

DATE 03/17/2011

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000029248

APPLICANT	AARON NICKELSON		PHONE	386.487.1466	
ADDRESS	426	SW COMMERCE DRIVE, TE. 130	LAKE CITY	FL	32025
OWNER	ANDREW SCHNEIDER		PHONE	386.623.6066	
ADDRESS	324	NIGHTSHADE DRIVE	LAKE CITY	FL	32024
CONTRACTOR	AARON NICKELSON		PHONE	386.487.1466	
LOCATION OF PROPERTY	47-S TO WESTER RD, TL GO TO S/D, TL GO TO DANDELION, TL TO NIGHTSHADE, TR LAST HOME ON R.				
TYPE DEVELOPMENT	ATTACHED GARAGE		ESTIMATED COST OF CONSTRUCTION	82400.00	
HEATED FLOOR AREA	563.00	TOTAL AREA	1648.00	HEIGHT	23.00
FOUNDATION	CONC	WALLS	FRAMED	ROOF PITCH	9'12
LAND USE & ZONING	RSF-1		MAX. HEIGHT	35	
Minimum Set Back Requirements:	STREET-FRONT		25.00	REAR	15.00
				SIDE	10.00
NO. EX.D.U.	1	FLOOD ZONE	X	DEVELOPMENT PERMIT NO.	
PARCEL ID	30-4S-17-08898-119		SUBDIVISION	WESTER WOODS	
LOT	19	BLOCK		UNIT	
		PHASE		TOTAL ACRES	1.19
		CBC1258040			
Culvert Permit No.	Culvert Waiver	Contractor's License Number		Applicant/Owner/Contractor	
EXISTING		BLK		TC	
Driveway Connection	Septic Tank Number	LU & Zoning checked by		Approved for Issuance	New Resident
COMMENTS:	ACCESSORY USE. NOC ON FILE.				

Check # or Cash 2709

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power	Foundation	Monolithic
date/app. by	date/app. by	date/app. by
Under slab rough-in plumbing	Slab	Sheathing/Nailing
date/app. by	date/app. by	date/app. by
Framing	Insulation	
date/app. by	date/app. by	
Rough-in plumbing above slab and below wood floor	Electrical rough-in	
date/app. by	date/app. by	
Heat & Air Duct	Peri. beam (Lintel)	Pool
date/app. by	date/app. by	date/app. by
Permanent power	C.O. Final	Culvert
date/app. by	date/app. by	date/app. by
Pump pole	Utility Pole	M/H tie downs, blocking, electricity and plumbing
date/app. by	date/app. by	date/app. by
Reconnection	RV	Re-roof
date/app. by	date/app. by	date/app. by

BUILDING PERMIT FEE \$	415.00	CERTIFICATION FEE \$	8.24	SURCHARGE FEE \$	8.24
MISC. FEES \$	0.00	ZONING CERT. FEE \$	50.00	FIRE FEE \$	0.00
FLOOD DEVELOPMENT FEE \$		FLOOD ZONE FEE \$		CULVERT FEE \$	
INSPECTORS OFFICE		CLERKS OFFICE		TOTAL FEE	481.48

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECEIVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECEIVED AN APPROVED INSPECTION WITHIN 180 DAYS OF THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL 1999
Page 1 of 1 Document ID: IU9Q487-Z0124094559

Truss Fabricator: Anderson Truss Company
Job Identification: 11-036--fill in later AARON NICKELSON/SCHNEIDER -- , **
Truss Count: 21
Model Code: Florida Building Code 2007 and 2009 Supplement
Truss Criteria: FBC2007Res/TPI-2002(STD); FBC2007Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 9.05.
Structural Engineer of Record: The identity of the structural EOR did not exist as of the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - 55.0 PSF @ 1.00 Duration
Wind - 110 MPH ASCE 7-05 - Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: A1103005-GBLLETIN-A1101505-PB120-

#	Ref	Description	Drawing#	Date
1	57014--A	11055007	02/24/11	
2	57015--A1	11055008	02/24/11	
3	57016--AG1	11055009	02/24/11	
4	57017--AG2	11055010	02/24/11	
5	57018--AGE	11055011	02/24/11	
6	57019--AG3	11055012	02/24/11	
7	57020--B	11055001	02/24/11	
8	57021--C	11055002	02/24/11	
9	57022--CGE	11055013	02/24/11	
10	57023--C1	11055003	02/24/11	
11	57024--DGE	11055014	02/24/11	
12	57025--D	11055004	02/24/11	
13	57026--F	11055015	02/24/11	
14	57027--FG	11055016	02/24/11	
15	57028--FGE	11055017	02/24/11	
16	57029--F1	11055018	02/24/11	
17	57030--F1G	11055019	02/24/11	
18	57031--M	11055005	02/24/11	
19	57032--M1	11055006	02/24/11	
20	57033--AP	11055020	02/24/11	
21	57034--APGE	11055021	02/24/11	

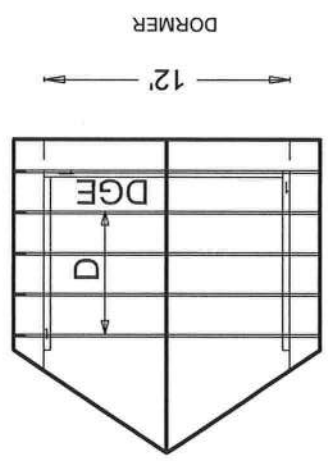


1950 Marley Drive
Haines City, FL 33844

Douglas M. Fleming
-Truss Design Engineer-

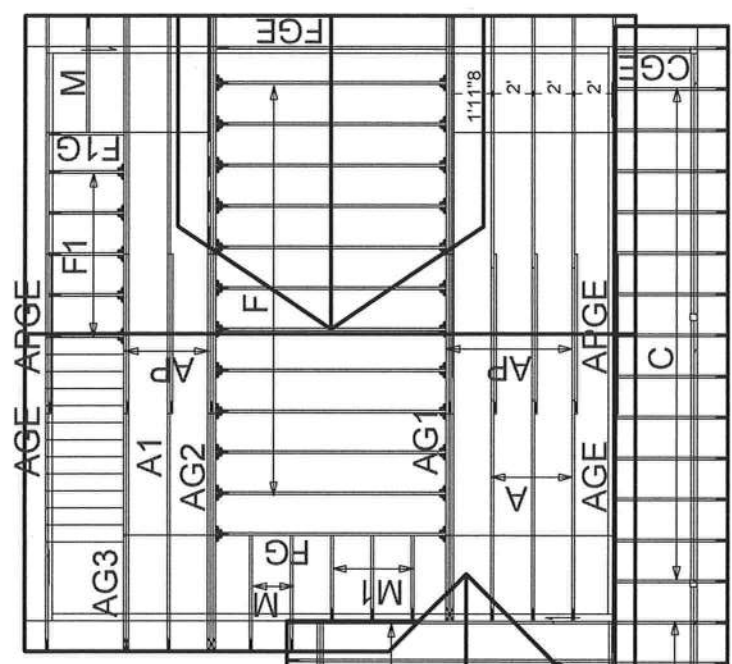


AARON NICKELSON/SCHNEIDER



DORMER

13'8"



JOB DESCRIPTION: Fill in later
/ : AARON NICKELSON/SCHNEIDER

JOB NO:
11-036

PAGE NO:

(**) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.73 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

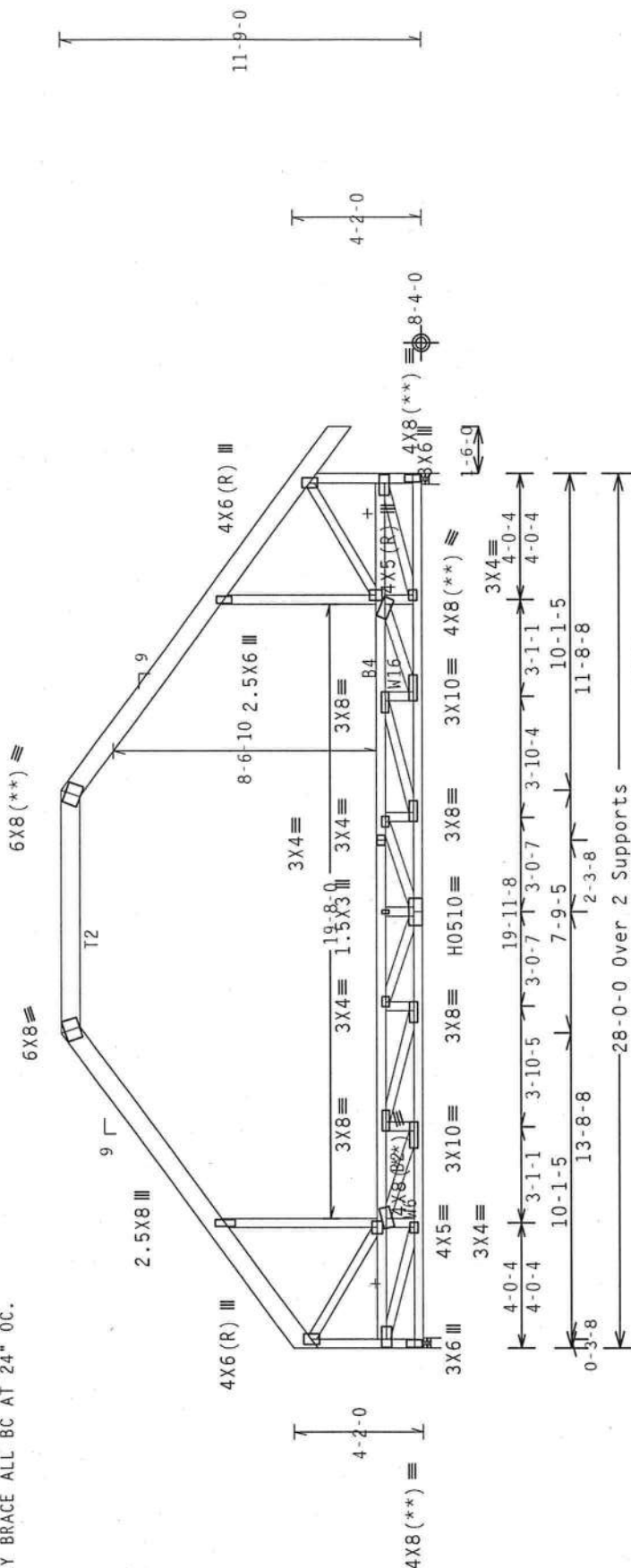
root overhang supports 2.00 psi soil load.

Calculated horizontal deflection is 0.19" due to live load and 0.20" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

3C attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 4-2-0 to 23-10-0.

+ Laterally brace all BC at 24" OC.



R=2517 U=106 W=4"
RL=212/-197

R-2628 U-118 W-4"

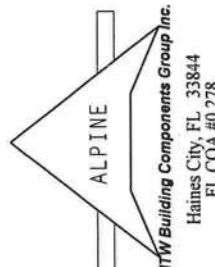
PLT TYP. 20 Gauge HS. Wave

Design Crit: FBC2007Res/TPI-2002 (5/11/07)
FT/RT=10%(0%)/0(0)

Scale = .1875"/Ft.

TC LL	20.0	PSF	REF R487--	57014
TC DL	10.0	PSF	DATE	02/24/11
BC DL	10.0	PSF	DRW	HCUSR487 11055007
BC LL	0.0	PSF	HC-ENG	DF/DF
TOT.LD.	40.0	PSF	SEQN-	184553
DUR.FAC.	1.25			
SPACING	24.0"		JREF-	1U9Q487_Z01

02/24/2011



	Top	chord	2x8	SP	SS	T2	2x8	SP	#1	Dense:
Bot	chord	2x4	SP	#1	:B2,	B4	2x4	SP	#2	Dense:
	webs	2x4	SP	#3	:W6,	W16	2x4	SP	#2	Dense:

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.16" due to live load and 0.24" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

8C attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 4-2-0 to 23-10-0.

Laterally brace chord member above filler @ 24" O.C. including a lateral brace at chord

(**) 5 plate(s) require special positioning. Refer to scaled plate lot details for special positioning requirements.

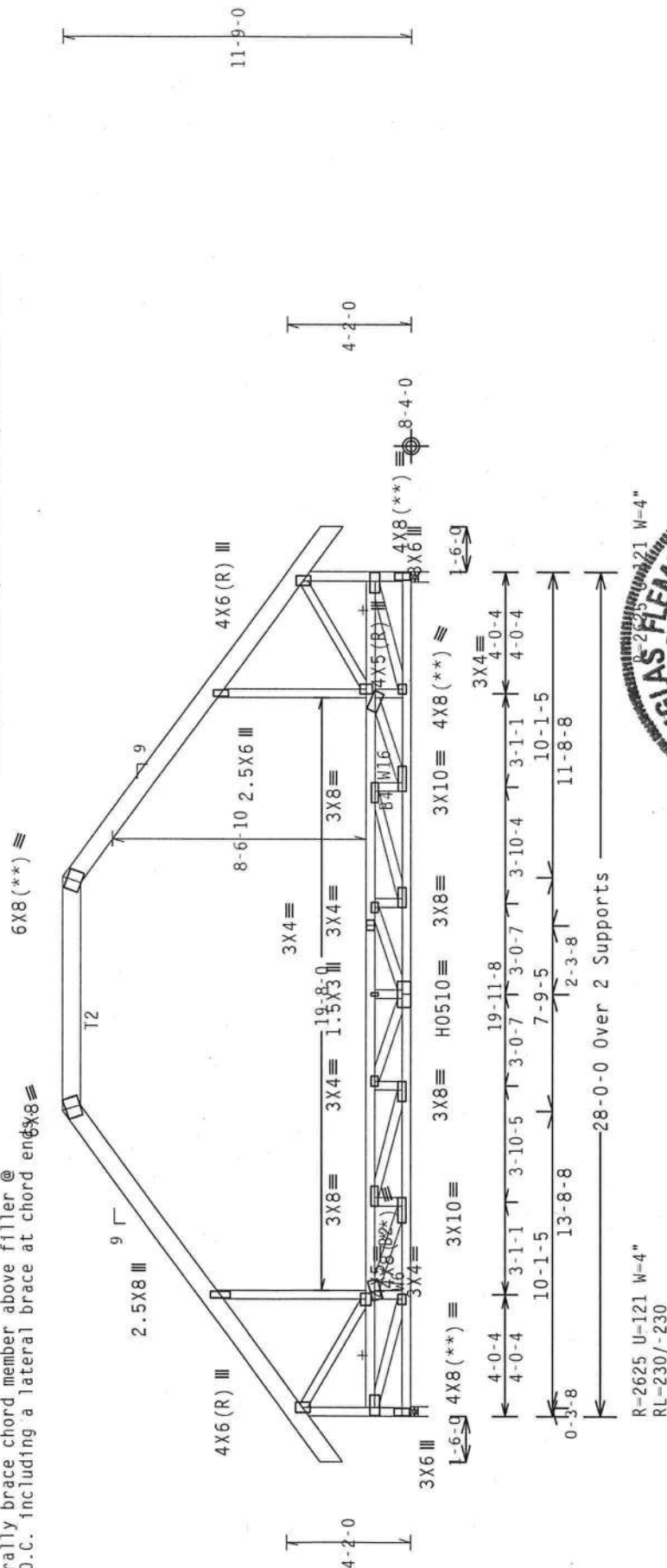
1110 mph wind, 15.73 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpi(+/-)=0.18

wind reactions based on MNFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.



R=2625 U=121 W=4"
RL=230/-230

PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2007Res/TPI-2002 (S
FT/RT=10%(0%)/0(0)

TY:1 FL/-/4/-/-/R/- Scale = .1875"/Ft.

TC LL	20.0	PSF	REF R487-- 57015
TC DL	10.0	PSF	DATE 02/24/11
BC DL	10.0	PSF	DRW HCUR487 11055008
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 184539
DUR.FAC.	1.25		
SPACING	24.0"		JREF- 1U90487_Z01

ALPINE

Haines City, FL 33844
 FL COA #0 278

02/24/2011

3 COMPLETE TRUSSES REQUIRED

Special loads

----- (Lumber	Dur. Fac. -1.25	Plate	Dur. Fac. -1.25)
TC-From	65 plf at 0.00	to	65 plf at 4.17
TC-From	247 plf at 4.17	to	247 plf at 10.11
TC-From	247 plf at 10.11	to	247 plf at 17.89
TC-From	247 plf at 17.89	to	247 plf at 18.69
TC-From	387 plf at 18.69	to	440 plf at 23.83
TC-From	415 plf at 23.83	to	458 plf at 28.00
TC-From	65 plf at 28.00	to	65 plf at 29.50
BC-From	20 plf at 0.00	to	20 plf at 4.17
BC-From	120 plf at 4.17	to	120 plf at 23.83
BC-From	20 plf at 23.83	to	20 plf at 28.00
BC-From	5 plf at 28.00	to	5 plf at 29.50
BC- 695.05 lb Conc.	Load at	4.23	
BC- 623.00 lb Conc.	Load at	6.23, 8.23, 10.23, 12.23	
4.23, 16.23, 18.23, 20.23, 22.23, 24.23, 26.23.			
BC- 100.73 lb Conc.	Load at	23.83	

Roof overhang supports 2.00 psf soffit load.

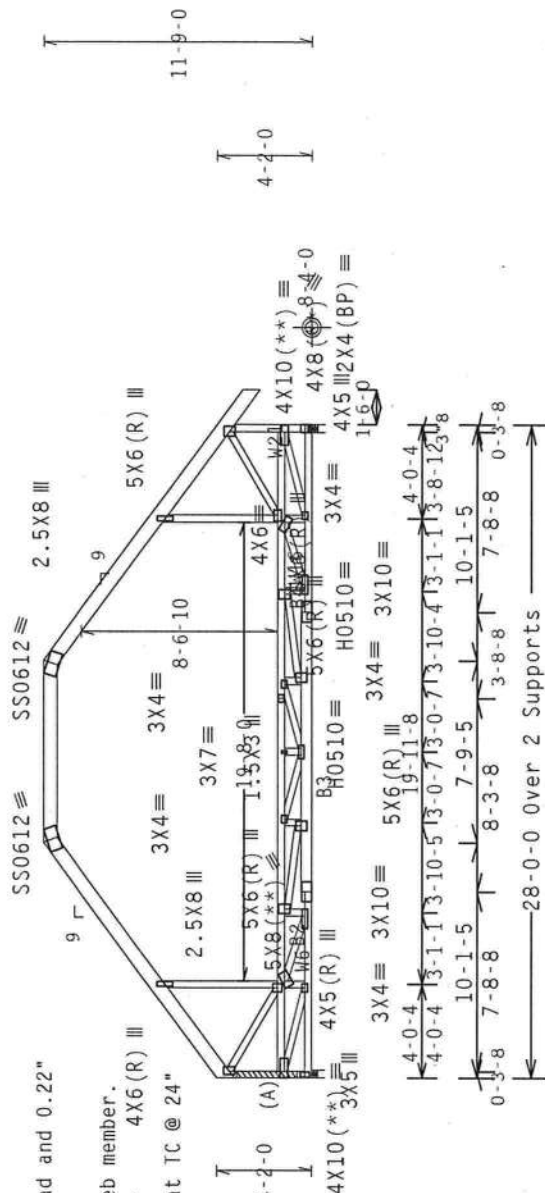
Calculated horizontal deflection is 0.24" due to live load and 0.22" due to dead load.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3".min.) nails @ 6" OC. 4Y6

In lieu of structural panels use purlins to brace all flat TC @ 24"

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.



R=7762 U=137 W=4"
RL=212/-197

R-10326 U-150 W-4"

ULT TYP. 20 Gauge HS.18 Gauge HS, Design Crit: FBC2007Res/TPI-2002 (0.0001)
Wave FT/RT=10%(0%)/0(0)

Scale = .125"/Ft.

QTY:1 FL/-/4/-/-/R/-/

REF	R487--	57016
DATE	02/24/11	
DRW	HCUSR487	11055009
HC-ENG	DF/DF	
SEQN-	184550	
JREF-	1U9Q487_Z01	

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

02/24/2011

AND SEC. NO. 66648
INSTITUTION, 1300
PERIPHERICA, 1300
UNLESS
100% SHALL HAVE
STATE OF FLORIDA
PROFESSIONAL
NOT
FLORIDA
TRUSS COMPANY
CONSISTENCY OF

[illegible]

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0778

1110 mph wind, 15.73 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf (+/-)=0.18

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Roof overhang supports 2.00 psf soffit load.

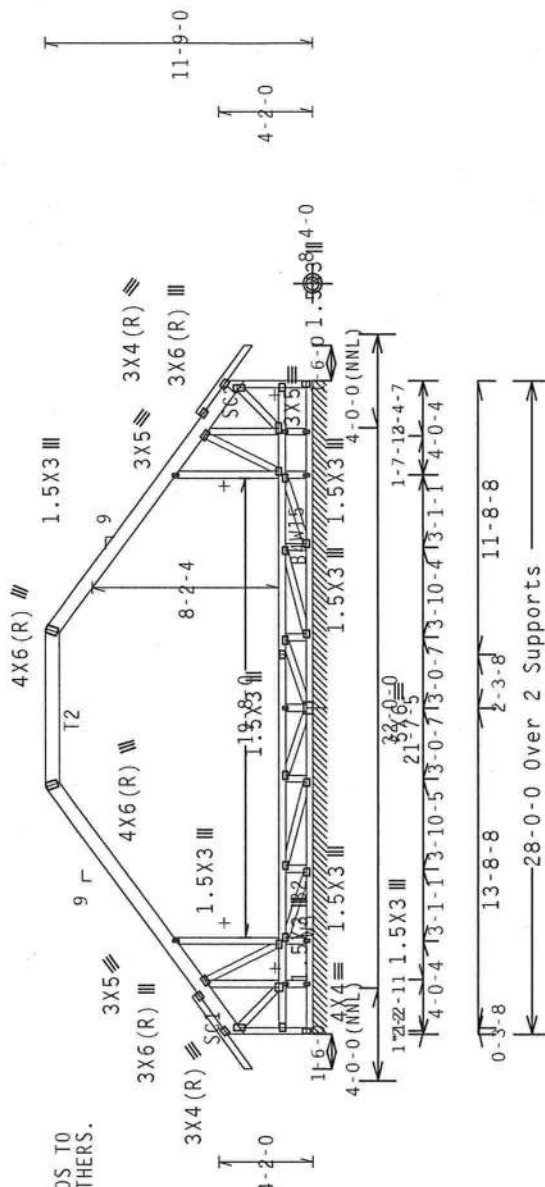
Stacked top chord must NOT be notched or cut in area (NUL). Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

8C attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 4-2-0 to 23-10-0.

[illegible]

$1^{\circ}22'-11''$ 1.5X III $21^{\circ}19'50''$ $1^{\circ}7'-12'-47''$
 $\leftarrow 4-0-4 \leftarrow 13-1-1 \leftarrow 13-10-5 \leftarrow 13-0-7 \leftarrow 13-10-4 \leftarrow 13-1-1 \leftarrow 4-0-4 \rightarrow$
 $\leftarrow 0-9-8 \leftarrow 13-8-8 \leftarrow 2-3-8 \leftarrow 11-8-8 \rightarrow$
 $\leftarrow 28-0-0 \text{ Over 2 Supports} \rightarrow$



PLF 14-0-0
PLF 13-13
PLF 14-0-0

Design Crit: FBC2007Res/TPI-2002(S)

Scale = .125"/Ft.

[illegible]

REF R487-- 57018

DATE 02/24/11

DATE	02/24/11
BILL NUMBER	100703

HW HCUSK487 11055011

IC-ENG DF/DF

184510 EQN-

REF- 1U9Q487_Z01

Top chord 2x8 SP SS
Bot chord 2x4 SP #1 :B2, B4 2x4 SP #2 Dense:
Webs 2x4 SP #3 :W6, W16 2x4 SP #2 Dense:

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 98 plf at -1.50 to 98 plf at 0.00
TC- From 98 plf at 0.00 to 98 plf at 4.17
TC- From 135 plf at 4.17 to 135 plf at 10.11
TC- From 135 plf at 10.11 to 135 plf at 17.89
TC- From 135 plf at 17.89 to 135 plf at 23.83
TC- From 98 plf at 23.83 to 98 plf at 28.00
TC- From 98 plf at 28.00 to 98 plf at 29.50
TC- From 7 plf at -1.50 to 7 plf at 0.00
BC- From 30 plf at 0.00 to 30 plf at 4.17
BC- From 180 plf at 4.17 to 180 plf at 23.83
BC- From 30 plf at 23.83 to 30 plf at 28.00
BC- From 8 plf at 28.00 to 8 plf at 29.50
BC- 151.09 lb Conc. Load at 4.17
BC- 203.96 lb Conc. Load at 13.90, 15.90, 17.90, 19.90
21.90
BC- 143.30 lb Conc. Load at 23.77
BC- 294.39 lb Conc. Load at 23.83

Calculated horizontal deflection is 0.19" due to live load and 0.17" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 36.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/240 live and L/180 total load.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

+ LATERALLY BRACE ALL BC AT 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3" nails
Top Chord: 1 Row @12.00" O.C.
Bot Chord: 1 Row @10.75" O.C.
Webs : 1 Row @ 4" O.C.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

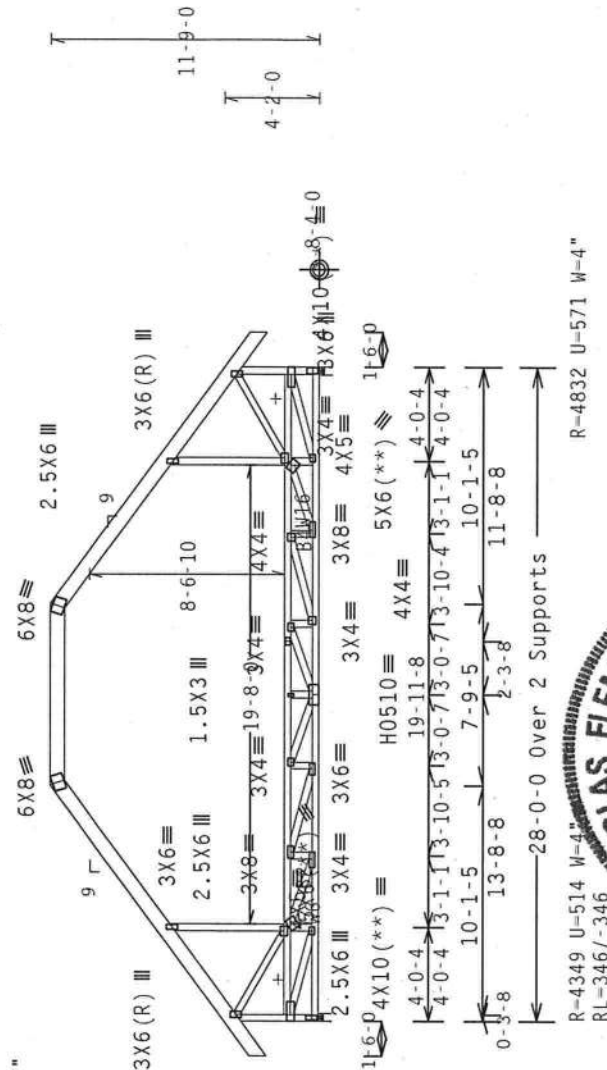
(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.73 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 GCpl(+/-)-0.18

Wind reactions based on MMFRS pressures.

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.



PLT TYP. 20 Gauge HS.Wave

Design Crit: FBC2007Res/TPI-2002 (S) FT/RT=10% (0%) / O (0)

QTY:1 FL/-/4/-/R/- Scale =.125"/Ft.

ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	02/24/2011		02/24/2011	
	BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.		BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.	
	DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELLY FOR THE TRUSS COMPANY. THE TRUSS COMPANY HAS THE RESPONSIBILITY OF THE TRUSS COMPANY. THE TRUSS COMPANY HAS THE RESPONSIBILITY OF THE TRUSS COMPANY.		DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELLY FOR THE TRUSS COMPANY. THE TRUSS COMPANY HAS THE RESPONSIBILITY OF THE TRUSS COMPANY. THE TRUSS COMPANY HAS THE RESPONSIBILITY OF THE TRUSS COMPANY.	
	ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.		ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3.	
	CONNECTOR PLATES ARE MADE OF 2018/16GA (N-H/SS/7) ASTM A653 GRADE 40/60 (N, K/H/SS) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING.		CONNECTOR PLATES ARE MADE OF 2018/16GA (N-H/SS/7) ASTM A653 GRADE 40/60 (N, K/H/SS) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWING.	
	DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD) AND TPI. TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. FOR WOOD) AND TPI. TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.	
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		
FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.		
A PROPERTY ATTACHED RIGID CEILING.		A PROPERTY ATTACHED RIGID CEILING.		
UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		
REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6500 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6500 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6500 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 6500 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHOULD HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORDS SHALL HAVE A PROPERTY ATTACHED RIGID CEILING.		
No. 66648		No. 66648		
STATE OF FLORIDA PROFESSIONAL ENGINEER		STATE OF FLORIDA PROFESSIONAL ENGINEER		
02/24/2011		02/24/2011		
SPACING		SPACING		
36.0"		36.0"		
DUR.FAC.		DUR.FAC.		
1.25		1.25		
TOT.LD.		TOT.LD.		
40.0 PSF		40.0 PSF		
BC LL		BC LL		
0.0 PSF		0.0 PSF		
BC DL		BC DL		
10.0 PSF		10.0 PSF		
TC DL		TC DL		
10.0 PSF		10.0 PSF		
TC LL		TC LL		
20.0 PSF		20.0 PSF		
REF		REF		
R487--		R487--		
57019		57019		
DATE		DATE		
02/24/11		02/24/11		
DRW		DRW		
HCUSR487		HCUSR487		
11055012		11055012		
HC-ENG		HC-ENG		
DF/DF		DF/DF		
SEQN-		SEQN-		
184536		184536		
JREF-		JREF-		
1U9Q487_Z01		1U9Q487_Z01		

02/24/2011

(11-036--Fill in later AARON NICKELSON/SCHNEIDER -- , ** - C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

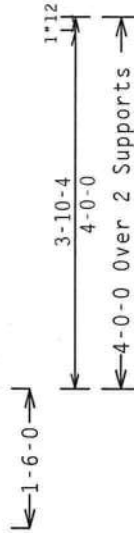
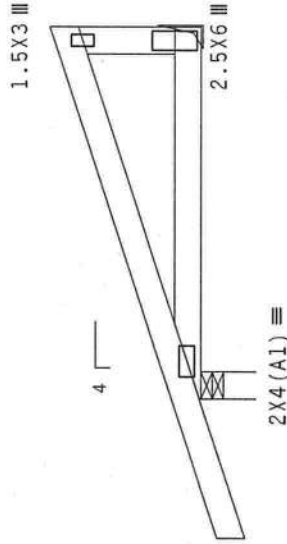
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, OPEN CLEAR bldg, Located
anywhere in roof, CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0
psf. lw=1.00 GCpi(+/-)-0.00

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



R-289 U-64 W-3.5"
RL-62/-24

R-133 U-26

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (S-0)
FT/RT=10%(0%/0/0)

Scale = .5"/Ft.

TY:15 FL/-/4/-/R/-

03.03.16

03.03.16

03.03.16

03.03.16

03.03.16

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487--	57021
DATE	02/24/11	
DRW	HCUSR487	11055002
HC-ENG	DF/DF	*
SEQN	184569	

JREF	1U9Q487_Z01
------	-------------

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

02/24/2011

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpf (+/-)=0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 0-6-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

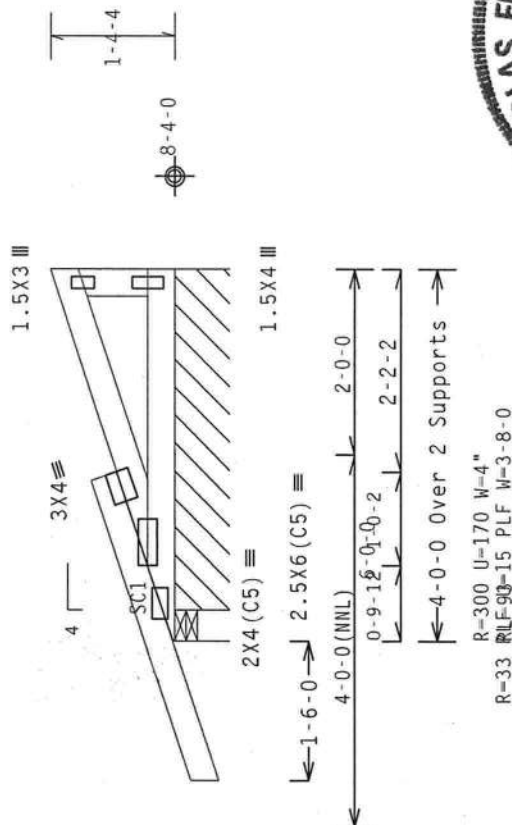
Deflection meets L/240 live and L/180 total load.

	Top chord	2x4	SP #1
Bot chord	2x4	SP #2	Dense
Webbs	2x4	SP #3	
Stack	Chord	SC1	2x4 SP #2

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in notchable area using 4x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



R-300 U=170 W=4"
R-33 PLF=15 PLF W=3-8-0

PLOT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (0.00%)
FT/RT=10%(0%) / 0 (0%)

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487-- 57022
TC DL	10.0 PSF	DATE 02/24/11
BC DL	10.0 PSF	DRW HCUSR487 11055013
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 184566
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U90487_Z01

02/24/2011



ALPINE
Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0278

(11-036--Fill in later AARON NICKELSON/SCHNEIDER -- , ** - C1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg. Located
anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0
psf. 1w=1.00 GCpf (+/-)-0.18

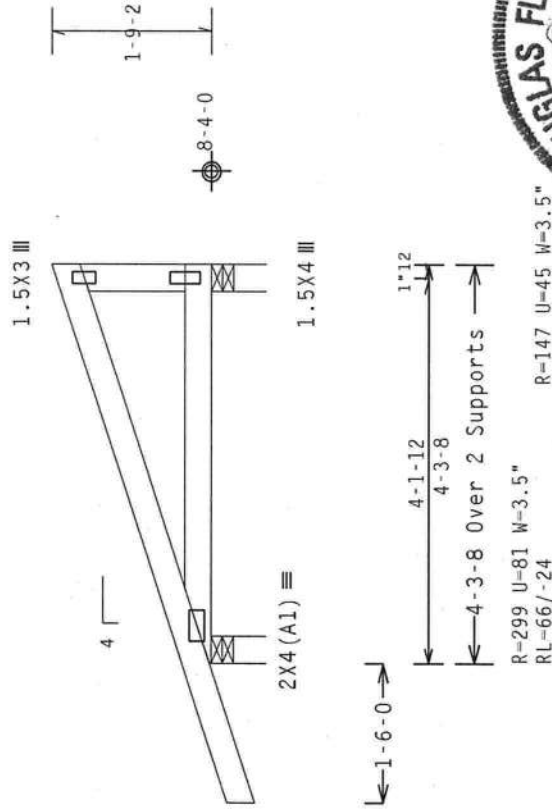
Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

All wind load cases on this truss have a 1.33 duration factor.



PLT TYP. Wave	Design Crit: FBC2007Res/TPI-2002 (S.D) FT/RT=10% (0%) / 0 (0)	Scale = .5" / Ft.
ALPINE Haines City, FL 33844 FL COA #0278	DOUGLAS FLEMING LICENSE No. 06848 03/19/16 03/19/16 03/19/16	REF R487-- 57023 DATE 02/24/11 DRW HCUSR487 11055003 HC-ENG DF/DF SEQN- 184572
	TC LL 20.0 PSF TC DL 10.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.15 SPACING 24.0"	JREF- 1U90487_Z01

(11-036--Fill) in later AARON NICKELSON/SCHNEIDER --, ** - DGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11030050109 & GBLETTIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

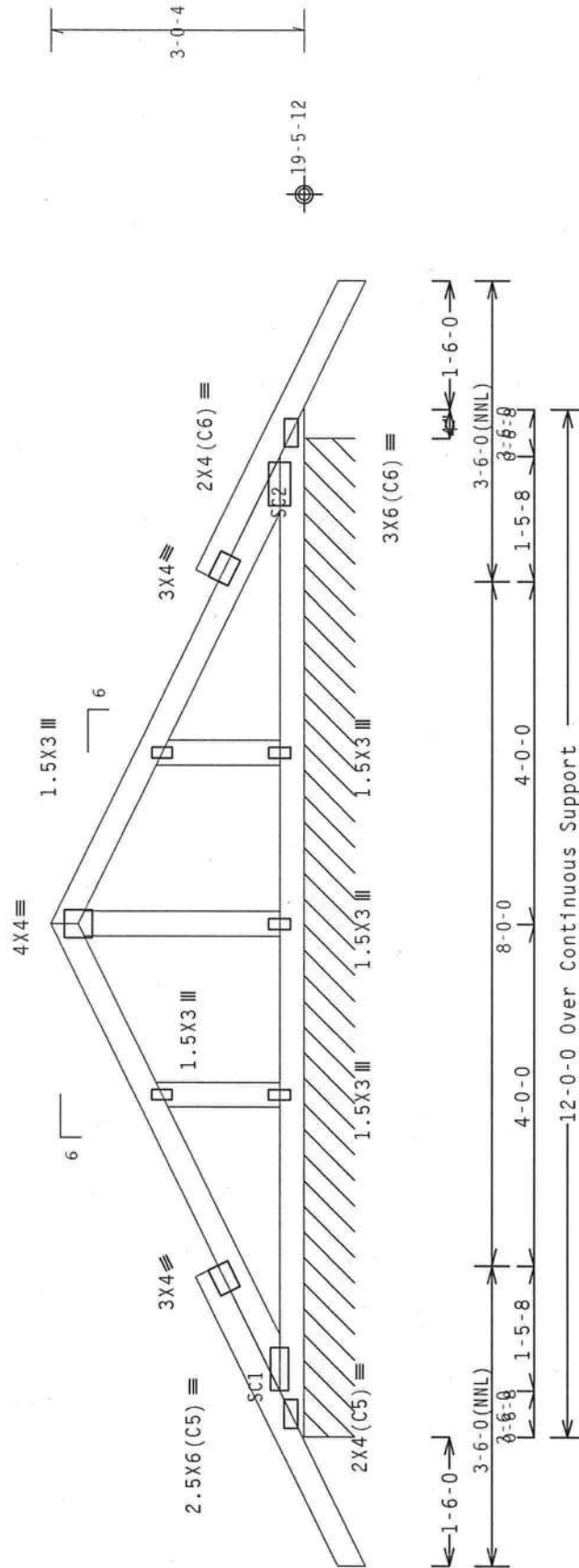
110 mph wind, 20.79 ft mean hgt, ASCE 7-05, CLOSED bldg. Located anywhere in roof. CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Iw=1.00 GCpi(+/-)-0.18

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 0-6-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



R-102 PLF U=19 PLF W=11-8-0
RL-8/-8 PLF

Design Crit: FBC2007Res/TPI-2002 (S)
FT/RT=10% (0%) / 0 (0%)

PLT TYP. Wave

QTY: 1

Scale = .5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 57024
DATE	02/24/11
DRW	HCUSR487 11055014
HC-ENG	DF/DF
SEQN-	184559
JREF-	1U90487_Z01

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

02/24/2011

Professional Engineer
Douglas Fleming
License No. 66648
State of Florida

IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY MODIFICATION TO THIS DESIGN. ANY MODIFICATION TO THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE DESIGN CRITERIA. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE DESIGN CRITERIA. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS IN CONFORMANCE WITH THE DESIGN CRITERIA.

(11-036--F111) in later AARON NICKELSON/SCHNEIDER --, ** - D)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

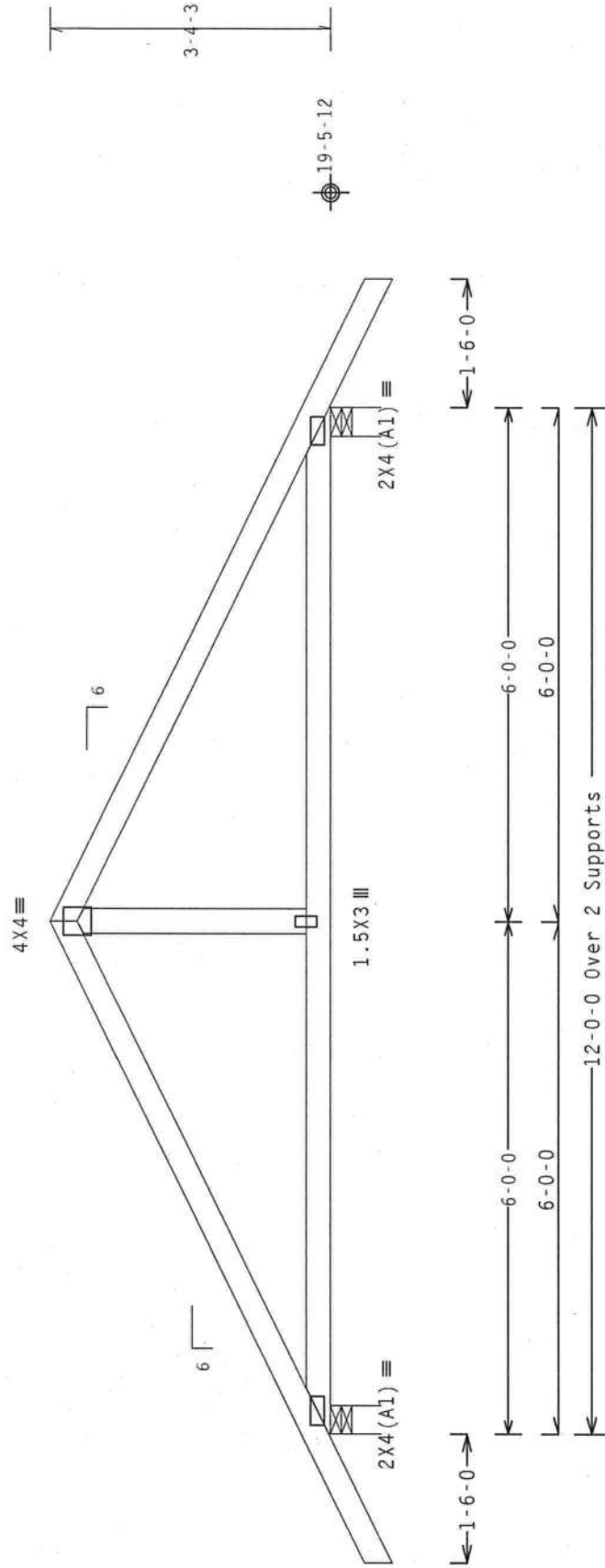
110 mph wind, 20.95 ft mean hgt, ASCE 7-05, CLOSED bldg. Located
anywhere in roof. CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-5.0
psf. Iw-1.00 GCpi (+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MWFRS pressures.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



R-594 U-115 W-4"
RL-96/-96

R-594 U-115 W-4"

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (S)
FT/RT=10%(0%)/0(0)

QTY: 4 FL/-/4/-/R/-

Scale =.5"/Ft.

TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

DOUGLAS FLEMING

LICENSE

Ab. 66648

STATE OF FLORIDA

PROFESSIONAL ENGINEER

02/24/2011

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATING INSTITUTE), 2319 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (WOOD TRUSS COUNCIL OF ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THE TRUSS IS CORRECTLY INSTALLED AND BRACED. THE DESIGNER SHALL NOT BE RESPONSIBLE FOR THE TRUSS BEING INSTALLED OR BRACED IN CONFORMANCE WITH THE DESIGN. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFPA) AND TPI (TRUSS PLATING INSTITUTE) DESIGN. CONNECTOR PLATES ARE MADE OF 2018/16GA (IN./H/SS/A) ASTM A653 GRADE 40/60 (IN. K/H/SS) GALV. STEEL. ALL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DIAGRAM. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER AS OF TPI-2002 SEC.3. A SECONDARY INSPECTION SHALL BE REQUIRED TO VERIFY THE TRUSS MEETS UP THE TRUSS MANUFACTURER'S DESIGN SHOWING THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

(11-036--Fill in later AARON NICKELSON/SCHNEIDER -- . ** - F)

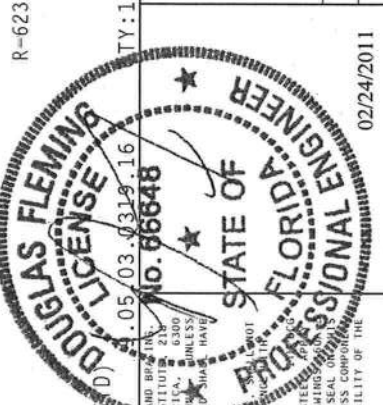
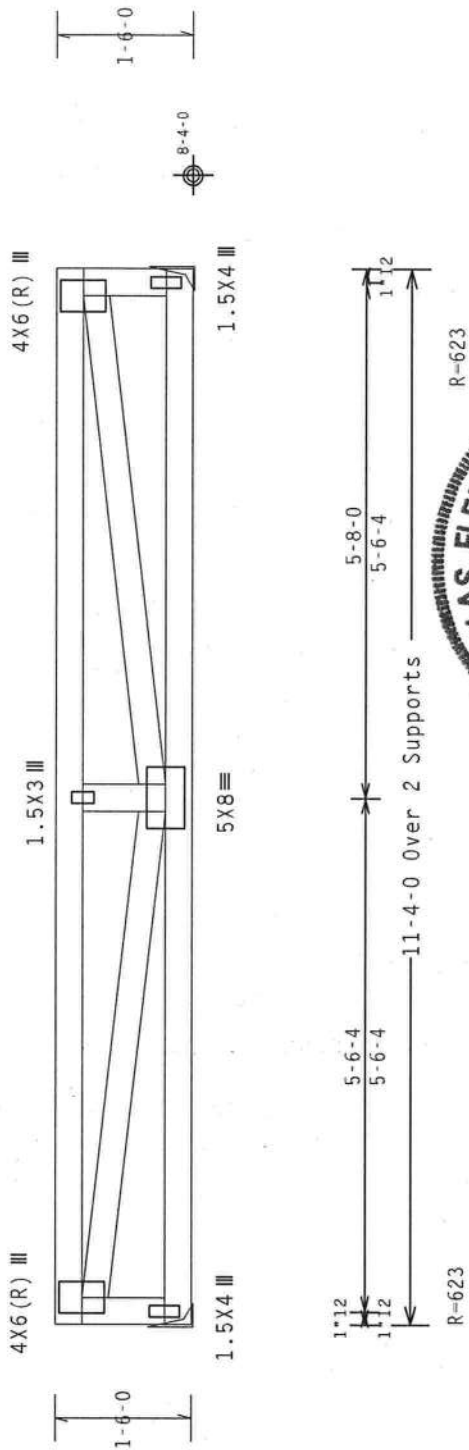
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #5 Dense
Webs 2x4 SP #3

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/360 live and L/360 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

- (H1) = (J) Hanger not calculated (3)2x4 SP #2 Dense supporting member.
- (H2) = (J) Hanger not calculated (3)2x6 SP #1 Dense supporting member.
- (H3) = (J) Hanger not calculated (3)2x4 SP #1 supporting member.
- (H4) = (J) Hanger not calculated (3)2x4 SP #2 Dense supporting member.
- (H5) = (J) Hanger not calculated (3)2x4 SP #1 supporting member.



PLT TYP. Wave	Design Crit: FBC2007Res/TPI-2007 (S.D) FT/RT=10% (0%) / 0 (0)	TY:11 FL/-/4/-/-/R/-	Scale = .5" / Ft.
REF R487-- 57026	TC LL 40.0 PSF	TC DL 10.0 PSF	BC DL 5.0 PSF
DATE 02/24/11	BC LL 0.0 PSF	TOT.LD. 55.0 PSF	DUR.FAC. 1.00
DRW HCUSR487 11055015	SPACING 24.0"	HC-ENG DF/DF	SEQN- 184530
JREF- 1U9Q487_Z01			

(11-036--Fill in later AARON NICKELSON/SCHNEIDER --, ** - FGE)

Top chord 2x4 SP #1
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss spaced at 24.0" OC designed to support 0-6-0 top chord
outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must
not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

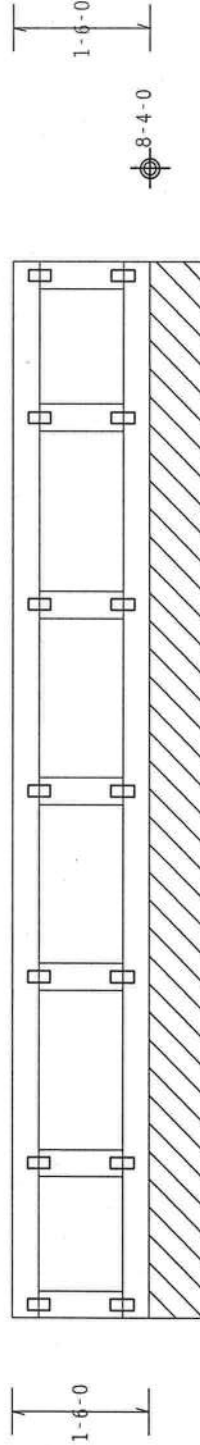
Deflection meets L/360 live and L/360 total load.

Provide for complete drainage of roof.

Truss must be installed as shown with top chord up.

** Wall girder loading on this truss. **

Fasten rated sheathing to one face of this frame.



11-4-0 Over Continuous Support

R-551 PLF W-11-4-0

Note: All Plates Are 1.5X3 Except As Shown.

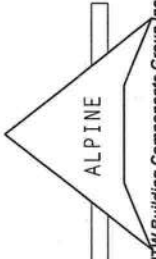
Design Crit: FBC2007Res/TPI-2002 (S.D.)

FT/RT-10%(0%/0/0) 0.05/03.0319.16

TY:1 FL/-/4/-/-/R/-

Scale = .5" / Ft.

PLT TYP. Wave

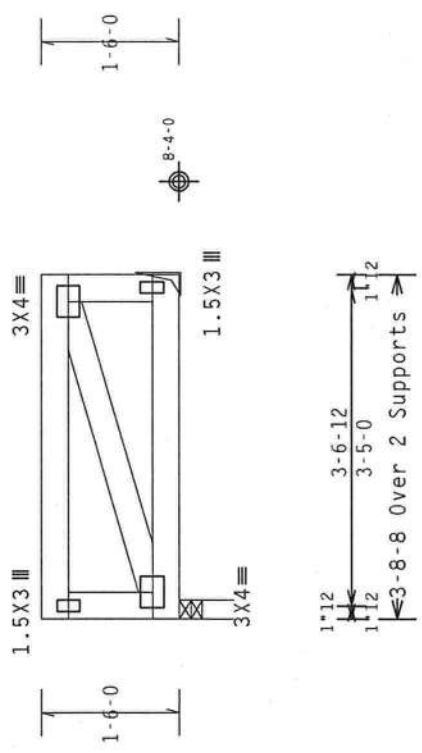
 ALPINE Haines City, FL 33844 FL COA #0278	TC LL 40.0 PSF		REF R487-- 57028
	TC DL 10.0 PSF		DATE 02/24/11
	BC DL 5.0 PSF		DRW HCUSR487 11055017
	BC LL 0.0 PSF		HC-ENG DF/DF
	TOT.LD. 55.0 PSF		SEQN- 184575
	DUR.FAC. 1.00		
	SPACING 24.0"		JREF- 1U9Q487_Z01

(11-036--F1) In later AARON NICKELSON/SCHNEIDER -- , ** - F1)

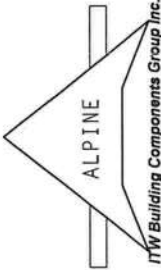
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Bottom chord checked for 10.00 psf non-concurrent live load.
Truss must be installed as shown with top chord up.

(H1) = (J) Hanger not calculated (2)2x4 SP #1 supporting member.
(H2) = (J) Hanger not calculated (2)2x4 SP #2 Dense supporting member.
Deflection meets L/360 live and L/360 total load.



PLT TYP. Wave



ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Design Crit: FBC2007Res/TPI-2002 (S0)
FT/RT=10%(0%) / 0(0)

Scale = .5"/Ft.

TY:5 FL/-/4/-/-/R/-

TC LL	40.0 PSF	REF	R487--	57029
TC DL	10.0 PSF	DATE	02/24/11	
BC DL	5.0 PSF	DRW	HCUSR487	11055018
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	55.0 PSF	SEON-	184516	
DUR.FAC.	1.00			
SPACING	24.0"	JREF-	1U9Q487_Z01	

02/24/2011



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

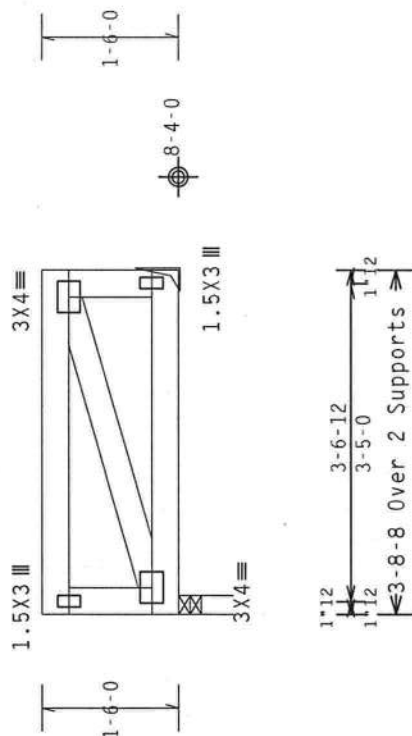
Special loads

Special loads	(Lumber	Dur.Fac.=1.00 /	Plate Dur.Fac.=1.00)
TC-	From 100 plf at 0.00 to 100 plf at 3.71		
BC-	From 10 plf at 0.00 to 10 plf at 3.71		
BC-	158.18 lb Conc. Load at 1.94		

Deflection meets L/360 live and L/360 total load.

Truss must be installed as shown with top chord up.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.



R=279 U=0 W=2 5"

R=287 U=0

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (S)

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

REF R487-- 57030

REL	R407	3/030
DATE	03/24/11	

DATE 02/24/11

DRW HCUSR487 11055019

HC-FNG DF/DF

104500

SEQN- 184523

11990487 701

02/24/2011

ALPINE

TTW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

(11-036--Fill in later AARON NICKELSON/SCHNEIDER --, ** - M)

Top	chord	2x4	SP	#2	Dense
30t	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

1110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg. Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.18

End verticals not exposed to wind pressure.

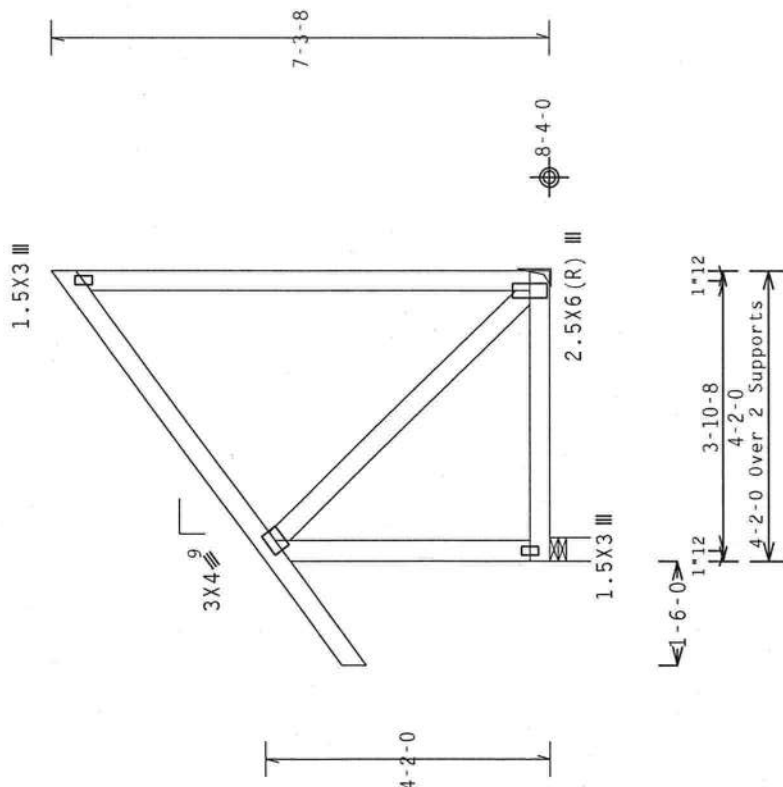
Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

MFERS loads based on trusses located at least 7.50 ft. from roof edge.



R-301 U=0 W=4"
RL-97/-58
R-158 U=124

Design Crit: FBC2007Res/TPI-2002 304
 FT/RT=10%(0%)/0(0)
 9-05-03-0319-161
 OTY:3
 FL/-4/-/-R/-
 Scale =.375"/Ft.

PLT TYP. Wave

TC LL	20.0 PSF	REF R487-- 57031
TC DL	10.0 PSF	DATE 02/24/11
BC DL	10.0 PSF	DRW HCUSR487 11055005
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 184520
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U9Q487 Z01

02/24/2011



ALPINE
Building Components Group Inc.
 Haines City, FL 33844
 FL COA #0 278

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webbs	2x4	SP	#3	

110 mph wind, 21.54 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ Gcpi (+/-)=-0.18

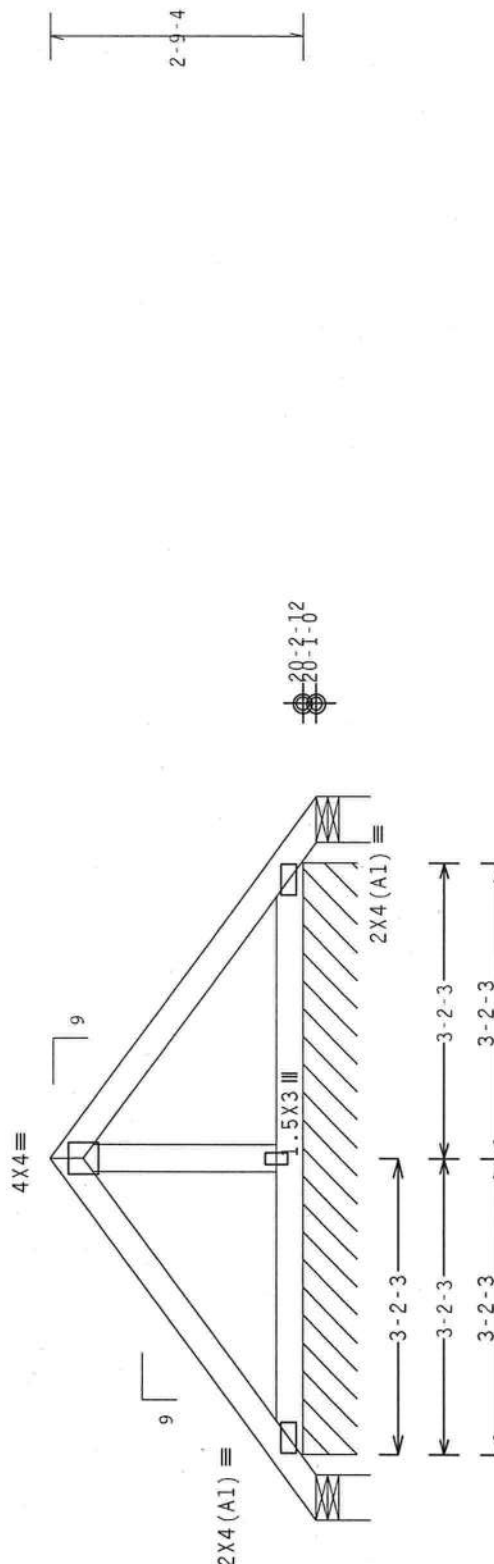
Wind reactions based on MWFRS pressures.

Refer to DWG PB1200109 for piggyback details.

Special loads

	----- (Lumber	Dur. Fac. = 1.25 /	Plate	Dur. Fac. = 1.25)
TC- From		65 plf at	0.00 to	65 plf at 3.89
TC- From		65 plf at	3.89 to	65 plf at 7.78
BC- From		4 plf at	0.00 to	4 plf at 7.78

Deflection meets L/240 live and L/180 total load.



← 7-9-5 Over 3 Supports

R--26 RW-61 U-63 W-5.833"

RL=68 R-~~687~~ PLF U=29 PLF W=6-4-5

R--26 Ruc--21-V-833"

POLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (S...
ET/RT=10%(0%)/0(0)

TY:7 FL/-/4/-/-/R/- Scale = 5"/Ft.

Scale = .5"/Ft.



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO UCSI SHIPBUILDING COMPONENT INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTION), 9300 MONTEFEE STREET, SUITE 312, ALEXANDRIA, VA, 22304, AND THE 4000 TRUSSES ECONOMICS OF CONSTRUCTION, 1987 EDITION, FOR PROPER ATTACHMENT OF PERMANENTLY ATTACHED PANELS TO TRUSSES. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CLIPS.

[illegible]

02/24/2011

REF- 1U90487 Z01

SPACING 24.0"

02/24/2011

Top	chord	2x4	SP	Dense
Bot	chord	2x4	SP	Dense
	webs	2x4	SP	Dense

1110 mph wind, 21.36 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, Iw=1.00 Gcpi (+/-)=0.18

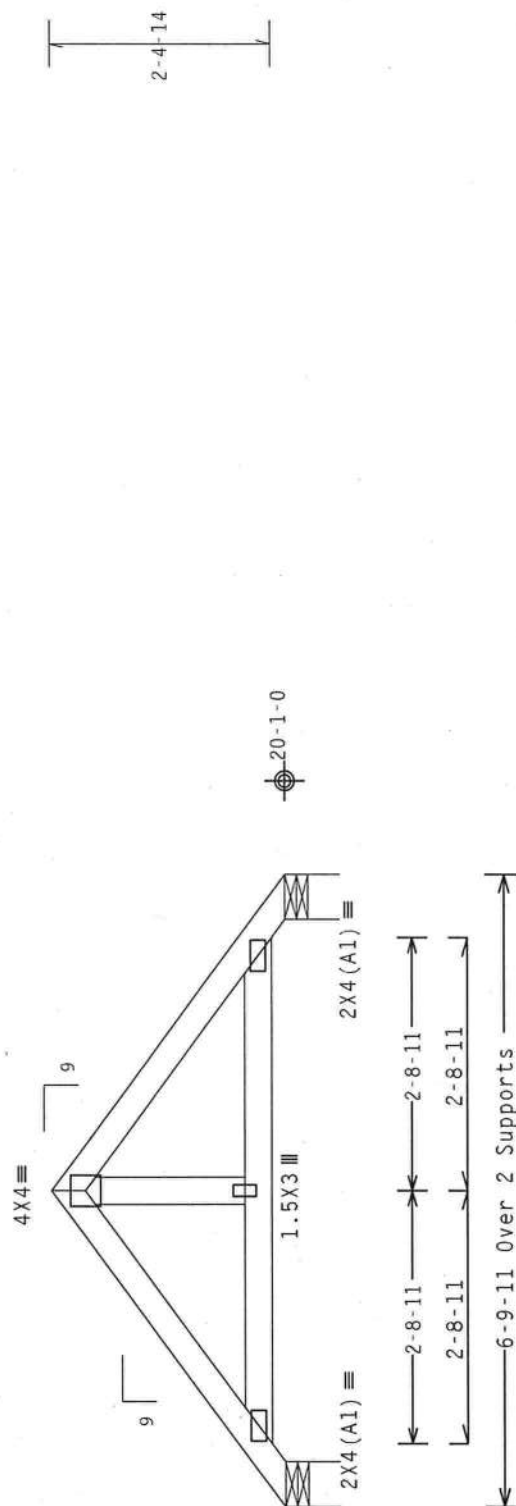
Wind reactions based on MWFRS pressures.

Refer to DWG PB1200109 for piggyback details.

Special loads

TC- From	Dur. Fac.=1.25 / Plate Dur. Fac.=1.25
TC- From	65 plf at -0.68 to 65 plf at 2.72
BC- From	65 plf at 2.72 to 65 plf at 6.13
BC- From	4 plf at -0.68 to 4 plf at 6.13

Deflection meets L/240 live and L/180 total load.



R-218 U=63 W=5.833"
RL-59/-59

R-218 11-63 W-5 8230

PLT TYP. Wave

Design Crit: FBC2007Res/TPI-2002 (FT/RT=10%(0%)/0(0))

9.05/03.0319.16 OTY:2

FL/-/4/-/-/R/-/-

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487-- 57034
TC DL	10.0 PSF	DATE 02/24/11
BC DL	10.0 PSF	DRW HCUR487 11055021
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 184507
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1U90487_Z01

*WARNING** THUSSES REQUIRE EXTREME CARE IN FABRICATION, INSTALLATION, AND BUILDING. REFER TO BCSP (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (4000 TRUSS COMPANY OF AMERICA), 100 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

*IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPLIANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGNER SHALL BE RESPONSIBLE FOR ANY VIOLATION OF ANY APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY ASEA) AND TPI (TRUSS PLATE INSTITUTE) DESIGN SPECIFICATIONS. CONNECTOR PLATES ARE MADE OF 2018/16GA. IN H/25FR) ASTM A653 GRADE 40/60 IN. K/H-55) GALV. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SPECIFIED ON THIS DESIGN, POSITION PER DRAWINGS. A SEAM IN THE PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI1-2002 SEC.3. ANY INSPECTION OF PLATES INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS CONSTRUCTION. THE DRAWING INDICATES THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

STATE OF FLORIDA
 PROFESSIONAL ENGINEER
 No. 66648
 02/24/2011



Alpine Building Components Group Inc.
Haines City, FL 33844
Tel. 800.444.278

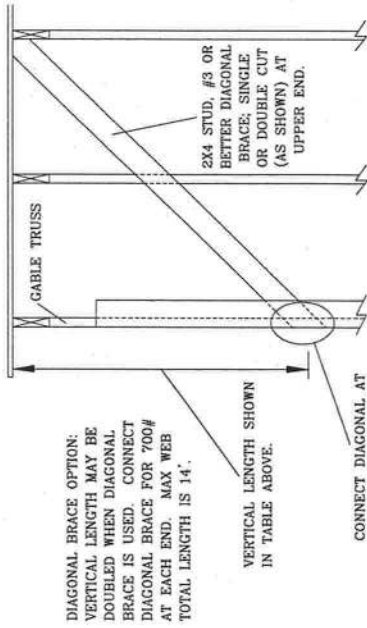
ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	GABLE VERTICAL SPECIES/ GRADE	BRACE		(1) 1X4 "L" BRACE		(2) 2X4 "L" BRACE		(1) 2X6 "L" BRACE		(2) 2X6 "L" BRACE	
		NO BRACES	BRACE	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	3' 8"	#1 / #2	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	8' 11"	11' 9"	12' 1"
	HF	3' 7"	#3	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"
	STANDARD	3' 7"	STUD	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"
	SP	3' 7"	STANDARD	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"
	DFL	4' 0"	#1	6' 4"	6' 4"	7' 6"	7' 6"	8' 11"	8' 11"	12' 8"	14' 0"
	DFL	3' 11"	#2	6' 4"	6' 10"	7' 6"	7' 6"	8' 11"	8' 11"	12' 8"	14' 0"
16" O.C.	SPF	3' 9"	#3	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	8' 11"	11' 5"	11' 5"
	HF	3' 9"	STUD	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 11"	11' 4"	11' 4"
	STANDARD	3' 8"	STANDARD	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"
	SP	4' 1"	#1 / #2	4' 2"	7' 3"	7' 5"	7' 5"	8' 10"	8' 10"	10' 3"	13' 5"
	DFL	4' 1"	#3	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"
	DFL	4' 1"	STUD	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"
24" O.C.	SPF	4' 7"	#1	7' 3"	7' 9"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	14' 0"
	HF	4' 6"	#2	7' 3"	7' 9"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	14' 0"
	STANDARD	4' 4"	#3	6' 10"	6' 10"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	14' 0"
	SP	4' 4"	STUD	6' 9"	6' 9"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	14' 0"
	DFL	4' 2"	STANDARD	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 3"	13' 5"	14' 0"
	DFL	4' 7"	#1 / #2	8' 0"	8' 2"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"
36" O.C.	SPF	4' 6"	#3	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"
	HF	4' 6"	STUD	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"
	STANDARD	4' 6"	#1	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	14' 0"	14' 0"
	SP	5' 1"	#2	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"
	DFL	4' 9"	#3	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"
	DFL	4' 9"	STANDARD	7' 9"	7' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	14' 0"

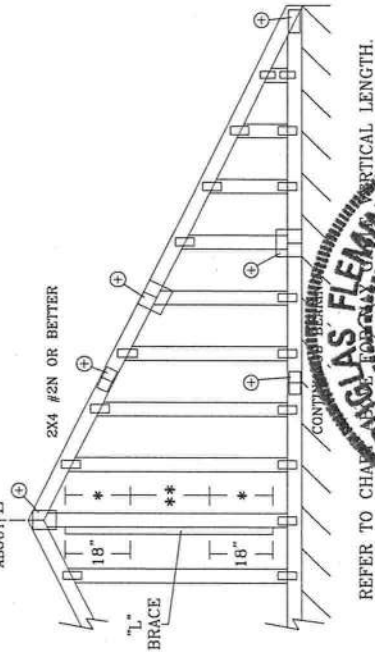
SYMM (C) ABOUT



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE FOR 700#
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
IN TABLE ABOVE.

CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL
WEB.



REFER TO CHANGING VERTICAL LENGTH.

BRACING GROUP SPECIES AND GRADES:

GROUP A:	
SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	STUD
STANDARD	STANDARD

DOUGLAS FIR-LARCH	
#3	STUD
STANDARD	STANDARD

GROUP B:	
HEM-FIR	DOUGLAS FIR-LARCH
#1 & BTR	#1
	#2

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2

GABLE TRUSS DETAIL NOTES:

- LIVE LOAD DEFLECTION CRITERIA IS L/240.
- PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).
- GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128 x3" min)
- * FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design of these functions. ITWBCG connector plates are made of 2018/16CA (W.H/S/K) ASTM A505 grade 47/407/60 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on every diagonal. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility. See the ITW Building Components Group Inc. (ITWBCG) website for the full disclaimer. ITWBCG is a registered trademark of ITW. See 2. ITW-BCCG: www.itwbcg.com; TPI: www.tpinet.com; WTC: www.wtcindustry.com; ICC: www.iccsafe.org



Earth City, MO 63045

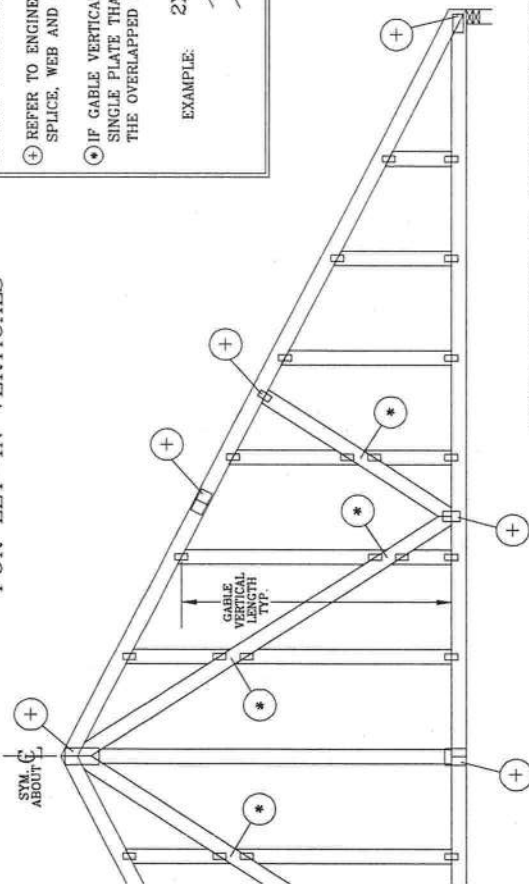
REF ASCE7-05-CAB1030
DATE 1/1/09
DRWG A11030050109

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

02/24/2011

GABLE DETAIL FOR LET-IN VERTICALS

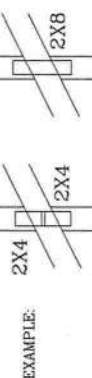


GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

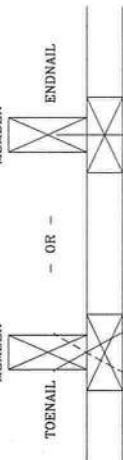
⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

⊙ IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.



EXAMPLE:

"T" REINFORCEMENT ATTACHMENT DETAIL



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	20 %
100 MPH	2x4	10 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	20 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "T" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

END DRIVEN NAILS:

10d COMMON (0.148"x3" MIN) NAILS AT 4" O.C. PLUS

(4) NAILS IN TOP AND BOTTOM CHORD.

TOENAIL NAILS:

10d COMMON (0.148"x3" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE

WIND LOAD.

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015980109, A12015980109, A11015980109, A10015980109,

A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 GABLE DETAIL DRAWINGS

A13015020109, A12015020109, A11015020109, A10015020109,

A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 GABLE DETAIL DRAWINGS

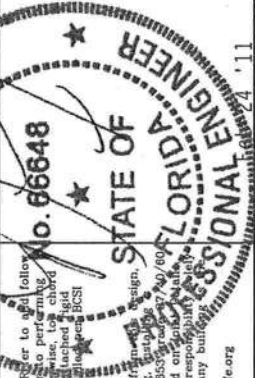
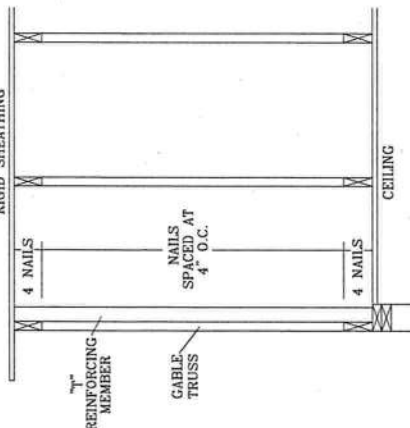
A13015050109, A12015050109, A11015050109, A10015050109,

A13030050109, A12030050109, A11030050109, A10030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM

UNREINFORCED GABLE VERTICAL LENGTH.

RIGID SHEATHING



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

Truss components shall be fabricated, shipped, installed and braced in accordance with the design. Refer to applicable building codes for details. BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design. any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, bracing or bracing of trusses. ITWBCG connector plates are made of 2018/16GA (W/H/S/K) ASTM A653 steel (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on opposite sides. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility for the design shown. This design is the property of ITW Building Components Group Inc. and shall not be used for any building project without the written consent of ITW Building Components Group Inc. ITW-B002: www.itwbcg.com; TPI: www.tpinat.com; WCA: www.theindustry.com; ICC: www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

REF LET-IN VERT

DATE 1/1/09

DRWG GBLTIN0109

MAX TOT. LD. 60 PSF

DUR. FAC. ANY

MAX SPACING 24.0"

02/24/2011

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

GROUP A:

DOUGLAS FIR-LARCH SOUTHERN PINE

DOUGLAS FIR-LARCH SOUTHERN PINE

10

10

1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
24	24
25	25
26	26
27	27
28	28
29	29
30	30
31	31
32	32
33	33
34	34
35	35
36	36
37	37
38	38
39	39
40	40
41	41
42	42
43	43
44	44
45	45
46	46
47	47
48	48
49	49
50	50
51	51
52	52
53	53
54	54
55	55
56	56
57	57
58	58
59	59
60	60
61	61
62	62
63	63
64	64
65	65
66	66
67	67
68	68
69	69
70	70
71	71
72	72
73	73
74	74
75	75
76	76
77	77
78	78
79	79
80	80
81	81
82	82
83	83
84	84
85	85
86	86
87	87
88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
24	24
25	25
26	26
27	27
28	28
29	29
30	30
31	31
32	32
33	33
34	34
35	35
36	36
37	37
38	38
39	39
40	40
41	41
42	42
43	43
44	44
45	45
46	46
47	47
48	48
49	49
50	50
51	51
52	52
53	53
54	54
55	55
56	56
57	57
58	58
59	59
60	60
61	61
62	62
63	63
64	64
65	65
66	66
67	67
68	68
69	69
70	70
71	71
72	72
73	73
74	74
75	75
76	76
77	77
78	78
79	79
80	80
81	81
82	82
83	83
84	84
85	85
86	86
87	87
88	88
89	89
90	90
91	91
92	92
93	93
94	94
95	95
96	96
97	97
98	98
99	99
100	100

22

22

GABLE TRUSS DETAIL, NOTES:

LIVE LOAD DEFLECTION CRITERIA IS 1/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER
CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"

ATTACH EACH "L" BRACE WITH 10d NAILS.

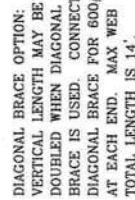
* (0.128" x 3" min) FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

*** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

4. "TOP & GENIC" SECTION FOR A REPRESENTATIVE OF THE

L BRACING MUS.
MEMBER 1 EN/ETH

DESIGN TRIPS TO COMMON SENSE



VERTICAL LENGTH SHOWN
IN TABLE ABOVE

CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL. WEB

15

[illegible]

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

The Trusses require extreme care in fabricating, handling, shipping, installing and erecting. The Contractor shall refer to the BCSI (Building Component Safety Information), by TPI and WCA) for safety precautions and instructions. Installers shall provide temporary bracing per BCSI unless otherwise specified. These panels and bottom chord shall have permanent lateral restraint of webs shall have general notes per information.

Locations B3 & B7. See this job's general notes per information.

ENTREPRENEURSHIP & REGIONAL DEVELOPMENT, 1999, VOL. 11, NO. 4, 331-344

IMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design.

any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, &

bracing of trusses. IWBCC connector plates are made of 20/16/16GA (W.H/S/K) ASTM A36C grade 3/4-40/60 (TK/W/H.S) galv. steel. Apply plates to each face of truss, positioned as shown above and for complete details.

A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely

of the truss component design shown. The suitability and use of this component for any building shall be the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.

TTW-BCG: www.itwbeg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

Refer to the following:
 1. Planning
 2. Design
 3. Construction
 4. Maintenance
 5. Demolition
 6. Relocation
 7. Abandonment
 8. Disposal
 9. Decommissioning
 10. Decontamination
 11. Dismantling
 12. Disposal
 13. Decommissioning
 14. Decontamination
 15. Dismantling
 16. Disposal
 17. Decommissioning
 18. Decontamination
 19. Dismantling
 20. Disposal
 21. Decommissioning
 22. Decontamination
 23. Dismantling
 24. Disposal
 25. Decommissioning
 26. Decontamination
 27. Dismantling
 28. Disposal
 29. Decommissioning
 30. Decontamination
 31. Dismantling
 32. Disposal
 33. Decommissioning
 34. Decontamination
 35. Dismantling
 36. Disposal
 37. Decommissioning
 38. Decontamination
 39. Dismantling
 40. Disposal
 41. Decommissioning
 42. Decontamination
 43. Dismantling
 44. Disposal
 45. Decommissioning
 46. Decontamination
 47. Dismantling
 48. Disposal
 49. Decommissioning
 50. Decontamination
 51. Dismantling
 52. Disposal
 53. Decommissioning
 54. Decontamination
 55. Dismantling
 56. Disposal
 57. Decommissioning
 58. Decontamination
 59. Dismantling
 60. Disposal
 61. Decommissioning
 62. Decontamination
 63. Dismantling
 64. Disposal
 65. Decommissioning
 66. Decontamination
 67. Dismantling
 68. Disposal
 69. Decommissioning
 70. Decontamination
 71. Dismantling
 72. Disposal
 73. Decommissioning
 74. Decontamination
 75. Dismantling
 76. Disposal
 77. Decommissioning
 78. Decontamination
 79. Dismantling
 80. Disposal
 81. Decommissioning
 82. Decontamination
 83. Dismantling
 84. Disposal
 85. Decommissioning
 86. Decontamination
 87. Dismantling
 88. Disposal
 89. Decommissioning
 90. Decontamination
 91. Dismantling
 92. Disposal
 93. Decommissioning
 94. Decontamination
 95. Dismantling
 96. Disposal
 97. Decommissioning
 98. Decontamination
 99. Dismantling
 100. Disposal
 101. Decommissioning
 102. Decontamination
 103. Dismantling
 104. Disposal
 105. Decommissioning
 106. Decontamination
 107. Dismantling
 108. Disposal
 109. Decommissioning
 110. Decontamination
 111. Dismantling
 112. Disposal
 113. Decommissioning
 114. Decontamination
 115. Dismantling
 116. Disposal
 117. Decommissioning
 118. Decontamination
 119. Dismantling
 120. Disposal
 121. Decommissioning
 122. Decontamination
 123. Dismantling
 124. Disposal
 125. Decommissioning
 126. Decontamination
 127. Dismantling
 128. Disposal
 129. Decommissioning
 130. Decontamination
 131. Dismantling
 132. Disposal
 133. Decommissioning
 134. Decontamination
 135. Dismantling
 136. Disposal
 137. Decommissioning
 138. Decontamination
 139. Dismantling
 140. Disposal
 141. Decommissioning
 142. Decontamination
 143. Dismantling
 144. Disposal
 145. Decommissioning
 146. Decontamination
 147. Dismantling
 148. Disposal
 149. Decommissioning
 150. Decontamination
 151. Dismantling
 152. Disposal
 153. Decommissioning
 154. Decontamination
 155. Dismantling
 156. Disposal
 157. Decommissioning
 158. Decontamination
 159. Dismantling
 160. Disposal
 161. Decommissioning
 162. Decontamination
 163. Dismantling
 164. Disposal
 165. Decommissioning
 166. Decontamination
 167. Dismantling
 168. Disposal
 169. Decommissioning
 170. Decontamination
 171. Dismantling
 172. Disposal
 173. Decommissioning
 174. Decontamination
 175. Dismantling
 176. Disposal
 177. Decommissioning
 178. Decontamination
 179. Dismantling
 180. Disposal
 181. Decommissioning
 182. Decontamination
 183. Dismantling
 184. Disposal
 185. Decommissioning
 186. Decontamination
 187. Dismantling
 188. Disposal
 189. Decommissioning
 190. Decontamination
 191. Dismantling
 192. Disposal
 193. Decommissioning
 194. Decontamination
 195. Dismantling
 196. Disposal
 197. Decommissioning
 198. Decontamination
 199. Dismantling
 200. Disposal
 201. Decommissioning
 202. Decontamination
 203. Dismantling
 204. Disposal
 205. Decommissioning
 206. Decontamination
 207. Dismantling
 208. Disposal
 209. Decommissioning
 210. Decontamination
 211. Dismantling
 212. Disposal
 213. Decommissioning
 214. Decontamination
 215. Dismantling
 216. Disposal
 217. Decommissioning
 218. Decontamination
 219. Dismantling
 220. Disposal
 221. Decommissioning
 222. Decontamination
 223. Dismantling
 224. Disposal
 225. Decommissioning
 226. Decontamination
 227. Dismantling
 228. Disposal
 229. Decommissioning
 230. Decontamination
 231. Dismantling
 232. Disposal
 233. Decommissioning
 234. Decontamination
 235. Dismantling
 236. Disposal
 237. Decommissioning
 238. Decontamination
 239. Dismantling
 240. Disposal
 241. Decommissioning
 242. Decontamination
 243. Dismantling
 244. Disposal
 245. Decommissioning
 246. Decontamination
 247. Dismantling
 248. Disposal
 249. Decommissioning
 250. Decontamination
 251. Dismantling
 252. Disposal
 253. Decommissioning
 254. Decontamination
 255. Dismantling
 256. Disposal
 257. Decommissioning
 258. Decontamination
 259. Dismantling
 260. Disposal
 261. Decommissioning
 262. Decontamination
 263. Dismantling
 264. Disposal
 265. Decommissioning
 266. Decontamination
 267. Dismantling
 268. Disposal
 269. Decommissioning
 270. Decontamination
 271. Dismantling
 272. Disposal
 273. Decommissioning
 274. Decontamination
 275. Dismantling
 276. Disposal
 277. Decommissioning
 278. Decontamination
 279. Dismantling
 280. Disposal
 281. Decommissioning
 282. Decontamination
 283. Dismantling
 284. Disposal
 285. Decommissioning
 286. Decontamination
 287. Dismantling
 288. Disposal
 289. Decommissioning
 290. Decontamination
 291. Dismantling
 292. Disposal
 293. Decommissioning
 294. Decontamination
 295. Dismantling
 296. Disposal
 297. Decommissioning
 298. Decontamination
 299. Dismantling
 300. Disposal
 301. Decommissioning
 302. Decontamination
 303. Dismantling
 304. Disposal
 305. Decommissioning
 306. Decontamination
 307. Dismantling
 308. Disposal
 309. Decommissioning
 310. Decontamination
 311. Dismantling
 312. Disposal
 313. Decommissioning
 314. Decontamination
 315. Dismantling
 316. Disposal
 317. Decommissioning
 318. Decontamination
 319. Dismantling
 320. Disposal
 321. Decommissioning
 322. Decontamination
 323. Dismantling
 324. Disposal
 325. Decommissioning
 326. Decontamination
 327. Dismantling
 328. Disposal
 329. Decommissioning
 330. Decontamination
 331. Dismantling
 332. Disposal
 333. Decommissioning
 334. Decontamination
 335. Dismantling
 336. Disposal
 337. Decommissioning
 338. Decontamination
 339. Dismantling
 340. Disposal
 341. Decommissioning
 342. Decontamination
 343. Dismantling
 344. Disposal
 345. Decommissioning
 346. Decontamination
 347. Dismantling
 348. Disposal
 349. Decommissioning
 350. Decontamination
 35

REF	ASCE7-05-GABI1015
DATE	1/1/09
DRWG	A11015050109

MAX. TOT. I.D. 60 PSF

MAX SPACING	24.0"
1.0"	1.0"
1.5"	1.5"
2.0"	2.0"
2.5"	2.5"
3.0"	3.0"
3.5"	3.5"
4.0"	4.0"
4.5"	4.5"
5.0"	5.0"
5.5"	5.5"
6.0"	6.0"
6.5"	6.5"
7.0"	7.0"
7.5"	7.5"
8.0"	8.0"
8.5"	8.5"
9.0"	9.0"
9.5"	9.5"
10.0"	10.0"
10.5"	10.5"
11.0"	11.0"
11.5"	11.5"
12.0"	12.0"
12.5"	12.5"
13.0"	13.0"
13.5"	13.5"
14.0"	14.0"
14.5"	14.5"
15.0"	15.0"
15.5"	15.5"
16.0"	16.0"
16.5"	16.5"
17.0"	17.0"
17.5"	17.5"
18.0"	18.0"
18.5"	18.5"
19.0"	19.0"
19.5"	19.5"
20.0"	20.0"
20.5"	20.5"
21.0"	21.0"
21.5"	21.5"
22.0"	22.0"
22.5"	22.5"
23.0"	23.0"
23.5"	23.5"
24.0"	24.0"

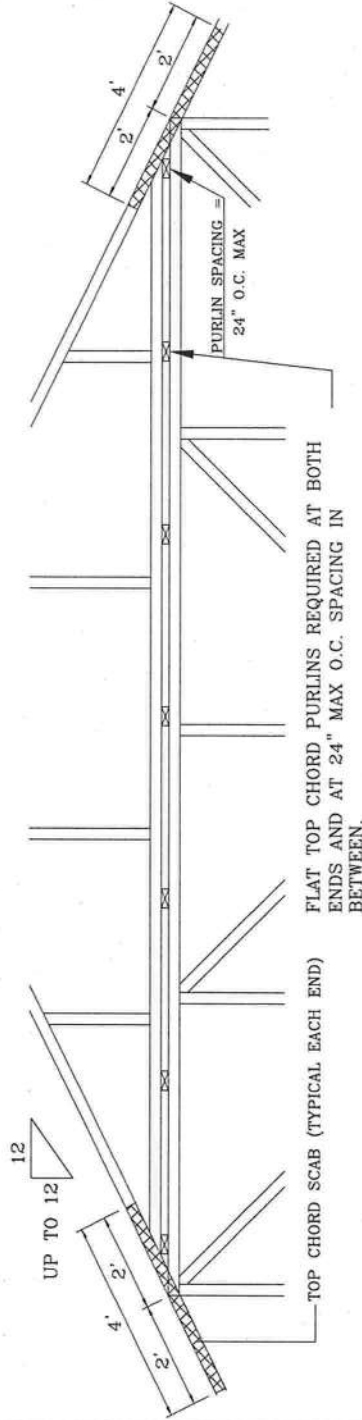
Earth City, MO 63045

120 PIGGYBACK DETAIL

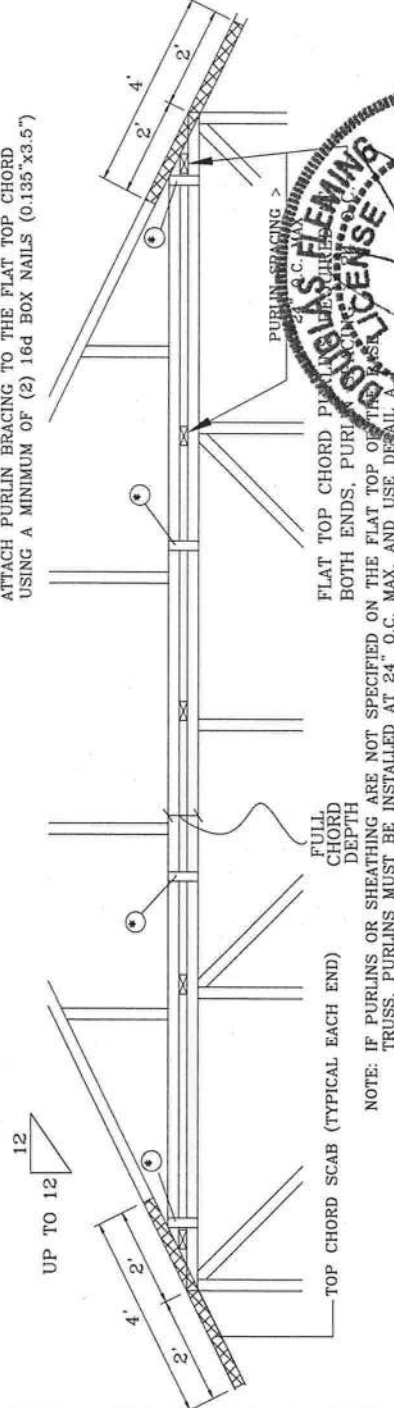
UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF (MIN), Kzt=1.0.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING OR ANY OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS, AND LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS. ** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS



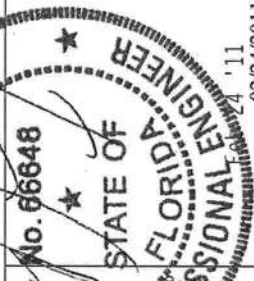
DETAIL B : PURLIN SPACING > 24" O.C.



Earth City, MO 63045

WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses are designed for extreme wind loading. Shipping, installing and bracing. Refer to and follow BCSI (Building Components Safety Institute) TPI (Truss Plate Institute) for safety instructions and these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Inc. (ITWBCI) shall be responsible for any deviation from the design. Any failure to build the truss in conformance with TPI or fabricating, handling, shipping, installing or bracing of trusses. ITWBCI connector plates are made of 2018/18CA (W.H/S/K) ASTM A653 grade 50 (K/W/H/S) galv. steel. Apply plates to each face of truss, positioned as shown above and on joint. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility for the truss component design shown. The suitability and use of this component for any building responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. ITW-BDC: www.itwbdc.com, TPI: www.tpiinst.com, WTK: www.sociindustry.com, ICC: www.iccsafe.org



REF	PIGGYBACK
DATE	03/15/10
DRWG	PB1200310
SPACING	24.0"

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8" O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

APA RATED GUSSET
8"x8"x7/16" (MIN) APA RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8" O.C. WITH (9) 6d COMMON (0.113"x2") NAILS PER GUSSET. (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS
2x4 SPT#2, FULL CHORD DEPTH SCABS (EACH FACE). ATTACH @ 8" O.C. WITH (6) 10d BOX NAILS (0.128"x3") PER SCAB. (3) IN CAP BOTTOM CHORD AND (3) IN BASE TRUSS TOP CHORD. SCABS MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8" O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4" O.C. FRONT TO BACK FACES.

MAXIMUM TRUSS SPACING IS 24" O.C. DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2x4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")

THE TOP CHORD #3 GRADE 2x4 SCAB MAY BE REPLACED WITH EITHER OF THE FOLLOWING: (1) 3x8 TRULOX PLATE ATTACHED WITH (8) 0.120"x1.375" NAILS, (4) INTO CAP TC & (4) INTO BASE TRUSS TC OR (1) 28PB WAVE PIGGYBACK PLATE PLATED TO THE PIGGYBACK TRUSS TC AND ATTACHED TO THE BASE TRUSS TC WITH (4) 0.120"x1.375" NAILS. NOTE: NAILING THRU HOLES OF WAVE PLATE IS ACCEPTABLE.