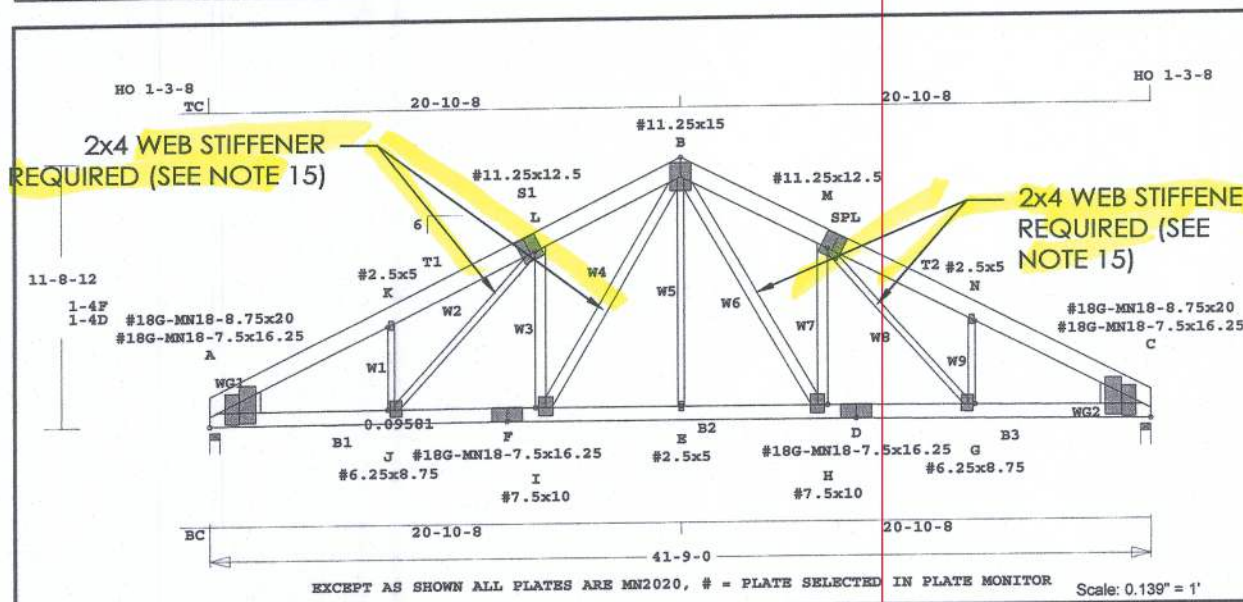


Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
4075-42' SC-4116	4408350004109(10)	1	ZRC44	410900	6	0	0	



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RUN DATE: 23-DEC-16

Southern Pine lumber design
values are those effective
06-01-13 by SPIB//ALSC UON

CSI -Size- Lumber-
TC 0.95 2x10 SP-#1
BC 0.58 2x 8 SP-2400f-2.0E
WB 0.84 2x 6 SP-2400f-2.0E
-- 0.38 2x 4 SP-#1
J -K J -S1 E -B M -G
G -N
-- 0.42 2x 6 SP-#1
I -B B -H
WG --- 2x12 SP-#1

Brace truss as follows:
O.C. From To
TC 24.0" 0- 0- 0 41- 9- 0
BC 90.0" 0- 0- 0 41- 9- 0

psf-Ld Dead Live
TC 6.0 31.0
BC 4.0 0.0
TC+BC 10.0 31.0
Total 41.0 Spacing 96.0"
Lumber Duration Factor 1.15
Plate Duration Factor 1.15
Fb Fc Ft Emin
TC 1.00 1.00 1.00 1.00
BC 1.00 1.00 1.00 1.00

Total Load Reactions (Lbs)
Jt Down Uplift Horiz-
A 6847
C 6847

Jt Brg Size
A 5.5"
C 5.5"

LC# 2 Dead Load Check
Dur Fctrs - Lbr 0.90 Plt 0.90
plf - Dead Live* From To
TC V 48 0 0.0' 41.8'
BC V 32 0 0.0' 41.8'

Membr CSI P Lbs Ax1-CSI-Bnd

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MITek® Online Plus™ APPROX. TRUSS WEIGHT: 579.0 LBS

Top Chords-
A -K 0.74 10697 C 0.22 0.52
K -S1 0.95 10512 C 0.40 0.55
S1-B 0.63 9340 C 0.14 0.49
B -M 0.63 9340 C 0.14 0.49
M -N 0.95 10512 C 0.40 0.55
N -C 0.74 10697 C 0.22 0.52

Bottom Chords-
A -J 0.58 9459 T 0.39 0.19
J -F 0.53 8387 T 0.34 0.19
F -I 0.53 8386 T 0.34 0.19
I -E 0.32 6616 T 0.27 0.05
E -H 0.32 6616 T 0.27 0.05
H -D 0.53 8386 T 0.34 0.19
D -G 0.53 8387 T 0.34 0.19
G -C 0.58 9459 T 0.39 0.19

Webs-
J -K 0.38 1423 C
J -S1 0.27 1665 T
I -S1 0.84 2704 C
I -B 0.42 3547 T
E -B 0.04 222 T
B -H 0.42 3547 T
H -M 0.84 2704 C
M -G 0.27 1665 T
G -N 0.38 1423 C

TL Defl -0.54" in J -I L/899
LL Defl -0.36" in J -I L/999
Hz Disp LL DL TL
Jt C 0.14" 0.07" 0.21"
Shear // Grain in A -K 0.54

Plates for each ply each face.
PLATING CONFORMS TO TPI.
REPORTS: ICC-ES ESR-3080
Plate - MN20 Ga, Gross Area
Plate - MN18 Ga, Gross Area
Plate - MS18 Ga, Gross Area
Plate - MT16 Ga, Gross Area
Jt Type Plt Size X Y JSI
A# MN18 7.5x16.2-7.7-3.7 0.77
A# MN18 8.8x20.0 Ctr Ctr 0.77
K# MN20 2.5x 5.0 Ctr Ctr 0.63
S1#MN20 11.2x12.5-0.6 2.5 0.67
B# MN20 11.2x15.0 Ctr Ctr 0.88
M# MN20 11.2x12.5 0.6 2.5 0.67
N# MN20 2.5x 5.0 Ctr Ctr 0.63
C# MN18 7.5x16.218.0-3.7 0.78

ALLIED DESIGN ARCHITECTURAL AND ENGINEERING GROUP, P.C. (ALLIED) CERTIFIES TRUSS DESIGN, ONLY, AND DOES NOT CERTIFY DESIGN OR OTHER ENGINEERING OF THE BUILDING IN WHICH THE TRUSS WILL BE INCORPORATED. ALLIED EXPRESSLY RELIES THAT THE BUILDING DESIGN AND/OR OTHER ENGINEERING WILL BE PROVIDED BY OTHERS AND ALLIED ASSUMES NO LIABILITY FOR PLANS OR DESIGNS OF OTHERS WHICH INCORPORATE THIS TRUSS DESIGN. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THIS TRUSS DESIGN AND VERIFY THAT DIMENSIONS AND LOADS CONFORM TO APPLICABLE STANDARDS FOR DESIGN AND ENGINEERING FOR THE USE OF A BUILDING.

ROUND SNOW LOAD: 0 PSF
IND SPEED (VULT) (EXP. B): 130 MPH

GENERAL SAFETY, DESIGN, AND FABRICATION NOTES:

1. PROVIDE COPIES OF THIS TRUSS DESIGN TO THE BUILDING DESIGNER, ERECTION SUPERVISOR, PROPERTY OWNER AND ALL OTHER INTERESTED PARTIES.
2. CUT MEMBERS TO BEAR TIGHTLY AGAINST EACH OTHER.
3. PLACE PLATES ON EACH FACE OF TRUSS AT EACH JOINT AND EMBED FULLY. AVOID KNOTS AND WANE AT JOINT LOCATIONS.
4. MOISTURE CONTENT OF LUMBER SHALL NOT EXCEED 19% AT THE TIME OF FABRICATION.
5. THIS DESIGN IS NOT APPLICABLE FOR USE WITH FIRE RETARDING OR PRESERVATIVE TREATED LUMBER.
6. CAMBER IS A NON-STRUCTURAL CONSIDERATION AND IS THE RESPONSIBILITY OF THE TRUSS FABRICATOR. GENERAL PRACTICE IS TO CAMBER FOR DEAD LOAD DEFLECTION.
7. PLATE TYPE AND SIZE SHOWN ARE MINIMUM REQUIREMENTS.
8. LUMBER SHALL BE THE SIZE AND SPECIES SHOWN. THE GRADES SHOWN ARE MINIMUM REQUIREMENTS.
9. TOP CHORDS MUST BE LATERALLY BRACED NOT TO EXCEED 24" O.C.
10. BOTTOM CHORDS MUST BE LATERALLY BRACED NOT TO EXCEED 6'-0" O.C.
11. ANCHORAGE AND/OR LOAD TRANSFERRING CONNECTIONS TO TRUSSES ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.
12. DO NOT OVERLOAD TRUSSES WITH STACKS OF CONSTRUCTION MATERIAL.
13. DO NOT CUT OR ALTER TRUSS MEMBERS OR PLATES WITHOUT WRITTEN APPROVAL OF THE TRUSS DESIGNER.
14. CARE SHOULD BE EXERCISED WHEN HANDLING, ERECTING, AND INSTALLING THESE TRUSSES TO AVOID DAMAGE.
15. WEBS INDICATED REQUIRE 2x4 (NO. 1 SYP) WEB STIFFENERS CENTERED ON TRUSS WEB NAILED W/ 3-12" HDG R.S. GUN NAILS OR 16d R.S. NAILS WITHIN 4" OF ENDS & 6" O.C.

LIVE LOAD 16 PSF
DEAD LOAD 6 PSF
CEILING LOAD 4 PSF
TOTAL LOAD 26 PSF

REQUIRED TRUSS LOAD = 26 PSF AT 8'-0" O.C. = 208 PLF
TRUSS DESIGN LOAD (SEE DESIGN OUTPUT) = 328 PLF

I HEREBY CERTIFY THAT THE STRUCTURAL DESIGN FOR THIS TRUSS WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED/REGISTERED PROFESSIONAL ENGINEER.

Benjamin J Zobrist
BENJAMIN J ZOBRIST
ben.zobrist@allieddesignaes.com
DATE: 03/26/2021 REG.# 89211



OFFICE:
GAINESVILLE, FL

JOB NO.
131-105306

DONNALEE OR MICHAEL RELYEA

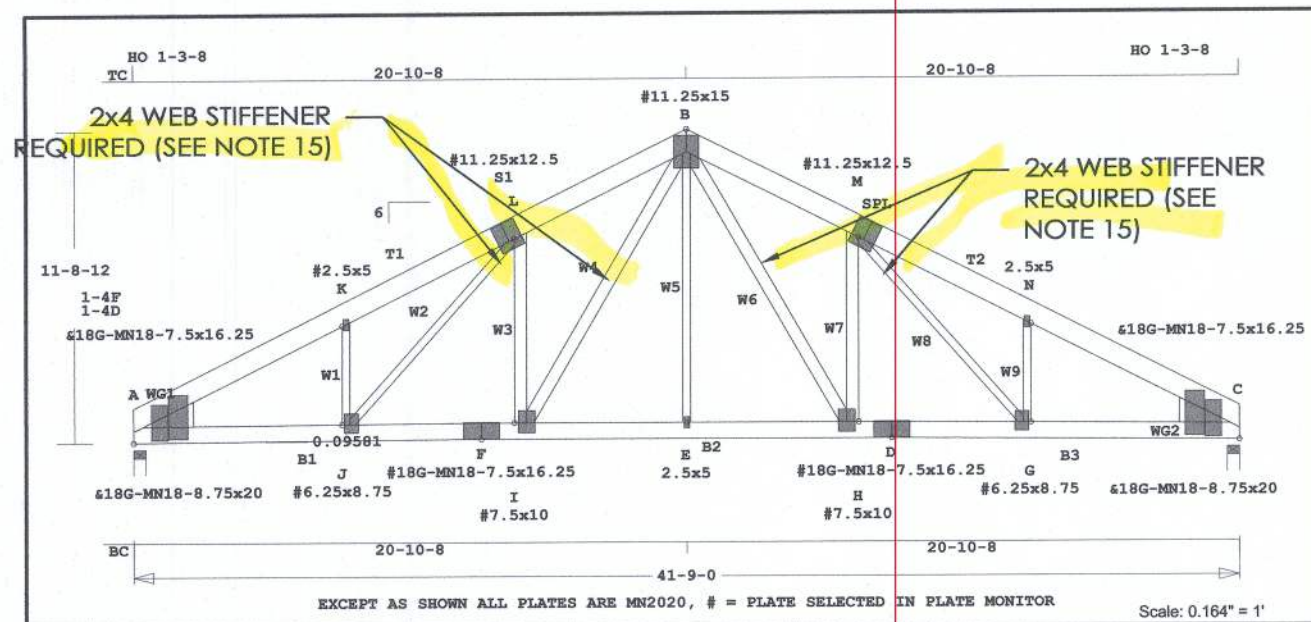
HIGH SPRINGS, FL

ALLIED DESIGN ARCHITECTURAL & ENGINEERING GROUP, P.C.
100 S. PERSHING P.O. BOX 110 MORTON, IL 61550
COA # 6400 (ENG) COA # AA003469 (AR)
BENJAMIN J. ZOBRIST, P.E.
REG. # 89211

DRAWN BY: V. DE VERA
DATE: 3/23/2021
CHECKED BY:
DATE:
REVISED DATE: ---
REVISED DATE: ---

SCALE: AS NOTED
SHEET NO.
1 OF 2

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
4075-42' SC-4116	4408350004109(2)	1	ZRC44	410900	6	0	0	



Online Plus -- Version 30.0.069
 RUN DATE: 23-MAR-21

Southern Pine lumber design values are those effective 06-01-13 by SPIB//ALSC UON

TC	BC	WB	---	TC	BC	WB	---
0.87	0.56	0.77	0.33	0.87	0.56	0.77	0.33
2x10	2x8	2x6	2x4	2x10	2x8	2x6	2x4
SP-#1	SP-2400f-2.0E	SP-2400f-2.0E	SP-#1	SP-#1	SP-2400f-2.0E	SP-2400f-2.0E	SP-#1

Brace truss as follows:

O.C.	From	To
24.0"	0-0-0	41-9-0
90.0"	0-0-0	41-9-0

psf-Ld	Dead	Live
TC	6.0	16.0
BC	4.0	0.0
TC+BC	10.0	16.0
Total	26.0	Spacing 144.0"

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	6513		
C	6513		

Jt	Brg Size	Required
A	5.5"	5.4"
C	5.5"	5.4"

LC# 2 Dead Load Check
 Dur Fctrs - Lbr 0.90 Plt 0.90
 plf - Dead Live* From To

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MITek® Online Plus™ APPROX. TRUSS WEIGHT: 579.0 LBS

TC V	BC V	72	48	0	0.0'	41.8'
TC V	BC V	72	48	0	0.0'	41.8'

Membr CSI P Lbs Ax1-CST-Bnd

Top Chords	Bottom Chords
A-K 0.69 10178 C 0.20 0.49	A-J 0.56 8998 T 0.37 0.19
K-S1 0.87 10000 C 0.37 0.50	J-F 0.50 7978 T 0.33 0.17
S1-B 0.58 8885 C 0.13 0.45	F-I 0.50 7977 T 0.33 0.17
B-M 0.58 8885 C 0.13 0.45	I-E 0.32 6292 T 0.26 0.06
M-N 0.87 10000 C 0.37 0.50	E-H 0.32 6292 T 0.26 0.06
N-C 0.69 10178 C 0.20 0.49	H-D 0.50 7977 T 0.33 0.17
	D-G 0.50 7978 T 0.33 0.17
	G-C 0.56 8998 T 0.37 0.19

Webbs

J-K	J-S1	I-S1	I-B	E-B	B-H	H-M	M-G	G-N
0.33	0.26	0.77	0.40	0.06	0.40	0.77	0.26	0.33
1240 C	1584 T	2458 C	3377 T	316 T	3377 T	2458 C	1584 T	1240 C

TL Defl	LL Defl	Hz Disp	LL	DL	TL
-0.55"	-0.28"	0.11"	0.10"	0.21"	
		Shear // Grain			0.49

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORTS: ICC-ES ESR-3080
 Plate - MN20 20 Ga, Gross Area
 Plate - MN18 18 Ga, Gross Area
 Plate - MS18 18 Ga, Gross Area
 Plate - MT16 16 Ga, Gross Area
 Jt Type Plt Size X Y JSI

Plates for each ply each face.
 PLATING CONFORMS TO TPI.
 REPORTS: ICC-ES ESR-3080
 Plate - MN20 20 Ga, Gross Area
 Plate - MN18 18 Ga, Gross Area
 Plate - MS18 18 Ga, Gross Area
 Plate - MT16 16 Ga, Gross Area
 Jt Type Plt Size X Y JSI

ALLIED DESIGN ARCHITECTURAL AND ENGINEERING GROUP, P.C. (ALLIED) CERTIFIES TRUSS DESIGN, ONLY, AND DOES NOT CERTIFY DESIGN OR OTHER ENGINEERING OF THE BUILDING IN WHICH THE TRUSS WILL BE INCORPORATED. ALLIED EXPRESSLY RELIES THAT THE BUILDING DESIGN AND/OR OTHER ENGINEERING WILL BE PROVIDED BY OTHERS AND ALLIED ASSUMES NO LIABILITY FOR PLANS OR DESIGNS OF OTHERS WHICH INCORPORATE THIS TRUSS DESIGN. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THIS TRUSS DESIGN AND VERIFY THAT DIMENSIONS AND LOADS CONFORM TO APPLICABLE STANDARDS FOR DESIGN AND ENGINEERING FOR THE USE IN A BUILDING.

GROUND SNOW LOAD: 0 PSF
 WIND SPEED (VULT) (EXP. B): 130 MPH

REQUIRED TRUSS LOAD = 26 PSF AT 12'-0" O.C. = 312 PLF
 TRUSS DESIGN LOAD (SEE DESIGN OUTPUT) = 312 PLF

GENERAL SAFETY, DESIGN, AND FABRICATION NOTES:

1. PROVIDE COPIES OF THIS TRUSS DESIGN TO THE BUILDING DESIGNER, ERECTION SUPERVISOR, PROPERTY OWNER AND ALL OTHER INTERESTED PARTIES.
2. CUT MEMBERS TO BEAR TIGHTLY AGAINST EACH OTHER.
3. PLACE PLATES ON EACH FACE OF TRUSS AT EACH JOINT AND EMBED FULLY. AVOID KNOTS AND WANE AT JOINT LOCATIONS.
4. MOISTURE CONTENT OF LUMBER SHALL NOT EXCEED 19% AT THE TIME OF FABRICATION.
5. THIS DESIGN IS NOT APPLICABLE FOR USE WITH FIRE RETARDING OR PRESERVATIVE TREATED LUMBER.
6. CAMBER IS A NON-STRUCTURAL CONSIDERATION AND IS THE RESPONSIBILITY OF THE TRUSS FABRICATOR. GENERAL PRACTICE IS TO CAMBER FOR DEAD LOAD DEFLECTION.
7. PLATE TYPE AND SIZE SHOWN ARE MINIMUM REQUIREMENTS.
8. LUMBER SHALL BE THE SIZE AND SPECIES SHOWN. THE GRADES SHOWN ARE MINIMUM REQUIREMENTS.
9. TOP CHORDS MUST BE LATERALLY BRACED NOT TO EXCEED 24" O.C.
10. BOTTOM CHORDS MUST BE LATERALLY BRACED NOT TO EXCEED 6'-0" O.C.
11. ANCHORAGE AND/OR LOAD TRANSFERRING CONNECTIONS TO TRUSSES ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.
12. DO NOT OVERLOAD TRUSSES WITH STACKS OF CONSTRUCTION MATERIAL.
13. DO NOT CUT OR ALTER TRUSS MEMBERS OR PLATES WITHOUT WRITTEN APPROVAL OF THE TRUSS DESIGNER.
14. CARE SHOULD BE EXERCISED WHEN HANDLING, ERECTING, AND INSTALLING THESE TRUSSES TO AVOID DAMAGE.
15. WEBS INDICATED REQUIRE 2X4 (NO. 1 SYP) WEB STIFFENERS CENTERED ON TRUSS WEB NAILED W/ 3-12" HDG R.S. GUN NAILS OR 16D R.S. NAILS WITHIN 4" OF ENDS & 6" O.C.

LIVE LOAD	16	PSF
DEAD LOAD	6	PSF
CEILING LOAD	4	PSF
TOTAL LOAD	26	PSF

I HEREBY CERTIFY THAT THE STRUCTURAL DESIGN FOR THIS TRUSS WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED/REGISTERED PROFESSIONAL ENGINEER.

Benjamin J Zobrist
 BENJAMIN J ZOBRIST
 ben.zobrist@allieddesignaes.com
 DATE: 03/26/2021 REG.# 89211



OFFICE:
 GAINESVILLE, FL

JOB NO.
 131-105306

DONNALEE OR MICHAEL RELYEA

HIGH SPRINGS, FL

BENJAMIN J. ZOBRIST, P.E.
 REG. # 89211
 ALLIED DESIGN ARCHITECTURAL & ENGINEERING GROUP, P.C.
 PHONE NUMBER: 309-263-4105
 COA # A-003469 (AR)

DRAWN BY:	V. DE VERA
DATE:	3/23/2021
CHECKED BY:	
DATE:	
REVISED DATE:	---
REVISED DATE:	---

SCALE: AS NOTED
 SHEET NO.
 2 OF 2

MORTON BUILDINGS GENERAL SPECIFICATIONS

LAMINATED COLUMNS - NO. 1 OR BETTER SOUTHERN YELLOW PINE NAIL LAMINATED 3 MEMBER S COLUMNS NAILED 8" O.C. STAGGERED ON EACH SIDE WITH 4" NAILS.

MFS PRE-CAST CONCRETE COLUMN - MORTON BUILDINGS FOUNDATION SYSTEM IS A PRE-ENGINEERED, 10,000 PSI, STEEL REINFORCED COLUMN FOR BELOW GROUND INSTALLATION. DESIGNED TO BE MECHANICALLY FASTENED TO ABOVE GROUND NAIL LAMINATED COLUMNS. THE SYSTEM IS DESIGNED TO RESIST BOTH AXIAL AND BENDING FORCES.

FOOTINGS AND ANCHORAGE - COLUMN HOLES ARE DUG A MINIMUM DEPTH OF 4'-0" BELOW G.D.E (SEE PLANS FOR DIAMETER AND DEPTH). MFS PRE-CAST CONCRETE COLUMNS ARE PLACED IN T.HOLE. CONCRETE (MINIMUM COMPRESSIVE STRENGTH 2500 PSI) IS POURED IN PLACE TO THE SPECIFIED THICKNESS (SEE PLANS FOR REQUIRED THICKNESS ABOVE AND BELOW THE COLUMN). THE COLUMN IS THEN BACKFILLED WITH SOIL AND COMPACTED AT 8" INTERVALS OR BACKFILLED WITH CONCRETE (SEE PLANS).

TREATED LUMBER -- PRESSURE PRESERVATIVE TREATED LUMBER OTHER THAN LAMINATED COLUMN ARE NO. 1 OR BETTER SOUTHERN YELLOW PINE AND CENTER MATCHED OR NOTCHED AND GROOVED OR S. PRESSURE TREATMENT TO GROUND CONTACT RETENTION WITH PRESERVATIVE TREATMENT COMPING WITH USE CATEGORY UC4B (AWPA OR ICC-ES) AND IN COMPLIANCE WITH USEPA GUIDELINES AND STANDARDS.

FRAMING LUMBER - SIDING NAILERS ARE 2x4 S4S OR 2x6 SPF NO. 2 OR BETTER SPACED APPROXIMATELY 36" O.C. WITH ALL JOINTS STAGGERED AT ATTACHMENT TO COLUMNS. ROOF PURLINS ARE 2x4 S4S N. 2 OR BETTER ON EDGE SPACED APPROXIMATELY 24" O.C. ALL OTHER FRAMING LUMBER IS NO. 2 OR BETTER.

ROOF TRUSSES - FACTORY ASSEMBLED WITH 18 OR 20 GAUGE GALVANIZED STEEL TRUSS PLATES ARE REQUIRED AND KILN DRIED LUMBER AS SPECIFIED, IN-PLANT QUALITY CONTROL INSPECTION IS CONDUCTED UNDER THE AUSPICES OF THE TPI INSPECTION BUREAU. TRUSSES ARE DESIGNED IN ACCORDANCE WITH CURRENT STANDARDS AND SPECIFICATIONS FOR THE STATED LOADING.

SIDING & ROOFING PANELS (FLUOROFLEX 1000 TM) - 0.019" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL WITH AN ADDITIONAL BAKED-ON 70% PVDF FINISH WITH A NOMINAL 1 MIL. PAINT THICKNESS ON EXTERIOR.

TRIM - DIE-FORMED TRIM OF 0.017" MIN., G90 GALVANIZED OR AZ55 GALVALUME STEEL ON GABLE RIDGES, CORNERS, BASE WINDOWS, AND DOORS WITH SAME FINISH AS ROOFING OR SIDING PANELS.

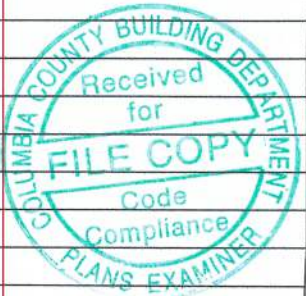
GUTTERS - 5" OR 6" K-STYLE, .030 HIGH TENSILE ALUMINUM GUTTER, 70% PVDF FINISH TO MATCH TRIM, ON BOTH SIDES OF THE BUILDING. 2x4 F1 F1 MFS 09/20

FLORIDA PRODUCT APPROVAL NUMBERS (FBC 2017)

PRODUCT	FL#
MBI HI-RIB WALL PANEL	25829.2
MBI HI-RIB ROOF PANEL	25577.2
THERMA-TRU ENTRY DOOR	TBD
PGT WINDOW	TBD
PLY GEM MASTIC SOFFITS	23352.1

SHEET INDEX

SHEET#	DESCRIPTION
G1 OF G1	SPECIFICATIONS & SHEET INDEX
S1 OF S12	COLUMN PLAN
S2 OF S12	TRUSS/BRACING PLAN
S3 OF S12	TRUSS DRAWINGS & DETAILS
S4 OF S12	ELEVATIONS
S5 OF S12	ELEVATIONS
S6 OF S12	SECTIONS A,B,C & DETAILS
S7 OF S12	SECTIONS D,E
S8 OF S12	SECTION E1,F
S9 OF S12	SECTION G
S10 OF S12	DETAILS
S11 OF S12	SECTIONS H,I & DETAILS
S12 OF S12	SHEARWALL DETAILS & FASTENING SCHEDULE



CURRENT LUMBER SPECIFICATIONS (06-01-2013)

SIZE	DESCRIPTION	BENDING VALUE Fb
2x	NO. 2 SPF	1313 PSI
2x	NO. 1 SYP	1500 PSI
2x	2100f MSR SPF	2100 PSI
2x	NO. 2 SPF	1138 PSI
2x	NO. 1 SYP	1350 PSI
2x	2100f MSR SPF	2100 PSI
2x	2400 MSR SYP	2400 PSI
2x	NO. 1 SYP	1250 PSI
2x	2400 MSR SYP	2400 PSI
2x	NO. 1 SYP	1050 PSI
2x	2400 MSR SYP	2400 PSI
2x	NO. 1 SYP	1000 PSI
2x	2250f MSR SYP	2250 PSI
1 1/2x6"	LAMINATED VENEER LUMBER	2800 PSI
3 1/2x5"	GLU-LAM	1650 PSI
5 1/4x11/2"	GLU-LAM	2400 PSI
5 1/4x11/2"	GLU-LAM	2400 PSI

DESIGN AND EXPLANATORY NOTES

- 1.) ALL PLOT PLANS AND RELATED DETAILS SHALL BE PROVIDED BY OWNER UNLESS INCORPORATED AS PART OF THESE DRAWINGS.
- 2.) MORTON BUILDINGS GENERAL SPECIFICATIONS APPLY UNLESS INDICATED DIFFERENTLY ON SPECIFIC JOB DRAWINGS OR SUPPLEMENTAL INFORMATION.
- 3.) MINIMUM LIVE ROOF LOAD b) DESIGNS FOR CONSTRUCTION, MAINTENANCE, REPAIR, AND OTHER TEMPORARY LOADS PER SECTION 1607.12.2
a.) ROOF PURLINS AND OTHER SECONDARY STRUCTURAL MEMBERS = 20 PSF
b.) ROOF TRUSSES, HEADERS, COLUMNS AND OTHER PRIMARY STRUCTURAL MEMBER = 16 PSF
c.) FOOTINGS = 12 PSF (DESIGNED FOR ROOF SNOW LOAD AND OTHER NON-TEMPORARY LOADS W/ APPROVAL FROM BUILDING OFFICIAL).
- 4.) NO ONE MAY ALTER ANY ENGINEERING ITEM UNLESS ACTING UNDER THE DIRECTION OF THE LICENSED / REGISTERED ENGINEER .
- 5.) ♦ THE PRECEDING SYMBOL IDENTIFIES ITEMS THROUGHOUT THE PLANS THAT ARE NOT PROVIDED BY MORTON BUILDINGS, INC. OR MORTON BUILDINGS' SUBCONTRACTORS AND ARE THE OWNER'S RESPONSIBILITY.

BUILDING DESIGN CRITERIA

BUILDING CODE	2020 FLORIDA BUILDING CODE	GC CODE		
CONSTRUCTION TYPE	VB			
RISK CATEGORY	II			
EAVE HEIGHT	13.89 FT			
PEAK HEIGHT	24.4 FT			
MEAN ROOF HEIGHT	19.14 FT			
MINIMUM LIVE ROOF LOAD DESIGN	SEE NOTE #3			
WIND SPEED (Vult)	130 MPH			
WIND SPEED (VAsd)	101 MPH			
EXPOSURE CATEGORY	B			
INTERNAL PRESSURE COEFFICIENT	±0.18			
BUILDING DESIGN CONDITION	ENCLOSED			
WIND LOAD DESIGN	ASCE 7 (ENVELOPE METHOD)	METHOD		
MAIN WINDFORCE RESISTING SYSTEM (ALL FORCES ACT NORMAL TO THE SURFACE) (FOR ZONES SEE MWFRS ON ELEVATIONS PAGE) (VALUES SHOWN = 0.6 * W) CASE 1 = (-) INTERNAL PRESSURE COEFFICIENT CASE 2 = (+) INTERNAL PRESSURE COEFFICIENT		CASE 1	CASE 2	
	ZONE 1E	23.31 PSF		14.07 PSF
	ZONE 2E	-0.26 PSF		-9.51 PSF
	ZONE 3E	-10.40 PSF		-19.65 PSF
	ZONE 4E	-9.12 PSF		-18.36 PSF
	ZONE 1	18.74 PSF		9.49 PSF
	ZONE 2	2.08 PSF		-7.17 PSF
	ZONE 3	-6.86 PSF		-16.11 PSF
	ZONE 4	-5.41 PSF		-14.65 PSF
	ZONE 1	16.50 PSF	-3.37	37.98 PSF
COMPONENT & CLADDING WIND LOADS (ALL FORCES ACT NORMAL TO THE SURFACE) (FOR ZONES SEE ELEVATIONS) (VALUES SHOWN = 0.6 * W) (ASSUMED EFFECTIVE WIND AREA = 10 SQ. FT.)	ZONE 2e	16.50 PSF	-3.37	37.98 PSF
	ZONE 2n	16.50 PSF	-6.60	58.58 PSF
	ZONE 2r	16.50 PSF	-6.60	58.58 PSF
	ZONE 3e	16.50 PSF	-6.60	58.58 PSF
	ZONE 3r	16.5 PSF	-6.55	10 PSF
	ZONE 4	26.67 PSF	-2.28	93.93 PSF
	ZONE 5	26.67 PSF	-3.35	72 PSF
	ZONE 1	16.50 PSF		
	ZONE 2e	16.50 PSF		
	ZONE 2n	16.50 PSF		

OFFICE: GAINESVILLE, FL

JOB NO. 131-105306

DONNALEE OR MICHAEL RELYEA

HIGH SPRINGS, FL

DRAWN BY:	AJN
DATE:	2/9/2021
CHECKED BY:	V. DE VERA
DATE:	3/18/2021
REVISED DATE:	---
REVISED DATE:	---
REVISED DATE:	---



MAR 26 2021

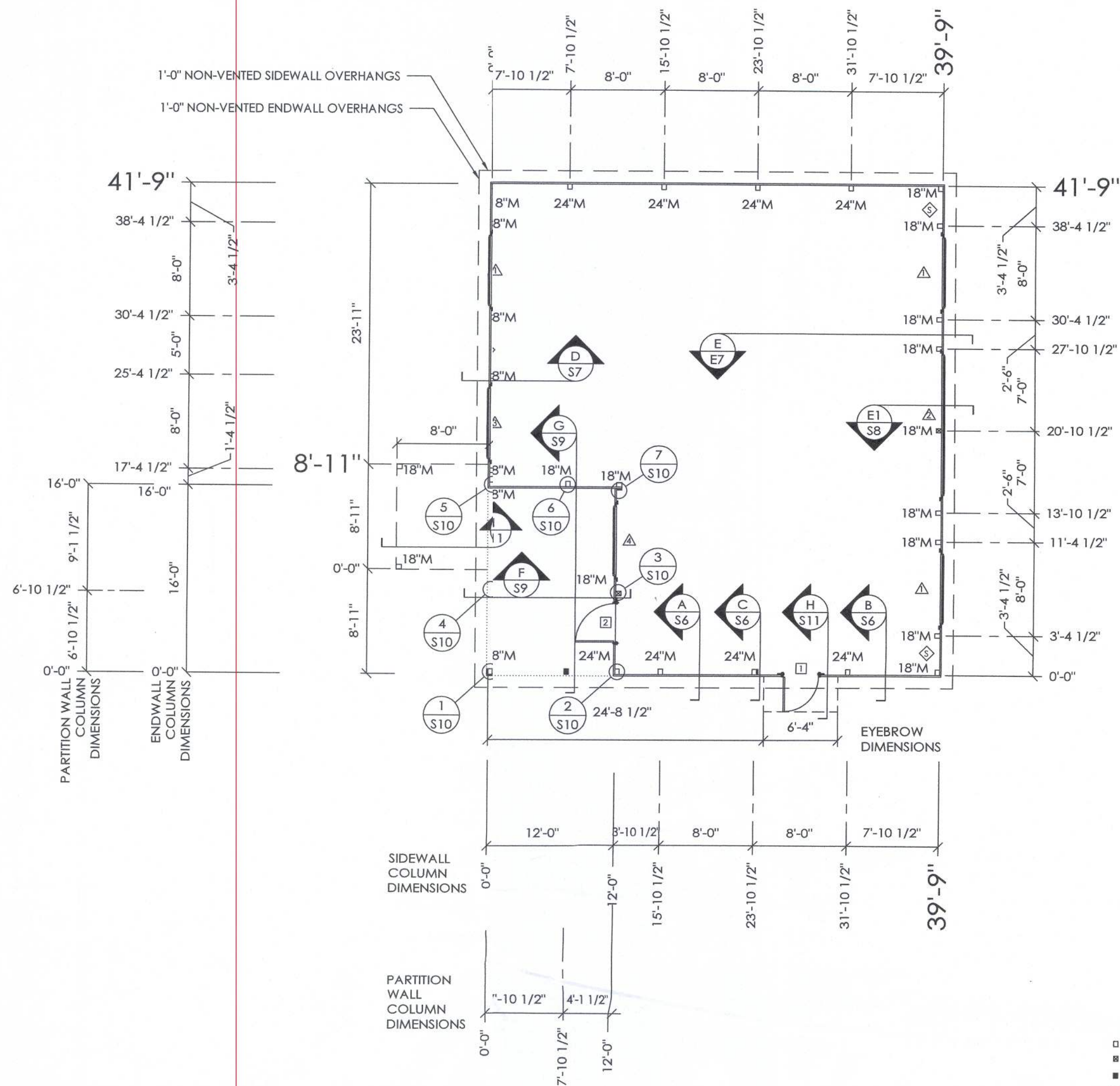
SCALE: AS NOTED
SHEET NO. G1 OF G1

DONNALEE OR MICHAEL RELYEA
HIGH SPRINGS, FL

DRAWN BY:	AJN
DATE:	2/9/2021
CHECKED BY:	V. DE VERA
DATE:	3/18/2021
REVISED DATE:	---
REVISED DATE:	---
REVISED DATE:	---
REVISED DATE:	---



MAR 26 2021
SCALE: AS NOTED
SHEET NO.
S1 OF S12

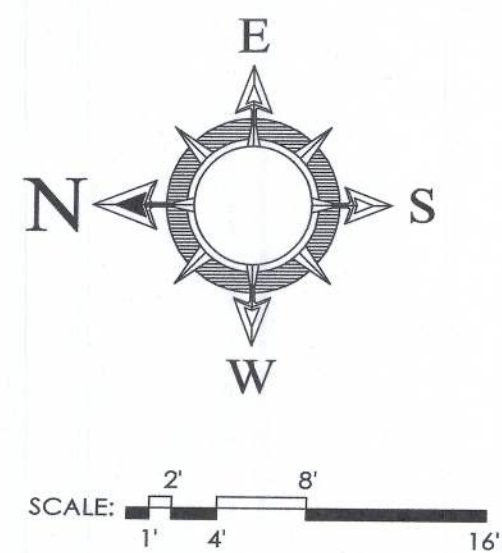


COLUMN PLAN

ROUGH OPENING SCHEDULE		
UNIT SYMBOL FROM LEGEND	WIDTH	HEIGHT
1	38"	82 1/4"
2	34"	82 1/4"
△	VERIFY	VERIFY
△	VERIFY	VERIFY

COLUMN PLAN LEGEND

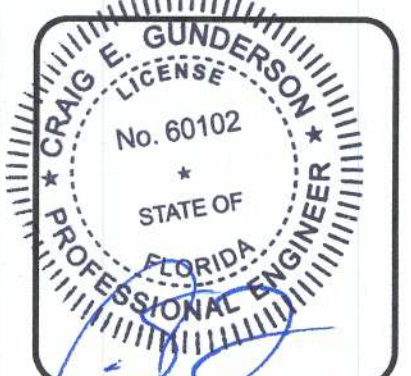
- - 3-2x6 LAMINATED COLUMN LOCATION
- - 3-2x6 STUD COLUMN LOCATION
- - HEADERED TRUSS LOCATION
- 1 - 37.625"X82" S262 LHIS THERMA-TRU WALK DOOR
- 2 - 33.625"X82" S262 LHIS THERMA-TRU WALK DOOR
- △ - (3) 2 EACH 35.5" X 59.5" SH B BASE WINDOWS W/SCREENS AND 2 EACH 35.5" X 17.375" PW TRANSOMS, ALL W/GRIDS AND MULLED TO A SINGLE UNIT
- △ - (1) 4 EACH 35.5" X 59.5" SH B BASE WINDOWS W/SCREENS AND 4 EACH 35.5" X 17.375" PW TRANSOMS, ALL W/GRIDS AND MULLED TO A SINGLE UNIT
- △ - (1) 71.5" X 17.375" PW WINDOW WITH GRIDS
- △ - (1) 71.5" X 47.5" HORIZONTAL ROLLING WINDOW W/GRIDS AND SCREEN
- ALL STEEL FASTENED WITH STAINLESS STEEL SCREWS
- 7/16" OSB AND ROOFING FELT UNDER HI-RIB STEEL
- ◇ - 7/16" OSB SHEARWALL (SEE E DETAILS ON S12)
- 18"M - 18" DIAMETER FOOTING WITH 4" TO BOTTOM OF 21" THICK CONCRETE PAD (2500 PSI MINIMUM). 20" BELOW BOTTOM OF PRECAST CONCRETE COLUMN AROUND EXPOSED REBAR CAGE AND 3/4"x14" THREADED ROD WITH AN ADDITIONAL MINIMUM 1" ABOVE BOTTOM OF PRECAST CONCRETE COLUMN. PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION.
- 24"M - 24" DIAMETER FOOTING WITH 4" TO BOTTOM OF 21" THICK CONCRETE PAD (2500 PSI MINIMUM). 20" BELOW BOTTOM OF PRECAST CONCRETE COLUMN AROUND EXPOSED REBAR CAGE AND 3/4"x14" THREADED ROD WITH AN ADDITIONAL MINIMUM 1" ABOVE BOTTOM OF PRECAST CONCRETE COLUMN. PLACE CONCRETE BELOW AND ABOVE BOTTOM OF LOWER COLUMN IN ONE OPERATION.



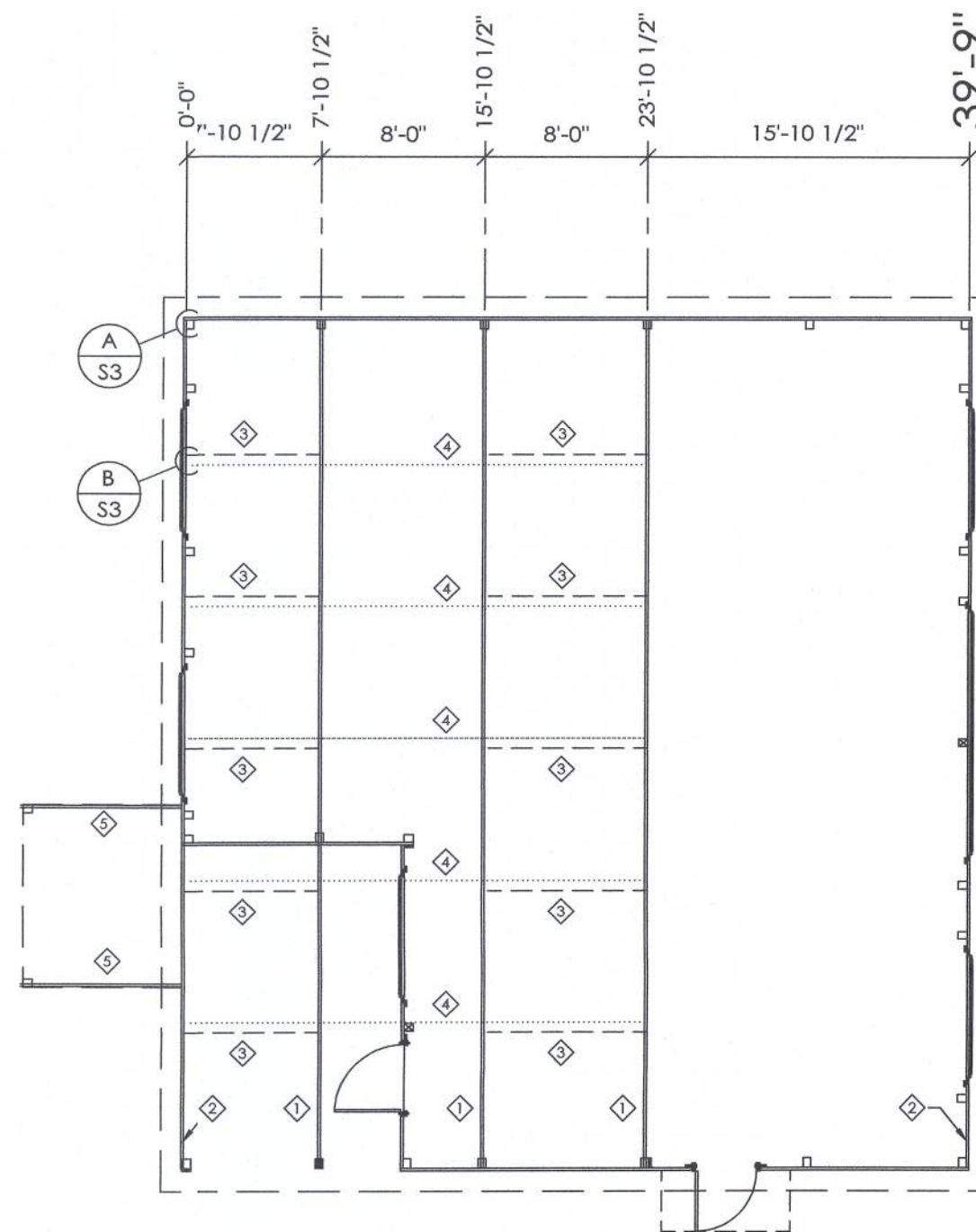
DONNALEE OR MICHAEL RELYEA

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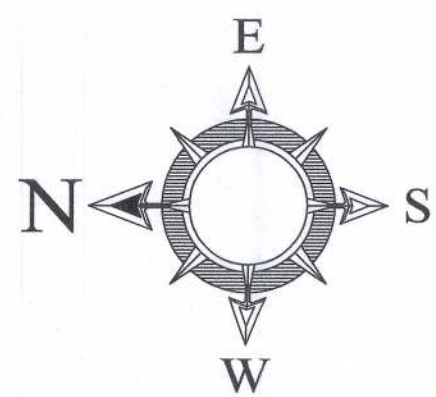
SCALE: AS NOTED
SHEET NO.
S2 OF S12



TRUSS/BRACING PLAN

TRUSS/BRACING PLAN LEGEND

- 42' 4075 S.C. TRUSS
- 42' END RAFTER ASSEMBLY
- 2x4 TRUSS TIES
- 2x6 DIAGONAL END BRACES
- CUSTOM 8' PORCH FRAME



SCALE: 1" = 4' 8"

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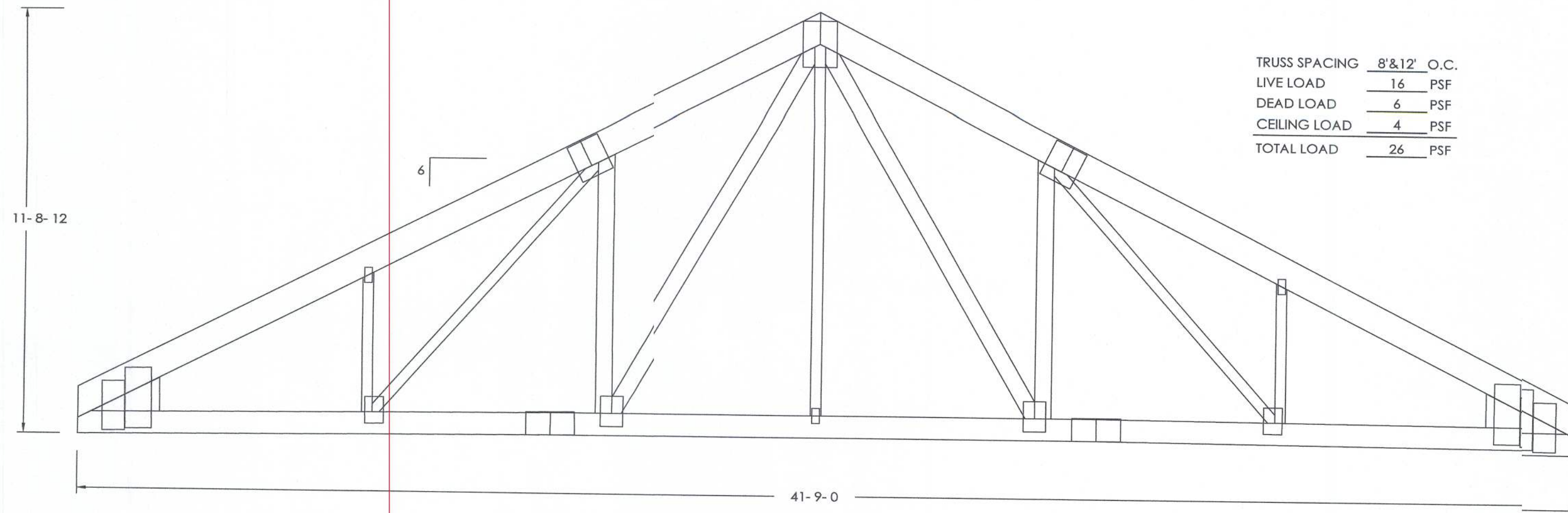
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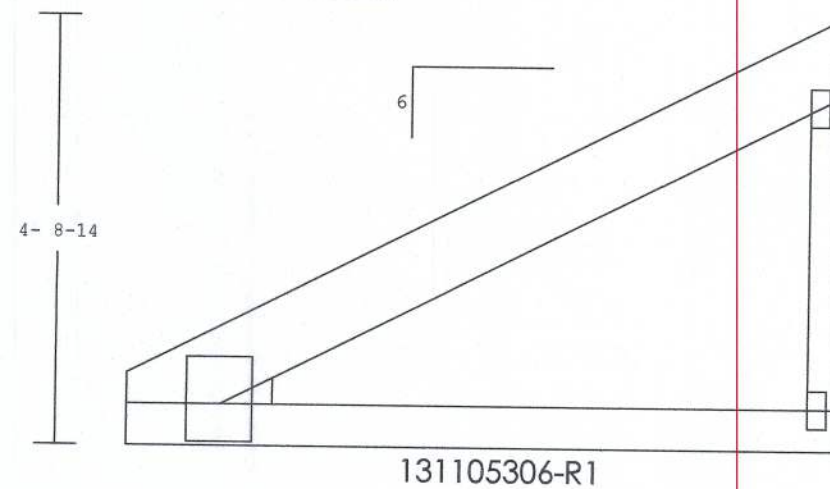
SCALE: AS NOTED
SHEET NO.
S3 OF S12

TRUSS SPACING	8' & 12' O.C.
LIVE LOAD	16 PSF
DEAD LOAD	6 PSF
CEILING LOAD	4 PSF
TOTAL LOAD	26 PSF

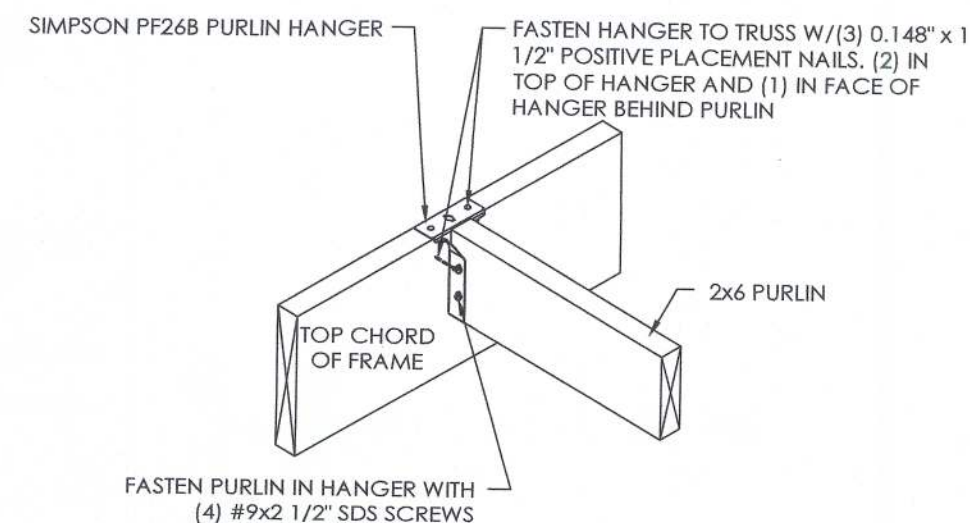


42' S.C. 4075 (4116) TRUSS
SCALE: 3/8" = 1'-0"

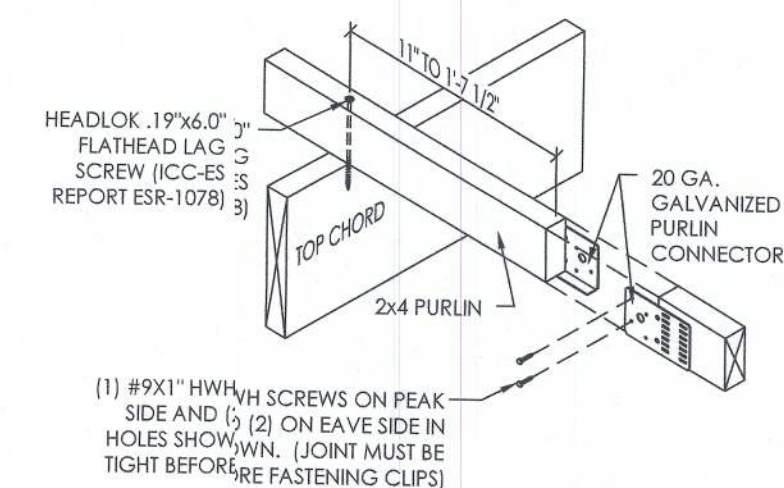
LIVE LOAD	20 PSF
DEAD LOAD	6 PSF
CEILING LOAD	2 PSF
TOTAL LOAD	28 PSF



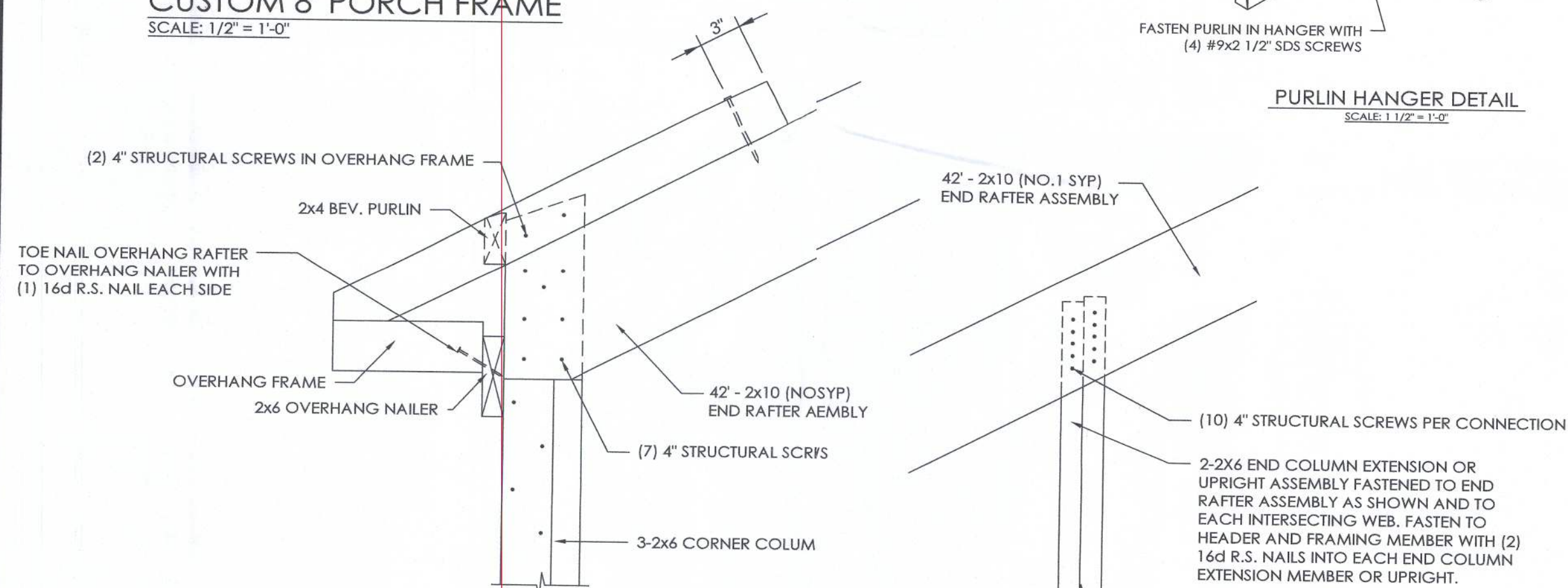
CUSTOM 8' PORCH FRAME
SCALE: 1/2" = 1'-0"



PURLIN HANGER DETAIL
SCALE: 1 1/2" = 1'-0"

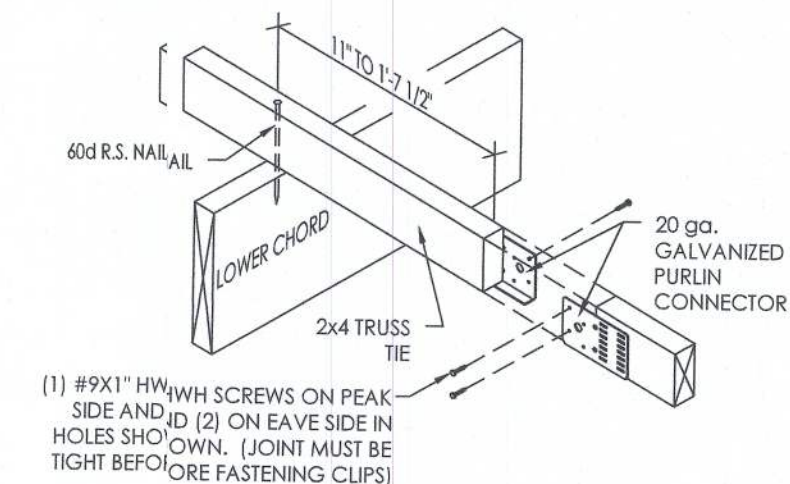


2x4 BUTTED PURLIN DETAIL
SCALE: 1" = 1'-0"

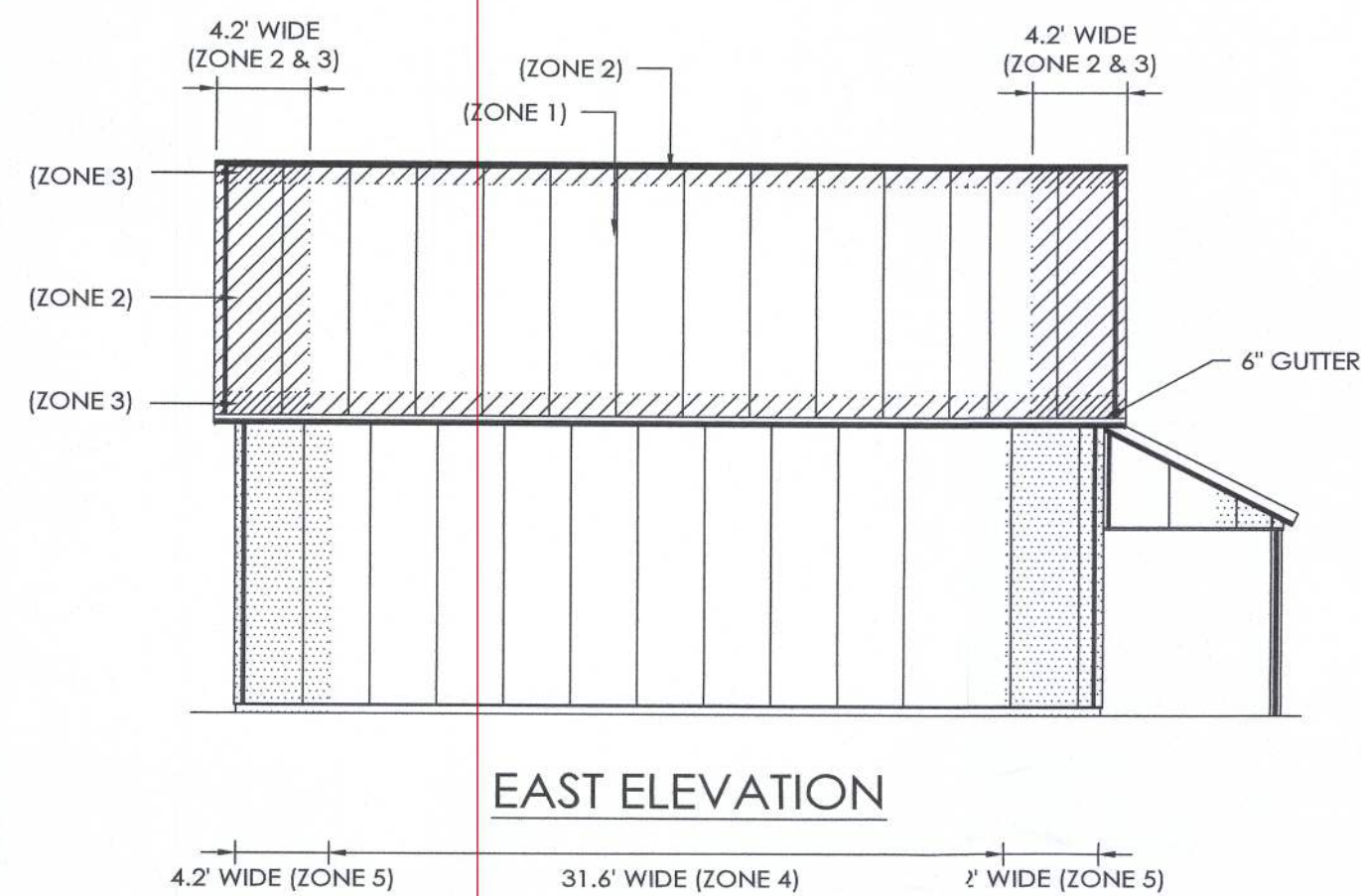
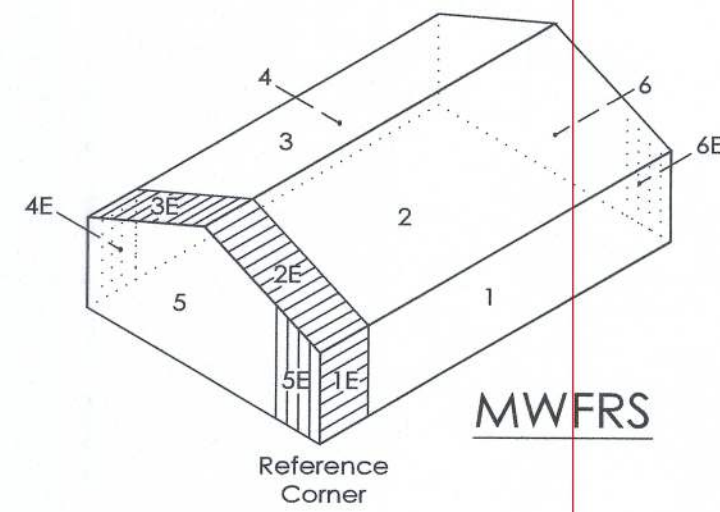


DETAIL A
SCALE: 1 1/2" = 1'-0"

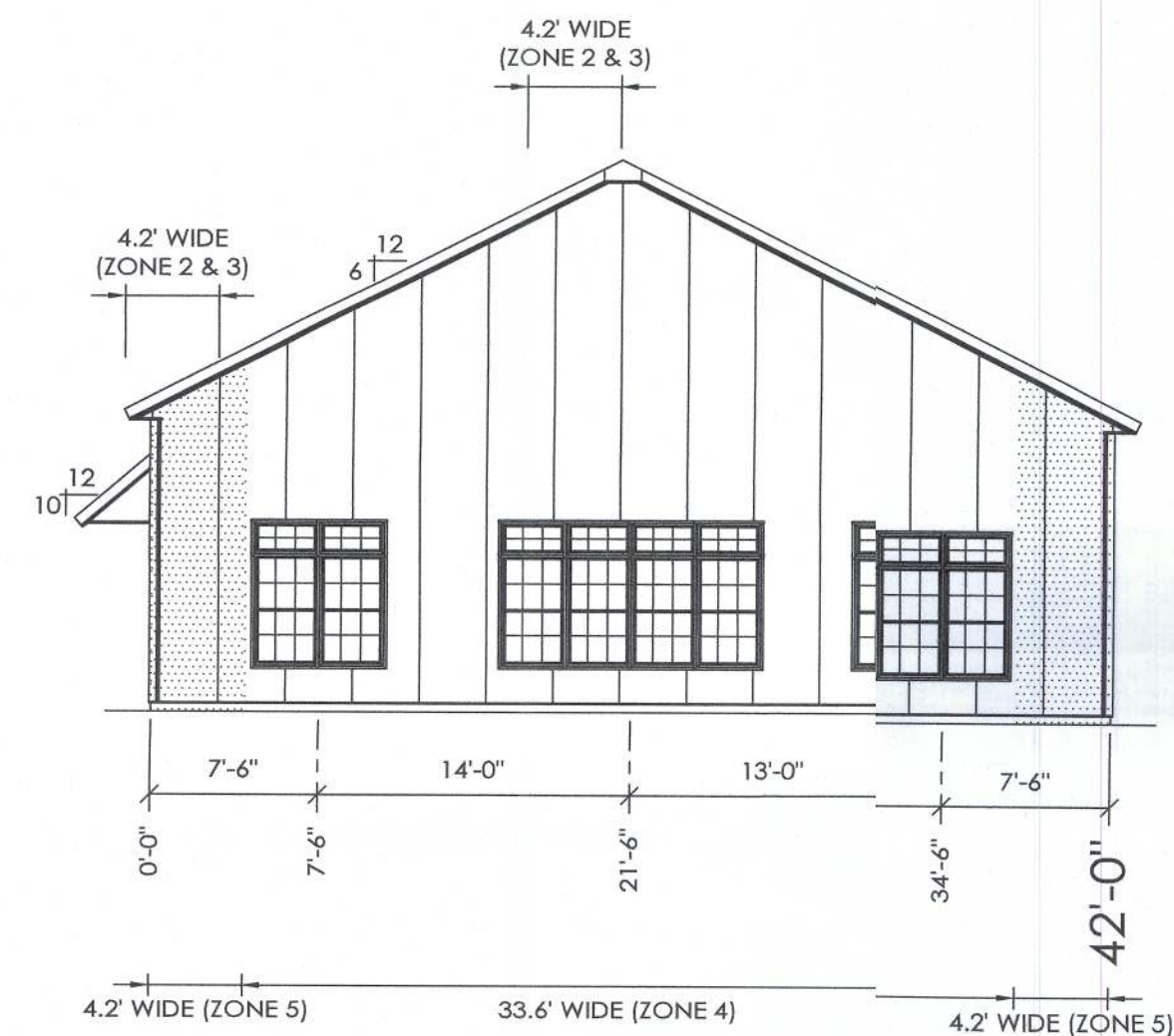
DETAIL B
SCALE: 1 1/2" = 1'-0"



2x4 TRUSS TIE DETAIL



EAST ELEVATION



SOUTH ELEVATION

DESIGN AND EXPLANATORY NOTES

- 1.) EXTERIOR DOOR AND WINDOW LOCATIONS ARE TAKEN FROM THE EXTERIOR FACE OF THE NAILERS AND ARE TO THE CENTER OF THE DOOR AND WINDOW UNITS. VERIFY ALL DOOR AND WINDOW LOCATIONS WITH THE OWNER.

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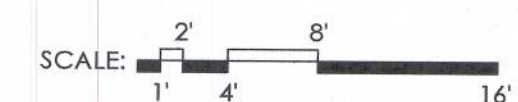
DONNALEE OR MICHAEL RELYEA
HIGH SPRINGS, FL

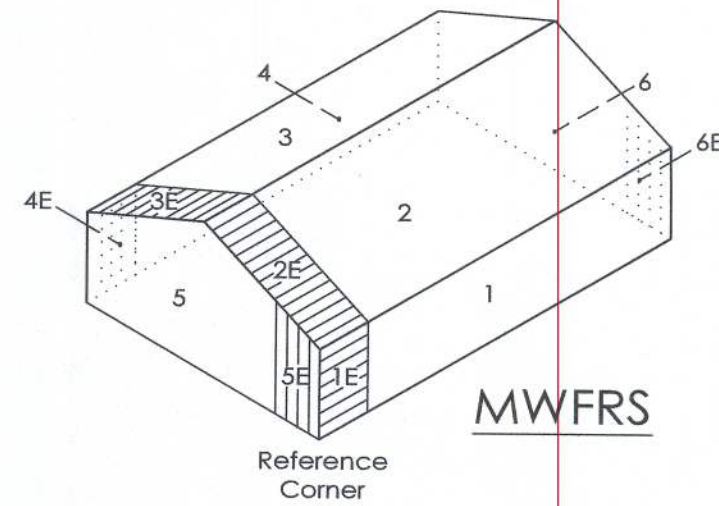
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SCALE: AS NOTED
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S4 OF S12



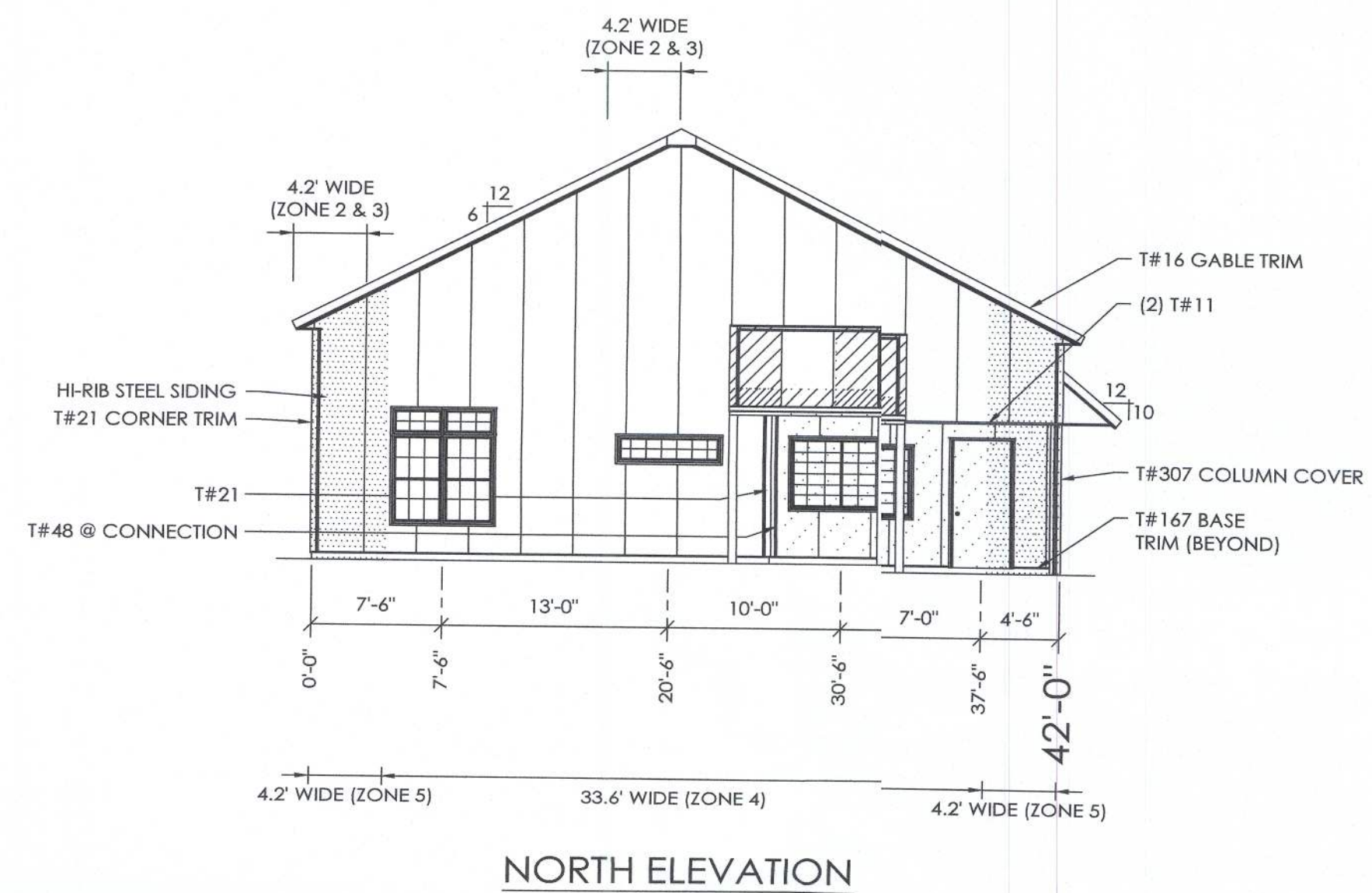
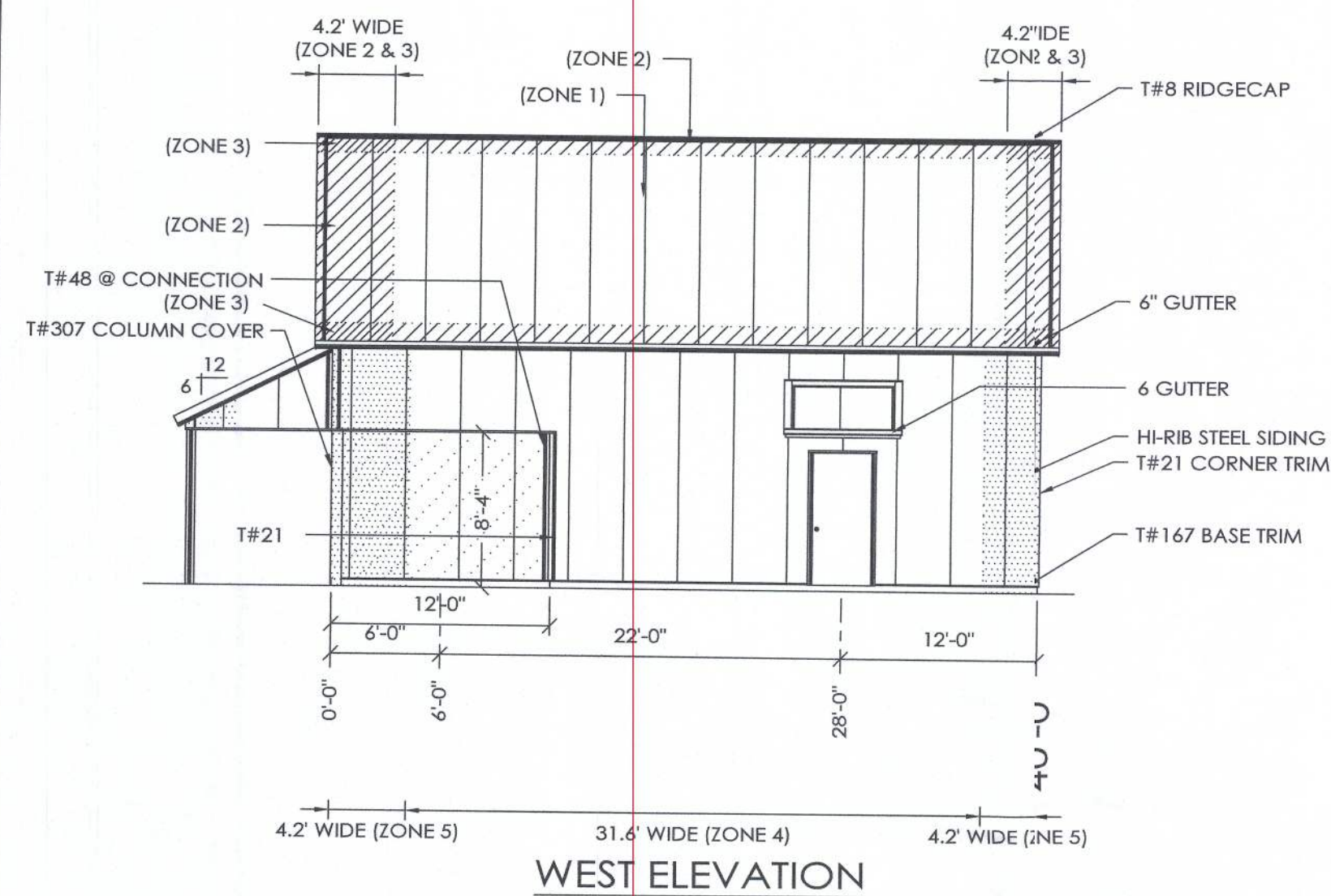


DESIGN AND EXPLANATORY NOTES

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HIGH SPRINGS, FL



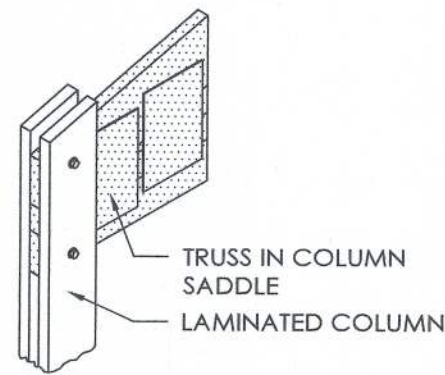
SCALE: 1" = 4'

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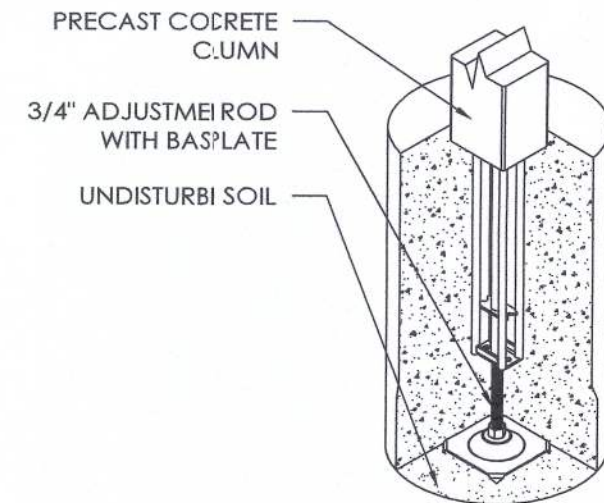


SCALE: AS NOTED
SHEET NO. S5 OF S12

TRUSS SADDLE ISOMETRIC



LOWER COLUMN ISOMETRIC



LOWER COLUMN INSTALLATION

1. INSTALL PRECAST CONCRETE COLUMN W/ADJUSTMENT ROD & BASE PLATE IN THE AUGERED HOLE.
2. PLUMB PRECAST CONCRETE COLUMN IN BOTH DIRECTIONS
3. ADJUST HEIGHT UP OR DOWN WITH ADJUSTMENT HEX ROD
4. POUR READI-MIX CONCRETE INTO THE HOLE AS SPECIFIED.
5. BACKFILL AND COMPACT THE ANNULAR SPACE AROUND THE COLUMN TO GRADE WITH SOIL AUGERED FROM THE SITE.

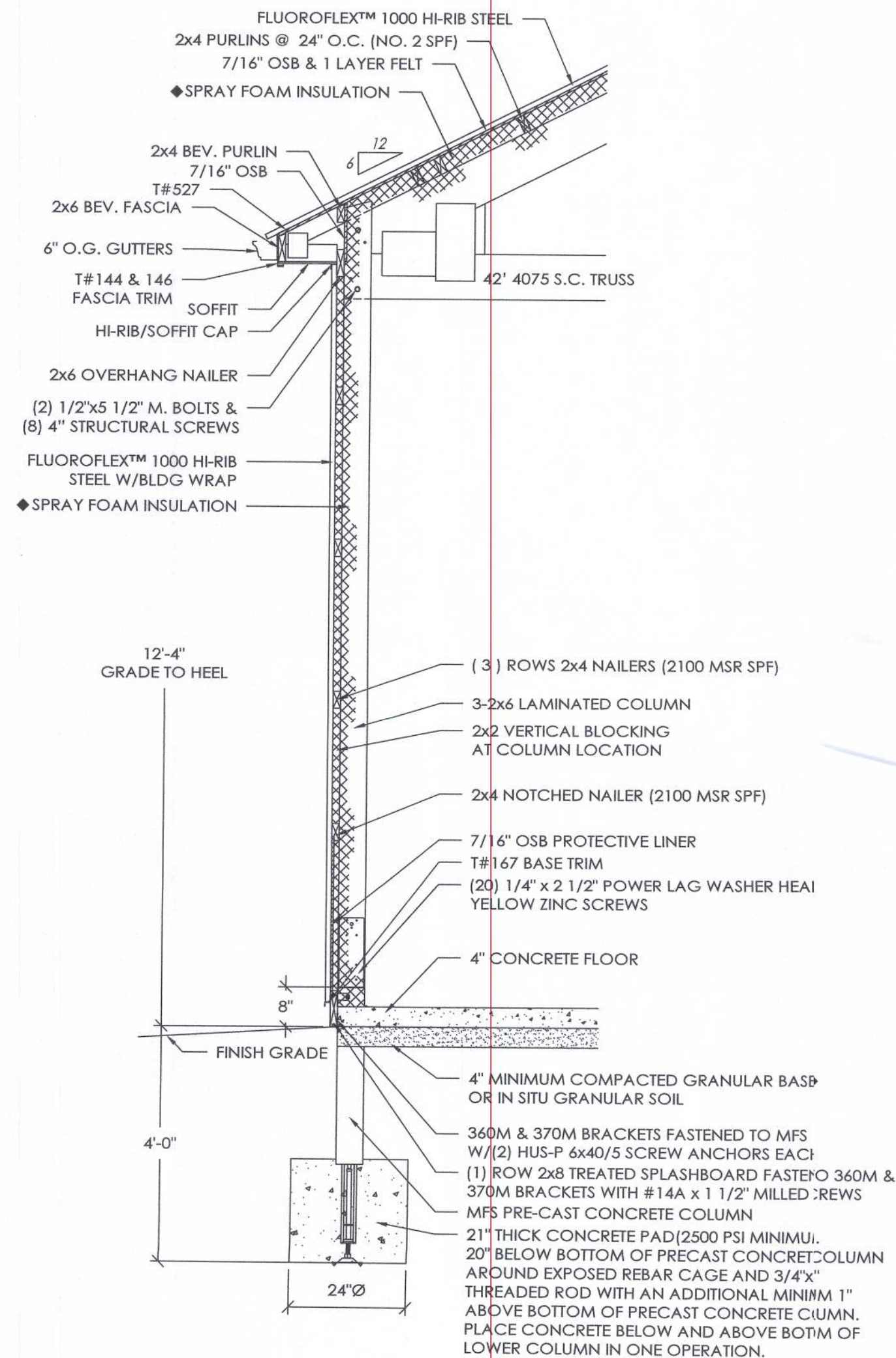
DESIGN AND EXPLANATORY NOTES

1. FOOTINGS ARE DESIGNED FOR A 2000 PSF SOIL BEARING CAPACITY. LOCAL CONDITIONS MAY REQUIRE MODIFICATIONS.
2. CONCRETE FLOOR NOTES:
 - a. 3500 PSI, 5 1/2 BAG MIX CONCRETE.
 - b. SLOPE GRADE AWAY FROM BUILDING @ 1" PER FOOT FOR A MINIMUM DISTANCE OF 10' PLUS OVERHANG WIDTH.
 - c. PLACE A MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER OVER A COMPACTED GRANULAR BASE AND DIRECTLY BELOW THE CONCRETE FLOOR.
 - d. CONTRACTION JOINTS UNIFORMLY SPACED 12' O.C. OR LESS.
3. PRIOR TO PLACING THE CONCRETE FOOTINGS, HAND TAMP THE BOTTOM 2'-3" OF LOOSE SOIL TO CONSOLIDATE. IF THE DRILLED HOLE CONTAINS MORE THAN 3" OF LOOSE SOIL, REMOVE EXCESS SOIL TO A UNIFORM THICKNESS OF 2'-3", HAND TAMP AND PROCEED WITH CONCRETE FOOTING PLACEMENT.
4. DO NOT PLACE CONCRETE FOOTING THROUGH MORE THAN 3" OF STANDING WATER. IF MORE THAN 3" OF STANDING WATER IS PRESENT IN THE FOOTING HOLE CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR INSTALLATION INSTRUCTIONS.

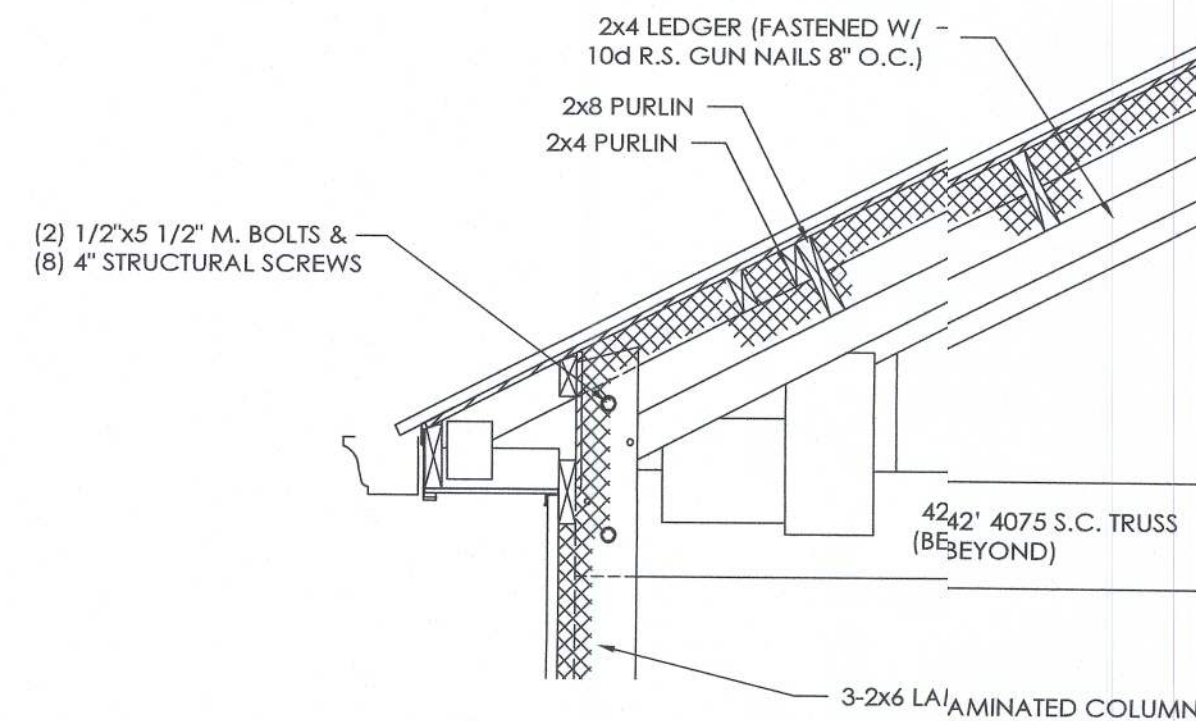
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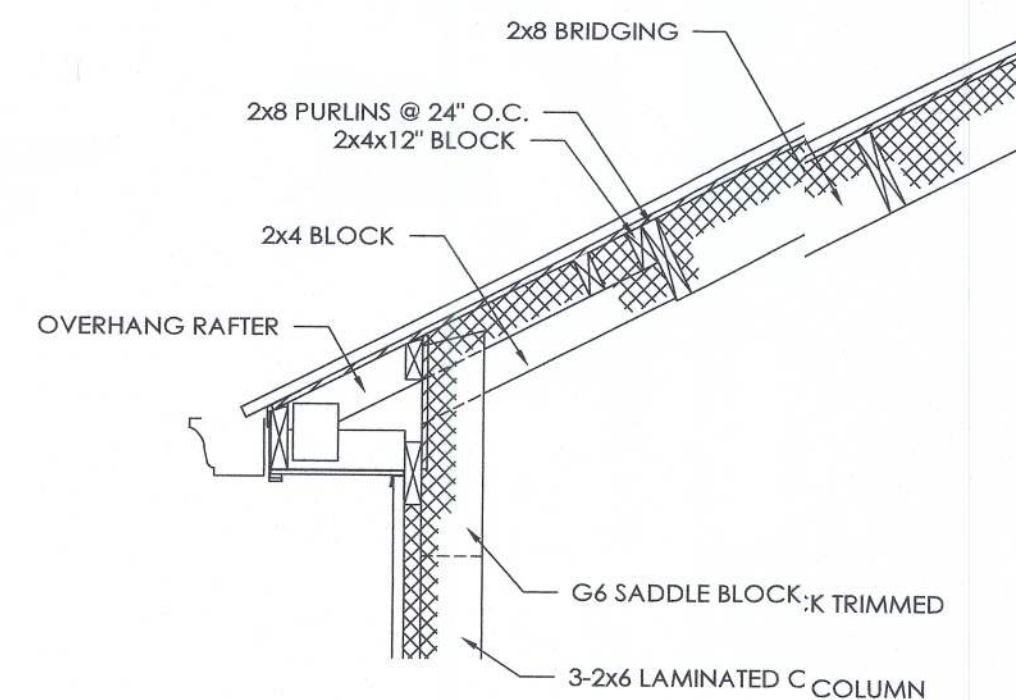
DONNALEE OR MICHAEL RELYEA
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SIDEWALL SECTION A
SCALE: 1/2" = 1'-0"



SIDEWALL SECTION C
SCALE: 1/2" = 1'-0"



SIDEWALL SECTION B
SCALE: 1/2" = 1'-0"

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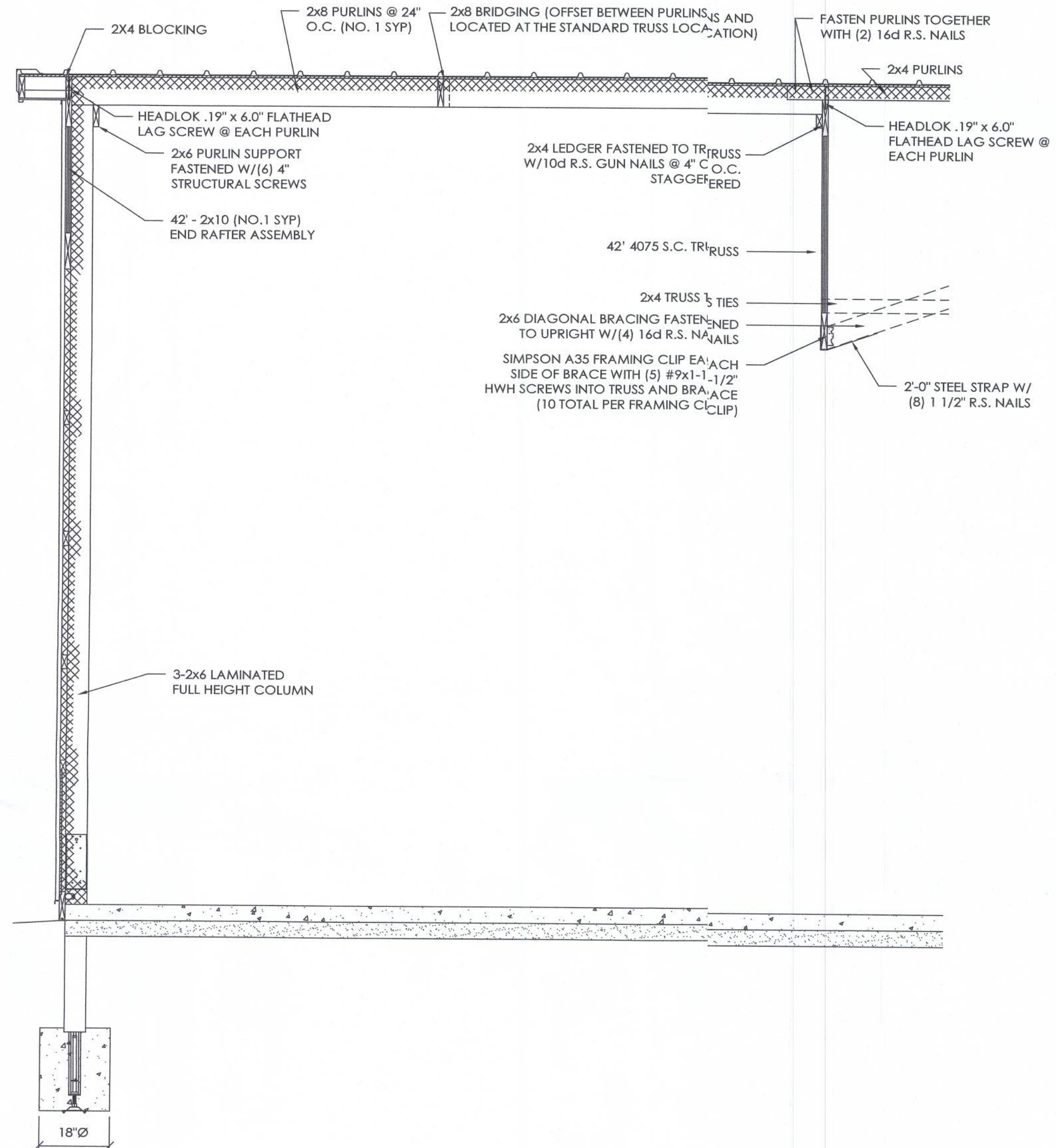
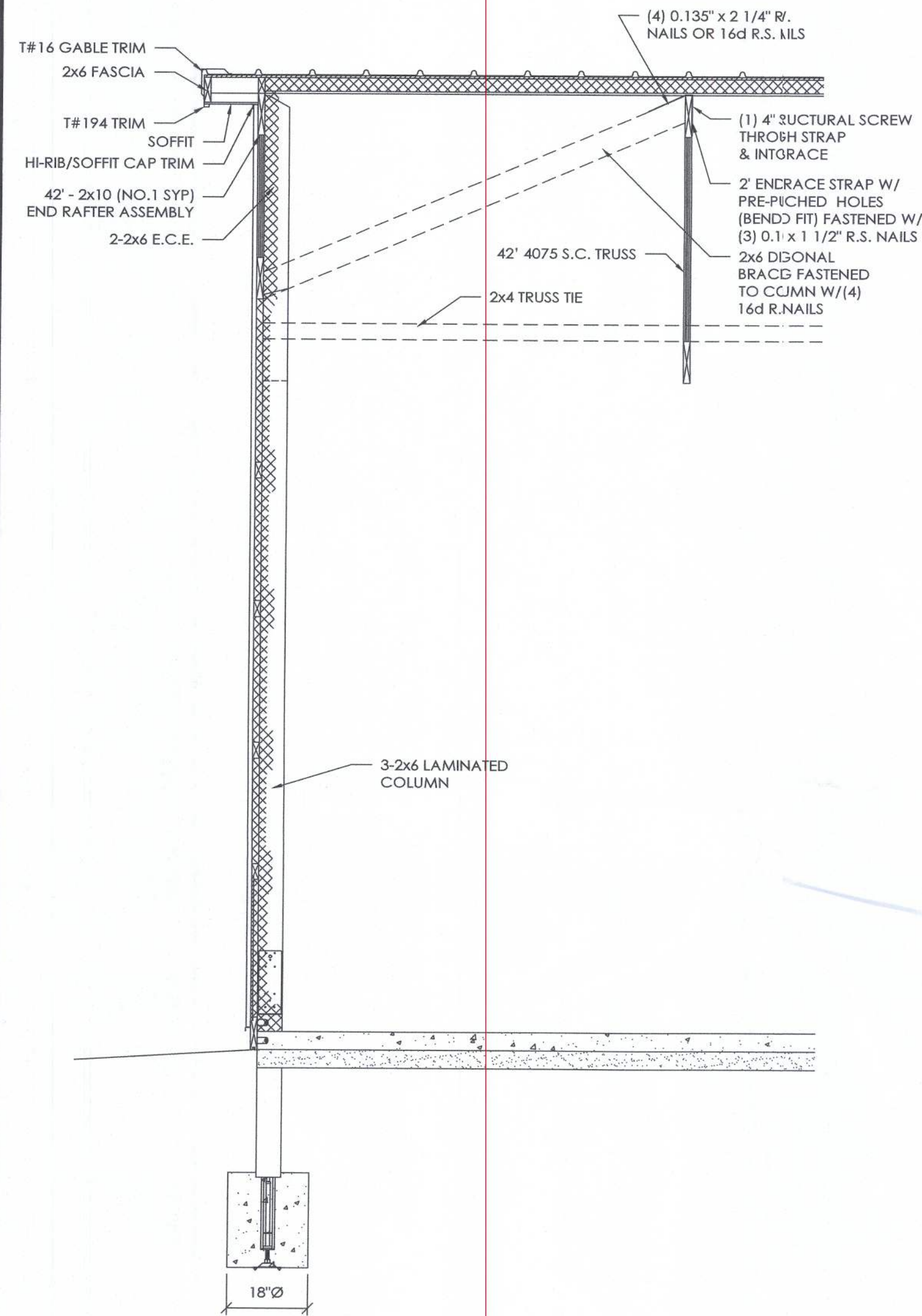
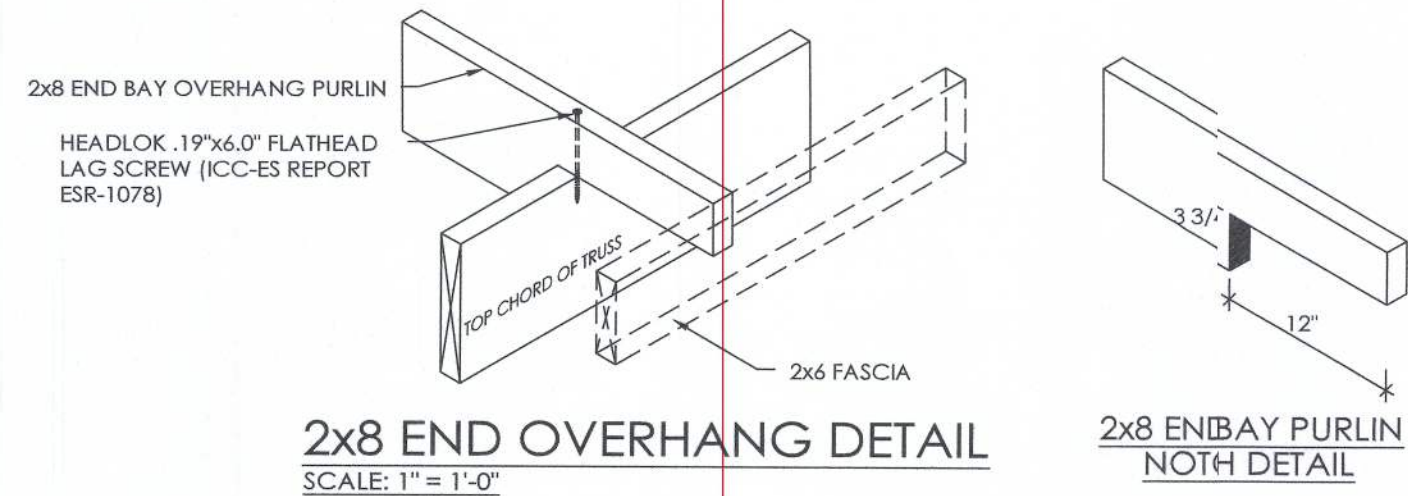
SCALE: AS NOTED
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DONNALEE OR MICHAEL RELYEA
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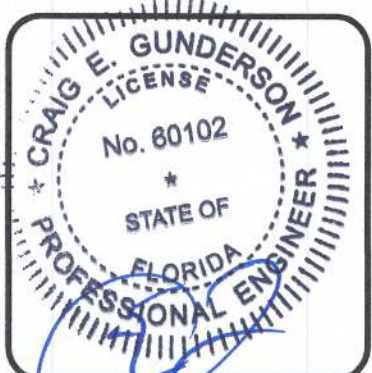


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SCALE: AS NOTED
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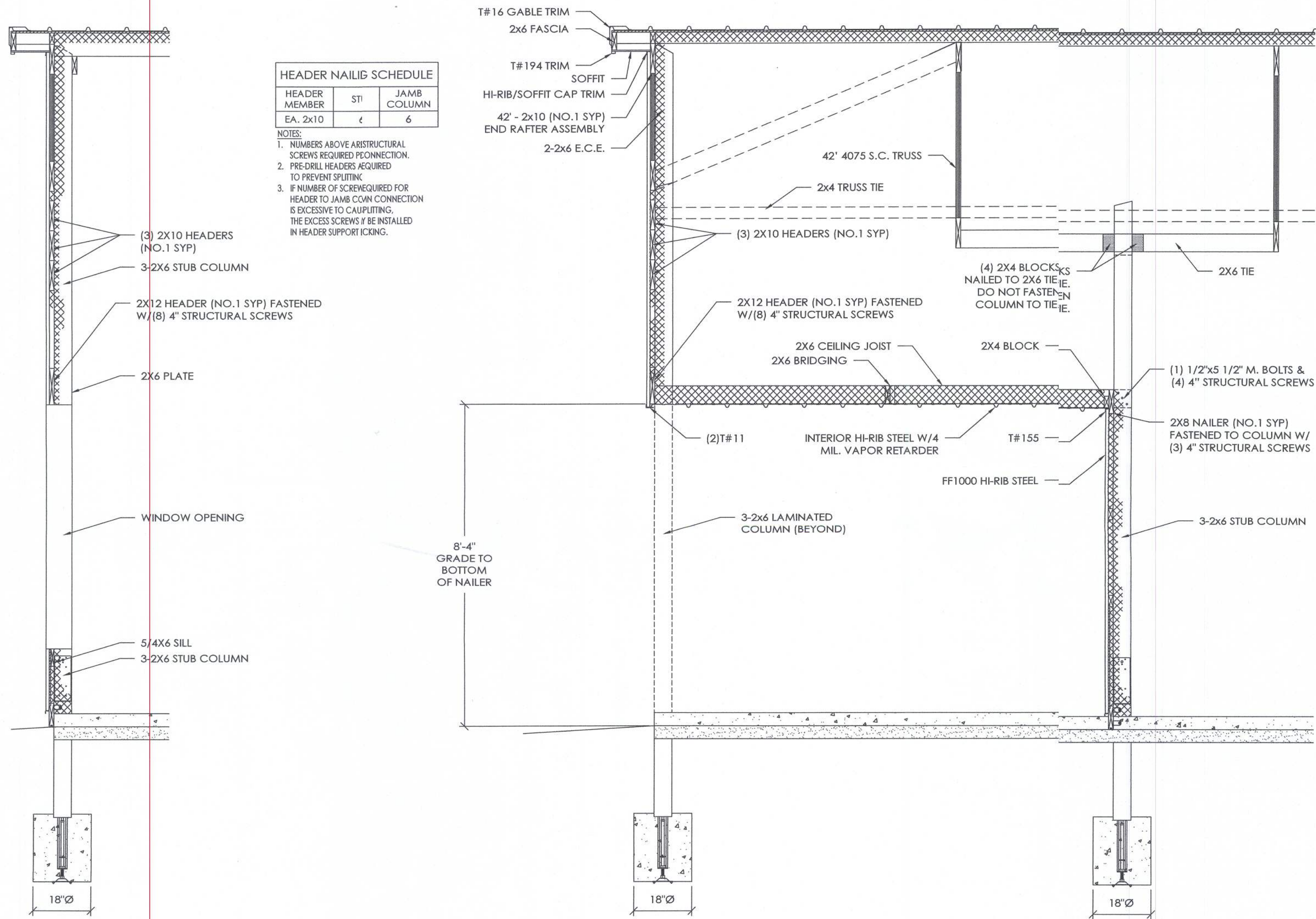
DONNALEE OR MICHAEL RELYEA
HIGH SPRINGS, FL

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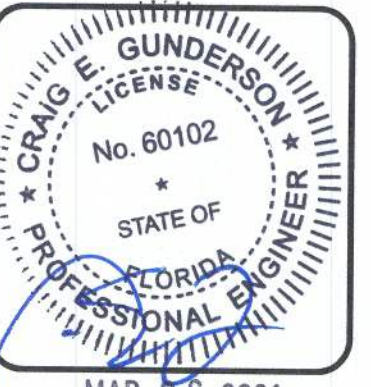
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SCALE: AS NOTED
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DONNALEE OR MICHAEL RELYEA
HIGH SPRINGS, FL

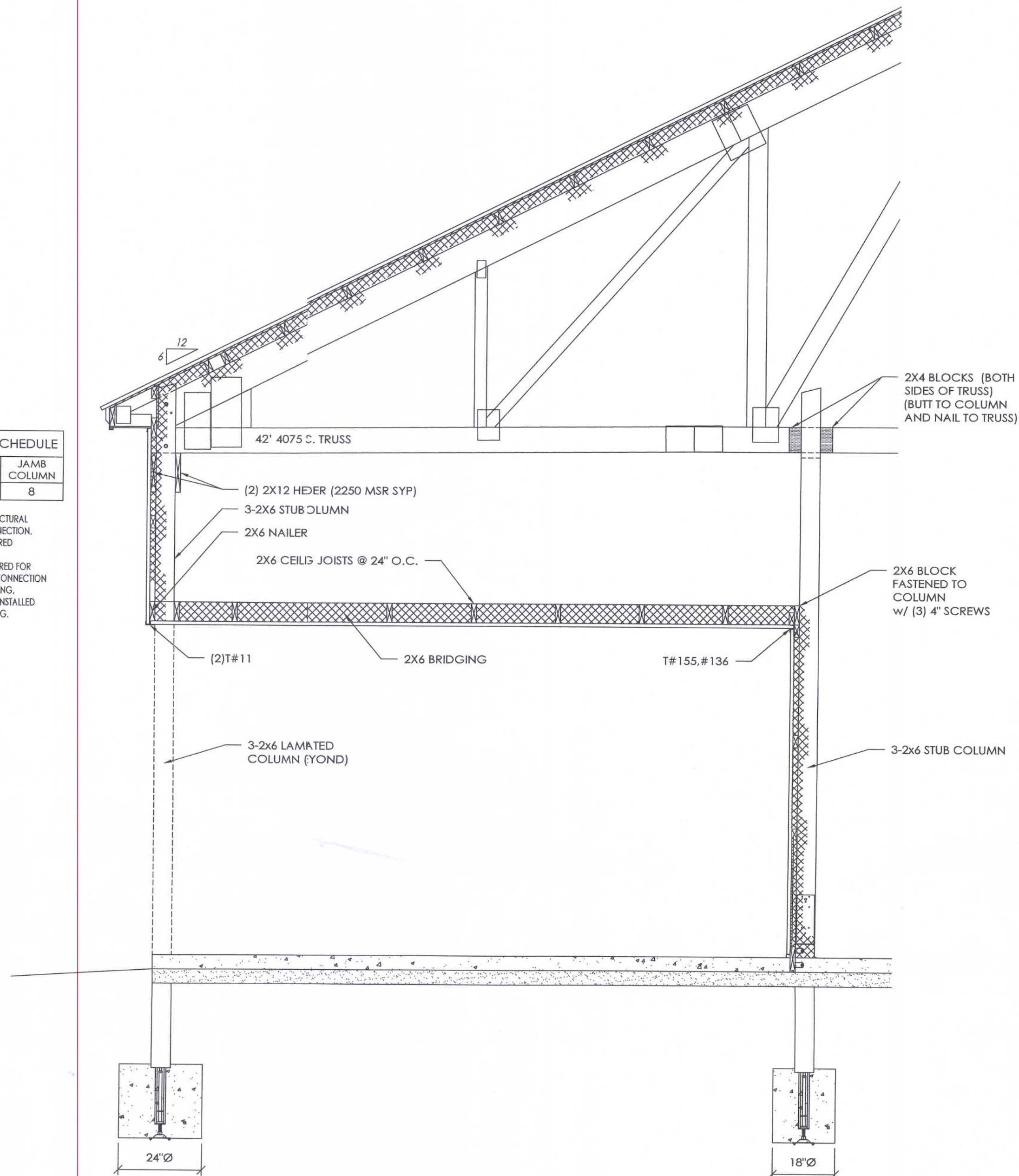
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SCALE: AS NOTED
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HEADER NAILING SCHEDULE		
HEADER MEMBER	STUB	JAMB COLUMN
EA. 2x12	12	8

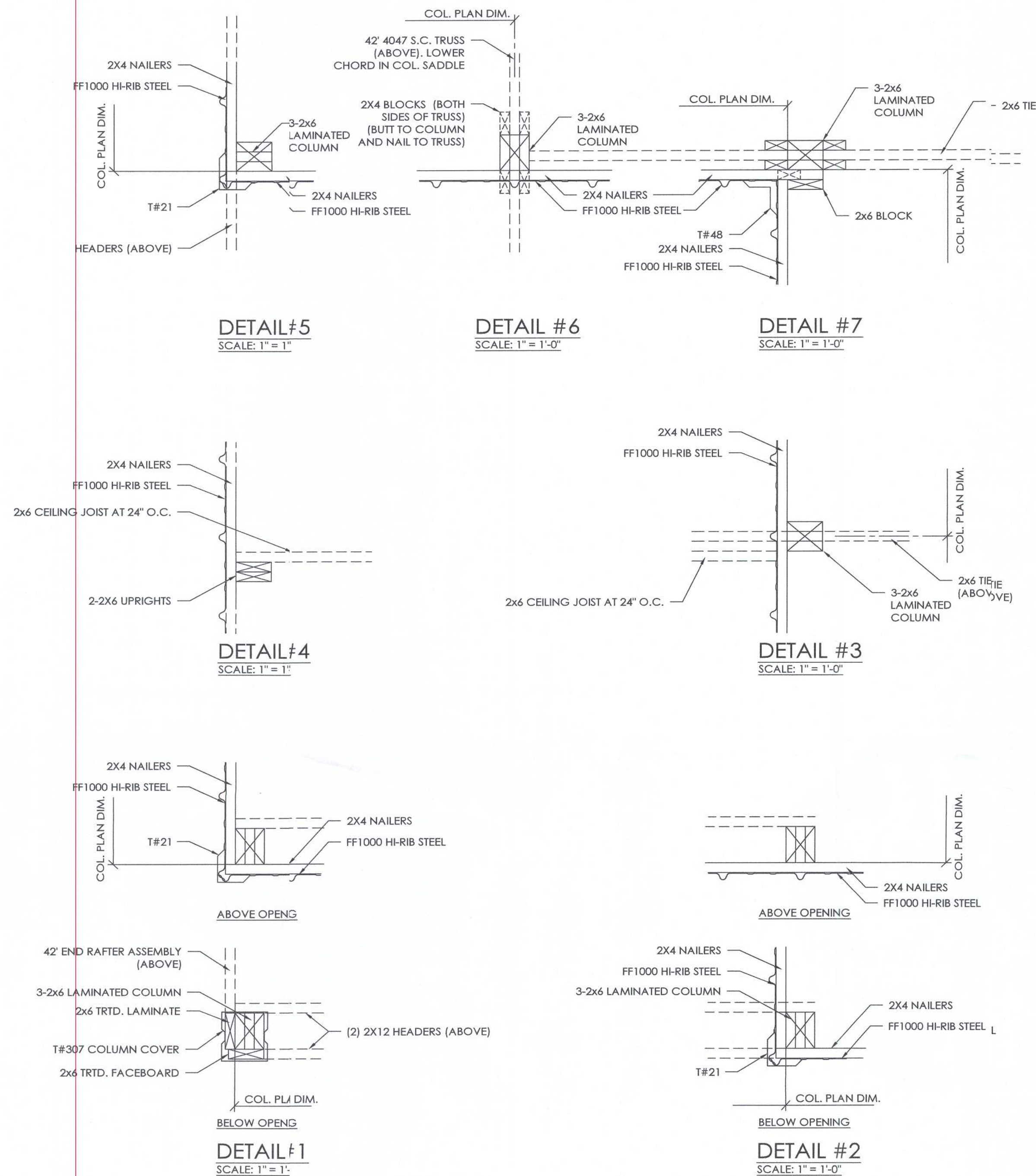
- NOTES:
1. NUMBERS ABOVE ARE 4" STRUCTURAL SCREWS REQUIRED PER CONNECTION.
 2. PRE-DRILL HEADERS AS REQUIRED TO PREVENT SPLITTING.
 3. IF NUMBER OF SCREWS REQUIRED FOR HEADER TO JAMB COLUMN CONNECTION IS EXCESSIVE TO CAUSE SPLITTING, THE EXCESS SCREWS MAY BE INSTALLED IN HEADER SUPPORT BLOCKING.



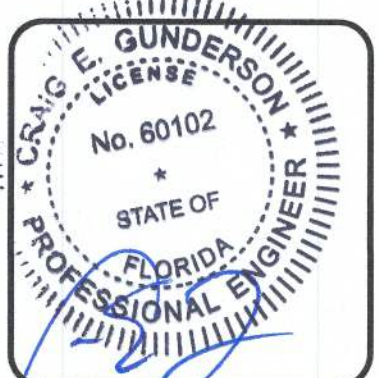
SIDEWALL SECTION G
SCALE: 1/2" = 1'-0"

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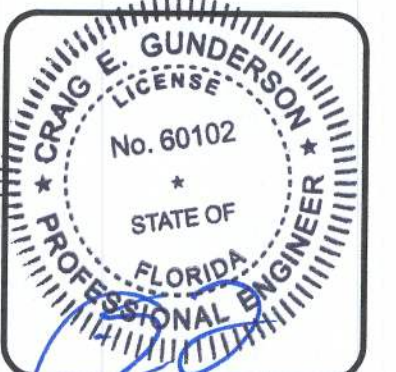
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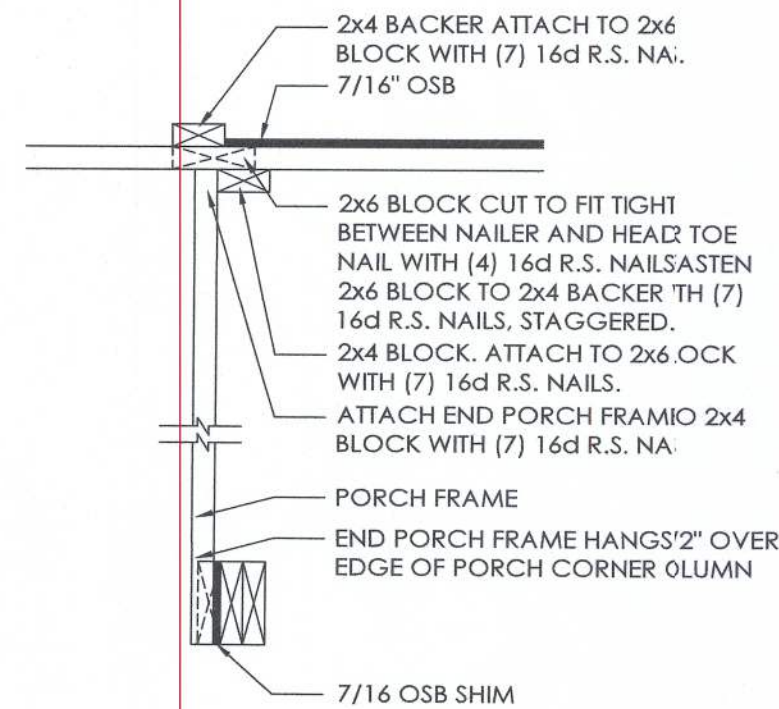


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SCALE: AS NOTED

SHEET NO.

S11 OF S12



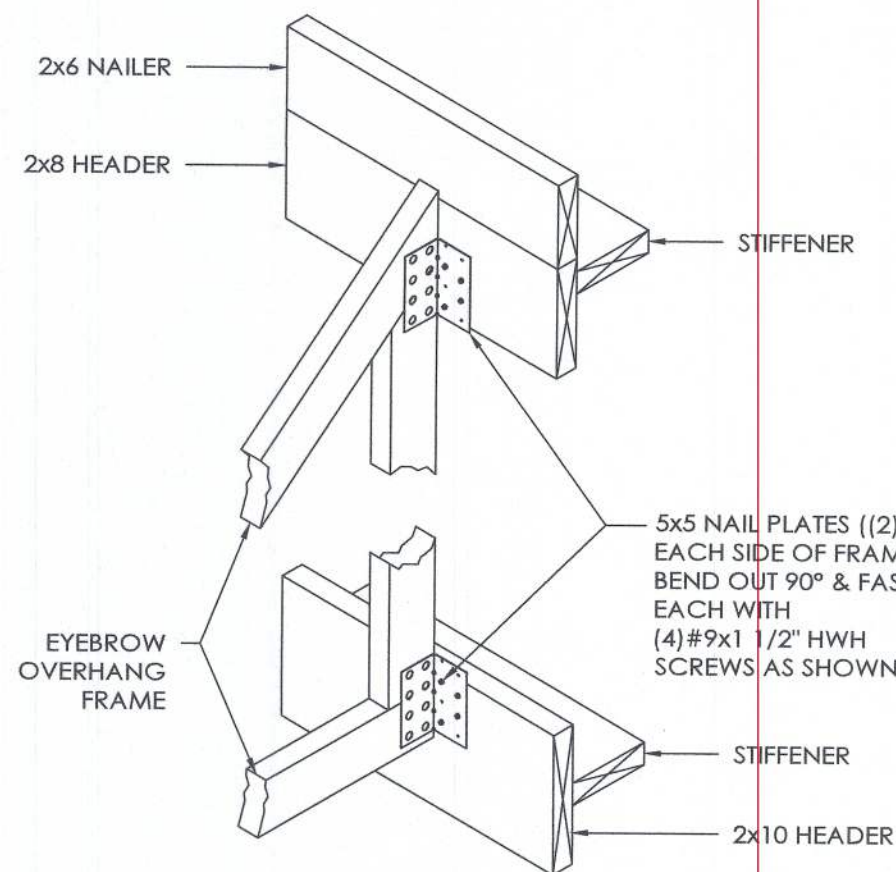
TOP VIEW OF END CORNER

2x4 BACKER ATTACH TO 2x6
BLOCK WITH (7) 16d R.S. NAILS.

2x6 BLOCK CUT TO FIT TIGHT
BETWEEN NAILER AND HEADER TOE
NAIL WITH (4) 16d R.S. NAILS. FASTEN
2x6 BLOCK TO 2x4 BACKER TH (7)
16d R.S. NAILS, STAGGERED.

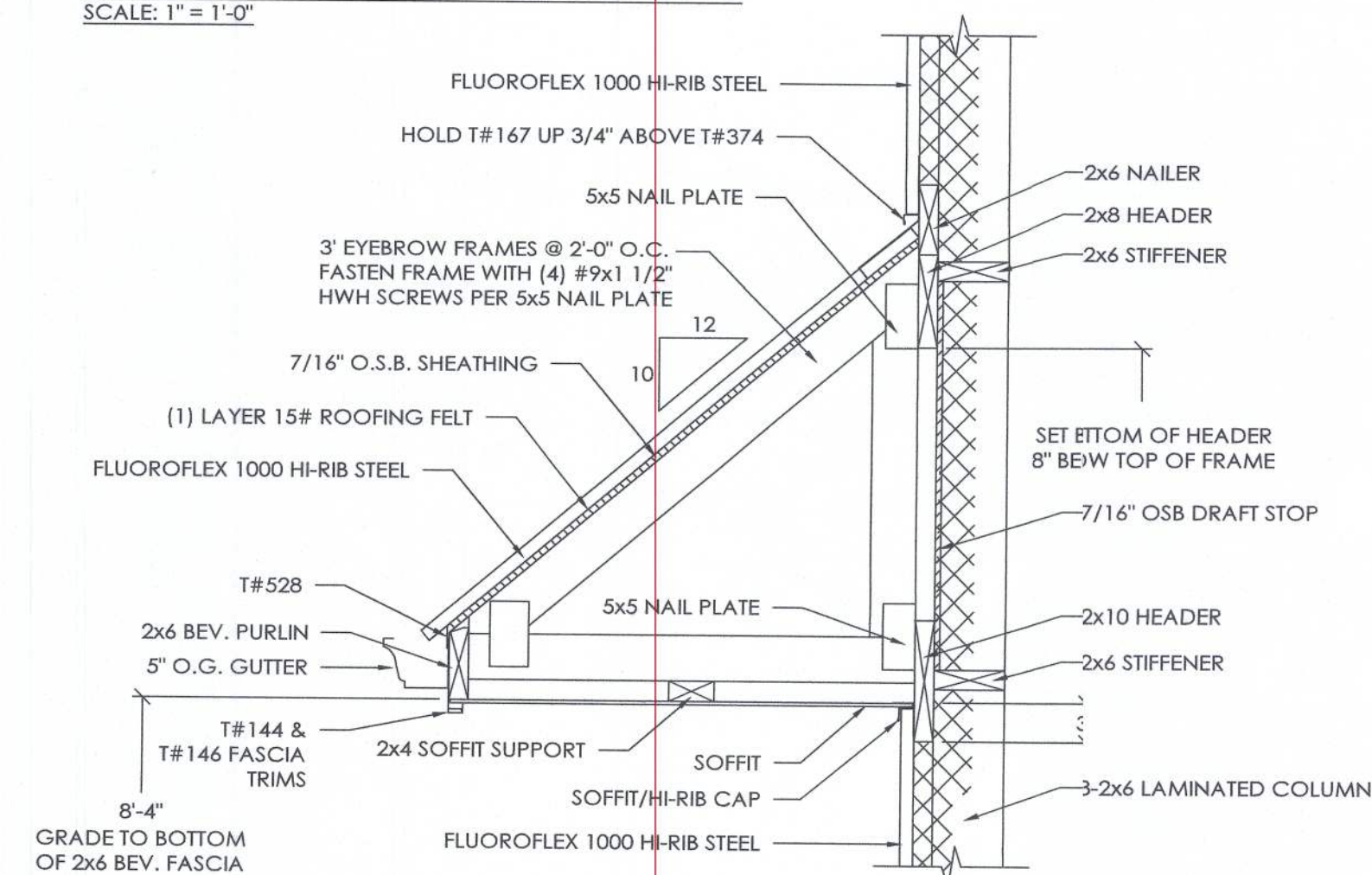
2x4 BLOCK. ATTACH TO 2x6 BLOCK
WITH (7) 16d R.S. NAILS.
ATTACH END PORCH FRAME TO
2x4 BLOCK WITH (7) 16d R.S. NAILS.

END VIEW OF END PORCH FRAME



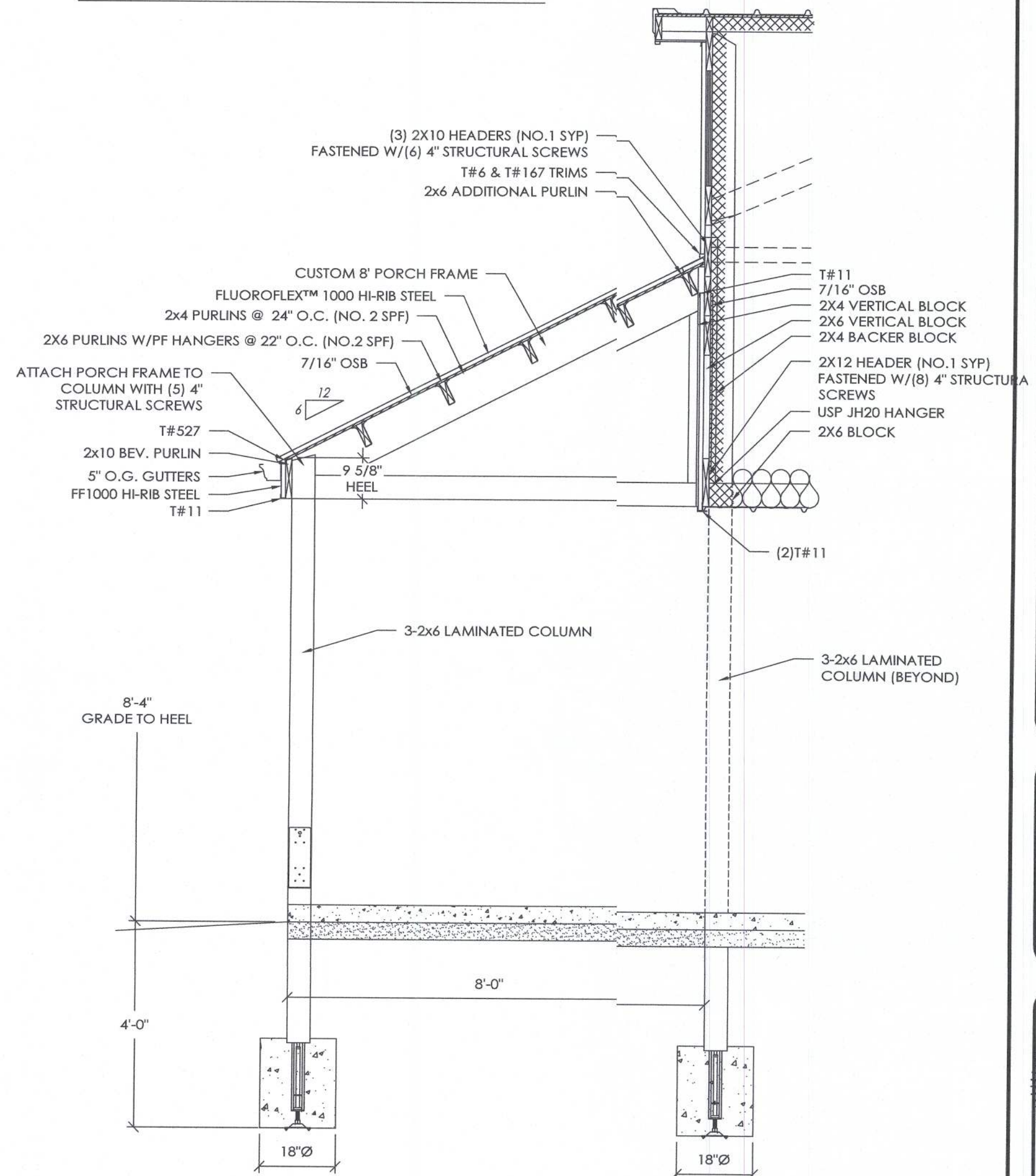
EYEBROW ATTACHMENT DETAIL

SCALE: 1" = 1'-0"



3'-0" EYEBROW OVERHANG SECTION H

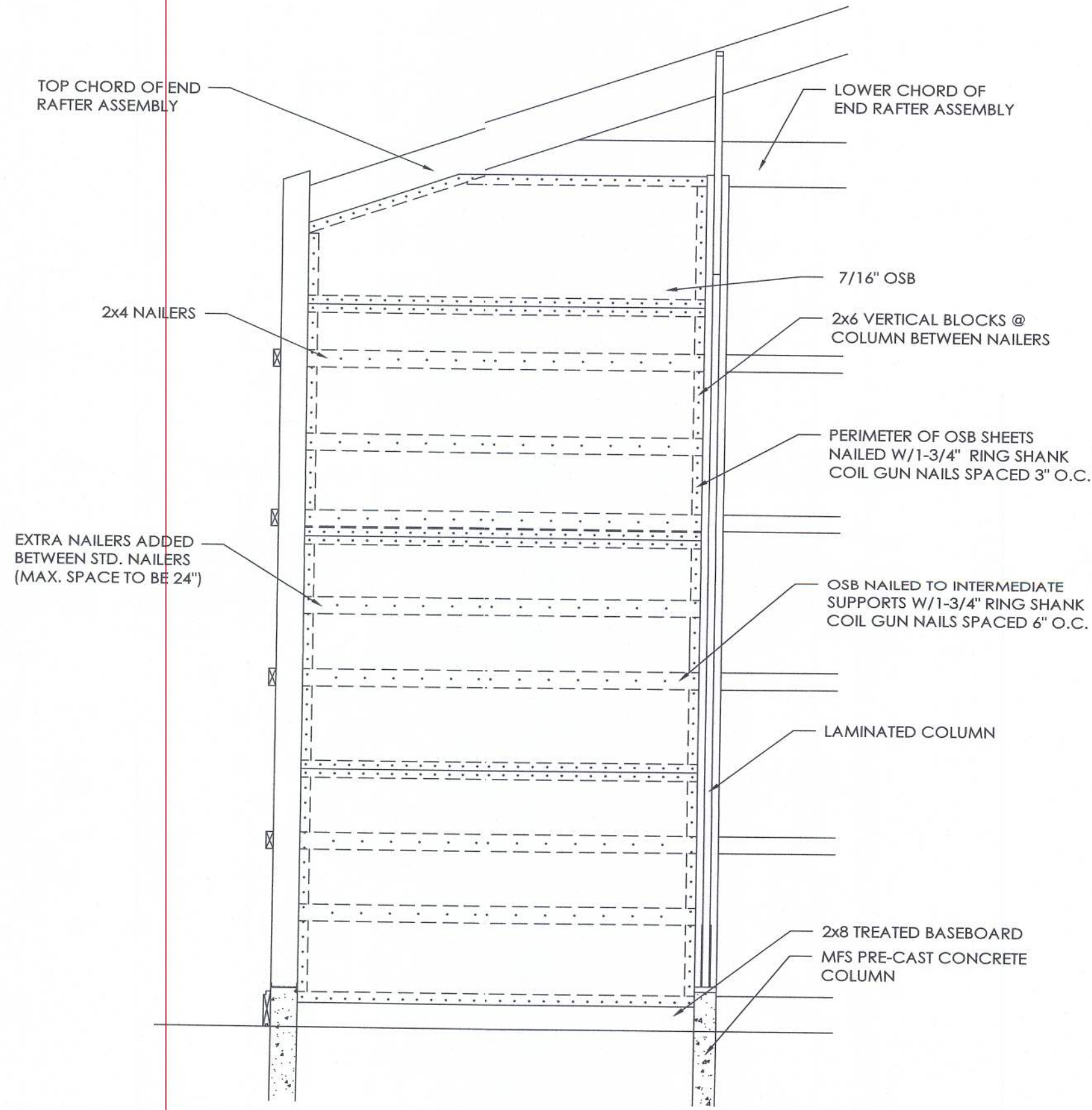
SCALE: 1" = 1'-0"



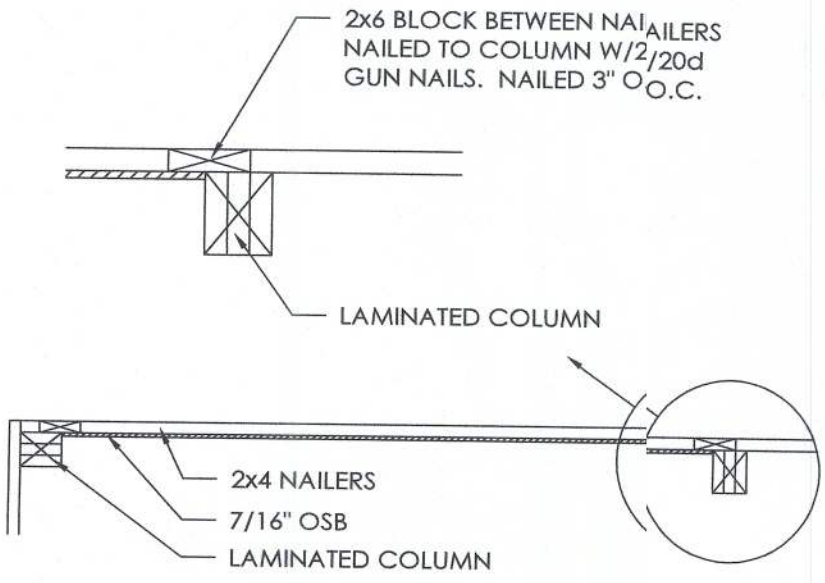
8' PORCH SECTION I

SCALE: 1/2" = 1'-0"

ROOF STRUCTURE FASTENING SCHEDULE		
⑧	VENT-A-RIDGE TO BASE TRIM	#9 x 1" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS @ 8" o.c.
⑨	RIDGE BASE TRIM TO 2x4 PURLINS	#9 x 2 1/2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
⑩	HI-RIB STEEL TO 2x4 PURLINS	#9 x 2 1/2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
⑪	7/16" OSB SHEATHING TO NAILERS	0.113" x 2-3/8" (8D) GUN NAILS @ 12" O.C. @ INTERMEDIATE FRAMING & 6" O.C. @ PANEL EDGES
WALL FRAMING FASTENING SCHEDULE		
①	2x4 NOTCHED NAILER TO COLUMN	(4) 0.148" x 3-1/2" (16d) NAILS @ SPLICE/ (3) 0.148" x 3-1/2" (16d) NAILS @ STANDARD CONNECTION
②	7/16" OSB TO SPLASHBOARD & NOTCHED NAIL	0.099" x 1-1/4" ASBESTOS SIDING NAILS
③	2x4 NAILER TO COLUMN	(4) 0.148" x 3-1/2" (16d) RING SHANK NAILS @ SPLICE/ (3) 0.148" x 3-1/2" (16d) RING SHANK NAILS @ STANDARD CONNECTION
④	END RAFTER ASSEMBLY TO 2-2x6 END COLUMN TENSIONS	(6) U2 #12x4" COUNTERSINK UNIVERSAL SCREW OR SPAX #14x4" FLAT HEAD MULTI-PURPOSE CONSTRUCTION SCREW
⑤	HI-RIB STEEL TO NAILERS	#9 x 2" STAINLESS STEEL RUBBER WASHER PANHEAD INTERNAL DRIVE SCREWS AT EVERY HI-RIB (1'-0" o.c.)
⑥	SOFFIT TO WALL	INSERTED IN PRE-FORMED SLOT IN SOFFIT/HI-RIB CAP
⑦	SOFFIT TO FASCIA	T-50 MONEL STAPLES (2) PER PIECE



7/16" OSB SHEARWALL ELEVATION
SCALE: 1/2" = 1'-0"



OSB SHEARWALL DETAIL
SCALE: 1/2" = 1'-0"

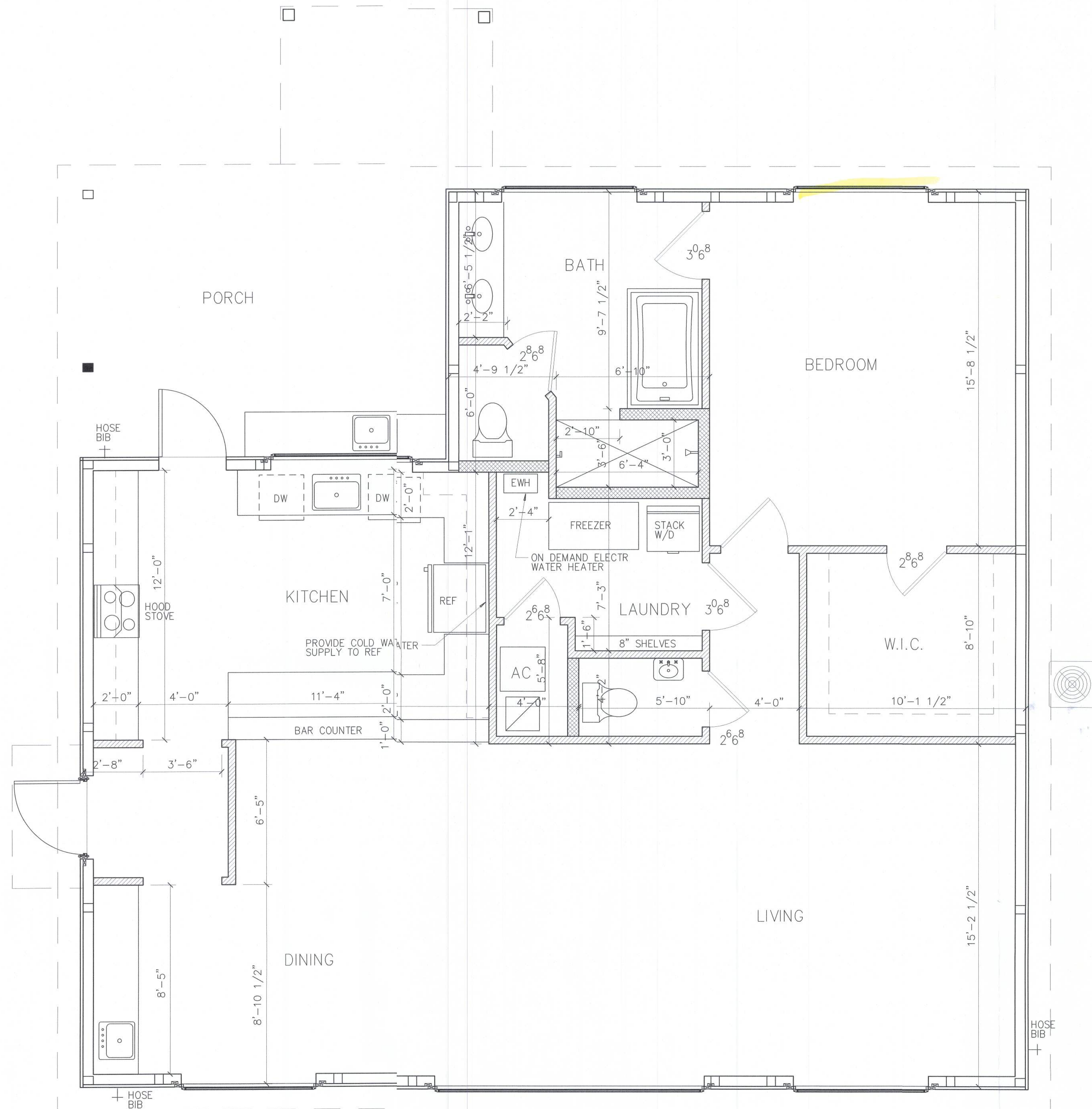
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DONNALEE OR MICHAEL RELYEA
HIGH SPRINGS, FL

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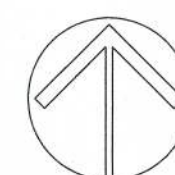
MAR 26 2021
SCALE: AS NOTED
SHEET NO.
S12 OF S12



CONDITIONED AREA: 1,468 S.F.

INTERIOR FLOOR PLAN

3/8"=1'-0"



LEGEND

- MORTON EXTERIOR WALL
- INTERIOR 2X6 WOOD STUD WALL
- INTERIOR 2X4 WOOD STUD WALL



Main Hs.

4/21/2021

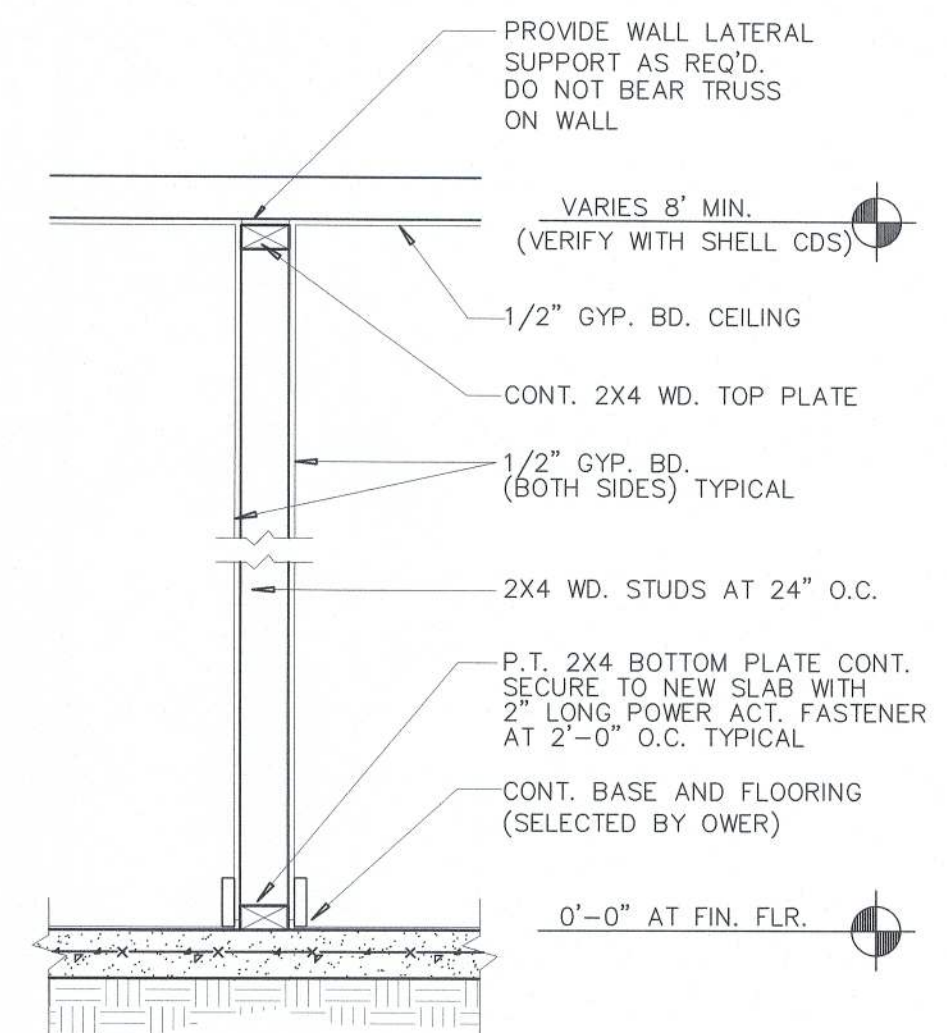
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PROJECT: SA-2106
DRAWN BY: J.W.S.
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A1.0

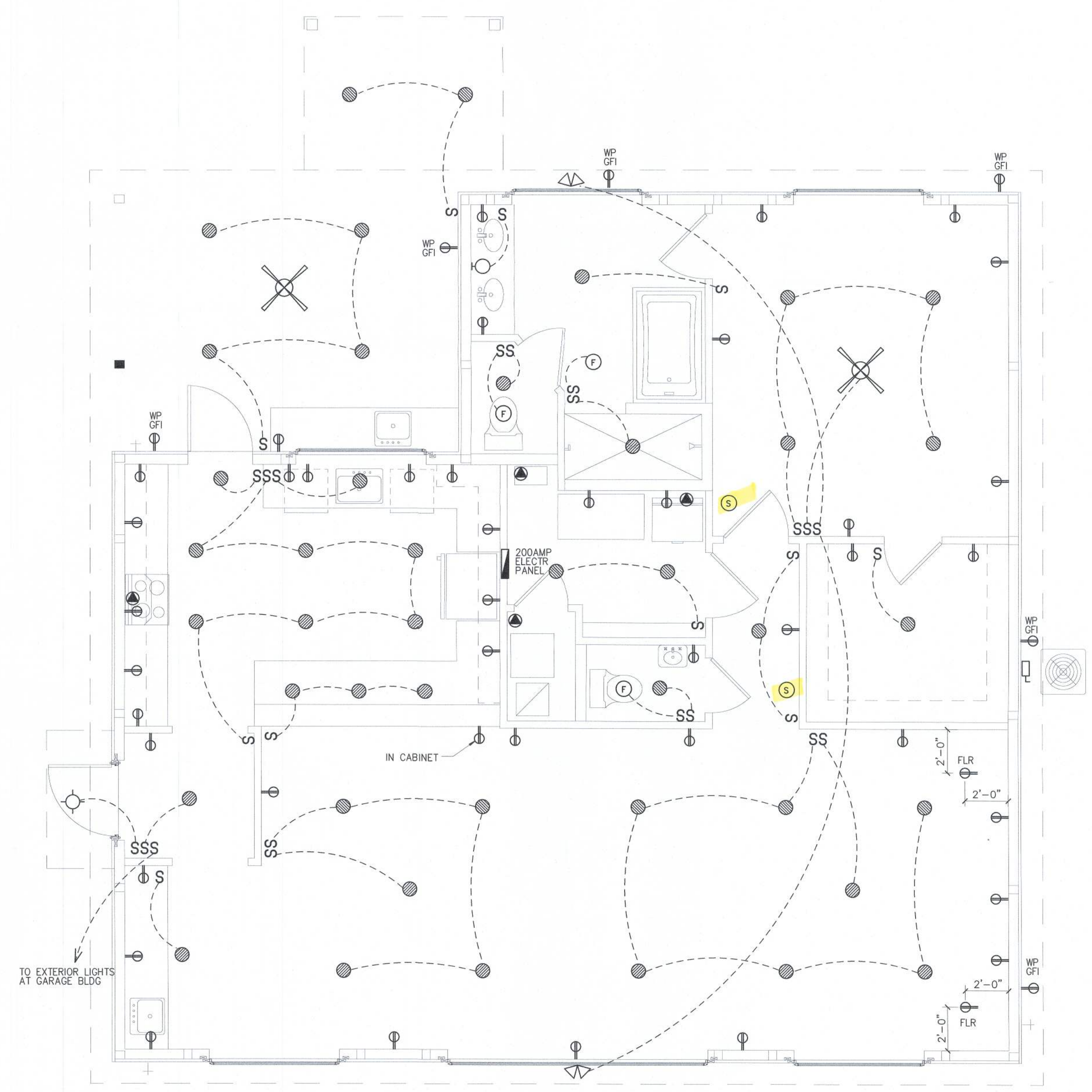


TYPICAL INTERIOR
NON-BEARING WALL SECTION 1
3/4"=1'-0" A1.0

- NOTES:**
1. ALL WORK SHALL COMPLY WITH FBC 2017 RESIDENTIAL 5TH EDITION CHAPTERS 34 THROUGH 43.
 2. PROVIDE GFCI OUTLETS AT KITCHEN, BATHROOMS, GARAGE AND SHOP.
 3. OUTLETS SUPPLYING POWER TO THE STOVE, RANGE, A/C, WATER HEATER, DRYER, AND DISH WASHER TO BE WIRED ON SEPARATE DEDICATED CIRCUITS.
 4. CONNECT ALL SMOKE DETECTORS TOGETHER.
 5. ALL BEDROOM OUTLETS TO BE ARC FAULT OUTLETS - AFCI.
 6. ALL OUTLETS TO BE TAMPER PROOF.

ELECTRICAL LEGEND

- | | |
|--|--|
| DUPLEX RECEPTACLE OUTLET
NUMBER DESIGNATES HEIGHT ABOVE FLR.
OUTLETS AT 18" UNLESS OTHERWISE NOTED | HEATER - CEILING MOUNTED |
| QUAD RECEPTACLE OUTLET | SCONCE LIGHT FIXTURE |
| 220V OUTLET | EXTERIOR FLOOD LIGHT
WITH MOTION DETECTOR
AND PHOTOCELL |
| SPECIAL PURPOSE OUTLET
3 PHASE CONNECTION | WALL MOUNTED EXHAUST FAN |
| PHONE OUTLET | 2X4 FLOURESCENT FIXTURE |
| DUPLEX PHONE/DATA OUTLET | LED LIGHT STRIP |
| CABLE TELEVISION OUTLET | LIGHT FIXTURE SURFACE MOUNT |
| EXHAUST FAN | LIGHT FIXTURE - WALL MOUNTED |
| SMOKE DETECTOR | WALL MOUNTED RETRACTABLE
LIGHT FIXTURE |
| SWITCH WITH DIMMER | CEILING FAN/
LIGHT COMBINATION
WITH 3' EXTENSION
FROM CEILING |
| ELECTRICAL PANEL | CEILING FAN |
| ELECTRICAL METER | TRACK LIGHTING |
| ELECTRICAL DISCONNECT | SPEAKER |
| ILLUMINATED EXIT SIGN
WITH BATTERY BACKUP | AFCI ARC FAULT CIRCUIT INTERRUPTER |
| EMERGENCY LIGHT WITH
BATTERY BACKUP | GFI GROUND FAULT INTERRUPTER |
| COMBINATION ILLUMINATED EXIT
SIGN AND EMERGENCY LIGHT
WITH BATTERY BACKUP | WP WATER PROOF |
| RECESSED LIGHT FIXTURE | ALL PHONE AND DATA OUTLETS SHALL
BE IN CONDUIT. |
| CEILING MOUNTED LIGHT FIXTURE | |
| EXTERIOR GROUND W.P. LIGHT FIXTURE | |
| CEILING MOUNTED LIGHT FIXTURE/
EXHAUST FAN COMBINATION | |
| PENDANT LIGHT FIXTURE | |



ELECTRICAL PLAN
1/4"=1'-0"

Joshua Shatkin
4/21/2021

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