

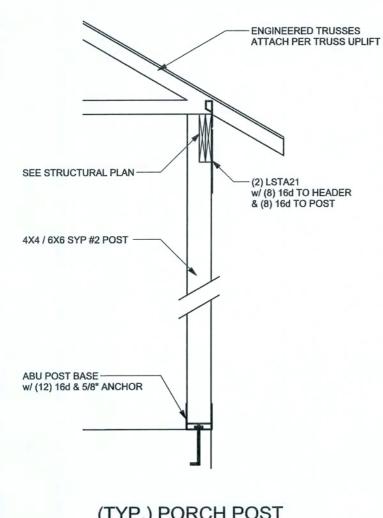
ONE STORY WALL SECTION

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

(TYP.) INTERSECTING WALL FRAMING



ONE STORY WOOD

—SPH4/6 ALL OPENINGS (U.N.O.)—

CRIPPLES IF REQUIRED

-WINDOW SILL PLATE-

(PER TABLE BELOW) TOE NAIL ENDS OF EACH PLY W/

 $2x4 = (4) .131" \times 3.25" NAILS$

-2x6 = (6) .131" x 3.25" NAILS -

TYPICAL STRAPPING (U.N.O.)

-SPH4/6 ALL OPENINGS (U.N.O.)

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6

TYPICAL HEADER STRAPING DETAIL

5'-3" 7'-9" 7'-8" 11'-4" FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE

TABLE A-3.23B

MAX. SPANS FOR SPF #2

(SEE STRUCTURAL PLAN)

SPH4/6 @ 48" O.C. (U.N.O.)

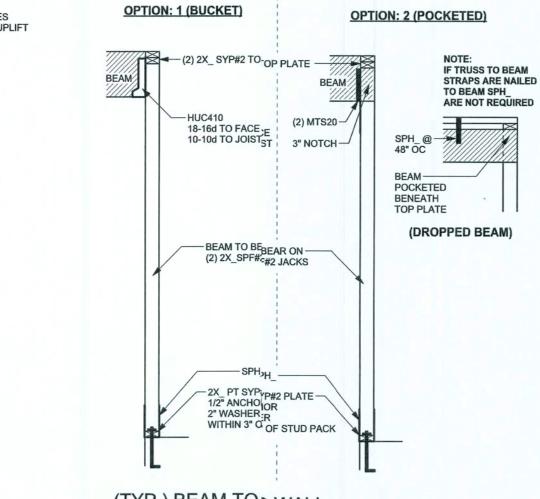
IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SPH4/6 @ 48" O.C.

ARE NOT REQUIRED

(6) .13x 3 1/4" GUN NAILS -

INTO KING STUD

TOE NAED THRU HEADER

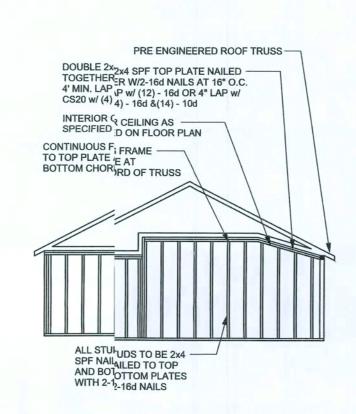


WOOD FRAME w/ STRAPS & ANCHORS

GRRADE & SPECIES TABLE

1265 LB

		Fb (psi)	E (10 ⁶ psi)
2x8 _{x8}	SYP #2	1200	1.6
2x1 _{k10}	SYP #2	1050	1.6
2x1 _{k12}	SYP #2	975	1.6
GLILB	24F-V3 SP	2400	1.8
LSISL	TIMBERSTRAND	1700	1.7
LVIVL	MICROLAM	1600	1.9
PSI _{SL}	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEILING DIAPHRAGM DETAIL** SCALE: N.T. T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" x 0" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	ER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR LLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	E CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND GHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	ERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 TS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE TH	ONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL AD ENGINEER IMMEDIATELY.
DESIGN, PL	RUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS EMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, IUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL CATIONS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

OFLIFT LDS. STP	UPLIFT LBS. SPF	IKUSS CONNECTOR	TOPLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12-100 1 1/2	12-100 1 1/2	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
			14-100	14-100	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
405		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone Effective Wind Area (ft2)

1 27.8 -30.5 25.3 -25.3 2 27.8 -35.7 25.3 -30.5

 2 O'hg
 -56.8
 -56.8

 3
 27.8
 -35.7
 25.3
 -30.5

 3 O'hg
 -95.6
 -59.3

 4
 30.5
 -33.0
 25.9
 -28.5

5 | 30.5 | -40.7 | 25.9 | -31.6

Doors & Windows 30.5 -40.7

x7 Garage Door | 27.3 | -32.0

6x7 Garage Door 25.9 -29.4

Worst Case

(Zone 5, 10 ft2)

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

.) BASIC WIND SPEED = 110 MPH

WIND IMPORTANCE FACTOR = 1.0

2.) WIND EXPOSURE = C

I DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

30 PSF (SLEEPING ROOMS)

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES

5.) MEAN ROOF HEIGHT = <30 FT

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS

ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%

MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT

SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS TO STUDS

PE No.53915, POB 88, Lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions sue dimensions. Refer all juestions to Mark Disosway, P.E. or resolution.

Do not proceed withot clarification.

WINDLOAD ENGINE:R: Mark Disosway,

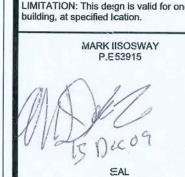
REVISIONS

SOFTPIXN

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CERTIFICATION: I heeby certify that I have examined this plan, and that the applicable portions of the plan, reating to wind engine comply with section R01.2.1, florida building code residential 2007 to the best of my

LIMITATION: This deign is valid for one



MarkTaylor

Michael & Marianne Pridgen

ADIRESS: Columbia County, Florida

Mark Disesway P.E. P.O. Box 868 Lake City, florida 32056 Phone: (38i) 754 - 5419 Fax: (386 269 - 4871

PRINTID DATE: December 15, 2009 STRUCTURAL B DRAWN BY: David Disosway

FINALS DATE: 14Dec09

JOB NUMBER: 912115 DRAWING NUMBER

OF 3 HEETS

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN. PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approva
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

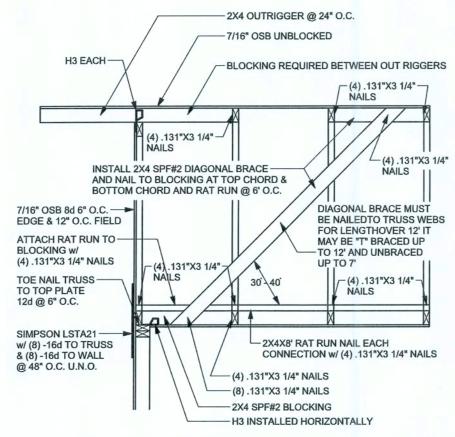
5d COOLER NAILS 7" OC EDGE 10" OC FIELD

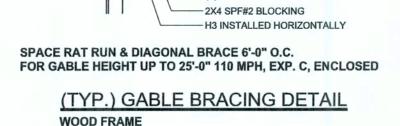
WIND SPEED

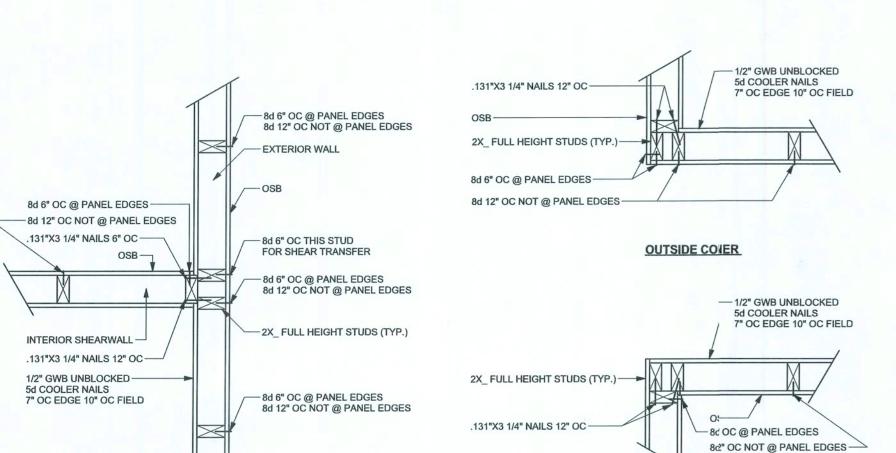
90-100 MPH

110-120 MPH

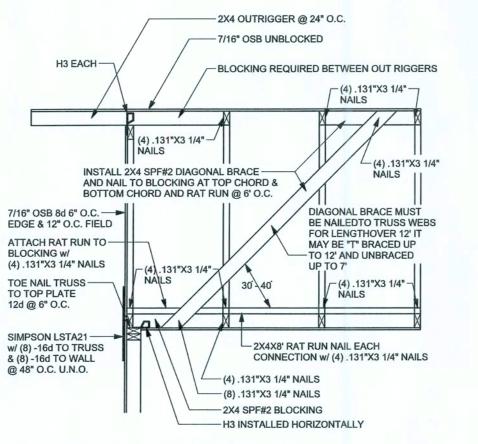






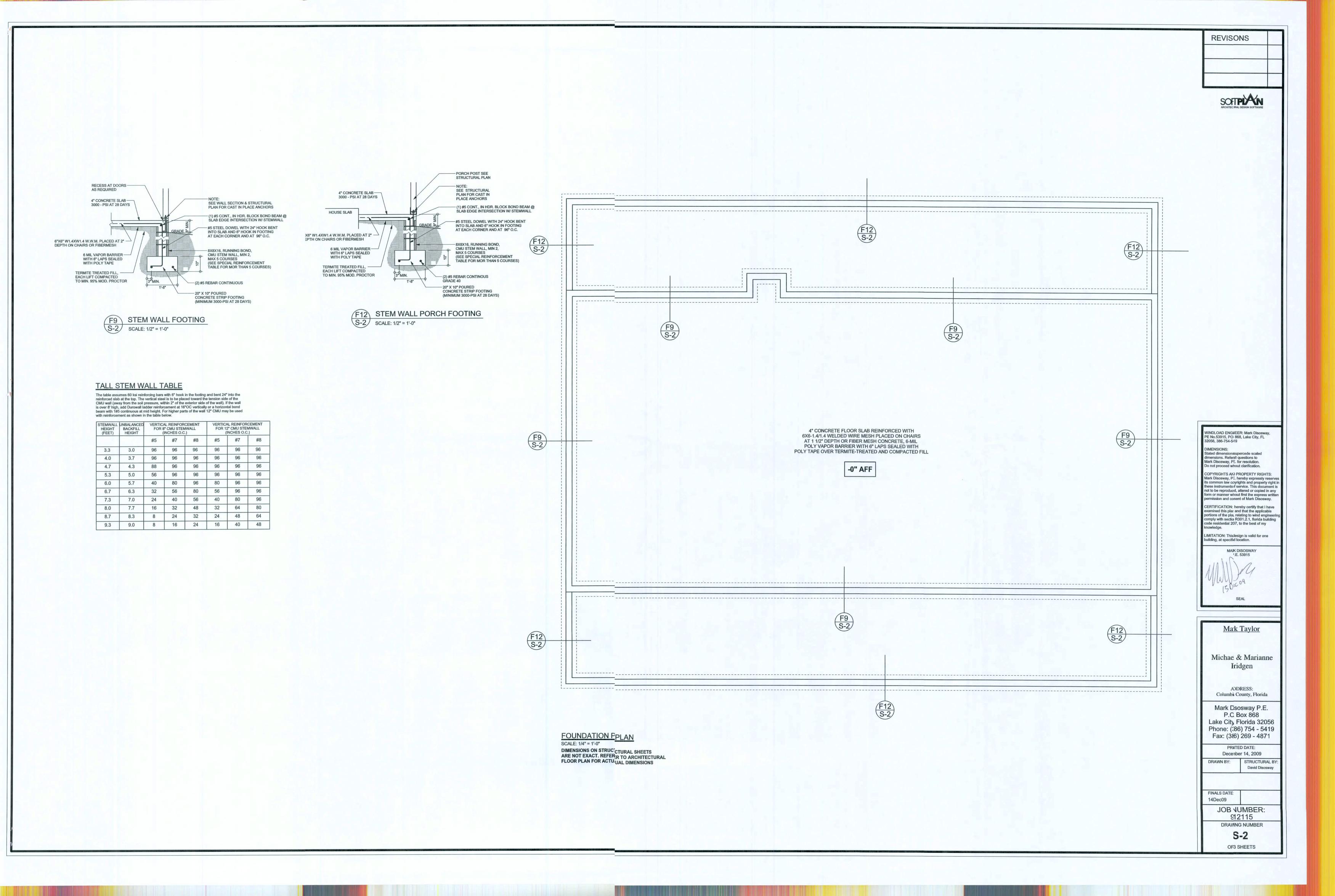


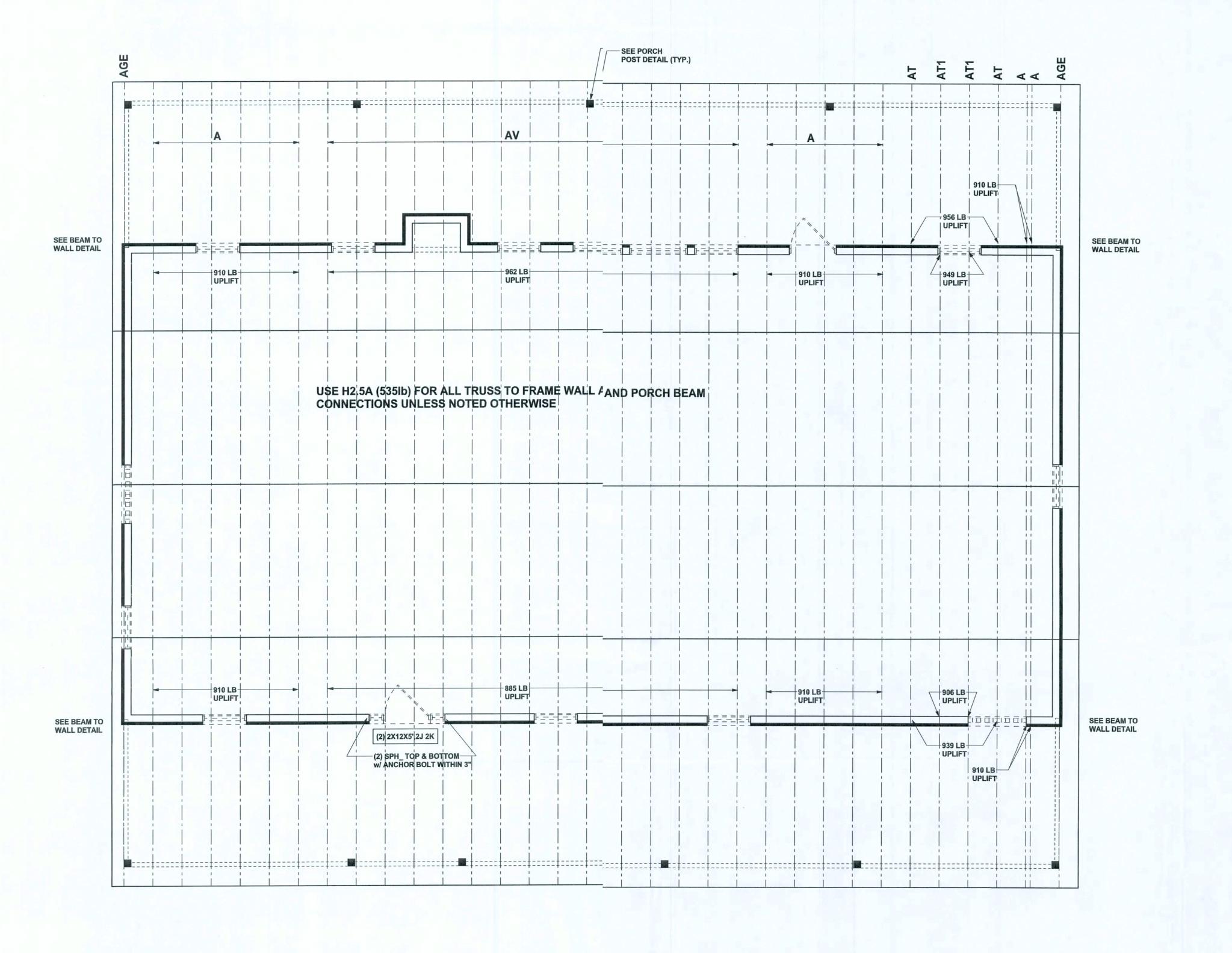




(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER

INTO KING STUD





STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

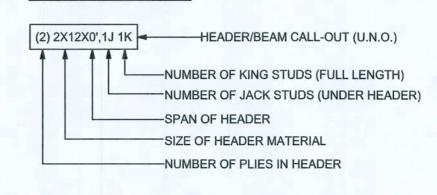
STRUCTURAL PLAN NOTES

- ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

EXTERI _{RIOR} WALL
INTERIGIOR NON-LOAD BEARING WALL
INTERIGIOR LOAD BEARING WALL w/ NO UPLIFT
INTERICIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

INDICATE	S SHEAR W	ALL SEGMENTS	
	REQUIRED	ACTUAL	
TRANSVERSE	46.8'	56.0'	
LONGITUDINAL	40.0'	81.0'	

REVISONS

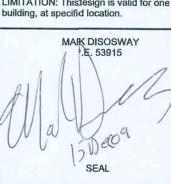
WINDLOAD ENGNEER: Mark Disosway, PE No.53915, POi 868, Lake City, FL 32056, 386-754-519

DIMENSIONS: Stated dimensionssupercede scaled dimensions. Refeall questions to Mark Disosway, PE. for resolution. Do not proceed whout clarification.

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CERTIFICATION: hereby certify that I have examined this plar and that the applicable portions of the pla, relating to wind engineerin comply with sectic R301.2.1, florida building code residential 207, to the best of my

LIMITATION: Thisdesign is valid for one building, at specifid location.



Mak Taylor

Michael & Marianne Iridgen

ADDRESS: Columbia County, Florida

Mark Dsosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRITED DATE: Decenber 14, 2009 STRUCTURAL BY: DRAWN BY: David Disosway

FINALS DATE

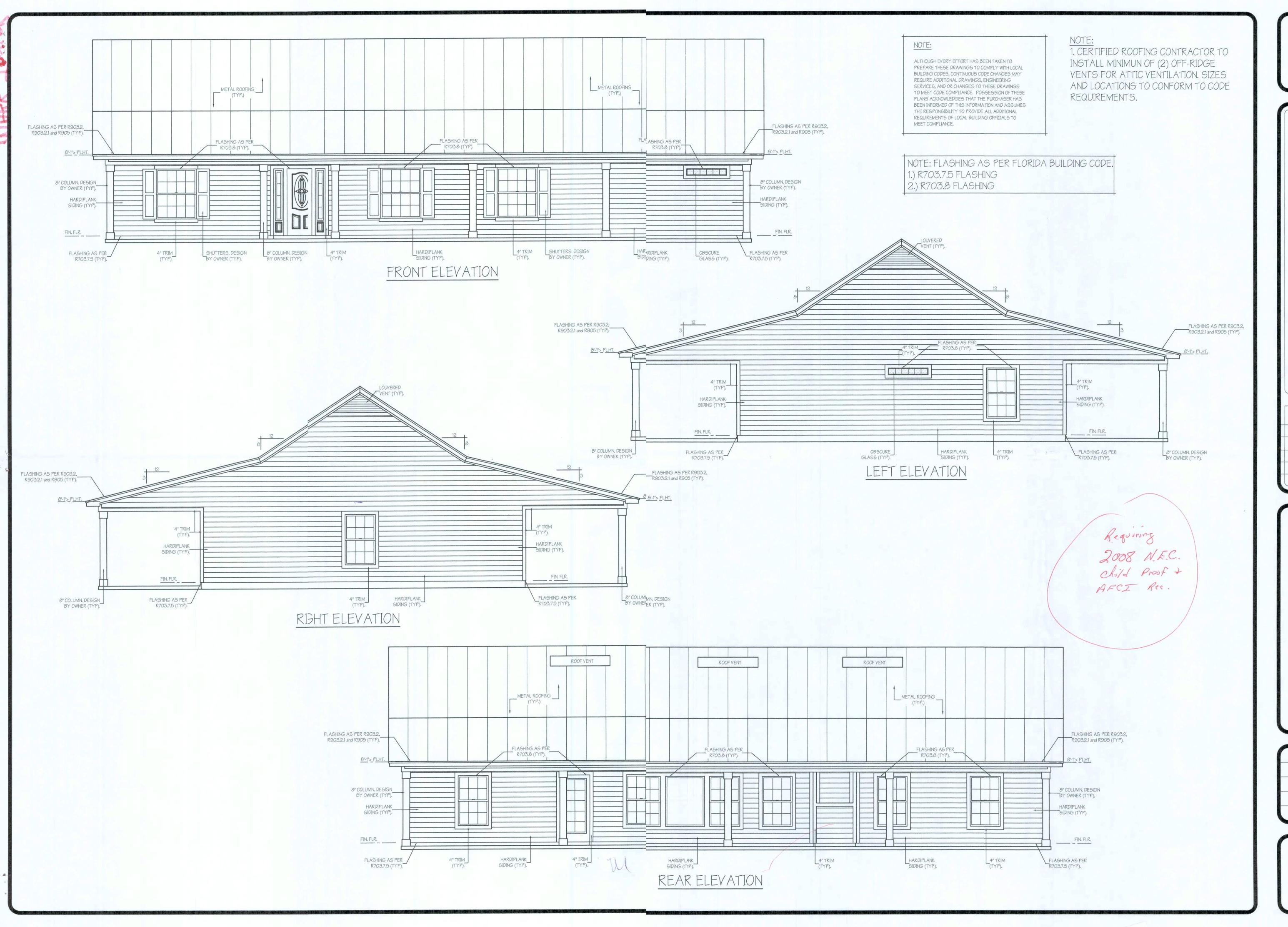
14Dec09 JOB NUMBER:

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING

FURNISHED BY BUILDER. ANDERSON TRUSS JOB #9-218

912115 DRAVING NUMBER

S-3 OF3 SHEETS



5

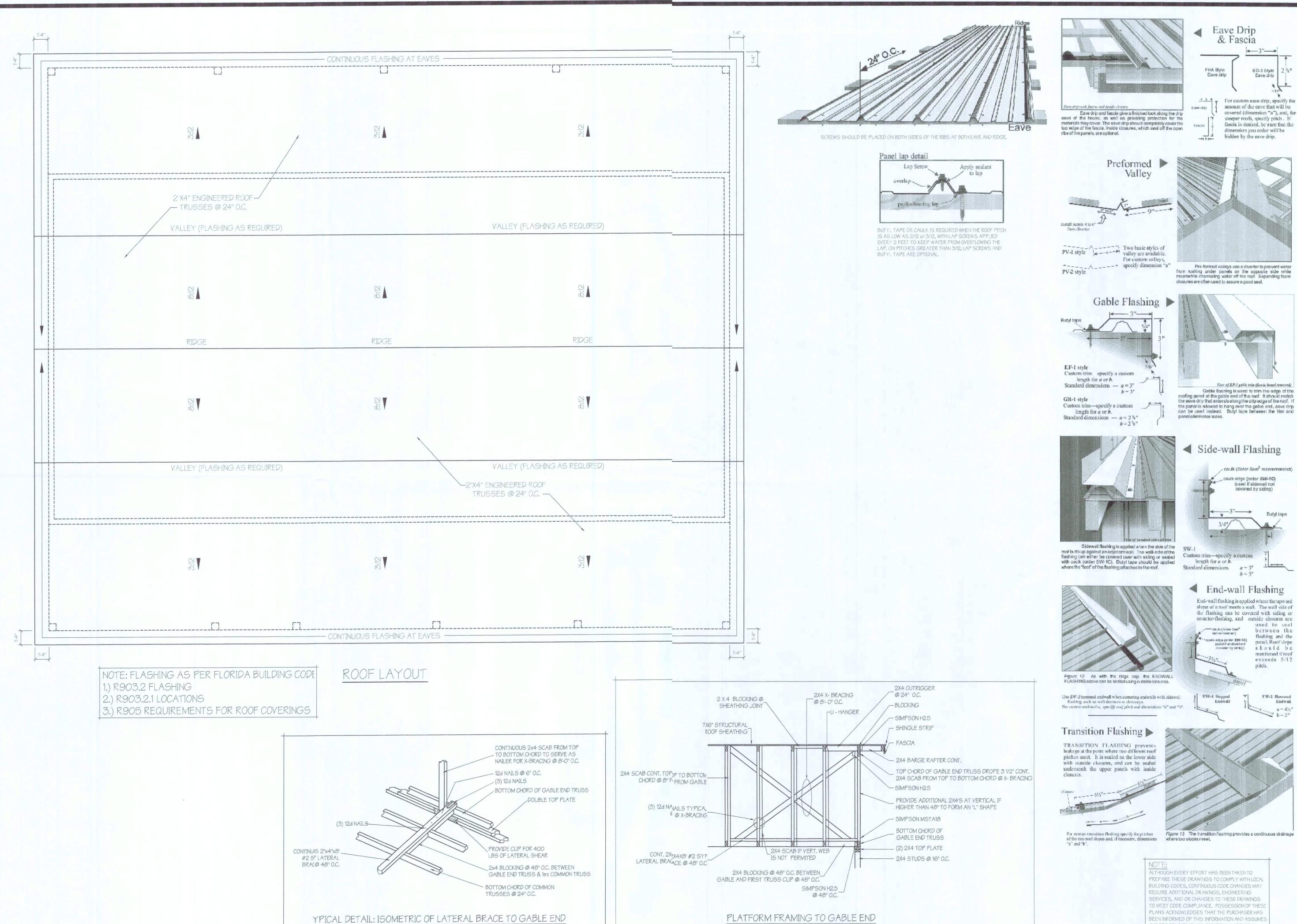
mson Brothers Construction Services LL

revisions b

Michael & Marianne Pridgen

No.	
	drawi by
	checkid by AEW
	da:e 08-21-09
	scde 1/4"=1'-0"
	file name Pridger.dwg
No.	Thagenawg

5 Elevctions



SCALE: 1/2" = 1'-0"

ED-3 Styla 2 %" Eave drip * b 4 For custom eave drip, specify the amount of the cave that will be covered (dimension "a"), and, for steeper mots, specify pitch. If fascia is desired, be sure that the Pre-formed valleys use a diverter to prevent water from rushing under panels on the opposite side with meanwhile channeling water off the roof. Expanding from View of EP-1 yable trim (funcia broad removed) Gable flashing is used to trim the edge of the roofing penol at the gable end of the roof. It should match the eave drip that extends along the drip edge of the roof. If the penel is allowed to hang over the gable end, eave drip can be used instead. Bufyl tape between the frim and revisions by ∠caulk (Solar Seal[®] recommended)

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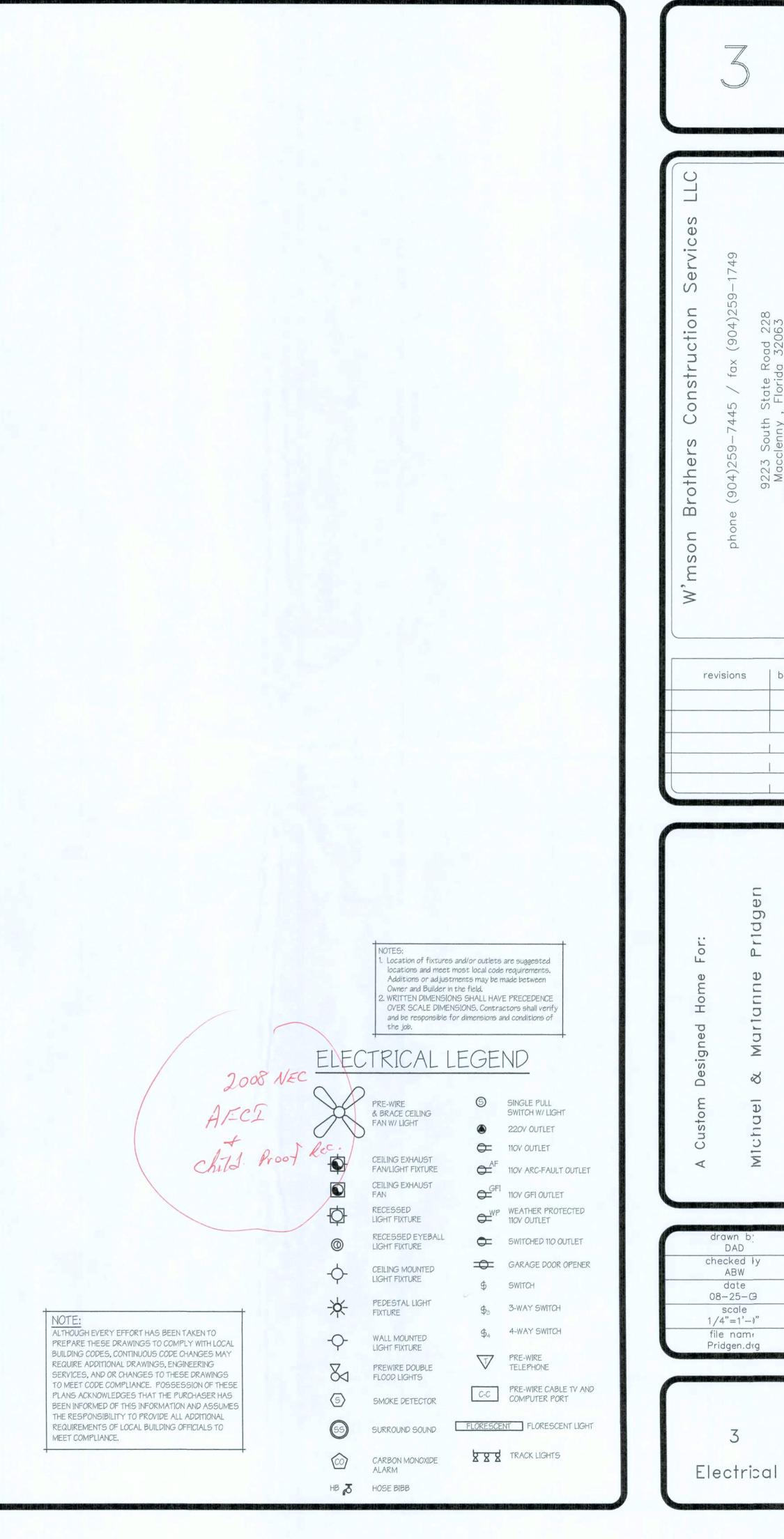
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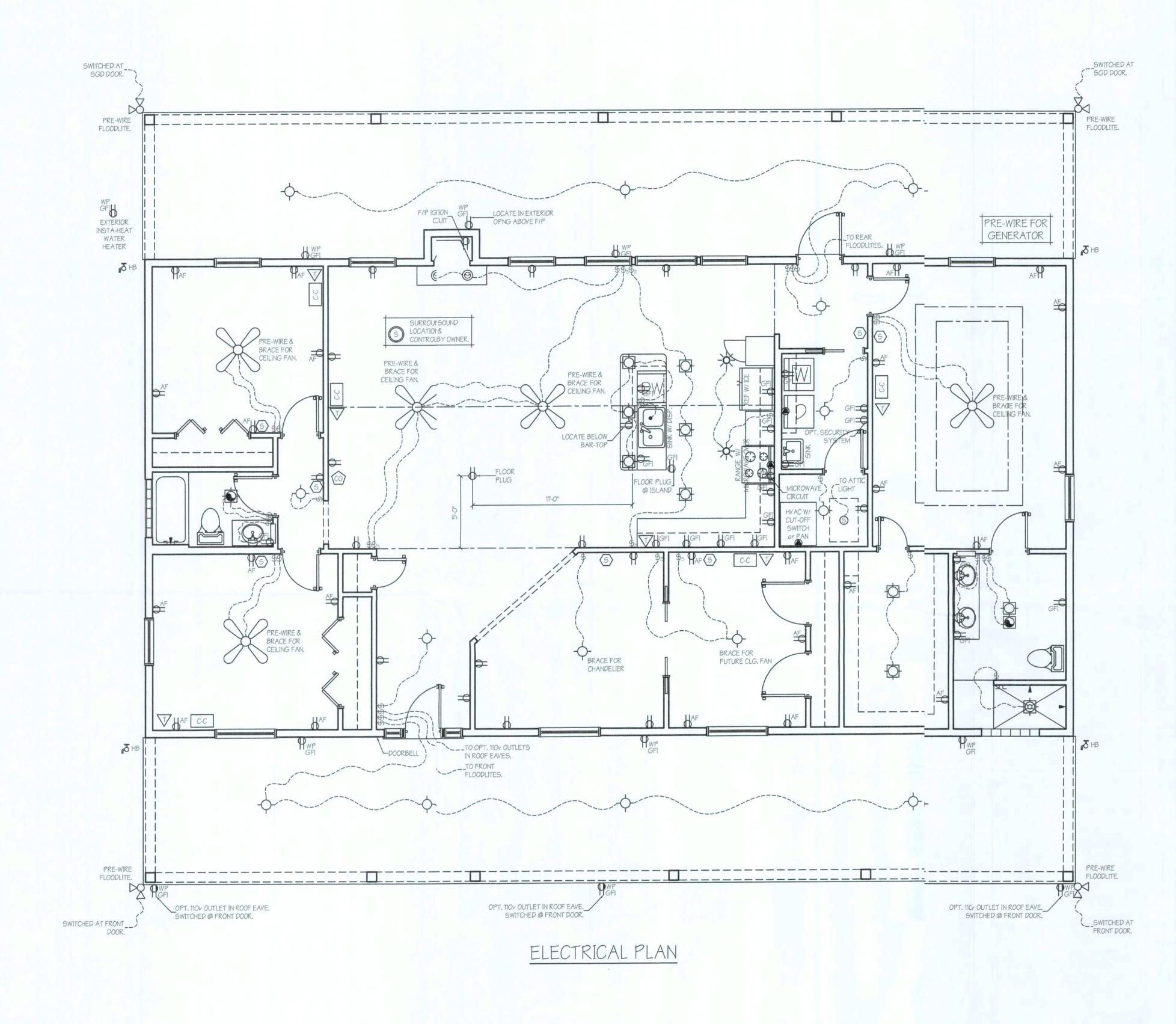
Roof

BEEN INFORMED OF THIS INFORMATION AND ASSUMES THE RESPONSIBILITY TO PROVIDE ALL ADDITIONAL

REQUIREMENTS OF LOCAL BUILDING OFFICIALS TO

MEET COMPLIANCE.





NOTE: SMOKE DETECTORS SHALL BE LOCATED AT A POINT CENTRALLY LOCATED IN THE CORRIDOR OF AREA GIVING ACCESS TO EACH GROUP OF ROOMS FOR SLEEPING PURPOSES.THEY SHALL BE INSTALLED IN ACCORDANCE WITH NFPA 72, CHAPTER 2.

NOTE

1. ARC-FAULT & GFI OUTLETS WIED PER CODE.

2. LOCATIONS OF OUTLETS ARE; HOWN FOR CONCEPTUAL PURPOSES ONLY. ELECTRICIAN WILL INSTALL TO MEET CODE RRUIREMENTS.



SECTION A

APPLY A 3/8" NOM. BEAD OF SEALANT TO THE BACKSIDE (INTERIOR) OF THE MOUNTING FLANGE AROUND THE ENTIRE PERIMETER, PLACE BEAD IN LINE WITH ANY PRE-PUNCHED HOLES OR SLOTS.

INTERIOR VIEW

TOP LEFT CORNER OF WINDOW

FLASHING AT HEAD EXTENDS 1" BEYOND JAMB FLASHING

THROUGH WINDOW JAMB

LWEATHERED RESITIVE BARRIER.

SEALANT BEAD BETWEEN MOUNTING FLANGE

SHIM AND ADJUST WINDOW TO ACHIEVE SQUARE PLUMB AND LEVEL CONDITION. USE CORROSION RESISTANT FASTNERS, FASTEN WINDOWS PER WINDOW MANUFACTURER SPECIFICATIONS,

AFTER INSTALLATION

SEAL ALL CORNERS OF MECHANICALLY JOINED FRAMES TO SEAL FRAME SEAM JUNCTURE.

⊘ HEAD FLASHING
APPLY A CONT. SEAL ALONG MOUNTING FLANGE AT THE
HEAD, FLASHING GOES OVER MOUNTING FLANGE, EMBED
BOTTOM OF HEAD FLASHING AGAINST SEALANT (FLASHING
GOES OVER SEALANT), EXTENDHEAD FLASHING BEYOND
EACH JAMB FLASHING, FASTEN IN PLACE.

→

RESITIVE BARRIER @ HEAD.

WINDOW INSTALLATION METHOD B-1

WEATHERED RESITIVE BARRIER APPLIED PRIOR TO THE WINDOW INSTALLATION. FLASHING APPLIED BEHIND THE MOUNTING FLANGE.

© JAMB FLASHING
EXTEND JAMB FLASHING TO OVERLAP SILL FLASHING.
EXTEND JAMB FLASHING 8-1/2" (220mm) ABOVE ROUGH
OPENING AT HEAD, APPLY TAPE AT CORNERS TO
TEMPORARILY HOLD FLASHING IN PLACE UNTIL WEATHER
RESISTIVE BARRIER IS APPLIED.

ALTHOUGH EVERY EFFORT HAS BEEN TAKEN TO

PREPARE THESE DRAWINGS TO COMPLY WITH LOCAL

BUILDING CODES, CONTINUOUS CODE CHANGES MAY

TO MEET CODE COMPLIANCE. POSSESSION OF THESE

PLANS ACKNOWLEDGES THAT THE PURCHASER HAS

REQUIREMENTS OF LOCAL BUILDING OFFICIALS TO

MEET COMPLIANCE.

BEEN INFORMED OF THIS INFORMATION AND ASSUMES THE RESPONSIBILITY TO PROVIDE ALL ADDITIONAL

REQUIRE ADDITIONAL DRAWINGS, ENGINEERING SERVICES, AND OR CHANGES TO THESE DRAWINGS

1. ALL FRAME WALLS ARE DRAWN @ 4" NOMINAL. ACTUAL SIZE, SPACING & GRADE OF LUMBER TO BE

DETERMINED BY BUILDER AND TO CONFORM TO

2. TOPS OF ALL WINDOWS ARE TO BE 6'-8" AFF

SQ. FTG. TABULATION

LOCAL BUILDING CODES.

ENTRY -

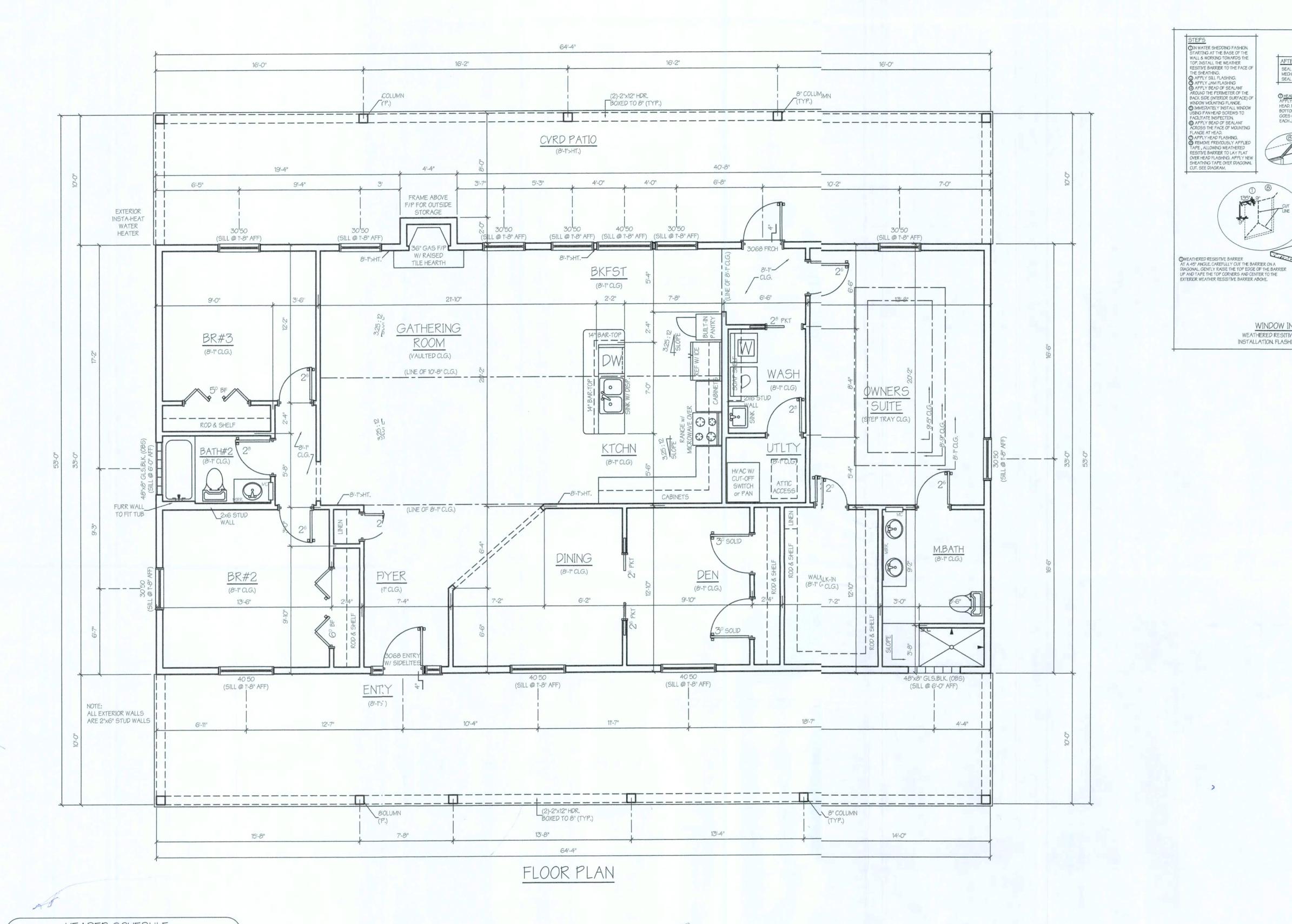
UNLESS NOTED OTHERWISE.

revisions

checked by ABW date 08 - 24 - 19scale 1/4"=1'-0" file name

Pridgen.owg

Floor Plan



HEADER SCHEDULE 2 X STUD CONTINUOUS TO TOP PLATE -HEADER - CONTINUOUS (GLUED & NAILED WHEN USING PLYWOOD FLITCH)

OPENING WIDTH	BEARING WALL OR SHEAR WALL	NON-BEARING WALLS
0'-0" T0 3'-0"	2-2×6'5	2-2 x 4'5
3'-1" TO 5'-0"	2 - 2 × 10'5	2-2 x 6's
5'-1" TO 7'-0"	2 - 2 x 12'S	2-2 x 8's
7'-1" TO 9'-0"	2 - 2 × 12'5 W/ 1/2" PLY- WOOD FLITCH	2-2 x 12's

STUD TABLE

	BEARING WALLS			NONBEARING WALLS		
TUD SIZE	LATERALLY UNSUPPORTED STUD HEIGHT# (FEET)	SUPPORTINOOF AND CEILINNLY	SUPPORTING ONE FLOOR, ROOF AND CEILING	SUPPORTING TWO FLOOR, ROOF AND CEILING	LATERALLY UNSUPPORTED	SPACING
NCHES)	SPACING (INCHES)			STUD HEIGHT® (FEET)	(INCHES)	
2"x3"b					10	16
2"x4"	10	24	16		14	24
3"x4"	10	24	24	16	14	24
2"x5"	10	24	24		16	24
2"x6"	10	24	24	16	20	24

a. LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LAKAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED b. SHALL NOT BE USED IN EXTERIOR WALLS.

