



## WIND LOAD CALCULATIONS FOR

**CAMBRIDGE PREP ACADEMY**

**SIGN REPLACEMENT**

**PROJECT NO.: J221127CPA**

**January 29, 2023**

### GENERAL NOTES

1. WHERE SITE CONDITIONS DEVIATE FROM THOSE NOTED HEREIN, REVISIONS MAY BE REQUIRED OR A SEPARATE SITE-SPECIFIC ENGINEERING EVALUATION PERFORMED.

2. DESIGN IS IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 7TH EDITION (2020).

3. WIND LOADS HAVE BEEN CALCULATED PER THE REQUIREMENTS OF ASCE 7-16 AS SHOWN HEREIN, EXCEPT WHERE NOTED OTHERWISE.

4. THESE ENGINEERING CALCULATIONS PERTAIN ONLY TO THE STRUCTURAL INTEGRITY OF THOSE SYSTEMS, COMPONENTS, AND/OR OTHER CONSTRUCTION EXPLICITLY SPECIFIED HEREIN AND/OR IN ACCOMPANYING ENGINEERING DRAWINGS. THE EXISTING HOST STRUCTURE (IF ANY) IS ASSUMED TO BE IN GOOD CONDITION, CAPABLE OF SUPPORTING THE LOADED SYSTEM, SUBJECT TO BUILDING DEPARTMENT APPROVAL. NO WARRANTY, EITHER EXPRESSED OR IMPLIED, IS CONTAINED HEREIN.

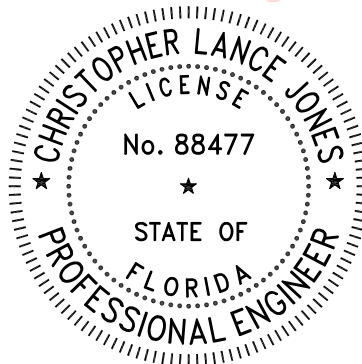
5. SYSTEM COMPONENTS SHALL BE AS NOTED HEREIN. ALL REFERENCES TO NAMED COMPONENTS AND INSTALLATION SHALL CONFORM TO MANUFACTURER'S OR INDUSTRY SPECIFICATIONS AS SUMMARIZED HEREIN.

6. ALUMINUM COMPONENTS IN CONTACT WITH STEEL OR EMBEDDED IN CONCRETE SHALL BE PROTECTED AS PRESCRIBED IN THE 2015 ALUMINUM DESIGN MANUAL, PART 1-A. STEEL COMPONENTS IN CONTACT WITH, BUT NOT ENCASED IN, CONCRETE SHALL BE COATED, PAINTED, OR OTHERWISE PROTECTED AGAINST CORROSION.

7. ENGINEER SEAL AFFIXED HERETO VALIDATES STRUCTURAL DESIGN AS SHOWN ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, ET. AL, INDEMNIFIES AND SAVES HARMLESS THIS ENGINEER FOR ALL COSTS & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM DEVIATION FROM THIS DESIGN.



Christopher L Jones  
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This item has been digitally signed and sealed by C. Lance Jones, PE, on the date adjacent to the seal.

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**CHRISTOPHER LANCE JONES, PE**  
**FLORIDA REGISTRATION NO.:**

**88477**

Job # J221127CPA  
 Project Cambridge Prep Academy Sign Replacement  
 Job Location 2658 SW Main Boulevard  
 Lake City, FL 32025  
 20'-0" Pole Sign, 6' x 14' Cabinet and 3' x 8' Cabinet  
 # of Supports

### INPUT DATA

Exposure Category (B, C, or D)

Risk Category

Ultimate Design Windspeed

Nominal Design Windspeed

Topographic Factor

Wind directionality factor (Table 26.6-1, page 266)

$K_z$  = velocity pressure exposure coefficient (Table 26.10-1)

$K_e$  = ground elevation factor, see (Table 26.9-1, page 268)

Gust effect factor (Sec. 26.11-1, page 269)

Net force coefficient (Fig. 29.3-1, page 323)

**Velocity Pressure at height h (Eq. 26.10-1 page 268)**

**Max horizontal wind pressure =**

Height of the sign

Average Vertical dimension (for wall,  $s = h$ )

Horizontal dimension

=	C	
=	II	
$V_{ULT}$ =	120	MPH
$V_{ASD}$ =	93	MPH
$K_{zt}$ =	1.00	FLAT
$K_d$ =	0.85	
$K_z$ =	0.91	
$K_e$ =	1.00	
$G$ =	0.85	
$C_f$ =	1.8	
$q_z = .00256k_z * k_{zt} * k_d * V^2$ =	28.51	PSF
$p = q_z G C_f$ =	43.63	PSF
$h$ =	20	FT
$s$ =	6	FT
$B$ =	14	FT

### ANALYSIS

ASCE 7-10, Load Case = 0.6W +D

Allowable Stress Design Wind Factor

Design Wind Pressure

$\phi$ =	1	
$\phi \times p$ =	43.63	PSF

Sign	Top Elev. (FT)	Height (FT)	Length (FT)	Centroid (FT)	Area <sub>Trib</sub> (FT <sup>2</sup> )	P = Force (LBS)	Moment (FT-LBS)
Top Sign	20.00	6.00	14.00	17.00	42.00	1832	31150
Bottom Sign	14.00	3.00	8.00	12.50	12.00	524	6544
Poles	14.00	14.00	1.67	7.00	11.67	509	3563
<b>Totals</b>						<b>2865</b>	<b>41256</b>

### FOOTING DESIGN (NON CONSTRAINED 1807.3.2.1)

Lateral Load = P

Height of applied force above grade = h

Base Diameter =

Soil Pressure =

Trial Depth =

$S_1$  =

$A = (2.34P / (S_1 b))$

Required Embedment Depth =  $d = 0.5A \{1 + [1 + (4.36 h/A)]^{1/2}\}$

$P$ =	2864.83	LBS
$h$ =	14.40	FT
$b$ =	3.92	FT
$q_b$ =	150.00	PSF/FT
$d$ =	5.67	FT
$S_1$ =	567.00	PSF/FT
$A$ =	3.02	FT
$d$ =	5.40	FT

**Design Depth = 6.00 FT**

**Note: Used allowable stress design wind factor of 1 instead of 0.6 due to unknowns**

## **FOUNDATION DESIGN CHECK AGAINST EXISTING STRUCTURE**

### **EXISTING STRUCTURE DIMENSIONS & LOADING**

Design Wind Pressure	P =	28.19 PSF
Overturning Safety Factor	FS =	1.5

#### **Existing Signage Area and Location of Applied Load**

Top	A1 =	42.00 SF
	h1 =	28.00 FT
Bottom	A2 =	23.25 SF
	h2 =	9.00 FT
Pole	A3 =	20.83 SF
	h3 =	12.50 FT

**Overturning Moment =  $M_e = P \cdot (A_1 h_1 + A_2 h_2 + A_3 h_3)$**

**$M_e = 46391.34 \text{ FT-LB}$**

### **PROPOSED STRUCTURE DIMENSIONS & LOADING**

Design Wind Pressure	P =	26.18 PSF
Overturning Safety Factor	FS =	1.5

#### **Proposed Signage Area and Location of Applied Load**

Top	A1 =	42.00 SF
	h1 =	17.00 FT
Bottom	A2 =	12.00 SF
	h2 =	12.50 FT
Pole	A3 =	11.67 SF
	h3 =	7.00 FT

**Overturning Moment =  $M_e = P \cdot (A_1 h_1 + A_2 h_2 + A_3 h_3)$**

**$M_p = 24757.55 \text{ FT-LB}$**

**$M_p < M_e \text{ OK}$**

**FS = 1.87**