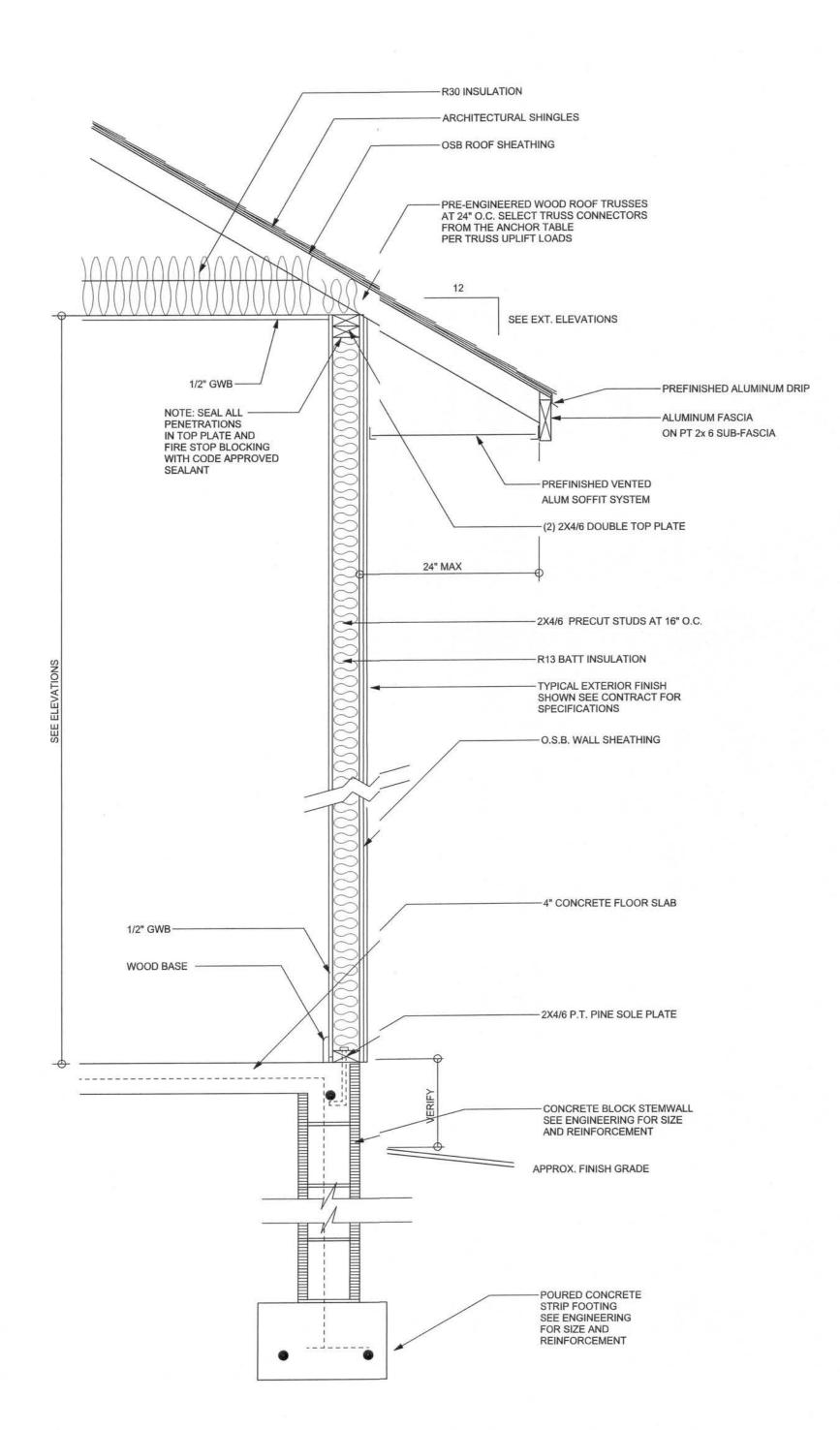


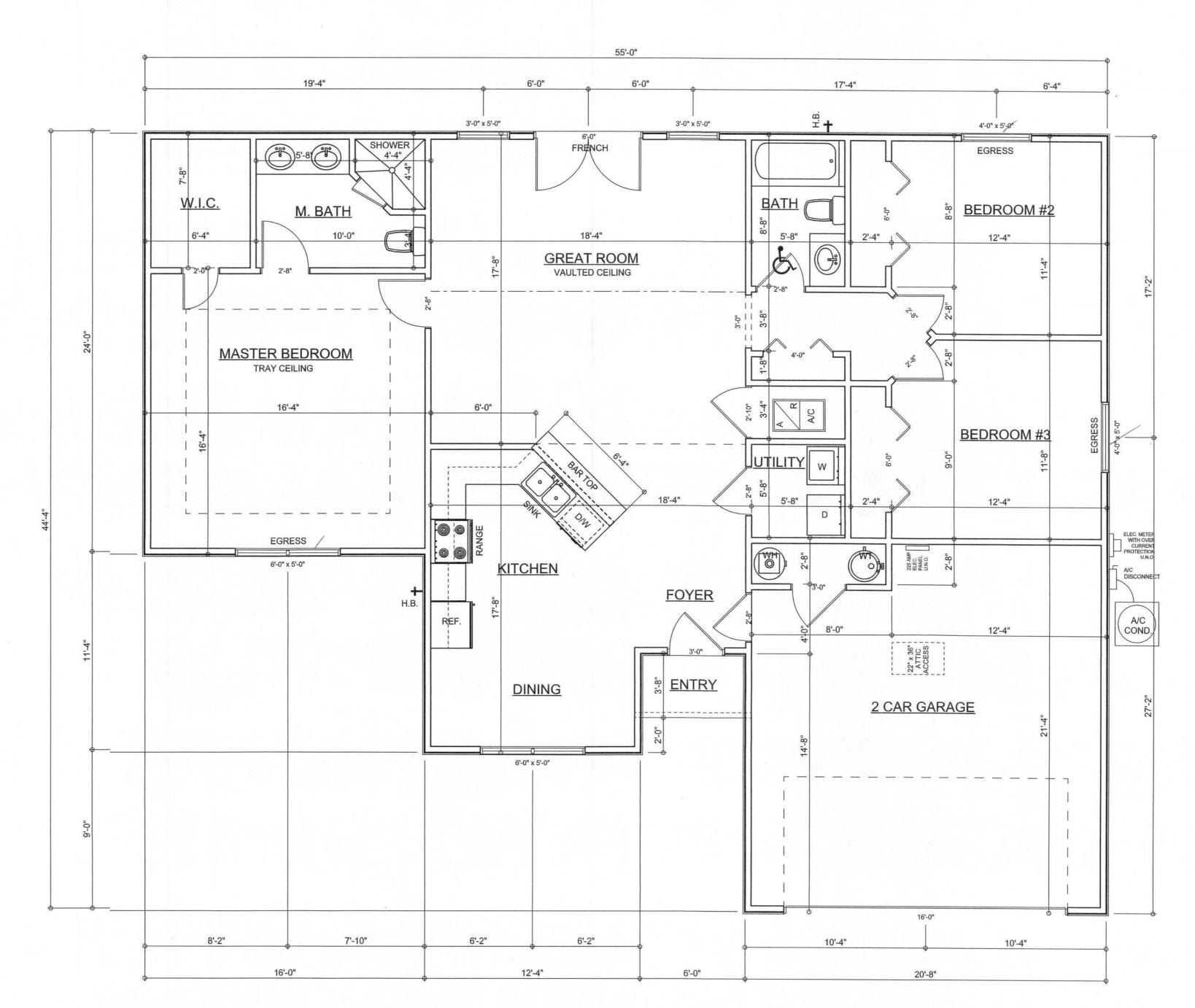
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TYPICAL DESIGNVALL SECTION

NON - STRUCTURAL DATA

SCALE: 1" = 1'- 0"



FLOOR PLAN SCALE: 1/4" = 1'-0"

ALL CEILING HEIGHTS TO BE 8'-0" UNLESS NOTED OTHERWISE

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.

3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

AREA SUMMARY

LIVING ADEA	4.470	
LIVING AREA	1478	S.F.
GARAGE AREA	436	S.F.
PORCH AREA	22	S.F.
TOTAL AREA	1936	S.F.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

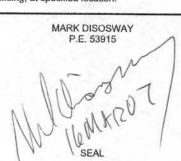
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building

code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



Wade Willis
Construction

Spec House Lot 39 Crosswinds S/D

> ADDRESS: Lot 39 Crosswinds S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

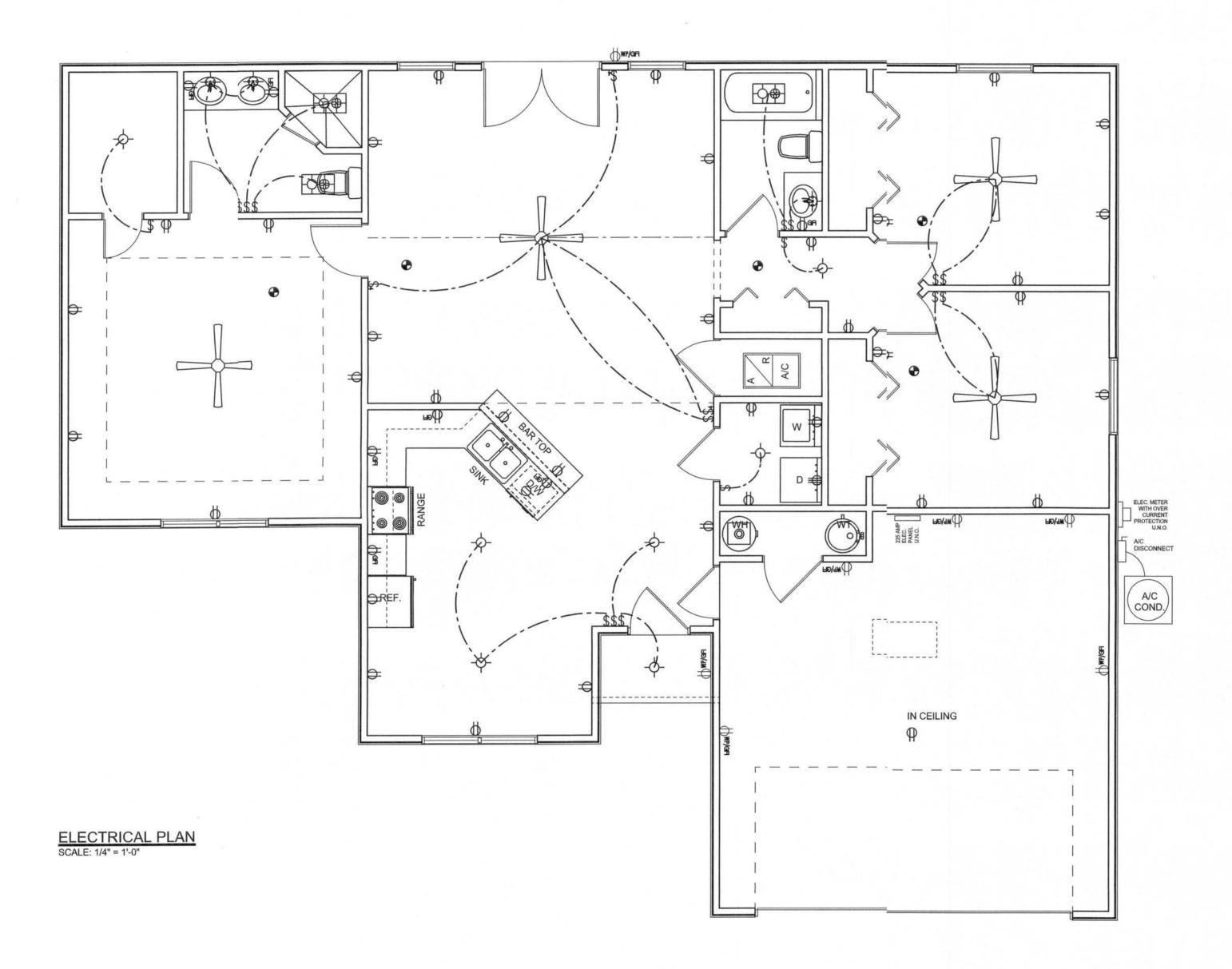
PRINTED DATE:
March 15, 2007

DRAWN BY: STRUCTURAL BY:
David Disosway

FINALS DATE: 01 / Mar / 07

> JOB NUMBER: 702263 DRAWING NUMBER

> > A-2 OF 6 SHEETS



ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
 BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE E -5

 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
 TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE
- E -10 CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD.	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
Ф	220v OUTLET
Фан	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
₩ _{WP/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
0	TELEVISION JACK
•	GARAGE DOOR OPENER
	WALL HEATER

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

REVISIONS

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MARK DISOSWAY P.E. 53915

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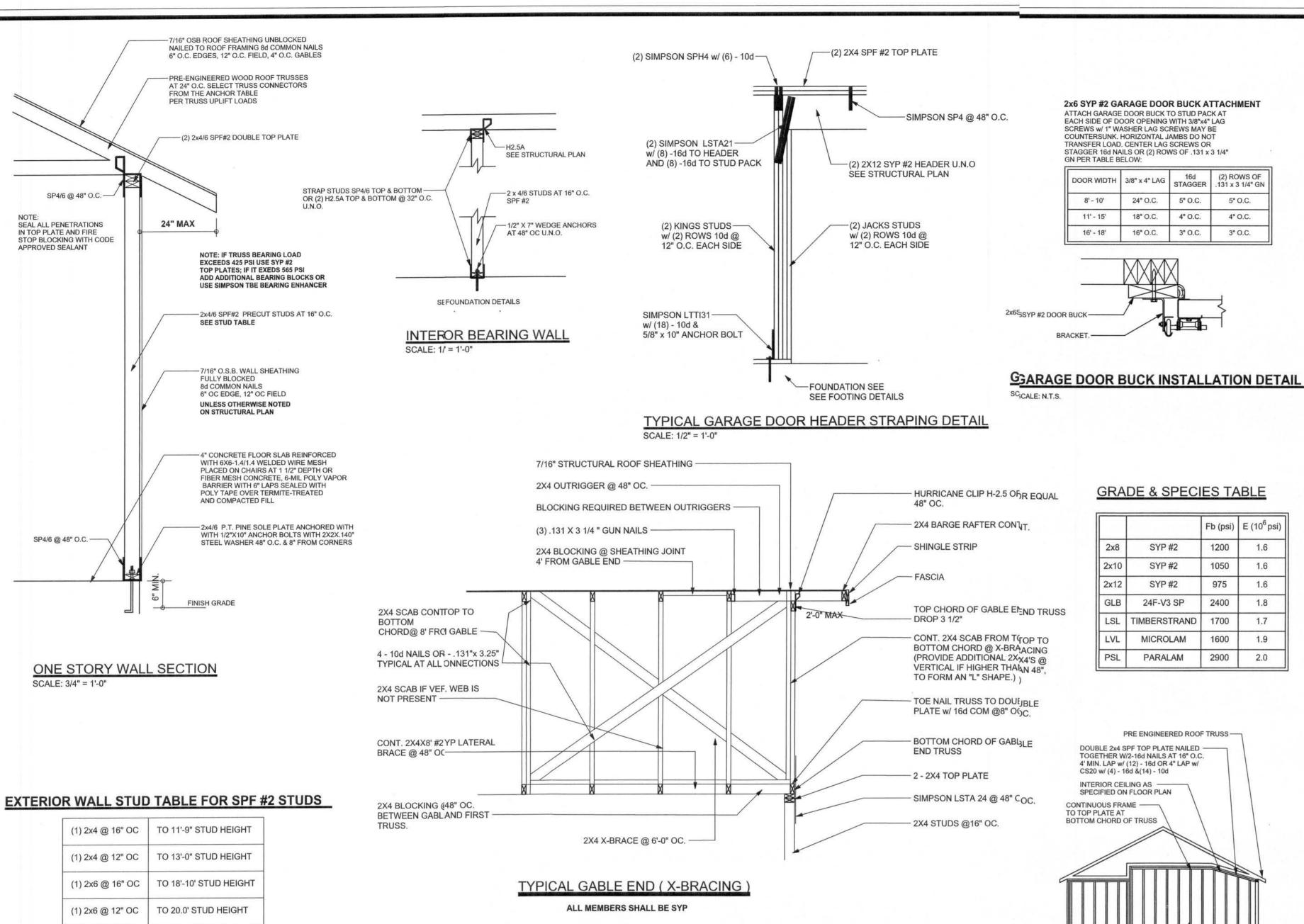
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OF 6 SHEETS



----NON-SUPPORTIVE

SUPPORTIVE POST TO BEAM

SUPPORTIVE CENTER POST TO BEAM DETAIL

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM -

SUPPORTIVE -

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON-

OPPOSITE SIDE) EA

NAILED WITH 14-10d

IF BEAM JOINT IS AT ----

(2-ONE SIDE, 2-ON

OTHER SIDE)

POST CONNECTION.

2X4 LADDER BEAM

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B,

EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

SEE STRUCTURAL PLA

SCALE: N.T.S.

BEAM W/4-16

BEAM MAY BE A ACHED IN EITHER METHOSHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

SEE STRUCTURAL PLAN

-(4)-2x4 SPF #2 NAILED

TOGETHER W/2-16d NAILS AT 16" O.C.

BEAM MID-VALL CONNECTION DETAIL

MIN. (SEE STRUCTURAL PLAN)

SÉE STRUCTURAL PLAN

RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.

STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

ROOF SYSTEM DESIGN

(6) .131 x 3 1/4" GUN NAILS

TOE NAILED THRU HEADER

INTO KING STUD

ALL STUDS TO BE 2x4 ----

-LSTA18 (U.N.O.-

CRIPPLES IF REQUIRED

(4) .131 x 3 1/4" GUN NAILS

- TOE NAILED THRU SILL -

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

-SP4 OR (2) H2.5A OR (2) SSP---

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

. TYPICAL HEADER STRAPING DETAIL

(SEE STRUCTURAL PLAN)

ALL OPENINGS (U.N.O.)

CONTINUOUS FRAME TO

CEILING DIAPHRAGM DETAIL

-NAIL SHEATHING TO HEADER AND TOP

PLATE WITH 8d AT 4" O.C. FOR UPLIFT

SPF NAILED TO TOP

WITH 2-16d NAILS

SCALE: N.T.S.

(6) .131 x 3 1/4" GUN NAILS ---

INTO KING STUD

TOE NAILED THRU HEADER

AND BOTTOM PLATES

BEARING LOCATIONS.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE

PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR

RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO

INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.

FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN

CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO

ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT

OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE

SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004

ONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND

REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE

FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND

ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S

SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR

REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT

CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNO.

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

BUILDER'S RESPONSIBILITY

THE WIND LOAD ENGINEER IMMEDIATELY.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

JPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*		10.5 0.50	TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

REVISIONS

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0 4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

19.9 -21.8 18.1 -18.1 19.9 -25.5 18.1 -21.8 2 O'hg -40.6 19.9 -25.5 18.1 -21.8 3 O'hg | -68.3 | 4 | 21.8 | -23.6 | 18.5 | -20.4 | 5 21.8 -29.1 18.5 -22.6 Doors & Windows 21.8 -29. Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9 16x7 Garage Door 18.5 -21.0

DESIGN	10ADS				
1	25 CONTRACTOR OF THE CONTRACTO				
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)				
	30 PSF (SLEEPING ROOMS)				
	30 PSF (ATTICS WITH STORAGE)				
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)				
ROOF	20 PSF (FLAT OR <4:12)				
	16 PSF (4:12 TO <12:12)				
	12 PSF (12:12 AND GREATER)				
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)				
SOIL BE	ARING CAPACITY 1000PSF				
NOT IN	FLOOD ZONE (BUILDER TO VERIFY)				

examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

INDLOAD ENGINEER: Mark Disosway,

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled

dimensions. Refer all questions to

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Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have

32056, 386-754-5419

Wade Willis Construction

Spec House Lot 39 Crosswinds S/D

ADDRESS: Lot 39 Crosswinds S/D Columbia County, Florida

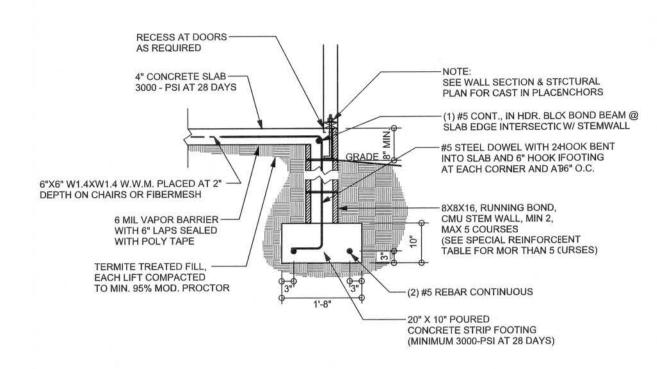
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

March 15, 2007 STRUCTURAL BY DRAWN BY: David Disosway

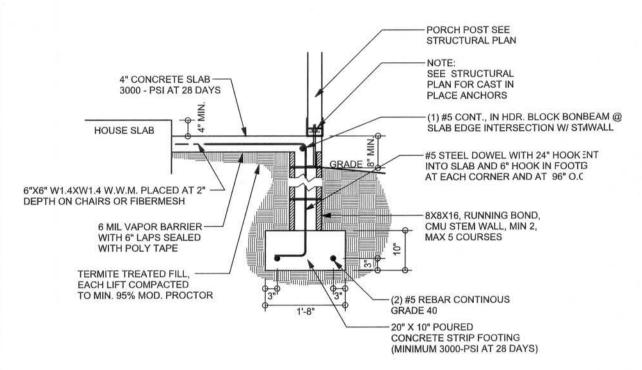
01 / Mar / 07

JOB NUMBER: 702263 DRAWING NUMBER

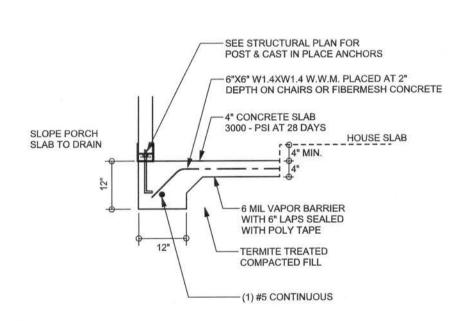
OF 6 SHEETS



F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

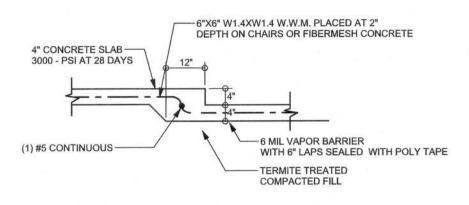




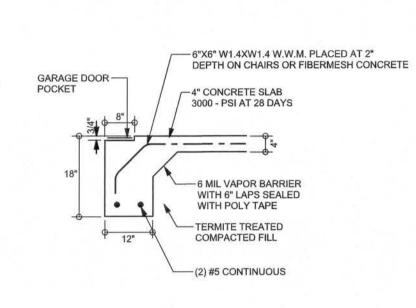
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

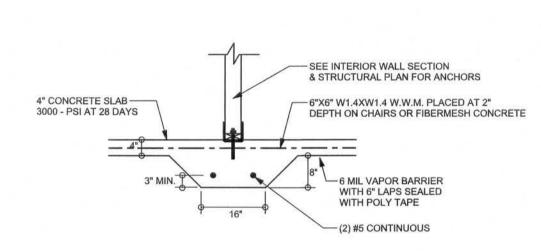
STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	FOR 8	AL REINFOR 5" CMU STEN INCHES O.C	WALL	VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



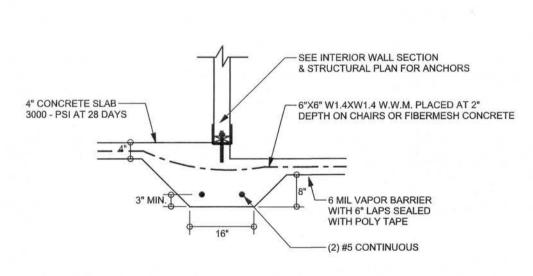
F6 TYPICAL NON - BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



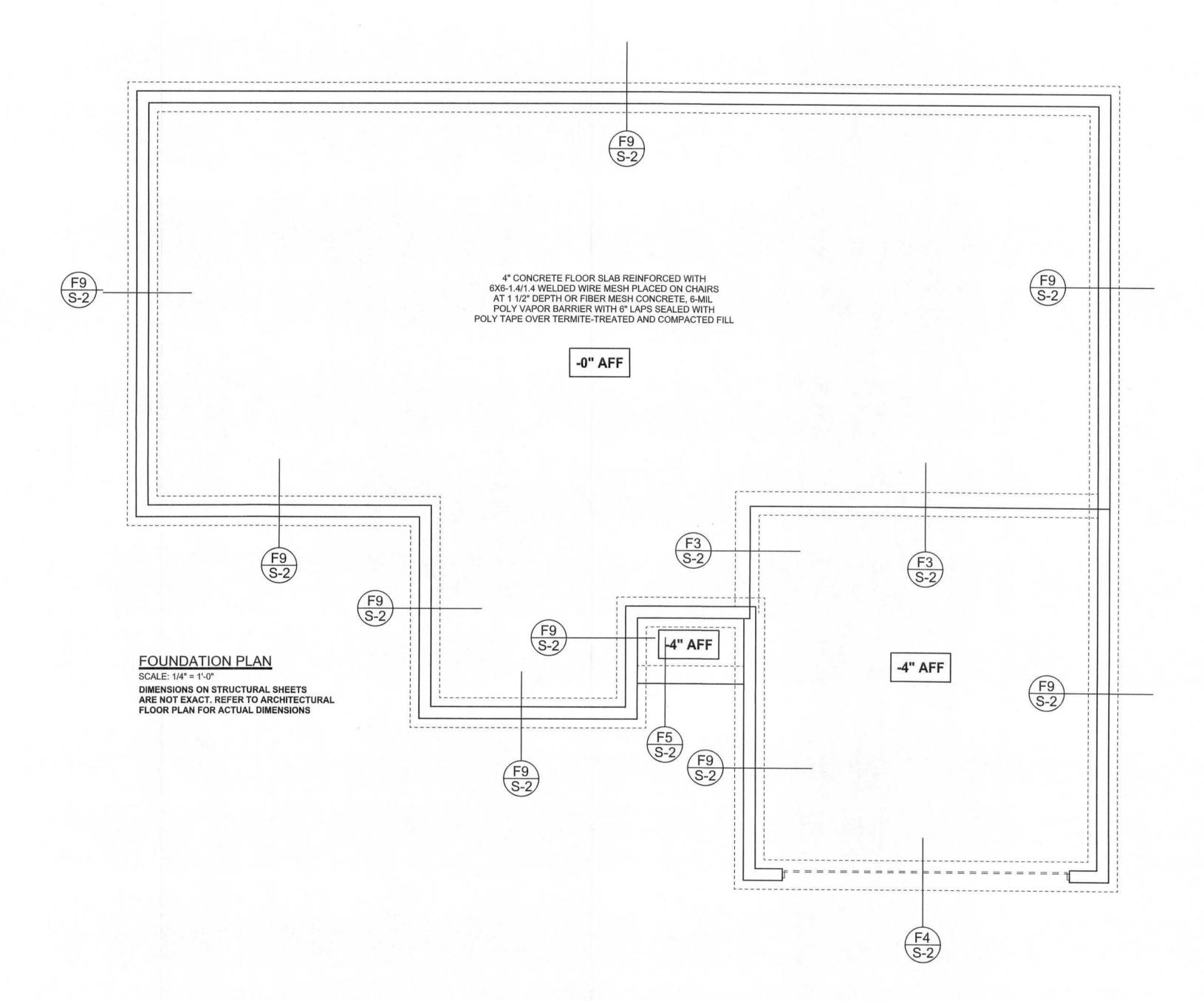
F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
S-2 SCALE: 1/2" = 1'-0"



REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercede scaled

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

Wade Willis Construction

Spec House Lot 39 Crosswinds S/D

> ADDRESS: Lot 39 Crosswinds S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
March 15, 2007

DRAWN BY: STRUCTURAL BY
David Disosway

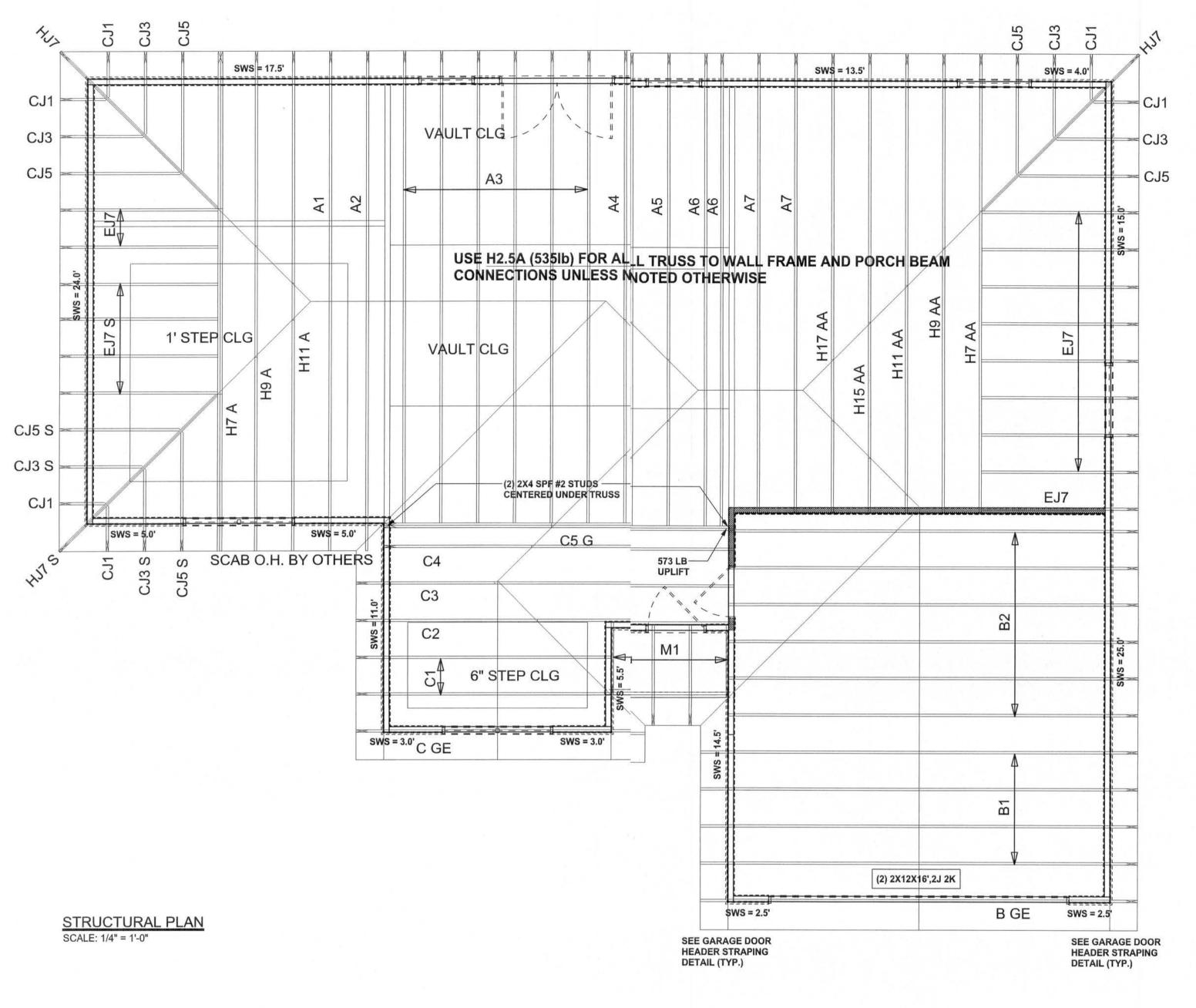
FINALS DATE: 01 / Mar / 07

> JOB NUMBER: 702263 DRAWING NUMBER

> > S-2 OF 6 SHEETS

REVISIONS

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS

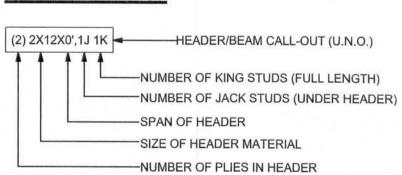
- SN-3 ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

\$\text{t=======1} \frac{1}{2}	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALLS
IBW	2ND FLOOR INTERIOR BEARING WALLS

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	29.5'	95.0'
LONGITUDINAL	28.2'	56.0'

VINDLOAD ENGINEER: Mark Disosway, IE No.53915, POB 868, Lake City, FL 2056, 386-754-5419

IIMENSIONS: tated dimensions supercede scaled imensions. Refer all questions to lark Disosway, P.E. for resolution. Io not proceed without clarification.

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MARK DISOSWAY P.E. 53915

Wade Willis Construction

Spec House Lot 39 Crosswinds S/D

> ADDRESS: Lot 39 Crosswinds S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE
March 15, 2007

DRAWN BY: STRU

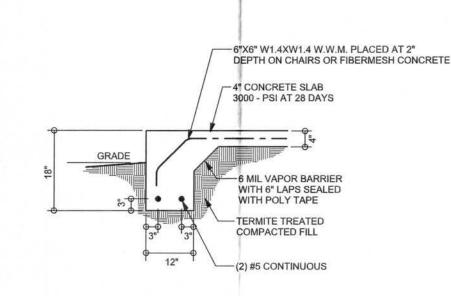
RAWN BY: STRUCTURAL BY David Disosway

FINALS DATE: 01 / Mar / 07

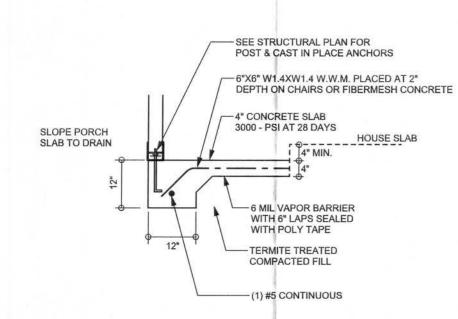
> JOB NUMBER: 702263 DRAWING NUMBER

> > S-3 OF 6 SHEETS

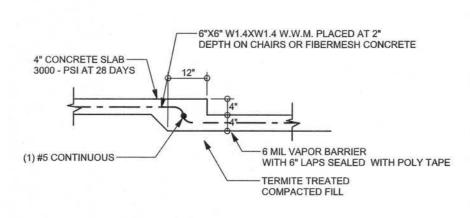
CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #7-075



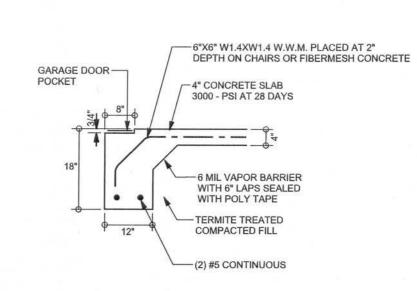
MONOLITHIC FOOTING SCALE: 1/2" = 1'-0"



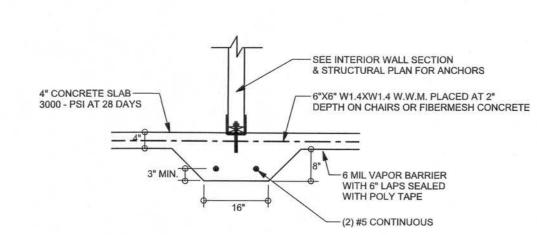
F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



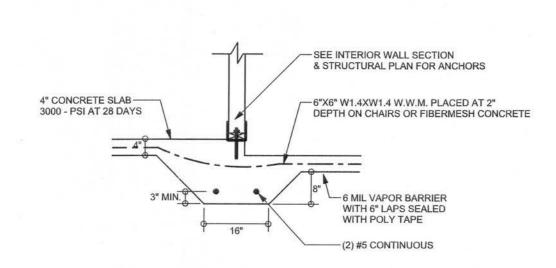
TYPICAL NON - BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



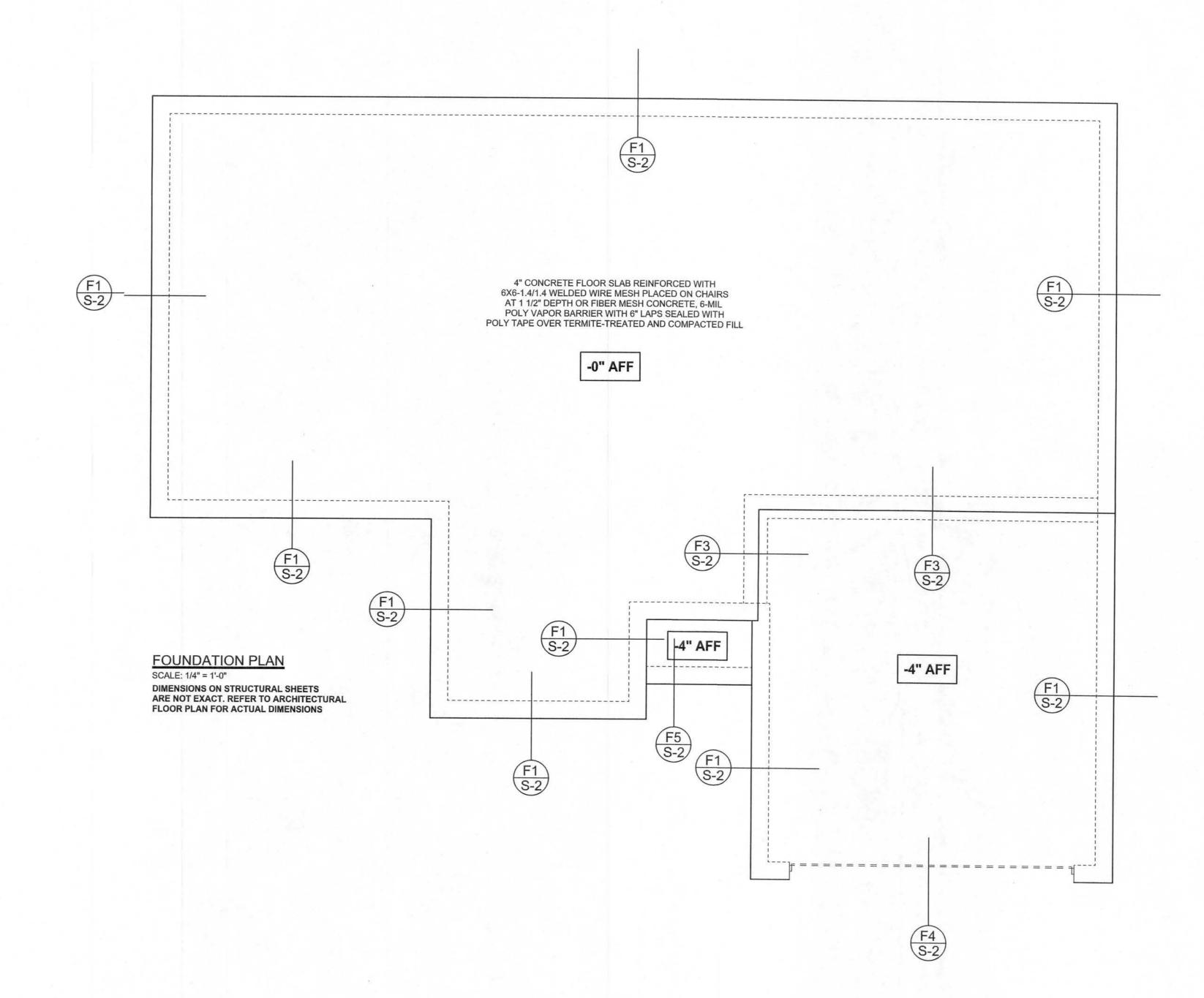
GARAGE DOOR FOOTING



INTERIOR BEARING FOOTING S-2 SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

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Wade Willis Construction

Spec House Lot 39 Crosswinds S/D

ADDRESS: Lot 39 Crosswinds S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March 22, 2007 DRAWN BY: STRUCTURAL BY: David Disosway

FINALS DATE: 01 / Mar / 07 JOB NUMBER: 702263

> DRAWING NUMBER **S-2**

OF 6 SHEETS