DATE 03/09/2006

Columbia County Building Permit

PERMIT 000024213

This Permit Expires One Year From the Date of Issue

APPLICANT CHARESE NORTON PHONE 752-3331

ADDRESS 3367 S US HWY 441 SIUTE 101 LAKE CITY FL 32025

OWNER JOHN & LANELLE HATCHER PHONE 754-9544

ADDRESS 247 SE PUEBLO WAY LAKE CITY FL 32025

CONTRACTOR JAMES NORTON PHONE 722-3331

LOCATION OF PROPERTY EAST BAYA AVE, R COUNTRY CLUB, R WOODHAVEN, R CLUSTER, L PATIENCE, R PUEBLO, LOT ON RIGHT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 81350.00

HEATED FLOOR AREA 1627.00 TOTAL AREA 2221.00 HEIGHT 20.50 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE AE DEVELOPMENT PERMIT NO. 06-007

PARCEL ID 09-4S-17-08300-089 SUBDIVISION WOODHAVEN

LOT 17 BLOCK PHASE UNIT 4 TOTAL ACRES 0.40

000000995		RB0031780	Charese J. North
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

PERMIT	05-1287-N	BK	JH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD, NOC ON FILE, WATER SYSTEM ONE FOOT RISE ON FILE, MINIMUM FLOOR ELEVATION 105', NEED ELEVATION CERTIFICATE BEFORE POWER

Check # or Cash 21410/21411

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 410.00 CERTIFICATION FEE $ 11.11 SURCHARGE FEE $ 11.11

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ 50.00 FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 582.22

INSPECTORS OFFICE L. H. CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

0602-9

Prepared by:
Marie E. Henkel, Esq.
M.E. Henkel, P.A.
3560 S. Magnolia Ave.
Orlando, FL 32806
File #: 06-08

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvement will be made to certain real property and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Description of Property:

Lot 17, WOODHAVEN UNIT IV, according to the plat thereof as recorded in Plat Book 4, Page 108 , Public Records of Columbia County, Florida.

Property Address: 247 S.E. Pueblo Way, Lake City, FL 32025

1. General Description of Improvements: Residential dwelling

2. Owner Information:
   a. John D. Hatcher, Sr. and Lanelle G. Hatcher
   2675 S.E. Country Club Rd., #101
   Lake City, FL 32025

   b. Interest in property: fee simple

   c. Name and address of fee simple title holder (if other than Owner): none

   Contractor: Norton Home Improvement Co. Inc.
   3367 S. US Hwy. 441, Suite 101
   Lake City, FL 32025

3. Surety: None

4. Lender Information: Orlando Federal Credit Union
   1117 Westmoreland St.
   Orlando, Florida 32805
   Attn.: Gloria Flower (407) 835-3500 Ext. 3529

5. Persons within the State of Florida designated by owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)(7): None

6. In addition to themselves, owners designate the following to receive a copy of the lienor's notice as provided in Section 713.13(1)(b):

7. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):

John D. Hatcher, Sr.

Lanelle G. Hatcher

STATE OF FLORIDA
COUNTY OF ORANGE

The foregoing instrument was acknowledged before me this 7th day of February, 2006 by John D. Hatcher, Sr. and Lanelle G. Hatcher who produced a driver's license as identification and who did not take an oath.

Notary Public – State of Florida

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Teagle
Deputy Clerk
Date 2-9-2006

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Inst:2006003156 Date:02/09/2006 Time:11:21
DC,P.DeWitt Cason,Columbia County B:1073 P:1500

Columbia County Building Permit Application

ck# 21410 Revised 9-23-04

For Office Use Only Application # 0602-09 Date Received 3/3 By JW Permit # 24213/995

Application Approved by - Zoning Official BLK Date 07.03.06 Plans Examiner OKSTH Date 3-8-06

Flood Zone AE Development Permit YES Zoning RSF-2 Land Use Plan Map Category RES. LOW DEN.

Comments ~~- NOC~~ - NEED to Provide Engineer Grading Plan MIN FFE 105'

HAS WATER METER

752-6427

Applicants Name Charese Norton Phone 386-752-3331

Address 3367 S. US. HWY 441, Suite 101, LAKE City, FL 32025

Owners Name JOHN & LANELLE HATCHER Phone 386-754-9455

911 Address 247 SE PUEBLO WAY, LAKE CITY, FL 32025

Contractors Name James H. Norton Phone 386-752-3331

Address 3367 S. US. HWY 441, Suite 101, LAKE City, FL 32025

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tim Delbene, 192 SW Sagewood Gln, LAKE City, FL 32024 MARK DISOWAY, PO BOX 868, LAKE City, FL 32056

Mortgage Lenders Name & Address Orlando Credit Union, Orlando, FL

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 09-4S-17-08300-089 Estimated Cost of Construction $110,000.00

Subdivision Name Woodhaven Lot 17 Block - Unit 4 Phase -

Driving Directions BAYA AVENUE EAST, TR ON COUNTRY CLUB, TR ON WOODHAVEN, TR ON CUSTER, TL ON PATIENCE, TR ON PUEBLO, LOT ON RIGHT

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage .4 acres Lot Size _____ Do you need a (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' Side 25' Side 69.2' Rear 40'

Total Building Height 20'5" Number of Stories 1 Heated Floor Area 1627 Roof Pitch 7/12

594 Porches TOTAL 2221

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H. Norton

Owner Builder or Agent (Including Contractor)

James H. Norton

Contractor Signature

Contractors License Number RB0031780

Competency Card Number 5553

STATE OF FLORIDA
COUNTY OF COLUMBIA

Patricia T Peeler
My Commission DD129986
Expires September 05, 2006

NOTARY STAMP/SEAL

Sworn to (or affirmed) and subscribed before me this 2nd day of February 2004.

Personally known _____ or Produced Identification ✓

Patricia T. Peele

Notary Signature

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000995

DATE 03/09/2006 PARCEL ID # 09-4S-17-08300-089

APPLICANT CHARESE NORTON PHONE 752-3331

ADDRESS 3367 S US HWY 441, SUITE 101 LAKE CITY FL 32025

OWNER JOHN & LANELLE HATCHER PHONE 754-9455

ADDRESS 247 SE PUEBLO WAY LAKE CITY FL 32025

CONTRACTOR JAMES NORTON PHONE 752-3331

LOCATION OF PROPERTY EAST BAYA AVE, R COUNTRY CLUB, R WOODHAVEN, R CLUSTER, L PATIENCE, R PUEBLO, LOT ON RIGHT

SUBDIVISION/LOT/BLOCK/PHASE/UNIT WOODHAVEN 17 4

SIGNATURE Charese J. Norton

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GAINESVILLE FLORIDA

Columbia County Building Department
Flood Development Permit

Development Permit
F 023- 06-007

DATE 03/09/2006 BUILDING PERMIT NUMBER 000024213

APPLICANT CHARESE NORTON PHONE 752-3331

ADDRESS 3367 S US HWY 441 SIUTE 101 LAKE CITY FL 32025

OWNER JOHN & LANELLE HATCHER PHONE 754-9544

ADDRESS 247 SE PUEBLO WAY LAKE CITY FL 32025

CONTRACTOR JAMES NORTON PHONE 722-3331

ADDRESS 3367 S US HWY 441 SUITE 101 LAKE CITY FL 32025

SUBDIVISION WOODHAVEN Lot 17 Block ____ Unit ____ Phase ____

TYPE OF DEVELOPMENT SFD, UTILITY PARCEL ID NO. 09-4S-17-08300-089

FLOOD ZONE AE BY BK 1-6-88 FIRM COMMUNITY #. 120070 - PANEL #. 200 B

FIRM 100 YEAR ELEVATION 104 PLAN INCLUDED YES or NO

REQUIRED LOWEST HABITABLE FLOOR ELEVATION 105

IN THE REGULATORY FLOODWAY YES or NO RIVER N/A

SURVEYOR / ENGINEER NAME Dale Johns LICENSE NUMBER 45-263

✓ ONE FOOT RISE CERTIFICATION INCLUDED

_____ ZERO RISE CERTIFICATION INCLUDED

_____ SRWMD PERMIT NUMBER _____
(INCLUDING THE ONE FOOT RISE CERTIFICATION)

DATE THE FINISHED FLOOR ELEVATION CERTIFICATE WAS PROVIDED _____

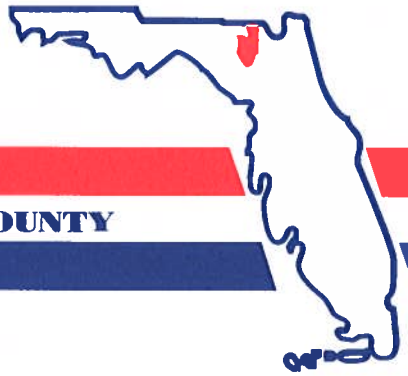
INSPECTED DATE _____ BY _____

COMMENTS _____

_____

135 NE Hernando Ave., Suite B-21
Lake City, Florida 32055
Phone: 386-758-1008
Fax: 386-758-2160

PERMIT EXPIRES ONE YEAR FROM THE DATE OF ISSUANCE



MEMORANDUM

Date: 28 February 2006

To: John Colson, P.E., County Engineer

From: To Brian L. Kepner, County Planner BLK

Re: Flood Resolution 2005R-26

OK to issue permit
J. Colson 3/02/06

Please find attached the items submitted for Lot 17, Woodhaven, Unit 4 Subdivision. Please review for compliance with Flood Resolution 2005R-26. The lot is in the 100 year flood zone with a flood elevation determined to be at 104 feet. County's regulations ~~will require that~~ the elevation of the 1st floor be no lower than 105 feet and the engineer must provide a 1 foot rise certification.

Need to revise to zero rise in regulations

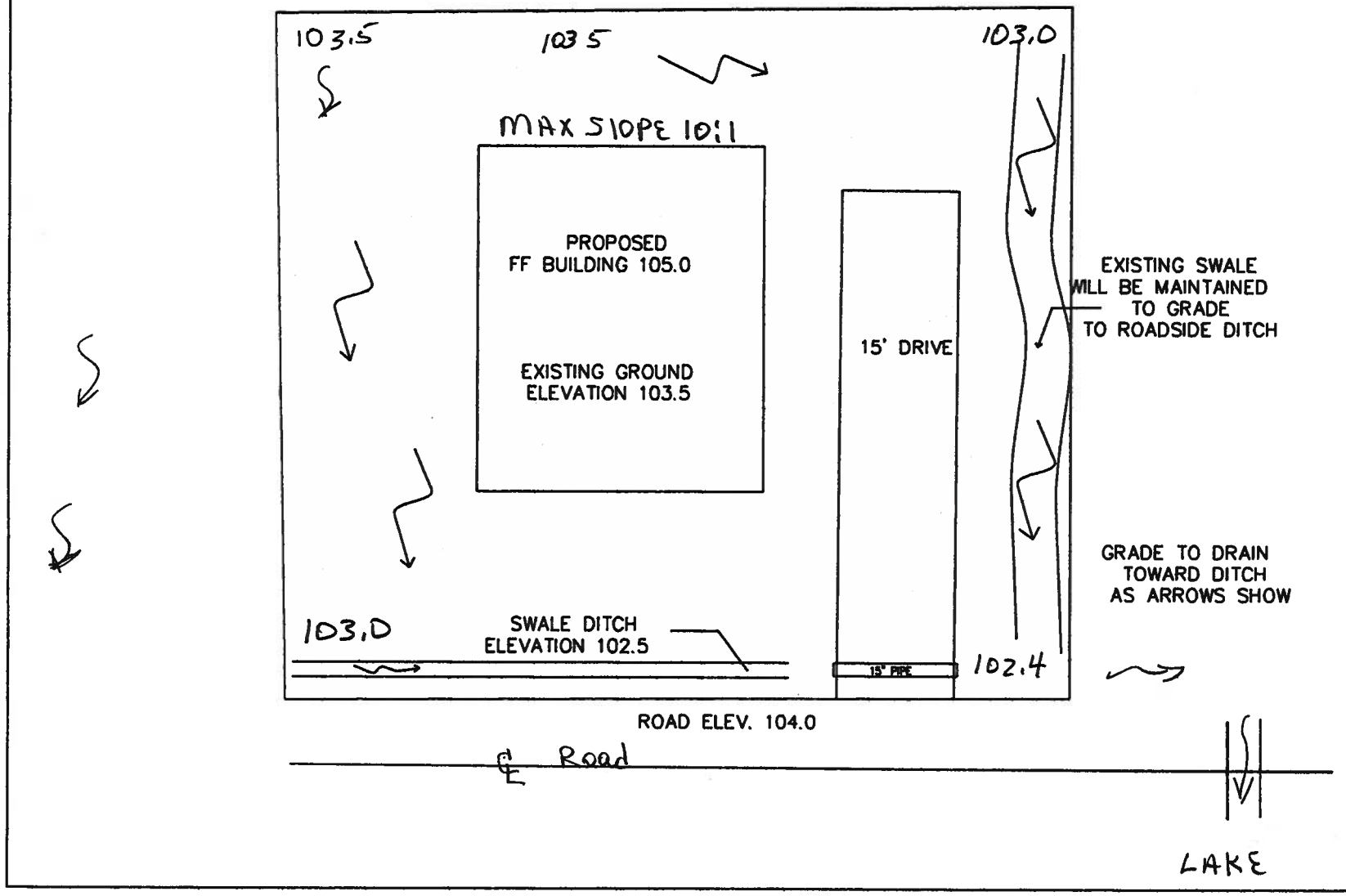
Hatcher house

This house is being built in an existing subdivision with houses on all sides. The existing ditch in the front of the lot flows to the south for one lot and crosses the road through a pipe, then to Alligator Lake. The slopes will vary between 10:1 and 30:1. The lot has an existing swale along the south property line that drains toward the front ditch. This will prevent any build up of water from the adjacent lots.

2-16-06

Ok

12.02.09



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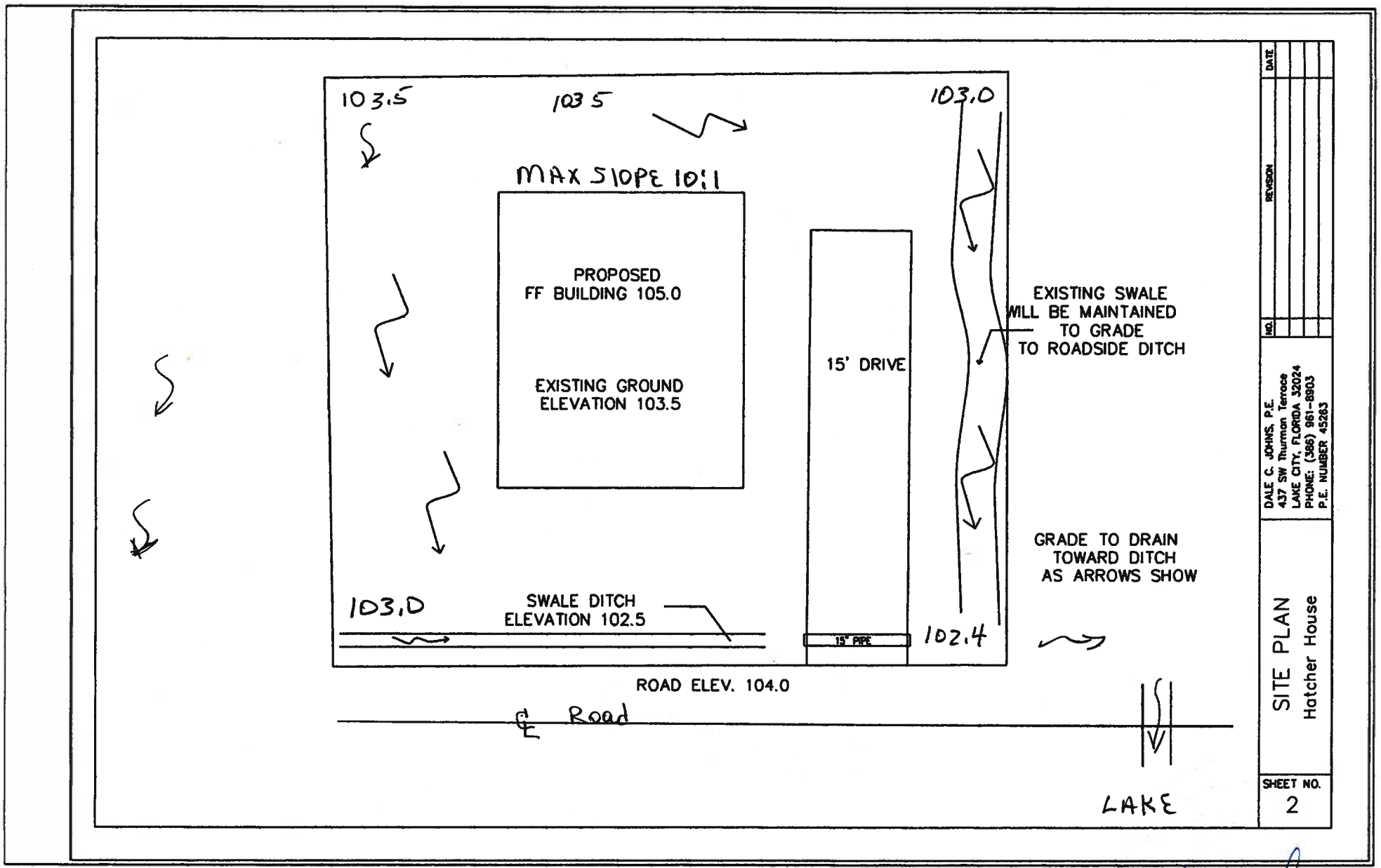
Dale Johns



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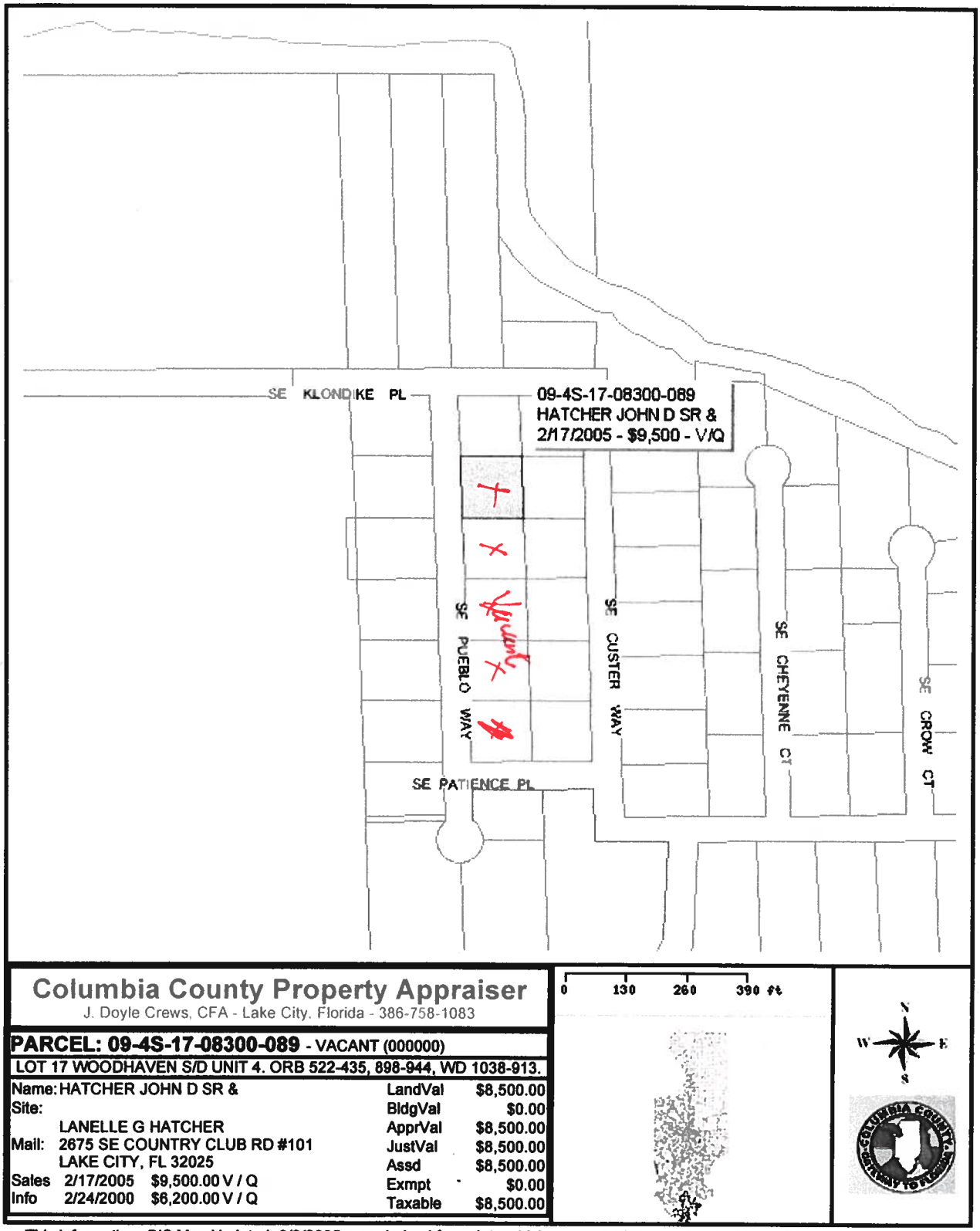


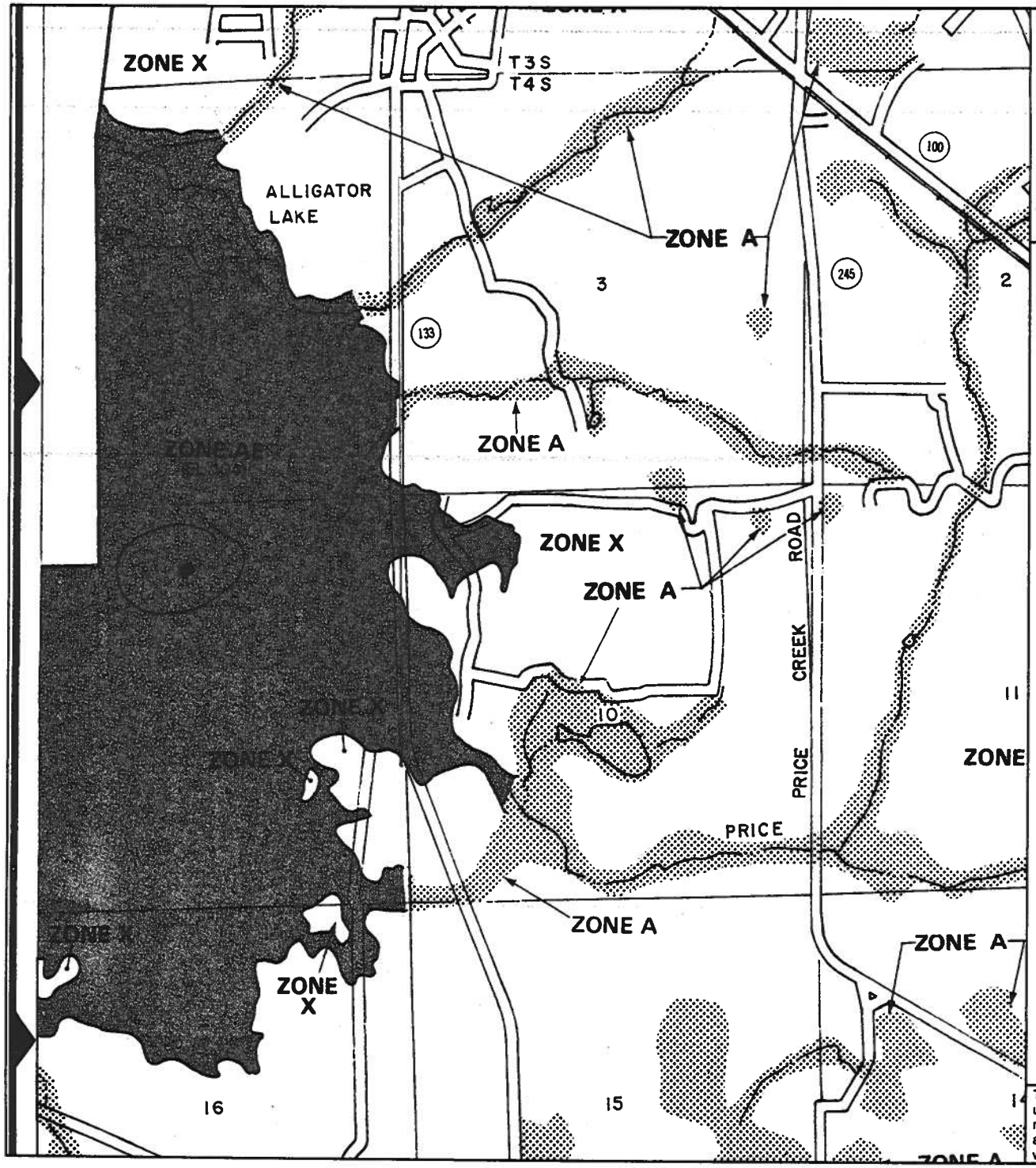
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 09-4S-17-08300-089 - VACANT (000000)

LOT 17 WOODHAVEN S/D UNIT 4. ORB 522-435, 898-944, WD 1038-913.

Name:	HATCHER JOHN D SR &	LandVal	$8,500.00
Site:		BldgVal	$0.00
	LANELLE G HATCHER	ApprVal	$8,500.00
Mail:	2675 SE COUNTRY CLUB RD #101	JustVal	$8,500.00
	LAKE CITY, FL 32025	Assd	$8,500.00
Sales	2/17/2005 $9,500.00 V / Q	Exmpt	$0.00
Info	2/24/2000 $6,200.00 V / Q	Taxable	$8,500.00



APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 200 OF 300

PANEL LOCATION

COMMUNITY-PANEL NUMBER
120070 0200 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 2/10/2006 (printed at scale and type A)

From:	The Columbia County Building Department Plans Review 135 NE Hernando Av. P. O Box 1529 Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0602-9

James Norton owner John Hatcher Lot 17 of Woodhaven Subdivision Unit IV

On the date of February 10, 2006 application 0602-09 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0602-09 when making reference to this application.

1. Application 0602-09 which was filed with the building department on the date of February 3, 2006 and will be reviewed under the Florida Building Code 2004. The Wind Load design by Mr. Mark Disosway was design under the Florida Building Code 2001. The wind Load design should reflect the code sections of the Florida Building Code 2004 that relate to wind Load design code requirements.
2. Please have Mr. Mark Disosway supply the following information, show all required connectors with uplift rating for the truss system and required number and size of fasteners for continuous tie from the roof to foundation. These

connection points shall be designed by an architect or engineer using the engineered roof truss plans.

3. Please submit a recorded (with the Columbia County Clerk Office) a notice of commencement before any inspections can be preformed by the Columbia County Building Department.
4. The shelter/ workshop structure which is shown on the plans submitted in application 0602-09 will be required to be permitted on a separate building permit application.
5. Upon review of Mr. Dale C. Johns One Foot Rise Certification Letter, which Mr. Johns states that an earth pad will be used to elevate the dwelling foundation to 104'. Please show compliance with the Columbia County Resolution No. 2005-26 which is attached.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

COLUMBIA COUNTY, FLORIDA
RESOLUTION NO. 2005R-26

A RESOLUTION OF COLUMBIA COUNTY, FLORIDA, PROVIDING FOR ADDITIONAL REQUIREMENTS FOR A DEVELOPMENT PERMIT ON PROPERTY WHICH HAS BEEN IDENTIFIED AS "FLOOD PRONE;" AND PROVIDING FOR AN EFFECTIVE DATE.

WHEREAS, since the hurricane season of 2004, Columbia County has experienced significant flooding and related issues impacting the public health, safety and welfare of the residents and citizens of Columbia County as well as their property; and

WHEREAS, the Board of County Commissioners of Columbia County, Florida, finds it is necessary and in the best interest of Columbia County and its residents and citizens for the protection of the health, safety and welfare, together with the protection of property interests in Columbia County, to provide requirements in addition to those currently set forth in local, state and federal statutes, ordinances, rules and regulations, including but not limited to the Columbia County Comprehensive Plan and Columbia County Land Development Regulations (LDRs), for the application and issuance of a development permit.

NOW, THEREFORE, BE IT RESOLVED BY THE BOARD OF COUNTY COMMISSIONERS OF COLUMBIA COUNTY, FLORIDA AS FOLLOWS:

1. Properties, including lots and acreage, which have been identified in Columbia County as "flood prone" shall, in addition to all other local, state and federal requirements, prior to issuance of a development permit through the Columbia County Building Department provide the following:

a. In addition to all other required submittals, the development permit applicant shall file a grading plan for the property proposed to be developed. The grading plan shall be signed and sealed by a Florida registered professional engineer.

b. The grading plan shall delineate proposed changes from natural ground elevation, if any, including the amount of fill material to be added to the site. The grading plan shall clearly demonstrate that the natural flow of water shall not be altered nor will adjacent properties be negatively impacted by the proposed development.

c. The grading plan shall further establish the lowest habitable floor elevation and building location on the lot or acreage.

d. Upon its completion, the applicant shall obtain from a Florida licensed land surveyor and provide to Columbia County certification as to the actual height of the finished floor established by the grading plan.

2. Additionally, all "flood prone" properties shall require written certification by a competent Florida licensed professional or agency stating that the property is not defined as a wetland as defined in the Columbia County Land Development Regulations.

3. The term "flood prone" is defined as those lots, acreage or properties that can be demonstrated on existing FEMA or other maps as flood prone properties which competent personal testimony through affidavit or otherwise establishes the property has a history of flooding which would adversely impact development upon the property.

4. There shall be exempt from the requirements of this Resolution lots, acreage or properties otherwise defined as "flood prone" where the ratio of "non-flood prone" property

(numerator) to the square footage of impervious surface development on the property (denominator) is no less than 3-to-1. However, all other permitting requirements of the County must be satisfied.

5. Any interested party who is subject to these additional permitting requirements and believes they have been inappropriately applied to them may appeal the decision to the Board of County Commissioners of Columbia County. All such appeals must be in writing and mailed to the Board of County Commissioners of Columbia County, Post Office Box 1529, Lake City, Florida 32056-1529. At this time no appeal fee is assessed.

6. This Resolution shall remain in effect until the Board of County Commissioners has approved an appropriate ordinance addressing the flood prone issues of Columbia County or until further action of the Board.

UNANIMOUSLY PASSED AND ADOPTED by the Board of County Commissioners at its regular meeting on the 16th day of June, 2005.

BOARD OF COUNTY COMMISSIONERS
COLUMBIA COUNTY, FLORIDA

By: Jennifer Flinn
Jennifer Flinn, Chairman

ATTEST: P. DeWitt Cason
P. DeWitt Cason, Clerk of Courts

(SEAL)

Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 09-4S-17-08300-089 - VACANT (000000)

LOT 17 WOODHAVEN S/D UNIT 4. ORB 522-435, 898-944, WD 1038-913.

Name:	HATCHER JOHN D SR &	LandVal	$8,500.00
Site:		BldgVal	$0.00
	LANELLE G HATCHER	ApprVal	$8,500.00
Mail:	2675 SE COUNTRY CLUB RD #101	JustVal	$8,500.00
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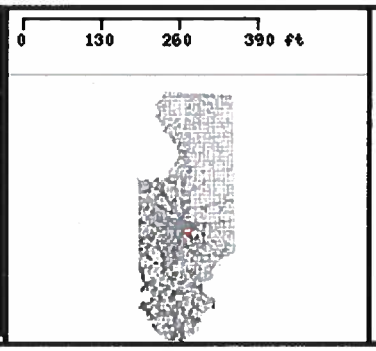

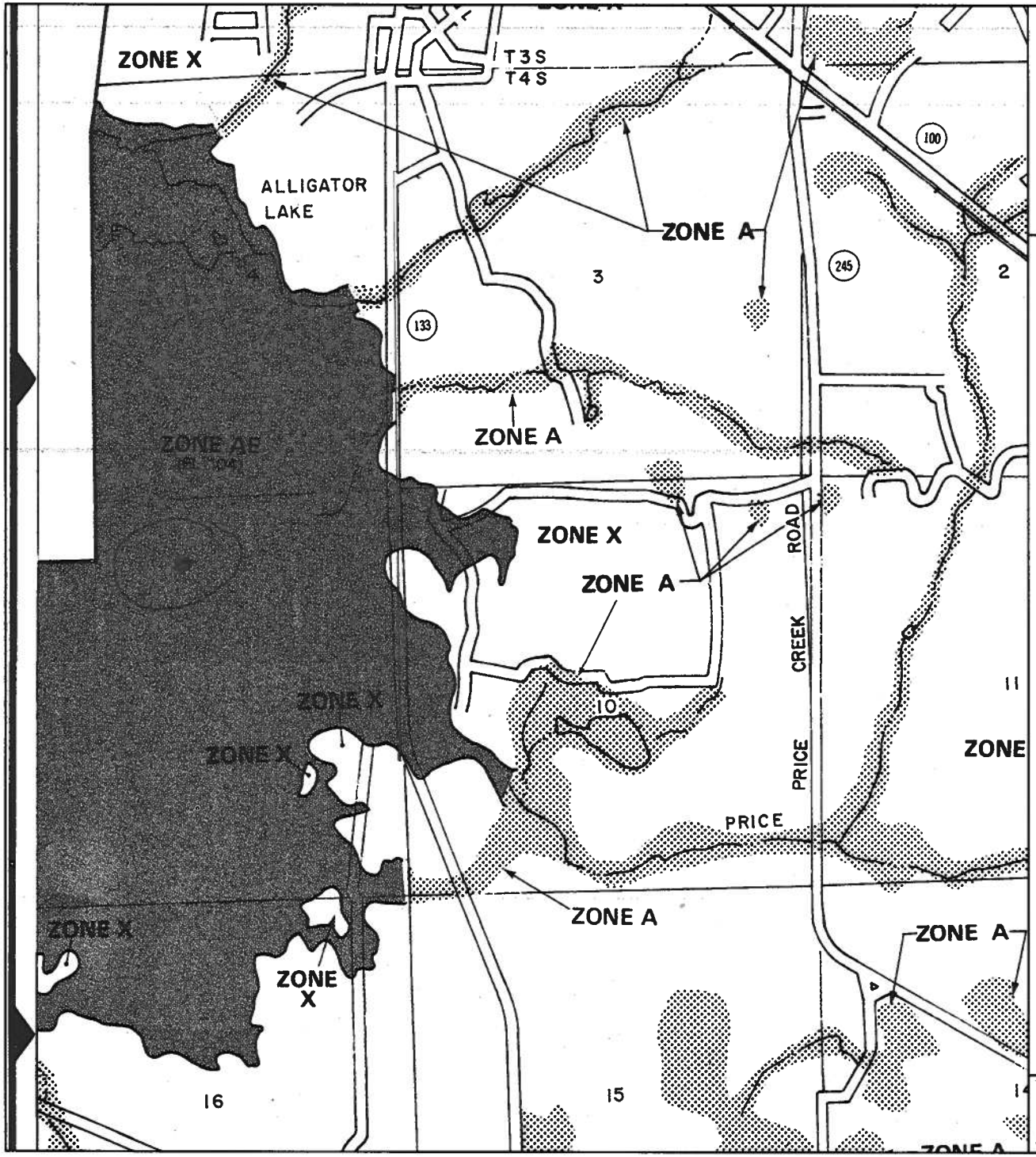

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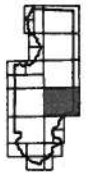
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 200 OF 300

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0200 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

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Print Date: 2/10/2006 (printed at scale and type A)

Columbia County Property Appraiser

DB Last Updated: 12/8/2005

2006 Proposed Values

Parcel: 09-4S-17-08300-089 Tax Record Property Card Interactive GIS Map Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	HATCHER JOHN D SR &
Site Address	
Mailing Address	LANELLE G HATCHER 2675 SE COUNTRY CLUB RD #101 LAKE CITY, FL 32025
Brief Legal	LOT 17 WOODHAVEN S/D UNIT 4. ORB 522-435, 898-944, WD 1038-913.

Use Desc. (code)	VACANT (000000)
Neighborhood	9417.01
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.000 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$8,500.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$8,500.00

Just Value	$8,500.00
Class Value	$0.00
Assessed Value	$8,500.00
Exempt Value	$0.00
Total Taxable Value	$8,500.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vimp	Sale Qual	Sale RCode	Sale Price
2/17/2005	1038/913	WD	V	Q		$9,500.00
2/24/2000	898/944	WD	V	Q		$6,200.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.000AC)	1.00/1.00/.85/1.00	$8,500.00	$8,500.00

Columbia County Property Appraiser DB Last Updated: 12/8/2005

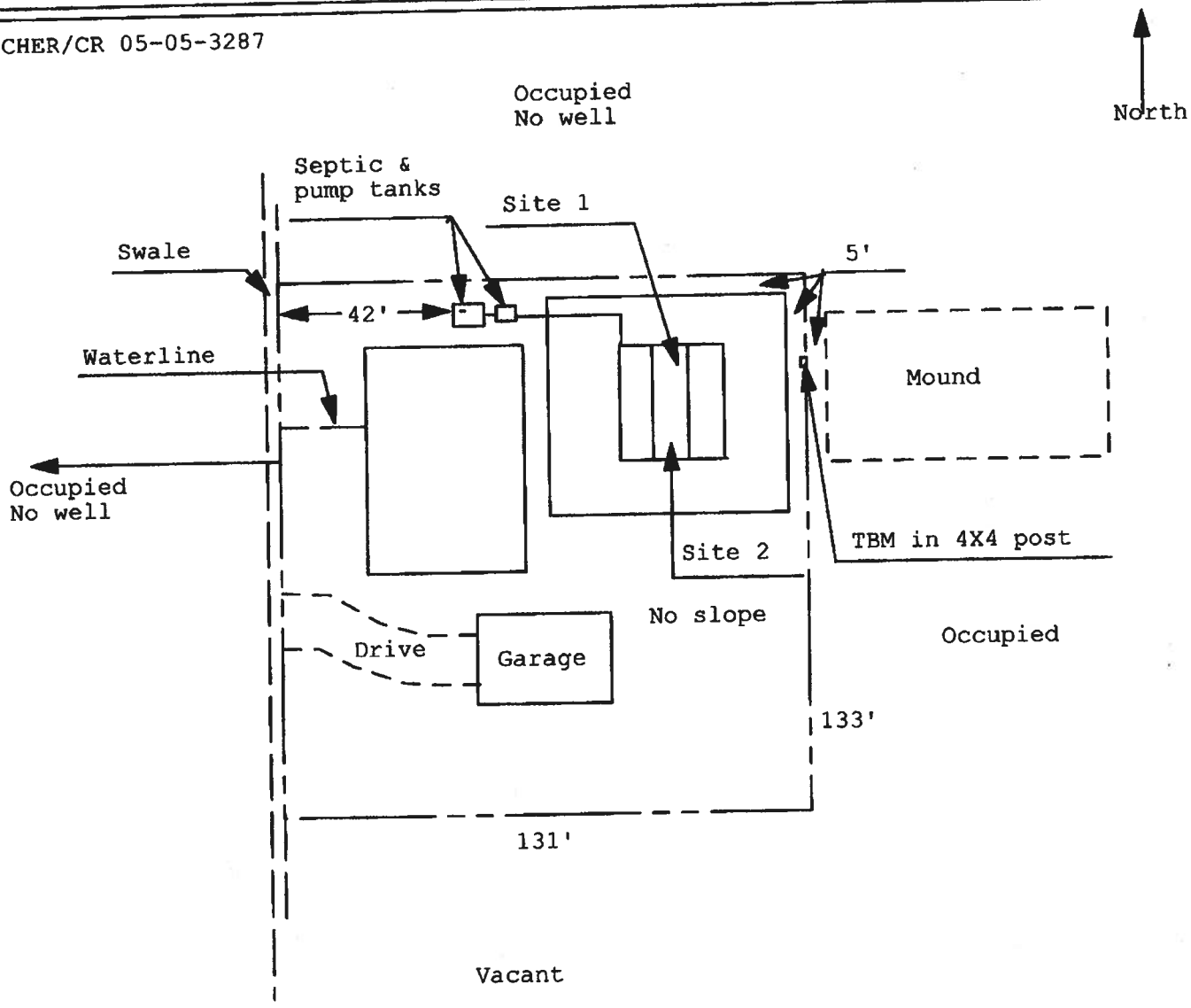
1 of 1

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-1087N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

HATCHER/CR 05-05-3287



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd Date 12/21/05

Plan Approved ✓ Not Approved ____ Date 1-3-06

By [signature] Columbia CPHU

Notes: ____

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Hatcher Residence**	Builder:	J. Norton
Address:	Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108	Permitting Office:	Columbia Co.
City, State:	Lake City, FL 32055-	Permit Number:	24213
Owner:	Lanelle Hatcher	Jurisdiction Number:	221000
Climate Zone:	North		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		2	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		1627 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	294.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 198.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1648.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1627.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Attic		Sup. R=6.0, 10.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 14.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.18

Total as-built points: 22234
Total base points: 23332

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 8/15/05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA — IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1627.0	20.04	5868.9	Double, Clear	N	2.0	8.0	10.0	19.20	0.94	180.2	
				Double, Clear	N	2.0	4.0	6.0	19.20	0.83	95.8	
				Double, Clear	N	2.0	8.0	39.0	19.20	0.94	702.9	
				Double, Clear	N	2.0	6.0	6.0	19.20	0.90	103.7	
				Double, Clear	S	2.0	6.0	21.0	35.87	0.78	584.5	
				Double, Clear	S	2.0	8.0	7.0	35.87	0.86	215.0	
				Double, Clear	E	2.0	8.0	30.0	42.06	0.91	1151.8	
				Double, Clear	E	9.0	8.0	30.0	42.06	0.49	614.1	
				Double, Clear	E	12.0	9.0	18.0	42.06	0.45	343.0	
				Double, Clear	E	12.0	8.0	24.0	42.06	0.43	437.1	
				Double, Clear	E	12.0	10.0	21.0	42.06	0.47	414.4	
				Double, Clear	W	10.0	6.0	16.0	38.52	0.43	264.6	
				Double, Clear	W	10.0	8.0	32.0	38.52	0.48	589.1	
				Double, Clear	W	2.0	8.0	34.0	38.52	0.91	1196.3	
				As-Built Total:				**294.0**			**6892.4**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1648.0	1.50	2472.0
Exterior	1648.0	1.70	2801.6					
Base Total:	**1648.0**		**2801.6**	**As-Built Total:**		**1648.0**		**2472.0**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Exterior Insulated	21.0	4.10	86.1
Exterior	21.0	6.10	128.1				
Base Total:	**21.0**		**128.1**	**As-Built Total:**	**21.0**		**86.1**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	1627.0	1.73	2814.7	Under Attic	30.0	1627.0	1.73 X 1.00	2814.7
Base Total:	**1627.0**		**2814.7**	**As-Built Total:**		**1627.0**		**2814.7**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	198.0(p)	-37.0	-7326.0	Slab-On-Grade Edge Insulation	0.0	198.0(p	-41.20	-8157.6
Raised	0.0	0.00	0.0					
Base Total:			**-7326.0**	**As-Built Total:**		**198.0**		**-8157.6**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BSPM	= Points				Area X	SPM	=	Points
	1627.0	10.21	16611.7				1627.0	10.21		16611.7
Summer Base Points:			**20899.0**	**Summer As-Built Points:**						**20719.3**
Total Summer Points	X System Multiplier		= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
20899.0	**0.4266**		**8915.5**	20719.3 **20719.3**	1.000 **1.00**	(1.090 x 1.147 x 1.11) **1.388**	0.244 **0.244**	0.902 **0.902**		6326.2 **6326.2**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1627.0	12.74	3731.0	Double, Clear	N	2.0	8.0	10.0	24.58	1.00	246.4	
				Double, Clear	N	2.0	4.0	6.0	24.58	1.01	148.8	
				Double, Clear	N	2.0	8.0	39.0	24.58	1.00	960.8	
				Double, Clear	N	2.0	6.0	6.0	24.58	1.00	148.2	
				Double, Clear	S	2.0	6.0	21.0	13.30	1.26	351.4	
				Double, Clear	S	2.0	8.0	7.0	13.30	1.12	104.0	
				Double, Clear	E	2.0	8.0	30.0	18.79	1.04	583.8	
				Double, Clear	E	9.0	8.0	30.0	18.79	1.32	744.1	
				Double, Clear	E	12.0	9.0	18.0	18.79	1.36	460.7	
				Double, Clear	E	12.0	8.0	24.0	18.79	1.39	626.9	
				Double, Clear	E	12.0	10.0	21.0	18.79	1.34	528.6	
				Double, Clear	W	10.0	6.0	16.0	20.73	1.21	402.4	
				Double, Clear	W	10.0	8.0	32.0	20.73	1.19	789.8	
				Double, Clear	W	2.0	8.0	34.0	20.73	1.02	721.3	
				As-Built Total:				**294.0**			**6817.1**	

WALL TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1648.0	3.40	5603.2
Exterior	1648.0	3.70	6097.6					
Base Total:	**1648.0**		**6097.6**	**As-Built Total:**		**1648.0**		**5603.2**

DOOR TYPES	Area X	BWPM =	Points	Type	Area X	WPM =	Points
Adjacent	0.0	0.00	0.0	Exterior Insulated	21.0	8.40	176.4
Exterior	21.0	12.30	258.3				
Base Total:	**21.0**		**258.3**	**As-Built Total:**	**21.0**		**176.4**

CEILING TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM X WCM =	Points
Under Attic	1627.0	2.05	3335.3	Under Attic	30.0	1627.0	2.05 X 1.00	3335.3
Base Total:	**1627.0**		**3335.3**	**As-Built Total:**		**1627.0**		**3335.3**

FLOOR TYPES	Area X	BWPM =	Points	Type	R-Value	Area X	WPM =	Points
Slab	198.0(p)	8.9	1762.2	Slab-On-Grade Edge Insulation	0.0	198.0(p	18.80	3722.4
Raised	0.0	0.00	0.0					
Base Total:			**1762.2**	**As-Built Total:**		**198.0**		**3722.4**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points				Area X	WPM	=	Points
	1627.0	-0.59	-959.9				1627.0	-0.59		-959.9
Winter Base Points:			**14224.6**	**Winter As-Built Points:**						**18694.5**
Total Winter Points	X System Multiplier	=	Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
14224.6	**0.6274**		**8924.5**	18694.5 **18694.5**	1.000 **1.00**	(1.069 x 1.169 x 1.10) **1.375**	0.432 **0.432**	0.950 **0.950**		10537.8 **10537.8**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
2		2746.00	5492.0	30.0	0.90	2		1.00	2684.98	1.00	5370.0
				As-Built Total:							**5370.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
8916	**8924**	**5492**	**23332**	**6326**	**10538**	**5370**	**22234**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 17, Sub: Woodhaven Ph4, Plat: 4, Pgg. 108, Lake City, FL, 32055** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ONE FOOT RISE CERTIFICATION

PROPERTY DESCRIPTION: Woodhaven Unit 4 Lot 17

OWNER: Lanelle Hatcher

BASE FLOOD ELEVATION: 104.0

COMMUNITY-PANEL NUMBER: 120070 0200 B

PROJECT: Min. Finished Floor 105.0
2000 sf house and garage

I hereby certify that construction of the proposed will cause less than one foot increase in flood elevations of the Alligator Lake floodplain.

Dale C. Johns, P.E # 45263
Date: 2/01/06
437 SW Thurman Terrace
Lake City, Fl 32024
Ph. 386-961-8903

BASE FLOOD ELEVATION = 104.0

BASIN AREA AT 104' BASE FLOOD = 1900 ACRES

PROPOSED BUILDING TYPE = HOUSE

PROPOSED BUILDING ENCROACHMENT = EARTH PAD TO ELEVATION 104

2000 SF BUILDING AND PORCH

GROUND ELEVATION AT BUILDING = 103.5 AVE.

The project only requires volume calculations in this area since in is not in a flowing or riverine area.

PERCENT FLOODPLAIN AREA REMOVED = $\frac{2000}{\frac{43560}{1900}}$ = 0.002%

FLOODPLAIN LEVEL INCREASE= $\frac{2000 \text{ X } 0.5}{1900 \text{ X } 43560}$ = 0.000012 FT.

U.S. DEPARTMENT OF HOMELAND SECURITY
Federal Emergency Management Agency
National Flood Insurance Program

ELEVATION CERTIFICATE

Important: Read the instructions on pages 1-8.

OMB No. 1660-0008
Expires February 28, 2009

SECTION A - PROPERTY INFORMATION

	For Insurance Company Use:
A1. Building Owner's Name John Lanelle Hatcher	Policy Number
A2. Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 247 SE Pueblo Way City Lake City State FL ZIP Code 32025	Company NAIC Number

A3. Property Description (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.)
Lot 17 Unit 4 Woodhaven

A4. Building Use (e.g., Residential, Non-Residential, Addition, Accessory, etc.) Residential

A5. Latitude/Longitude: Lat. _____ Long. _____ Horizontal Datum: ☐ NAD 1927 ☐ NAD 1983

A6. Attach at least 2 photographs of the building if the Certificate is being used to obtain flood insurance.

A7. Building Diagram Number 1

A8. For a building with a crawl space or enclosure(s), provide
- a) Square footage of crawl space or enclosure(s) _____ sq ft
- b) No. of permanent flood openings in the crawl space or enclosure(s) walls within 1.0 foot above adjacent grade _____
- c) Total net area of flood openings in A8.b _____ sq in

A9. For a building with an attached garage, provide:
- a) Square footage of attached garage _____ sq ft
- b) No. of permanent flood openings in the attached garage walls within 1.0 foot above adjacent grade _____
- c) Total net area of flood openings in A9.b _____ sq in

SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION

B1. NFIP Community Name & Community Number	B2. County Name	B3. State
Columbia 120070	Columbia	FL

B4. Map/Panel Number	B5. Suffix	B6. FIRM Index Date	B7. FIRM Panel Effective/Revised Date	B8. Flood Zone(s)	B9. Base Flood Elevation(s) (Zone AO, use base flood depth)
120070 0200	B	6 Jan 1988	6 Jan 1988	AE	104.00

B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in Item B9.
☐ FIS Profile ☒ FIRM ☐ Community Determined ☐ Other (Describe) _____

B11. Indicate elevation datum used for BFE in Item B9: ☒ NGVD 1929 ☐ NAVD 1988 ☐ Other (Describe) _____

B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? ☐ Yes ☒ No
Designation Date _____ ☐ CBRS ☐ OPA

SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED)

C1. Building elevations are based on: ☐ Construction Drawings* ☐ Building Under Construction* ☒ Finished Construction
*A new Elevation Certificate will be required when construction of the building is complete.

C2. Elevations – Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO. Complete Items C2.a-g below according to the building diagram specified in Item A7.
Benchmark Utilized 16" Oak previously set Vertical Datum N/A
Conversion/Comments None

			Check the measurement used.
a)	Top of bottom floor (including basement, crawl space, or enclosure floor)	105.64	☒ feet ☐ meters (Puerto Rico only)
b)	Top of the next higher floor	N.A	☐ feet ☐ meters (Puerto Rico only)
c)	Bottom of the lowest horizontal structural member (V Zones only)	N.A	☐ feet ☐ meters (Puerto Rico only)
d)	Attached garage (top of slab)	N.A	☐ feet ☐ meters (Puerto Rico only)
e)	Lowest elevation of machinery or equipment servicing the building (Describe type of equipment in Comments)	N.A	☐ feet ☐ meters (Puerto Rico only)
f)	Lowest adjacent (finished) grade (LAG)	103.8	☒ feet ☐ meters (Puerto Rico only)
g)	Highest adjacent (finished) grade (HAG)	104.4	☒ feet ☐ meters (Puerto Rico only)

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION

This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information. *I certify that the information on this Certificate represents my best efforts to interpret the data available. I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001.*

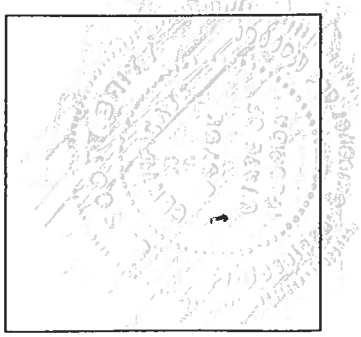
☐ Check here if comments are provided on back of form.

Certifier's Name L. Scott Britt License Number PLS #5757

Title Chief Surveyor Company Name Britt Surveying

Address 830 W Duval St. City Lake City State FL ZIP Code 32055

Signature [signature] Date 06/28/06 Telephone 386-752-7163



IMPORTANT: In these spaces, copy the corresponding information from Section A.	For Insurance Company Use:
Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No. 247 SE Pueblo Way	Policy Number
City Lake City State FL ZIP Code 32025	Company NAIC Number

SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION (CONTINUED)

Copy both sides of this Elevation Certificate for (1) community official, (2) insurance agent/company, and (3) building owner.

Comments L-17534

Signature Date

☐ Check here if attachments

SECTION E - BUILDING ELEVATION INFORMATION (SURVEY NOT REQUIRED) FOR ZONE AO AND ZONE A (WITHOUT BFE)

For Zones AO and A (without BFE), complete Items E1-E5. If the Certificate is intended to support a LOMA or LOMR-F request, complete Sections A, B, and C. For Items E1-E4, use natural grade, if available. Check the measurement used. In Puerto Rico only, enter meters.

E1. Provide elevation information for the following and check the appropriate boxes to show whether the elevation is above or below the highest adjacent grade (HAG) and the lowest adjacent grade (LAG).
a) Top of bottom floor (including basement, crawl space, or enclosure) is _____.____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.
b) Top of bottom floor (including basement, crawl space, or enclosure) is _____.____ ☐ feet ☐ meters ☐ above or ☐ below the LAG.

E2. For Building Diagrams 6-8 with permanent flood openings provided in Section A Items 8 and/or 9 (see page 8 of Instructions), the next higher floor (elevation C2.b in the diagrams) of the building is _____.____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E3. Attached garage (top of slab) is _____.____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E4. Top of platform of machinery and/or equipment servicing the building is _____.____ ☐ feet ☐ meters ☐ above or ☐ below the HAG.

E5. Zone AO only: If no flood depth number is available, is the top of the bottom floor elevated in accordance with the community's floodplain management ordinance? ☐ Yes ☐ No ☐ Unknown. The local official must certify this information in Section G.

SECTION F - PROPERTY OWNER (OR OWNER'S REPRESENTATIVE) CERTIFICATION

The property owner or owner's authorized representative who completes Sections A, B, and E for Zone A (without a FEMA-issued or community-issued BFE) or Zone AO must sign here. *The statements in Sections A, B, and E are correct to the best of my knowledge.*

Property Owner's or Owner's Authorized Representative's Name

Address City State ZIP Code

Signature Date Telephone

Comments

☐ Check here if attachments

SECTION G - COMMUNITY INFORMATION (OPTIONAL)

The local official who is authorized by law or ordinance to administer the community's floodplain management ordinance can complete Sections A, B, C (or E), and G of this Elevation Certificate. Complete the applicable item(s) and sign below. Check the measurement used in Items G8. and G9.

G1. ☐ The information in Section C was taken from other documentation that has been signed and sealed by a licensed surveyor, engineer, or architect who is authorized by law to certify elevation information. (Indicate the source and date of the elevation data in the Comments area below.)

G2. ☐ A community official completed Section E for a building located in Zone A (without a FEMA-issued or community-issued BFE) or Zone AO.

G3. ☐ The following information (Items G4.-G9.) is provided for community floodplain management purposes.

G4. Permit Number	G5. Date Permit Issued	G6. Date Certificate Of Compliance/Occupancy Issued

G7. This permit has been issued for: ☐ New Construction ☐ Substantial Improvement

G8. Elevation of as-built lowest floor (including basement) of the building: _____.____ ☐ feet ☐ meters (PR) Datum _____

G9. BFE or (in Zone AO) depth of flooding at the building site: _____.____ ☐ feet ☐ meters (PR) Datum _____

Local Official's Name Title

Community Name Telephone

Signature Date

Comments

☐ Check here if attachments

The following eight diagrams illustrate various types of buildings. Compare the features of the building being certified with the features shown in the diagrams and select the diagram most applicable. Enter the diagram number in Item C2 and the elevations in Items C3a-C3g.

In A zones, the floor elevation is taken at the top finished surface of the floor indicated; in V zones, the floor elevation is taken at the bottom of the lowest horizontal structural member (see drawing in instructions for Section C).

DIAGRAM 1

All slab-on-grade single- and multiple-floor buildings (other than split-level) and high-rise buildings, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor is at or above ground level (grade) on at least one side.*

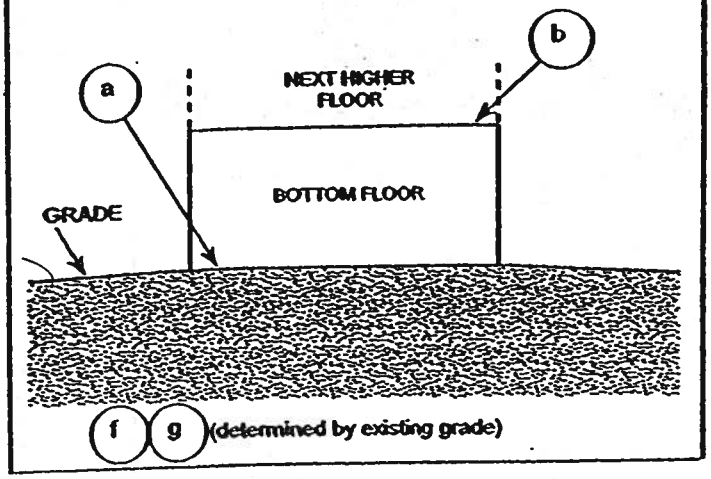


DIAGRAM 2

All single- and multiple-floor buildings with basement (other than split-level) and high-rise buildings with basement, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (basement or underground garage) is below ground level (grade) on all sides. Buildings constructed above crawl spaces that are below grade on all sides should also use this diagram.*

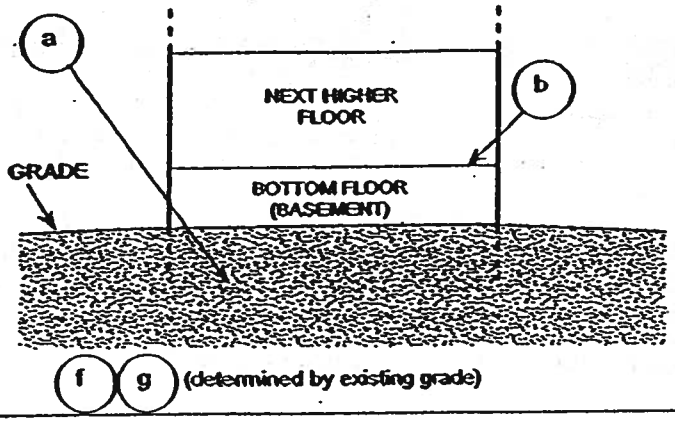


DIAGRAM 3

All split-level buildings that are slab-on-grade, either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (excluding garage) is at or above ground level (grade) on at least one side.*

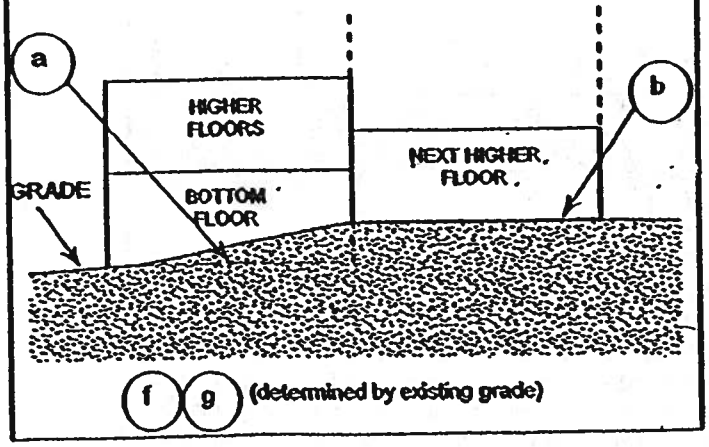
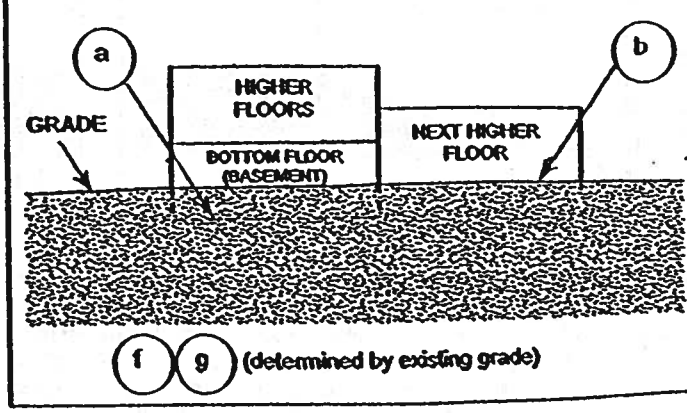


DIAGRAM 4

All split-level buildings (other than slab-on-grade), either detached or row type (e.g., townhouses); with or without attached garage.

Distinguishing Feature – The bottom floor (basement or underground garage) is below ground level (grade) on all sides. Buildings constructed above crawl spaces that are below grade on all sides should also use this diagram.*



* A floor that is below ground level (grade) on all sides is considered a basement even if the floor is used for living purposes, or as an office, garage, workshop, etc.

Building Photographs

See Instructions for Item A6.

Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No.	For Insurance Company Use: Policy Number
City State ZIP Code	Company NAIC Number

If using the Elevation Certificate to obtain NFIP flood insurance, affix at least two building photographs below according to the instructions for Item A6. Identify all photographs with: date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View." If submitting more photographs than will fit on this page, use the Continuation Page, following.

Building Photographs

Continuation Page

Building Street Address (including Apt., Unit, Suite, and/or Bldg. No.) or P.O. Route and Box No.	For Insurance Company Use: Policy Number
City State ZIP Code	Company NAIC Number

If submitting more photographs than will fit on the preceding page, affix the additional photographs below. Identify all photographs with: date taken; "Front View" and "Rear View"; and, if required, "Right Side View" and "Left Side View."

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

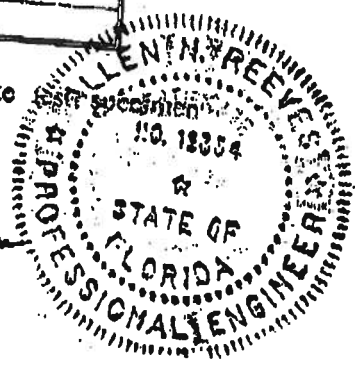
MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 June 2002



Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

ALLEN M. REEVES
CERTIFICATE
NO. 19256
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen M. Reeves
7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

ALLEN N. REEVES
CERTIFICATE
NO. 19364
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 JUNE 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Slopod sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

*	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 June 2002

ALLEN N. REEVES
CERTIFICATE NO. 43254
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97)		
	(Measurements reported were taken on the mullion)		
	(Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2 .6.2	Deglazing Test (ASTM E 987-88)		
	In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE NO. 12354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess
Technician

MAH:nlb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002

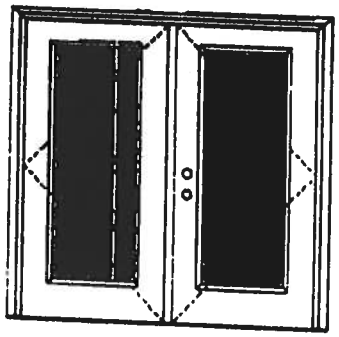
ALLEN N. REEVES
CERTIFICATE NO. 19364
STATE OF FLORIDA
PROFESSIONAL ENGINEER

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size - 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


130, 135 Series


136 Series


600 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and produc... subject to change without notice.

PREMDOR Collection Premium Quality Doors

Masonite Masonite International Corporation

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:


404 Series


410 Series


480 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


148 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson Entry Systems

March 29, 2002

PREMDOOR Collection Premium Quality Doors

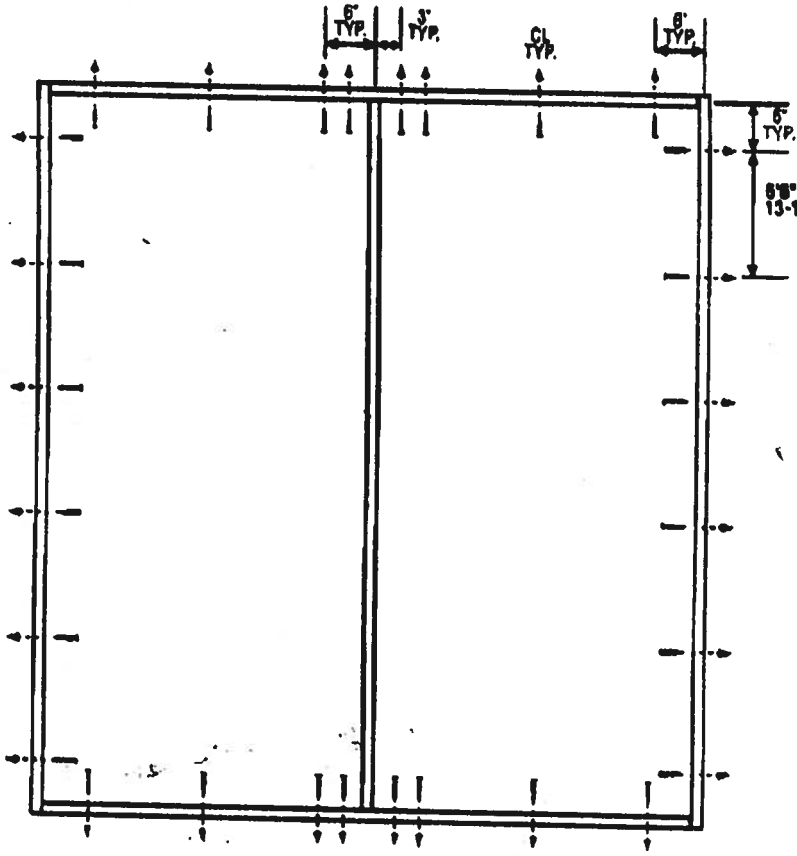
Exclusively from Masonite

Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and penetrement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dado Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

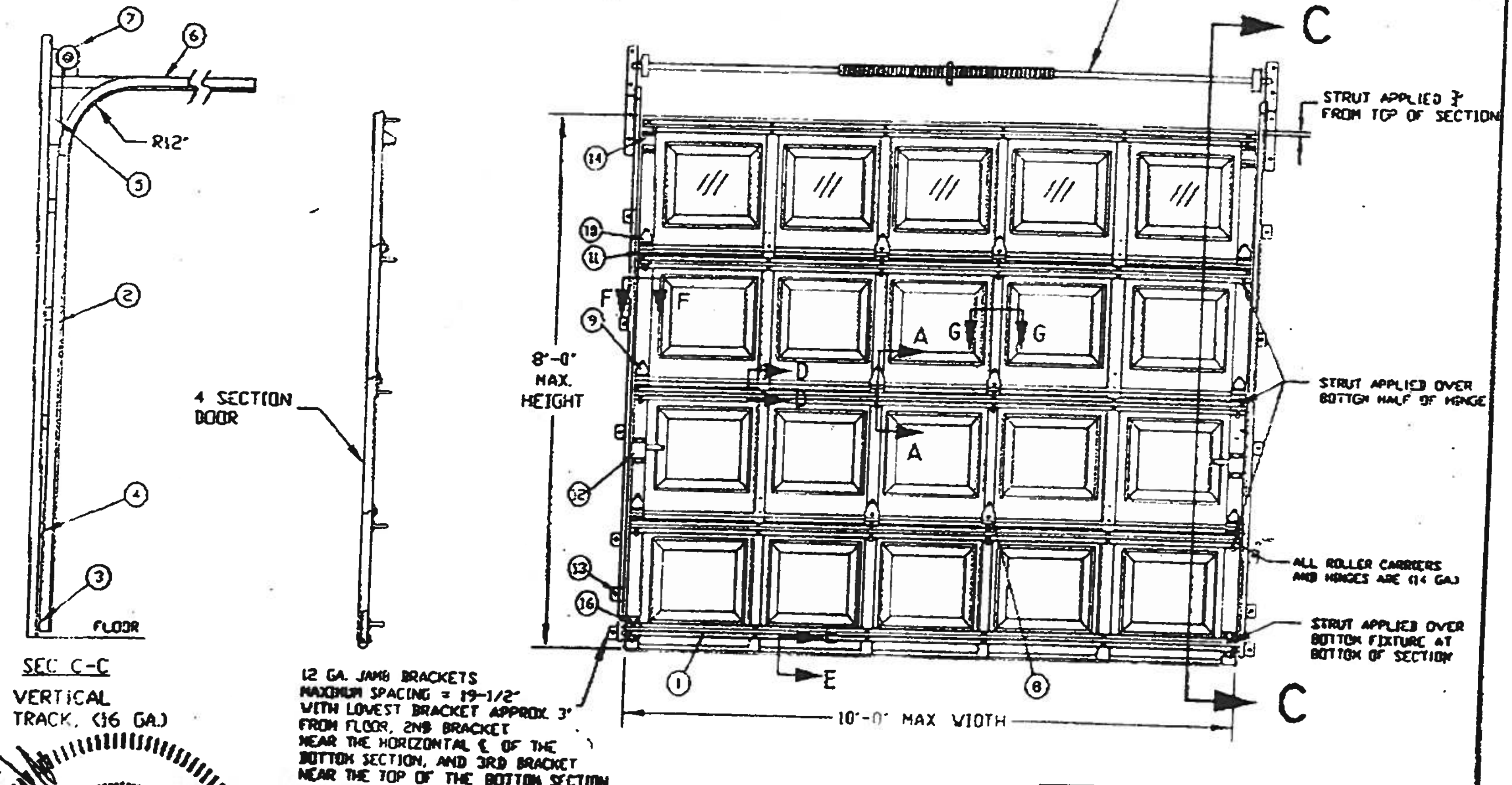
Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

2300 35th STREET P.O. BOX 2149 TUSCALOOSA

NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 32 PSF DESIGN AND POSITIVE AND NEGATIVE 48 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT= 24"
3. SECTION HEIGHTS OF 24.00", 22.50, 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.
4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION (AS TESTED) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STEM IS 3(...) AS TESTED
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOORS SHOWN
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOCKS CAN BE 0,1 OR 2 AS TESTED.
9. DROP IN TYPE INSULATION IS OPTIONAL



LISTED
SBCCI
REPORT No. 2202

SEAL
NORTH CAROLINA PROFESSIONAL
PE No. 024280
ENGINEER

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

DESIGN LOAD +32.0 PSF & -32.0 PSF
TEST LOAD +48.0 PSF & -48.0 PSF

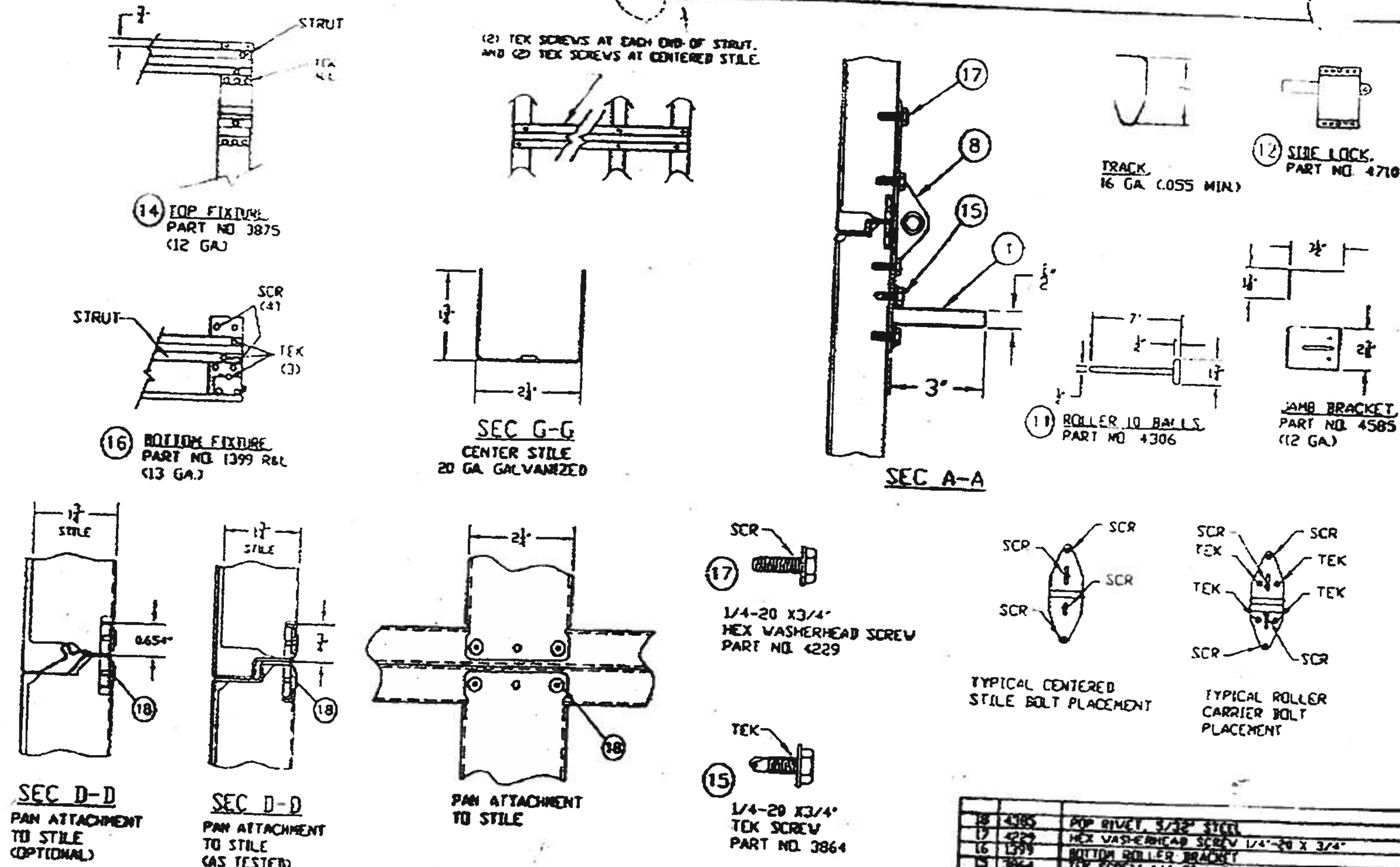
TEST REPORTS ON FILE: VIDEO 01/20/00 (C0028)

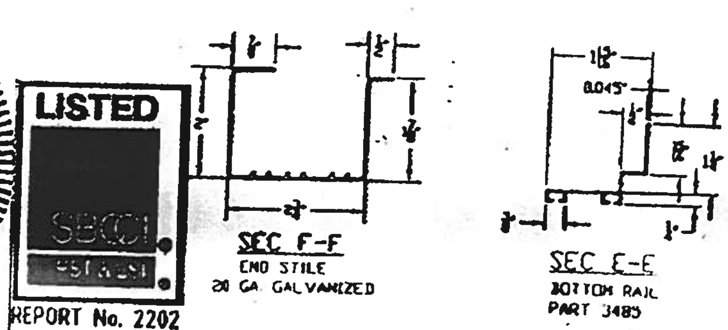
GADCO DOOR
SERIES 7025, EXTERIOR, STEEL .019" MIN (TESTED) WITH WINDOWS
SERIES 7324, EXTERIOR, STEEL .024" MIN

MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	MAXIMUM STILE SPACING	STRUTS SIZE	STRUTS QTY.	VERTICAL TRACK/TRACK
10'-0"	8'-0"	24"	3"	5	2 IN

GADCO GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE: 1:30
DATE: 2-1-00
APPROVED BY:
DRAWN BY: B VICZMAN
REVISED:
DESCRIPTION: 10' X 8' MAX RAISED PANEL STEEL DOOR-WIND:DAB +32 PSF
PART NUMBER:
PAGE 1 OF 2
DRAWING NUMBER: V12132-1





ITEM	PART NO	DESCRIPTION
18	4385	POP RIVET, 5/32" STEEL
17	4229	HEX WASHERHEAD SCREW 1/4"-20 X 3/4"
16	1399	BOTTOM ROLLER BRACKET
15	3864	TEK SCREW 1/4-20 X 3/4
14	3875	TOP SUPPORT BRACKET
13	4843	5/16" X 1-3/8" LAG SCREW
12	4710	SIDE LOCK
11	4306	LONG STEM ROLLER 2"
10	4198	ROLLER CARRIER #3 (14 GA)
9	4197	ROLLER CARRIER #2 (14 GA)
8	4196	ROLLER CARRIER #1 (14 GA)
7		COUNTER BALANCE (TORSION BAR & SPRINGS)
6		HORIZONTAL TRACK & ANGLE (16 GA)
5		VERTICAL FLAG ANGLE
4	4585-6	TRACK BRACKET #6 (12 GA)
3	4585-5	TRACK BRACKET #5 (12 GA)
2		VERTICAL TRACK (16 GA)
1		3" STRUT (20 GA)

GADCO
GENERAL AMERICAN DOOR COMPANY
3050 BASELINE ROAD
MONTGOMERY, IL 60538

SCALE NONE · DATE 2-1-00 · DRAWN BY B. VELDMAN
DESCRIPTION: 18' X 8' MAX RAISED PANEL STEEL DOOR-WINDLOAD +32 PSF
PAGE 2 OF 2 · DRAWING NUMBER V12132-2

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL PE No. 024280 NASER R. PEYVAN

The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650
TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-41641.01 for complete test specimen description and data.

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
NO. 18354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

ALLEN N. REEVES
CERTIFICATE
NO. 19256
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

7 JUNE 2002

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The meeting rail was secured to the frame utilizing two 1-1/4" screws. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stiles' screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

ALLEN N. REEVES
CERTIFICATE
NO. 18354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
7 June 2002

Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspan of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES
CERTIFICATE NO. 33[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	0.15"	0.41" max.
	@ 15.0 psf (negative)	0.29"	0.41" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.01"	0.29" max.
	@ 22.5 psf (negative)	0.01"	0.29" max.
2.2. .6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.12"/25%	0.50"/100%
	Right sash, bottom rail	0.12"/25%	0.50"/100%
	Middle sash, meeting rail	0.12"/25%	0.50"/100%
	Middle sash, bottom rail	0.12"/25%	0.50"/100%
	Left sash, meeting rail	0.12"/25%	0.50"/100%
	Left sash, bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/12%	0.50"/100%
	Right sash, left stile	0.06"/12%	0.50"/100%
	Middle sash, right stile	0.06"/12%	0.50"/100%
	Middle sash, left stile	0.06"/12%	0.50"/100%
	Left sash, right stile	0.06"/12%	0.50"/100%
	Left sash, left stile	0.06"/12%	0.50"/100%
2 .8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Allen N. Reeves
7 JUNE 2002

ALLEN N. REEVES CERTIFICATE NO. 15884 STATE OF FLORIDA PROFESSIONAL ENGINEER

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 35.3 psf (positive)	0.46"*	0.41" max
	@ 47.2 psf (negative)	0.67"*	0.41" max
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 53.0 psf (positive)	0.03"	0.29" max
	@ 52.5 psf (negative)	0.02"	0.29" max

**Exceeds L/175 for deflection, but meets all other test requirements.*

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC

Mark A. Hess
Technician

MAH:mlb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19364
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:mlb

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
No. 18554
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen M. Reeves
1 APRIL 2002

ALLEN M. REEVES
No. 12334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	2	Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen H. Reeves
1 APRIL 2002

ALLEN H. REEVES
CERTIFICATE NO. 19[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 18[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987)		
	In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.
	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 67.5 psf (positive)	0.05"	0.15" max.
	@ 70.2 psf (negative)	0.05"	0.15" max.

Exceeds L/175 for deflection, but passes all other test requirements.

Allen M. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 18334
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

MAH:nlb
01-41134.01

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

ALLEN N. REEVES
No. 1[illegible]
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Project Information for:	L136839		
Builder:	NORTON HOMES	Date:	1/20/2006
Lot:	N/A	Start Number:	**1204**
Subdivision:	247 SE PUEBLO WAY		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	15		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORTON, JAMES H. RB 0031780
Address: RT 28 BOX 388A
HIGH SPRINGS, FL. 32643 Designer: **94**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	T01	120061204	1/20/2006				
2	T07	120061205	1/20/2006				
3	T07G	120061206	1/20/2006				
4	T08	120061207	1/20/2006				
5	T08G	120061208	1/20/2006				
6	T09	120061209	1/20/2006				
7	T09G	120061210	1/20/2006				
8	T10	120061211	1/20/2006				
9	T10G	120061212	1/20/2006				
10	T02	120061213	1/20/2006				
11	T03	120061214	1/20/2006				
12	T04	120061215	1/20/2006				
13	T04G	120061216	1/20/2006				
14	T05	120061217	1/20/2006				
15	T06	120061218	1/20/2006				

JAN 20 2006

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- Change My PIN
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- Term Glossary
- Online Help

01:57:03 PM 10/6/2004

Licensee Details

Licensee Information

Name:	**NORTON, JAMES H** (Primary Name) **NORTON HOME IMPROVEMENT COMPANY INC** (DBA Name)
Main Address:	**RT 28 BOX 388A LAKE CITY, Florida 32025**
Lic. Location:	**RT 28 BOX 388A LAKE CITY, FL 32025 Columbia**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB0031780**
Status:	**Current, Active**
Licensure Date:	**02/16/1978**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	02/20/2004

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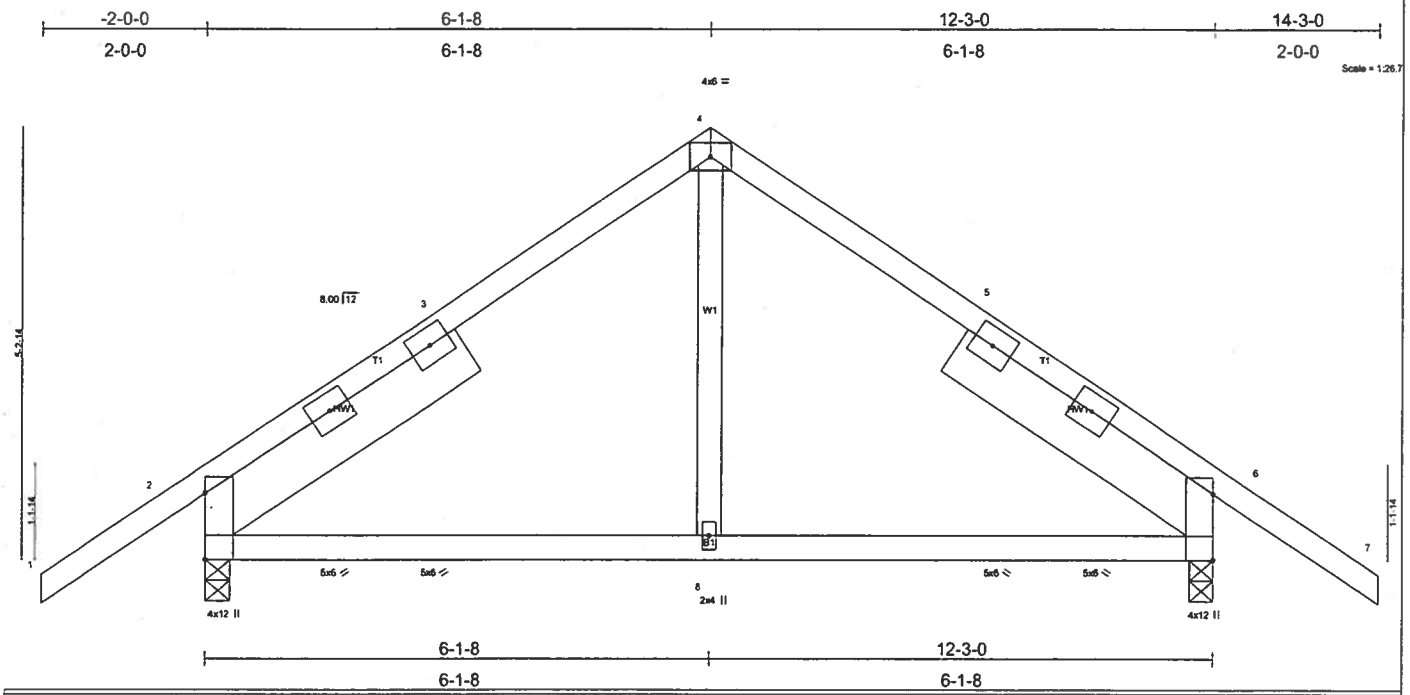
JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061204

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T01	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:41 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	0.03	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.05	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 3-10-8, Right 2 X 8 SYP No.1D 3-10-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=623/0-3-8, 6=623/0-3-8
Max Horz 2=-173(load case 3)
Max Uplift2=-278(load case 5), 6=-278(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-543/177, 3-4=-411/194, 4-5=-411/194, 5-6=-543/177, 6-7=0/26
BOT CHORD 2-8=-39/342, 6-8=-39/342
WEBS 4-8=0/194

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 278 lb uplift at joint 2 and 278 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061205

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T07	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:50 2006 Page 1

-2-0-0	5-6-0	9-11-0	14-4-0	19-10-0	21-10-0
2-0-0	5-6-0	4-5-0	4-5-0	5-6-0	2-0-0

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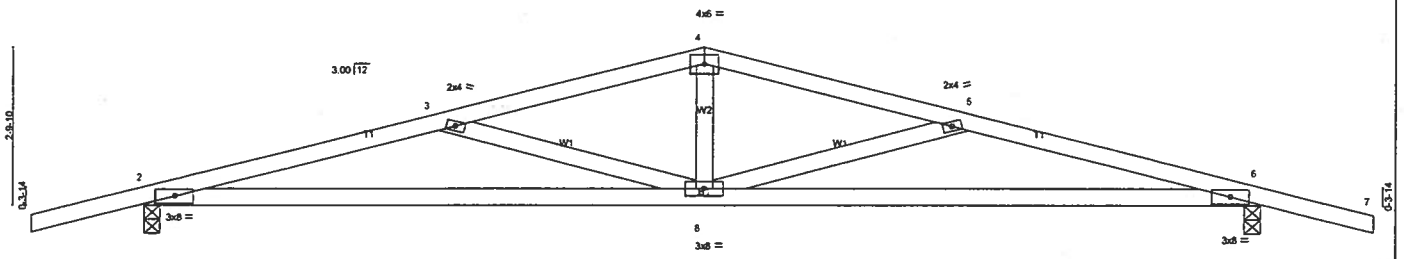

9-11-0	19-10-0
9-11-0	9-11-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	0.56	2-8	>415	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	0.46	2-8	>509	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.22	Horz(TL)	-0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 83 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-8-0 oc bracing.

REACTIONS (lb/size) 2=937/0-3-8, 6=937/0-3-8
Max Horz 2=-61(load case 4)
Max Uplift2=-644(load case 5), 6=-644(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-2261/2367, 3-4=-1777/1993, 4-5=-1777/1993, 5-6=-2261/2367, 6-7=0/25
BOT CHORD 2-8=-2220/2165, 6-8=-2219/2165
WEBS 3-8=-531/443, 4-8=-765/584, 5-8=-531/443

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 644 lb uplift at joint 2 and 644 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061206

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T07G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:51 2006 Page 1

-2-0-0 | 9-11-0 | 19-10-0 | 21-10-0

2-0-0 | 9-11-0 | 9-11-0 | 2-0-0

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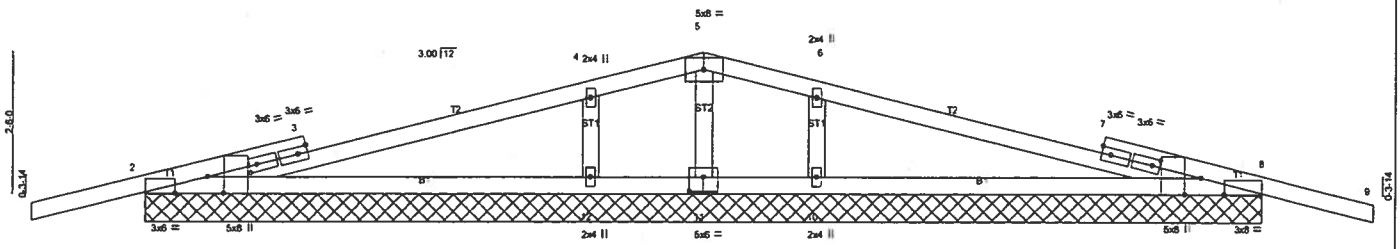

19-10-0

19-10-0

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-6-12,Edge], [3:0-1-11,0-1-8], [7:0-1-11,0-1-8], [8:0-3-8,Edge], [8:0-5-0,Edge], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.94	Vert(LL)	0.10	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.15	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.15	Horz(TL)	-0.03	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 79 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 9-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-4 oc bracing.

REACTIONS (lb/size) 2=534/19-10-0, 8=534/19-10-0, 11=-274/19-10-0, 12=1259/19-10-0, 10=1259/19-10-0
Max Horz 2=-56(load case 4)
Max Uplift2=-282(load case 3), 8=-287(load case 4), 11=-274(load case 1), 12=-456(load case 5), 10=-458(load case 6)
Max Grav 2=564(load case 9), 8=564(load case 10), 11=80(load case 6), 12=1259(load case 1), 10=1259(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8/53, 2-3=-431/934, 3-4=-451/1066, 4-5=-331/897, 5-6=-331/897, 6-7=-451/1066, 7-8=-431/934, 8-9=-8/53
BOT CHORD 2-12=-928/501, 11-12=-928/501, 10-11=-928/501, 8-10=-928/501
WEBS 5-11=-69/27, 4-12=-925/529, 6-10=-925/529

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 282 lb uplift at joint 2, 287 lb uplift at joint 8, 274 lb uplift at joint 11, 456 lb uplift at joint 12 and 458 lb uplift at joint 10.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061207

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T08	COMMON	7	1	Job Reference (optional)

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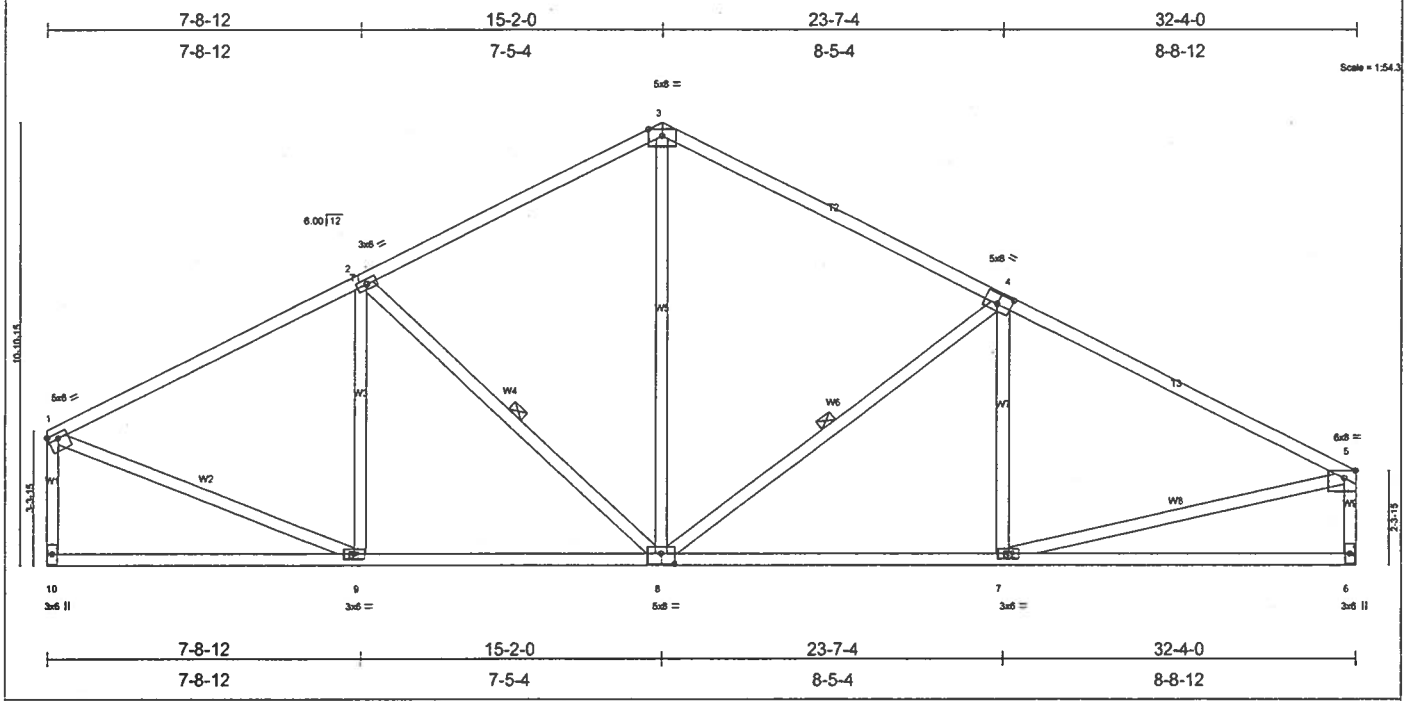

Plate Offsets (X,Y): [1:0-3-0,0-1-8], [4:0-4-0,0-3-0], [5:0-3-8,Edge], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.14	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.23	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-0 oc bracing.
WEBS 1 Row at midpt 2-8, 4-8

REACTIONS (lb/size) 10=1346/Mechanical, 6=1346/Mechanical
Max Horz 10=-151(load case 6)
Max Uplift10=-425(load case 5), 6=-433(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1416/650, 2-3=-1267/709, 3-4=-1282/702, 4-5=-1670/741, 1-10=-1230/603, 5-6=-1215/599
BOT CHORD 9-10=-130/130, 8-9=-425/1187, 7-8=-543/1402, 6-7=-139/237
WEBS 2-9=-218/222, 2-8=-281/219, 3-8=-289/630, 4-8=-499/327, 4-7=-29/160, 1-9=-441/1156, 5-7=-413/1198

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 425 lb uplift at joint 10 and 433 lb uplift at joint 6.

LOAD CASE(S) Standard

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Dwg.#0120061208

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T08G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:54 2006 Page 1

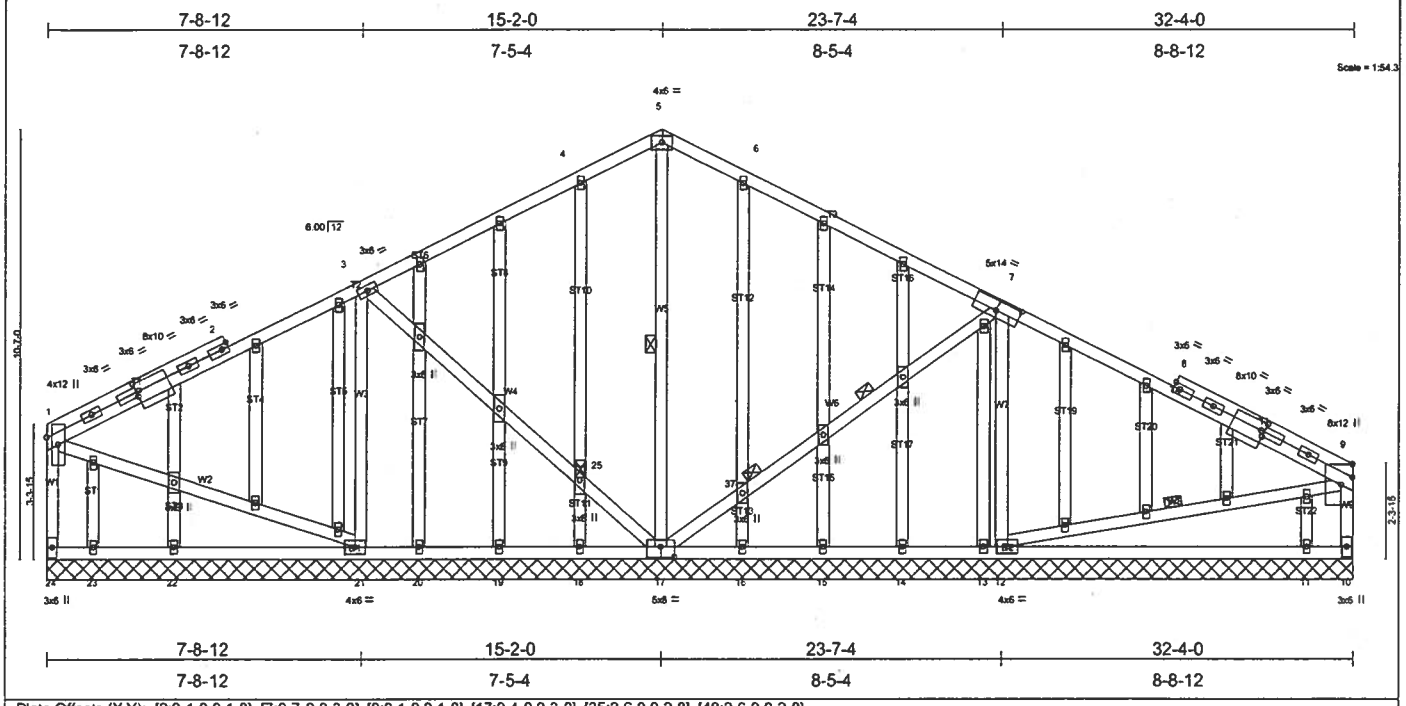

Plate Offsets (X,Y): [2:0-1-0,0-1-8], [7:0-7-0,0-3-0], [8:0-1-0,0-1-8], [17:0-4-0,0-3-0], [35:2-6-0,0-2-8], [48:2-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 327 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except* W1 2 X 4 SYP No.2, W9 2 X 4 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-17, 7-37, 9-12
JOINTS 1 Brace at Jt(s): 25, 37

REACTIONS (lb/size) 24=474/Mechanical, 21=783/Mechanical, 17=536/Mechanical, 12=1295/Mechanical, 10=409/Mechanical, 18=520/Mechanical, 19=69/Mechanical, 20=14/Mechanical, 22=135/Mechanical, 23=-131/Mechanical, 16=608/Mechanical, 15=56/Mechanical, 14=74/Mechanical, 13=-329/Mechanical, 11=100/Mechanical
Max Horz 24=-149(load case 6)
Max Uplift 24=-185(load case 6), 21=-360(load case 5), 17=-196(load case 5), 12=-357(load case 6), 10=-277(load case 6), 18=-269(load case 5), 23=-132(load case 9), 16=-323(load case 6), 13=-329(load case 10)
Max Grav 24=494(load case 9), 21=798(load case 9), 17=536(load case 1), 12=1313(load case 10), 10=422(load case 10), 18=520(load case 1), 19=69(load case 10), 20=14(load case 9), 22=136(load case 9), 23=88(load case 5), 16=608(load case 1), 15=56(load case 9), 14=74(load case 10), 11=163(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-67/56, 2-3=-94/251, 3-4=-73/347, 4-5=0/231, 5-6=0/225, 6-7=-74/369, 7-8=-95/282, 8-9=-165/63, 1-24=-337/145, 9-10=-381/175
BOT CHORD 23-24=-160/152, 22-23=-160/152, 21-22=-160/152, 20-21=-51/201, 19-20=-51/201, 18-19=-51/201, 17-18=-51/201, 16-17=-50/153, 15-16=-50/153, 14-15=-50/153, 13-14=-50/153, 12-13=-50/153, 11-12=-282/394, 10-11=-282/394
WEBS 3-21=-611/332, 3-25=-215/237, 17-25=-216/239, 5-17=-214/0, 17-37=-198/248, 7-37=-197/247, 7-12=-737/403, 1-21=-211/244, 9-12=-461/451, 4-25=-465/320, 18-25=-463/317, 6-37=-548/380, 16-37=-547/379

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 24, 360 lb uplift at joint 21, 196 lb uplift at joint 17, 357 lb uplift at joint 12, 277 lb uplift at joint 10, 269 lb uplift at joint 18, 132 lb uplift at joint 23, 323 lb uplift at joint 16 and 329 lb uplift at joint 13.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 10-24=-30

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Dwg.#0120061209

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T09	POLYNESIAN	4	1	Job Reference (optional)

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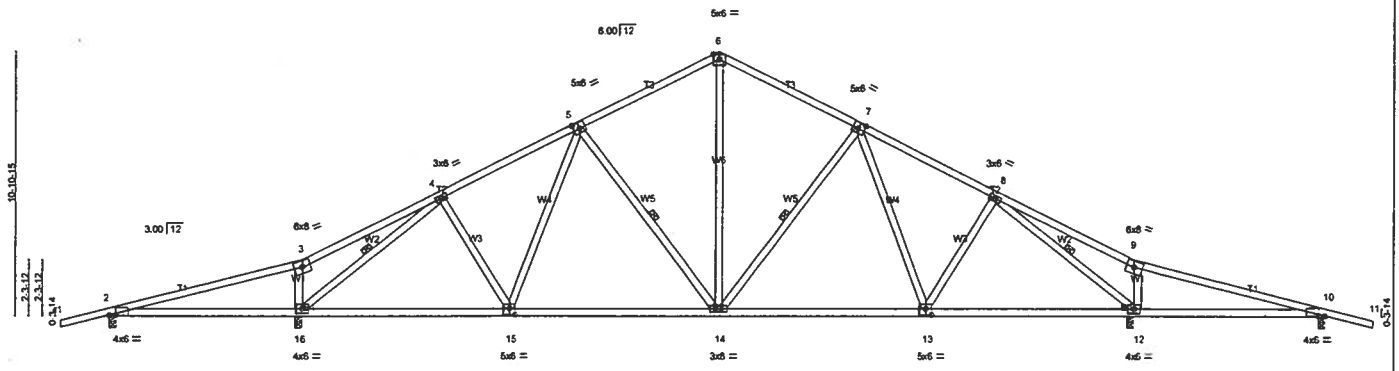
2-0-0	7-11-11	13-8-7	19-5-4	25-2-0	30-10-12	36-7-9	42-4-5	50-4-0	52-4-0
2-0-0	7-11-11	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	7-11-11	2-0-0

Scale = 1:91.1



7-9-12	7-11-11	16-6-13	25-2-0	33-9-3	42-2-4	42-4-5	50-4-0
7-9-12	0-1-15	8-7-3	8-7-3	8-7-3	8-5-1	0-2-1	7-11-11

Plate Offsets (X,Y): [2:0-3-4,0-0-2], [5:0-3-0,0-3-0], [7:0-3-0,0-3-0], [10:0-3-4,0-0-2], [13:0-3-0,0-3-0], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	0.21	2-16	>437	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	0.18	2-16	>521	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.08	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 273 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-16, 5-14, 7-14, 8-12

REACTIONS (lb/size) 2=368/0-3-8, 16=1850/0-3-8, 12=1850/0-3-8, 10=368/0-3-8
Max Horz 2=167(load case 5)
Max Uplift2=-334(load case 3), 16=-736(load case 5), 12=-719(load case 6), 10=-326(load case 4)
Max Grav 2=376(load case 9), 16=1850(load case 1), 12=1850(load case 1), 10=376(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-141/156, 3-4=-42/131, 4-5=-1648/807, 5-6=-1367/774, 6-7=-1367/774, 7-8=-1648/807, 8-9=-42/129, 9-10=-93/156, 10-11=0/25
BOT CHORD 2-16=-96/205, 15-16=-407/1328, 14-15=-355/1379, 13-14=-355/1379, 12-13=-405/1328, 10-12=-96/167
WEBS 3-16=-415/388, 4-16=-1812/707, 4-15=0/218, 5-15=-21/166, 5-14=-419/295, 6-14=-435/845, 7-14=-419/295, 7-13=-33/166, 8-13=0/218, 8-12=-1812/707, 9-12=-415/388

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 334 lb uplift at joint 2, 736 lb uplift at joint 16, 719 lb uplift at joint 12 and 326 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061210

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T09G	POLYNESIAN	1	1	Job Reference (optional)

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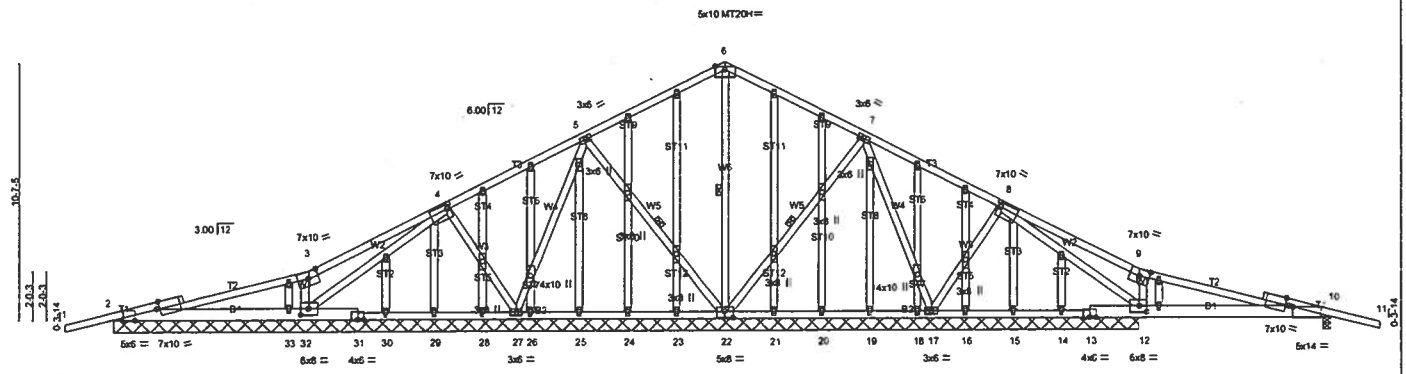
2-0-0	7-11-11	13-8-7	19-5-4	25-2-0	30-10-12	36-7-9	42-4-5	50-4-0	52-4-0
2-0-0	7-11-11	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	7-11-11	2-0-0

Scale = 1:91.1



7-11-11	16-6-13	25-2-0	33-9-3	42-4-5	50-4-0
7-11-11	8-7-3	8-7-3	8-7-3	8-7-3	7-11-11

Plate Offsets (X,Y): [2:0-1-12,Edge], [2:0-10-7,Edge], [3:0-5-0,0-3-6], [4:0-1-0,0-2-8], [8:0-1-0,0-2-8], [9:0-5-0,0-3-6], [10:0-2-0,Edge], [10:0-1-12,Edge], [12:0-3-8,0-3-0], [22:0-4-0,0-3-0], [32:0-3-8,0-3-0], [49:0-0-0,0-0-0], [51:0-0-0,0-0-0], [53:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	0.07	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.08	10-12	>999	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 412 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B2 2 X 4 SYP No.2, B2 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-33,32-33,10-12.
WEBS 1 Row at midpt 5-22, 6-22, 7-22

REACTIONS (lb/size) 2=704/42-4-0, 10=744/0-4-0, 32=527/42-4-0, 27=905/42-4-0, 22=1137/42-4-0, 17=919/42-4-0, 12=1288/42-4-0, 23=60/42-4-0, 24=59/42-4-0, 25=62/42-4-0, 26=48/42-4-0, 28=58/42-4-0, 29=54/42-4-0, 30=86/42-4-0, 33=719/42-4-0, 21=60/42-4-0, 20=59/42-4-0, 19=62/42-4-0, 18=46/42-4-0, 16=46/42-4-0, 15=90/42-4-0, 14=-33/42-4-0
Max Horz 2=-165(load case 6)
Max Uplift2=-361(load case 3), 10=-555(load case 4), 32=-449(load case 6), 27=-500(load case 5), 22=-621(load case 6), 17=-625(load case 6), 12=-827(load case 6), 33=-137(load case 6), 15=-11(load case 4), 14=-33(load case 1)
Max Grav 2=704(load case 1), 10=744(load case 1), 32=529(load case 9), 27=911(load case 9), 22=1137(load case 1), 17=925(load case 10), 12=1289(load case 10), 23=60(load case 10), 24=59(load case 9), 25=62(load case 10), 26=48(load case 1), 28=58(load case 1), 29=54(load case 10), 30=86(load case 1), 33=719(load case 1), 21=60(load case 1), 20=59(load case 9), 19=62(load case 1), 18=46(load case 9), 16=48(load case 9), 15=90(load case 1), 14=90(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-25/61, 2-3=-111/92, 3-4=-246/223, 4-5=-109/198, 5-6=-141/272, 6-7=-140/272, 7-8=-104/190, 8-9=-243/220, 9-10=-89/93, 10-11=-25/62
BOT CHORD 2-33=-3/187, 32-33=-3/187, 31-32=-77/238, 30-31=-77/237, 29-30=-77/237, 28-29=-77/237, 27-28=-77/237, 26-27=0/156, 25-26=0/156, 24-25=0/156, 23-24=0/156, 22-23=0/156, 21-22=0/153, 20-21=0/153, 19-20=0/153, 18-19=0/153, 17-18=0/153, 16-17=-34/234, 15-16=-34/234, 14-15=-34/234, 13-14=-34/234, 12-13=-37/239, 10-12=-4/137
WEBS 3-32=-853/766, 4-32=-206/132, 4-27=-502/510, 5-27=-510/369, 5-22=-273/333, 6-22=-658/365, 7-22=-269/330, 7-17=-515/373, 8-17=-505/510, 8-12=-205/130, 9-12=-843/765

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 361 lb uplift at joint 2, 555 lb uplift at joint 10, 449 lb uplift at joint 32, 500 lb uplift at joint 27, 621 lb uplift at joint 22, 625 lb uplift at joint 17, 827 lb uplift at joint 12, 137 lb uplift at joint 33, 11 lb uplift at joint 15 and 33 lb uplift at joint 14.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-114(F=-60), 3-6=-114(F=-60), 6-9=-114(F=-60), 9-11=-114(F=-60), 2-10=-30

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0120061211

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T10	POLYNESIAN	8	1	Job Reference (optional)

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2-0-0	7-11-11	13-8-7	19-5-4	25-2-0	30-10-12	36-7-9	42-4-5	50-4-0	52-4-0
2-0-0	7-11-11	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	5-8-12	7-11-11	2-0-0

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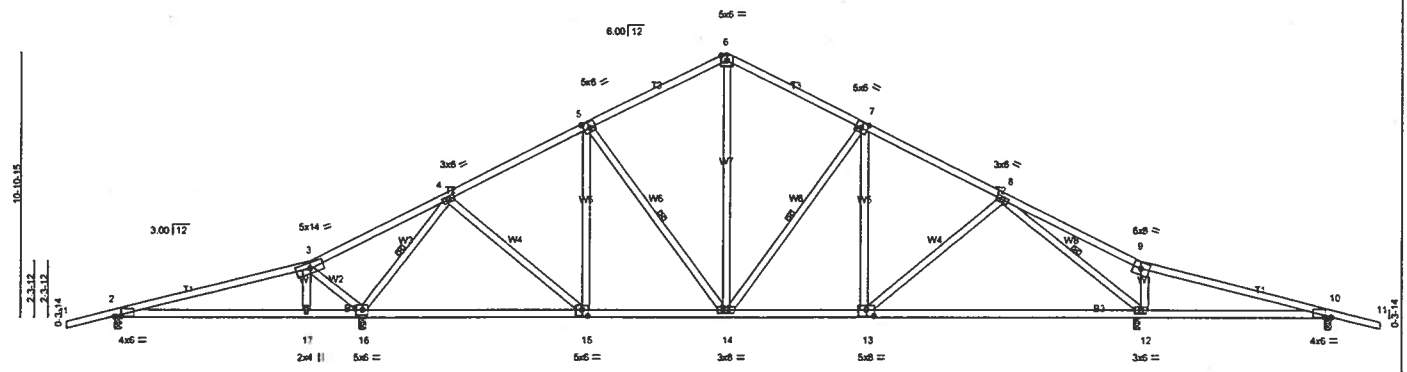

7-11-11	10-1-12	19-5-4	25-2-0	30-10-12	42-2-4	42-4-5	50-4-0
7-11-11	2-2-1	9-3-8	5-8-12	5-8-12	11-3-8	0-2-1	7-11-11

Plate Offsets (X,Y): [2:0-3-4,0-0-2], [5:0-3-0,0-3-0], [7:0-3-0,0-3-0], [10:0-3-4,0-0-2], [13:0-4-0,0-3-0], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	0.22	10-12	>414	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.54	12-13	>715	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 279 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-16, 5-14, 7-14, 8-12

REACTIONS (lb/size) 2=379/0-3-8, 16=1968/0-3-8, 12=1718/0-3-8, 10=370/0-3-8
Max Horz 2=167(load case 5)
Max Uplift2=-368(load case 3), 16=-822(load case 5), 12=-673(load case 6), 10=-332(load case 4)
Max Grav 2=398(load case 9), 16=1968(load case 1), 12=1718(load case 1), 10=373(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-25/181, 3-4=-320/597, 4-5=-1213/598, 5-6=-1111/668, 6-7=-1112/669, 7-8=-1439/697, 8-9=-79/133, 9-10=-67/102, 10-11=0/25
BOT CHORD 2-17=-134/87, 16-17=-144/100, 15-16=-128/559, 14-15=-199/1026, 13-14=-256/1222, 12-13=-356/1149, 10-12=-45/142
WEBS 3-17=-288/158, 3-16=-534/650, 4-16=-1768/912, 4-15=-165/610, 5-15=-142/160, 5-14=-248/164, 6-14=-351/629, 7-14=-532/296, 7-13=-19/258, 8-13=0/164, 8-12=-1512/610, 9-12=-414/394

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 2, 822 lb uplift at joint 16, 673 lb uplift at joint 12 and 332 lb uplift at joint 10.

LOAD CASE(S) Standard

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061212

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T01G	COMMON	1	1	Job Reference (optional)

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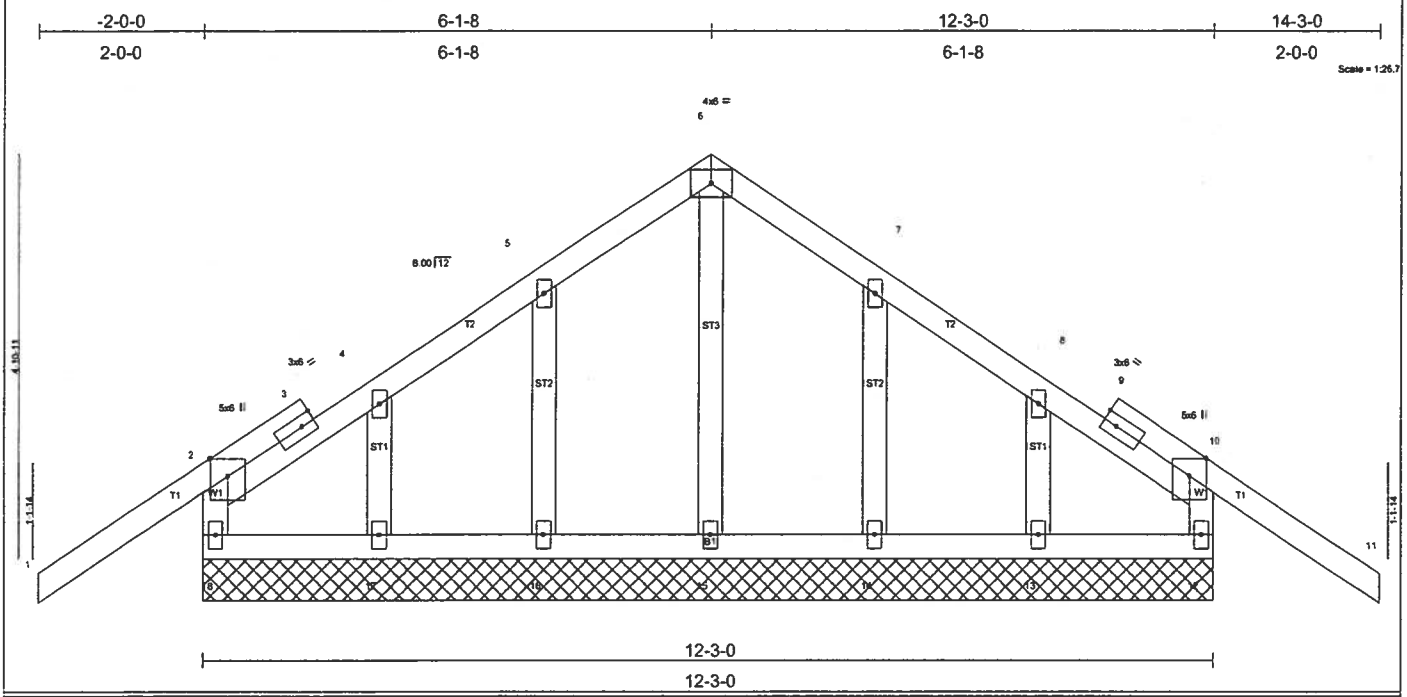

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.06	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.10	11	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 74 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 18=474/12-3-0, 12=474/12-3-0, 15=323/12-3-0, 16=312/12-3-0, 17=159/12-3-0, 14=312/12-3-0, 13=159/12-3-0
Max Horz 18=181(load case 4)
Max Uplift18=-271(load case 5), 12=-284(load case 6), 16=-153(load case 5), 17=-102(load case 4), 14=-154(load case 6), 13=-88(load case 3)
Max Grav 18=481(load case 9), 12=481(load case 10), 15=323(load case 1), 16=316(load case 9), 17=159(load case 1), 14=316(load case 10), 13=159(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-17/136, 2-3=-69/99, 3-4=-77/107, 4-5=-39/116, 5-6=-34/181, 6-7=-34/181, 7-8=-19/116, 8-9=-46/78, 9-10=-47/73, 10-11=-17/136, 2-18=-450/339, 10-12=-450/339
BOT CHORD 17-18=-44/134, 16-17=-44/134, 15-16=-44/134, 14-15=-44/134, 13-14=-44/134, 12-13=-44/134
WEBS 6-15=-263/10, 5-16=-255/172, 4-17=-99/84, 7-14=-255/171, 8-13=-99/77

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 271 lb uplift at joint 18, 284 lb uplift at joint 12, 153 lb uplift at joint 16, 102 lb uplift at joint 17, 154 lb uplift at joint 14 and 88 lb uplift at joint 13.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-114(F=-60), 2-6=-114(F=-60), 6-10=-114(F=-60), 10-11=-114(F=-60), 12-18=-30

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Dwg.#0120061213

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T02	COMMON	4	1	Job Reference (optional)

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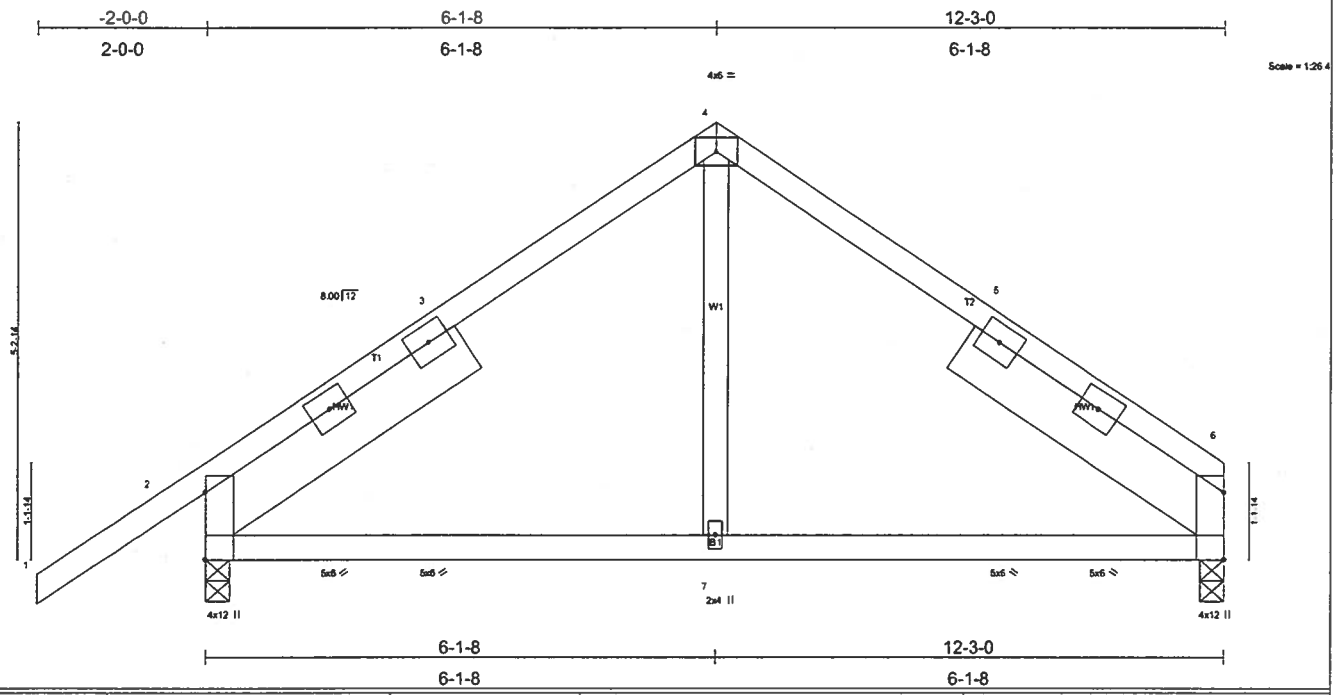

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.04	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 3-10-8, Right 2 X 8 SYP No.1D 3-10-8

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=506/0-3-8, 2=631/0-3-8
Max Horz 2=182(load case 4)
Max Uplift6=-156(load case 6), 2=-281(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-559/185, 3-4=-375/214, 4-5=-428/204, 5-6=-527/183
BOT CHORD 2-7=-50/356, 6-7=-50/356
WEBS 4-7=0/197

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 6 and 281 lb uplift at joint 2.

LOAD CASE(S) Standard

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Dwg.#0120061214

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T03	COMMON	1	2	Job Reference (optional)

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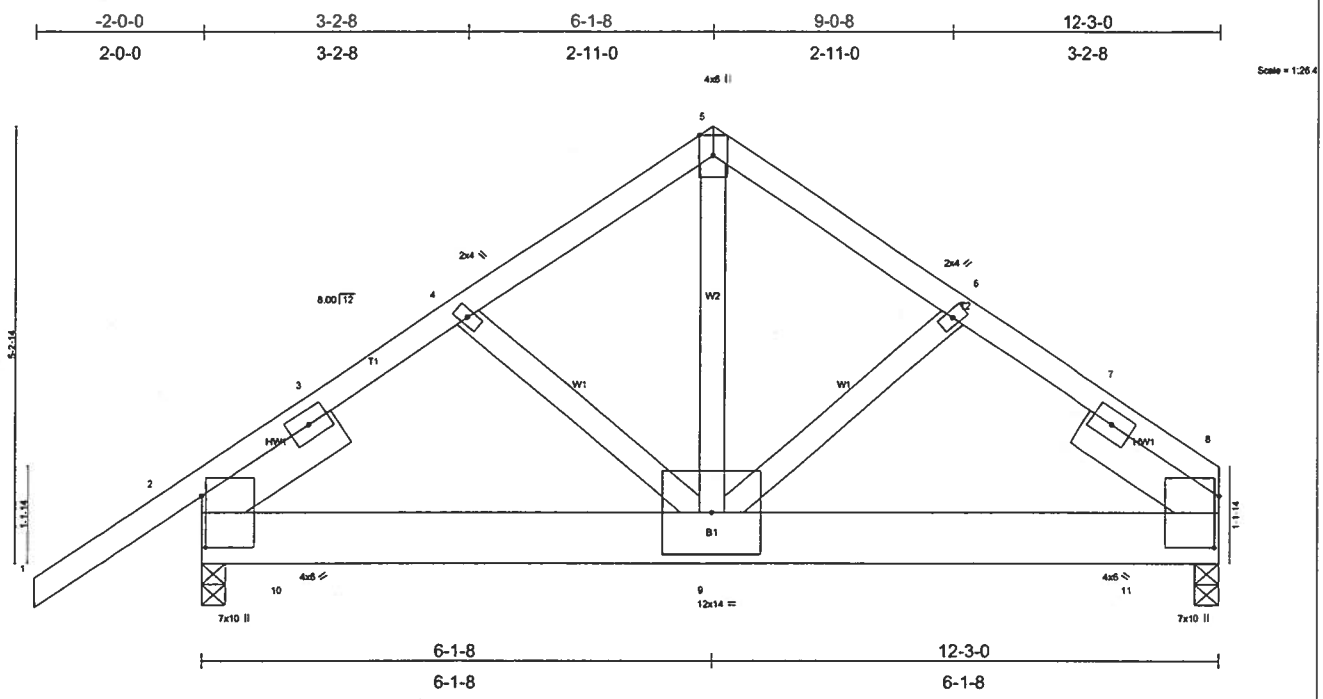

Plate Offsets (X,Y): [2:0-7-6,0-0-11], [8:0-7-6,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.04	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.07	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.62	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 1-11-9, Right 2 X 6 SYP No.1D 1-11-9

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=3739/0-3-8, 2=3865/0-3-8
Max Horz 2=180(load case 3)
Max Uplift8=-1422(load case 5), 2=-1548(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-3783/1436, 3-4=-3799/1479, 4-5=-3780/1496, 5-6=-3780/1483, 6-7=-3805/1483, 7-8=-3785/1438
BOT CHORD 2-10=-1157/3050, 9-10=-1157/3050, 9-11=-1121/3062, 8-11=-1121/3062
WEBS 4-9=-131/206, 5-9=-1509/3874, 6-9=-127/199

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1422 lb uplift at joint 8 and 1548 lb uplift at joint 2.
6) Girder carries tie-in span(s): 32-4-0 from 1-0-0 to 11-3-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-10=-30, 10-11=-661(F=-631), 8-11=-30

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0120061215

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T04	COMMON	1	1	Job Reference (optional)

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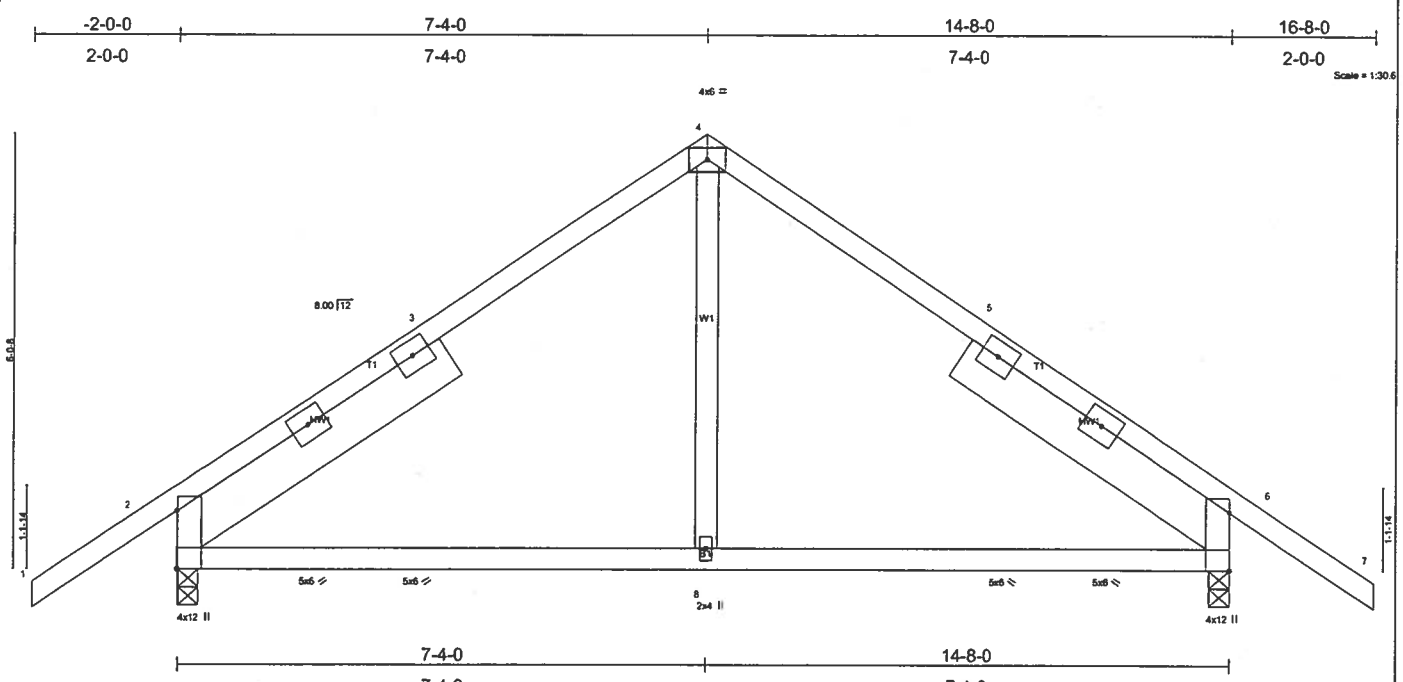

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.07	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.10	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 4-7-4, Right 2 X 8 SYP No.1D 4-7-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=724/0-3-8, 6=724/0-3-8
Max Horz 2=201(load case 4)
Max Uplift2=-309(load case 5), 6=-309(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-643/214, 3-4=-523/235, 4-5=-523/235, 5-6=-642/214, 6-7=0/26
BOT CHORD 2-8=-55/435, 6-8=-55/435
WEBS 4-8=0/238

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 2 and 309 lb uplift at joint 6.

LOAD CASE(S) Standard

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Dwg.#0120061216

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T04G	COMMON	1	1	Job Reference (optional)

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-2-0-0 | 7-4-0 | 14-8-0 | 16-8-0

2-0-0 | 7-4-0 | 7-4-0 | 2-0-0

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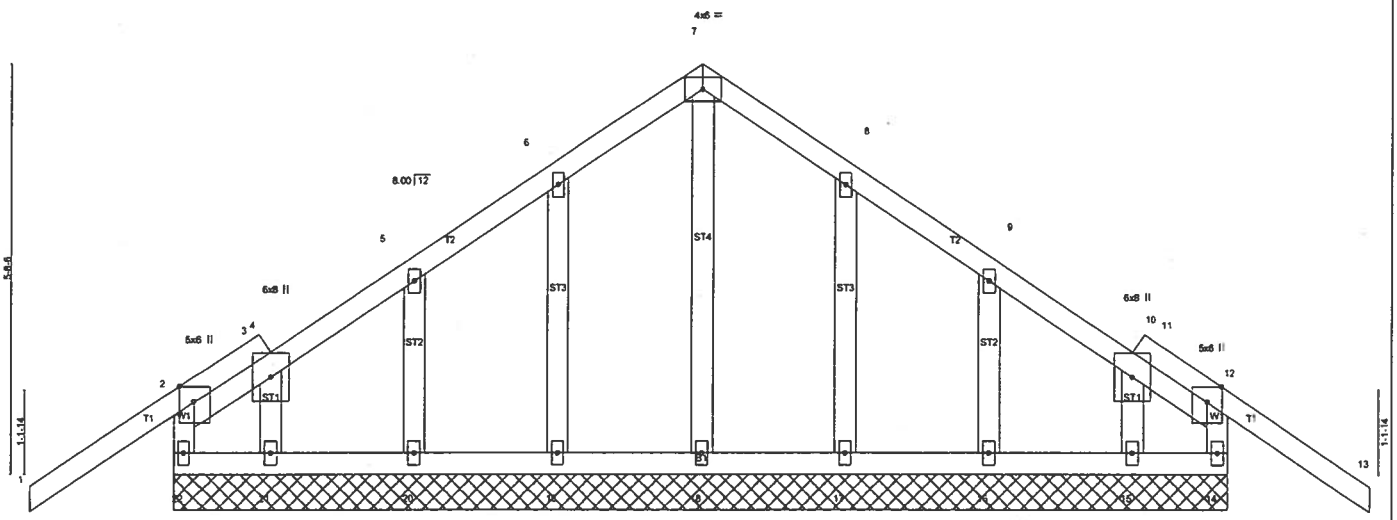

14-8-0

14-8-0

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.57	Vert(LL) -0.06 13 n/r 120	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(TL) -0.09 13 n/r 90	
BCLL 10.0	Rep Stress Incr NO	WB 0.14	Horz(TL) 0.00 14 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 91 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 22=476/14-8-0, 14=476/14-8-0, 18=331/14-8-0, 19=290/14-8-0, 20=292/14-8-0, 21=57/14-8-0, 17=290/14-8-0, 16=292/14-8-0, 15=57/14-8-0
Max Horz 22=161(load case 4)
Max Uplift22=-243(load case 5), 14=-265(load case 6), 19=-146(load case 5), 20=-147(load case 5), 21=-95(load case 4), 17=-144(load case 6), 16=-149(load case 6), 15=-79(load case 3)
Max Grav 22=484(load case 9), 14=484(load case 10), 18=331(load case 1), 19=294(load case 9), 20=292(load case 9), 21=107(load case 3), 17=294(load case 10), 16=292(load case 10), 15=91(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-17/136, 2-3=-94/125, 3-4=-60/95, 4-5=-74/107, 5-6=-28/124, 6-7=-31/180, 7-8=-31/180, 8-9=-28/114, 9-10=-47/107, 10-11=-61/95, 11-12=-76/95, 12-13=-17/136, 2-22=-466/332, 12-14=-466/332
BOT CHORD 21-22=-36/137, 20-21=-36/137, 19-20=-36/137, 18-19=-36/137, 17-18=-36/137, 16-17=-36/137, 15-16=-36/137, 14-15=-36/137
WEBS 7-18=-270/0, 6-19=-235/156, 5-20=-230/166, 3-21=-65/54, 8-17=-235/155, 9-16=-230/167, 11-15=-60/49

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 243 lb uplift at joint 22, 265 lb uplift at joint 14, 146 lb uplift at joint 19, 147 lb uplift at joint 20, 95 lb uplift at joint 21, 144 lb uplift at joint 17, 149 lb uplift at joint 16 and 79 lb uplift at joint 15.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-114(F=-60), 2-7=-114(F=-60), 7-12=-114(F=-60), 12-13=-114(F=-60), 14-22=-30

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
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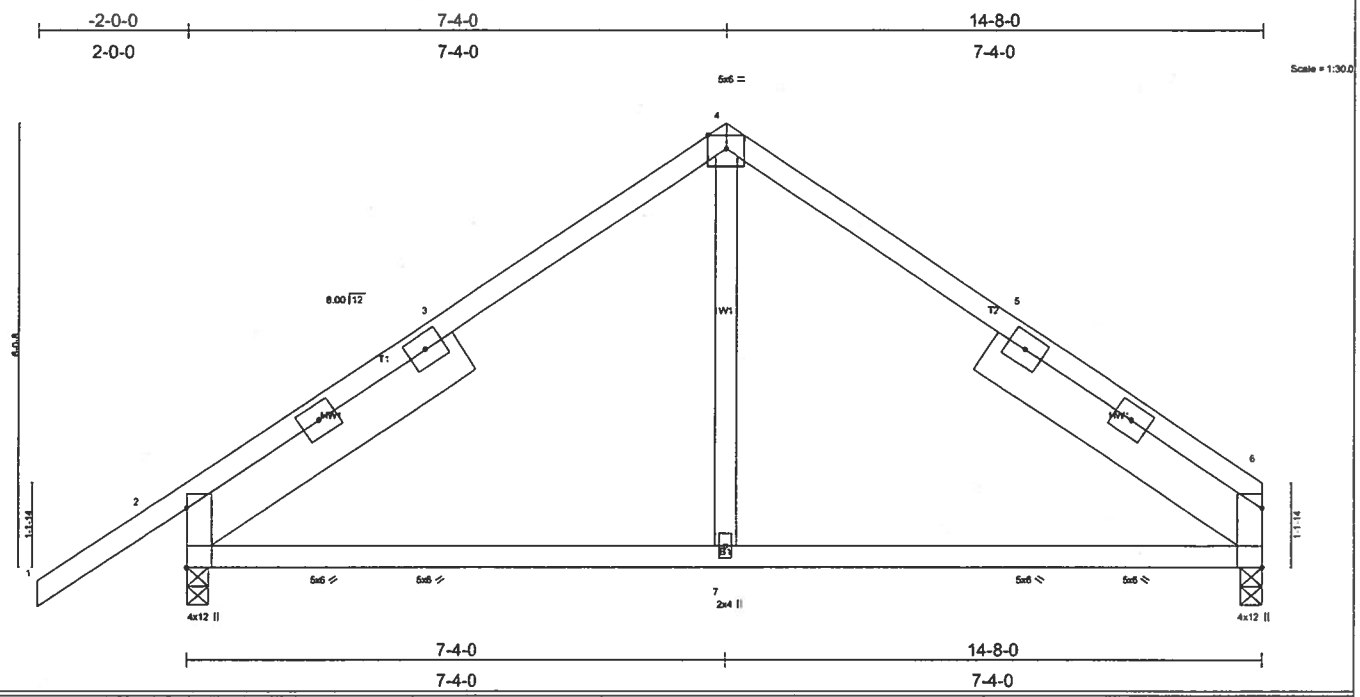
Dwg.#0120061217

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T05	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:48 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 8 SYP No.1D 4-7-4, Right 2 X 8 SYP No.1D 4-7-4

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=609/0-3-8, 2=731/0-3-8
Max Horz 2=210(load case 4)
Max Uplift6=-189(load case 6), 2=-312(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-657/230, 3-4=-538/252, 4-5=-538/244, 5-6=-648/220
BOT CHORD 2-7=-72/447, 6-7=-72/447
WEBS 4-7=0/241

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 6 and 312 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0120061218

Job	Truss	Truss Type	Qty	Ply	NORTON HOME IMP.CO.INC.-HATCHER
L136839	T06	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jan 20 11:25:49 2006 Page 1

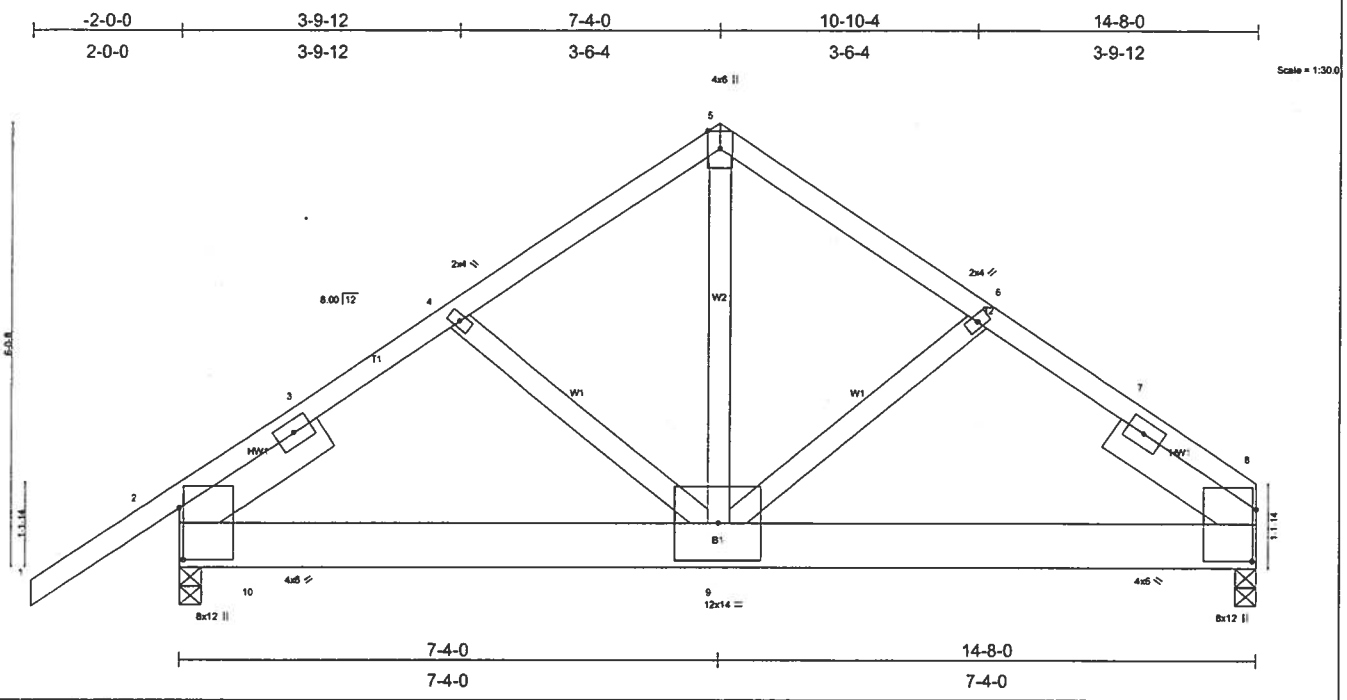


Plate Offsets (X,Y) [2:0-8-6,0-0-11], [8:0-8-6,0-0-11]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.08	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.13	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(TL)	0.02	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 214 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-3-15, Right 2 X 6 SYP No.1D 2-3-15

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=5214/0-3-8, 2=4748/0-3-8
Max Horz 2=208(load case 3)
Max Uplift8=-1992(load case 5), 2=-1886(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-4810/1825, 3-4=-4822/1873, 4-5=-4769/1884, 5-6=-4773/1874, 6-7=-4840/1885, 7-8=-4818/1832
BOT CHORD 2-10=-1492/3918, 9-10=-1492/3918, 8-9=-1459/3957
WEBS 4-9=-124/157, 5-9=-1928/4965, 6-9=-107/120

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1992 lb uplift at joint 8 and 1886 lb uplift at joint 2.
6) Girder carries tie-in span(s): 32-4-0 from 1-0-0 to 14-8-0

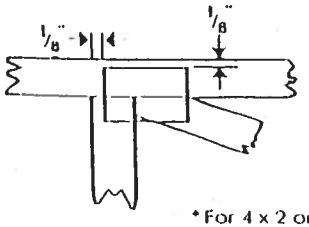
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-10=-30, 8-10=-661(F=-631)

JANUARY 20, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

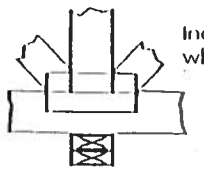
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

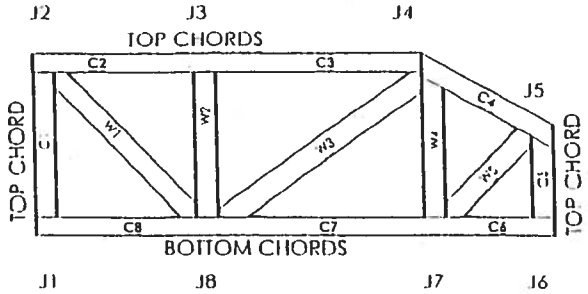
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transfering connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS; Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☑ N/A	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

- a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ NA ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☐ NA ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☐ NA ☐ b) Floor joist size and spacing

☐ NA ☐ c) Girder size and spacing

☐ NA ☐ d) Attachment of joist to girder

☐ NA ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC Information

☐ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ NA ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

☐ ☐ **Disclosure Statement for Owner Builders**

☐ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done**

☐ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

* HAS water system

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 09-4S-17-08300-089 **Building permit No.** 000024213

Use Classification SFD, UTILITY **Fire:** 17.76

Permit Holder JAMES NORTON **Waste:** 36.75

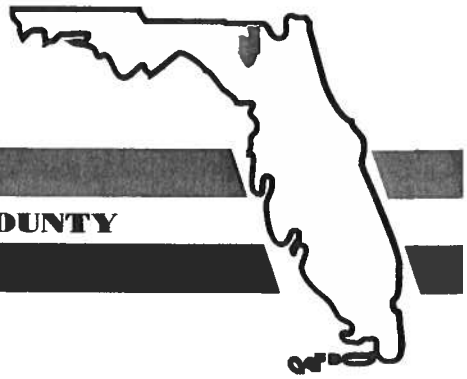
Owner of Building JOHN & LANELLE HATCHER **Total:** 54.51

Location: 247 SE PUEBLO WAY

Date: 07/13/2006

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

MEMORANDUM

Date: 28 February 2006

To: John Colson, P.E., County Engineer

From: Brian L. Kepner, County Planner BLK

Re: Flood Resolution 2005R-26

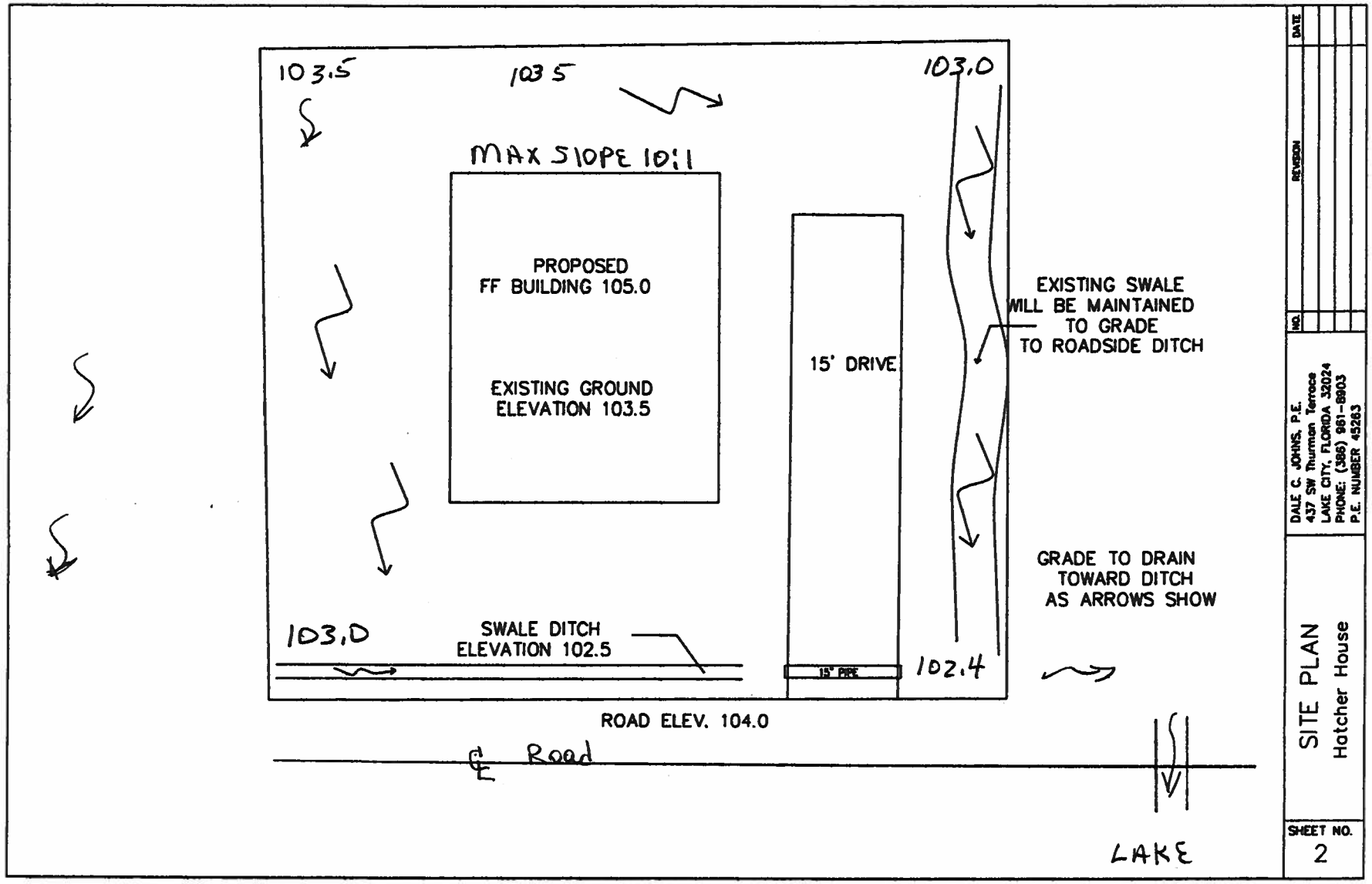
Please find attached the items submitted for Lot 17, Woodhaven, Unit 4 Subdivision. Please review for compliance with Flood Resolution 2005R-26. The lot is in the 100 year flood zone with a flood elevation determined to be at 104 feet. County's regulations will require that the elevation of the 1st floor be no lower than 105 feet and the engineer must provide a 1 foot rise certification.

Hatcher house

This house is being built in an existing subdivision with houses on all sides. The existing ditch in the front of the lot flows to the south for one lot and crosses the road through a pipe, then to Alligator Lake. The slopes will vary between 10:1 and 30:1. The lot has an existing swale along the south property line that drains toward the front ditch. This will prevent any build up of water from the adjacent lots.

2-16-06

12-09



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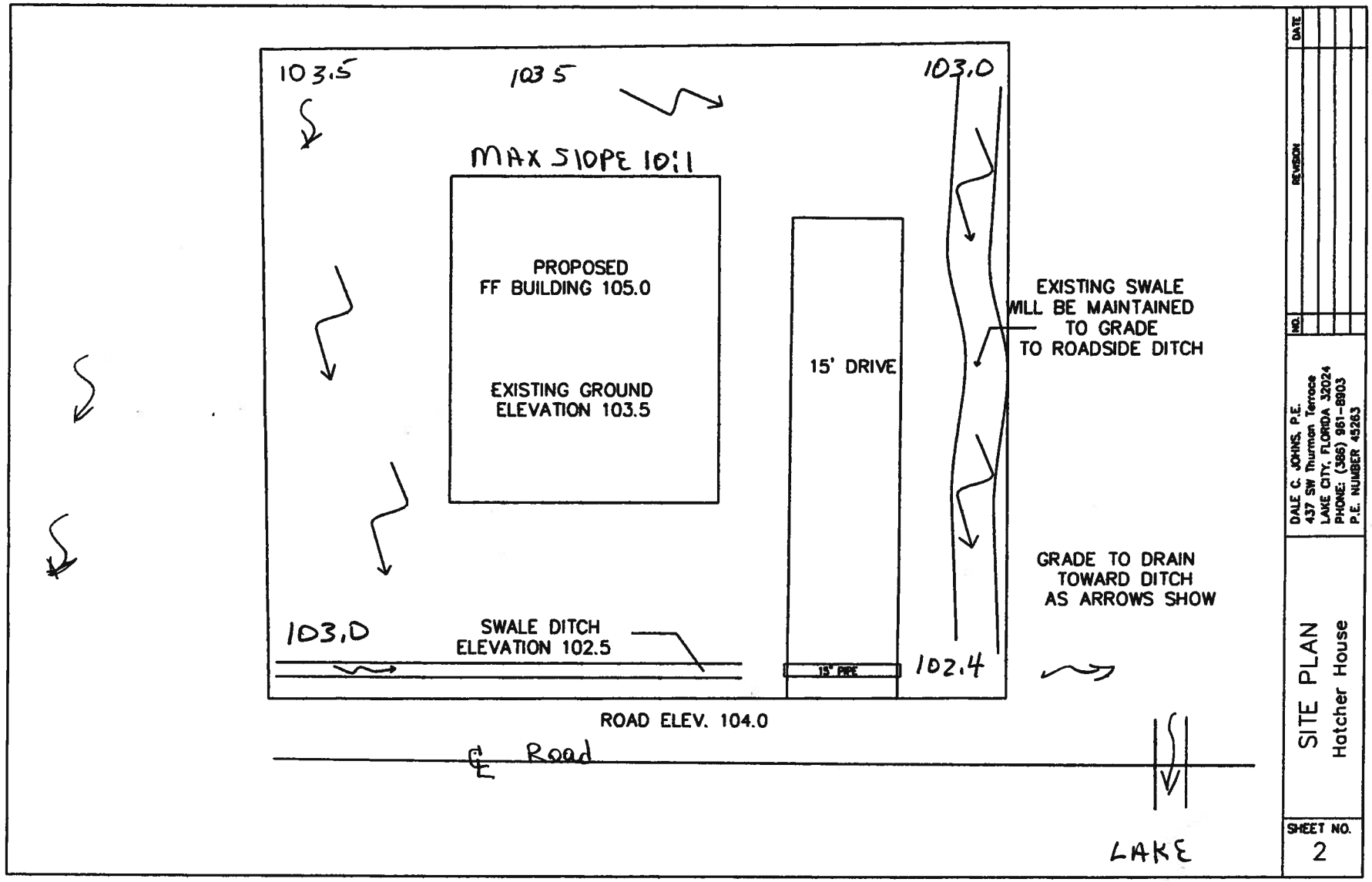
Dale Johns



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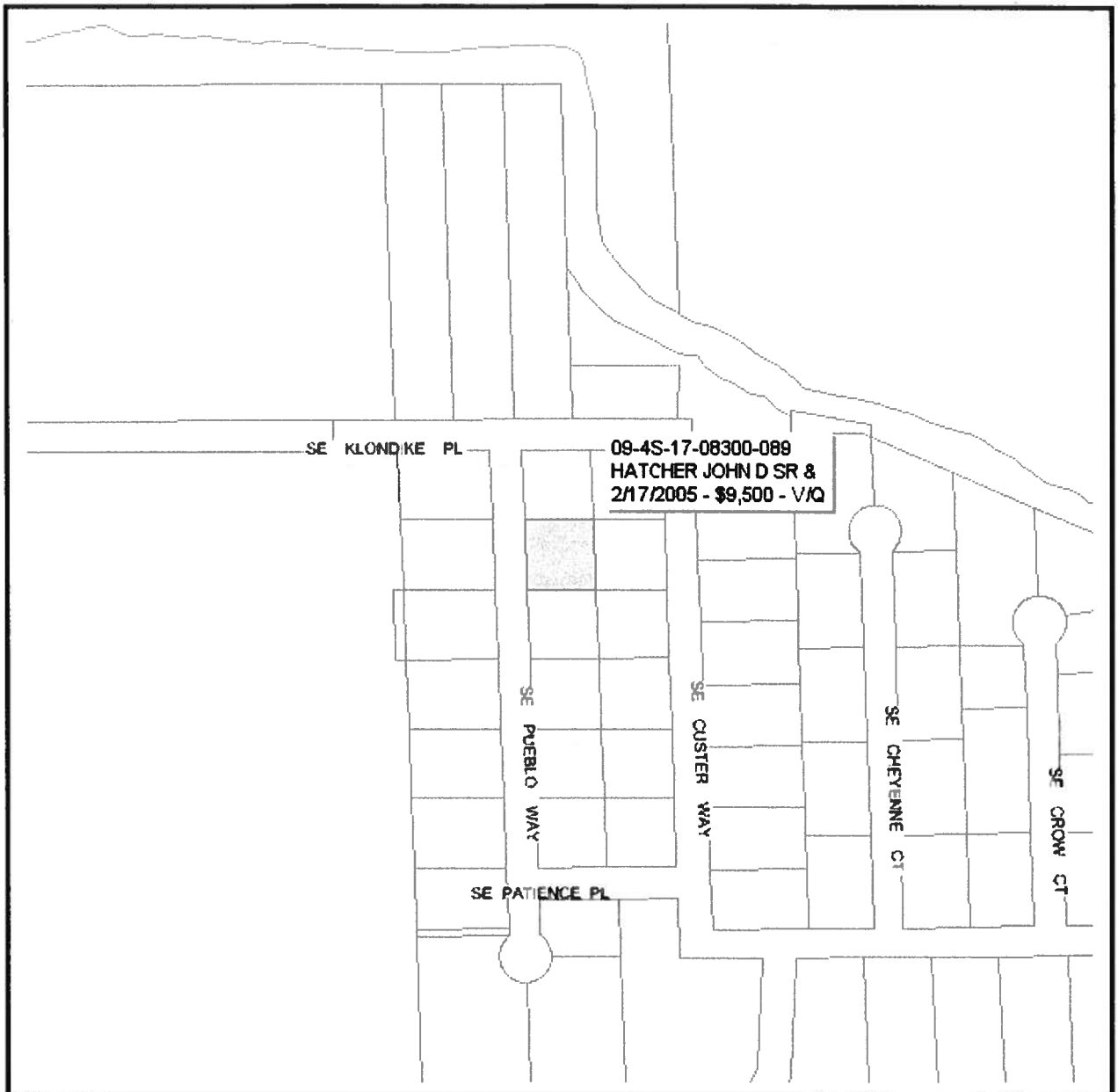


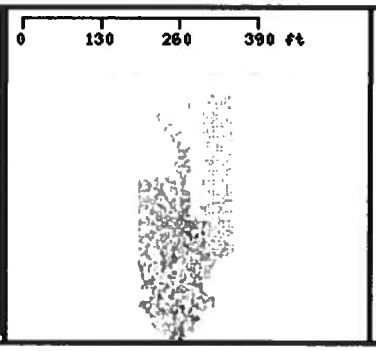
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

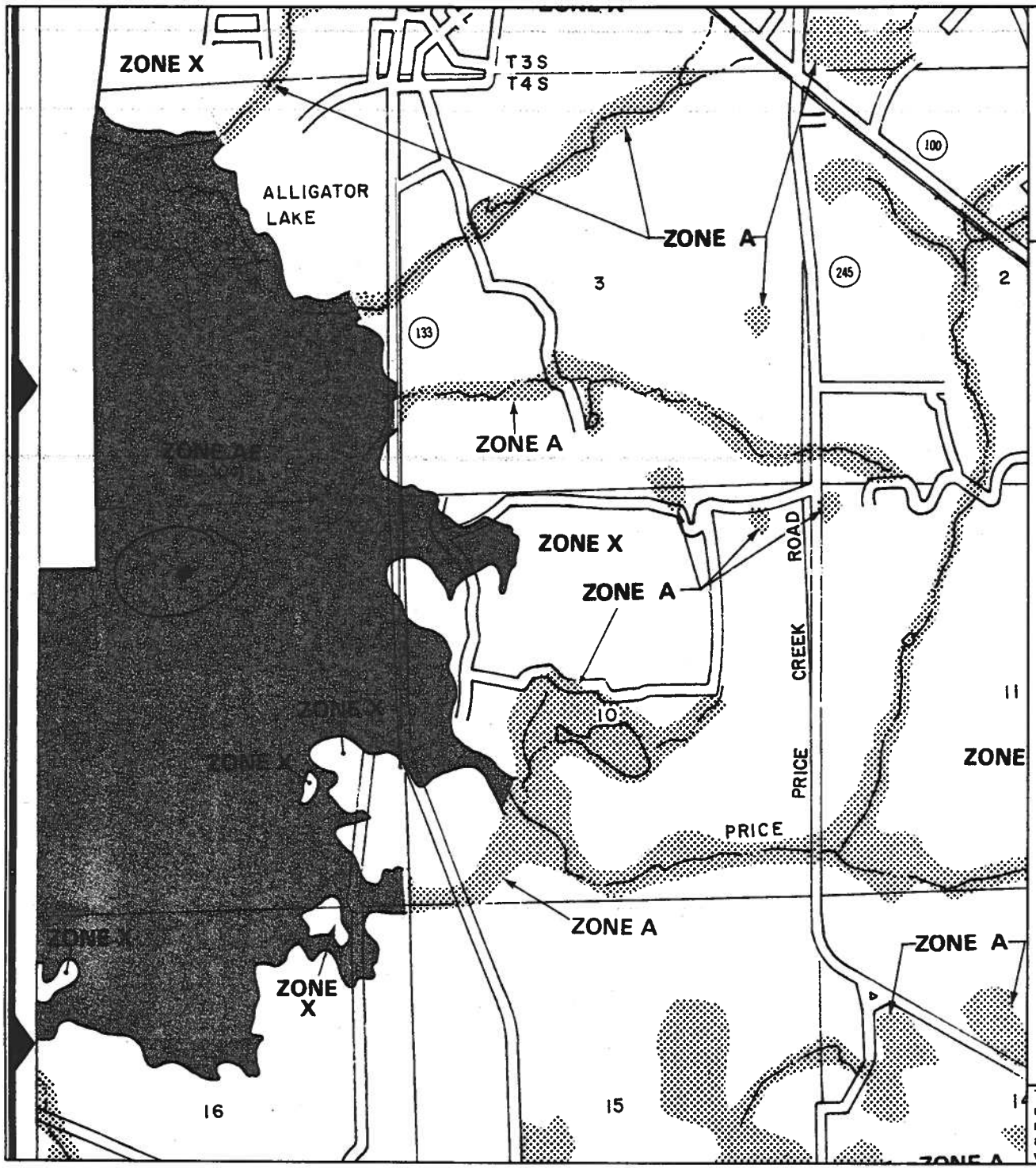
PARCEL: 09-4S-17-08300-089 - VACANT (000000)

LOT 17 WOODHAVEN S/D UNIT 4. ORB 522-435, 898-944, WD 1038-913.

Name:	HATCHER JOHN D SR &	LandVal	$8,500.00
Site:		BldgVal	$0.00
	LANELLE G HATCHER	ApprVal	$8,500.00
Mail:	2675 SE COUNTRY CLUB RD #101	JustVal	$8,500.00
	LAKE CITY, FL 32025	Assd	$8,500.00
Sales	2/17/2005 $9,500.00 V / Q	Exmpt	$0.00
Info	2/24/2000 $6,200.00 V / Q	Taxable	$8,500.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.



APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

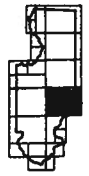
FIRM

FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 200 OF 300

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0200 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 2/10/2006 (printed at scale and type A)

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE ISSUED: 28 December 2005

ENHANCED 9-1-1 ADDRESS:

247 SE PUEBLO WAY (LAKE CITY, FL 32025)

Addressed Location 911 Phone Number: NOT AVAIL.

OCCUPANT NAME: NOT AVAIL.

OCCUPANT CURRENT MAILING ADDRESS:

PROPERTY APPRAISER PARCEL NUMBER: 09-4S-17-08300-089

Other Contact Phone Number (If any):

Building Permit Number (If known):

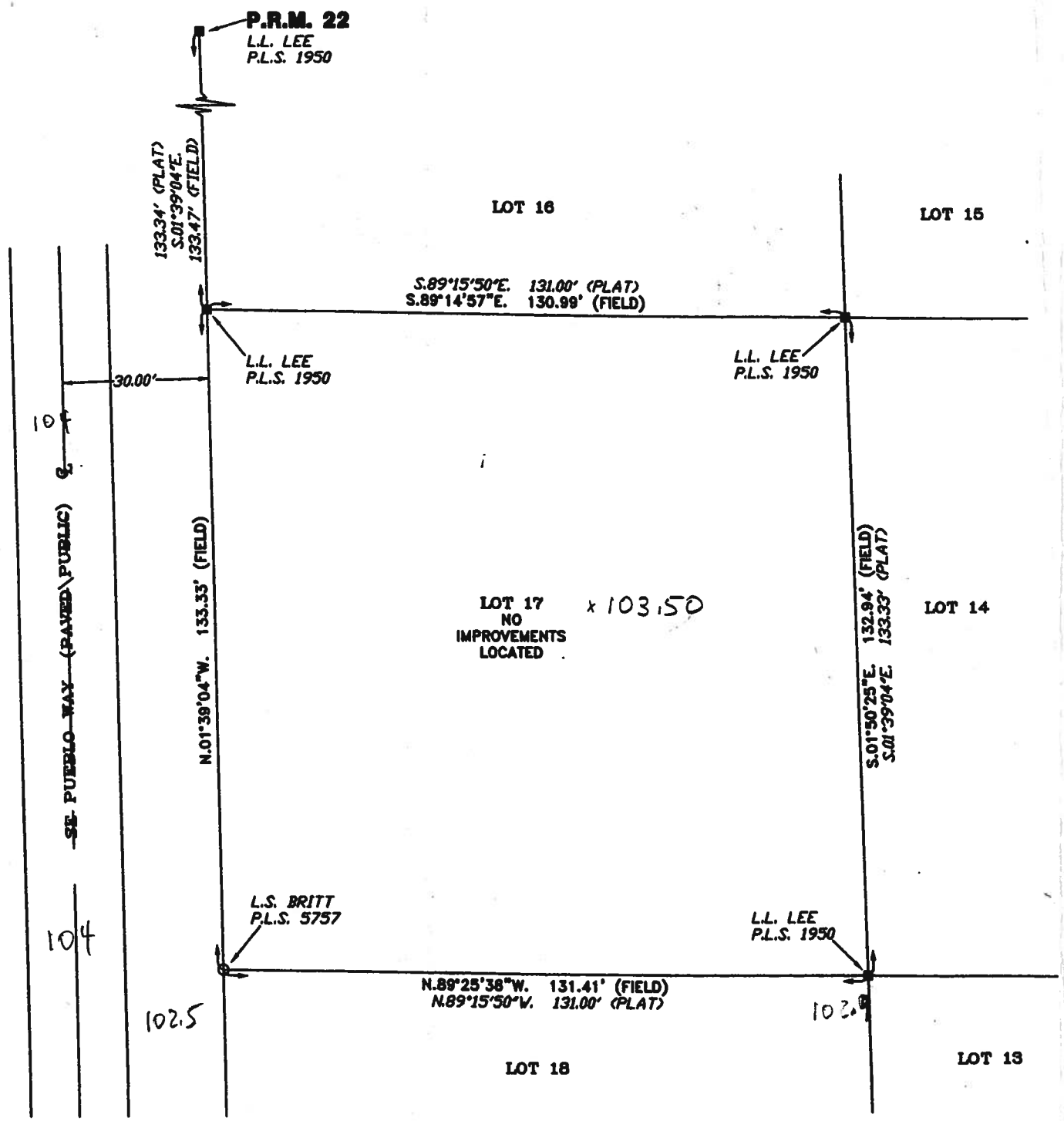
Remarks: LOT 17, UNIT 4, WOODHAVEN S/D

Address Issued By:
Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

COLUMBIA COUNTY
9-1-1 ADDRESSING
APPROVED

BOUNDARY SURVEY IN SECTION 9, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA.



N

SCALE: 1" = 30'

SYMBOL LEGEND:

- ■ 4"X4" CONCRETE MONUMENT FOUND
- □ 4"X4" CONCRETE MONUMENT SET
- ● IRON PIPE FOUND
- ○ IRON PIN AND CAP SET
- ⊕ POWER POLE
- ▲ WATER METER
- ℄ CENTERLINE
- ✱ WELL
- ◉ SATELLITE DISH
- ⊙ TELEPHONE BOX
- —E— ELECTRIC LINES
- —X— WIRE FENCE
- —o— CHAIN LINK FENCE
- —▫— WOODEN FENCE

DESCRIPTION:
LOT 17 OF "WOODHAVEN UNIT IV" AS PER PLAT THEREOF RECORDED IN PLAT BOOK 4, PAGE 108 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
3. THIS PARCEL IS IN ZONE "AE" AND IS SUBJECT TO FLOODING. A BASE FLOOD ELEVATION IS ESTABLISHED TO BE 104.00 FEET AS PER FLOOD INSURANCE RATE MAP, DATED 6 JAN. 1988 COMMUNITY PANEL NO. 120070 0200 B. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.

CERTIFIED TO:

JOHN & LANELLE HATCHER

FIELD BOOK: 272 PAGE(S): 74

SURVEYOR'S CERTIFICATION

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

01/26/05 FIELD SURVEY DATE

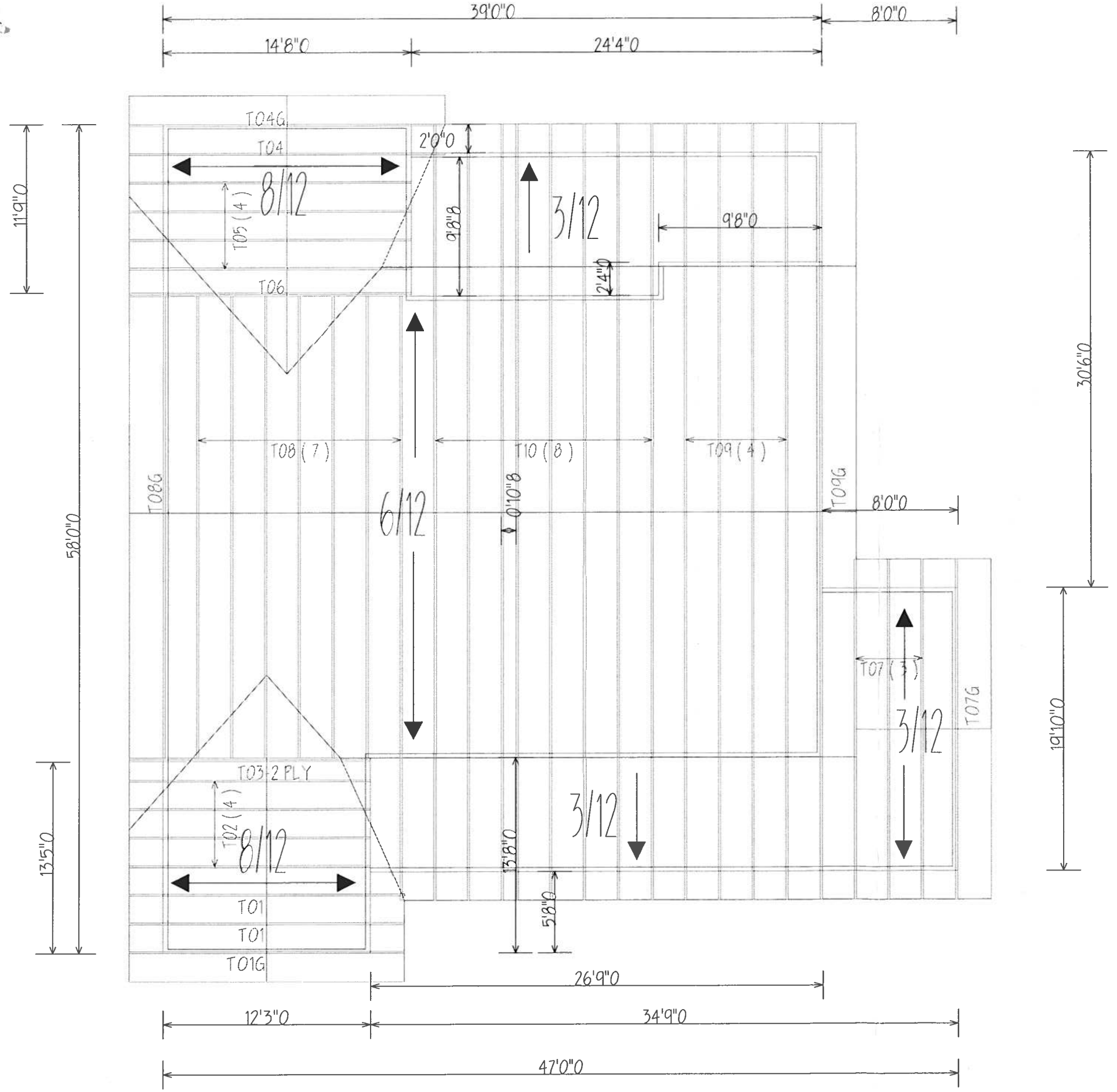
01/30/05 DRAWING DATE

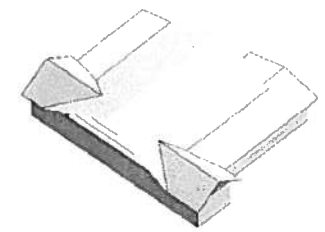
L. SCOTT BRITT, P.S.M. CERTIFICATION # 5757

NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

BRITT SURVEYING
LAND SURVEYORS AND MAPPERS
830 WEST DUVAL STREET LAKE CITY, FLORIDA 32055
(386)752-7163 FAX (386)752-5573

WORK ORDER # L-15742





HANGERS

12-HTU26

BEARING HEIGHT SCHEDULE

8'-0"

3/12

6/12

8/12

2' 0/H

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date : _____

Approved by: _____ Date: _____

Builders FirstSource

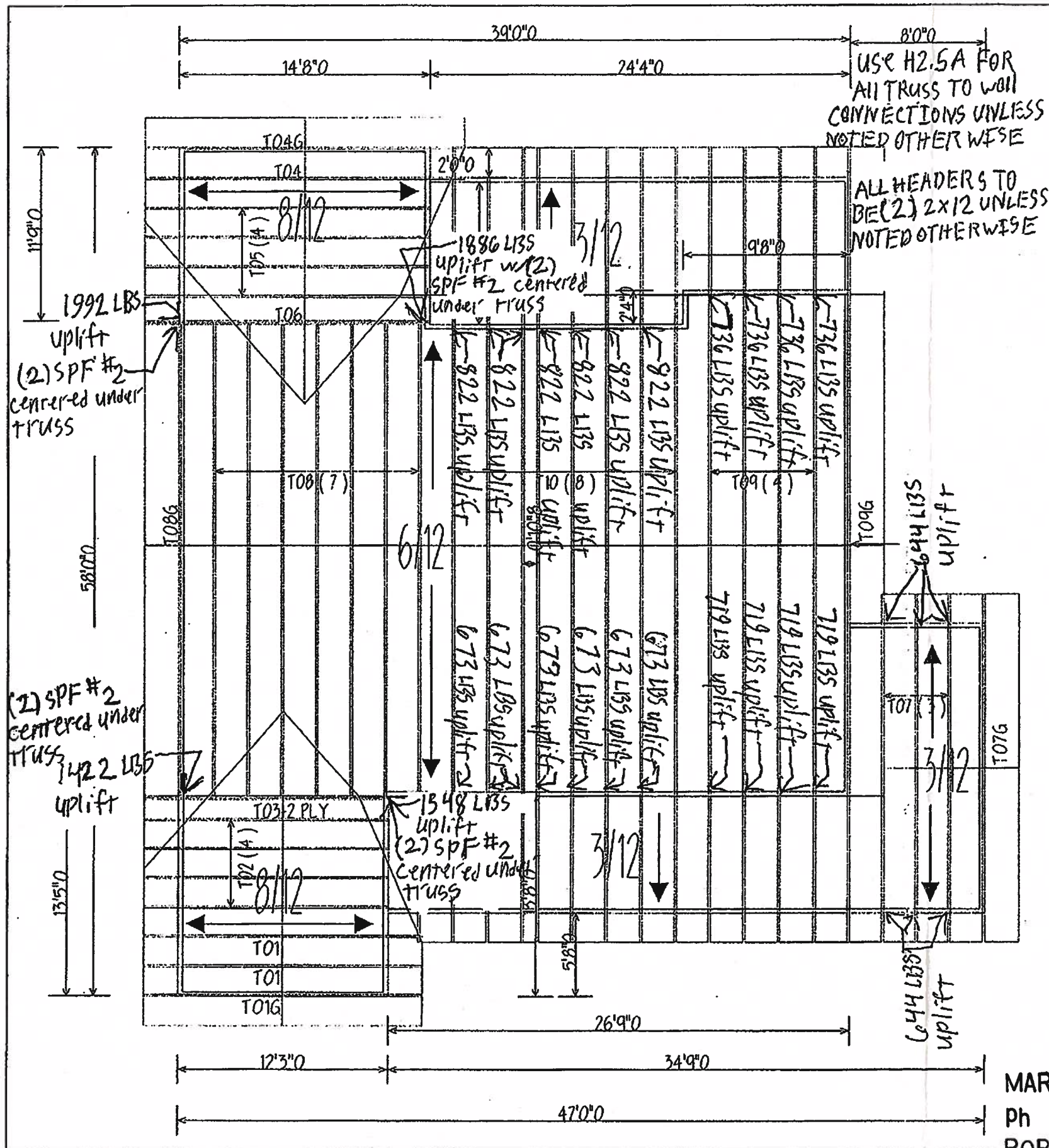
Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:	NORTON HOME IMP.-HATCHER
LEGAL ADDRESS:	LAKE CITY
MODEL:	CUSTOM
REVISION:	
SCALE:	NTS
DATE:	1/20/2006
DRAWN BY:	TJR
JOB #:	L136839



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 585	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 000	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT1I1	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU83	18 - 16d		2-5/8" AB

BEARING HEIGHT SCHEDULE

8'-0"

3/12
6/12
8/12
2' 0/H

NOTES:

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8) DRAWINGS ASSEMBLED (QTY) TO BE FURNISHED BY BUILDER.

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PHONE: 407-322-0059 FAX: 407-322-5553

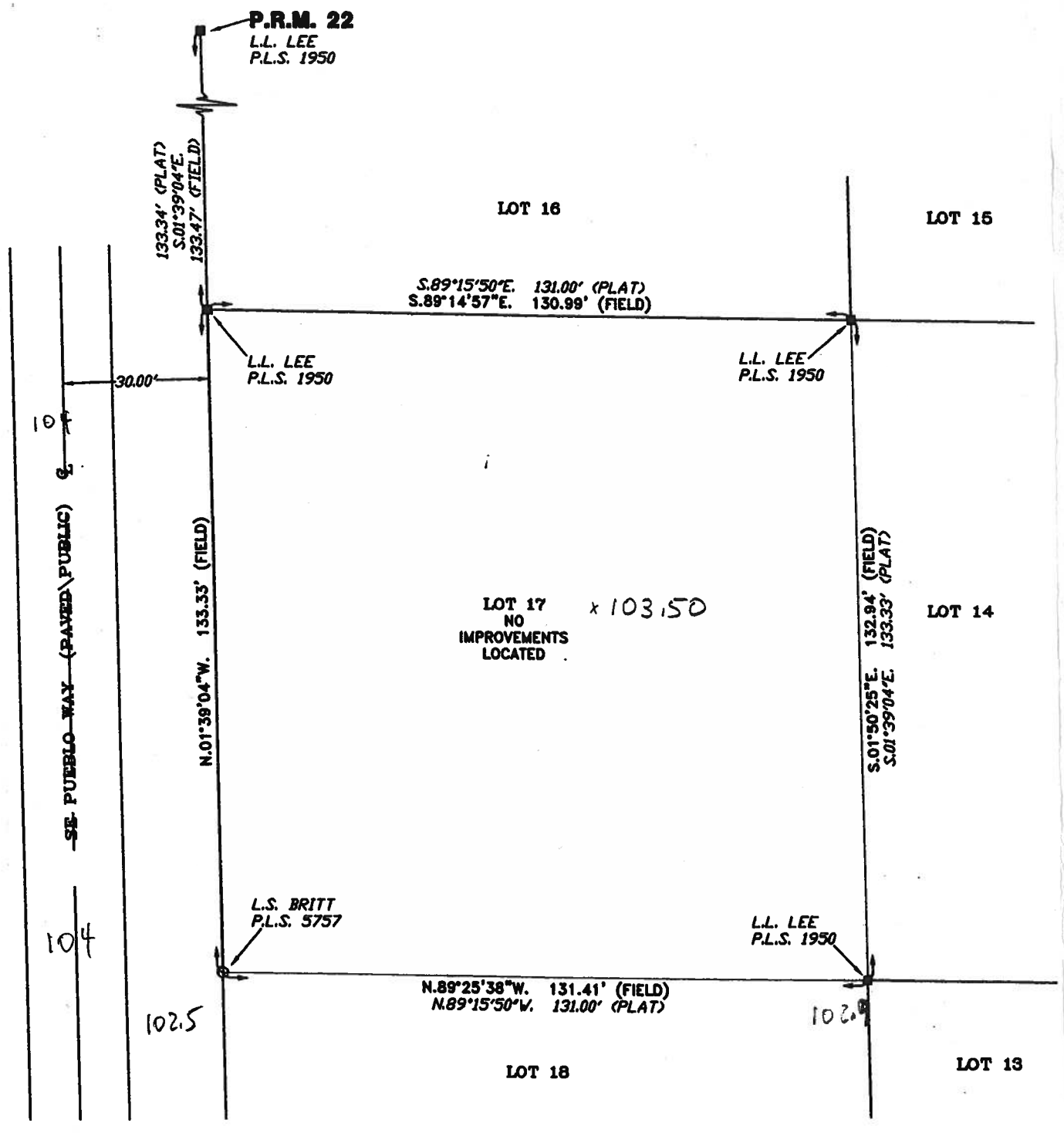
BUILDER: NORTON HOME IMP.-HATCHER
DEVELOPMENT: LAKE CITY
MATL: CUSTOM
REVISION:
SCALE: NTS
DATE: 1/20/2006
DESIGNER: TJR
JOB#: L136839

12-HTU26

MARK DISOSWAY P.E.
Ph 386-754-5419
~~POB868, Lake City, F 32056~~
PE No FL-53915 NC-26032

23FEB06

BOUNDARY SURVEY IN SECTION 9, TOWNSHIP 4 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA.



SCALE: 1" = 30'

SYMBOL LEGEND:

- ■ 4"X4" CONCRETE MONUMENT FOUND
- □ 4"X4" CONCRETE MONUMENT SET
- ● IRON PIPE FOUND
- ○ IRON PIN AND CAP SET
- ⊕ POWER POLE
- ▲ WATER METER
- ℄ CENTERLINE
- ✱ WELL
- ⊛ SATELLITE DISH
- ⊙ TELEPHONE BOX
- —E— ELECTRIC LINES
- —X— WIRE FENCE
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- —▫— WOODEN FENCE

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CERTIFIED TO:

JOHN & LANELLE HATCHER

FIELD BOOK: 272 PAGE(S): 74

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01/26/05 FIELD SURVEY DATE

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L. SCOTT BRITT, P.S.M. CERTIFICATION # 5757

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(386)752-7163 FAX (386)752-5573

WORK ORDER # L-15742