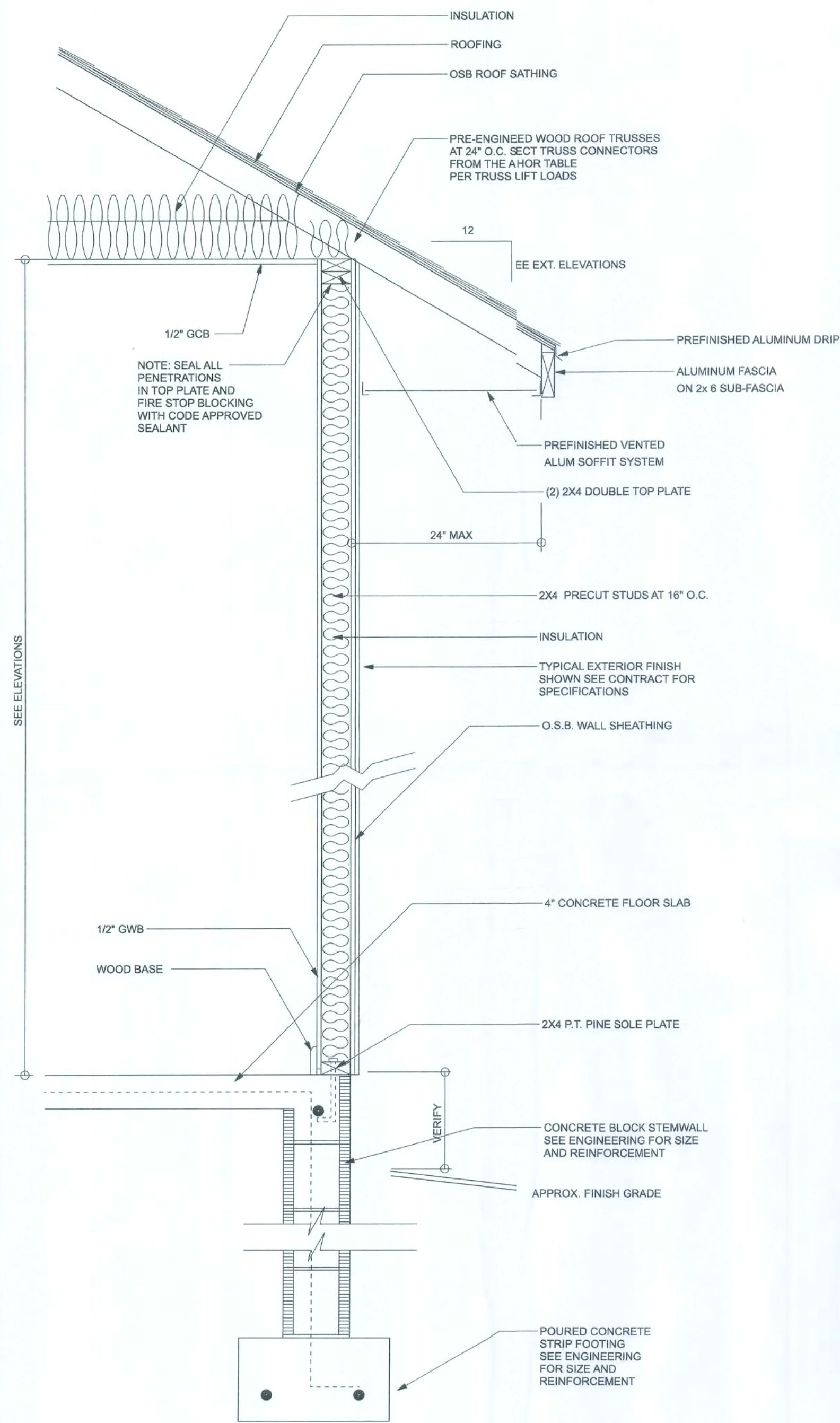
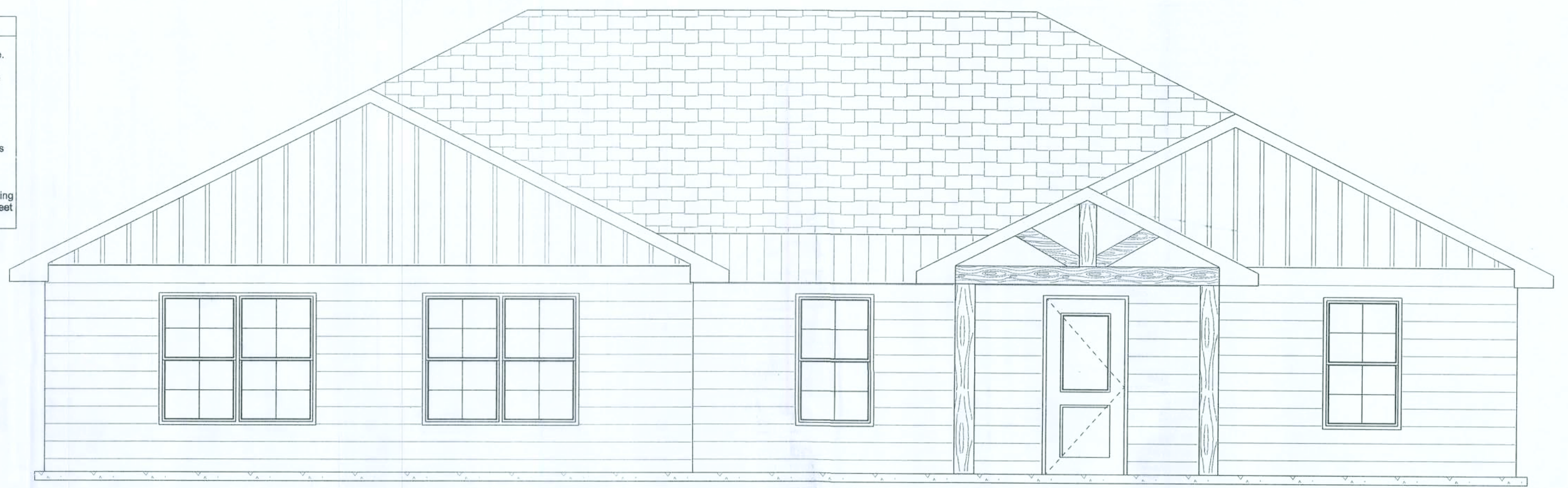


ROOF VENTILATION:
R606.2 Minimum vent area.
The minimum net free ventilating area shall be 1/150 of the area of the ventilated space.
Exception: The minimum net free ventilating area shall be 1300 of the ventilated space provided one or more of the following conditions are met:
1. In Climate Zones 6, 7 and 8, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.
2. At least 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space.
Upper ventilators shall be located no more than 3 feet below the ridge or highest point of the space, measured vertically, with the balance of the required ventilating provided by eave or cornice vents. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet below the ridge or highest point of the space shall be permitted.



**TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA**

SCALE: 1" = 1'-0"



Front Elevation



Rear Elevation



Left Elevation

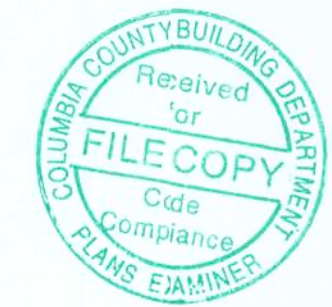


Right Elevation

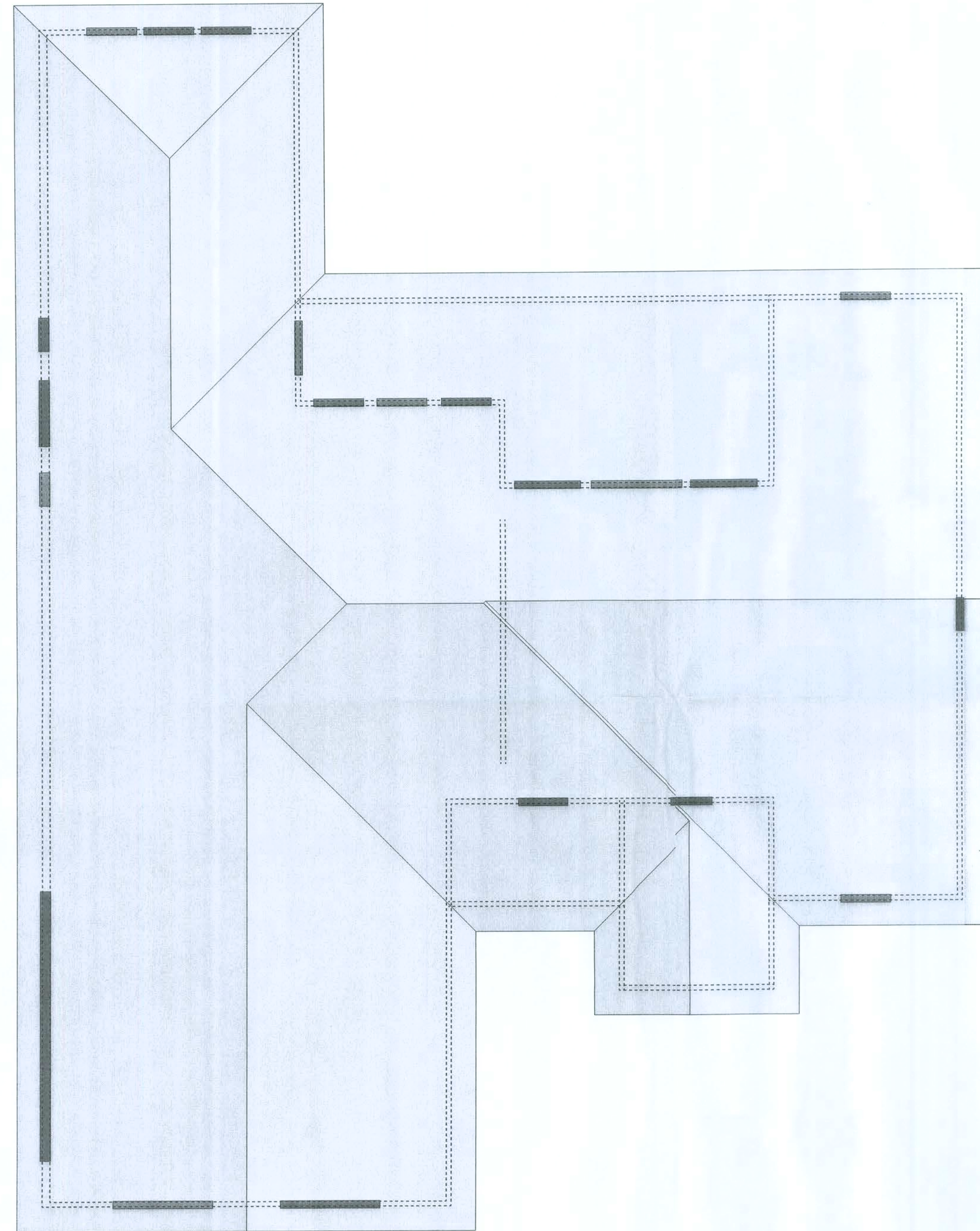
Cornerstone Development

11 Leather Model - Lot 9 Olunchenge

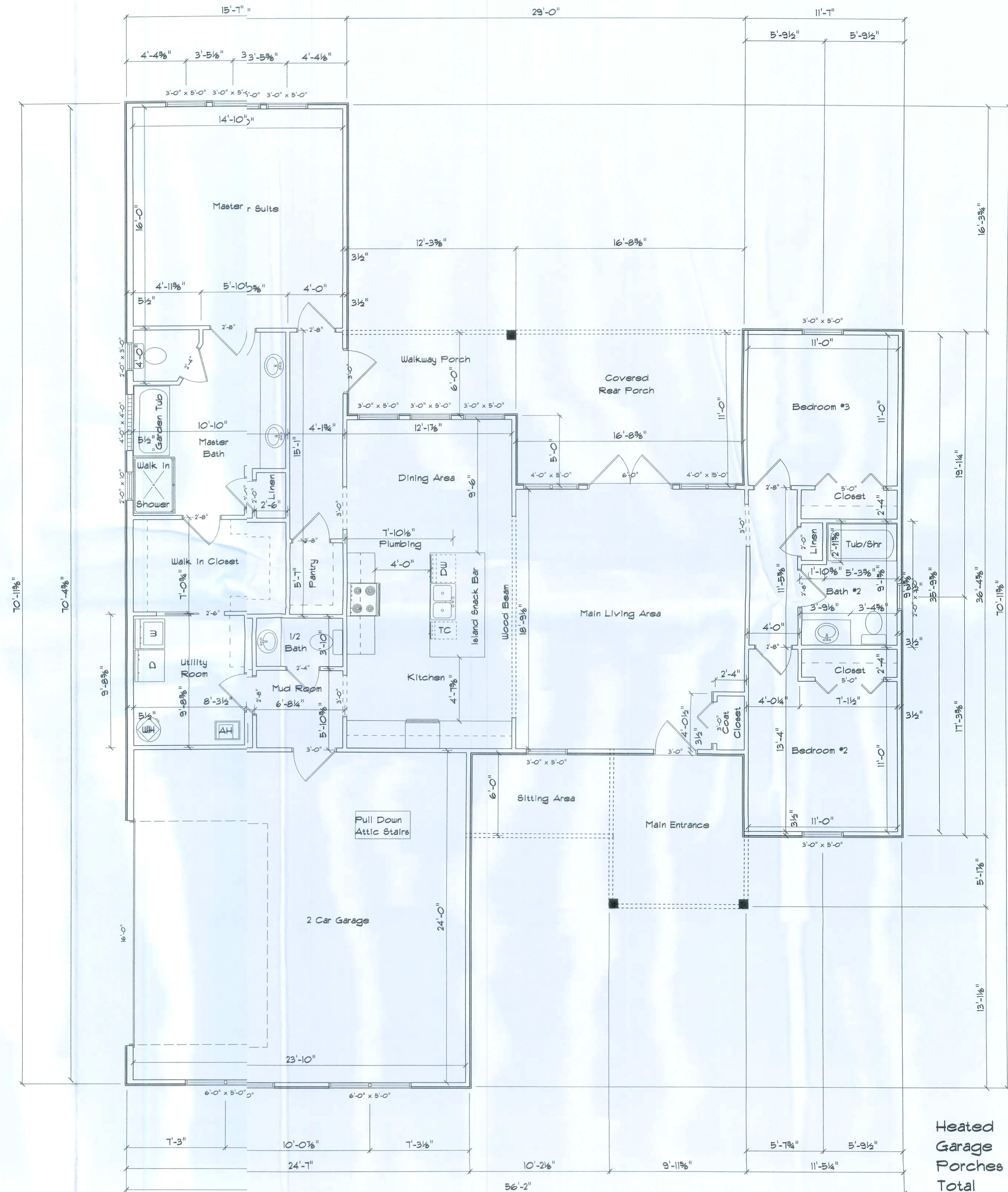
PROJECT ADDRESS:
Lot 9 Sonenhege
Columbia County, FL



Cornerstone Development
386.552.2690
chris@hplumbing.net



Roof Plan



Floor Plan

Cornerstone Development

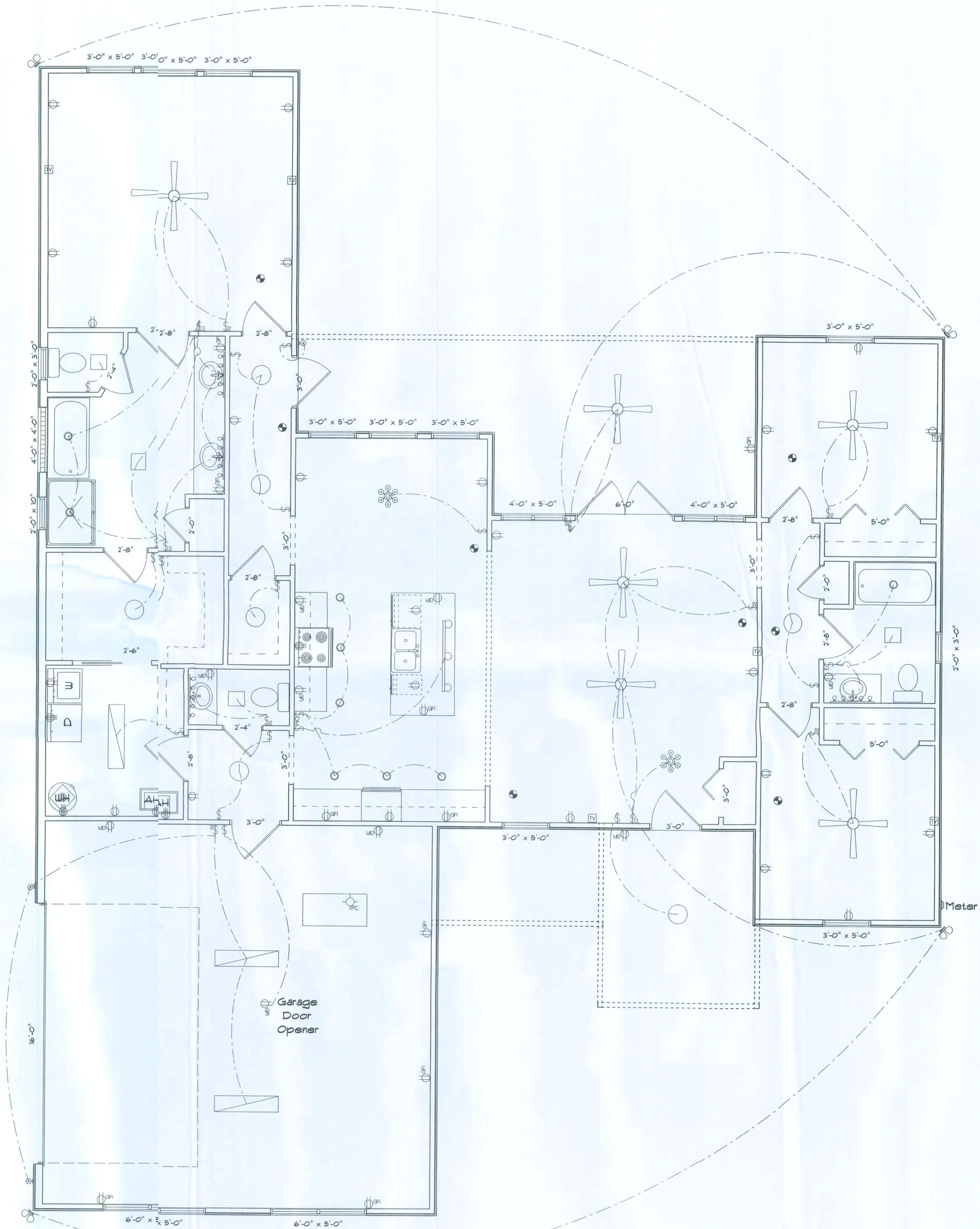
Heather Model - Lot 9 Stonehenge

PROJECT ADDRESS:
Lot 9 Stonehenge
Columbia County, FL

Cornerstone Development
386.7522690
chris@hplumbing.net

ELECTRICAL PLAN NOTES:	
E - 1	WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
E - 2	CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
E - 3	ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
E - 4	ALL SMOKE DETECTORS SHALL BE 120V BATTERY BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE INTERLOCKED TOGETHER. INSTALL IDE AND NEAR ALL BEDROOMS.
E - 5	TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER OWNER'S DIRECTIONS, & IN ACCORDANCE WITH APPLICABLE SECTIONS OF NEC LATEST EDITION.
E - 6	ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
E - 7	ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
E - 8	ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT: FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLOR, LIBRARIES, DEN, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
E - 9	ALL OUTLETS TO BE LOCATED ABOVE FINISHED FLOOR ELEVATION.
E - 10	A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE WHERE CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MUST BE LOCATED INSIDE OF THE BUILDING WITH SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
E - 11	CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL-FUEL-BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
E - 12	ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.
E - 13	A MINIMUM OF 75% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC E.C. SEC. R404.1

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2x4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM



Electrical Plan

Cornerstone Development

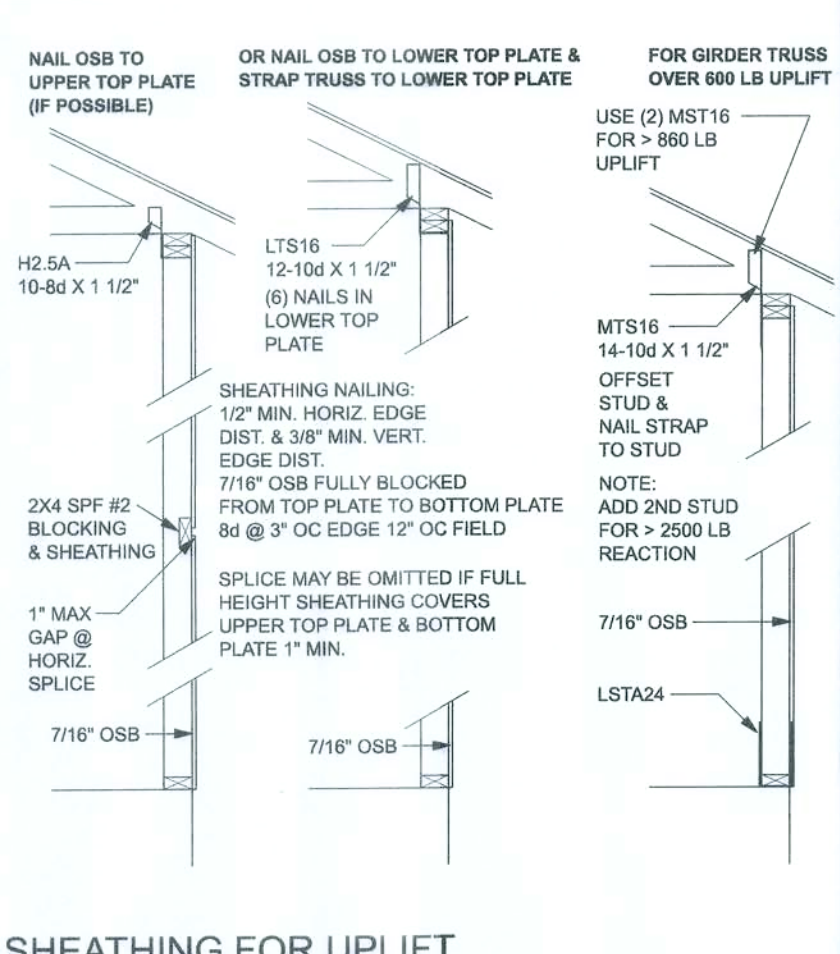
Heather Model - Lot 9 Stonehenge

PROJECT ADDRESS:
Lot 9 Stonehenge
Columbia County, FL

Cornerstone Development
386.752.2690
chris@hplumbing.net

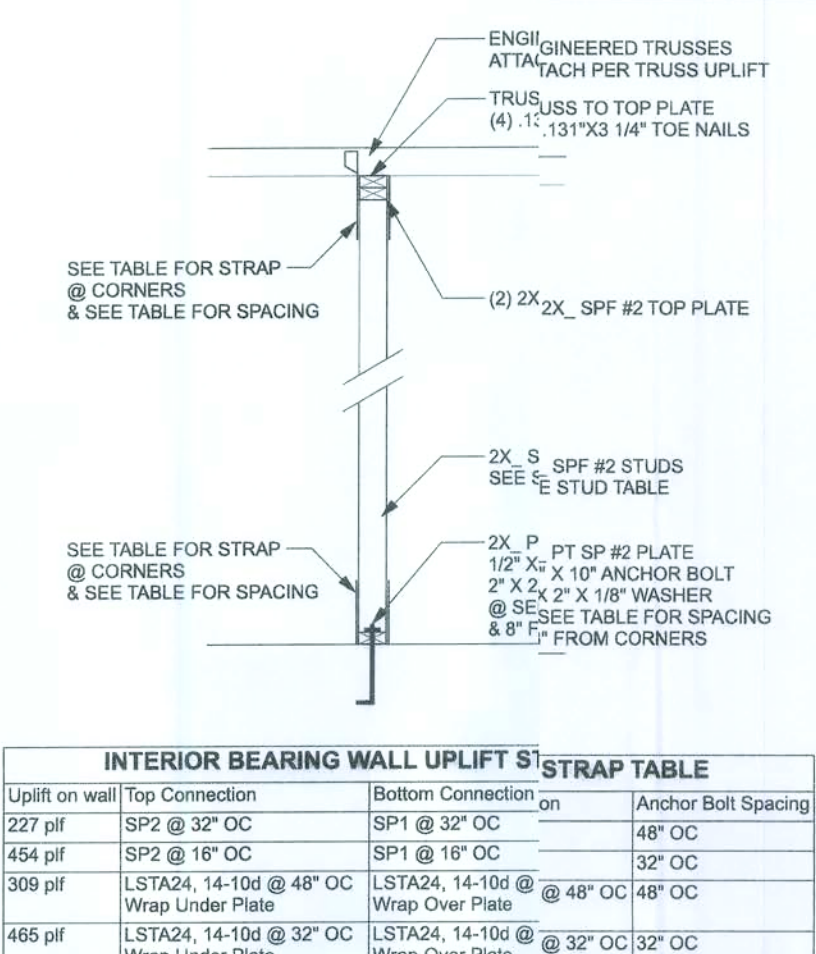
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing in the panel
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	12"
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6"
120 mph Exp. D	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6"
130 mph Exp. C	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
130 mph Exp. D	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6"
140 mph Exp. C	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
140 mph Exp. D	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
150 mph Exp. C	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"
150 mph Exp. D	1/2"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6"

Note: For sheathing located a minimum of 4 feet from the perimeter edge of shed, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 8 inches on center along intermediate supports in the panel field. Note: Table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS

ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

Uplift SP# Uplift SP#	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	
415	290	H3	4-8x1 1/2"
575	465	H2 SA	5-8x1 1/2"
1340	1015	H10A	9-10x1 1/2"
720	620	LTS12-20	6-10x1 1/2"
1000	860	MTS12-30	7-10x1 1/2"
1450	1245	HTS20-30	12-10x1 1/2"
Uplift SP# Uplift SP#	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10x1
1640	1455	MSTA24	9-10x1
1030	1030	CS20	7-10x1
Uplift SP# Uplift SP#	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10x1
1065	605	SP2	6-10x1
771	771	LSTA24	10-10x1
1235	1235	LSTA24	14-10x1
Uplift SP# Uplift SP#	Holdowns @ Stemm	To Stud / Post	Anchor
1685	1800	DT122	8-SDS 1/4"x1 1/2"
4235	3540	HTT4	18-16x2 1/2"
Uplift SP# Uplift SP#	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"
4235	3540	HTT4	18-16x2 1/2"
Uplift SP# Uplift SP#	Post Bases @ Stemm	To Post	Anchor
1900	1900	ABU442	12-16x1
2475	2475	ABU662	12-16x1
Uplift SP# Uplift SP#	Post Bases @ Mono	To Post	Anchor
1900	1900	ABU442	12-16x1
2475	2475	ABU662	12-16x1

EXTERIOR WALL STUD TABLE FOR SP#2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.2.0B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 9.8' x 12.8' O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

Grade	Species	Fd	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PAROLAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCL TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR DESIGN. PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR DESIGN. PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR DESIGN. PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR DESIGN.

FOUNDATION: FOUNDATION DESIGN AND SITE CONDITIONS MUST BE CONFIRMED BY THE FOUNDATION DESIGNER. FOUNDATION DESIGNER SHALL PROVIDE FOUNDATION DESIGN AND SITE CONDITIONS MUST BE CONFIRMED BY THE FOUNDATION DESIGNER. FOUNDATION DESIGNER SHALL PROVIDE FOUNDATION DESIGN AND SITE CONDITIONS MUST BE CONFIRMED BY THE FOUNDATION DESIGNER.

WELDED WIRE REINFORCED SLAB: 8" x W14 x W14, FB = 80KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 10 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116, SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10 FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO DISCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 40 KSI, ALL LAP SPICES 40" DB (25" FOR #6 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, UNO, U.S.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

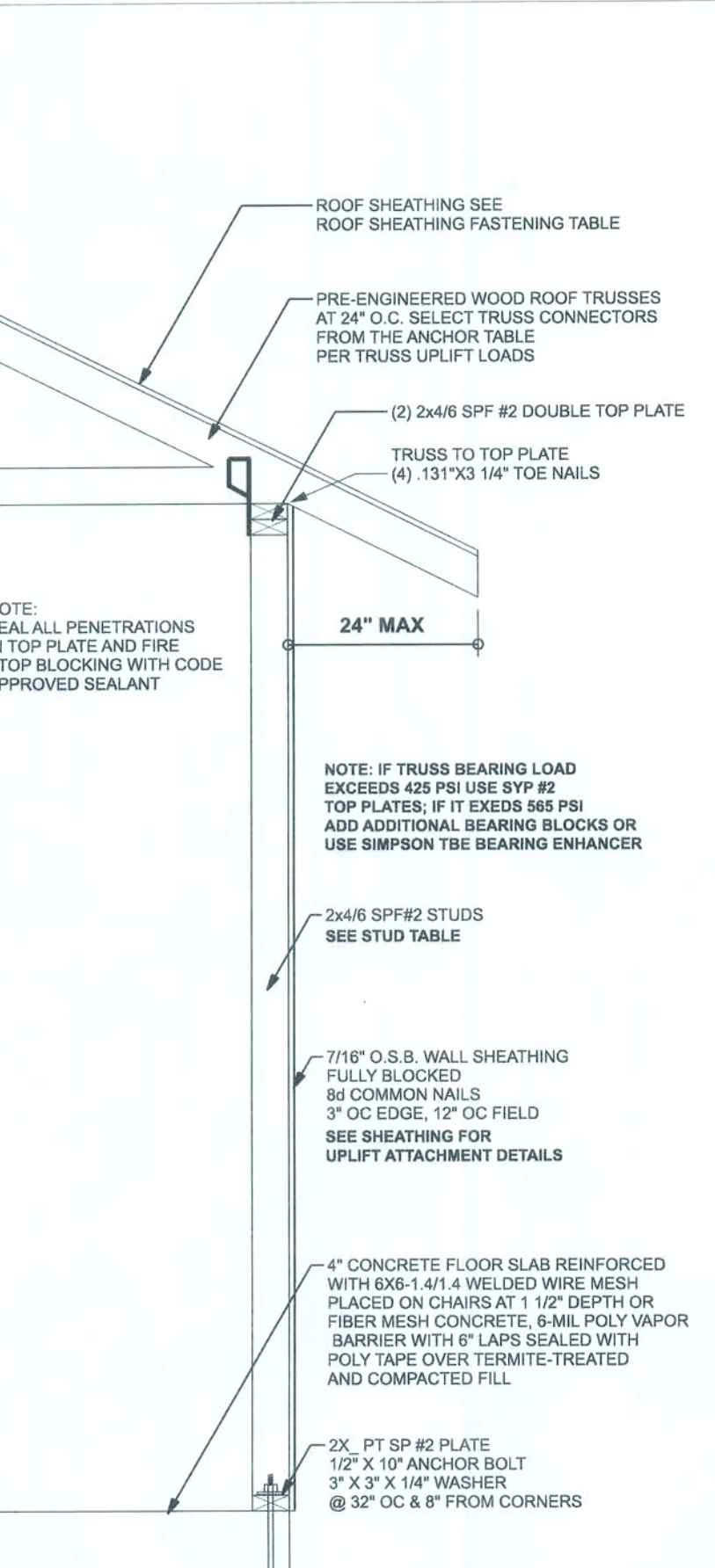
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FIBER REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCL, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCL REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESISTANT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF SYSTEM BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

(TYP.) GABLE BRACING DETAIL

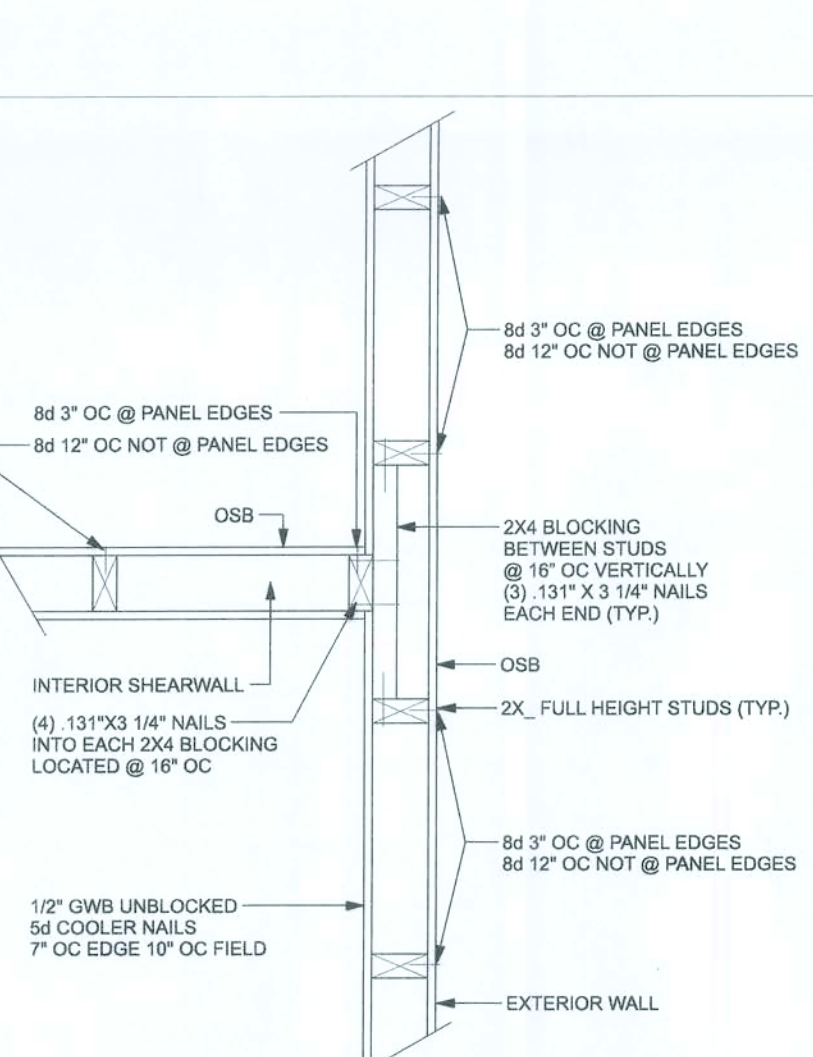
WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME

(TYP.) PORCH POST

ONE STORY WOOD



(TYP.) INTERSECTING WALL FRAMING

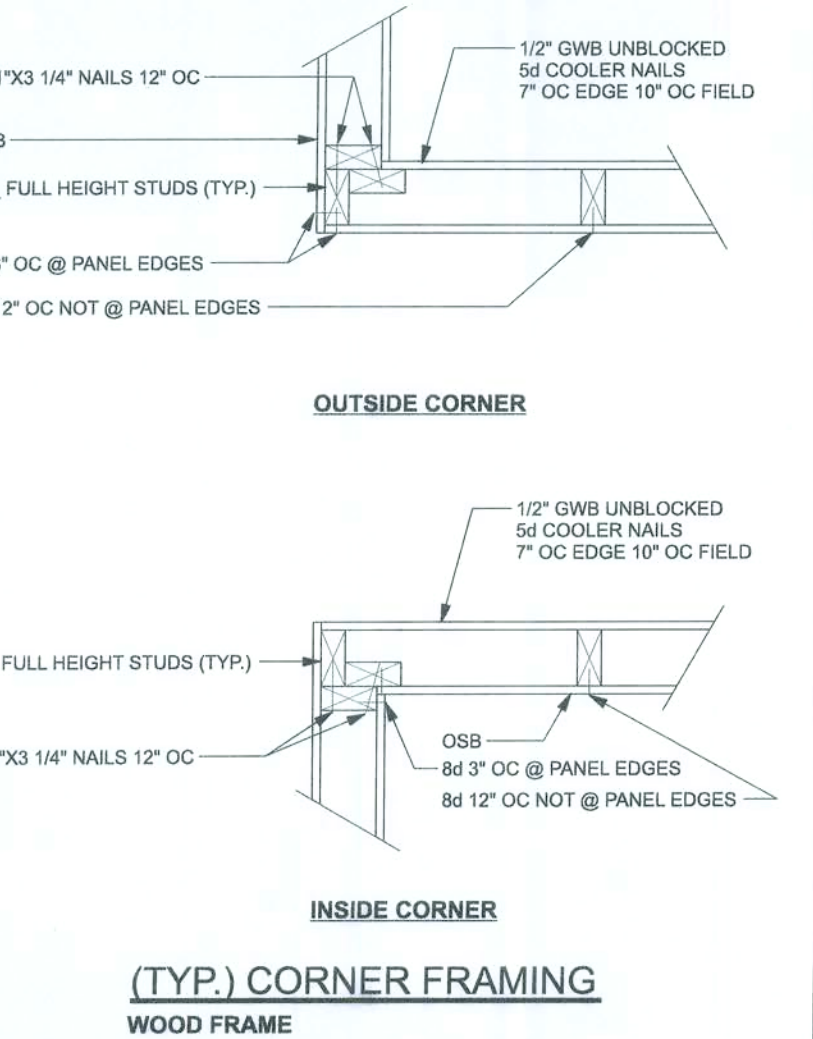
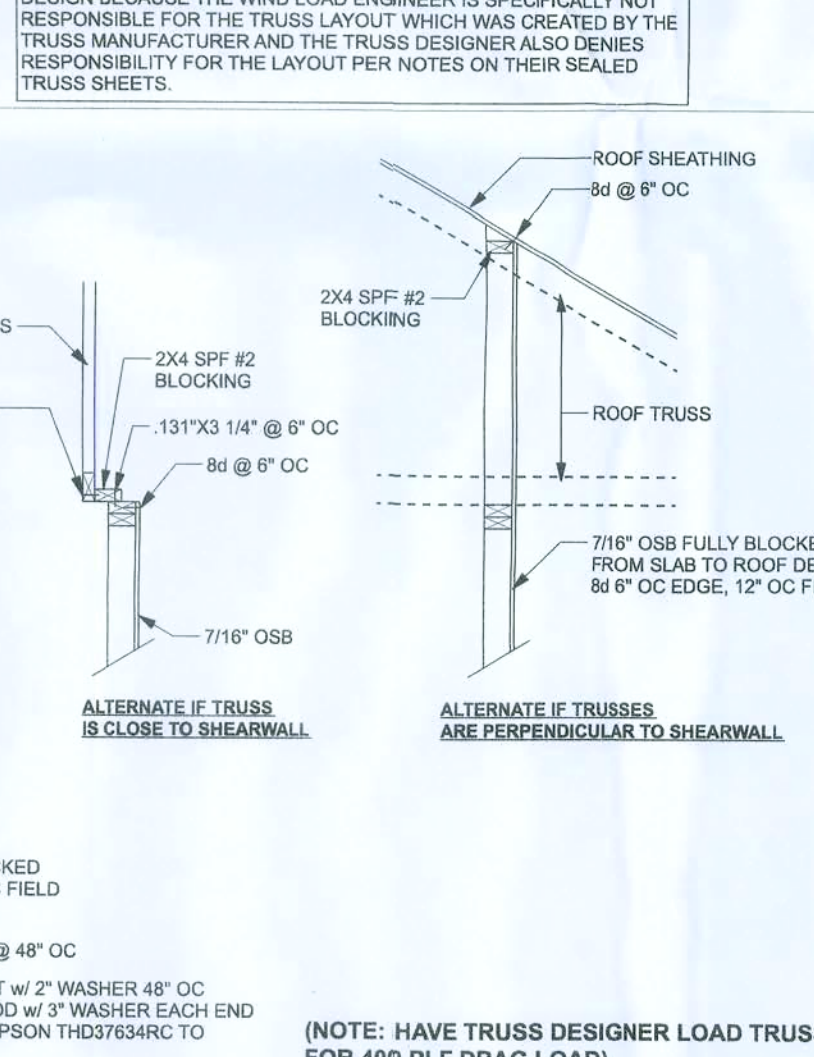
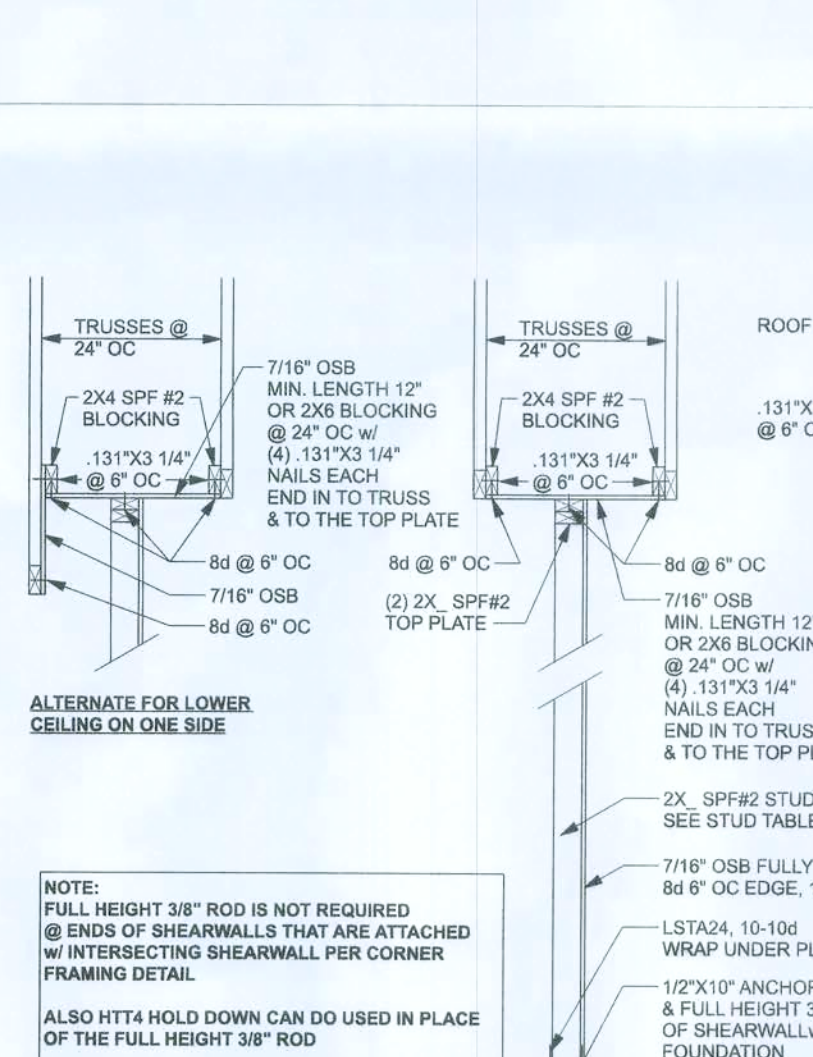
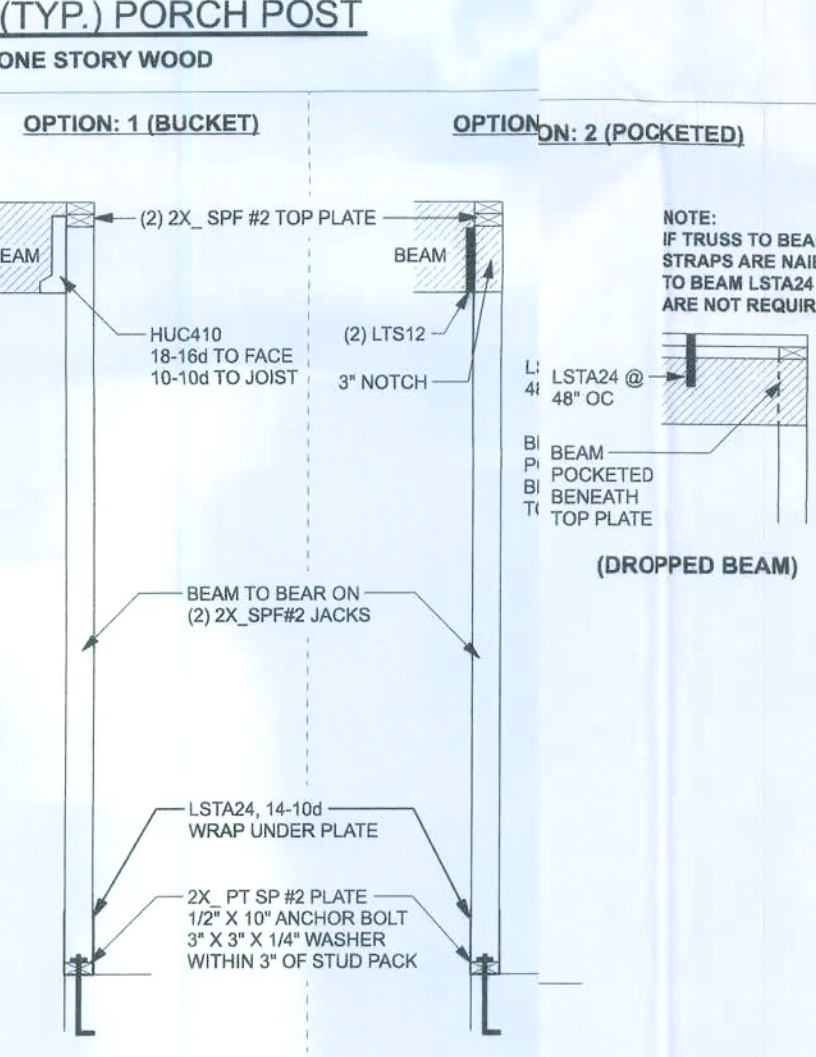
WOOD FRAME

TYPICAL HEADER STRAPPING DETAIL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

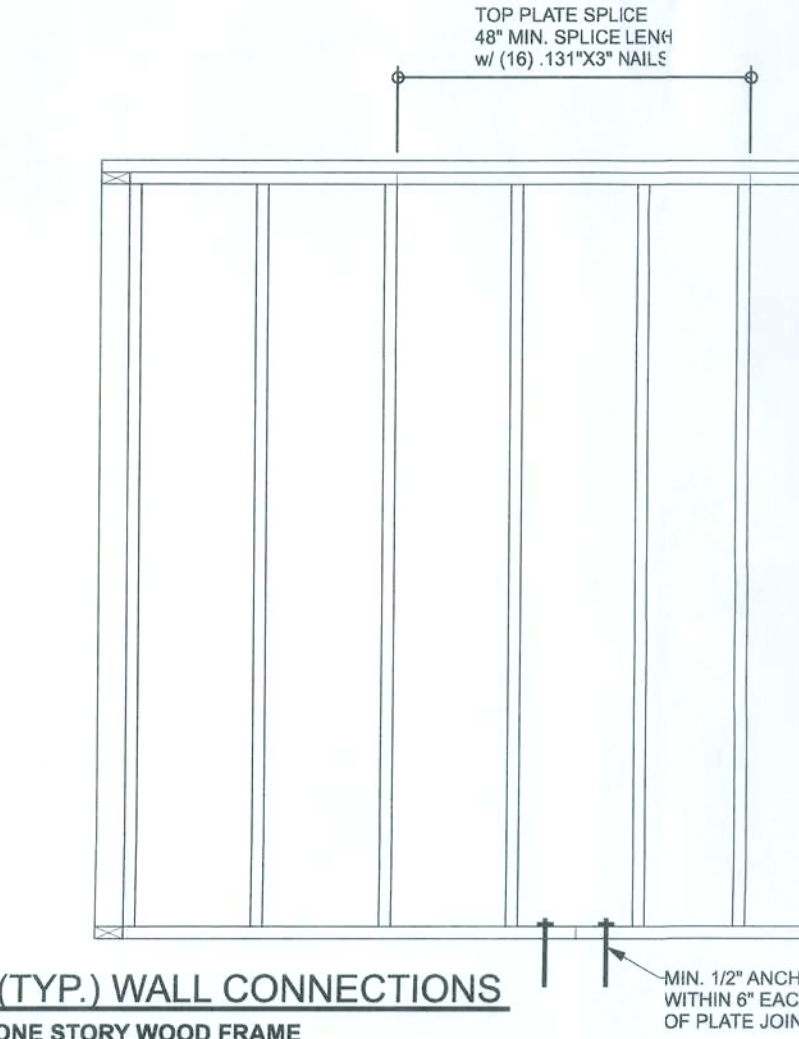
(TYP.) BEAM TO WALL

WOOD FRAME w/ STRAPS & ANCHORS



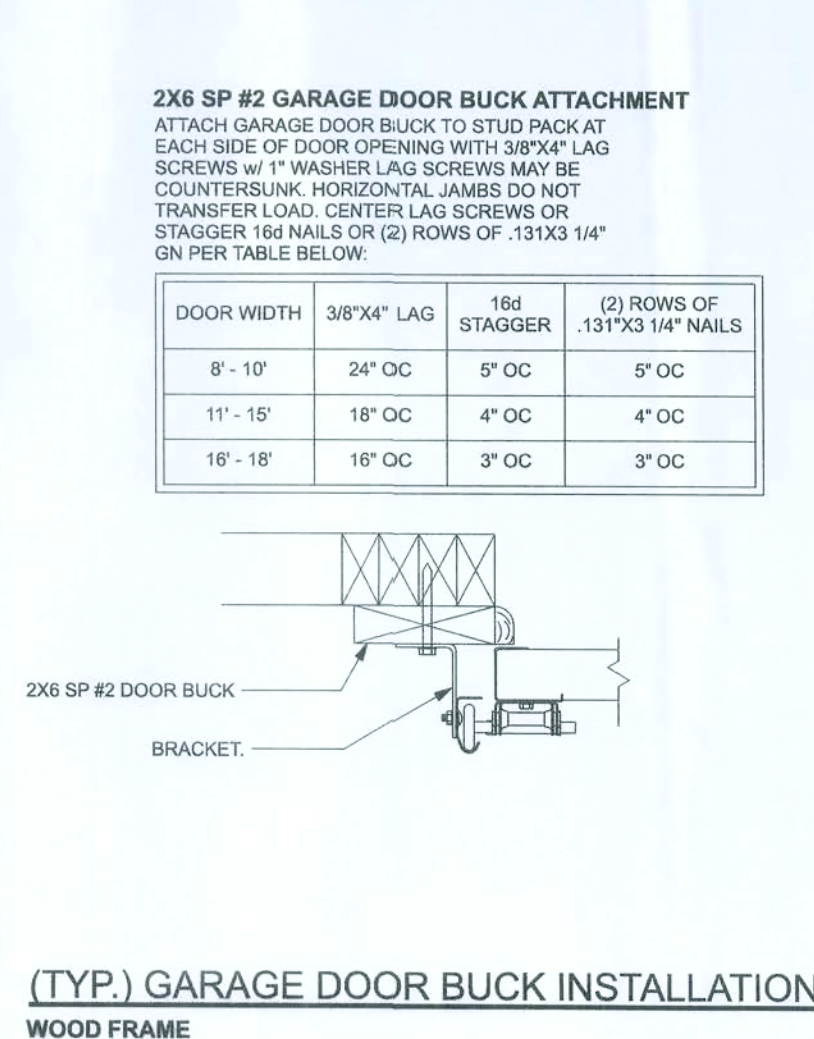
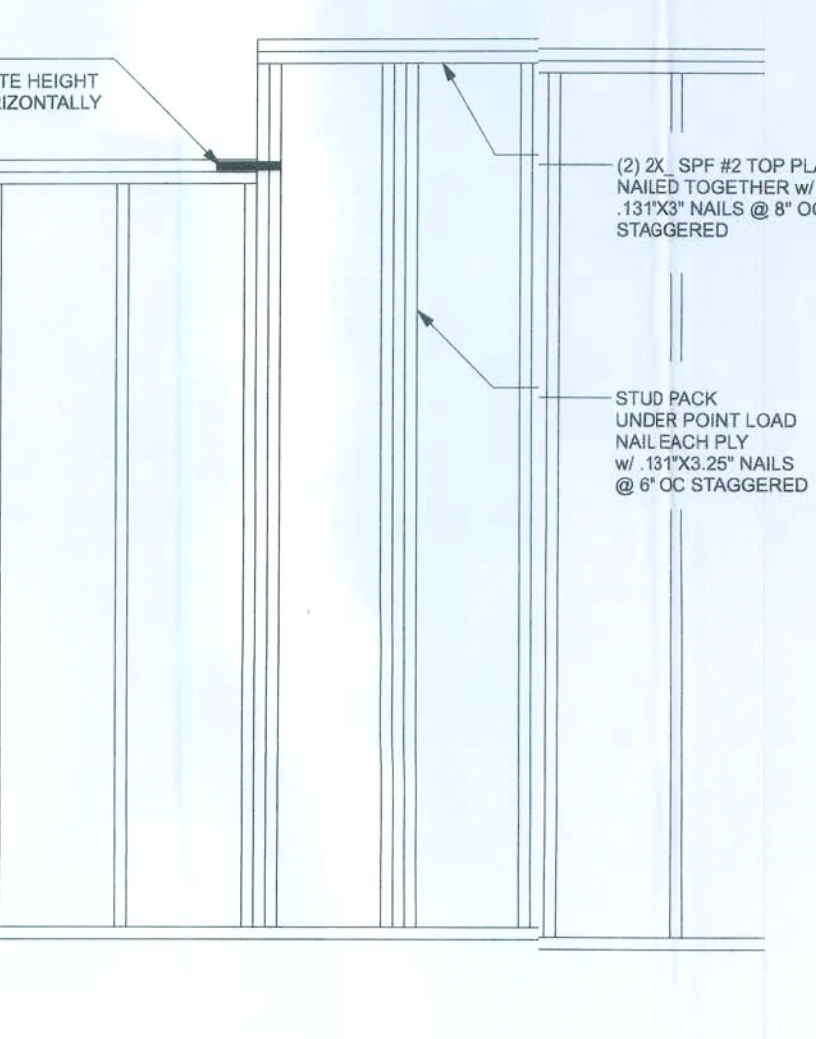
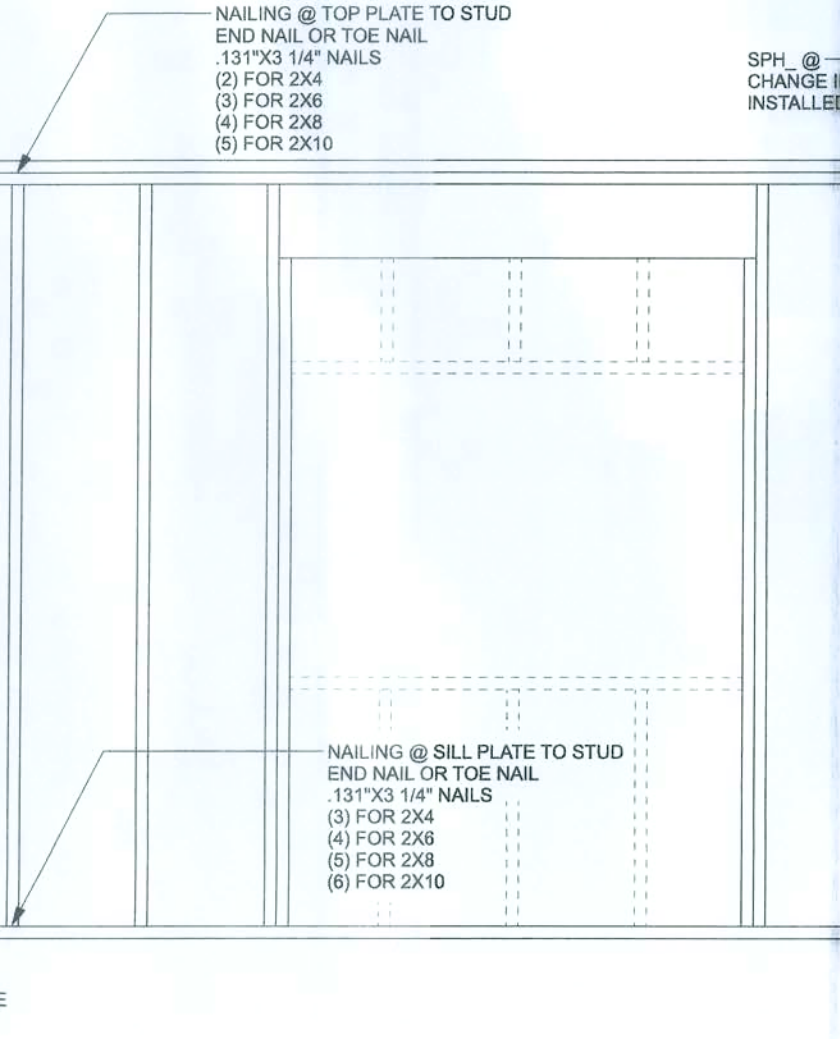
(TYP.) CORNER FRAMING

WOOD FRAME



(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

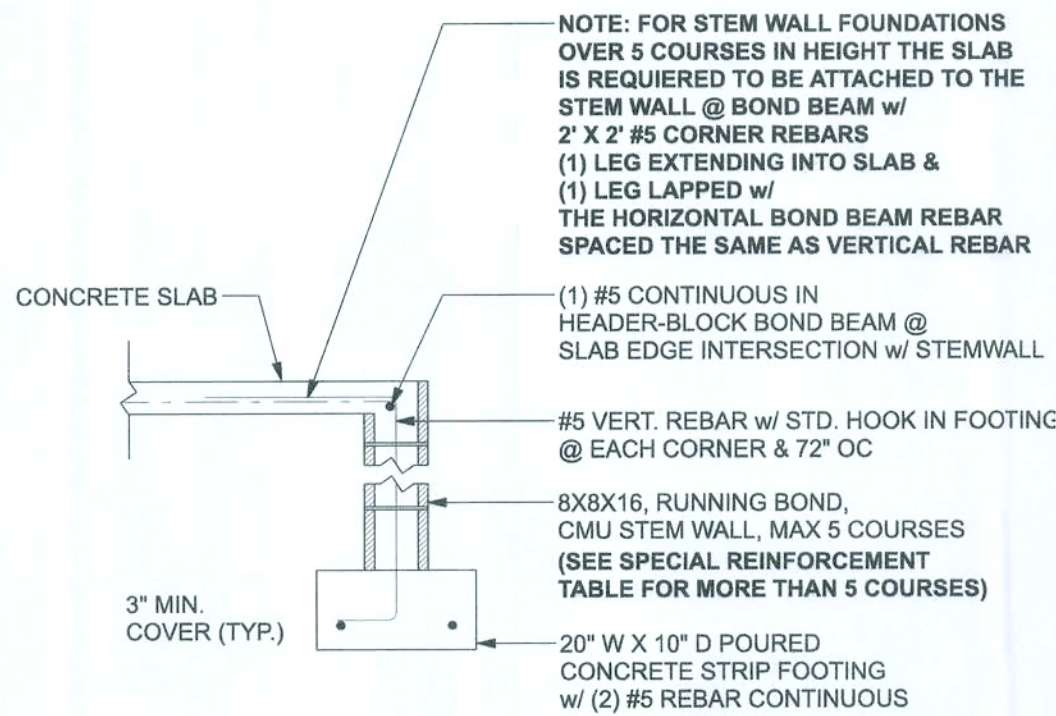


(TYP.) GARAGE DOOR BUCK INSTALLATION

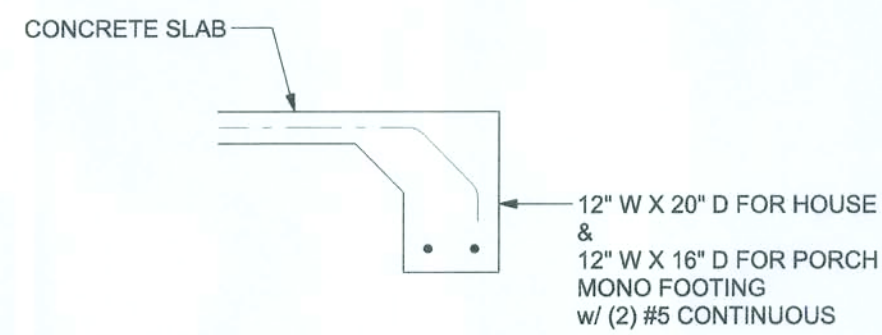
WOOD FRAME

DESIGN CRITERIA & LOADS:	
BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	B
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	1
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

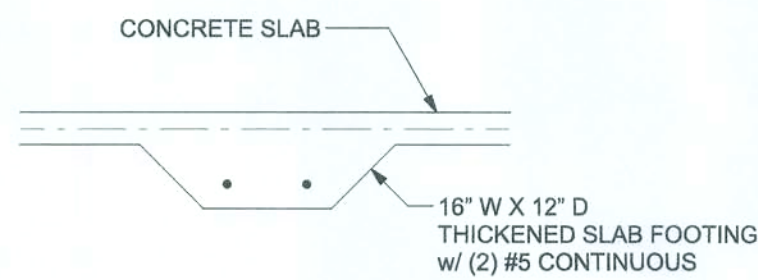
COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP B)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
EFFECTIVE WIND AREA (Ft2)		
0 - 20	+25.6(Vult)	-27.8(Vult)
0 - 20	+42.6(Vult)	-46.2(Vult)
	+25.6(Vult)	+42.6(Vult)
	-27.8(Vult)	-46.2(Vult)
	-46.2(Vult)	-57.1(Vult)
	-57.1(Vult)	-68.0(Vult)
	-68.0(Vult)	-78.9(Vult)
	-78.9(Vult)	-89.8(Vult)
	-89.8(Vult)	-100.7(Vult)
	-100.7(Vult)	-111.6(Vult)
	-111.6(Vult)	-122.5(Vult)
	-122.5(Vult)	-133.4(Vult)
	-133.4(Vult)	-144.3(Vult)
	-144.3(Vult)	-155.2(Vult)
	-155.2(Vult)	-166.1(Vult)
	-166.1(Vult)	-177.0(Vult)
	-177.0(Vult)	-187.9(Vult)
	-187.9(Vult)	-198.8(Vult)
	-198.8(Vult)	-209.7(Vult)
	-209.7(Vult)	-220.6(Vult)
	-220.6(Vult)	-231.5(Vult)
	-231.5(Vult)	-242.4(Vult)
	-242.4(Vult)	-253.3(Vult)
	-253.3(Vult)	-264.2(Vult)
	-264.2(Vult)	-275.1(Vult)
	-275.1(Vult)	-286.0(Vult)
	-286.0(Vult)	-296.9(Vult)
	-296.9(Vult)	-307.8(Vult)
	-307.8(Vult)	-318.7(Vult)
	-318.7(Vult)	-329.6(Vult)
	-329.6(Vult)	-340.5(Vult)
	-340.5(Vult)	-351.4(Vult)
	-351.4(Vult)	-362.3(Vult)
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	-460.4(Vult)	-471.3(Vult)
	-471.3(Vult)	-482.2(Vult)
	-482.2(Vult)	-493.1(Vult)
	-493.1(Vult)	-504.0(Vult)
	-504.0(Vult)	-514.9(Vult)
	-514.9(Vult)	-525.8(Vult)
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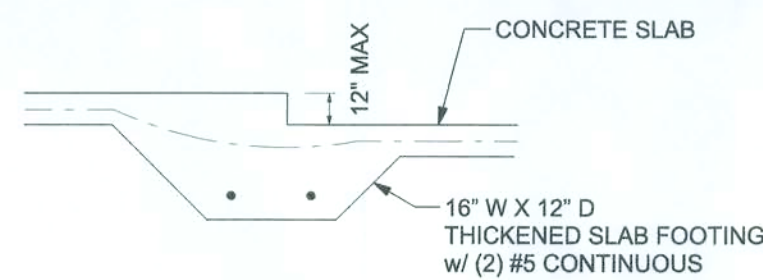
F1 S-2 OPTIONAL STEM WALL FOOTING



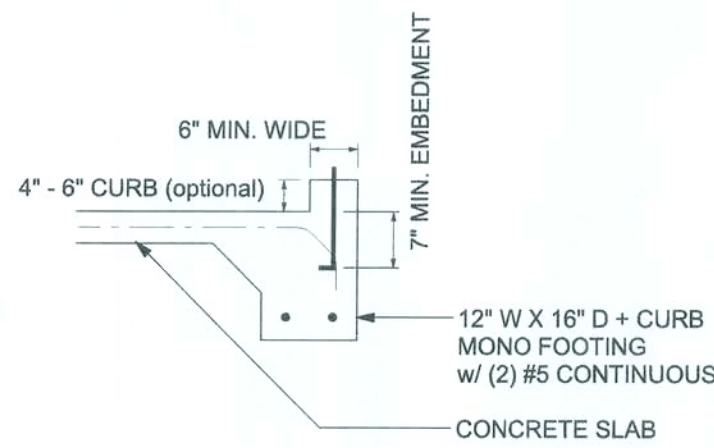
F1 S-2 MONOLITHIC FOOTING



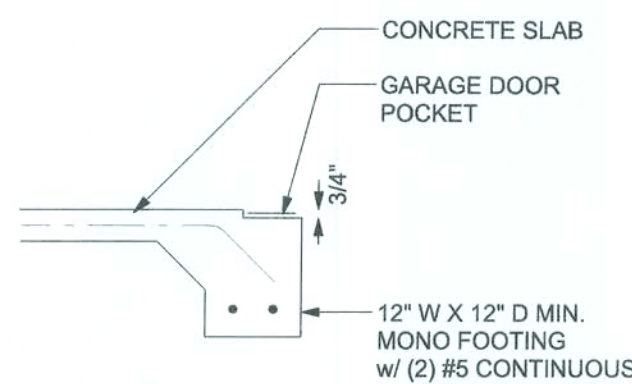
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:

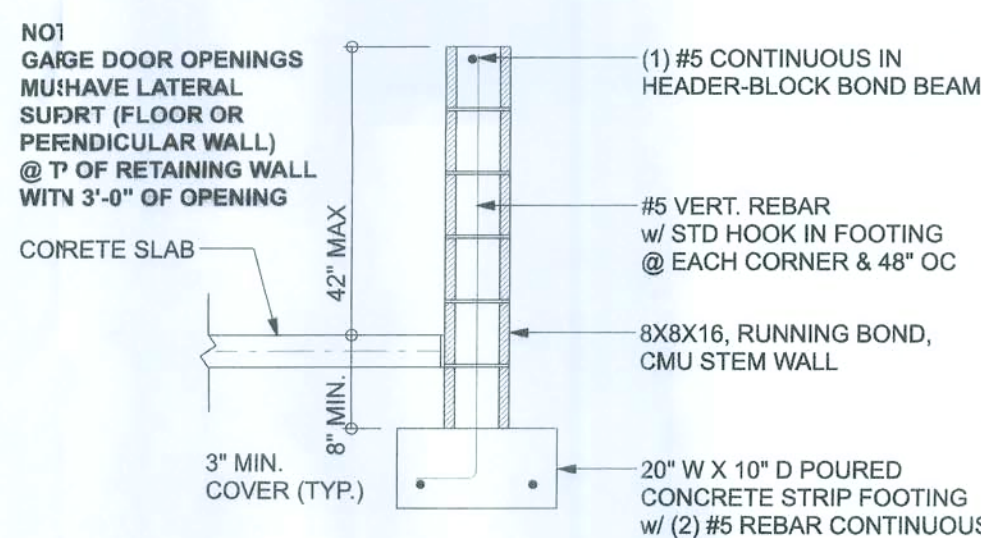
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

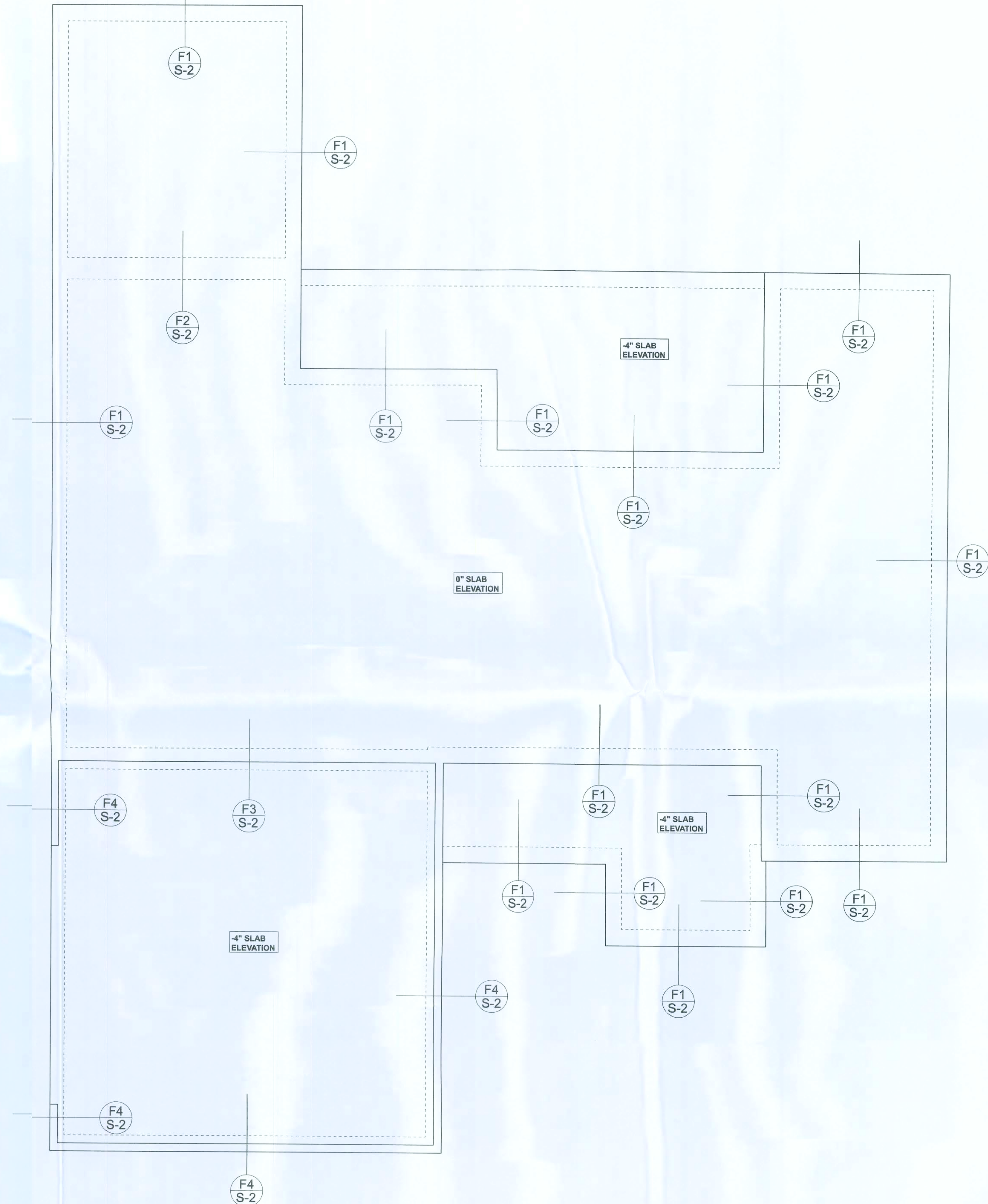
MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls $P_m = 1500$ psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.25"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, $F_y = 40$ ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A625, Class G80, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.00 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



F4 S-2 OPTIONAL STEM WALL CURB FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN-1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN-2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING WALLS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN-3 THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-1/4" WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES-TREATMENT METHOD CAN BE USED INSTEAD)

Cornerstone Development

Heather Model - Lot 9 Stonehenge

PROJECT ADDRESS:
Lot 9 Stonehenge
Colman County, FL

DIMENSIONS:
Stated dimensions supersede called dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to civil engineering, comply with the 7th Edition Florida Building Code Residential (2022) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY, P.E. 53915



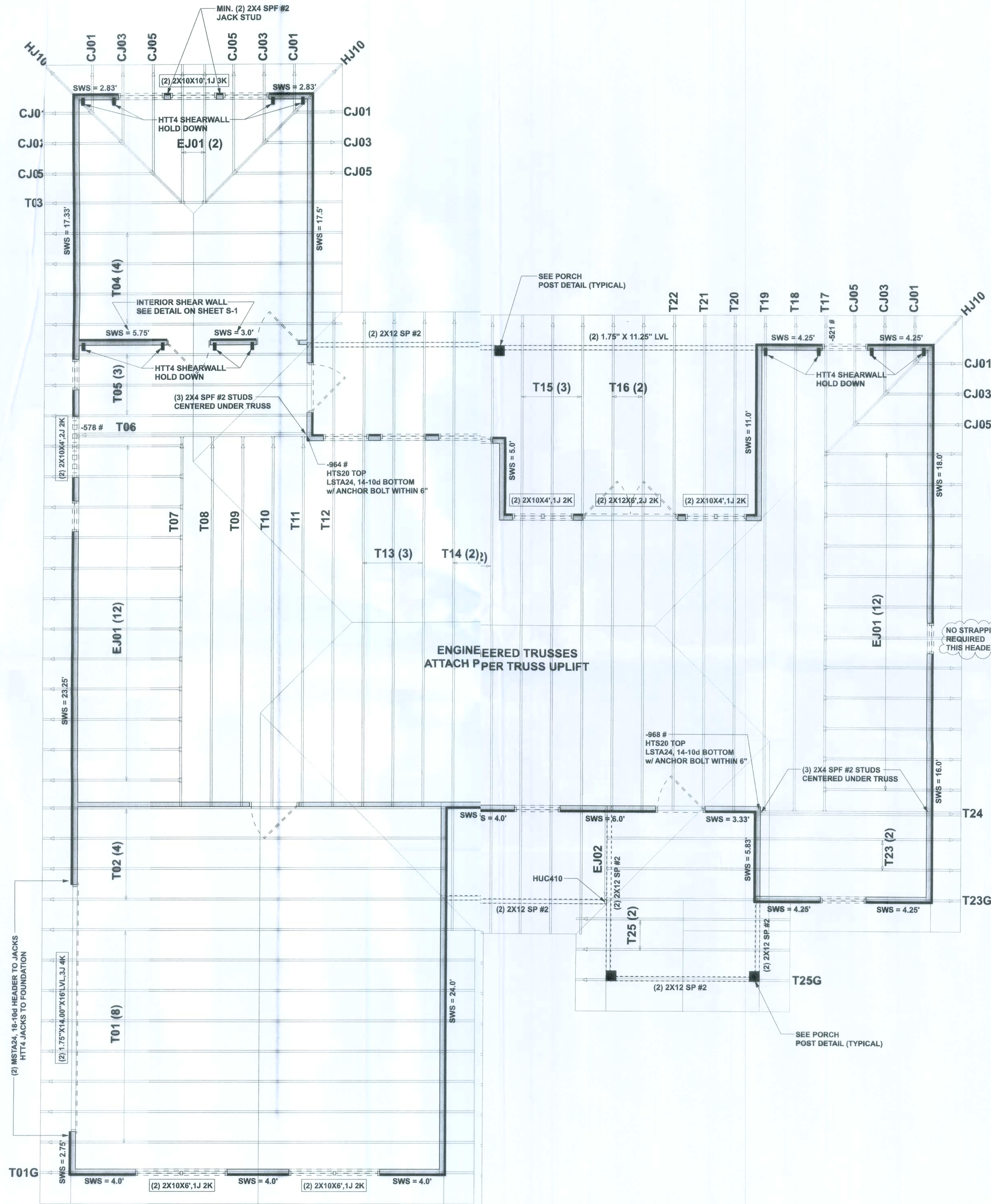
Wednesday, July 21 2021

Mark Disosway P.E.
163 SW Midtown Place
Suite 101
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
211048

S-2

OF 3 SHEETS

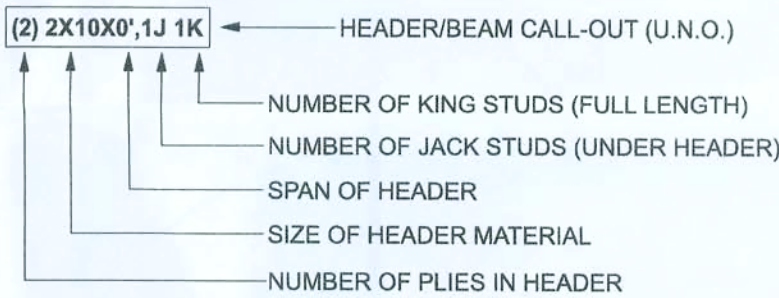


STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-51, BCSI-52, & BCSI-83. BCSI-51, BCSI-52, & BCSI-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	33758 LBF	13617 LBF
REQUIRED	8652 LBF	10422 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2620753

Cornerstone Development

Heather Model - Lot 9 Stonehenge

PROJECT ADDRESS:
100 Stonehenge
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede said dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to civil engineering, comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Wednesday, July 21, 2021

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386.754.549
disoswaydesign@gmail.com

JOB NUMBER:
211048

S-3

OF 3 SHEETS