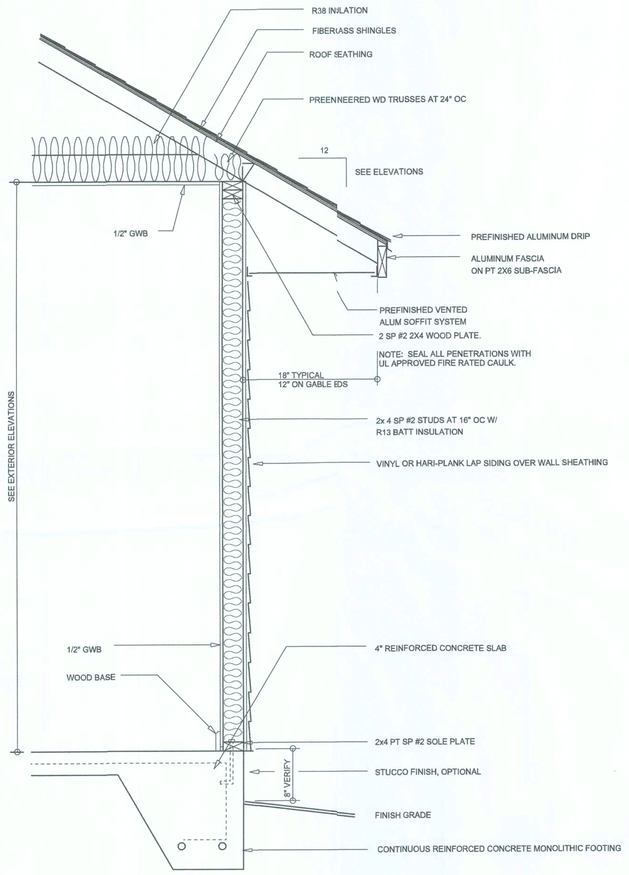
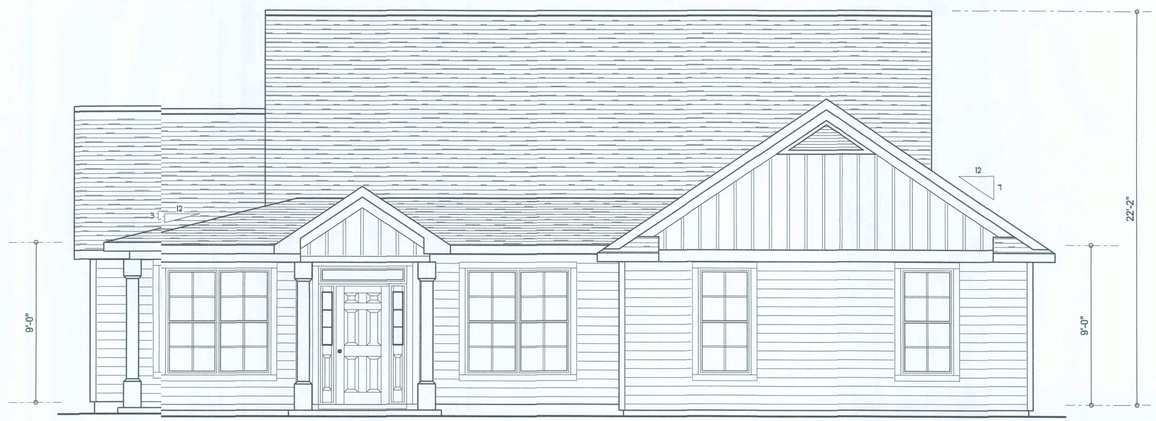


TYPICAL WALL SECTION

SCALE: 1" = 1'- 0"

REAR ELEVATION

SCALE: 1/4" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

COLUMBIA COUNTY BUILDING DEPARTMENT
FILE COPY
Received for Code Compliance
PLANS EXAMINER

REVISIONS
February 13, 2019
March 18, 2020

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

FRONT & REAR ELEVATIONS
SCALE: 1/4" = 1'-0"

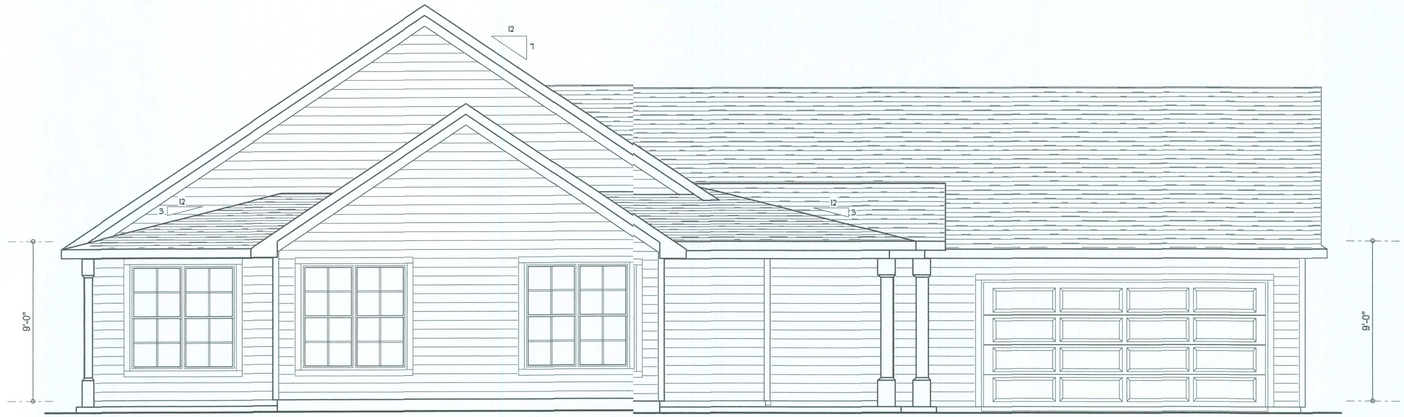
TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

MODEL 1840 (RIGHT-HAND) FOR:
LOT 9, *EMERALD COVE*
Project Address: : 445 SW Heatheridge Dr, Lake City, Florida 32024
GIBRALTAR CONTRACTING, LLC.
LIC# 1259833 HIGH SPRINGS, FLORIDA

426 SW COMMERCE DR, STE 130
LAKE CITY, FL 32025
(386 758-8406
will@willmyers.net

JOB NUMBER
20200312

SHEET NUMBER
A.1
OF 4 SHEETS



LEFT ELEVATION

SCALE: 1/4" = 1'-0"

RGHT ELEVATION

SCAL: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
February 13, 2019
March 18, 2020

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

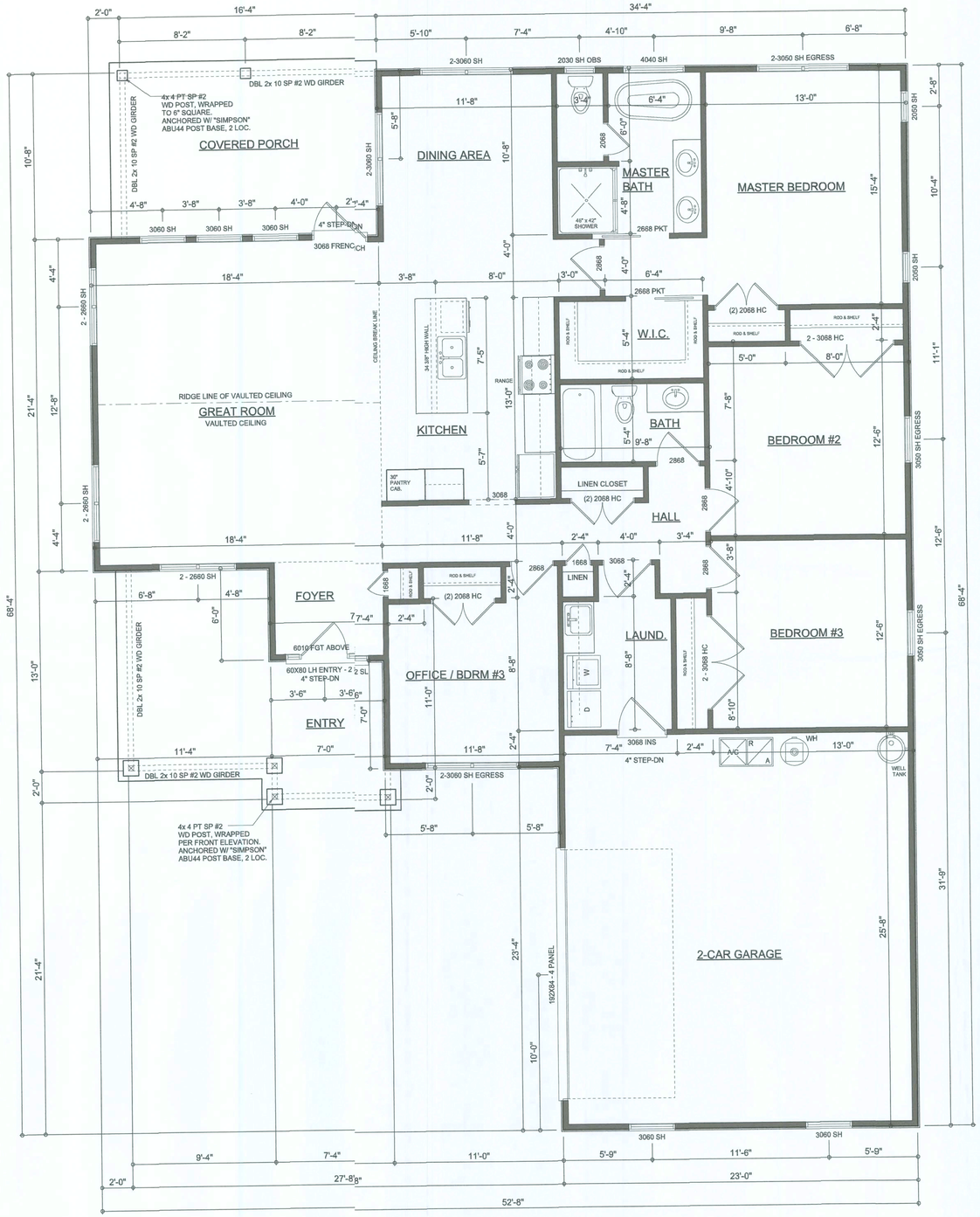
LEFT & RIGHT ELEVATIONS
SCALE: 1/4" = 1'-0"

MODEL 1840 (RIGHT-HAND) FOR:
LOT 9, *EMERALD COVE*
Project Address: : 445 SW Heatherlidge Dr, Lake City, Florida 32024
GIBRALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.
426 SW COMMERCE DR, STE 130
LAKE CITY, FL 32025
(386 758-8406
will@willmyers.net

JOBNUMBER
20200312

SHEET NUMBER
A.2
OF 4 SHEETS



FLOOR PLAN

SCALE: 1/4" = 1'-0"

NOTE: ALL WALLS SHALL BE 9'-0" UNLESS OTHERWISE NOTED.

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Grages beneath habitable rooms shall be separated from all habitable rooms above by not les than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and tb dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doc not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a prate garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport providethe carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, evise shall have a minimum 20 min. fire rating.

AREA SUMMARY

LIVING AREA	1,933	S.F.
GARAGE AREA	582	S.F.
COVERED PORCH AREA	174	S.F.
ENTRY PORCH AREA	180	S.F.
TOTAL AREA	2,869	S.F.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
February 13, 2019
March 18, 2020

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

DIMENSIONED FLOOR PLAN
SCALE: 1/4" = 1'-0"

MODEL 1840 (RIGHT-HAND) FOR:
LOT 9, EMERALD COVE
Project Address: : 445 SW Heatheridge Dr, Lake City, Florida 32024
GIBRALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.
426 SW COMMERCE DR, STE 130
LAKECITY, FL 32025
(386) 758-8406
will@willmyers.net

JOB NUMBER
20200312

SHEET NUMBER
A.3
OF 4 SHEETS

ELECTRICAL LEGEND

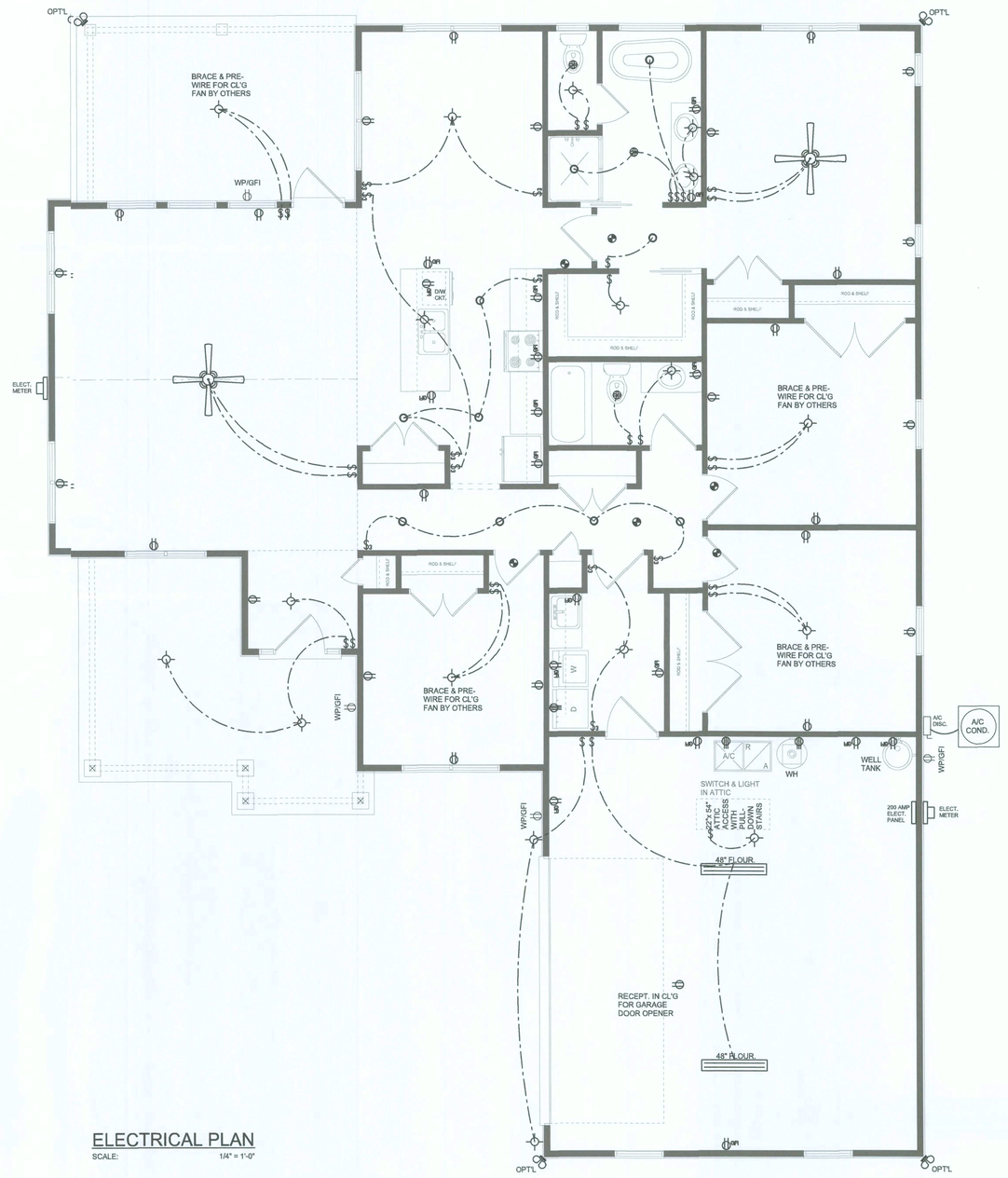
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
GFI	GFI DUPLEX OUTLET (PER NEC 406.8)
TV	TELEVISION JACK
PH	TELEPHONE JACK
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
$	WALL SWITCH
$3	3 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
48" FLOUR.	2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL INTERIOR RECPTACLES SHALL BE AFCI
(ARC FAULT CIRCU INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL SMOKE DETECORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE ETTERY BACKUP POWER
AND ALL WIRED TCETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SRVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THEXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USE FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVFOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED ASN EQUIPMENT GROUND.

IT IS THE LICENSEELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL
WORK PERFORMEAND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE NFPA70 2014 NATIONAL
ELECTRIC CODE AD ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
February 13, 2019
March 10, 2020

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

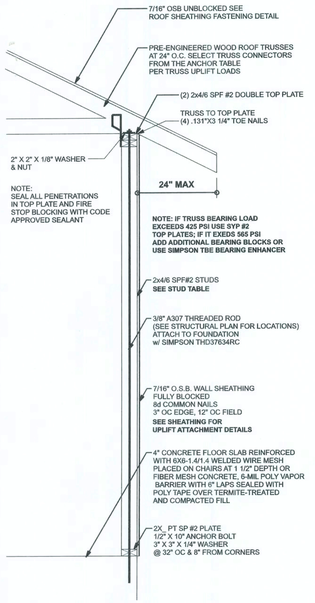
ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

MODEL 1840 (RIGHT-HAND) FOR:
LOT 9, *EMERALD COVE*
Project Address: : 445 SW Heatheridge Dr, Lake City, Florida 32024
GIBRALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WMDESIGN & ASSOCIATES, INC.
425 SW COMMERCE DR, STE 130
LAKE CTY, FL 32025
(386)758-8406
will@willmyers.net

JOB NUMBER
20200312

SHEET NUMBER
A.4
OF I SHEETS



ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

RING-SHANK NAILS SHALL HAVE THE FOLLOWING MINIMUM DIMENSIONS:

1. 0.113 INCH NOMINAL ROOT SHANK DIAMETER
2. RING DIAMETER OF 0.010 OVER SHANK DIAMETER
3. 16 TO 20 RINGS PER INCH
4. 0.280 INCH FULL ROUND HEAD DIAMETER
5. 2-3/8 INCH NAIL LENGTH

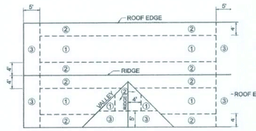
NAILING PATTERN SHALL BE:

4" OC GABLE END (SEE GABLE BRACING DETAIL)

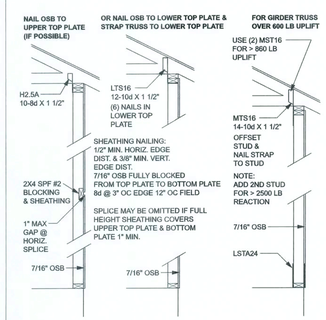
6" OC @ EDGES ALL ZONES

6" OC @ INTERMEDIATE FRAMING IN ZONE 3

12" OC @ INTERMEDIATE FRAMING IN ZONE 1 & 2

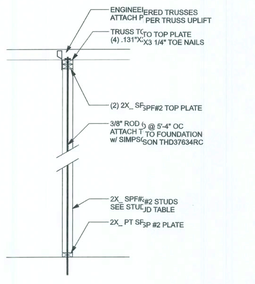


ROOF SHEATHING FASTENING



SHEATHING FOR UPLIFT ATTACHMENT DETAILS

ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ RODS

CONNECTOR TABLE				
Uplift SP	**Uplift SPF**	**Truss Connector**	**To Plate**	**To Truss/Rafter**
615	485	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	495	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	620	LTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1240	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
Uplift SP	**Uplift SPF**	**Strap Ties**	**To One Member**	**To Other Member**
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	**Uplift SPF**	**Stud Plate Ties**	**To Stud**	**To Plate**
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	**Uplift SPF**	**Holdowns @ Stemwall**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Holdowns @ Mono**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x9" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Post Bases @ Stemwall**	**To Post**	**Anchor**
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	**Uplift SPF**	**Post Bases @ Mono**	**To Post**	**Anchor**
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI, ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

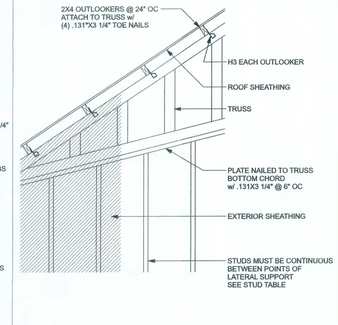
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



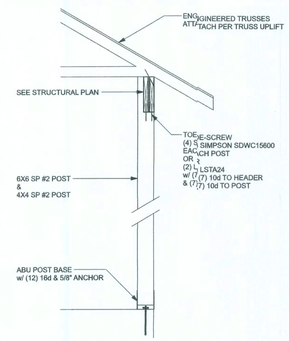
SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCISED

(TYP.) GABLE BRACING DETAIL

WOOD FRAME

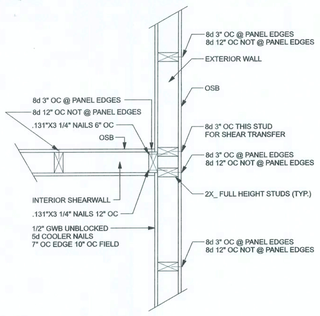
(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



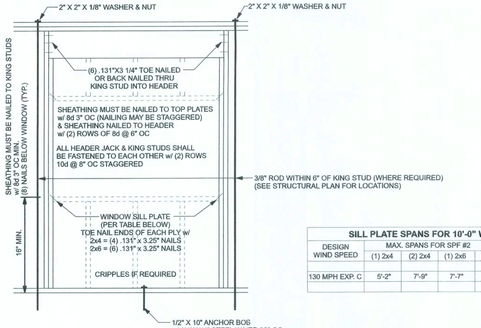
(TYP.) PORCH POST

ONE STORY WOOD



(TYP.) INTERSECTING WALL FRAMING

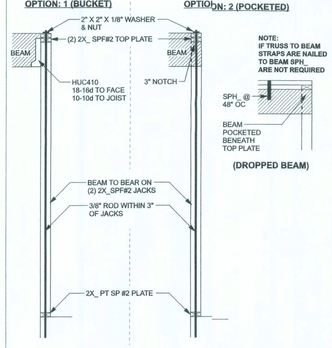
WOOD FRAME



SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.23B
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

TYPICAL HEADER STRAPING DETAIL

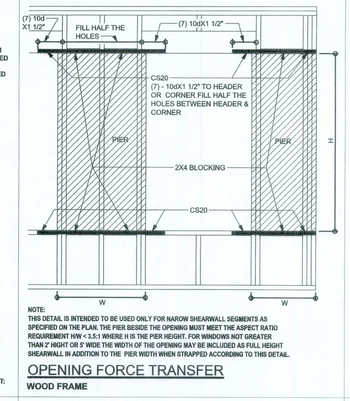
ONE STORY WOOD FRAME



(TYP.) BEAM TO WALL

WOOD FRAME w/ RODS

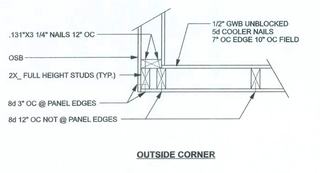
ALLOWABLE UPLIFT: 1770 LB



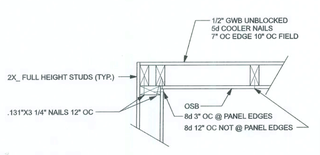
NOTE:
THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT H/W < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGHT OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

OPENING FORCE TRANSFER

WOOD FRAME



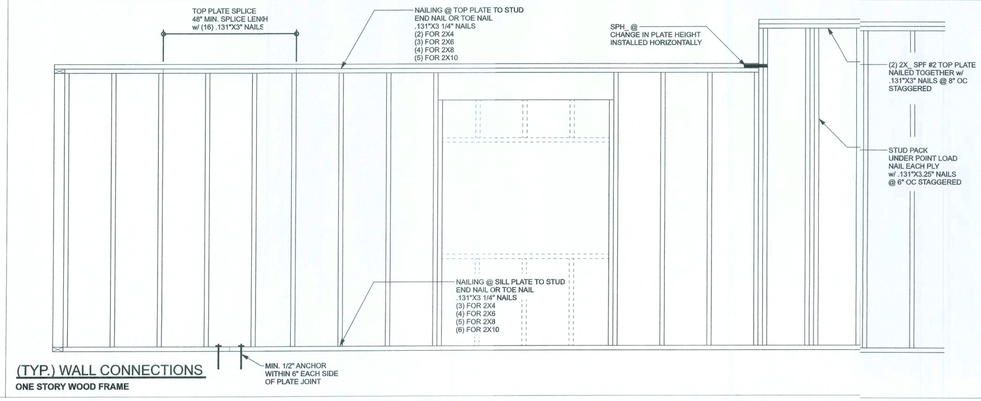
OUTSIDE CORNER



INSIDE CORNER

(TYP.) CORNER FRAMING

WOOD FRAME



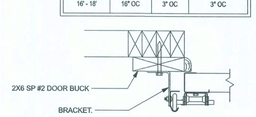
(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK, HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME

DESIGN CRITERIA & LOADS:	
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)
CODE FOR DESIGN LOADS	ASCE 7-10
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C) (Vult)				
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+42.6	-46.2	+42.6	-57

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)		
9x7 GARAGE DOOR	+22.6	-25.5
16x7 GARAGE DOOR	+21.7	-24.1

Gibraltar Contracting, LLC

1840 Model - Lot 9 Emerald Cove

PROJECT ADDRESS: Lot 9 Emerald Cove 445 Heatheridge Dr. Lake City, FL

DIMENSIONS: Stated dimensions supecede scaled dimensions. Refer all questions to Mark Disoway, P.E. for esolution. Do not proceed without larification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relang to wind engineering comply with the 6th Editin Florida Building Code Residentil (2017) to the best of my knowlage.

LIMITATION: This desig is valid for one building, at specified loction.

MARK DISOSWAY P.E. 53915

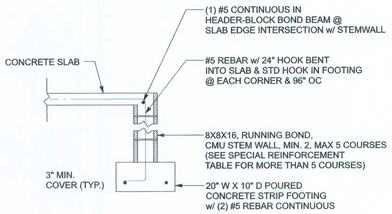
Tuesday, Mach 24, 2020

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Fbrida 32025
386.755.5419
disoswaydesign@gmail.com

JOB NUMBER: 200328

S-1

OF 3 SHEETS



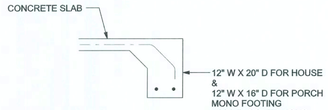
F1 S-2 OPTIONAL STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:

The table assumes 1 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at thiop. The vertical steel is to be placed toward the tension side of the CMU wall (away frothe soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add krowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 contiuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement ashown in the table below.

STEMWALL HEIGHT (FEET)	UNBALACED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



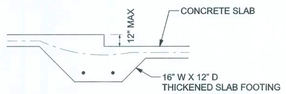
F1 S-2 MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"

MASONRY NOTE:
MASONRY CONSTUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY SUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTC AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NCIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 531-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONSTO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IMRITING.

	ACI530.1-02 Ection	Specific Requirements
1.4A	Compressive ength	8" block bearing walls Fm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standarc	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick staard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing ba, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for cosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for cosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduitand accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joir	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

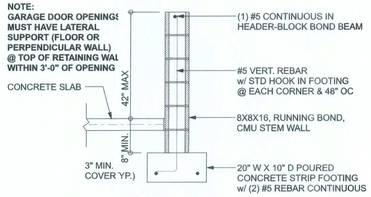
BOTTOM OF EXERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNSTURBED SOIL OR ENGINEERED FILL PER FBC 2014-ES. SECTION R403.1.4



F3 S-2 INTERIOR BEARING STEP FOOTING

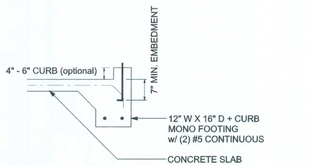
SCALE: 1/2" = 1'-0"

NOTE:
GARAGE DOOR OPENINGS MUST HAVE LATERAL SUPPORT (FLOOR OR PERPENDICULAR WALL) @ TOP OF RETAINING WALL WITHIN 3'-0" OF OPENING



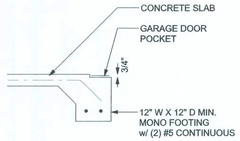
F4 S-2 OPTIONAL STEM WALL CURB FOOTING

SCALE1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING

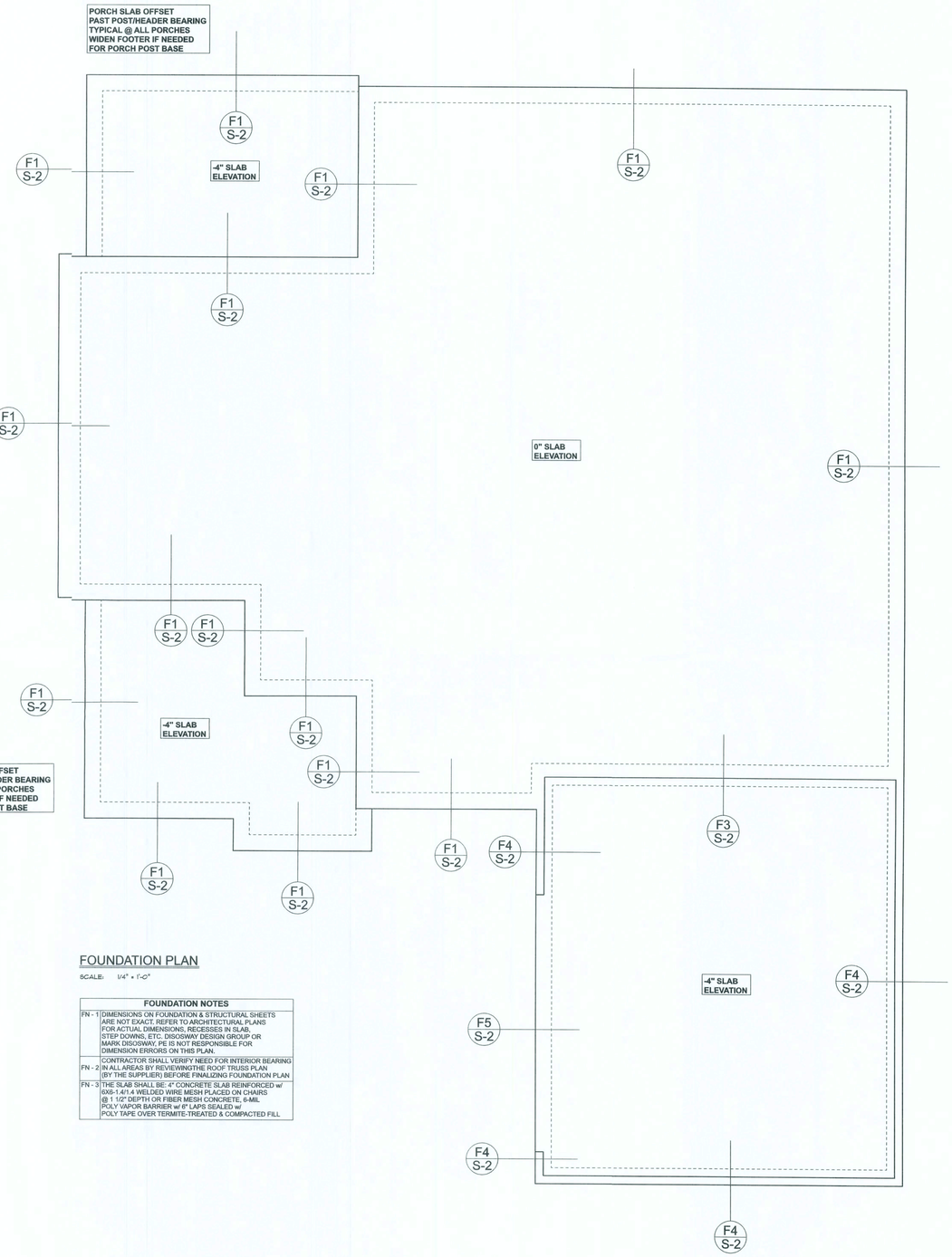
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING

SCALE: 1/2" = 1'-0"

PORCH SLAB OFFSET PAST POST/HEADER BEARING TYPICAL @ ALL PORCHES WIDEN FOOTER IF NEEDED FOR PORCH POST BASE



PORCH SLAB OFFSET PAST POST/HEADER BEARING TYPICAL @ ALL PORCHES WIDEN FOOTER IF NEEDED FOR PORCH POST BASE

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

Gibraltar Contracting, LLC

1840 Model - Lot 9 Emerald Cove

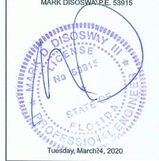
PROJECT ADDRESS:
Lot 9 Emerald Cove 445 Hatharidge Dr.
Lake City, FL

DIMENSIONS:
Stated dimensions supercde scaled dimensions. Refer all questons to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby ertify that I have examined this plan, and tht the applicable portions of the plan, relatis to wind engineering comply with the 6th EdiionFlorida Building Code Residential (017) to the best of my knowledg.

LIMITATION: This design invalid for one building, at specified locatin.

MARK DISOSWAY P.E. 53915

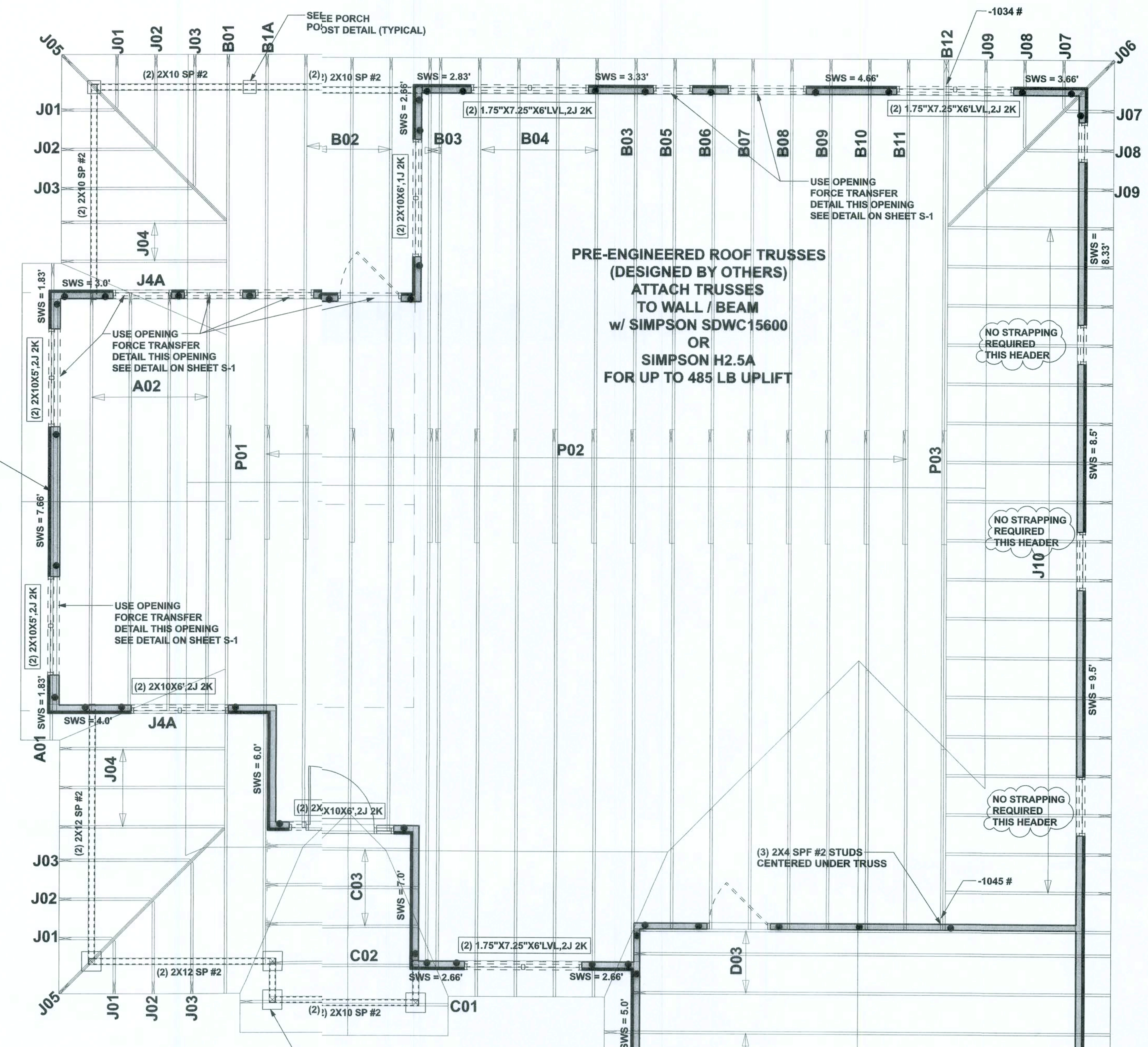


Tuesday, March24, 2020

Mark Disosway P.E.
163 SW Midtown Place
Suite '03
Lake City, Floida 32025
386.754.3419
disoswaydesign@gmail.com

JOB NUMBER:
200328

S-2
OF 3 SHEETS



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD

SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

(2) 2X10X0',1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

THREADED ROD LEGEND

INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

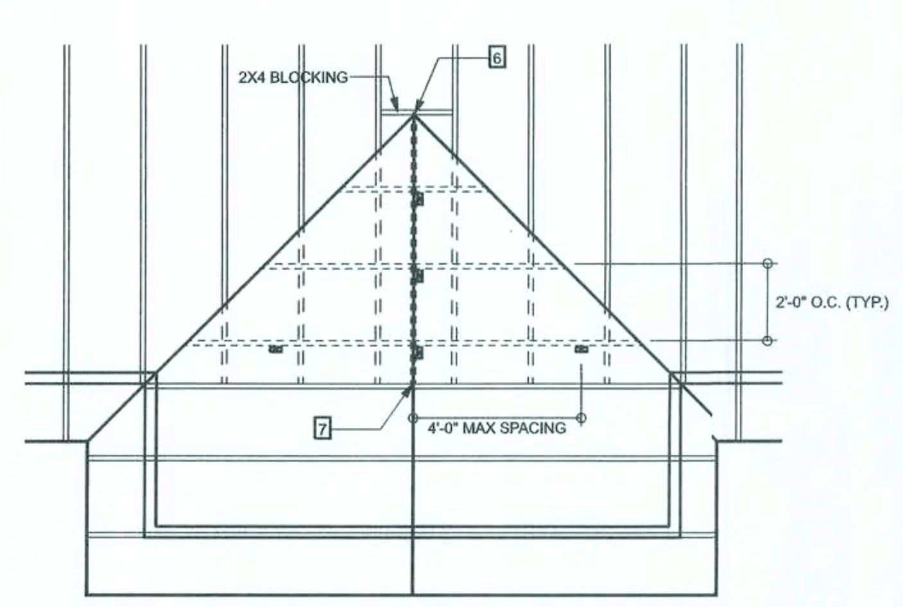
ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDUNAL
ACTUAL	38496 LBF	15984 LBF
REQUIRED	14231 LBF	14719 LBF

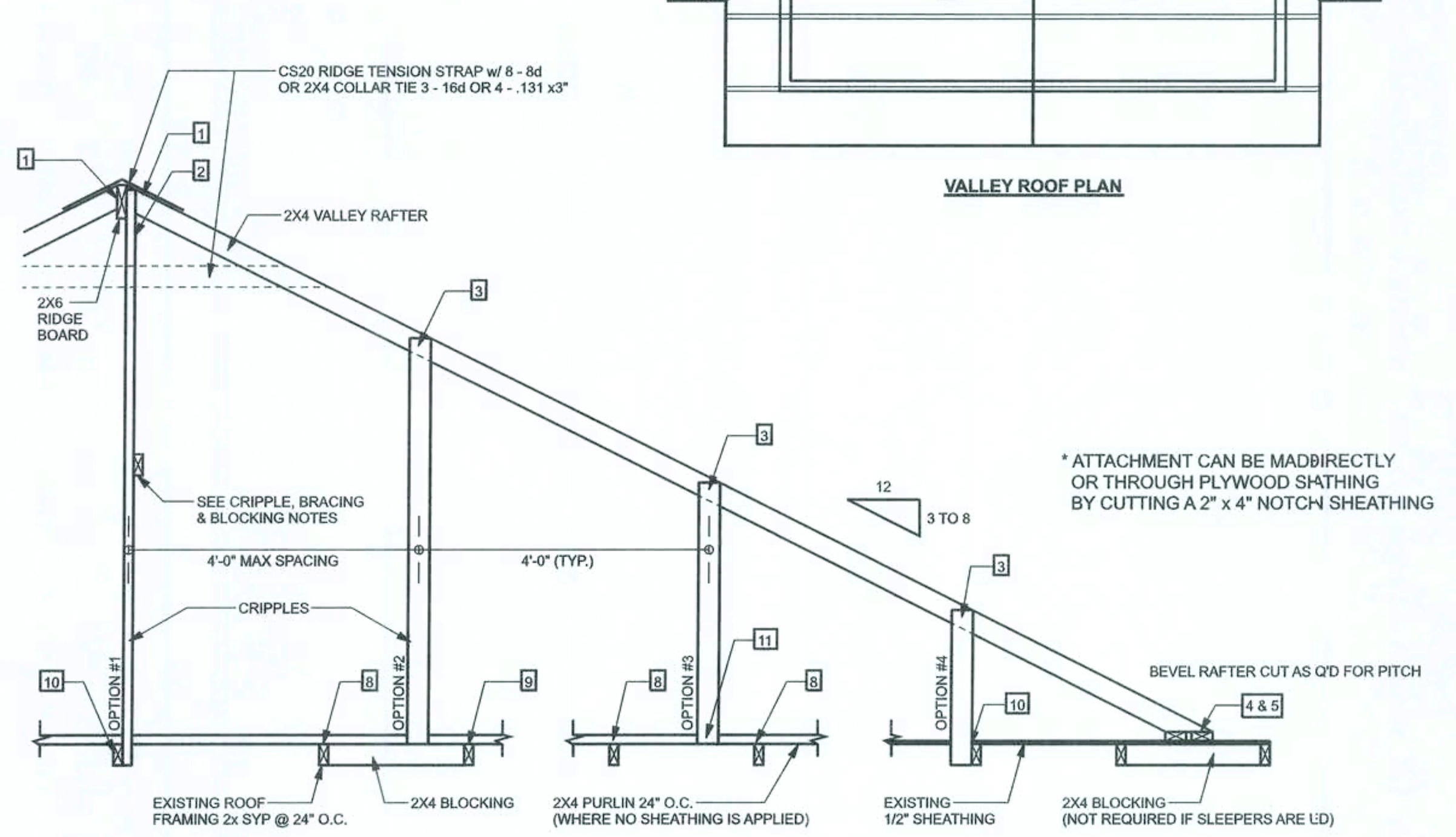
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #20-4050

LUMBER SIZE & GRADE MINUMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHEREN PINE MATERIAL



VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3 -16d OR 6 - .131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6 - .131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6 - .131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12 - .131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6 - .131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6 - .131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6 - .131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8 - .131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6 - .131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6 - .131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6 - .131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF AREA PER SUPPORT
16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 16ft2 OR 2'-0" X 8'-0" SPAN = 16ft2)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
-SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
-MAXIMUM WIND SPEED: 130 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS
- EXPOSURE CATEGORY "C", I = 1.0, Kzt = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQURE TWO CLB's OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S

Gibraltar Contracting, LLC

1840 Model - Lot 9 Emerald Cove

PROJECT ADDRESS:
Lot 9 Emerald Cove 445 Heatheridge Dr.
Lake City, FL

DIMENSIONS:
Stated dimensions supecede scaled dimensions. Refer all qustions to Mark Disosway, P.E. for esolution. Do not proceed without carification.

CERTIFICATION: I herely certify that I have examined this plan, and hat the applicable portions of the plan, relatng to wind engineering comply with the 6th Editin Florida Building Code Residentia (2017) to the best of my knowlege.

LIMITATION: This desigris valid for one building, at specified locaion.

MARK DISOSWAY P.E. 53915

MARK D DISOSWAY III LICENSE No 53915 STATE OF FLORIDA PROFESSIONAL ENGINEER

Tuesday, March 24, 2020

Mark Disosway P.E.
163 SW Midtown Place
Suite103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
200328

S-3
OF 3 SHEETS