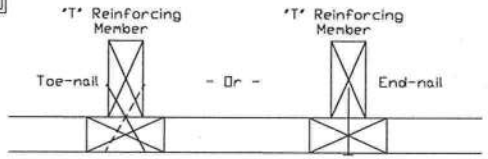
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X4 2X8

'T' Reinforcement Attachment Detail

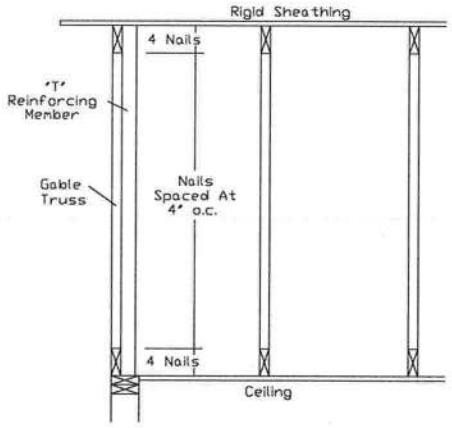


Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A1001505[illegible] A14015050109,
A13030050109, A12030050109, A11030050109, A1003[illegible]050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015[illegible]212, A10015PED100212,
A18015ENC100212, A20015ENC100212, A2001[illegible]212, A20015PED100212,
A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212,
A18030ENC100212, A20030ENC100212, A2003[illegible]212, A20030PED100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate ITW gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ "T" Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 Ft, Kzt = 1.00
Gable Vertical = 24'o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF LET-IN VERT
DATE 2/16/12
DRWG GBLLETIN0212

Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace * Group A	(1) 1x4 'L' Brace * Group B	(1) 2x4 'L' Brace * Group A	(1) 2x4 'L' Brace * Group B	(2) 2x4 'L' Brace ** Group A	(2) 2x4 'L' Brace ** Group B	(1) 2x6 'L' Brace * Group A	(1) 2x6 'L' Brace * Group B	(2) 2x6 'L' Brace ** Group A	(2) 2x6 'L' Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Stud	3' 10"	6' 10"	7' 1"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	3' 10"	6' 5"	6' 10"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 9"	10' 2"	12' 11"	13' 5"	14' 0"	14' 0"
		#2	4' 1"	6' 10"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	5' 6"	5' 10"	7' 4"	7' 10"	9' 8"	10' 0"	11' 6"	12' 4"	14' 0"	14' 0"
		Stud	3' 10"	5' 6"	5' 10"	7' 4"	7' 10"	9' 8"	10' 0"	11' 6"	12' 4"	14' 0"	14' 0"
		Standard	3' 8"	4' 9"	5' 1"	6' 4"	6' 10"	8' 7"	9' 3"	10' 0"	10' 8"	13' 7"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 9"	8' 0"	8' 3"	9' 5"	9' 9"	11' 2"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	6' 9"	7' 2"	9' 0"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	6' 9"	7' 2"	9' 0"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	5' 10"	6' 3"	7' 9"	8' 4"	10' 6"	11' 3"	12' 3"	13' 1"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 3"	8' 9"	9' 1"	10' 4"	10' 9"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	7' 9"	8' 3"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	7' 9"	8' 3"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	6' 9"	7' 2"	9' 0"	9' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch	Southern Pine***
#3	#3
Stud	Stud
Standard	Standard

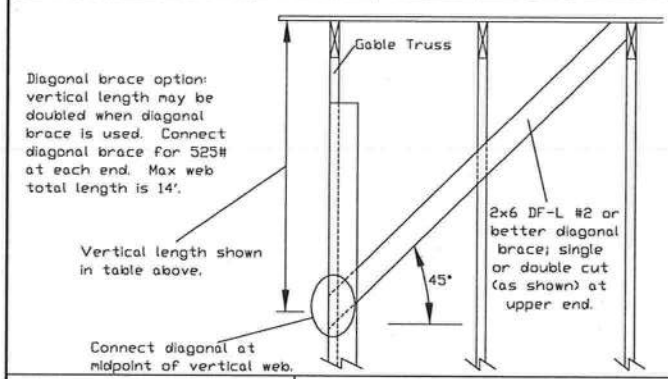
Group B:

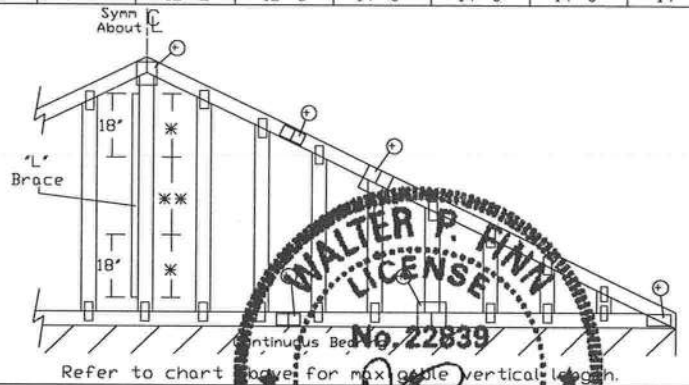
Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch	Southern Pine***
#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.





Refer to chart above for max gable vertical length.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' braces: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

**For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1.5X4
Greater than 4' 0", but less than 11' 6"	2.5X4
Greather than 11' 6"	3.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

05/14/2013

May 14 '13

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for general practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-10-GAB14030
DATE 2/14/12
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