

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1ULY8228Z0109132811

Truss Fabricator: Anderson Truss Company
Job Identification: 12-088--Stephen Crawford Construc Thomas Res -- , **
Truss Count: 38
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

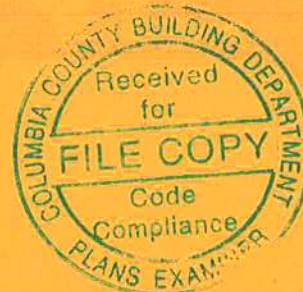
Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-PB16010-

#	Ref	Description	Drawing#	Date
1	47709--AS2		12130001	05/09/12
2	47710--AS1		12130002	05/09/12
3	47711--AS		12130003	05/09/12
4	47712--ASGE		12130004	05/09/12
5	47713--EV		12130005	05/09/12
6	47714--AVS3		12130006	05/09/12
7	47715--AVS2		12130007	05/09/12
8	47716--AVS1		12130008	05/09/12
9	47717--AGE		12130009	05/09/12
10	47718--AVS		12130010	05/09/12
11	47719--AVD		12130011	05/09/12
12	47720--AV		12130012	05/09/12
13	47721--B		12130013	05/09/12
14	47722--BGE		12130014	05/09/12
15	47723--CA		12130015	05/09/12
16	47724--CAGE		12130016	05/09/12
17	47725--CA1		12130017	05/09/12
18	47726--C		12130018	05/09/12
19	47727--C1		12130019	05/09/12
20	47728--CGE		12130020	05/09/12
21	47729--C2		12130021	05/09/12
22	47730--C3		12130022	05/09/12
23	47731--DG		12130023	05/09/12
24	47732--D		12130024	05/09/12
25	47733--DGE		12130025	05/09/12
26	47734--D0		12130026	05/09/12
27	47735--E		12130027	05/09/12
28	47736--E1		12130028	05/09/12
29	47737--E2		12130029	05/09/12
30	47738--EG		12130030	05/09/12
31	47739--FG		12130031	05/09/12
32	47740--GGE		12130032	05/09/12
33	47741--G		12130033	05/09/12
34	47742--G1		12130034	05/09/12
35	47743--GG		12130035	05/09/12
36	47744--M		12130036	05/09/12

#	Ref	Description	Drawing#	Date
37	47745--AP		12130037	05/09/12
38	47746--APGE		12130038	05/09/12

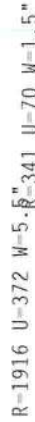
-Truss Design Engineer-
Doug Fleming

Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844



Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

The Building Components Group, (THRCO) shall not be responsible for any violation on this design. Any failure to build the truss in accordance with ABS/PTI 1 for handling, shipping, installation or bracing of trusses, apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this design shown. The suitability and use of this design for any structure is the responsibility solely for the design shown. The responsibility and use of this design for any structure is the responsibility of the building designer per ABS/PTI 1 Sec.2. For more information see: This Job's drawing sheet. THRCO: www.thrcg.com; PTI: www.pti.org; WFLA: www.wfla.com.



TC LL	20.0 PSF	REF	R8228-	47709
TC DL	10.0 PSF	DATE	05/09/12	
BC DL	10.0 PSF	DRW	HCUSR8228	12130000
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	93729	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1ULY8228Z01	

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Web	2x4	SP	#3

:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.661'

Left end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

(H1) - (J) Hanger not calculated (2)2x6 SP M-26 supporting member.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS Loads based on trusses located at least 7.50 ft. from roof edge.

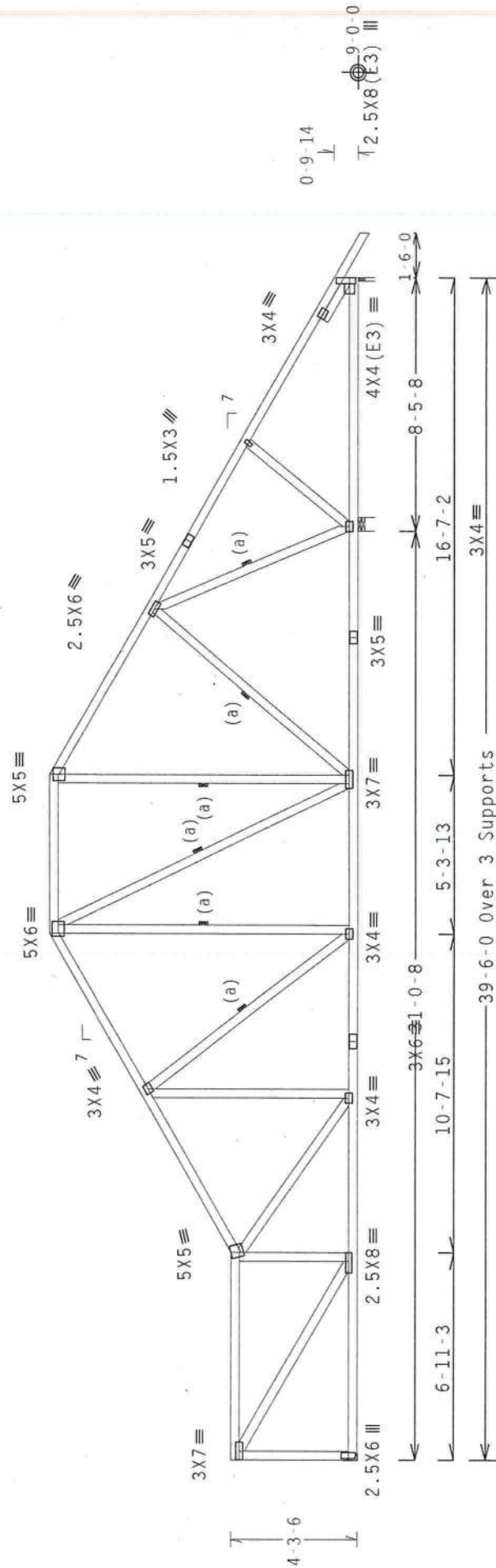
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-1.2 psf. GCpi (+/-) 0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-1238 II-259

RI-192/-214 H-Simpson IUS26

Supported Member Face: (4)

Supported Member Face: (4)

Supporting Member	Face: (4)	2x6	SP #2
Supporting Member	Face: (2)	2x6	SP #2

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

OTY:1 FI /- /4 /- /- /R /-
Scale = 1875"/Ft.

R-1917 U=370 W=5.5" 11-24 W-1 E"

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Towers require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of RCSI Building Component Safety Information, by IPI and AISCa) for safety practices and performance. Install and provide temporary bracing per RCSI Building Component Safety Information, by IPI and AISCa) for safety practices and performance. RCSI shall have a properly Attached Field section. Locations shown for permanent lateral restraint of webs shall have bracing installed per RCSI sections B2, R40 B10, as applicable.

The Building Components Group Inc. (BIBCO) shall not be responsible for any deviation from this design or any failure to build the mass in accordance with ANSI/HFSS-1, or for handling, shipping, installation or storage of the mass. The responsibility for the mass shall remain solely with the building owner. BIBCO will provide detailed notes noted above and refer to drawings I605.2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/HFSS-1 Sec.2. For more information see: This Job's general notes page; ITW-HCG; www.bcbco.com; ITFI; www.itfincast.com; MICA; www.micaindust.com.



ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278



FL/-/4/-/-/R/-		Scale = .1875"/Ft.
TC LL	20.0 PSF	REF R8228 47710
TC DL	10.0 PSF	DATE 05/09/12
BC DL	10.0 PSF	DRW HCUR8228 12130002
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 93740
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1ULY8228Z01

:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.661'

	(Lumber	Dur.Fac.-1.25 / Plate	Dur.Fac.-1.25)
TC-From	61 pif at 0.00 to	61 pif at 7.75	
TC-From	63 pif at 7.75 to	63 pif at 17.59	
TC-From	61 pif at 17.59 to	61 pif at 22.91	
TC-From	197 pif at 22.91 to	197 pif at 22.93	
TC-From	197 pif at 22.93 to	266 pif at 31.50	
TC-From	63 pif at 31.50 to	63 pif at 41.00	
BC-From	20 pif at 0.00 to	20 pif at 10.42	
BC-From	60 pif at 10.42 to	60 pif at 15.00	
BC-From	20 pif at 15.00 to	20 pif at 18.04	
BC-From	60 pif at 18.04 to	60 pif at 20.79	
BC-From	20 pif at 20.79 to	20 pif at 25.52	
BC-From	60 pif at 25.52 to	60 pif at 29.27	
BC-From	20 pif at 29.27 to	20 pif at 39.50	
BC-From	5 pif at 39.50 to	5 pif at 41.00	
TC-137.50 lb Conc.	Load at 31.50		

Left end vertical not exposed to wind



FT/RT=20%(0%)/0(0)

QTY: 3 FL/-/4/-/-/R/-/-
Scale = .125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Components Safety Information, by IP1 and MCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Trusses shall have a properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

TIU Building Components Group Inc. (TIUBCG) shall not be responsible for any deviation from this design or any failure to build the product in accordance with ANSI/TPI 1, or for handling, shipping, installation & details not contemplated otherwise. Refer to drawing TIBO-2 for standard plate positions. A seal on this drawing or cover page listing this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec. 2. For more information see: This job's general notes page; TIU MFG. web site: www.tibuco.com; TPI: www.tpinet.org; MICA: www.micadirect.com



ITW Building Components Group Inc.

mes City, FL 33844
FL COA #0 278

JREF- JULY8228Z01

```
Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1:
```

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.16" due to live load and 0.31" due to dead load.

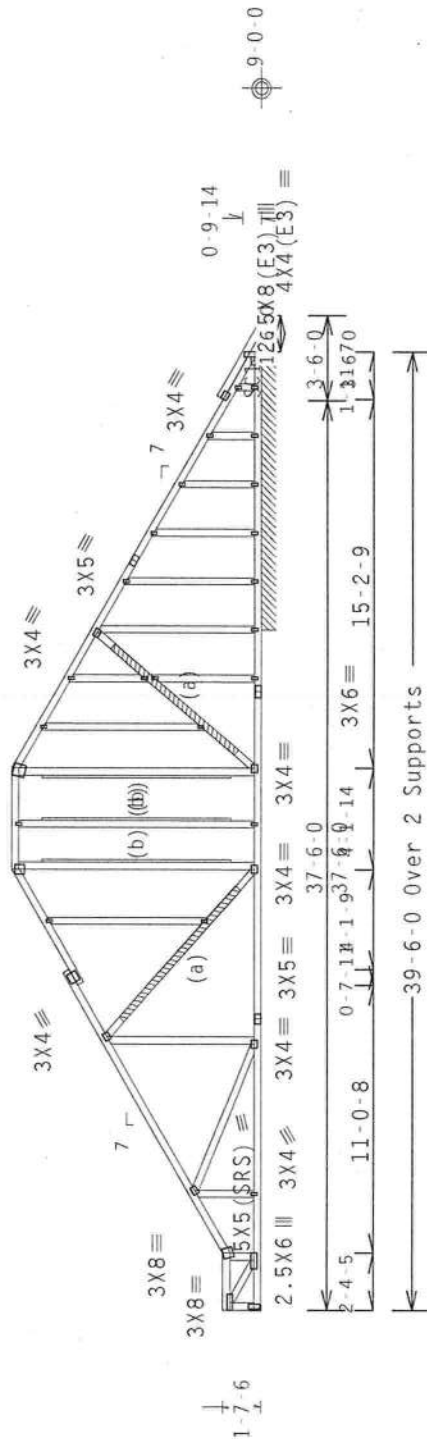
See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) - (J) Hanger not calculated (2)2x6 SP M-26 supporting member.

(b) 1x4 #3SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

$$5 \times 5 (R) \equiv 5 \times 5 \equiv 6 \times 6 \equiv$$


R-1443 U-269

RL-236/-236 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (2) 2x6 SP #2

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=20%(0%)/0(0)

10.03.11.020000

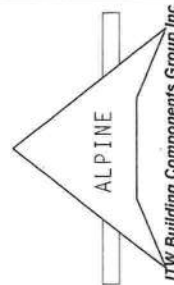
QTY:1 FL/-/4/-/-/R/-/

Scale = .125"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
FOLLOW THE INSTRUCTIONS OF EACH TEST. CONTACT YOUR LOCAL ENVIRONMENTAL AGENCY FOR MORE INFORMATION.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BECS (Building Component Safety Information, by IPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BECS. Notes mentioned otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BECS sections 03, 07 or B10, as applicable.

The Building Components Group, Inc. (BUCOG) shall be responsible for any deviation from this design. The manufacturer shall be responsible for any deviation from the manufacturing instructions. Any failure to build the truss in accordance with AISC/TPI standards or other applicable codes, detailing or engineering drawings, specifications, drawings, or manuals shall constitute a breach of contract. The responsibility for bracing of trusses, applied plates, for each face of truss and position as shown above and on the joint details, unless noted otherwise, refer to drawing, BUCO-2 for standard plate positions. A seal on this drawing or cover page listing this design, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per AISC/TPI Section 2. For more information see: This Job's general notes page; ITU-BCO; www.tubeed.com; TPI: www.tpiint.org; HGA: www.shelindust.com;



es City, FL 33844

es City, FL 33844

es City, FL 338



TC LL	20.0	PSF	REF R8228- 47712
TC DL	10.0	PSF	DATE 05/09/12
BC DL	10.0	PSF	DRW HCUSR8228 12130004
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	40.0	PSF	SEQN- 93835
DUR.FAC.	1.25		
SPACING	24.0"		JREF- JULY8228Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1 :B3 2x6 SP SS:
Webs 2x4 SP #3
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.821'
Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.25" due to live load and 0.57" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Calculated vertical deflection is 0.36" due to live load and 0.80" due to dead load at X = 28-9-0.

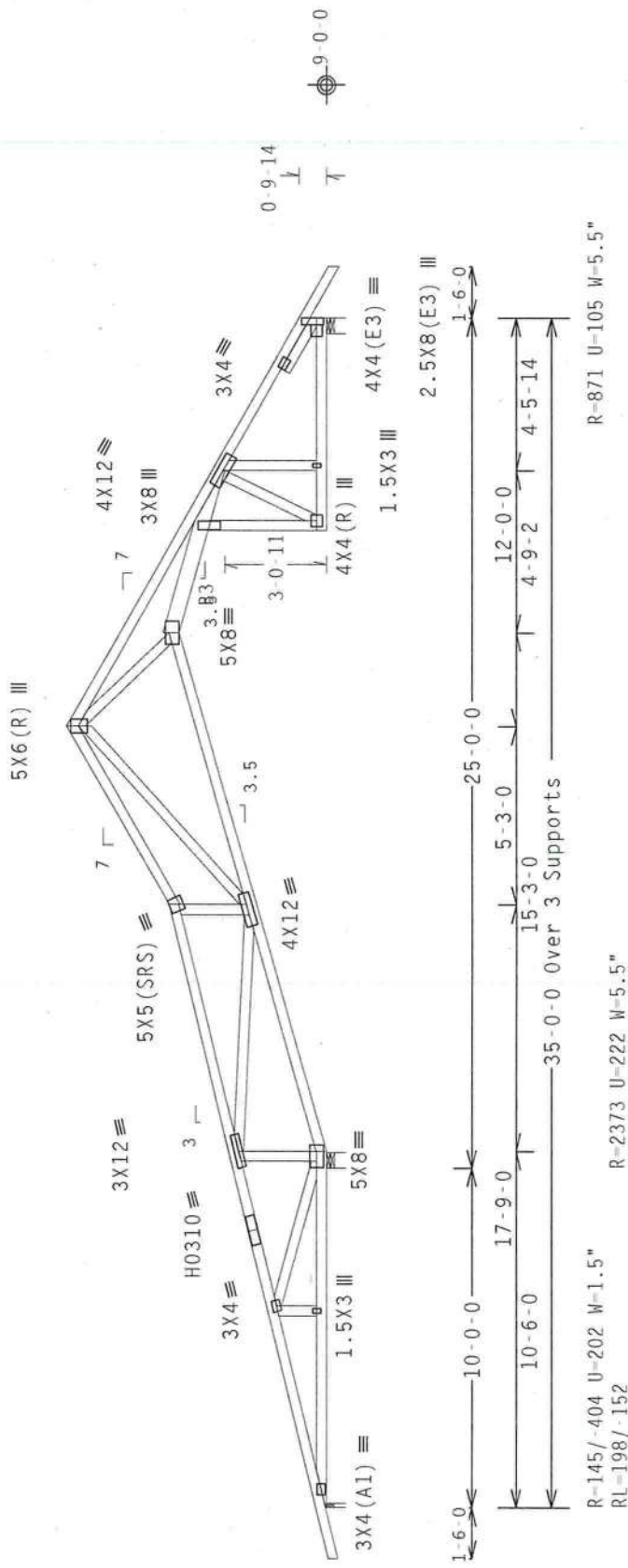
Negative reaction(s) of -403# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS, Wave		FT/RT=20%(0%)/0(0)		10.03.11.02	QTY:1	FL/-/4/-/R/-	Scale = .1875"/Ft.	
ALPINE ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278		**WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET			TC LL 20.0 PSF			
		FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.			TC DL 10.0 PSF			
		Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and BCS) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.			BC DL 10.0 PSF			
		ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on this truss is required for the warranty. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW BCG: www.itwbcg.com; TPI: www.tpiinst.org; MICA: www.sbciindustry.com; ICC: www.iccsafe.org			BC LL 0.0 PSF			
					TOT.LD. 40.0 PSF			
			DUR.FAC. 1.25					
			SPACING 24.0"					
			JREF - JULY8228Z01					



Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.661'
Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

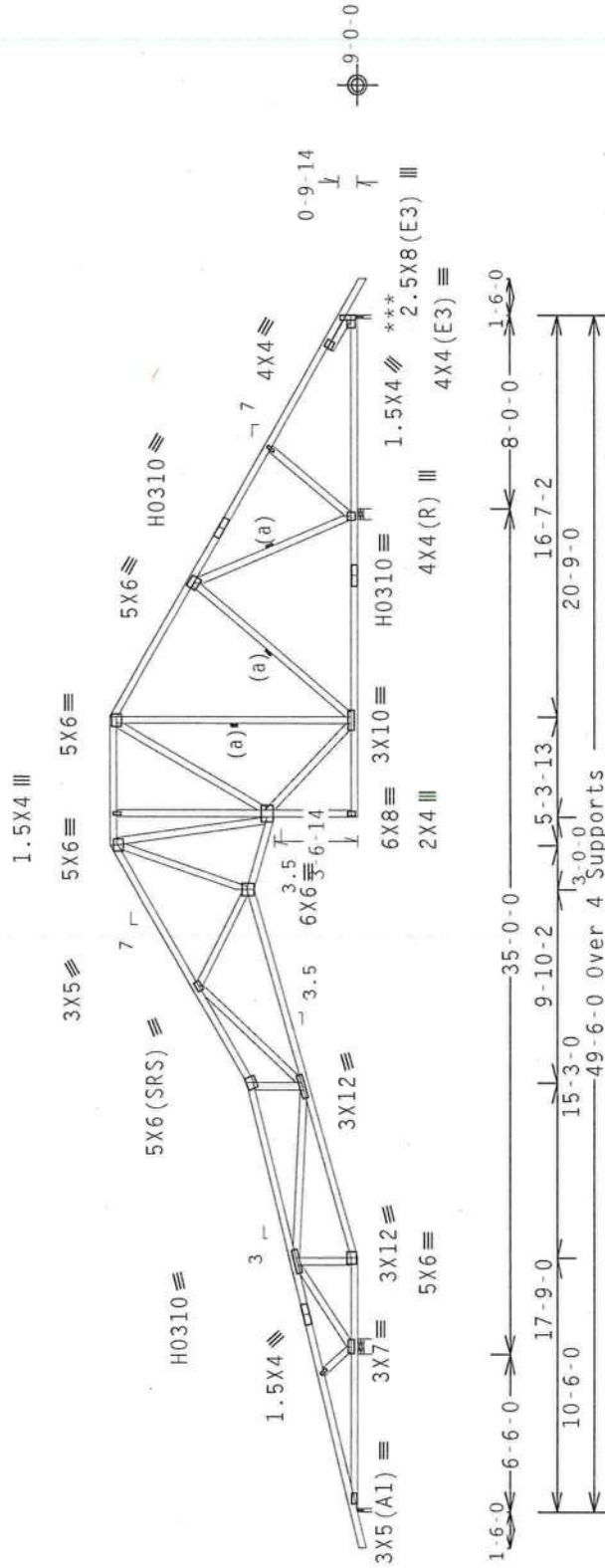
*** Negative reaction(s) of -727# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=1.2 psf, G_{CF}(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R-123/-290 U=153 W=1.5"
RL-272/-220 R-2025 U=135 W=7.778"

R-2847 U=121 W=5.5"
R=-727 Rw=205 U=447 W=1.5"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS, Wave

10.03.11.0000

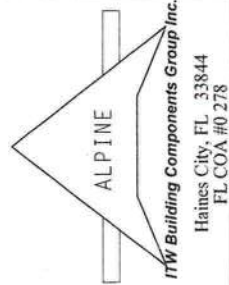
QTY:1

Scale = .125"/Ft.

***WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.

***IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by IPI and UICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1600A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering. The structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec 2. For more information see: This Job's general notes page; ITW BCG: www.itwbog.com; IPI: www.tpinet.org; UICA: www.sbrindustry.com; JCC: www.tccsafe.org



TC LL	20.0 PSF	REF	R8228- 47714
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	93788
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	IULY8228Z01

Top chord 2x4 SP #1 :T5 2x4 SP M-30:
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W2, W15 2x4 SP #1:
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

Special loads

----- (Lumber
Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)
IC- From 61 pif at 1.50 to 61 pif at 17.75
IC- From 63 pif at 17.75 to 63 pif at 27.59
IC- From 61 pif at 27.59 to 61 pif at 32.91
IC- From 63 pif at 32.91 to 63 pif at 32.93
IC- From 197 pif at 32.93 to 266 pif at 41.50
IC- From 63 pif at 41.50 to 63 pif at 51.00
BC- From 4 pif at 1.50 to 4 pif at 0.00
BC- From 20 pif at 0.00 to 20 pif at 10.50
BC- From 21 pif at 10.50 to 21 pif at 25.75
BC- From 21 pif at 25.75 to 21 pif at 28.75
BC- From 20 pif at 28.75 to 20 pif at 49.50
BC- From 5 pif at 49.50 to 5 pif at 51.00
TC- 137.50 lb Conc. Load at 41.50

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

*** Negative reaction(s) of -653# MAX. (See below) from a non-wind load case requires uplift connection.
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-1.2 psf. GCPI(+/-)-0.18

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.11" due to live load and 0.20" due to dead load.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24" wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

5X6 (SRS) =

H0310 =

4X8 =

3X12 =

6X10 =

3X4 =

3X5 (A1) =

3X4 =

3X5 =

3X6 =

3X7 =

3X8 =

3X9 =

3X10 =

3X11 =

3X12 =

3X13 =

3X14 =

3X15 =

3X16 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

4X18 =

4X19 =

4X20 =

4X21 =

4X22 =

4X23 =

4X24 =

4X25 =

4X26 =

4X27 =

4X28 =

4X8 =

4X9 =

4X10 =

4X11 =

4X12 =

4X13 =

4X14 =

4X15 =

4X16 =

4X17 =

Top chord 2x4 SP #1 :15 2x4 SP M-30:
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W15 2x4 SP #1:
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

Special loads

(Lumber
Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)
61 plf at 1.50 to 61 plf at 17.75
63 plf at 17.75 to 63 plf at 27.59
61 plf at 27.59 to 61 plf at 32.91
63 plf at 32.91 to 63 plf at 32.93
197 plf at 32.93 to 266 plf at 41.50
63 plf at 41.50 to 63 plf at 51.00
4 plf at 1.50 to 4 plf at 0.00
20 plf at 0.00 to 20 plf at 10.50
21 plf at 10.50 to 21 plf at 25.75
20 plf at 25.75 to 21 plf at 28.75
20 plf at 28.75 to 20 plf at 49.50
5 plf at 49.50 to 5 plf at 51.00
TC- 137.50 lb Conc. Load at 41.50

Roof overhang supports 2.00 psf soffit load.
Calculated horizontal deflection is 0.09" due to live load and 0.17" due to dead load.

(a) Continuous lateral bracing equally spaced on member.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MWFRS loads based on trusses located at least 30.00 ft. from roof areas with 42"-high x 24"-wide clearance.

Wind loads and reactions based on MWFRS with additional C&C member design.

1.5X4 III

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

3X4 III

5X6 III

5X6 (SRS) III

H0310 III

3X5 (A1) III

4X5 III

SS0514 III

5X8 III

3X8 III

H0310 III

7X8 III

4X4 (E3) III

2.5X8 (E3) III

0-9-14

1-6-0

7-6-0

10-6-0

15-3-0

17-9-0

34-0-0

9-10-2

16-7-2

20-9-0

1-6-0

16-7-2

20-9-0

1-6-0

16-7-2

20-9-0

1-6-0

16-7-2

R-4319 U-959 W=5.5"
R-792 RW=183 U=389 W=1.5"

PLT TYP. 20 Gauge HS.18 Gauge HS. Design Crit: FBC2010Res/TPI-2007(STD)

Wave FT/RT=20%(0%)/0(0)

TC LL	20.0 PSF	REF	R8228- 47716
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	93901
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	IULY8228Z01

10.03.11
QTY:1
FL/-/4/-/-/R/-
Scale = .125"/Ft.

DUGLAS FLEMING
LICENSE
No. 66648
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

12

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing indicates the design of the truss. The seal is the responsibility of the professional engineering structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's general notes page; ITW BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.sbrindustry.com; FCC: www.fccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord	2x4	SP	#1
Bot chord	2x4	SP	#1
Wbs	2x4	SP	#3
:Stack	Chord	SC1	2x4 SP #1:

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

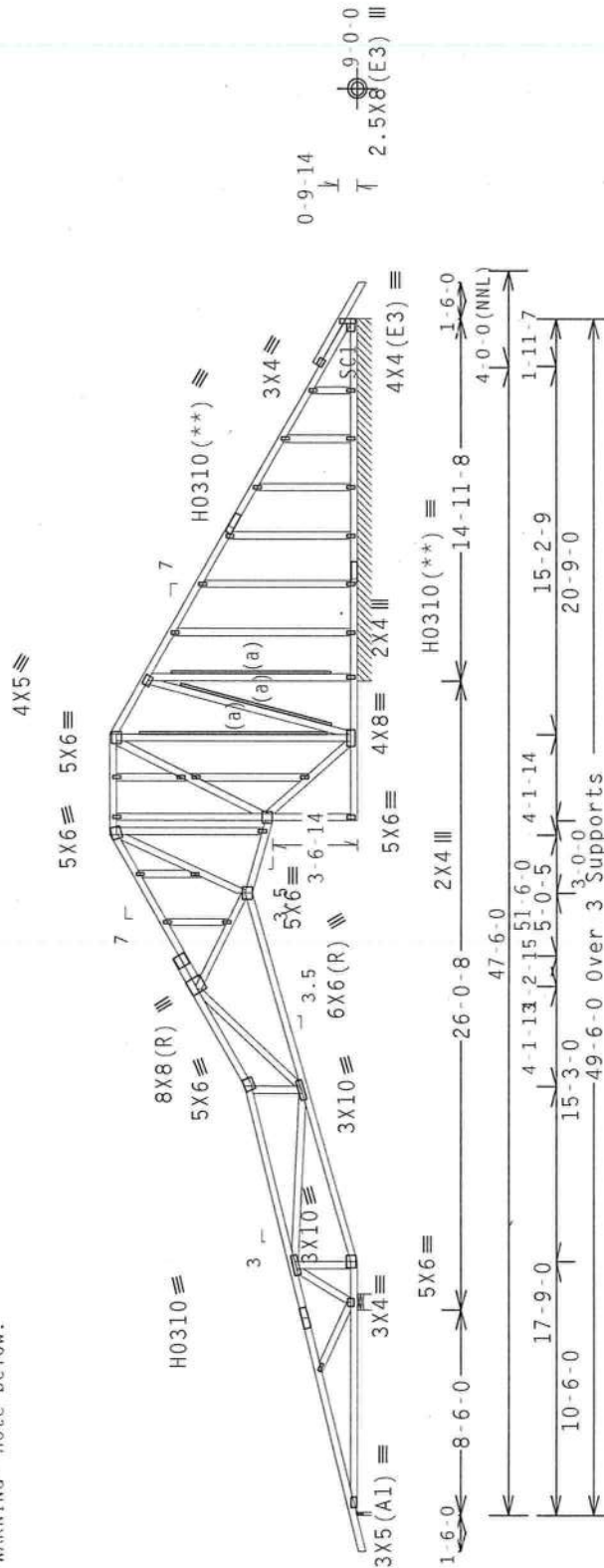
See DWGS A12015ENC100212, GBLLETIN0212, & GABRSTI00212 for more requirements.

(a) 1x4 #3SRB SPF-S or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



R-281 U-81 W-1.5"

R-1575 U-337 W-7.778"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS.Wave

FT/RT=20%(0%)/0(0)

Scale = .125"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!

****IMPORTANT**** These instructions are designed to ALL CONTRACTORS INCLUDING INSTALLERS.

Tennesses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. If tennesses noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Conditions shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

LTH Building Components Group Inc., (LTHBGC) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI-1 "or-for-handling, shipping, installation & bracing of trusses." Apply plates to each face of top chord at all joints where members meet above and on the joint details, unless noted otherwise. Refer to drawings IBCGA-Z for standard plate positions. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user. LTH Bldg Comp. Grp. Inc. has no liability for damage or injury caused by its jobs.

LTH-BGCG.com TPI news@lthinc.org HGTCS News-SOCIOHSTC@aig.com
www.lthcsa.ca



ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

Haines City, FL 33828
FL COA #0278

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.661'

Roof overhang supports 2.00 psf soffit load.

(a) continuous lateral bracing, equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Trusses to be spaced at 26.0" OC maximum.

WARNING: Furnish a copy of this DWG to the installation contractor.
Special care must be taken during handling, shipping and installation of
trusses. See "WARNING" note below.

Negative reaction(s) of -493# MAX. (See below) from a non-wind load
case requires uplift connection.

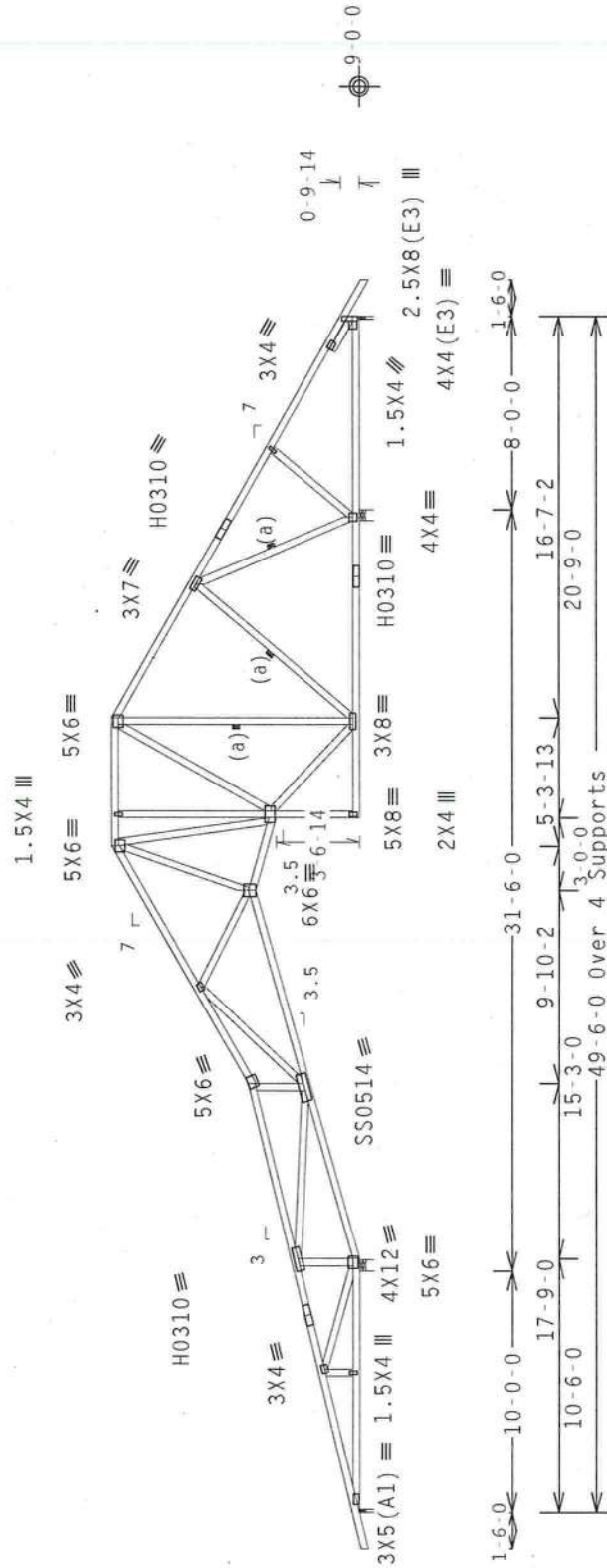
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 14.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0
psf, wind BC DL-1.2 psf. GCPI(+/-)-0.18

Wind loads and reactions based on MMFRS with additional C&C member
design.

Truss passed check for 20 psf additional bottom chord live load in
areas with 42" high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

MMFRS loads based on trusses located at least 15.00 ft. from roof
edge.



R-338 U-93 W-1.5"
RL-294/-239

R-2137 U-159 W-5.5"

R-2715 U-153 W-5.5"

R-102/-494 Rw=202 U-296 W-1.5"

PLT TYP. 20 Gauge HS.18 Gauge HS, Design Crit: FBC2010Res/TPI-2007 (STD)
Wave FT/RT=20% (0%)/0 (0)

Scale = .125"/Ft.

QTY: 5 FL/-4/-/-/R/-

10.03.11.03.0000

REF R8228- 47718

DATE 05/09/12

DRW HCUSR8228 12130010

HC-ENG DF/DF

SEON- 94364

JREF- JULY8228Z01



ALPINE
Haines City, FL 33844
FL COA #0 278

ITW Building Components Group Inc.

DOUGLAS FLEMING
LICENSE
10.03.11.03.0000
STATE OF FLORIDA
PROFESSIONAL ENGINEER
12

WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings for standard plate positions. A seal on this drawing indicates that the design was prepared by a registered professional engineer. The seal on this drawing is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpiinst.org; MCA: www.bcindustry.com; ICC: www.iccsafe.org

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W4, W12 2x4 SP #1:
:W10 2x6 SP #2:

Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

Negative reaction(s) of -253# MAX. (See below) from a non-wind load case requires uplift connection.

Special loads

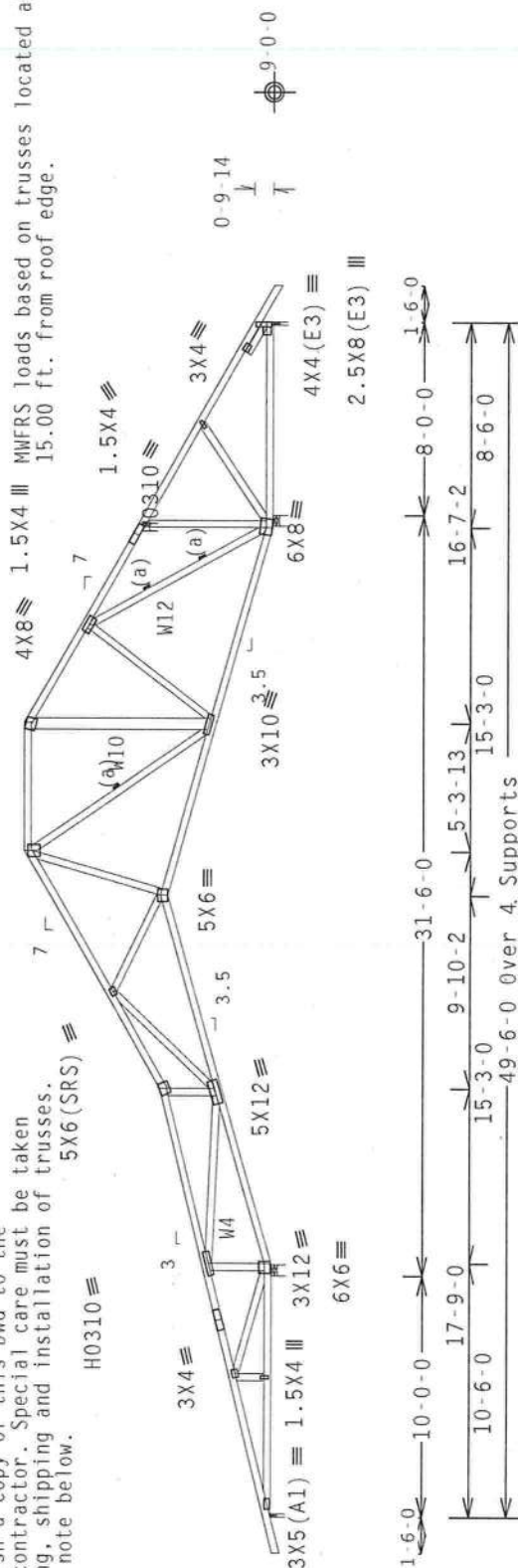
	(Lumber	Dur. Fac. -1.25 /	Plate	Dur. Fac. -1.25)
TC - From	61 pif at 1.50 to	61 pif at 17.59		
TC - From	63 pif at 17.75 to	63 pif at 27.59		
TC - From	61 pif at 27.59 to	61 pif at 32.91		
TC - From	197 pif at 32.91 to	197 pif at 33.93		
TC - From	197 pif at 32.93 to	266 pif at 41.50		
TC - From	63 pif at 41.50 to	63 pif at 51.00		
BC - From	4 pif at 1.50 to	4 pif at 0.00		
BC - From	20 pif at 0.00 to	4 pif at 10.50		
BC - From	21 pif at 10.50 to	21 pif at 25.75		
BC - From	21 pif at 25.75 to	21 pif at 41.00		
BC - From	20 pif at 41.00 to	20 pif at 49.50		
BC - From	5 pif at 49.50 to	5 pif at 51.00		
TC - 68.75 lb Conc.	Load at 41.50			
PL - 590.90 lb Conc.	Load at (32.74, 19.25)			

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

4X8 $\equiv$ 1.5X4 III MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R-201/-236 U-89 W-1.5"
RL-251/-200 R-2

R-3836 U=852 W=5.5"
R=174/-253 RW=183 U=168 W=1.5"

Design Crit: FBC2010Res/TPI - 2007 (STD)
FT/RT=20% (0%) / 0 (0)

10.03.11 ~~ASSEMBLED~~ QTY: 2 FL / - 4 / - / - / R / -
Scale = .125" / Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING ELECTRICALS

Trusses require extra care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IP1 and WICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Trusses must otherwise, (top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7, or B10, as applicable.

The Building Components Group Inc. (TIBCOG) shall not be responsible for any deviation from this design and/or construction details that is not in accordance with ANSI/TPI 1, or for handling, shipping, installation or use of the product. The user is responsible for the proper use of the product. Refer to drawings, TIBCOG-2 for standard plate positions, a seal on this drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per TPI Sec.2. For more information see: This Job's general notes page; TIB HCO; www.tbicog.com; TPI: www.tpiinst.org; HCO: www.tbicoinst.com; www.tbicog.com.



ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

JREF- 1ULY8228Z01

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=1.2 psf, GCpl(+/-)=0.8

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

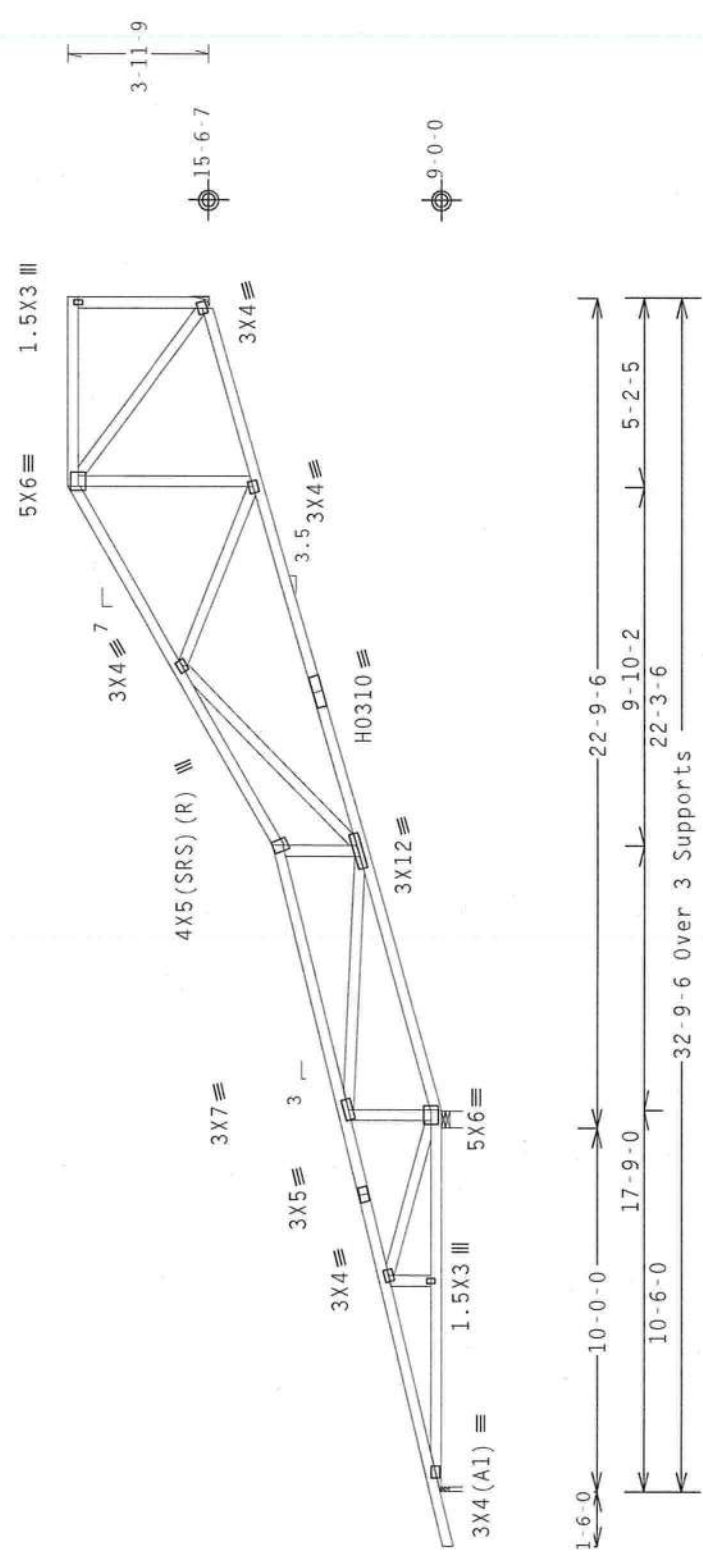
Shim all supports to solid bearing.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Wind Loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-832 U-149

R=1678 U=157 W=5.5"

Design Crit: FBC2010Res/TPI-2007 (STD)

[illegible]

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 2.050'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 2.050'

Roof overhang supports 2.00 psf soffit load.

Truss passed check for 20 psf additional bottom chord live load in areas with 42" high x 24" wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

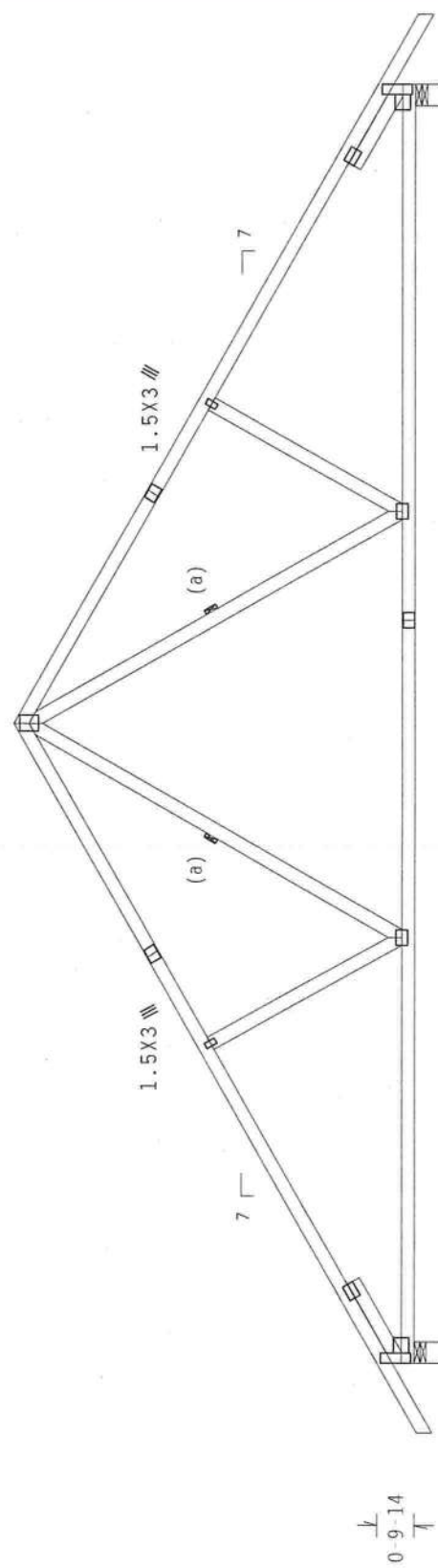
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

4X5 (R) III



4X4 (E3) III

2.5X8 (E3) III

1-6-0

13-9-0

27-6-0 Over 2 Supports

R-1344 U-136 W-5.5"

RL-213/-213

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=20%(0%)/0(0)

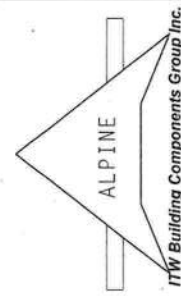
10.03.11

QTY: 2

FL/-/4/-/R/-

Scale = .25"/Ft.

PLT TYP. Wave



ITW Building Components Group Inc.

Haines City, FL 33844

FLCOA #0 278

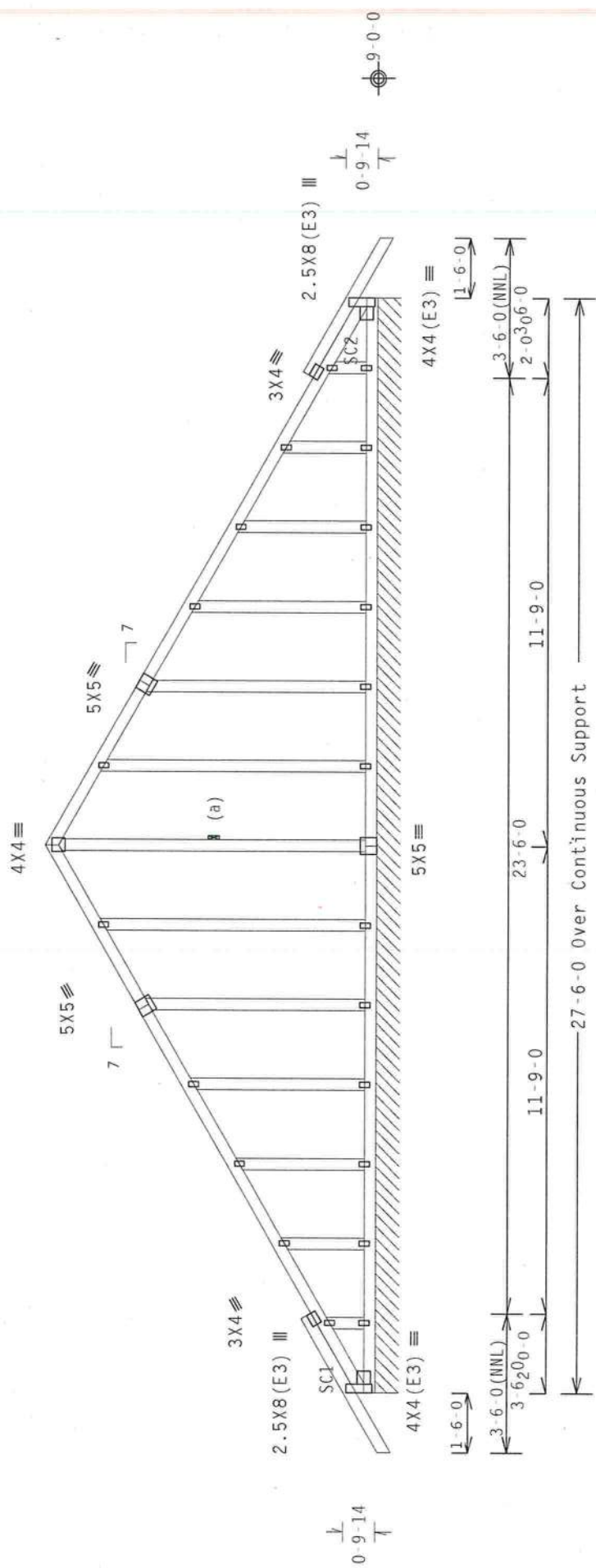


****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MGA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings look-2 for standard plate positions. A seal on this design shall not be construed as a warranty or endorsement of the product or the design. The responsibility for the building design shall remain with the building designer. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec. 2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MGA: www.sbcindustry.com; TCC: www.tccsafe.org

TC LL	20.0 PSF	REF	R8228- 47721
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130013
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	93672
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1ULY8228Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:
Roof overhang supports 2.00 psf soffit load.
See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.
(a) Continuous lateral bracing equally spaced on member.
Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpl(+/-)-0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.



R-91 PLF U=10 PLF W=27-6-0
RL=8/-8 PLF

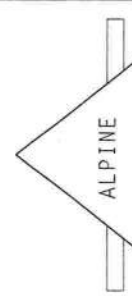
Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/0(0)

Scale = .25" / Ft.

	PLT TYP. Wave	QTY:1	FL/-/4/-/R/-	REF R8228- 47722	JREF- 1ULY8228Z01
		TC LL	20.0 PSF	DATE 05/09/12	
		TC DL	10.0 PSF	DRW HCUSR8228 12130014	
		BC DL	10.0 PSF	HC-ENG DF/DF	
		BC LL	0.0 PSF	SEQN- 93669	
		TOT.LD.	40.0 PSF	DUR.FAC. 1.25	
		SPACING	24.0"		

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings, book 2 for standard plate positions. A seal on this document is required for the building designer to accept responsibility for the design shown. The suitability of the general notes page; ITW BCG: www.itwbcg.com; TPI: www.tpiinst.org; WCA: www.bcsindustry.com; FCC: www.fccsafe.org



ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #1 :T2, T4 2x8 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP #2: 26:
Webs 2x4 SP #3
:LT Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:RT Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.10" due to live load and 0.20" due to dead load.

Bottom chord checked for 10.00 psf non-concurrent live load.
Collar tie braced with continuous lateral bracing at 24"
OC, or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

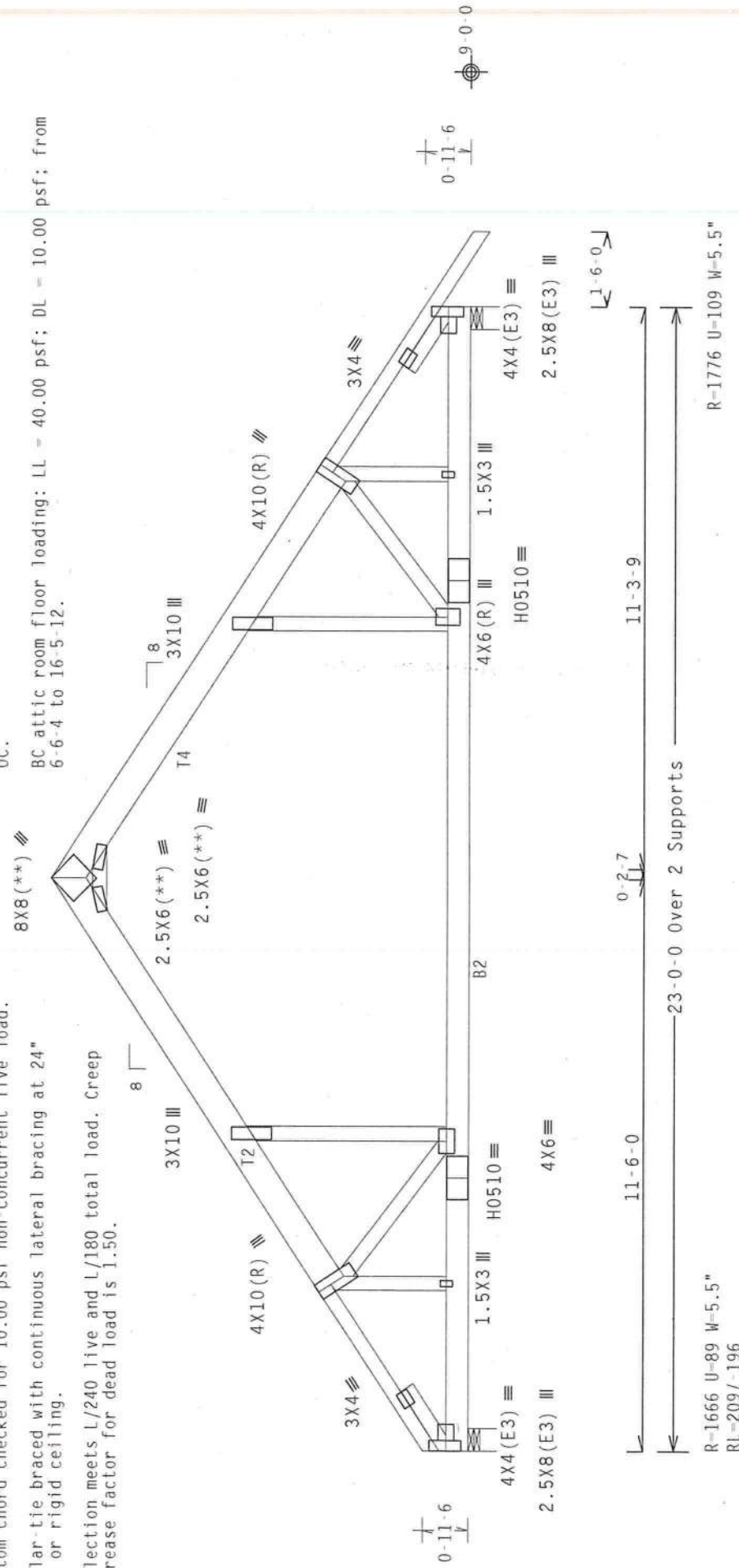
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg.; Located anywhere in roof; RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-6-4 to 16-5-12.



PLT TYP. 20 Gauge HS. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
 $FT/RT=20\%(0\%)/0(0)$

Scale = .3125"/Ft.

QTY:9 FL/-/4/-/-/R/-

10.03.11, 09:52:19

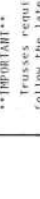
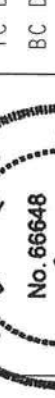
(0) 0 / (%) 0
(glc) / 100 T H

FT/RT=20%

100

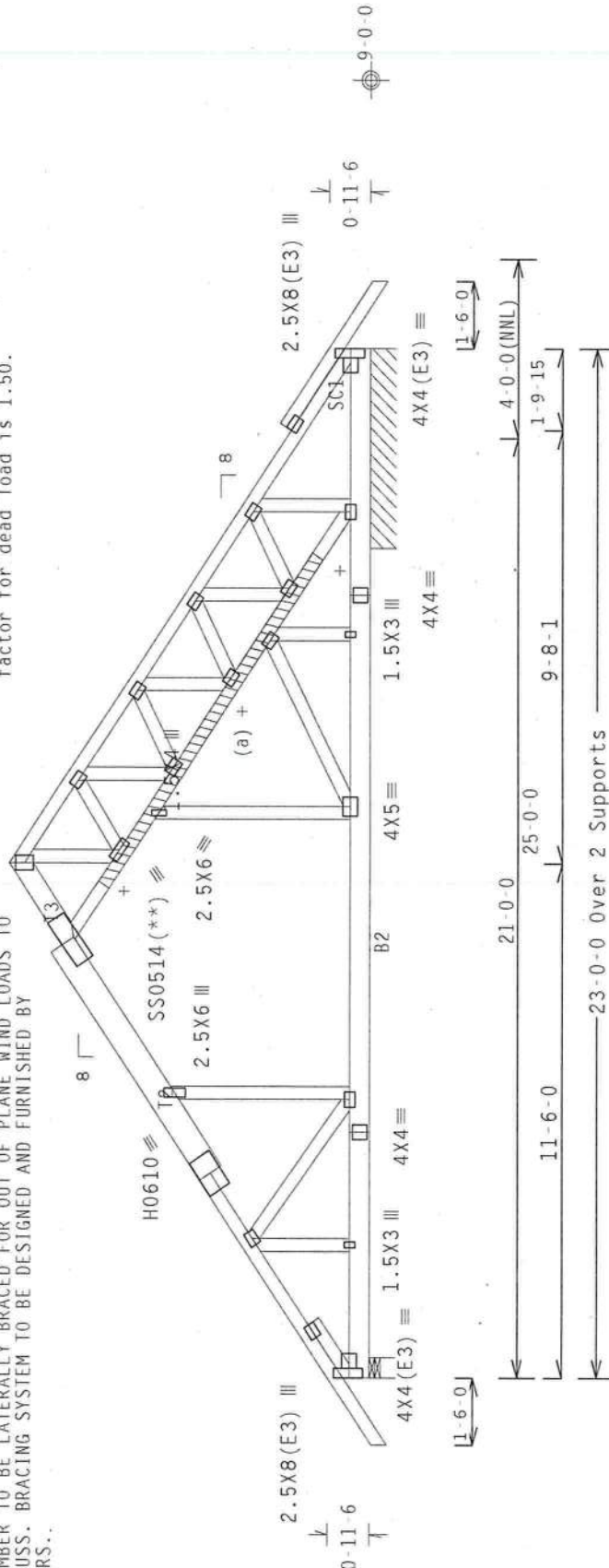
ive

PLT TYP. 20 Gauge HS, Wa

 ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0278	**IMPORTANT** READ AND FOLLOW ALL NOTES ON THIS SHEET! FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and MGA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. 1TH Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ABSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ABSI/TPI 1 Sec.2. For more information see: This Job's general notes page: T1N BCG: www.1thbcg.com ; TPI: www.tpinst.org ; MGA: www.bcsiindustry.com ; ICC: www.iccsafe.org		TC LL	20.0 PSF	REF	R8238 - 47723
			TC DL	10.0 PSF	DATE	05/09/12
			BC DL	10.0 PSF	DRW	HCUSR8228 1213001
			BC LL	0.0 PSF	HC - ENG	DF /DF
			TOT.LD.	40.0 PSF	SEQN -	93692
			DUR.FAC.	1.25		
	SPACING	24.0"	JREF - 1ULY8228Z01			

Top chord 2x4 SP #1 :T2 2x8 SP #1 Dense:
:T3 2x6 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP M-26:
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
Roof overhang supports 2.00 psf soffit load.
(a) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.
Bottom chord checked for 10.00 psf non-concurrent live load.
Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.
BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-6.4 to 12-6-12.
+ MEMBER TO BE Laterally Braced for Out of Plane Wind Loads To Truss. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS..

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II OR III, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Gcpi(+/-)-0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Stacked top chord must NOT be notched or cut in area (NNL). Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-1436 U-90 W-5.5"
RL-224/-226

Note: All Plates Are 3X4 Except As Shown.
PLT TYP. 20 Gauge HS.18 Gauge HS. Design Crit: FBC2010Res/TPI-2007 (STD)
Wave FT/RT=20%(0%)/0(0)

QTY:1	FL/-/4/-/-/R/-	Scale = .25" / Ft.
TC LL	20.0 PSF	REF R8228- 47724
TC DL	10.0 PSF	DATE 05/09/12
BC DL	10.0 PSF	DRW HCUSR8228 12130016
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 94194
DUR.FAC.	1.25	
SPACING	24.0"	JREF- JULY8228Z01



WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100-2 for standard plate positions. A seal on this document is required for the design of the truss. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.micaindustry.com; ICCI: www.iccsafe.org

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Top chord 2x4 SP #1 :T2, T4 2x8 SP #1 Dense:
Bot chord 2x6 SP #2 :B2 2x6 SP M-26:
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 5.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. Gcpi(+/-)=0.18

Right cantilever is exposed to wind

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" O.C.
Bot Chord: 1 Row @12.00" O.C.
Webs : 1 Row @ 4" O.C.

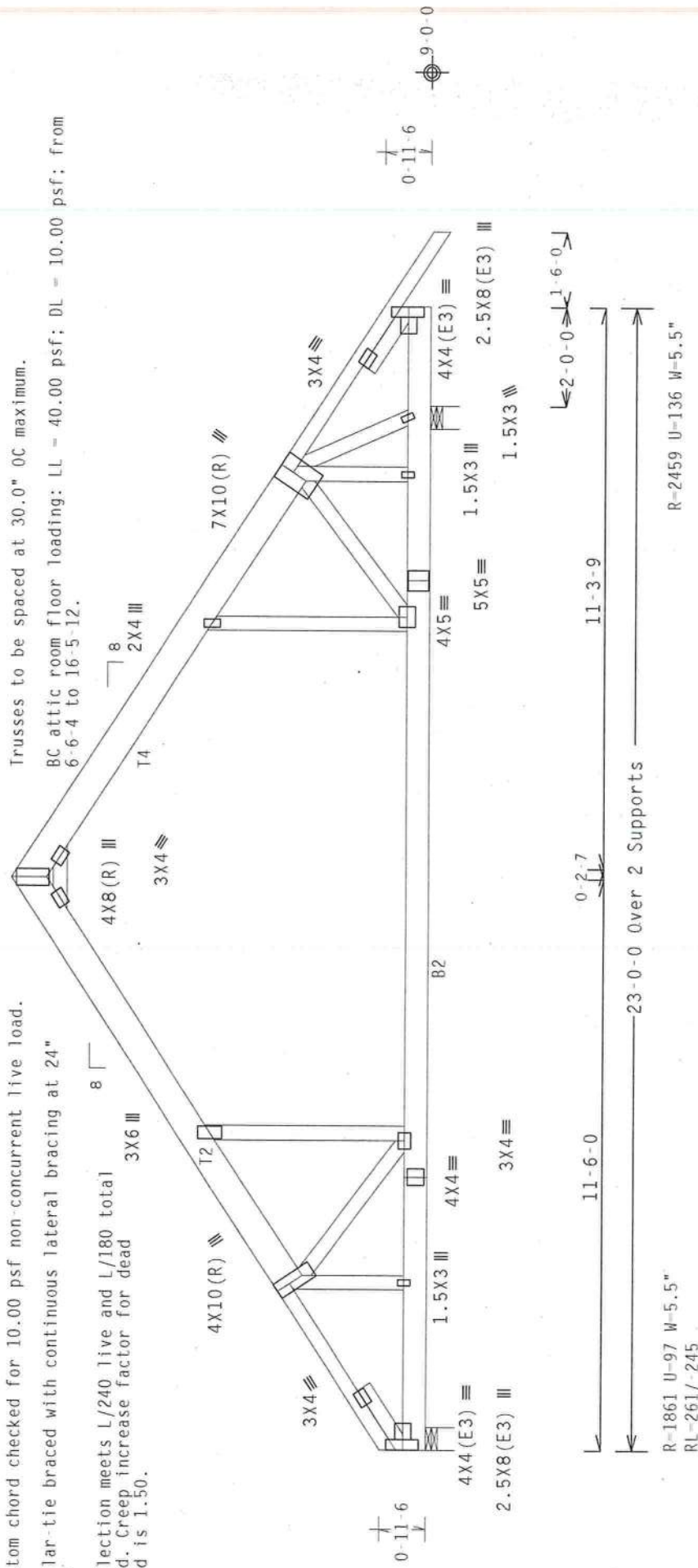
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 30.0" OC maximum.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-6-4 to 16-5-12.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave	10.03.11.0208.20	QTY:1	FL/-/4/-/R/-	Scale = .3125"/Ft.
	WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET! FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.			
	Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections 83, 87 or 810, as applicable.			
	TPI Building Components Group Inc. (TIBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Detailing or cover page listing this drawing. Indicates compliance with standard plate positions. A seal on this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page; TIBCG: www.tibcg.com; TPI: www.tpinet.org; WCA: www.shcindustry.com; ICC: www.iccsafe.org			
	ALPINE			
	TPI Building Components Group Inc. Haines City, FL 33844 FL COA #0278			
	T.C. LL 20.0 PSF			
	T.C. DL 10.0 PSF			
B.C. DL 10.0 PSF				REF R8228- 47725
B.C. LL 0.0 PSF				DATE 05/09/12
TOT.LD. 40.0 PSF				DRW HCUSR8228 12130017
DUR.FAC. 1.25				HC-ENG DF/DF
SPACING 30.0"				SEQN- 94450
				JREF- JULY8228Z01

A circular professional engineer seal for Douglas Fleming, State of Florida, License No. 66648. The seal features the text "DOUGLAS FLEMING" at the top, "LICENSE" on the left, "STATE OF FLORIDA" on the right, and "PROFESSIONAL ENGINEER" at the bottom. The license number "NO. 66648" is in the center, with a signature over it. Two stars are positioned at the top and bottom of the seal.

TC LL	20.0	PSF	REF	R8228- 47727
TC DL	10.0	PSF	DATE	05/09/12
BC DL	10.0	PSF	DRW	HCUSR8228 12130019
BC LL	0.0	PSF	HC-ENG	DF/DF
TOT.LD.	40.0	PSF	SEQN-	93709
DUR.FAC.	1.25			
SPACING	24.0"		JREF-	1ULY8228Z01

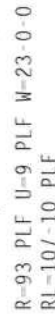
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

10.03.11.02.000000

$$\text{Scale} = .3125"/\text{ft.}$$

IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Crews require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IPI and MICA) for safety practices noted for performing these functions. Installers shall provide temporary bracing per BCSP. Unitless notes otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B2 or B10, as applicable.

A circular professional engineer seal for the State of Florida. The outer ring contains the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom. In the center, there is a signature and the number "12345".

Haines City, FL 33844
FL COA #0278

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

:LL Slider 2x6 SP #2: BLOCK LENGTH = 1.860'
:RT Slider 2x6 SP #2: BLOCK LENGTH = 1.860'

Roof overhang supports 2.00 psf soffit load.

(H1) = (J) Hanger not calculated (2)2x6 SP M-26 supporting member.

(a) continuous lateral bracing, equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

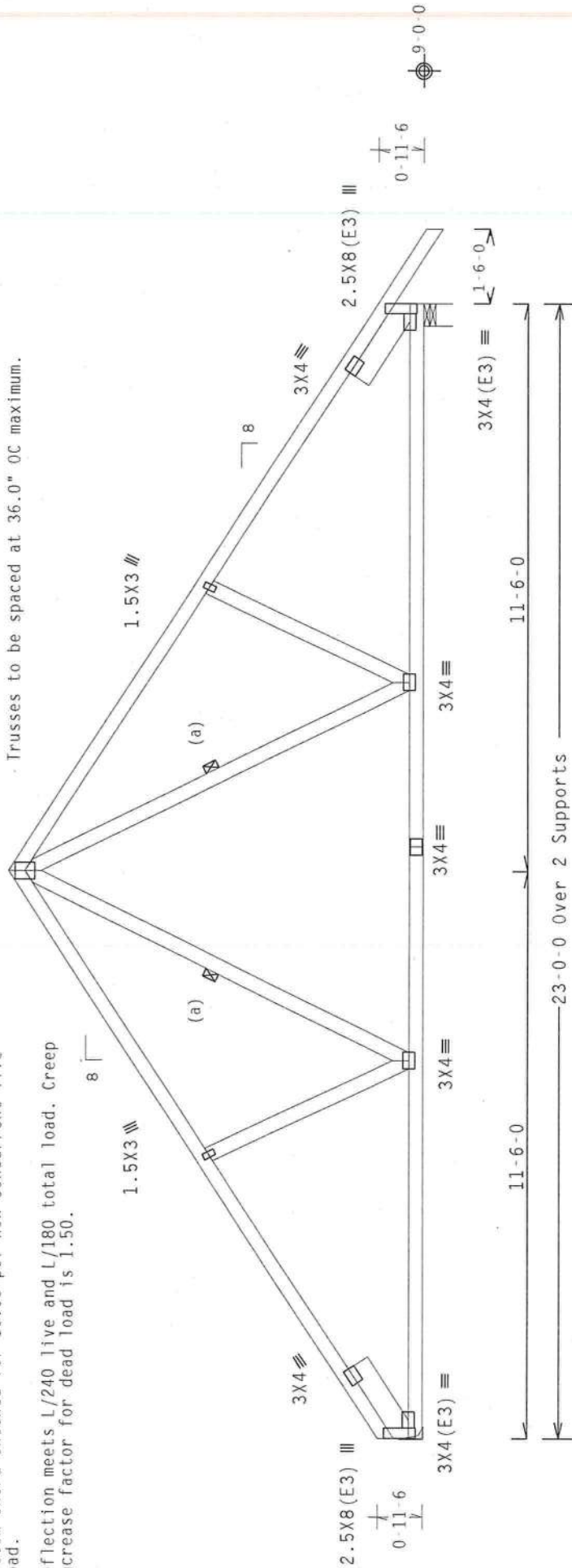
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 6Cpi(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 36.0" OC maximum.



R-1445 U-137

RL-315/-295 H-Simpson HUS26

Supported Member Face: (4) 0.148"x3" nails

Supporting Member Face: (14) 0.148"x3" nails

Supporting Member : (2) 2x6 SP #2

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=20%(0%)/0(0)

Scale = .3125"/Ft.

QTY: 1

10.03.11

PLT TYP. Wave

REF R8228- 47729

DATE 05/09/12

DRW HCUSR8228 12130021

HC-ENG DF/DF

SEQN- 94421

JREF- JULY8228Z01

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and HCCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (TIBC) shall not be responsible for any deviation from this design, any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on this drawing, unless noted otherwise, indicates acceptance of professional engineering. The responsibility for the design, construction and installation of the structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; TIBC-BCG: www.tibcgroup.com; TPI: www.tpiinst.org; HCCA: www.hccsafe.org



rtw Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

Top chord	2x4	SP #1
Bot chord	2x4	SP #1
Webs	2x4	SP #3

Block	SP #1	SP #2	Block Length
1	1	1	1
2	1	1	1
3	1	1	1
4	1	1	1
5	1	1	1
6	1	1	1
7	1	1	1
8	1	1	1
9	1	1	1
10	1	1	1
11	1	1	1
12	1	1	1
13	1	1	1
14	1	1	1
15	1	1	1
16	1	1	1
17	1	1	1
18	1	1	1
19	1	1	1
20	1	1	1
21	1	1	1
22	1	1	1
23	1	1	1
24	1	1	1
25	1	1	1
26	1	1	1
27	1	1	1
28	1	1	1
29	1	1	1
30	1	1	1
31	1	1	1
32	1	1	1
33	1	1	1
34	1	1	1
35	1	1	1
36	1	1	1
37	1	1	1
38	1	1	1
39	1	1	1
40	1	1	1
41	1	1	1
42	1	1	1
43	1	1	1
44	1	1	1
45	1	1	1
46	1	1	1
47	1	1	1
48	1	1	1
49	1	1	1
50	1	1	1
51	1	1	1
52	1	1	1
53	1	1	1
54	1	1	1
55	1	1	1
56	1	1	1
57	1	1	1
58	1	1	1
59	1	1	1
60	1	1	1
61	1	1	1
62	1	1	1
63	1	1	1
64	1	1	1
65	1	1	1
66	1	1	1
67	1	1	1
68	1	1	1
69	1	1	1
70	1	1	1
71	1	1	1
72	1	1	1
73	1	1	1
74	1	1	1
75	1	1	1
76	1	1	1
77	1	1	1
78	1	1	1
79	1	1	1
80	1	1	1
81	1	1	1
82	1	1	1
83	1	1	1
84	1	1	1
85	1	1	1
86	1	1	1
87	1	1	1
88	1	1	1
89	1	1	1
90	1	1	1
91	1	1	1
92	1	1	1
93	1	1	1
94	1	1	1
95	1	1	1
96	1	1	1
97	1	1	1
98	1	1	1
99	1	1	1
100	1	1	1

Roof overhang supports 2.00 psf soffit load.

(H1) - (J) Hanger not calculated (2)2x6 SP #1 Dense supporting member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 42.0" OC maximum.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

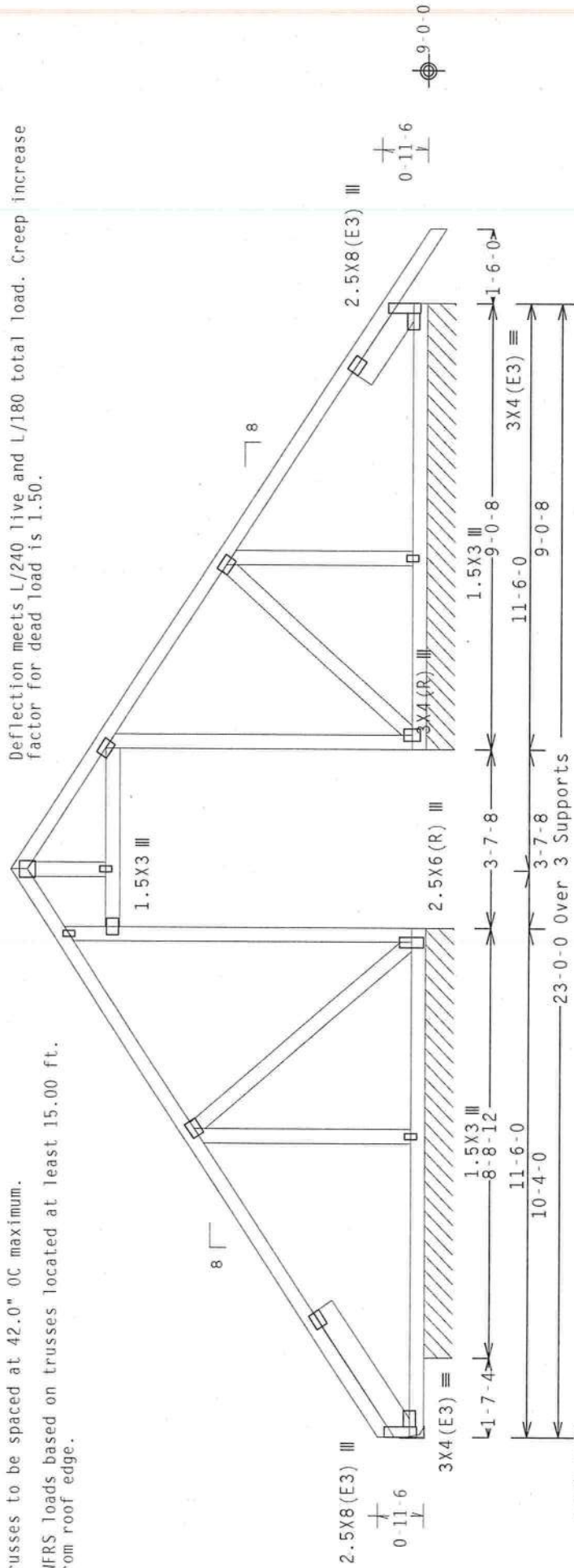
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 8.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=1.2 psf. GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-581 U-69

RL-368/-345 H-Simpson LU26

Supported Member Face: (4) 0.148"x1.5" nails
Supporting Member Face: (6) 0.148"x3" nails

Supporting Member : (2) 2x6 Sp #2

R-134 PLF U=24 PLF W=8-8-12

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

Scale = .3125"/Ft.

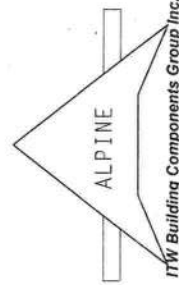
QTY:1 FL/-/4/-/-/R/-/-

10.03.11.0209156

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**

Trussess require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BECS (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BECS. Installers noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BECS sections B3, B30 and B31, as applicable.

ITW Building Components Group Inc. (IBCG) shall not be responsible for any deviation from this design or specification for any reason, including but not limited to, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings IBCO-2 for standard plate positions. A seal on this drawing or cover plate indicating this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This Job's Specification, Section 011800; web: www.ibcgroup.com; TPI: www.tpi.net; MICA: www.micaindust.com; ECI: www.eci-usa.com



Haines City, FL 33844
FL COA #0 278

JREF- 1ULY8228Z01



21.

DUR.FAC. 1.25

SPACING 42 0"

TRFE- 1111 Y8228701

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:
Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 19.83 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpi(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

WARNING! 1 UNPLATED JOINT(S)

QTY: 3 FL / - / 4 / - / - / R / -

Scale = .5" / Ft.

REF R8228- 47731

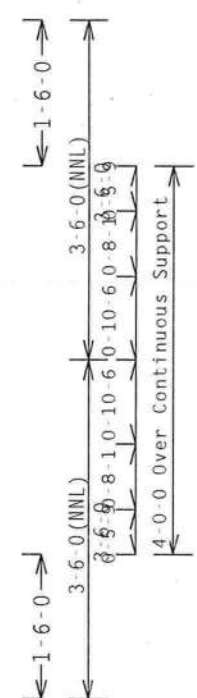
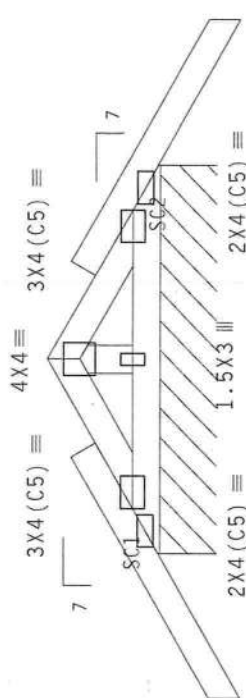
DATE 05/09/12

DRW HCUSR8228 12130023

HC-ENG DF/DF

SEON- 93764

JREF- 1ULY8228Z01



R-134 PLF U-26 PLF W-4-0-0
RL=17/-17 PLF

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/0(0)

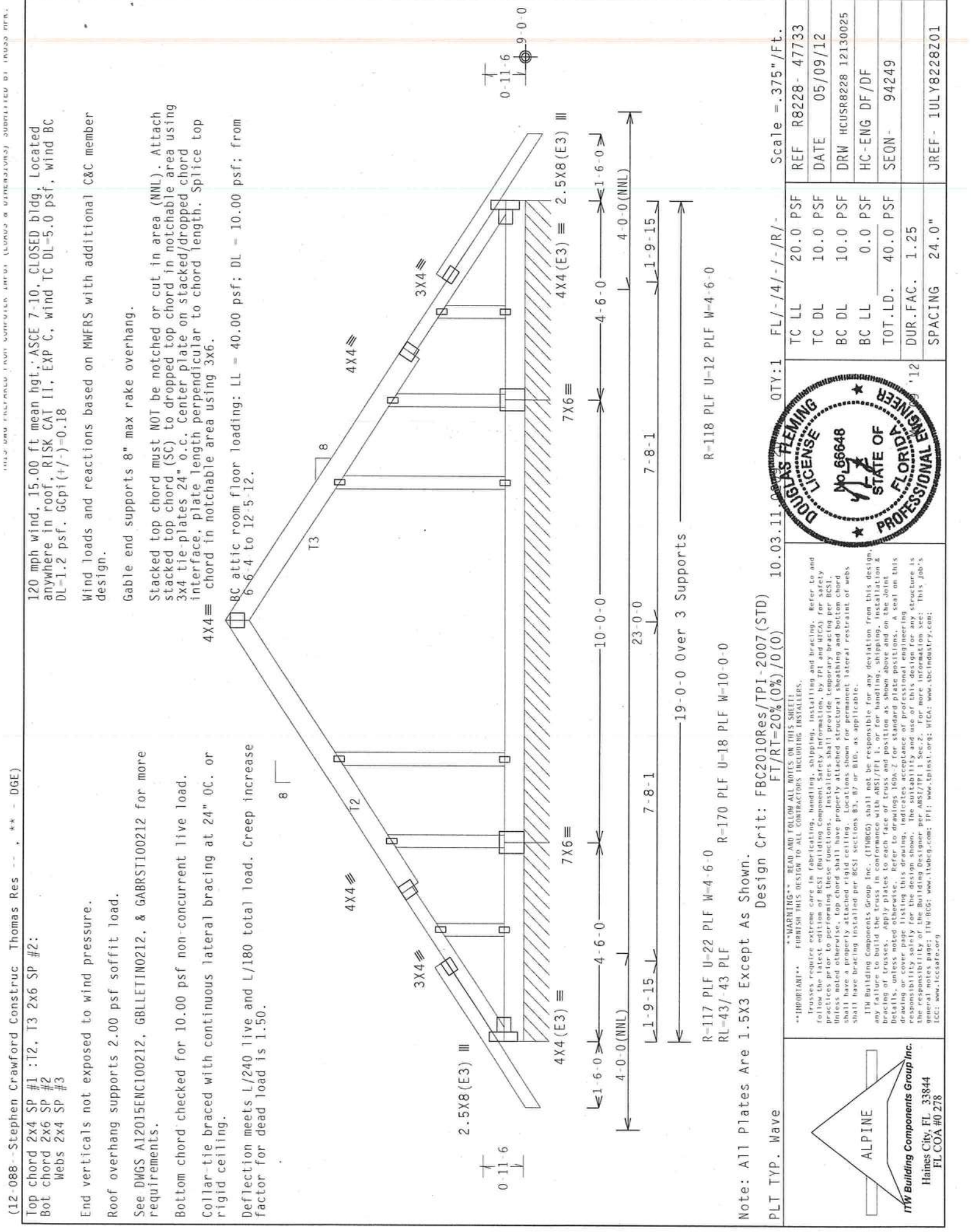
PLT TYP. Wave

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING THIS SHEET.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Ingress noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs. Shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. The responsibility of the Building Designer per ANSI/TPI 1 Sec.2, for more information see: This Job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpiinc.org; MICA: www.alcindustry.com; ICC: www.iccsafe.org



Top chord 2x4 SP #1
Bot chord 2x4 SP #1

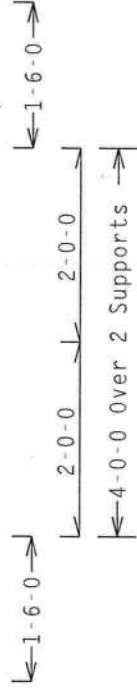
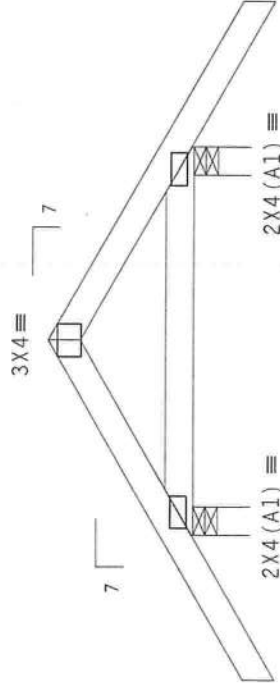
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 19.99 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

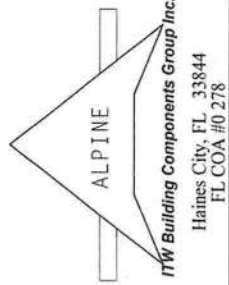


R-268 U-53 W-3.5"
RL-67/-67

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20% (0%)/0 (0)

QTY: 12 FL/-4/-/-/R/-
Scale = .5"/Ft.

PLT TYP. Wave



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B5, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings Table 2 for standard plate positions. A seal on this drawing is required to indicate the date of issuance. The seal shall be the responsibility of the structure fabricator. The responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	20.0 PSF	REF	R8228- 47734
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130026
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN	93752
DUR.FAC.	1.25		
SPACING	24.0"	JREF	1ULY8228Z01

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-5.0 psf. GCpi (+/-) 0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

ACCEPTED FOR PUBLICATION



R=1646 U=185 W=7.778"

Scale = .1875" / Ft

Scale = .1875"/Ft

FL/-/4/-/-/R/-/-

QTY:1

10.03.11.0209-20

FT/RT=20%(0%)/0(0)

1

PLT TYP. Wave

****IMPORTANT****
-WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
 FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS,

Troosens require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety) information, by IP1 and A16A for BCSP practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. If/less noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7, or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build in conformance with ABSE/TFL 1, or for handling, shipping, installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the detail. Details, unless noted otherwise. Refer to drawings 1606-2 for standard plate positions. A seal on this plate indicates acceptance of professional engineering responsibility for the design. The responsibility for the design is the responsibility of the design engineer per ABSE/TFL 1 Sec. 2. For more information see: THIS JOB's general notes page. ITW-BCG: www.itwbcg.com; TFL: www.tflinst.org; WCA: www.sbcindustry.com; www.lccsafe.org



ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

JREF- 1ULY8228Z01

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

:Rt Slider 2x4 SP #3: BLOCK LENGTH = 1.791'

Roof overhang supports 2.00 psf soffit load.

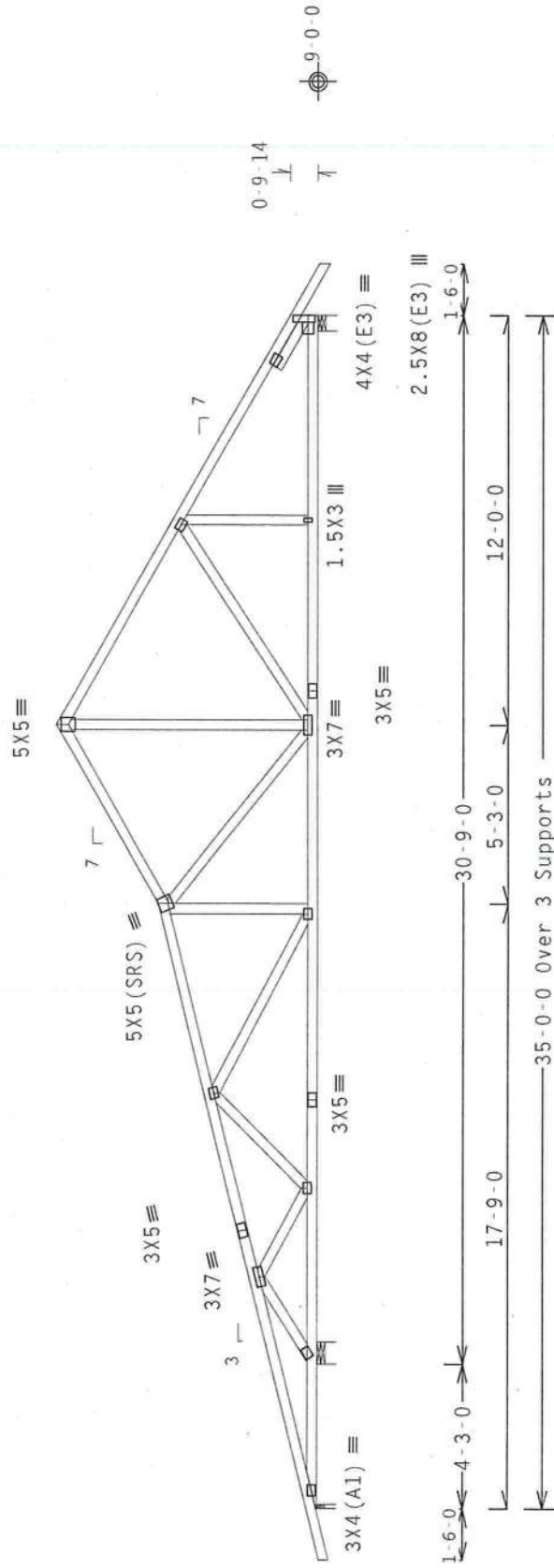
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-152 U-102 W-1.5"

R1=178/-132 R=1745 U=193 W=7.778"

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

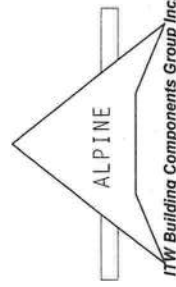
PLT TYP. Wave

FT/RT=20% (0%) / 0 (0)

10.03.11.0200-20

QTY:1 FL/-/4/-/-/R/-

Scale = .1875"/Ft.



Haines City, FL 33844
FL COA #0 278

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the instructions on the BCSI (Building Code Safety Institute) bracing manual for the correct bracing practices prior to performing these functions. Installers shall provide temporary bracing per BCSI instructions unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid deck. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (TIBCOG) shall not be responsible for any deviation from this design, or any other drawings, specifications, or applicable codes. The contractor shall be responsible for the erection and bracing of trusses, apply plates to each face of truss and position as shown above and on the Joiner Details, unless noted otherwise. Refer to drawings TD9-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ASCE/PEI Sec. 2. For more information see: This Job's General notes page; TIBCOG web site: www.tibcoinc.com; TPI website: www.tpi-usa.com.



TC LL	20.0 PSF	REF	R8228 - 47737
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130029
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN -	93684
DUR.FAC.	1.25		
SPACING	24.0"	JREF -	1ULY8228Z01

Top chord 2x4 SP #1
Bot chord 2x6 SP M-26 :B2 2x6 SP #1 Dense:
:B3 2x4 SP #1:
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 3.794'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.791'

Special loads

(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 61 pif at 0.00 to 61 pif at 17.75
TC- From 63 pif at 17.75 to 63 pif at 23.00
TC- From 63 pif at 23.00 to 63 pif at 35.00
BC- From 20 pif at 0.00 to 20 pif at 3.06
BC- From 10 pif at 3.06 to 10 pif at 13.94
BC- From 20 pif at 13.94 to 20 pif at 35.00
PLB- 1041.11 lb Conc. Load at (3.06, 9.04), (5.06, 9.04), (7.06, 9.04)
(9.06, 9.04)
PLB- 1444.59 lb Conc. Load at (10.48, 9.04)
PLB- 581.39 lb Conc. Load at (13.94, 9.04)

2 COMPLETE TRUSSES REQUIRED

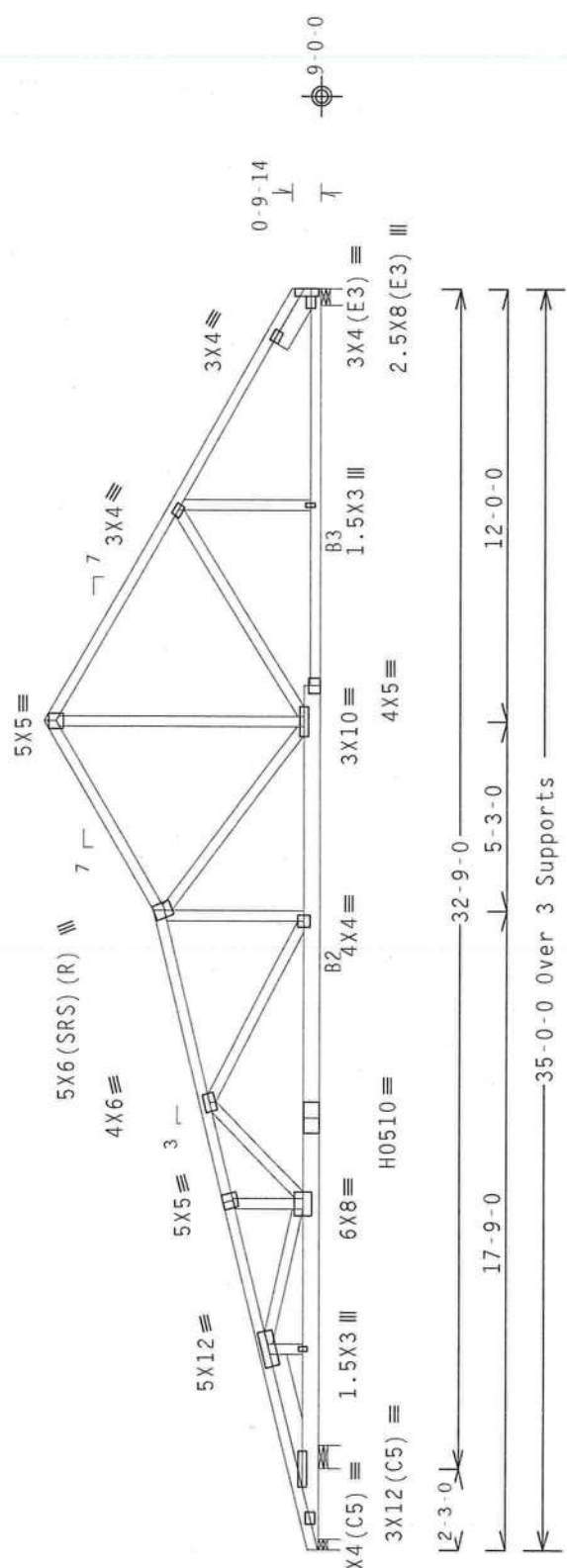
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 8.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -578# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 5.50 ft from roof edge, RISK CAT II, EXP C, wind TC DL-5.0 psf, wind BC DL-1.2 psf. GCpi (+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=579 RW-42 U=0 W=3.5"
R=7168 U=865 W=7.778"

R-2280 U=382 W=5.5"

PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/0(0)

10.03.11

QTY: 1

FL/-/4/-/-/R/-

Scale = .1875"/Ft.

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B5, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. The building owner and installer shall be responsible for the proper installation of the truss on the drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: TPI-BCG: www.tpi-bcg.com; TPI: www.tpinet.org; MICA: www.sbcindustry.com; ICC: www.iccsafe.org

DONALD S. FLEMING
LICENSE
No 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

10.03.11

12

REF R8228- 47738
DATE 05/09/12
DRW HCUSR8228 12130030
HC-ENG DF/DF
SEQN- 94489
JREF- 1ULY8228Z01

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25
SPACING 24.0"

12

Top chord 2x4 SP #1
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Special loads

----- (Lumber Dur.Fac.-1.25 / Plate Dur.Fac.-1.25)
TC- From 60 plf at 0.00 to 60 plf at 3.42
BC- From 20 plf at 0.00 to 20 plf at 3.42
BC- 818.70 lb Conc. Load at 1.52

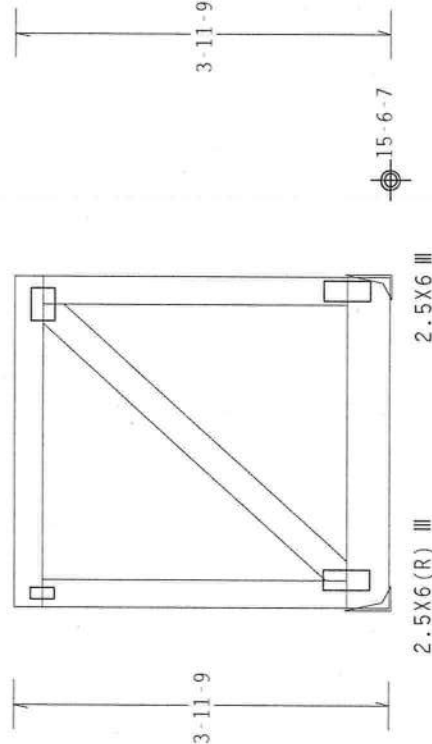
Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

MWFRS loads based on trusses located at least 9.75 ft. from roof edge.

1.5X3 III 3X4 III



3-5-0 Over 2 Supports

R-591 U-113 H-Simpson LSU26
Supported Member Face: (5) 0.148"x1.5" nails
Supporting Member Face: (6) 0.148"x3" nails
Supporting Member : (1) 2x6 SP #1 Dense

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

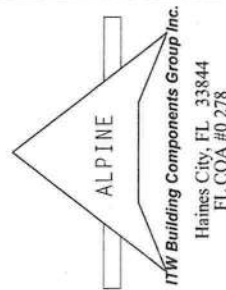
QTY: 1 FL/- /4/- /- /R/-

Scale = .5" / Ft.



WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
TYPICAL DESIGN TO ALL CONNECTIONS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Trusses shall be properly braced and bracing shall be properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections 93, 97 or 810, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design and/or to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing. The user of this design shall be responsible for the proper use of the design. Details, unless noted otherwise, refer to drawings 100A-2 for details. The user shall be responsible for listing this drawing, indicating acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinet.org; MICA: www.stcindustry.com; ICC: www.iccsafe.org



TC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R8228- 47739
DATE	05/09/12
DRW	HCUSR8228 12130031
HC-ENG	DF/DF
SEQN-	94318
JREF-	1ULY8228Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #1::Stack Chord SC2 2x4 SP #1:

End verticals not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

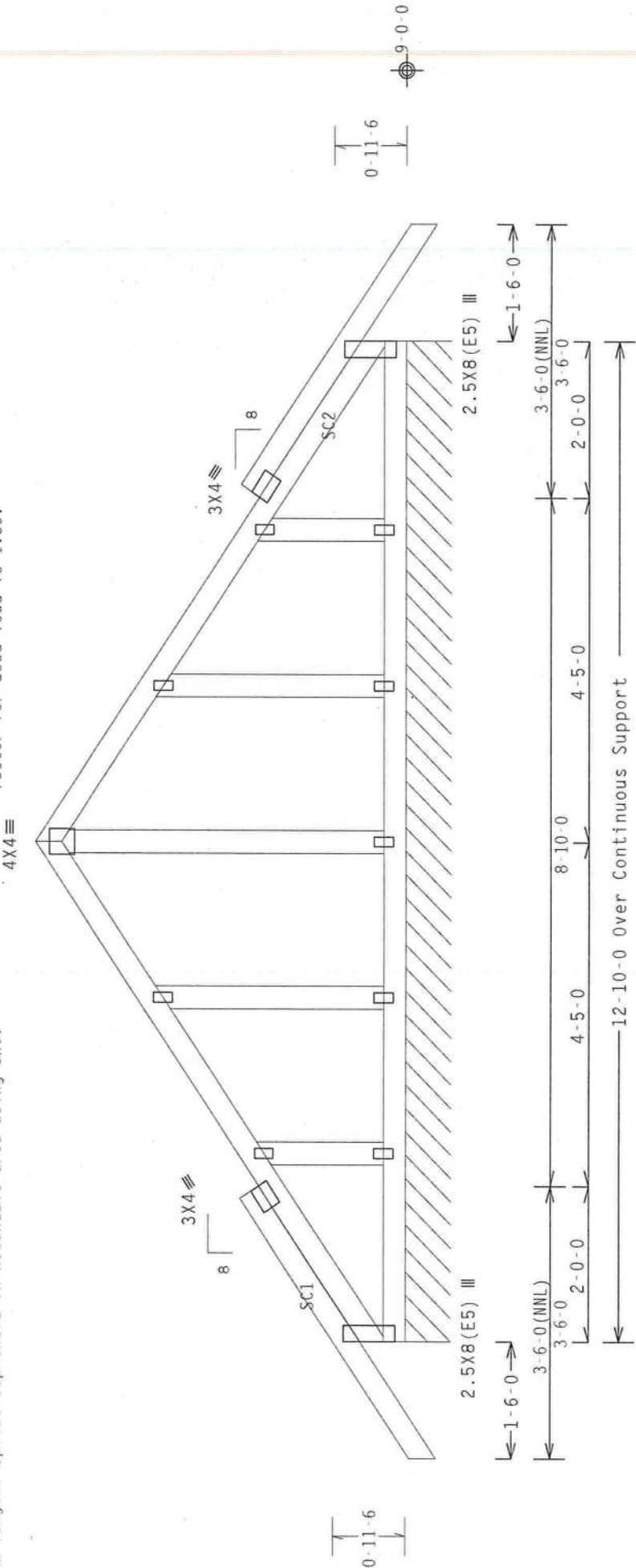
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=100 PLF U=10 PLF W=12-10-0
RL=11/-11 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007 (STD)

FT/RT=20%(0%) / 0 (0)

10.03.11.02000000

QTY: 1

FL/- /4/- /- /R/-

Scale = .5" / Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and UTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B1, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on this drawing covers page 1 of 2. This drawing is the property of ITW Building Components Group Inc. and the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. For more information see: This job's general notes page; ITW BCG: www.itwbcg.com; TPI: www.tpinet.org; UTCA: www.bcsiindustry.com; ICC: www.iccsafe.org

ALPINE

ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

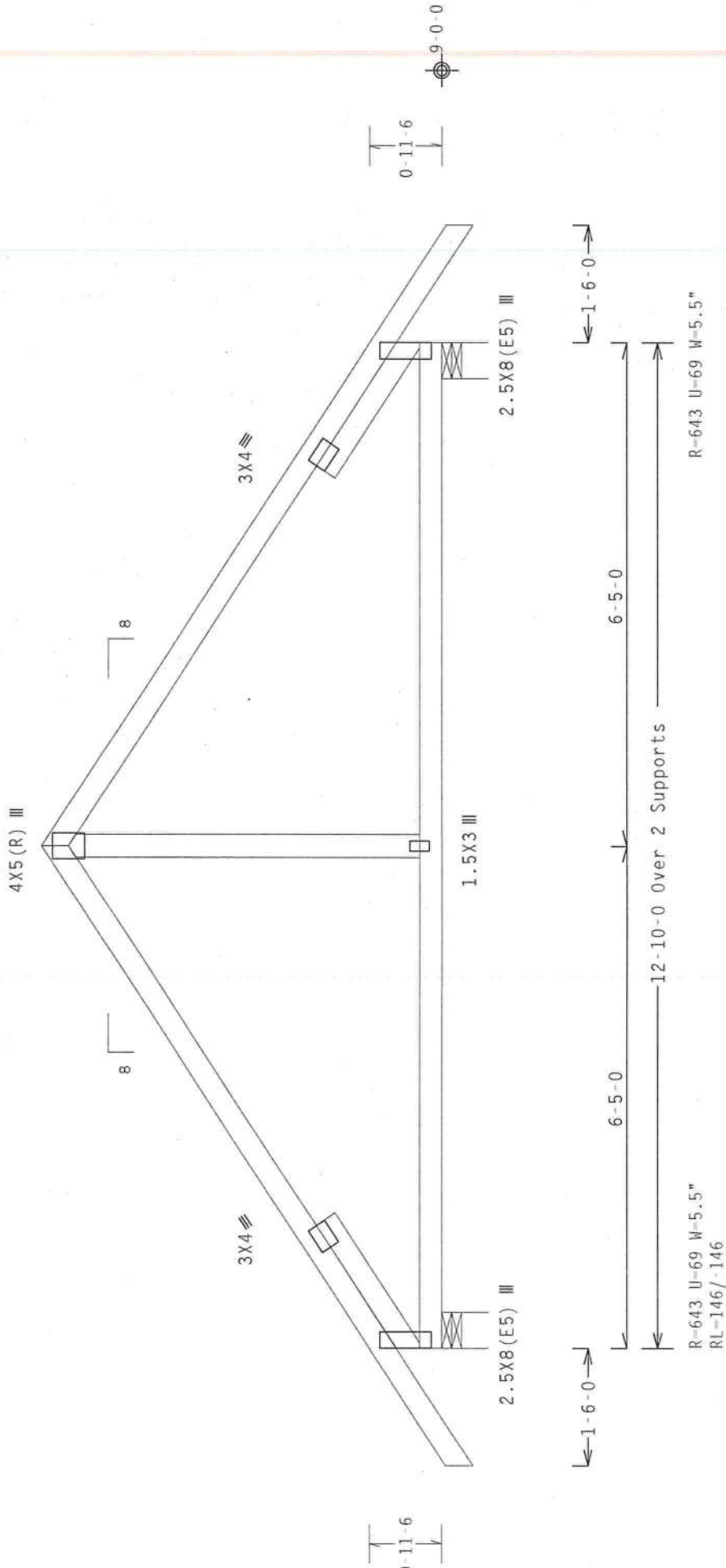


TC LL	20.0 PSF	REF	R8228- 47740
TC DL	10.0 PSF	DATE	05/09/12
BC DL	10.0 PSF	DRW	HCUSR8228 12130032
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	93663
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	IULY8228Z01

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 2.053'
:Rt Slider 2x4 SP #3: BLOCK LENGTH = 2.053'
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 6Cpi(+/-)-0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007 (STD)

PLT TYP. Wave	QTY: 1	FL / - / 4 / - / - / R / -	Scale = .5" / Ft.
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET **IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bolted chord shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.2. For more information see: This Job's ITC: www.tccsafe.org ITW Building Components Group Inc. Haines City, FL 33844 FL COA #0 278	TC LL	20.0 PSF	REF R8228- 47741
	TC DL	10.0 PSF	DATE 05/09/12
	BC DL	10.0 PSF	DRW HCUSR8228 12130033
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	40.0 PSF	SEQN- 93659
	DUR.FAC.	1.25	
	SPACING	24.0"	JREF- 1ULY8228Z01

Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Webs	2x4	SP	#3

:Lt Slider	2x4 SP #3:	BLOCK LENGTH =	2.053'
:Rt Slider	2x4 SP #3:	BLOCK LENGTH =	2.053'

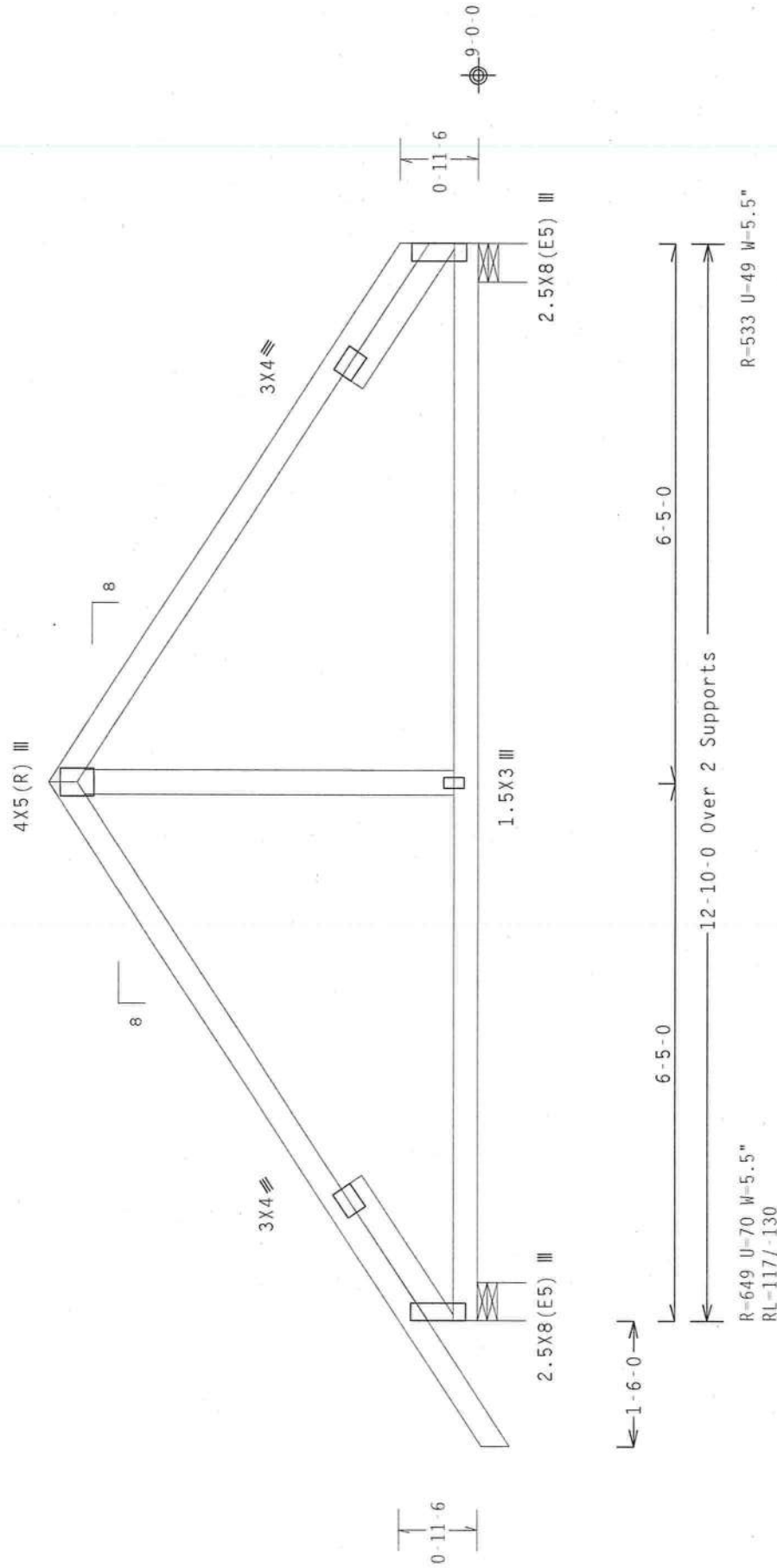
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, Exp C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007 (STD)

PLT TYP. Wave

QTY:5 FL/-/4/-/-/R/-

Scale = .5"/Ft.

* **WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

[illegible]

ITW Building Components Group, Inc. (ITWBCG) shall be responsible for any deviation from this design. Any failure to build the truss in conformance with AISC/TPI 1 for bracing, detailing, installation or bracing of trusses, apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering and responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per AISC/TPI 1 Sec. 2. For more information see: This Job's General Notes page; ITW-BG00: www.bcg.com; TPI: www.tpiinc.org; HFA: www.structure.com;

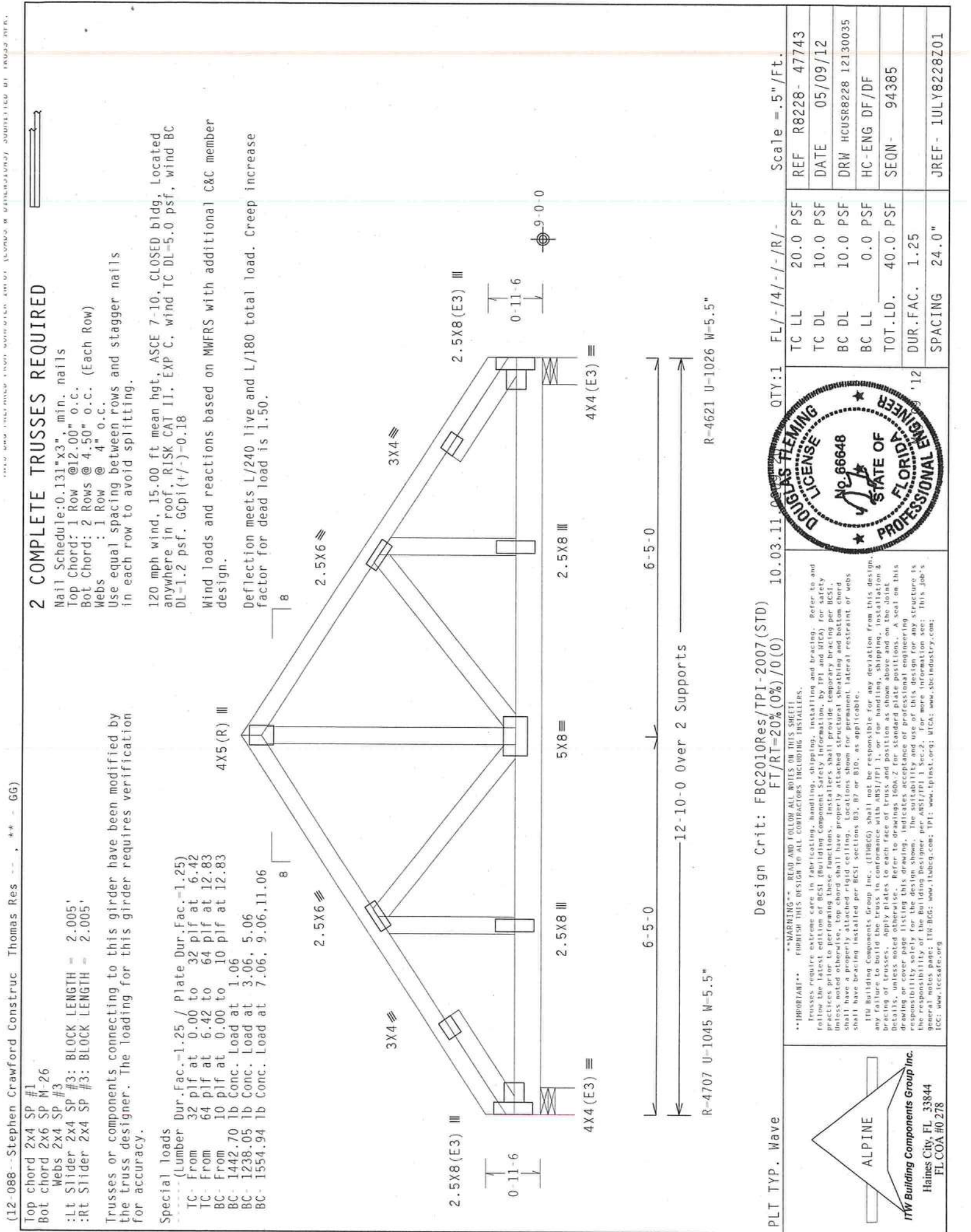


ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

Haines City, FL 33844
FL COA #0 278

nes City, FL 338
FL COA #0 278

nes City, FL 338
FL COA #0 278



(12-088--Stephen Crawford Construc Thomas Res --, ** - GG) Top chord 2x4 SP #1 Bot chord 2x6 SP M-26 Webs 2x4 SP #3 :LT Slider 2x4 SP #3: BLOCK LENGTH = 2.005' :RT Slider 2x4 SP #3: BLOCK LENGTH = 2.005' Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy. Special loads ----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25) TC From 32 plf at 0.00 to 32 plf at 6.42 TC From 64 plf at 6.42 to 64 plf at 12.83 BC From 10 plf at 0.00 to 10 plf at 12.83 BC 1442.70 lb Conc. Load at 1.06 BC 1238.05 lb Conc. Load at 3.06, 5.06 BC 1554.94 lb Conc. Load at 7.06, 9.06, 11.06		2 COMPLETE TRUSSES REQUIRED Nail Schedule: 0.131"x3", min. nails Top Chord: 1 Row @ 12.00" o.c. Bot Chord: 2 Rows @ 4.50" o.c. (Each Row) Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting. 120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=1.2 psf, GGpi(+/-)-0.18 Wind loads and reactions based on MWFRS with additional C&C member design. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.		PLT TYP. Wave Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=20%(0%)/0(0) 10.03.11 QTY: 1 FL/-/4/-/R/- Scale = .5" / Ft.		REF R8228- 47743 DATE 05/09/12 DRW HCUSR8228 12130035 HC-ENG DF/DF SEQN- 94385 JREF- 1ULY8228Z01	
ALPINE rtw Building Components Group Inc. Haines City, FL 33844 FL COA #0 278		DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER		TC LL 20.0 PSF TC DL 10.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.25 SPACING 24.0"		12	

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf, GCpi(+/-)0.18

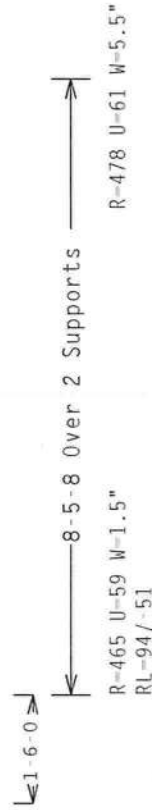
Right end vertical not exposed to wind pressure.

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/0(0)

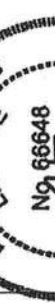
QTY:1 FL/-/4/-/-/R/-
Scale = .375"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (IBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise. Refer to drawings I600.2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: this job's general notes page; TPI-BCG: www.itbucg.com; TPI: www.tpinst.org; MICA: www.sbcindustry.com; ICC: www.iccsafe.org

REF	R8228- 47744
DATE	05/09/12
DRW	HCUSR8228 12130036
HC-ENG	DF/DF
SEQN	94361
TOT.LD.	40.0 PSF
DUR.FAC.	1.25
SPACING	24.0"





ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844

FL COA #0 278

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3

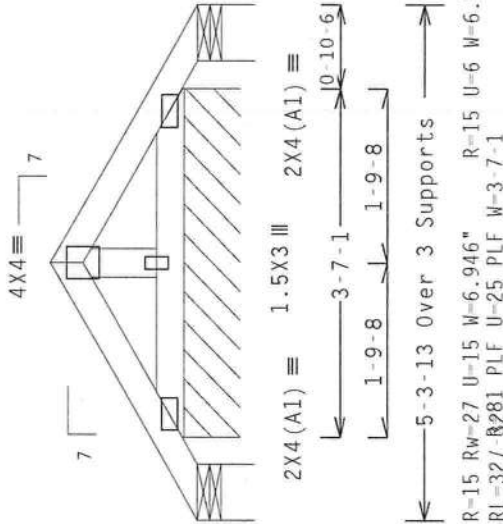
120 mph wind, 20.28 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=1.2 psf, GCpi(+/-)-0.18

Wind loads and reactions based on MWFRS with additional C&C member design. Refer to DWG PB160100212 for piggyback details.

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 63 plf at 0.00 to 63 plf at 2.66
TC- From 63 plf at 2.66 to 63 plf at 5.32
BC- From 4 plf at 0.00 to 4 plf at 5.32

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209-20

QTY:17 FL/-/4/-/R/-

Scale = .5" / Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached ceiling. All connections shall be made in accordance with the BCSI specifications for permanent lateral restraint of webs. Shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (TIBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each end of truss and position as shown above and on the joint. Details are shown for reference only. Refer to the BCSI specifications for details. The responsibility for the design shown, indicates acceptance of professional engineering seal on this drawing or cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; TIB-BCG; www.tibcg.com; TPI: www.tpiinst.org; MICA: www.sbcindustry.com; ICC: www.iccsafe.org

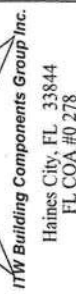
ALPINE

rtw Building Components Group Inc.
Haines City, FL 33844
FL COA #0278



TC LL	20.0 PSF	REF	R8228-	47745
TC DL	10.0 PSF	DATE	05/09/12	
BC DL	10.0 PSF	DRW	HCUSR8228	12130037
BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.	40.0 PSF	SEQN-	94338	
DUR.FAC.	1.25			
SPACING	24.0"	JREF-	1ULY8228Z01	

R-120 U-41 W-6.946"

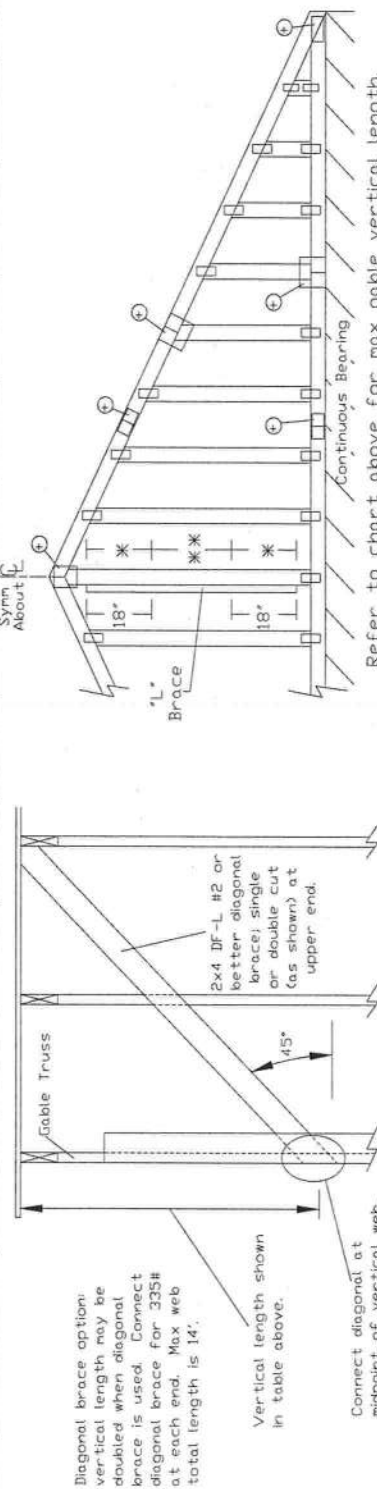




Gable Stud Reinforcement Detail

Dr: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dr: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	Gable Vertical Species	Brace Grade	2x4		(1) 1x4 "L" Brace		(2) 2x4 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace		Group B
			No Braces	Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
12" o.c.	SPF	#1 / #2	4' 10"	8' 2"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 6"	14' 0"	14' 0"	14' 0"
	HF	#3	4' 7"	7' 9"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"
	Standard	Stud	4' 7"	8' 1"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"
	SP	#1	4' 11"	8' 3"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"
24" o.c.	DFL	#2	4' 10"	8' 2"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"
	Standard	#3	4' 7"	6' 11"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"
	SPF	#1 / #2	4' 7"	6' 0"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	14' 0"	14' 0"	14' 0"
	HF	Stud	5' 6"	9' 5"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 7"	14' 0"	14' 0"	14' 0"
16" o.c.	SP	#3	5' 3"	9' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"
	DFL	Stud	5' 3"	9' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"
	SPF	#1	5' 8"	9' 5"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"
	HF	#2	5' 6"	9' 5"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"
12" o.c.	SP	#3	5' 3"	8' 6"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"
	DFL	Stud	5' 3"	7' 4"	7' 4"	7' 10"	9' 9"	10' 5"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"
	SPF	#1 / #2	6' 1"	10' 4"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	5' 9"	10' 2"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SP	#3	6' 2"	10' 5"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	Stud	5' 9"	9' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#1	5' 9"	9' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	5' 9"	9' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



Refer to chart above for max gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information, by IPI and VITA for safety information on trusses. Trusses shall be installed in accordance with the BCSI and VITA instructions. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.

ITV Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any other purpose is the responsibility of the user. For more information see this job's general notes page and these web sites: ITVBCCI www.itvbcci.com, TPI www.tpinet.org, VITA www.vitaindustry.org, ICC www.iccsafe.org



Earth City, MO 63045



MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

REF	ASCE 7-10-GABI2015
DATE	2/14/12
DRWG	A12015ENC100212

Bracing Group Species and Grades:			
Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

Group A:		Group B:	
Spruce-Pine-Fir	#1 / #2	Douglas Fir-Larch	#1
	Standard		#2
	Stud		Standard
#3	Standard	#3	Standard
	Standard		Standard
	Standard		Standard
Southern Pine***			
Hem-Fir	#2	Douglas Fir-Larch	#1
	#3		#2
	Standard		Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

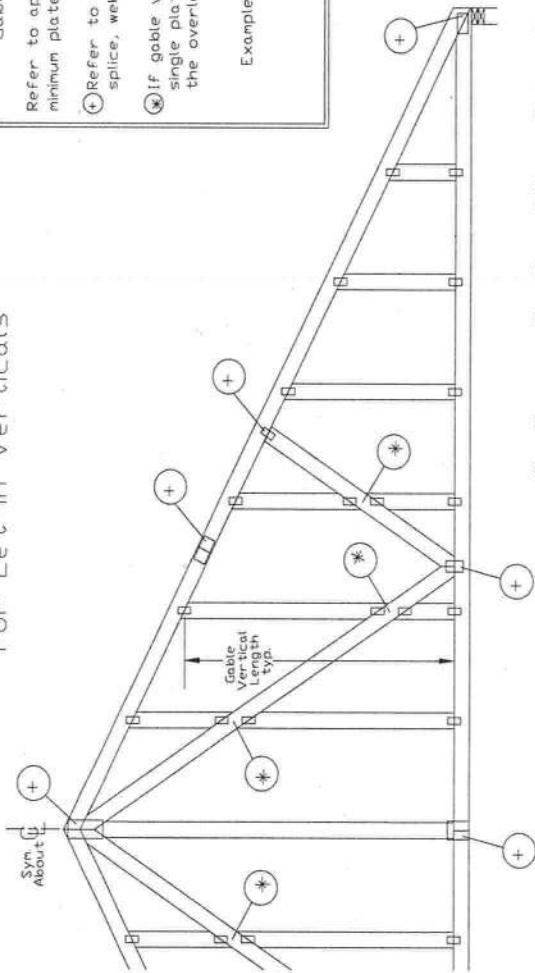
Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).
 Gable end supports load from 4' 0" outleakers with 2' 0" overhang, or 12' plywood overhang.
 So. Pine lumber design values based on the ALSC January, 2012 rule.
 Attach 'L' braces with 10d (0.128"x3.0" min) nails.
 * For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.
 * For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.
 'L' bracing must be a minimum of 80% of web member length.

Vertical Length	No Splice
Less than 4' 0"	1x4 or 2x3
Greater than 4' 0", but less than 11' 6"	2x4
Greater than 11' 6"	2x4x4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Gable Detail For Let-in Verticals



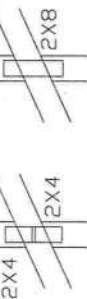
Gable Truss Plate Sizes

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



T Reinforcement Attachment Detail

T Reinforcing Member

T Reinforcing Member



To convert from 'L' to 'T' reinforcing members, multiply 'L' increase by length (based on appropriate ITW gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

T Reinf.	Mr. Size	Increase
2x4		30 %
2x6		20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length

1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.1min) Nails at 4' o.c. plus

(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x 3.1min) Toenails at 4' o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings

A13015980109, A12015980109, A11015980109, A10015980109,

A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings

A13015020109, A12015020109, A11015020109, A10015020109,

A13030020109, A12030020109, A11030020109, A10030020109

ASCE 7-05 Gable Detail Drawings

A13015050109, A12015050109, A11015050109, A10015050109,

A13030050109, A12030050109, A11030050109, A10030050109

ASCE 7-10 Gable Detail Drawings

A11515ENC100212, A12015ENC100212, A14015ENC100212, A16015ENC100212,

A18015ENC100212, A20015ENC100212, A22015ENC100212, A24015ENC100212,

A11530ENC100212, A12030ENC100212, A14030ENC100212, A16030ENC100212,

A18030ENC100212, A20030ENC100212, A22030ENC100212, A24030ENC100212

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow all instructions and warnings on the ITW Building Components Group Inc. product literature. Practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B2, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover, page listing this drawing, indicates acceptance of the design and construction of the truss by ITW Building Components Group Inc. and use of this drawing for any structural application is the responsibility of the Building Designer. ANSI/TPI 1 Seal. For more information see this job's general notes page and these web sites: www.itwbcg.com; www.tpinet.org; www.stcindustry.org; ICC: www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

REF LET-IN VERT

DATE 2/16/12

DRWG GBLLETIN0212

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"



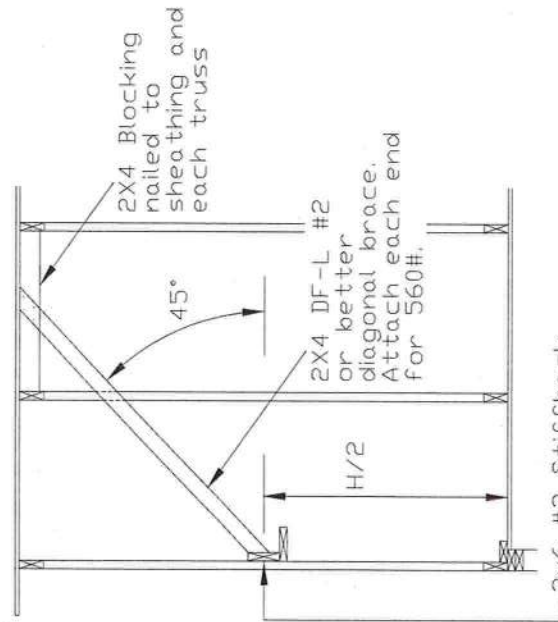
ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

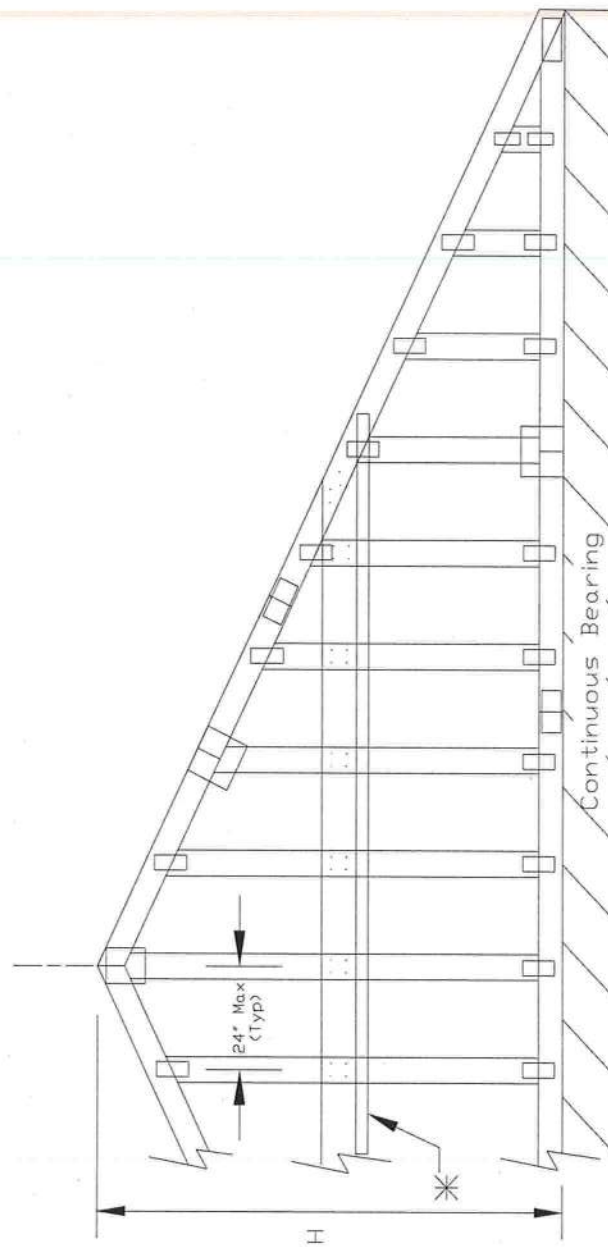
Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

Nails: 10d box or gun (0.128"x3",min) nails.



2x6 #2 Stiffback
attached to each
Stud w/ (4) 10d bolts



*Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCIS (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCIS. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached nailing. Locations shown for permanent lateral restraint (crikwall) shall be nailing (see Figure 3587-2) as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless otherwise specified. Refer to drawings 160A-2 for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites:

Earth City, MO 63045

REF GE WHALER

DATE 2/14/12

DRWG GABRST100212

MAX. TPT. 1 D. 60 PSE

MAX. SPACING



