

GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.
2. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE COMPLETE DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO BRACING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT AND BRACING FOR CRANES AND GIN POLES.
3. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE STRUCTURAL ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
4. CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON ROOFS IN EXCESS OF THE POSTED ROOF LIVE LOAD. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO INSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON ROOFS.
5. PLANS, SECTIONS AND DETAILS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
6. SUBMIT WRITTEN REQUESTS TO THE ENGINEER FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, SPlicing, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.
7. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK FROM THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
8. PERIODIC SITE OBSERVATION, IF PROVIDED, BY FIELD REPRESENTATIVES OF NORTH FLORIDA PROFESSIONAL SERVICES, INC. IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHOULD NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO GUARD THE OWNER AGAINST DEFECTS OR DEFICIENCIES IN THE WORK OF THE CONTRACTOR.

DESIGN CRITERIA

1. THE DESIGN IS BASED ON, AND ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (FBC) WITH AMENDMENTS AND DESIGN CODES REFERENCED WITHIN THESE DOCUMENTS. USE THE REFERENCED EDITIONS FROM THE FBC CHAPTER 35 OR THE LATEST EDITIONS IF NOT REFERENCED.
- AMERICAN SOCIETY OF CIVIL ENGINEERS, ASCE 7-22
MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES"
STRUCTURAL CONCRETE.
"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
THE AMERICAN CONCRETE INSTITUTE (ACI 318-19 AND ACI 350-06)
MASONRY:
"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
THE MASONRY SOCIETY (TMS 402/602-16)
STRUCTURAL STEEL:
STEEL CONSTRUCTION MANUAL - FIFTEENTH EDITION BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC 360-16)
WOOD:
"NATIONAL DESIGN SPECIFICATION" AND SUPPLEMENT (ANS/AWC NDS-18)
2. LIVE LOADS:
ROOF 20 PSF (REDUCIBLE BY CODE)
3. SUPERIMPOSED DEAD LOADS:
ROOFING / CEILING / M.E.P. 10 PSF
4. WIND LOAD DESIGN DATA:
WIND LOADS SHALL BE IN ACCORDANCE WITH THE 2023 FLORIDA BUILDING CODE (REFERENCING ASCE 7-22).
MAIN WIND FORCE RESISTING SYSTEM:
WIND DESIGN DATA:
a. ULTIMATE DESIGN WIND SPEED, 3 SECOND GUSTS, VULT. 125 MPH
b. HURRICANE PRONE REGION YES
c. WINDBORNE DEBRIS REGION NO
d. BUILDING RISK CATEGORY II
e. WIND EXPOSURE CATEGORY C
f. WIND TOPOGRAPHIC FACTOR (Kzt) 1.0
g. ENCLOSURE CATEGORY PARTIALLY OPEN
h. INTERNAL PRESSURE COEFFICIENT +0.18
i. MEAN ROOF HEIGHT 15 FEET
j. WIND DIRECTIONALLY FACTOR, KD 0.85
k. VELOCITY PRESSURE COEFFICIENT (KH) 0.85
l. ULTIMATE VELOCITY PRESSURE (QHULTI) 34.0 PSF
m. COMPONENT & CLADDING WIND PRESSURES SEE TABLE THIS SHEET
n. DIMENSION "s" 5'-0"
o. GROUND ELEVATION FACTOR, KE 1.0
5. 60-MINUTE RAINFALL INTENSITY 4.5 INCHES PER HOUR.
6. DISTRIBUTE THE MAXIMUM LOAD HUNG FROM ANY STRUCTURAL MEMBERS FOR MEP DUCTWORK, PIPING ETC OVER THE MEMBER'S TRIBUTARY AREA IN A WAY THAT THE DESIGN SUPERIMPOSED DEAD LOADS LISTED IN CONTRACT DOCUMENTS ARE NOT EXCEEDED. THE CONTRACTOR SHALL COORDINATE THE LOADS OF ALL TRADES AND PROVIDE ADDITIONAL SUPPORT OR DISTRIBUTION FRAMING AS REQUIRED TO ACHIEVE THE ALLOWABLE LOAD DISTRIBUTION.
7. STRUCTURAL COMPONENTS ARE NOT DESIGNED FOR VIBRATING EQUIPMENT. MOUNT VIBRATING EQUIPMENT ON VIBRATOR ISOLATORS.

POST-INSTALLED ANCHORS

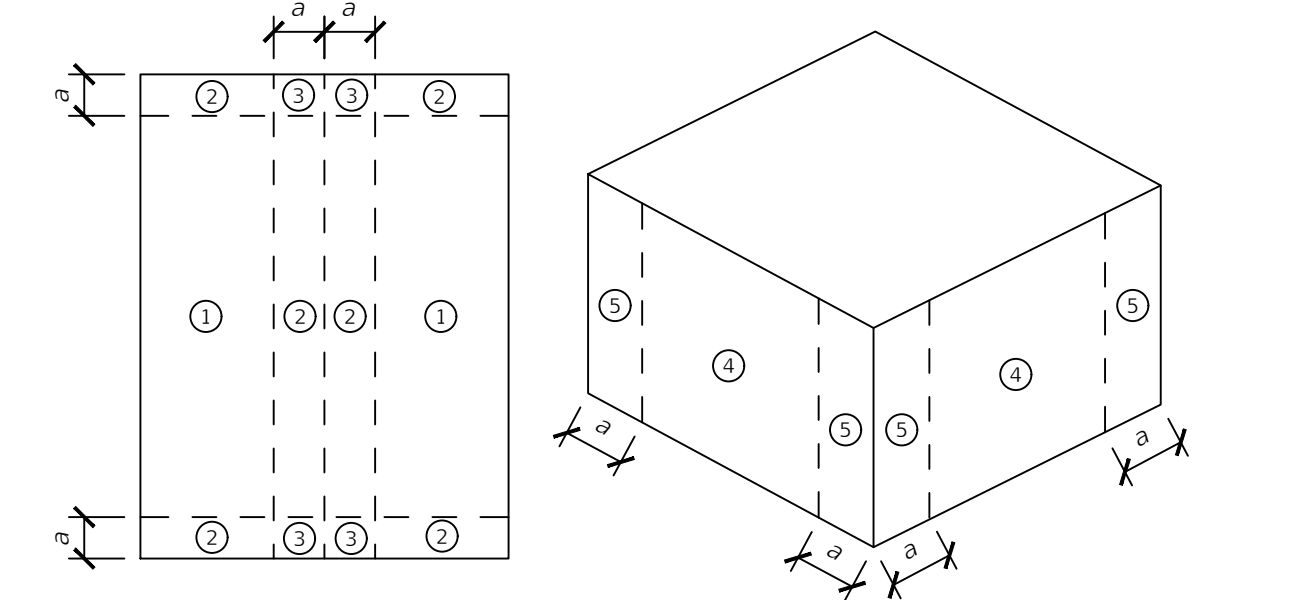
1. ANCHOR PRODUCTS APPROVED FOR USE ON THIS PROJECT ARE LISTED BELOW UNLESS OTHERWISE SPECIFIED IN SECTIONS/DETAILS.
- 1.a. ADHESIVE ANCHORS INTO CONCRETE SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED CONCRETE:
- 1.a.1. USE THE FOLLOWING (UNO):
- 1.a.1.a. HILTI HIT-HY 200 ADHESIVE (ICC-ES ESR 3187)
- 1.a.1.b. HILTI HIT-RE 500-SD ADHESIVE (ICC-ES ESR2322)
- 1.a.1.c. EPOCON "G5" ADHESIVE (ICC-ES ESR1137)
- 1.a.1.d. SIMPSON STRONG-TIE "SET-XP" ADHESIVE (ICC-ES ESR2508)
- 1.a.1.e. SIMPSON STRONG-TIE "AT-XP" ADHESIVE (IAPMO-ES ER263)
- 1.a.1.f. EPOCON "57" ADHESIVE (ICC-ES ESR2308)
2. INSTALL ANCHORS TO MEET THE REQUIREMENTS INDICATED IN THE CONTRACT DOCUMENTS AND THE MANUFACTURER'S RECOMMENDATIONS.
3. LOCATE, BY NON-DESTRUCTIVE MEANS, AND AVOID ALL EXISTING REINFORCEMENT PRIOR TO INSTALLATION OF ANCHORS. IF EXISTING REINFORCING LAYOUT PROHIBITS THE INSTALLATION OF ANCHORS AS INDICATED IN THE DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE DESIGN PROFESSIONALS IMMEDIATELY.
4. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DRAWINGS. CONTRACTOR SHALL OBTAIN APPROVAL FROM STRUCTURAL ENGINEER OF RECORD (SER) PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
5. ANCHOR INSTALLER SHALL BE TRAINED BY THE MANUFACTURER ON PROPER INSTALLATION METHODS.
6. CARE SHALL BE EXERCISED TO AVOID CONFLICTS WITH EXISTING REINFORCING WHEN DRILLING HOLES. PILOT HOLES SHALL BE INSTALLED AS REQUIRED. HOLES SHALL BE DRILLED AND CLEANED PER THE MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTALLATION INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACINGS INDICATED IN THE MANUFACTURER'S LITERATURE OR ON THE STRUCTURAL DRAWINGS. EMBEDMENT SHALL BE THE MINIMUM SPECIFIED ON THE STRUCTURAL DRAWINGS.

WOOD

1. STRUCTURAL FRAMING PLANS DEPICT THE PRIMARY STRUCTURAL FRAMING SYSTEM. CONTRACTOR SHALL PROVIDE SECONDARY AND MISCELLANEOUS FRAMING AS REQUIRED TO COMPLETE THE PROJECT (SEE ARCHITECTURAL DRAWINGS).
2. DRESSED SEASONED LUMBER: S4S, 19% MAXIMUM MOISTURE CONTENT AT TIME OF DRESSING.
- 2.1. COLUMNS AND STUD FRAMING: SOUTHERN PINE NO.2 OR STRONGER.
- 2.2. LINTELS, FLOOR JOISTS AND BEAMS: SOUTHERN PINE, NO.2 GRADE
- 2.3. WOOD IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO WEATHER, ABOVE GRADE PRESSURE-TREATED (AWPA/UC4A OR UC3B) OR GROUND CONTACT RATED PRESSURE TREATED (AWPA/UC4A). GROUND CONTACT RATED WOOD IS RECOMMENDED AT THE CRAWLSPACE AND DECK AREAS (IF PRESENT). USE HOT-DIP GALVANIZED NAILS IN PRESSURE TREATED WOOD.
3. STRUCTURAL PANELS
- 3.1. WALL PANELS: 1/2" APA RATED SHEATHING.
- 3.2. ROOF PANELS: 1/2" APA RATED SHEATHING.
4. WOOD SHEAR WALLS
- 4.1. PANELS SHALL BE ORIENTED WITH THE LONG DIMENSION IN THE VERTICAL DIRECTION.
- 4.2. SOLID 2x BLOCKING SHALL BE PROVIDED AT UNSUPPORTED, HORIZONTAL PANEL EDGES.
- 4.3. NAIL PANELS WITH 8d HOT-DIPPED GALVANIZED RINGSHANK NAILS SPACED AT 6" AT THE PERIMETER OF THE PANELS AND AT 6" AT INTERMEDIATE SUPPORTS, UNO.
- 4.4. DOUBLE 2x FRAMING STUDS SHALL BE USED AT THE ENDS OF EACH SHEAR WALL, UNO.
- 4.5. CONNECTIONS FOR STRUCTURAL TIMBER: GALVANIZED STRONG-TIE CONNECTORS BY THE SIMPSON STRONG TIE COMPANY OR APPROVED EQUAL.
5. LAMINATED VENEER LUMBER (LVL) SHALL BE WEYERHAEUSER/TRUS JOIST MICROLAM LVL (OR EQUAL) WITH FB NOT LESS THAN 2,600 PSI AND MINIMUM 2.0E.
6. BOLTED CONNECTIONS SHALL CONSIST OF ASTM A307 BOLTS, FASTENED TO A SNUG-TIGHT CONDITION.

WIND PRESSURE DIAGRAM

1. DESIGN WIND PRESSURES TO BE USED IN THE DESIGN OF ALL COMPONENTS AND CLADDING ELEMENTS. PRESSURES INDICATED IN TABLE ARE SERVICE LOADS, MULTIPLY TABULATED VALUES BY 0.6 FOR ALLOWABLE STRESS DESIGN (ASD) AND BY 1.0 FOR LOAD AND RESISTANCE FACTOR DESIGN (LRFD).
2. REFER TO WIND PRESSURE DIAGRAM FOR ZONE LOCATIONS AND EXTENTS.
3. POSITIVE PRESSURES ACT TOWARD COMPONENT SURFACES AND NEGATIVE PRESSURES ACT AWAY FROM COMPONENT SURFACES.



COMPONENTS AND CLADDING WIND PRESSURES ON ROOF AND WALLS (PSF)								
ZONE	1, 2, 3	1	2	3	4	5		
TRIB AREA	(+)	(-)	(-)	(-)	(+)	(-)	(+)	(-)
10	23	-49	-77	-92	34	-37	34	-46
20	21	-44	-67	-79	33	-35	33	-43
50	18	-38	-51	-61	30	-33	30	-38
100	16	-33	-40	-46	29	-32	29	-35
200	14	-28	-40	-46	27	-30	27	-32
500	14	-28	-40	-46	25	-28	25	-28

WINDOWS, DOORS, AND ROOFING

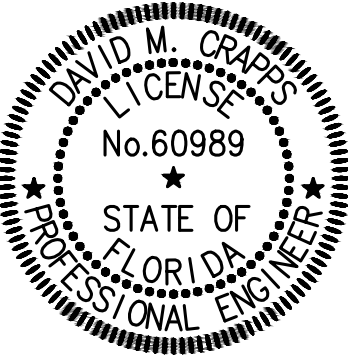
1. FOR THE SELECTION OF WINDOW, DOOR, AND ROOFING PRODUCTS, TABULATED VALUES ARE NORMALLY MULTIPLIED BY 0.6 PRIOR TO COMPARISON WITH THE POSITIVE AND NEGATIVE PRESSURE RATINGS PROVIDED IN EACH FLORIDA PRODUCT APPROVAL. IT IS RECOMMENDED THAT THE MANUFACTURER'S REPRESENTATIVE REVIEW THESE DRAWINGS FOR VERIFICATION. THE TRIBUTARY AREA FOR ROOFING PRODUCTS IS TYPICALLY BASED ON 10 SQUARE FEET, AND FOR DOORS AND WINDOWS IT IS BASED ON THE SURFACE AREA OF THE WALL OPENINGS.

ROOF OVERHANG PRESSURES
(WHERE NOT TABULATED ABOVE)

1. ROOF OVERHANG PRESSURES ARE DETERMINED BY SUMMING THE ABSOLUTE VALUE OF THE NEGATIVE ROOF ZONE (1, 2, OR 3) PRESSURE AND THE POSITIVE WALL ZONE (4 OR 5) PRESSURE, BASED ON THE APPLICABLE TRIBUTARY AREA. THE SUM IS THEN MULTIPLIED BY -1 TO GET THE CORRESPONDING ROOF OVERHANG UPLIFT PRESSURE.


SYMBOLS AND ABBREV.

ALT	ALTERNATE/ALTERNATIVE	K	KIP = 1000 LB
ACI	AMERICAN CONCRETE INSTITUTE	KO	KNOCK OUT
AFF	ABOVE FINISHED FLOOR		
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LGTH	LENGTH
AISI	AMERICAN IRON AND STEEL INSTITUTE	LH	LONG LEG HORIZONTAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	LLV	LONG LEG VERTICAL
AWS	AMERICAN WELDING SOCIETY	LONG	LONGITUDINAL
AB	ANCHOR BOLTS		
ARCH	ARCHITECTURE/ARCHITECTURAL	MANUF	MANUFACTURE/MANUFACTURER
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS	M B	MASONRY BEAM
AWs	AMERICAN WELDING SOCIETY	MATL	MATERIAL
		MAX	MAXIMUM
BB	BOND BEAM	MECH	MECHANICAL
BFB	BOTTOM FLANGE BRACE	MEZZ	MEZZANINE
BP	BASE PLATE/BEARING PLATE	MIN	MINIMUM
BRG	BEARING	MISC	MISCELLANEOUS
BM	BEAM	MO	MASONRY OPENING
BLK	BLOCK	MTL	METAL
B/	BOTTOM OF		
BLDG	BUILDING	N I C	NOT IN CONTRACT
		NOM	NOMINAL
CANT	CANTILEVER	N T S	NOT TO SCALE
CL	CENTERLINE	N W T	NORMAL WEIGH TOPPING
CLR	CLEAR/CLEARANCE		
COL	COLUMN	O C	ON CENTER
CB	CONCRETE BEAM	OPNG	OPENING
CC	CONCRETE COLUMN	OPP	OPPOSITE
CMU	CONCRETE MASONRY UNIT		
CONT	CONTINUOUS	PAF	POWER ACTUATED FASTENER
CONV	CONNECTION	PL	PLATE
CONST	CONSTRUCTION	PLY	PLYWOOD
CS J	CONSTRUCTION JOINT	PSF	POUNDS PER SQUARE FOOT
	CONTRACTION JOINT / CONTROL JOINT	PSI	POUNDS PER SQUARE INCH
		PC	PRECAST CONCRETE
DET	DETAIL	PRE-ENG	PRE-ENGINEERED
DEPT	DEPARTMENT	PREFAB	PREFABRICATED
DBA	DEFORMED BAR ANCHOR	PROJ	PROJECTION
DFT	DRY FILM THICKNESS	PT	PRESSURE TREATED
DIA	DIAMETER	PW	PANEL WIDTH
DIM	DIMENSION		
DIST	DISTANCE	REF	REFERENCE
DN	DOWN	REINF	REINFORCING
DR	DRAIN	R C P	REINFORCED CONCRETE PIPE
DWG	DRAWING	REQD	REQUIRED
		R W	RETAINING WALL
EA	EACH	R D	ROOF DRAIN
EE	EACH END		
EF	EACH FACE	SCHED	SCHEDULE
EL	ELEVATION	SIM	SIMILAR
EL	ELEVATION	SPC	SPACES/SPACES
EMB	EMBEDMENT	SPECS	SPECIFICATIONS
ENGR	ENGINEER	SPP	SPRUCE PINE FUR
EOS	EDGE OF SLAB	SQ	SQUARE
EQ	EQUAL	S A	STUD ANCHOR
ES	EACH SIDE	S S	STAINLESS STEEL
EW	EACH WAY	STD	STANDARD
EXIST	EXISTING	STL	STEEL
EXP	EXPANSION	STRUC	STRUCTURAL
EXT	EXTERIOR	SYM	SYMMETRICAL
		SF	STEPPED FOOTING
FBC	FLORIDA BUILDING CODE	SYP	SOUTHERN YELLOW PINE
FL	FULL LENGTH WELD, WELD ENTIRE DIST.		
F V	FIELD VERIFY	THK	THICK
F F	FINISHED FLOOR	THD	THREAD/THREADED
FLR	FLOOR	TB	TIE BEAM
F D	FLOOR DRAIN	T & B	TOP AND BOTTOM
FTG	FOOTING	T & G	TONGUE AND GROOVE
		T O C	TOP OF CONCRETE
GA	GAGE/GAUGE	T O S	TOP OF STEEL
GALV	GALVANIZED	TRANS	TRANSVERSE
GC	GENERAL CONTRACTOR	TYP	TYPICAL
GLU-LAM	GLUE LAMINATED	T/	TOP OF
		UNO	UNLESS NOTED OTHERWISE
HAS	HEADED ANCHOR STUD		
HC	HOLLOW CORE	VERT	VERTICAL
HK	HOOK	VOL	VOLUME
HORIZ	HORIZONTAL		
HP	HIGH POINT	WF	WALL FOOTING
HSS	HOLLOW STRUCTURAL SECTION	WPF	WATERPROOF
HT	HEIGHT	WWF	WELDED WIRE FABRIC
ID	INSIDE DIAMETER	WH	WEEP HOLE
IF	INSIDE FACE	WT	WEIGHT
INT	INTERIOR	WF	WIDE FLANGE
		W /	WITH
JT	JOINT	W / O	WITHOUT
JST	JOIST	WD	WOOD
		WP	WORKING POINT



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REVISIONS			NORTH FLORIDA PROFESSIONAL SERVICES, INC. P.O. BOX 3823 LAKE CITY, FL 32056 PH. 386-752-4675 LIC NO. LB8356	2551 BLAIRSTONE PINES DR. TALLAHASSEE, FL 32301 WWW.NFPS.NET CA# 29011	JOB NUMBER: L250421KAY EOR: DAVID M. CRAPPS P.E. NO.: 60989	STRUCTURAL GENERAL NOTES MORALES RESIDENCE 412 SW BEAR LN, FORT WHITE COLUMBIA COUNTY, FLORIDA 32038	SHEET NO. S1
DATE	DESCRIPTION						

PLAN NOTES

1. SPIKE MULTIPLE PLY BEAMS TOGETHER WITH 2 ROWS OF 10d HOT DIPPED GALV. COMMON NAILS @12"O.C. STAGGERED, PER PLY.
2. ROOF SHEATHING SHALL BE 1/2" APA RATED PLYWOOD OR OSB WITH 8d x 2-1/2" LONG HOT DIPPED GALV. RINGSHANKED NAILS @6" O.C. AT PANEL EDGES AND @6" O.C. IN FIELD OF PANEL. PROVIDE 1/8" GAP BETWEEN ADJACENT PANELS AND/OR USE SIMPSON PSL CLIPS.
3. WALL SHEATHING SHALL BE 1/2" APA RATED PLYWOOD OR OSB WITH 8d HOT DIPPED GALV x 2-1/2" LONG RINGSHANK NAILS @6" O.C. AT PANEL EDGES AND @6" O.C. IN FIELD OF PANEL. PROVIDE 2x SOLID BLOCKING AT HORIZONTAL PANEL EDGES.
4. AT STUD PACKS, SPIKE STUD PLIES TOGETHER WITH 10d HOT DIPPED GALV COMMON NAILS @6" O.C. STAGGERED, PER PLY.

CONTRACTOR TO FIELD VERIFY
CONDITION OF ROOF TRUSSES
DURING THE REPLACEMENT OF
THE EXISTING ATTIC INSULATION.
NOTIFY THE ENGINEER OF ANY
OBSERVED TERMITE OR
MOISTURE-RELATED DAMAGE

TRUSS DIRECTION:
PERPENDICULAR TO WALL
AREAS WITH REPAIRS AND
WINDOW OPENING ADDITION

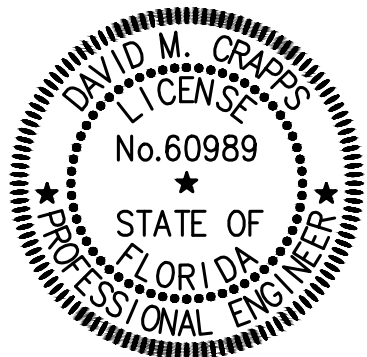
TYPICAL TRUSS SHORING
LOCATION AT EAVE. PROVIDE
TEMPORARY SHORING WITH
2,500 LB MINIMUM CAPACITY
UNDER EACH TRUSS.

BEDROOM FRAMING REPAIR AREA

KITCHEN FRAMING
REPAIR AREA

LOCATION OF NEW 42"x12"
WINDOW OPENING FOR BATHROOM

1 STRUCTURAL REPAIR PLAN
SCALE: 1/4" = 1'-0"



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EOR:
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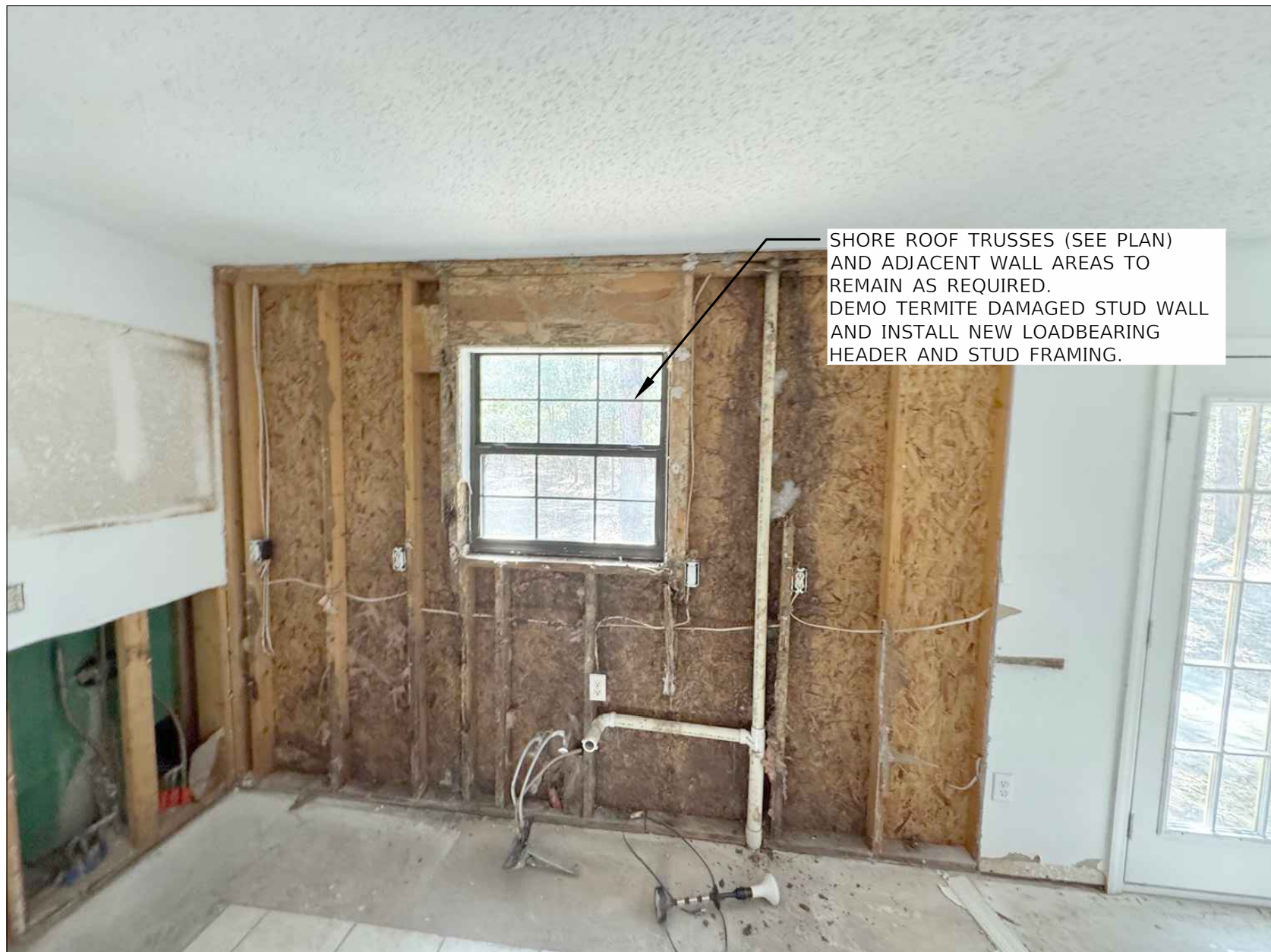
STRUCTURAL REPAIR PLAN
MORALES RESIDENCE
412 SW BEAR LN, FORT WHITE
COLUMBIA COUNTY, FLORIDA 32038

SHEET
NO.

S2



A BATHROOM REPAIR PHOTO DETAIL (INTERIOR)
SCALE: N.T.S.



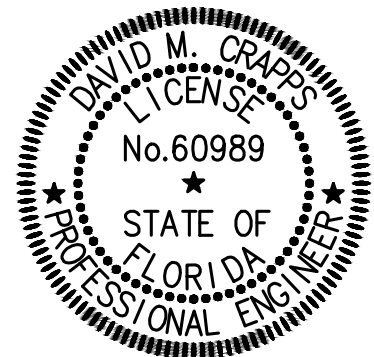
C KITCHEN AREA REPAIR PHOTO DETAIL (INTERIOR)
SCALE: N.T.S.



B BATHROOM REPAIR PHOTO DETAIL (EXTERIOR)
SCALE: N.T.S.



D BEDROOM REPAIR PHOTO DETAIL (INTERIOR)
SCALE: N.T.S.



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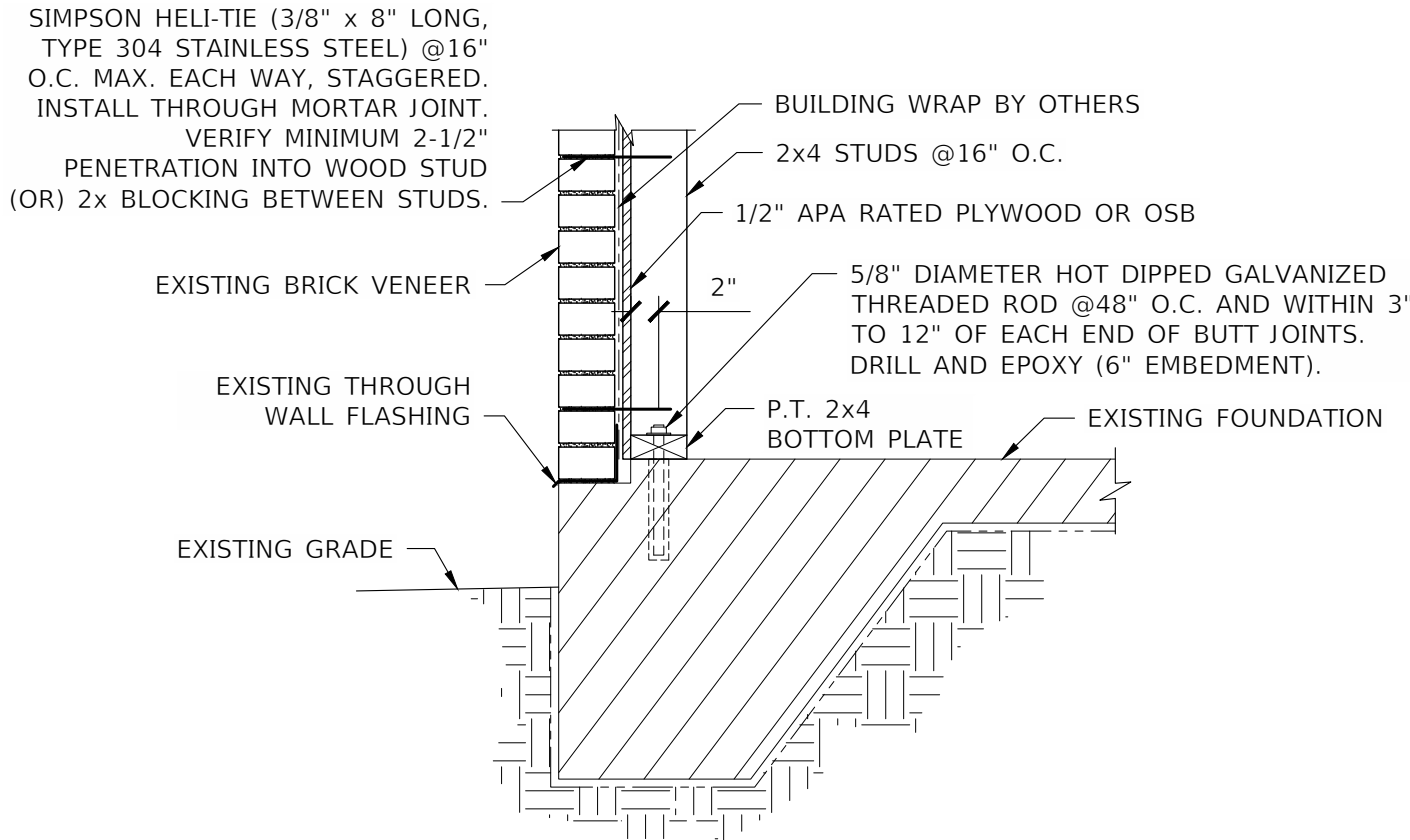
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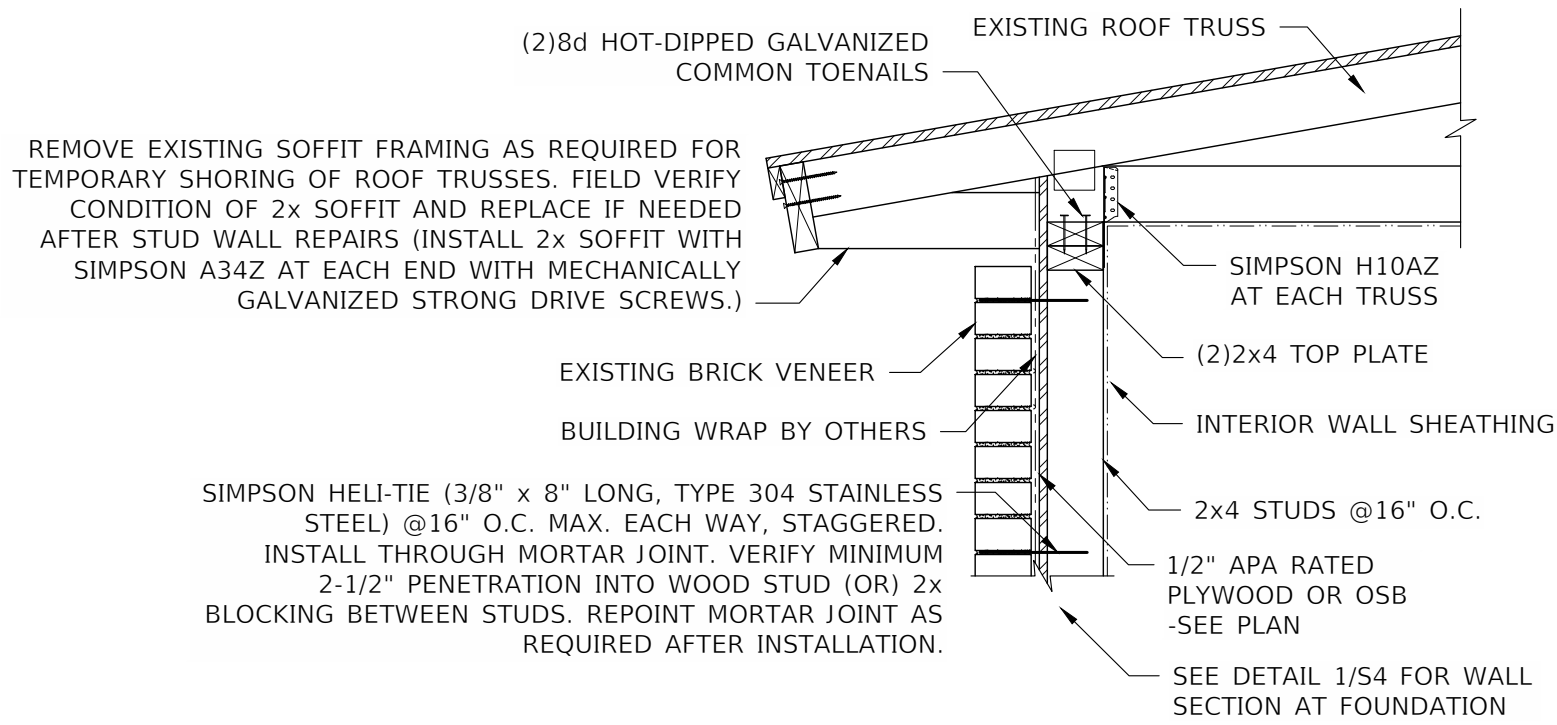
STRUCTURAL REPAIR PHOTOS
MORALES RESIDENCE
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COLUMBIA COUNTY, FLORIDA 32038

SHEET
NO.

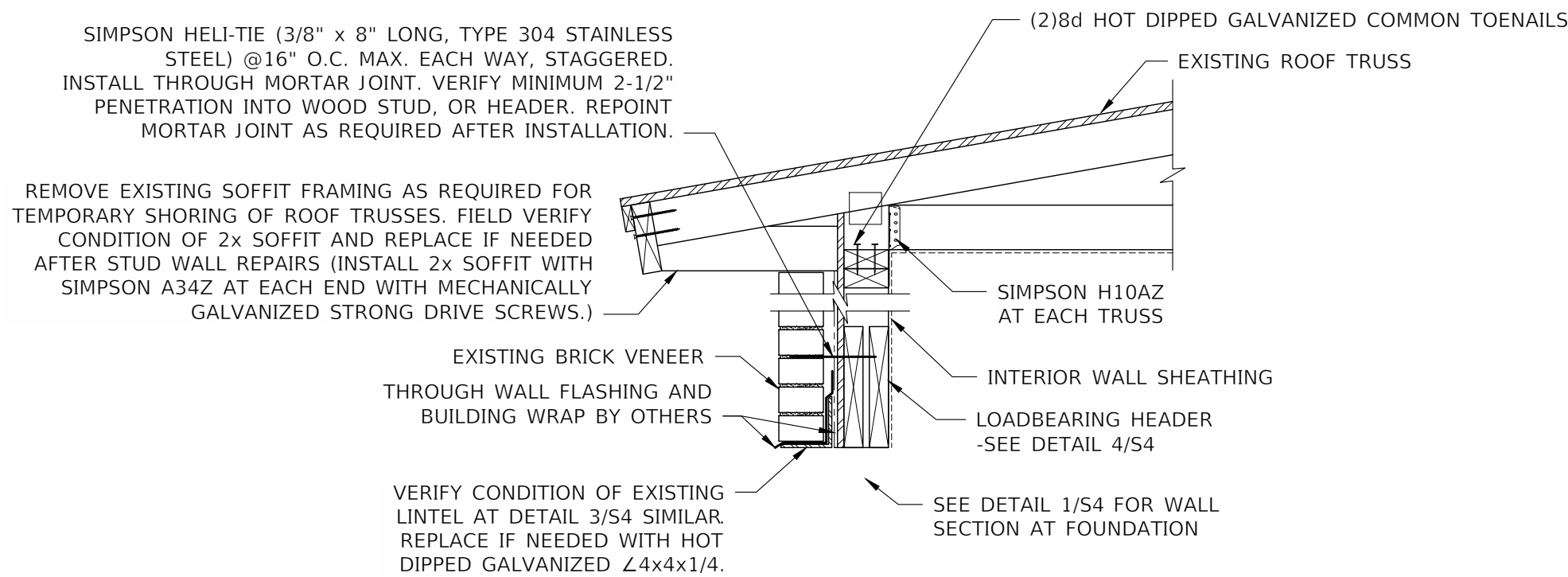
S3



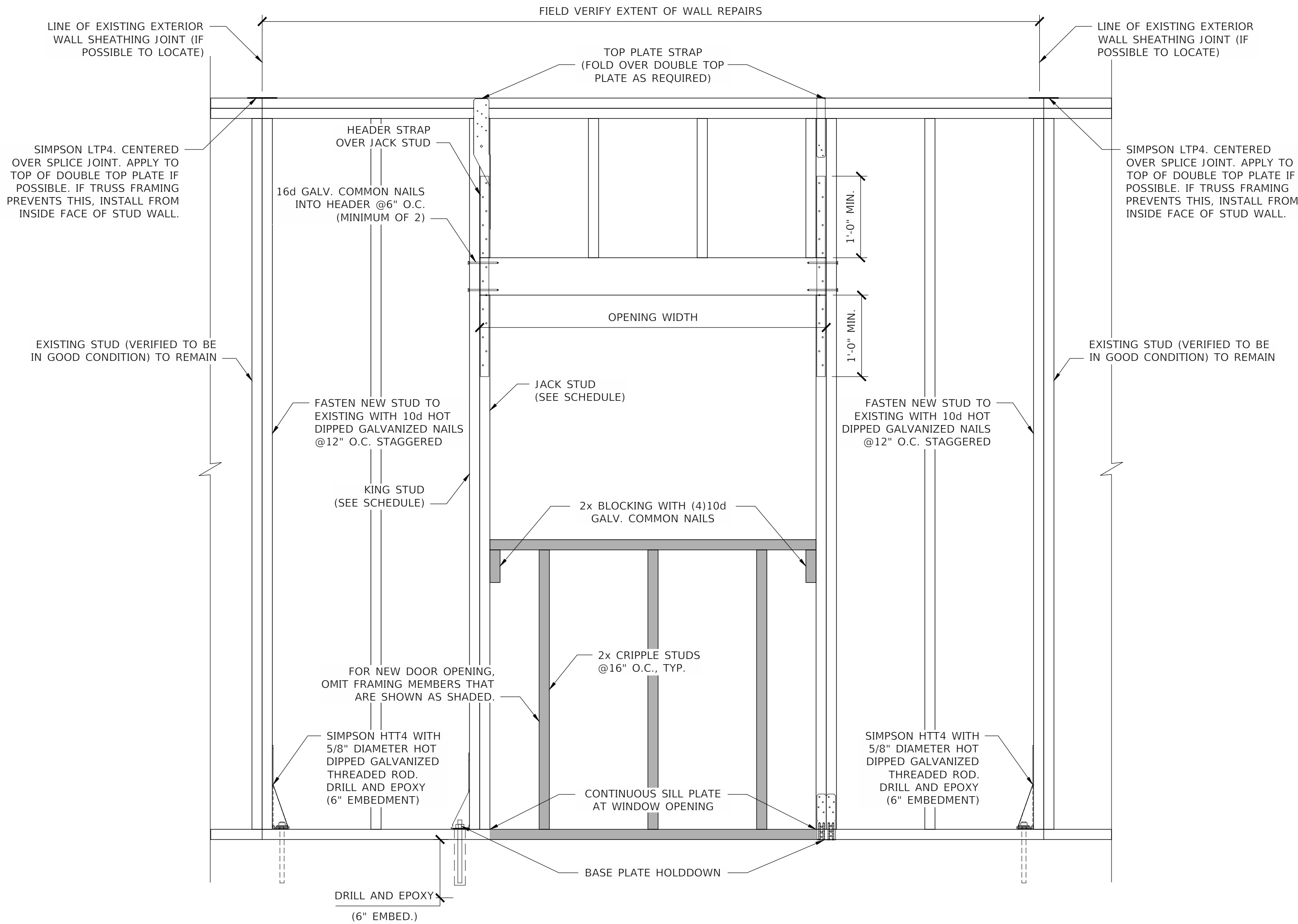
1 FOUNDATION SECTION
SCALE: 1" = 1'-0"



2 SECTION AT WALL
SCALE: 1" = 1'-0"



3 BATHROOM WINDOW HEADER / LINTEL DETAIL
SCALE: 1" = 1'-0"



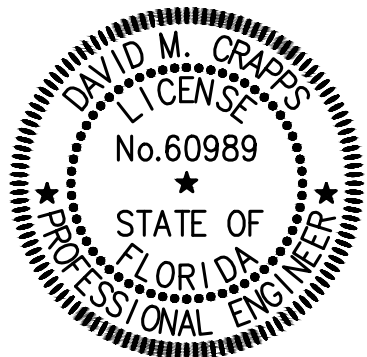
4 TYPICAL EXTERIOR WALL OPENING DETAIL
SCALE: N.T.S.

NOTES

- ALL LUMBER SHALL BE SOUTHERN YELLOW PINE NO.2 OR BETTER.
- USE PLYWOOD SPACERS BETWEEN HEADER PLIES AS REQUIRED TO MATCH STUD DEPTH.
- NAIL STUD PACKS TOGETHER WITH 10d HOT DIPPED GALVANIZED COMMON NAILS @6" O.C. STAGGERED.
- INTERIOR NON-LOAD BEARING WALL HEADERS SHALL NOT BE LESS THAN CODE MINIMUMS.
- USE 8d HOT DIPPED GALVANIZED COMMON NAILS ON SIMPSON H6, FULLY NAILED.
- USE 10d HOT DIPPED GALVANIZED COMMON NAILS ON SIMPSON LSTA36, FULLY NAILED.


HEADER SCHEDULE

MAXIMUM OPENING WIDTH	HEADER SIZE	JACK STUDS	KING STUDS	SIMPSON STRONG-TIE CONNECTORS		
				TOP PLATE	HEADER	HOLDDOWN
3'-0"	(2) 2x6	(1) 2x4	(1) 2x4	H6 (OR) SP4	LSTA36	DSPZ
5'-0"	(2) 2x8	(1) 2x4	(2) 2x4	H6 (OR) SPH4	LSTA36	DTTZ



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