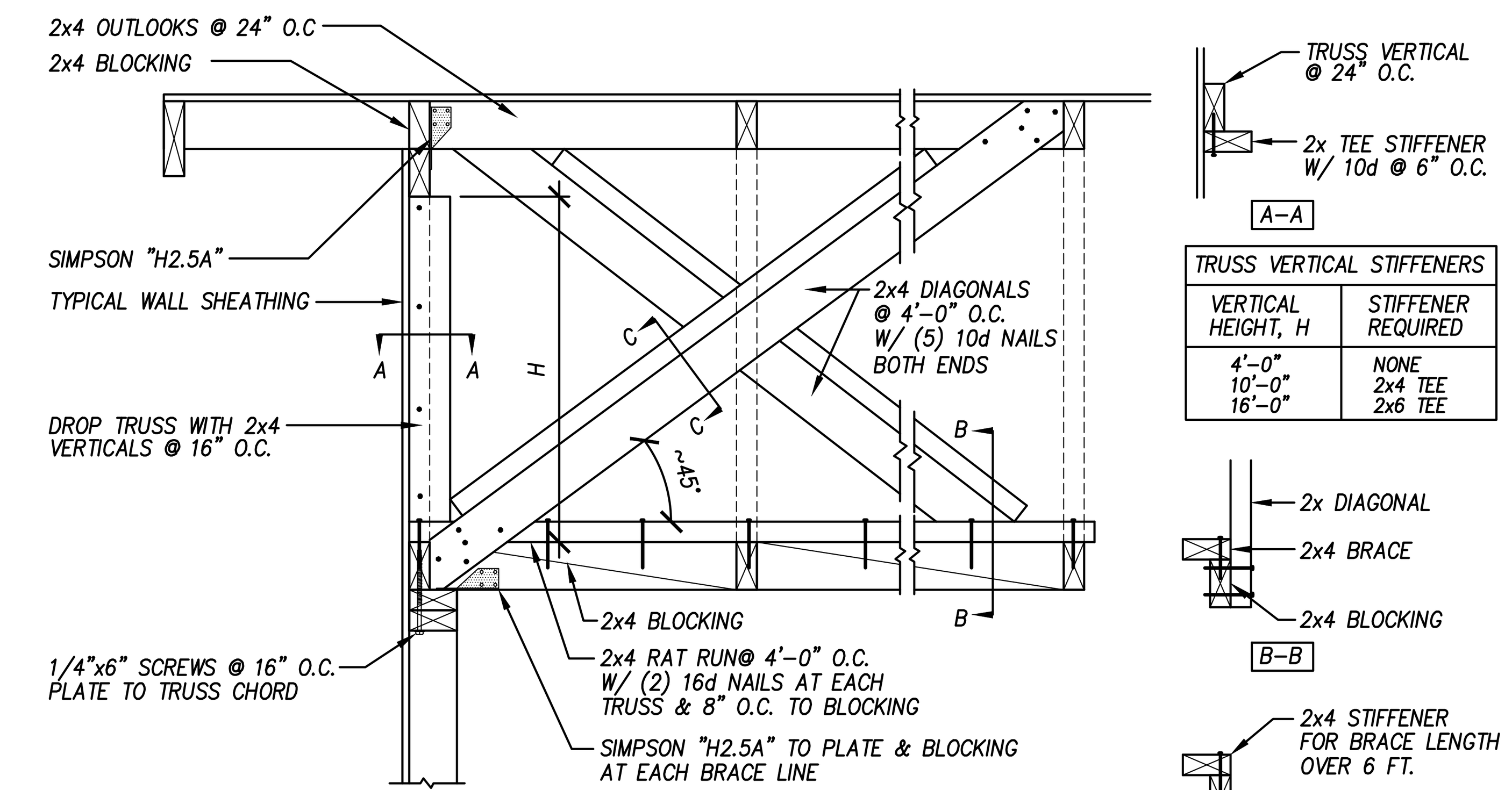
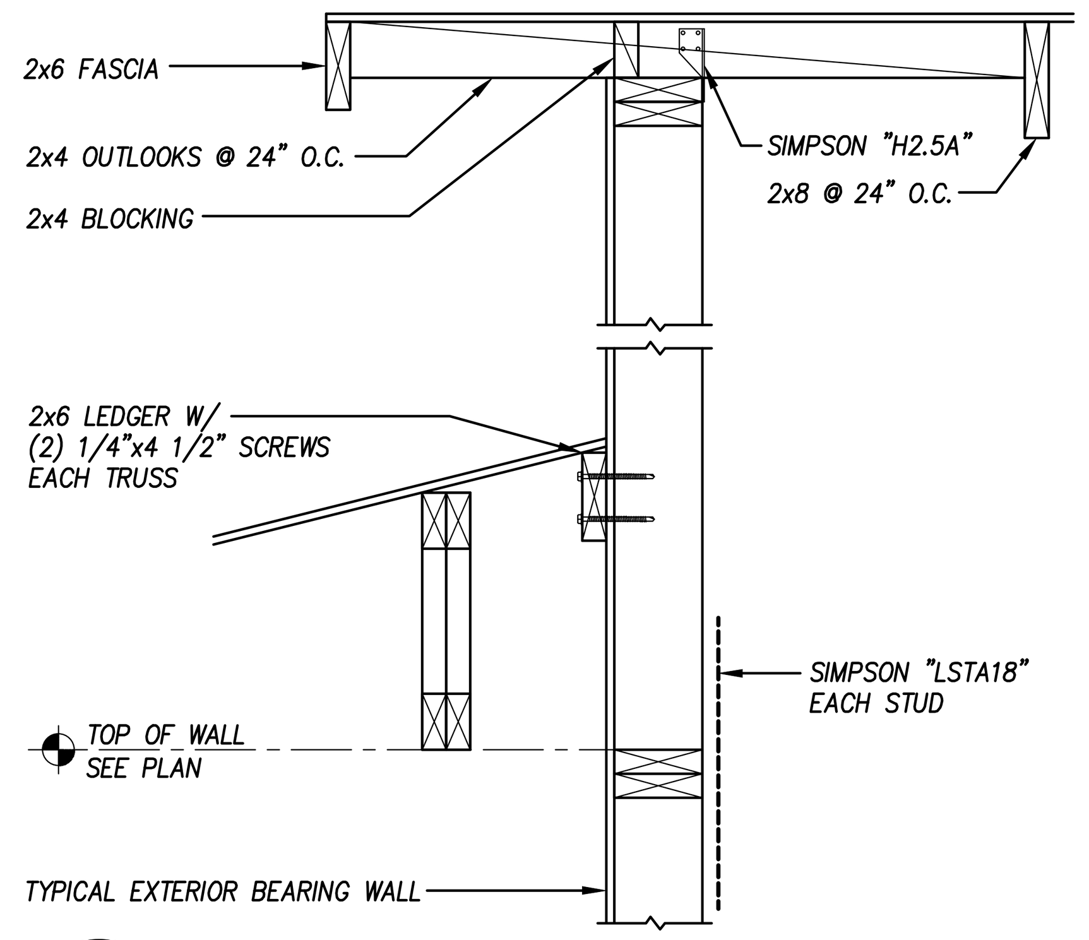
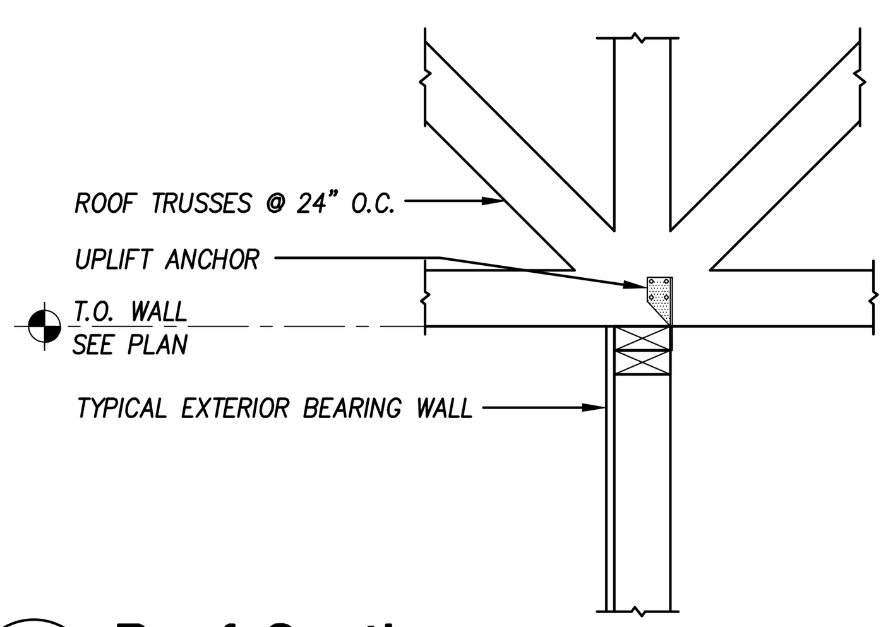


TRUSS VERTICAL STIFFENERS	
VERTICAL HEIGHT, H	STIFFENER REQUIRED
4'-0"	NONE
10'-0"	2x4 TEE
16'-0"	2x6 TEE

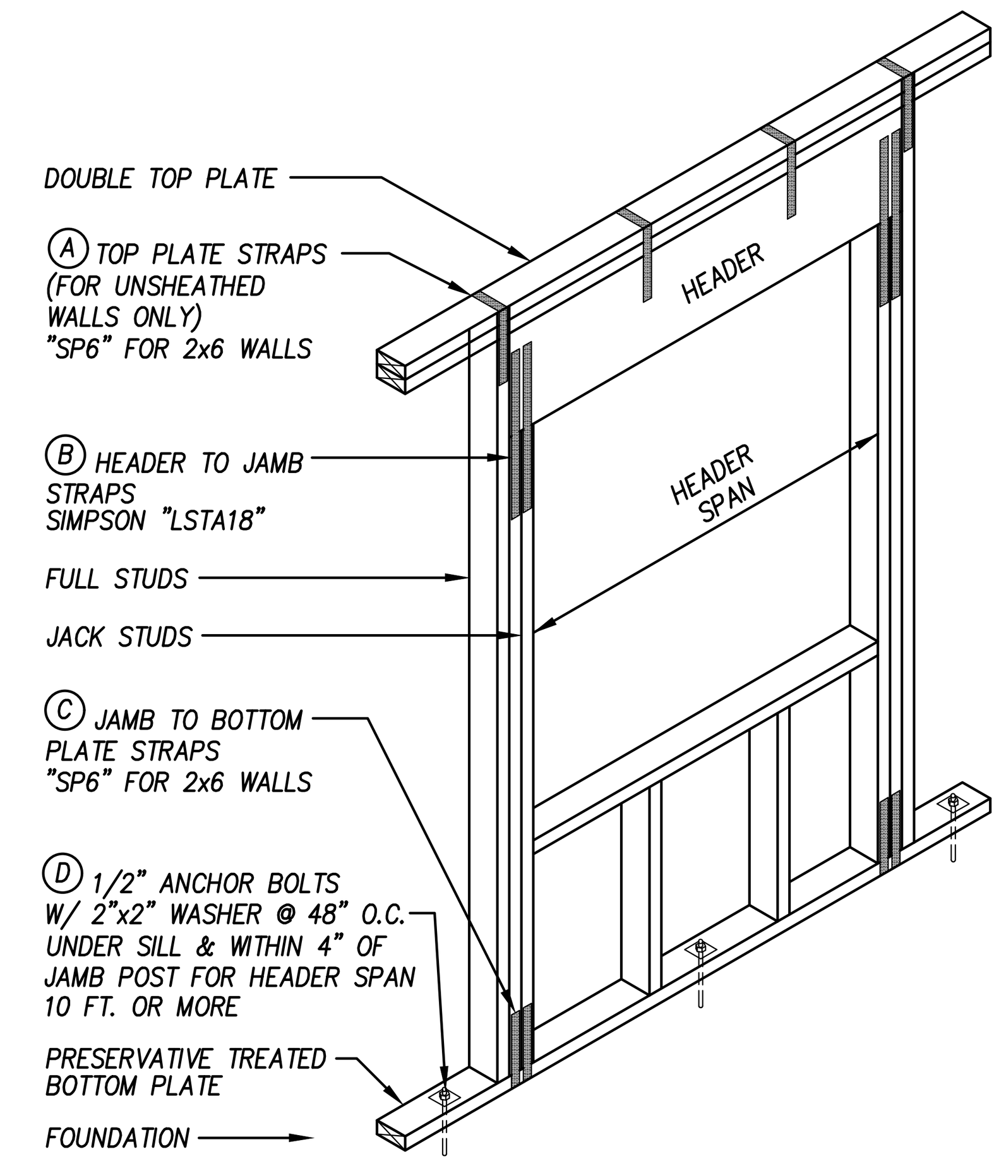
E — **Typical Gable End Bracing**
1" = 1'-0" WOOD-R015



J — **Roof Section**
1" = 1'-0" WOOD-R008



F — **Roof Section**
1" = 1'-0" WOOD-R007

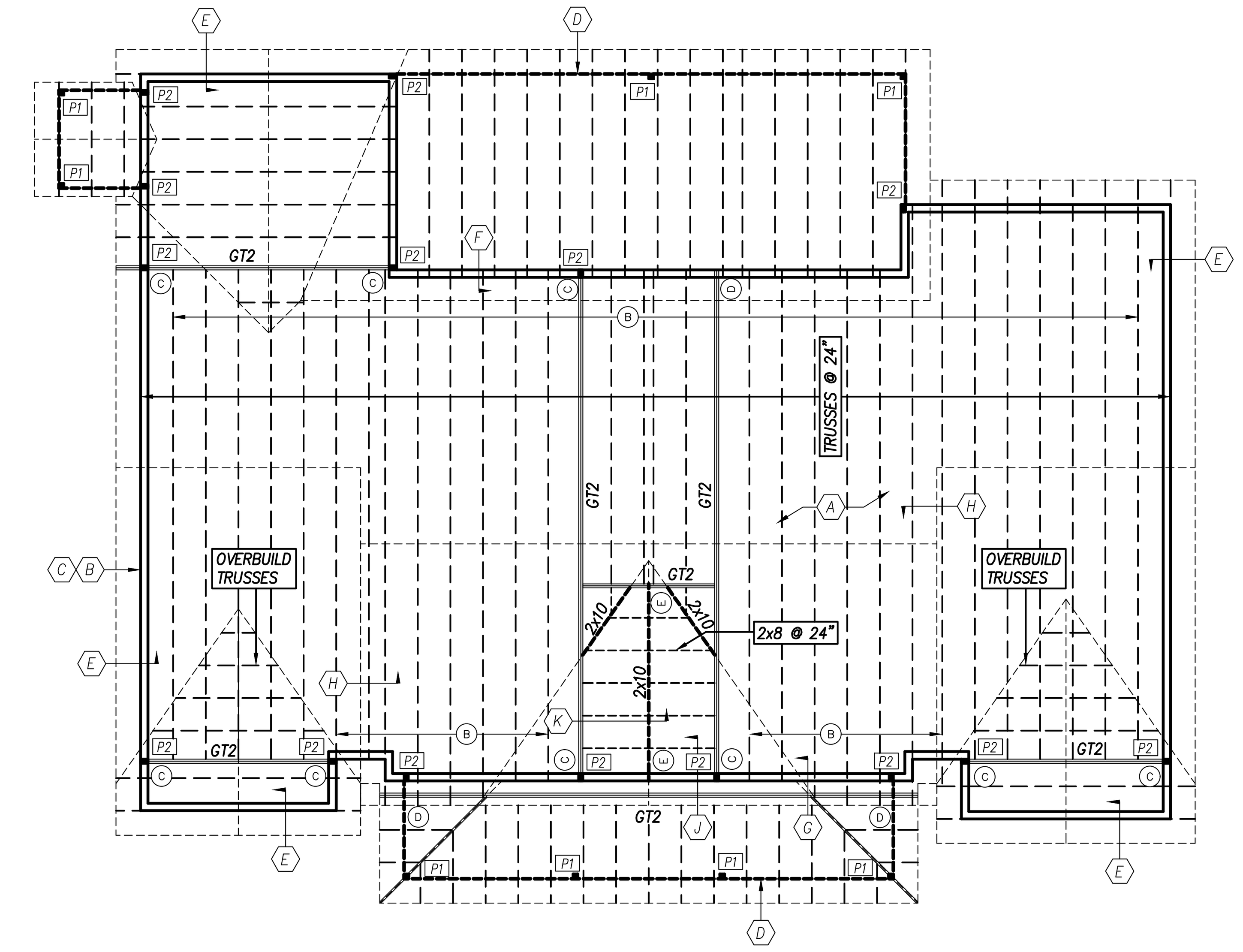


HEADER SPAN (FT)	HEADER	QTY JACK STUDS	QTY FULL STUDS	STRAP (A) SPACING	STRAP (B) QTY	STRAP (C) QTY	BOLT (D) QTY
4'-0"	(3) 2x8	1	1	32" O.C.	1	1	*
6'-0"	(3) 2x10	2	1	↓	1	1	*
8'-0"	(3) 2x12	2	2	↓	2	2	*

* PROVIDE BOLTS AT SPACING SPECIFIED FOR TYPICAL WALL CONSTRUCTION.

1. ALL CONNECTORS ARE BY SIMPSON STRONG-TIE OR EQUIVALENT.
2. PROVIDE 1/2" OSB SPACER BETWEEN PLIES FOR DIMENSIONAL LUMBER.
3. FASTEN HEADER PLYS TOGETHER WITH 16d NAILS @ 12" O.C. STAGGERED.
4. FASTEN JAMB STUDS TOGETHER WITH 10d NAILS @ 12" O.C.
5. DIMENSIONAL LUMBER HEADERS: NO. 2 SOUTHERN PINE (SP).

C — **Typical Header Schedule**
1/2" = 1'-0" WOOD-W300



1 — **Roof Framing Plan**
1/8" = 1'-0"

SYMBOL KEY	
(B) hexagon	DETAIL
(A) circle	ANCHOR
double line	EXTERIOR BEARING/ SHEAR WALL
hatched line	INTERIOR BEARING WALL
— —	TRUSS OR RAFTER
heavy dashed	BEAM
GT	GIRDER TRUSS
----	JOIST

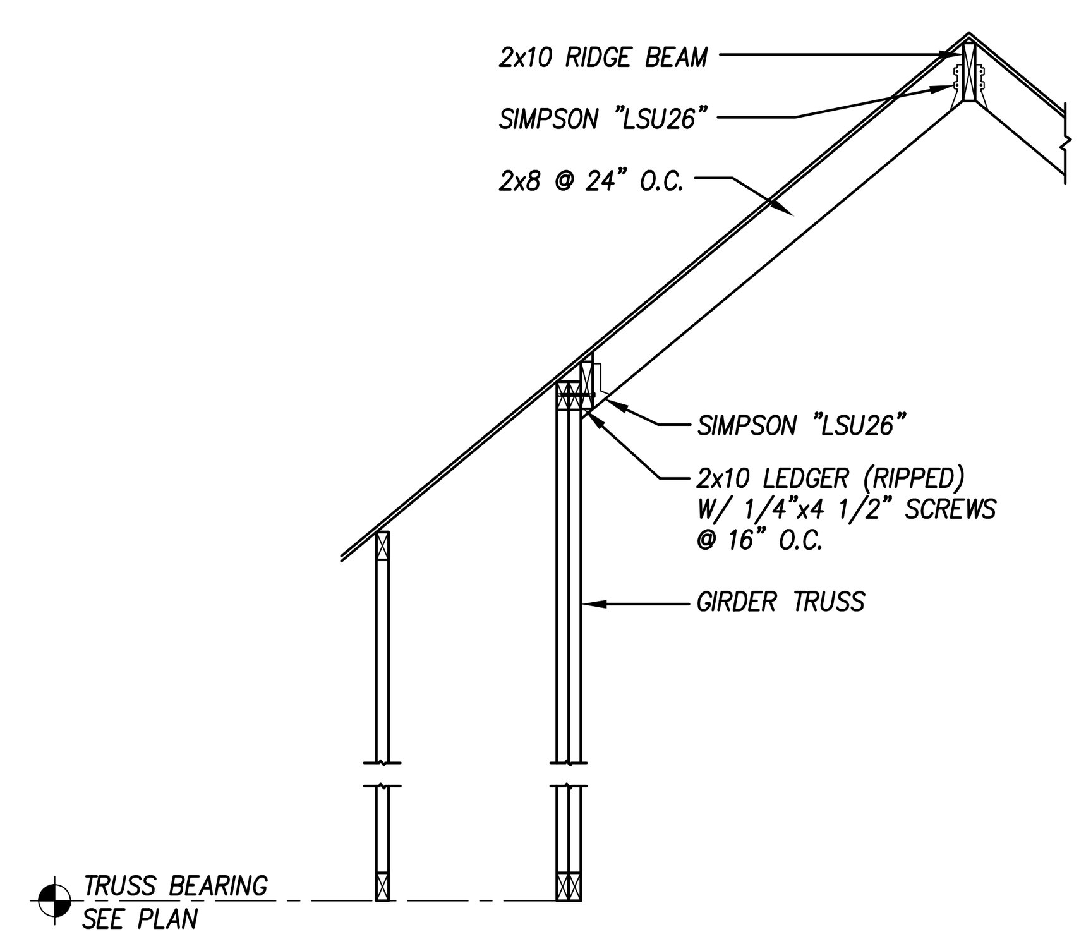
WOOD POST SCHEDULE

MARK	POST SIZE	POST SPECIES	POST GRADE
P1	4x4 PT	SP	NO. 2
P2	(2) 2x6	SPF	NO. 2

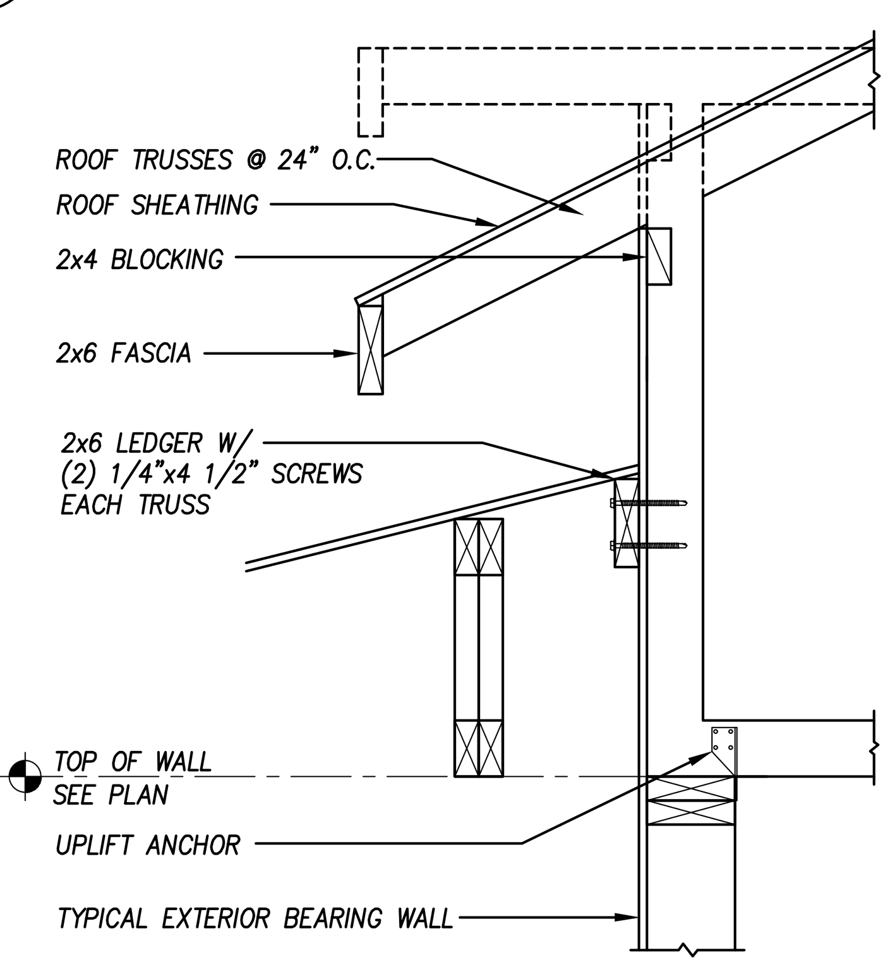
ANCHOR SCHEDULE

MARK	ANCHOR
(A)	H2.5A + (3) 10d TOE-NAILS UNLESS NOTED OTHERWISE
(B)	H10A
(C)	H10A-2 TOP, HTT4 BASE
(D)	H10A-2 TO HEADER OR BEAM
(E)	MTS18

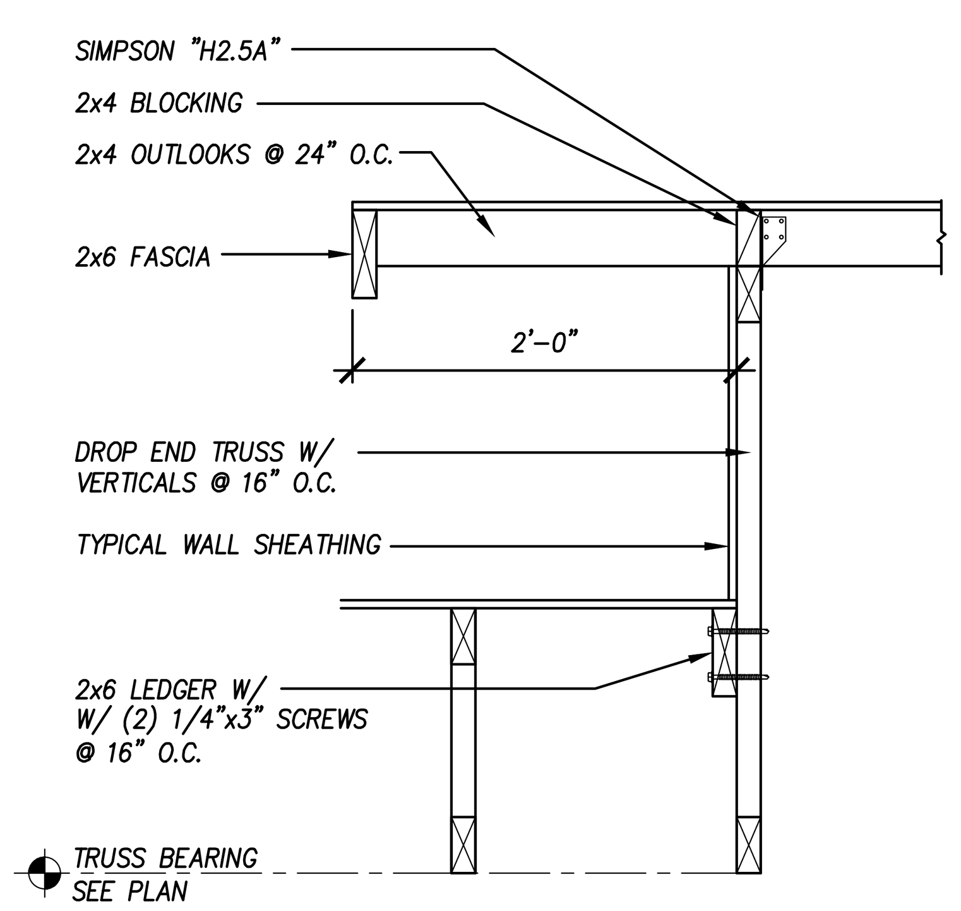
1. ANCHORS: SIMPSON STRONG-TIE.
2. ANCHORS EXPOSED TO WEATHER OR PT LUMBER TO BE "ZMAX."



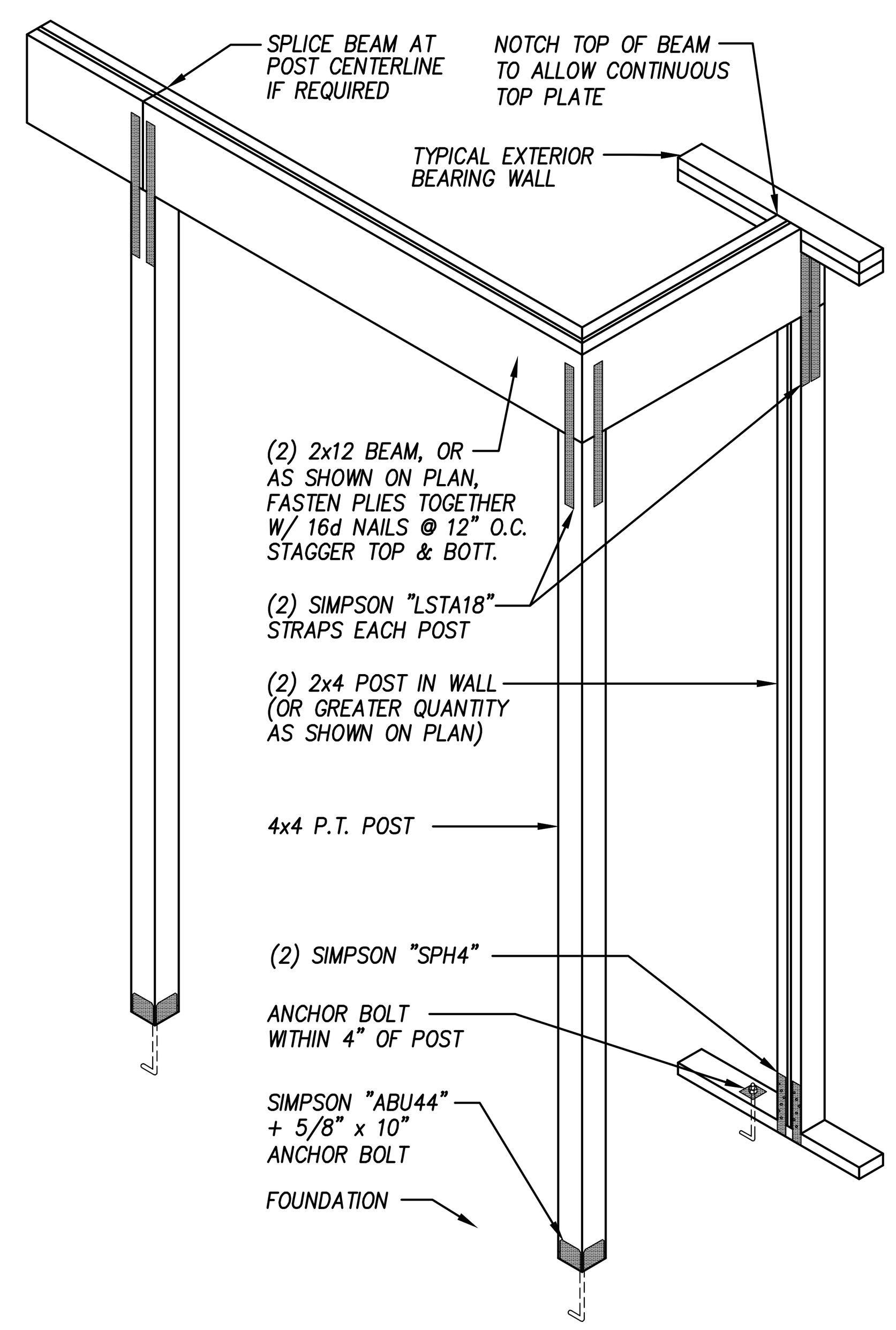
K — **Roof Section**
1/2" = 1'-0" WOOD-R009



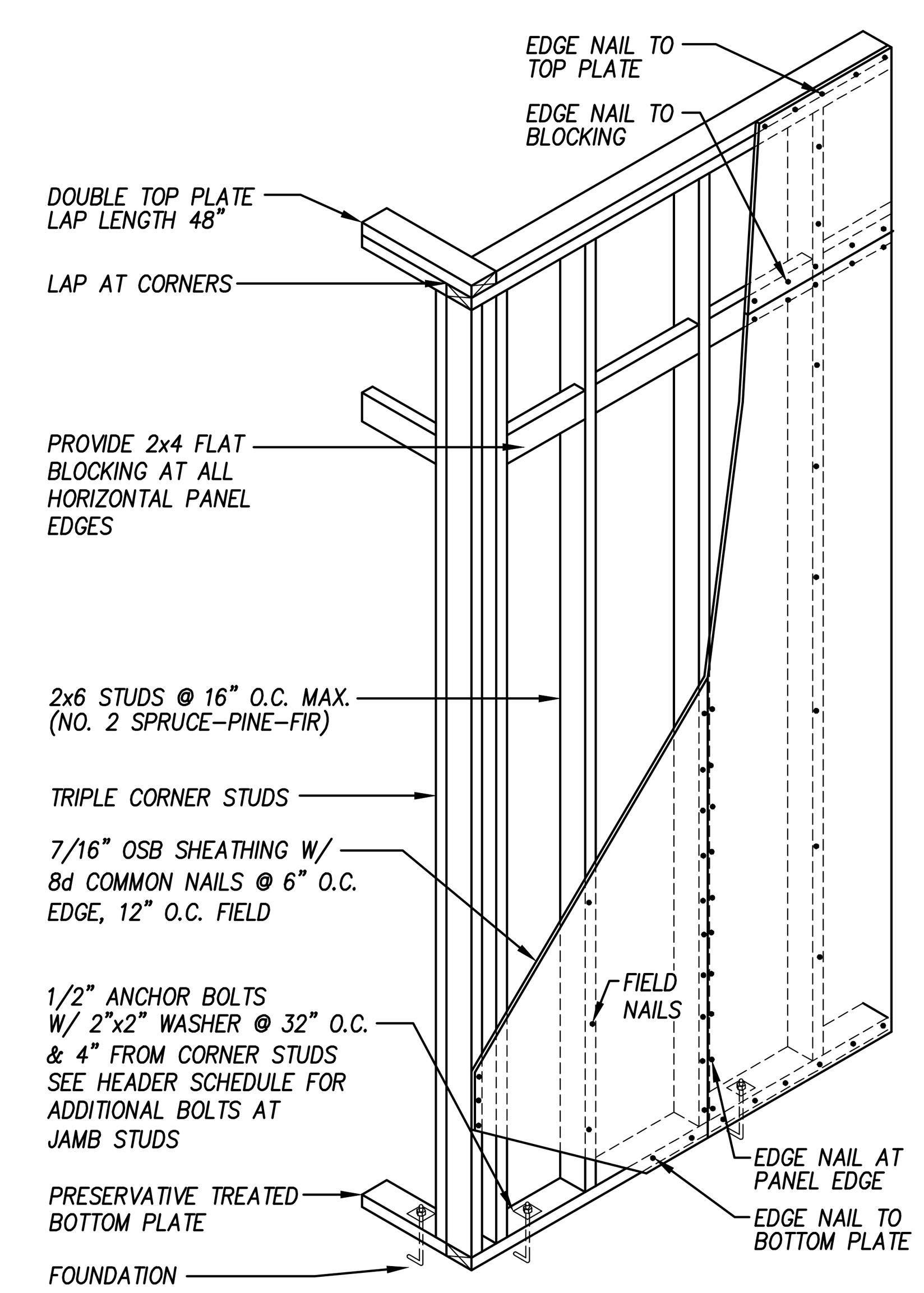
G — **Typical Truss Bearing**
1" = 1'-0" WOOD-R006



H — **Typical Roof Step**
1" = 1'-0" WOOD-R200



D — **Typical Post & Beam**
1/2" = 1'-0" WOOD-W500



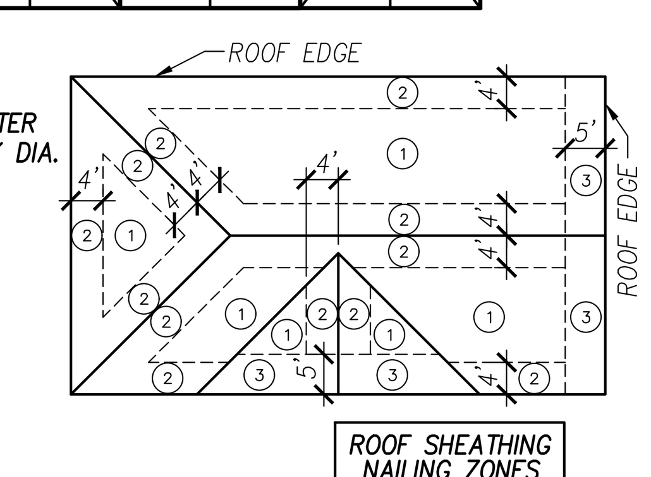
ANCHOR BOLTS: ASTM A36 STEEL, J-BOLTS W/ 7" EMBEDMENT, OR THREADED ROD SET W/ EPOXY, MIN. 5" EMBEDMENT

B — **Typical Exterior Bearing Wall**
1/2" = 1'-0" WOOD-W400

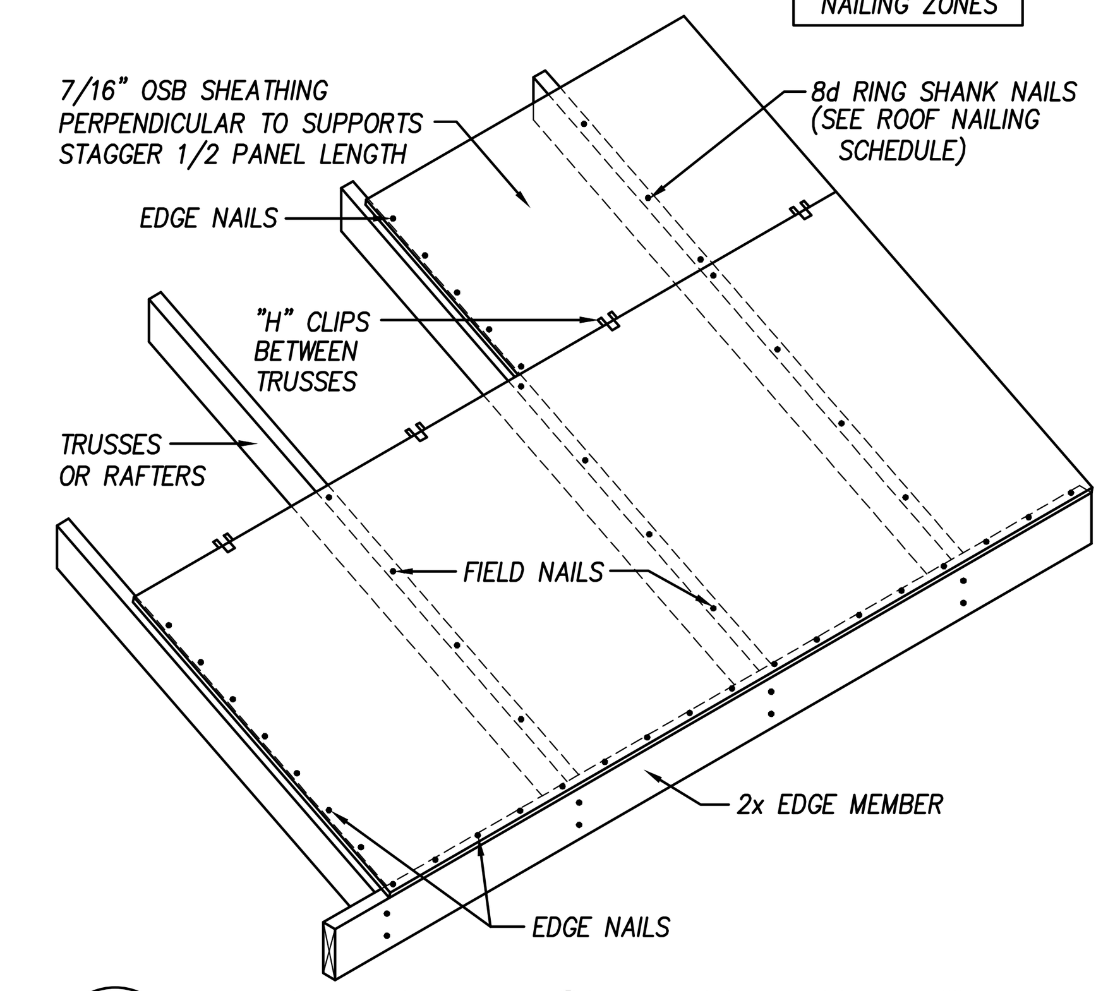
ROOF NAILING SCHEDULE

ROOF SHEATHING NAILING ZONE	NAIL SPACING 120 MPH EDGE	120 MPH FIELD	130 MPH EDGE	130 MPH FIELD	140 MPH EDGE	140 MPH FIELD
(1)	6"	12"	6"	12"	6"	12"
(2)	6"	12"	6"	12"	6"	6"
(3)	6"	6"	6"	6"	6"	6"

8d RING SHANK NAILS:
1. 0.113" NOMINAL SHANK DIAMETER
2. RING DIA. 0.012" OVER SHANK DIA.
3. 16 TO 20 RINGS PER INCH
4. 0.280" HEAD DIAMETER
5. 2 1/2" NAIL LENGTH



ROOF SHEATHING NAILING ZONES



A — **Typical Roof Sheathing Detail**
1/2" = 1'-0" WOOD-R000

Drawing Status: **For Permit & Construction**

Rev. No.:	Date:	Description:

WSE WAYLAND STRUCTURAL ENGINEERING
2801 SW 81st Street Gainesville, FL 32608
Phone: 352-317-4120
Email: waylandgs@aol.com
Florida COA: #8236

G. S. WAYLAND LICENSE No. 54396 STATE OF FLORIDA PROFESSIONAL ENGINEER

This item has been signed and sealed using Digital Signature. Printed copies of this document are not considered signed and sealed and the authentication code must be verified on any electronic copies.

Gregory S Wayland
2020.09.11
23:06:48-04'00'

Client: **Robinson Renovation & Custom Homes, Inc**
6910 W. University Avenue Gainesville, FL 32607

Project Name: **Rendek Residence**
SW Mapleton Street Fort White, FL

Drawing Name: **Roof Framing Plan and Details**

Drawn by: GS WAYLAND

Date: SEPTEMBER 10, 2020

WSE Project No.: **20087**

Drawing No. **S1**

GENERAL REQUIREMENTS

1. CODE COMPLIANCE:
   A. TO THE BEST OF THE ENGINEER'S KNOWLEDGE, PLANS AND SPECIFICATIONS COMPLY WITH APPLICABLE MINIMUM BUILDING CODES AS DETERMINED BY THE LOCAL AUTHORITY IN ACCORDANCE WITH FLORIDA BUILDING CODE AND CHAPTER 633, FLORIDA STATUTES.
   B. ALL CONSTRUCTION SHALL COMPLY WITH FLORIDA BUILDING CODE, 2017, AND ANY APPLICABLE LOCAL ORDINANCES AND REGULATIONS.

2. STRUCTURE: SIGNING AND SEALING DRAWINGS BY WSE CERTIFIES ONLY THE STRUCTURAL SYSTEMS FOR THIS STRUCTURE AND IS NOT A CERTIFICATION OF ANY CIVIL/SITE WORK, ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING OR OTHER SYSTEMS.

3. CONTRACT DOCUMENTS:
   A. STRUCTURAL DRAWINGS AND SPECIFICATIONS ARE PROPERTY OF WSE AND SHALL NOT BE REPRODUCED, REUSED OR ALTERED UNLESS SPECIFICALLY ALLOWED BY WSE.
   B. STRUCTURAL DRAWINGS AND SPECIFICATIONS SHALL BE USED IN CONJUNCTION WITH DRAWINGS AND SPECIFICATIONS BY OTHER DISCIPLINES.

4. CONSTRUCTION RESPONSIBILITIES: WSE HAS NO CONSTRUCTION PHASE SUPERVISORY RESPONSIBILITIES. CONTRACTOR IS SOLELY RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION, PROCEDURES, TECHNIQUES, SEQUENCES, INCLUDING TEMPORARY SHORING AND/OR BRACING. CONTRACTOR IS SOLELY RESPONSIBLE FOR JOB SITE SAFETY AND COMPLIANCE WITH APPLICABLE OSHA REGULATIONS.

5. TESTING AND INPECTION: REFER TO INDIVIDUAL SPECIFICATION SECTIONS FOR ITEMS REQUIRING TESTING AND INSPECTION. CONTRACTOR SHALL COMPLY WITH AND ACCOMMODATE LOCAL TESTING AND INSPECTION REQUIREMENTS.

EXTERIOR DOORS, WINDOWS, SKYLIGHTS

1. CODE COMPLIANCE: COMPLY WITH FLORIDA BUILDING CODE,, CHAPTER 14, 17 AND 24.

2. DESIGN RESPONSIBILITY: WSE IS NOT RESPONSIBLE FOR DESIGN, CONSTRUCTION OR PERFORMANCE OF WINDOW, DOOR OR SKYLIGHT UNITS. THIS SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER.

3. DESIGN WIND PRESSURE: THE BUILDING IS DESIGNED AS A FULLY ENCLOSED STRUCTURE, THEREFORE, WINDOWS, DOORS AND SKYLIGHTS SHALL BE DESIGNED TO RESIST COMPONENT AND CLADDING WIND PRESSURES SHOWN IN STRUCTURAL LOADS.

4. CERTIFICATION: CONTRACTOR SHALL OBTAIN CERTIFICATION FROM WINDOW, DOOR OR SKYLIGHT MANUFACTURER INDICATING THAT EACH UNIT IS CAPABLE OF RESISTING SPECIFIED DESIGN WIND PRESSURES.

5. FASTENING: CONTRACTOR SHALL OBTAIN FASTENING INFORMATION OR DIAGRAMS FROM MANUFACTURER INDICATING FASTENER TYPE, SIZE AND SPACING FOR EACH TYPE OF WINDOW, DOOR AND SKYLIGHT UNIT.

4. IMPACT PROTECTION OF GLAZING:
_X_ THIS BUILDING IS NOT LOCATED IN A WIND–BORNE DEBRIS REGION.
___ THIS BUILDING IS LOCATED IN A WIND–BORNE DEBRIS REGION.

STRUCTURAL LOADS

1. CODE COMPLIANCE: STRUCTURAL SYSTEMS FOR THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT VERTICAL AND LATERAL LOADS AS SPECIFIED IN FLORIDA BUILDING CODE, 2017, CHAPTER 16 AND ASCE 7–10 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES."

2. DEAD LOADS

ITEM	TOP CHORD (PSF)	BOTTOM CHORD (PSF)	TOTAL (PSF)
WOOD ROOF TRUSS:	15	5	20

3. FLOOR LIVE LOADS (FBC 1607.1)
   LIVE LOAD REDUCTION MAY BE CONSIDERED
   A. RESIDENTIAL 40 PSF

4. ROOF LIVE LOADS (FBC 1607.12)
   A. ROOF SLOPE: FLAT TO LESS THAN 4:12 20 PSF
   B. ROOF SLOPE: 4:12 TO LESS THAN 12:12 16 PSF
   C. ROOF SLOPE: GREATER THAN 12:12 12 PSF

5. WIND LOADS (FBC 1609)
   WIND LOADS HAVE BEEN DETERMINED BASED ON ASCE 7–10,
   A. ULTIMATE DESIGN WIND SPEED: 130 MPH
   B. NOMINAL WIND SPEED: 101 MPH
   C. BUILDING RISK CATEGORY: II
   D. WIND EXPOSURE CATEGORY: B
   E. INTERNAL PRESSURE COEFFICIENTS: +0.18/–0.18
   F. ENCLOSURE CLASSIFICATION: FULLY ENCLOSED
   G. COMPONENT & CLADDING DESIGN WIND PRESSURES:

WIND SPEED	COMPONENT AREA (SF)	INWARD PRESSURE (PSF)	OUTWARD PRESSURE (PSF)
130 MPH	10	+18.2	–24.4
	20	+17.5	–22.9
	50	+16.2	–20.5
	100	+15.4	–19.0
	200	+14.8	–17.5
	>500	+13.6	–15.1

02300 EARTHWORK

1. GEOTECHNICAL REPORT: SOIL BORINGS AND A GEOTECHNICAL REPORT HAVE NOT BEEN PREPARED FOR THIS SITE. IT IS RECOMMENDED THAT THE OWNER OR CONTRACTOR EMPLOY THE SERVICES OF A GEOTECHNICAL ENGINEER TO PERFORM SOIL BORINGS AND PROVIDE RECOMMENDATIONS FOR PREPARATION OF THE SOILS SPECIFIC TO THIS BUILDING SITE, AND CONFIRM THE SOIL TYPE ASSUMED IN THIS SPECIFICATION. WSE HAS NO KNOWLEDGE OF THE ON–SITE SOILS AND THEREFORE ACCEPTS NO RESPONSIBILITY FOR THEIR BEARING CAPACITY OR PERFORMANCE.

2. BEARING SOIL: SOILS ARE ASSUMED TO BE SANDY SOILS WITH NO ORGANICS, PEAT, CLAY, EXPANSIVE CLAYS, OR BOULDERS. SEASONAL HIGH WATER TABLE IS ASSUMED TO BE AT LEAST TWO FEET BELOW BOTTOM OF FOOTING ELEVATION. ALLOWABLE DESIGN BEARING CAPACITY IS ASSUMED TO BE 2,000 PSF.

3. SITE PREPARATION: STRIP AND GRUB ALL TREES, ROOTS, GRASSES, TOPSOIL, MUCK, ORGANICS, DEBRIS, PAVEMENTS AND OTHER DELETERIOUS MATERIALS TO 5 FEET BEYOND BUILDING LIMITS.

4. PROOF–ROLLING: FOLLOWING SITE PREPARATION, PRIOR TO FILL PLACEMENT, PROOF–ROLL BUILDING FOOTPRINT TO DENSIFY BEARING SOILS AND IDENTIFY AREAS OF LOOSE AND/OR SOFT SOILS. IF LOOSE OR SOFT SOILS ARE ENCOUNTERED, OVERCUT UNSUITABLE MATERIAL AND REPLACE WITH COMPACTED STRUCTURAL FILL. USE FULLY LOADED RUBBER TIRED EQUIPMENT.

5. EXCAVATION: EXCAVATIONS ARE TO BE PERFORMED IN COMPLIANCE WITH CURRENT OSHA REQUIREMENTS. CONTRACTOR IS RESPONSIBLE FOR EXCAVATION SAFETY. COMPACT ALL EXCAVATION BOTTOMS UNTIL FIRM AND UNYIELDING TO 95% OF MAXIMUM DRY DENSITY PER MODIFIED PROCTOR TEST (ASTM D 1557) TO A DEPTH OF 12 INCHES.

6. FOOTING BEARING: FOOTINGS ARE TO BEAR ON SUITABLE EXISTING SOILS OR PREPARED STRUCTURAL FILL. FOOTINGS SHALL BEAR A MINIMUM OF 12 INCHES BELOW ADJACENT GRADE.

7. GROUND/SURFACE WATER CONTROL: EXCAVATION AND BACKFILL OPERATIONS ARE TO BE MAINTAINED IN A DRY CONDITION. SLOPE OR CROWN EXPOSED BUILDING SUBGRADES TO PROMOTE RUN–OFF AND PREVENT PONDING. SURFACE AND INFILTRATING WATER ARE TO BE REMOVED BY GRADING AND PUMPING FROM SUMPS AS REQUIRED. GROUNDWATER ELEVATION IS TO BE MAINTAINED AT LEAST 2 FEET BELOW GRADE DURING COMPACTION OPERATIONS.

8. BACKFILL & COMPACTION: USE ONLY STRUCTURAL FILL FOR GENERAL FILL WITHIN BUILDING FOOTPRINT AND FOR ALL SLAB SUBGRADES. USE ONLY WELL GRADED SAND WITH MAXIMUM 10 PERCENT PASSING #200 SIEVE. DO NOT USE ON–SITE SOILS FOR BACKFILL UNLESS THEY ARE TESTED AND MEET GRADATION DESCRIBED ABOVE. PLACE BACKFILL IN MAXIMUM 12 INCH LOOSE LIFTS. USE 4 INCH LOOSE LIFTS WHEN FILL IS COMPACTED WITH WALK–BEHIND PLATE TAMPER. COMPACT TO 95% OF MAXIMUM DRY DENSITY PER MODIFIED PROCTOR TEST (ASTM D 1557) FOR EACH LIFT.

9. SLAB SUBGRADE TOLERANCE: TOP SURFACE OF SLAB SUBGRADE IS TO BE GRADED TO A TOLERANCE OF +0" TO –1/2".

10. PEST CONTROL: TREAT ALL SLAB SUBGRADES FOR TERMITES PRIOR TO SLAB INSTALLATION. OBTAIN CERTIFICATE OF TREATMENT FOR BUILDING INSPECTOR.

11. EXTERIOR GRADING: EXTERIOR GRADE IS TO BE KEPT MINIMUM 6 INCHES BELOW WOOD SIDING AND/OR FOAM INSULATION. SLOPE EXTERIOR GRADE AWAY FROM BUILDING TO PROMOTE DRAINAGE.

03300 STRUCTURAL CONCRETE

1. GENERAL: ALL CONCRETE CONSTRUCTION SHALL COMPLY WITH FLORIDA BUILDING CODE, CHAPTER 19, AND THE CURRENTLY ADOPTED EDITION OF THE FOLLOWING ACI STANDARDS:
   ACI 211.1 "SELECTING MIX PROPORTIONS"
   ACI 301 "SPECIFICATION FOR STRUCTURAL CONCRETE"
   ACI 304 "MEASURING, MIXING, TRANSPORTING, PLACING"
   ACI 305 "HOT WEATHER CONCRETING"
   ACI 306 "COLD WEATHER CONCRETING"
   ACI 315 "REINFORCING DETAILING AND PLACEMENT"
   ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"

2. CONCRETE MIXES: ALL CONCRETE MIXES SHALL BE NORMAL WEIGHT (140–150 PCF) UNLESS OTHERWISE NOTED. PROVIDE MIXES DESIGNED TO MEET THE FOLLOWING CRITERIA FOR VARIOUS ELEMENTS IN THE STRUCTURE:

ELEMENT	MINIMUM CEMENTITIOUS CONTENT (LB/CU. YD)	MAX. W/C RATIO	28 DAY COMPRESSIVE STRENGTH (PSI)	MAXIMUM AGGREGATE SIZE (INCHES)
SPREAD FOOTINGS, PIERS BELOW GRADE	470	0.60	3,000	1 1/2
SLAB ON GRADE	520	0.50	3,000	1
GRADE BEAMS	470	0.50	3,000	1 1/2
COLUMNS	520	0.50	4,000	1
BEAMS	520	0.50	4,000	1
SLABS ON DECK	540	0.50	3,500	3/4

   A. AIR ENTRAINMENT: PROVIDE AIR ENTRAINMENT PER ACI TABLE 4.2.2.7.b.1 FOR EXPOSURE CLASS F0.
   B. PORTLAND CEMENT: ASTM C 150, TYPE 1.
   C. FLY ASH: (OPTIONAL) ASTM C 618, CLASS C OR F. WHEN USED QUANTITY SHALL BE NOT LESS THAN 15% AND NOT MORE THAN 25%.
   D. AGGREGATES: ASTM C33.
   E. WATER: ASTM C94, CLEAN & POTABLE.
   F. ADMIXTURES: WATER REDUCING AND AIR ENTRAINING AGENTS SHALL BE USED. HIGH RANGE WATER REDUCING AGENTS (SUPERPLASTICIZERS) MAY BE USED AT CONTRACTOR OPTION. DO NOT USE ADMIXTURES CONTAINING CHLORIDES.

3. REINFORCING STEEL: ASTM A 615, GRADE 60, DEFORMED BARS.
   A. SEE LAP & BEND SCHEDULE FOR LAP & BEND LENGTHS.
   B. BAR COVER:
      IN CONTACT WITH GROUND . . . . . 3"
      EXPOSED TO WEATHER . . . . 2"
      BEAM & COLUMN STIRRUPS . . . .1 1/2"
      SLABS . . . . .3/4"
   C. PROVIDE CORNER BARS OR 90 DEGREE BEND AT ALL CORNERS.
   D. SUPPORT ON CHAIRS OR BOLSTERS.

4. WELDED WIRE REINFORCING (W.W.R.) ASTM A 185.
   A. REFER TO DRAWINGS FOR W.W.R. SIZE AND DIAMETER.
   B. USE FLAT SHEETS ONLY.
   C. LAP W.W.R. MINIMUM 10 INCHES.
   D. SUPPORT W.W.R. ON CHAIRS SPACED 3'–0" O.C. EACH WAY.

5. CURING: USE SPRAYED–ON MEMBRANE CURING COMPOUND ON SLABS, ASTM C 309, TYPE 1, SOLVENT FREE, OR PROVIDE CONTINUOUS WATER SPRINKLING FOR MINIMUM 7 DAYS.
   A. CURING COMPOUND: ASTM C 309, TYPE 1, SOLVENT FREE.
   B. ENSURE COMPATIBILITY WITH FLOORING ADHESIVES.

6. SLAB CONTRACTION JOINT FILLER: "SIKADUR 51 SL" EPOXY RESIN JOINT FILLER BY SIKA CORP. OR EQUIVALENT. USE ON SLABS–ON–GRADE WHERE JOINTS ARE EXPOSED TO VIEW ONLY.

7. SLAB JOINTS: SAW–CUT SLABS ON GRADE AT LOCATIONS SHOWN ON PLANS, OTHERWISE SAW–CUT IN ROUGHLY 10 FOOT SQUARES. SAW–CUT WITHIN 4 TO 12 HOURS OF FINISHING SLAB.

8. SLAB CRACKING: AS CONCRETE SLABS–ON–GRADE CURE AND DRY OUT THEY WILL SHRINK CAUSING CRACKS TO FORM ON THE SURFACE OF THE SLAB. W.W.R. IS INSTALLED TO HELP LIMIT THE WIDTH OF CRACKS THAT FORM. REPAIR CRACKS OVER 1/8" WIDE THAT DO FORM BY ROUTING AND PLACEMENT OF "SIKADUR 35, HI–MOD LV" EPOXY RESIN ADHESIVE BY SIKA CORP. OR EQUIVALENT.

9. CONCRETE FORMWORK:
   A. DESIGN, ERECT, SUPPORT, BRACE AND MAINTAIN ALL FORMWORK AS RECOMMENDED BY ACI 347 "RECOMMENDED STANDARD PRACTICE FOR CONCRETE FORMWORK.
   B. CONTRACTOR IS RESPONSIBLE FOR DESIGN, CONSTRUCTION AND SAFETY OF ALL FORMWORK. ALL FORMS, SHORES, AND BRACING SHALL BE ENGINEERED TO SUPPORT ALL LOADS IMPOSED INCLUDING WET CONCRETE, EQUIPMENT, LIVE LOADS, LATERAL LOADS DUE TO WIND AND CONCRETE IMBALANCE.
   C. PROVIDE "SMOOTH FORM" FINISH FOR ALL CONCRETE EXPOSED TO PUBLIC VIEW.
   D. PROVIDE 3/4 INCH CHAMFER FOR ALL EXPOSED EDGES OF COLUMNS AND WALLS.
   E. PATCH ALL TIE HOLES.

10. PENETRATIONS: PLUMBING SLEEVE SPACING SHALL BE THE LARGER OF (3) THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE, OR 6 INCHES CLEAR BETWEEN SLEEVES. PENETRATIONS ARE NOT PERMITTED IN ANY STRUCTURAL MEMBERS OTHER THAN THOSE SPECIFICALLY DESIGNATED ON STRUCTURAL DRAWINGS.

11. STEEL EMBEDMENTS: STEEL ANGLES, BARS, BOLTS EMBEDDED IN CONCRETE AND EXPOSED TO WEATHER SHALL BE HOT–DIP GALVANIZED.

12. TOLERANCES: TOLERANCES FOR CONCRETE CONSTRUCTION SHALL BE IN SLAB LEVELNESS AND FLATNESS SHALL BE ACCORDANCE WITH ACI 117. SLAB LEVELNESS SHALL BE "CONVENTIONAL STRAIGHTEDGE." CONTRACTOR SHALL MAKE EVERY EFFORT TO REDUCE SHRINKAGE AND CURLING OF SLABS BY SELECTING APPROPRIATE MIX DESIGN AND ADOPTING APPROPRIATE PLACEMENT, FINISHING AND CURING METHODS. CONTRACTOR SHALL CORRECT SLABS THAT DO NOT MEET REQUIRED TOLERANCES BY FLASH PATCHING OR GRINDING AS APPROPRIATE.

13. HOT & COLD WEATHER PROTECTION:
   A. INSTITUTE HOT WEATHER PROTECTION PROCEDURES WHEN TEMPERATURE EXCEEDS 90° F.
   B. INSTITUTE COLD WEATHER PROTECTION PROCEDURES WHEN TEMPERATURES ARE BELOW 40° F.

14. TESTING & INSPECTION: INSPECT/TEST THE FOLLOWING ITEMS:
   A. INSPECT ALL REINFORCING FOR GRADE, SIZE AND PLACEMENT PRIOR TO CONCRETE PLACEMENT.
   B. TEST CONCRETE FOR SLUMP AND COMPRESSIVE STRENGTH.

15. SUBMITTALS: SUBMIT THE FOLLOWING ITEMS FOR REVIEW:
   A. CONCRETE MIX DESIGN FOR EACH MIX TYPE, INCLUDING AGGREGATE GRADATION, ADMIXURES, AND COMPRESSIVE STRENGTH TEST DATA IN COMPLIANCE WITH ACI 301.
   B. REINFORCING PLACEMENT DRAWINGS.

04200 UNIT MASONRY

1. CODE COMPLIANCE: COMPLY WITH FLORIDA BUILDING CODE, CHAPTER 21, AND ACI 530.1–08/ASCE 6–08 "SPECIFICATION FOR MASONRY STRUCTURES."

2. CONCRETE MASONRY UNITS (CMU): ASTM C 90, TYPE 1, TWO CORE, NORMAL WEIGHT UNITS, 1,900 PSI NET AREA COMPRESSIVE STRENGTH, f'm = 1,500 PSI.
   A. INSTALL IN RUNNING BOND ONLY.
   B. DO NOT WET CMU BEFORE LAYING.
   C. PROVIDE HALF–LAP BOND AT CORNERS AND INTERSECTIONS.

3. MORTAR: ASTM C 270.
   A. TYPE S – FOR CONCRETE MASONRY UNITS.
   B. TYPE N – FOR BRICK VENEER.
   C. DO NOT USE MORTAR ADMIXTURES.
   D. DISCARD UNUSED MORTAR AFTER 2½ HOURS AFTER MIXING.
   E. USE FULL FACE, WEB AND HEAD BEDDING.

4. GROUT: ASTM C 476, FINE OR COARSE GROUT, MINIMUM 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS, 8–11 INCH SLUMP.
   A. HIGH AND LOW LIFT GROUT POURS ARE ACCEPTABLE.
   B. POURS LESS THAN 5 FEET HIGH ARE LOW LIFT AND DO NOT REQUIRE CLEANOUTS AT BASE OF WALL.
   C. POURS OVER 5 FEET HIGH ARE HIGH LIFT AND INSPECTION CLEANOUTS AT BASE OF WALL.
   D. MECHANICALLY CONSOLIDATE ALL GROUT POURS.

5. JOINT REINFORCING: ASTM A 951, LADDER TYPE, HOT–DIP GALVANIZED (1.50 OZ./SF) PER ASTM A 153, CLASS B, 9 GAUGE WIRES.
   A. INSTALL IN EVERY OTHER COURSE OF CMU (16 INCHES O.C.)
   B. PROVIDE EYE AND PINTEL TIES TO VENEER.

6. REINFORCING STEEL: ASTM A 615, GRADE 60, DEFORMED BARS.
   A. PROVIDE STANDARD HOOK INTO FOOTINGS FOR VERTICAL BARS.
   B. PROVIDE STANDARD HOOK INTO BOND BEAM AT TOP OF WALL.
   C. PROVIDE 90 DEGREE BEND OR CORNER BAR AT ALL CORNERS.
   D. LAP ALL BARS MINIMUM 48 BAR DIAMETERS.
   E. CENTER VERTICAL BARS IN CELL UNLESS NOTED OTHERWISE.
   F. USE VERTICAL BAR POSITIONERS FOR POURS OVER 5 FEET.
   G. MAINTAIN ½ INCH CLEAR BETWEEN CMU AND FACE OF BARS.

7. ANCHORS, TIES, ACCESSORIES: PROVIDE ANCHORS, TIES AND ACCESSORIES THAT COMPLY WITH THE FOLLOWING SPECIFICATIONS:
   A. PLATE & BENT BAR ANCHORS: ASTM A 36.
   B. SHEET METAL ANCHORS & TIES: ASTM A 1008.
   C. WIRE TIES & ANCHORS: ASTM A 82.
   D. ANCHOR BOLTS: ASTM A 307 OR A 36, Fy = 36 KSI.
   E. THREADED ROD ANCHORS: ASTM A 307.
   F. PROVIDE HOT–DIP GALVANIZING FOR ALL ANCHORS, TIES & ACCESSORIES IN EXTERIOR WALLS.

8. HOT & COLD WEATHER CONSTRUCTION:
   A. INSTITUTE "HOT WEATHER CONSTRUCTION PROCEDURES" WHEN TEMPERATURE IS OVER 90 DEGREES F.
   B. INSTITUTE "COLD WEATHER CONSTRUCTION PROCEDURES" WHEN TEMPERATURE IS BELOW 40 DEGREES F.

9. WALL BRACING: CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY SHORING FOR ALL UNCOMPLETED WALLS AND MAINTAIN UNTIL PERMANENT FLOOR AND/OR ROOF STRUCTURE IS IN PLACE.

11. LOADING: DO NOT APPLY UNIFORM LOADS TO MASONRY UNTIL 3 DAYS AFTER COMPLETION OF CONSTRUCTION. DO NOT APPLY CONCENTRATED LOADS FOR AT LEAST 7 DAYS.

12. TESTING & INSPECTION: INSPECT REINFORCEMENT FOR SIZE AND PLACEMENT.

06100 WOOD

1. CODE COMPLIANCE: COMPLY WITH FLORIDA BUILDING CODE, CHAPTER 23.

2. WOOD FRAMING: PS20 GRADING, NOMINAL SIZES, MAXIMUM 19 PERCENT MOISTURE CONTENT. PROVIDE SPECIES & GRADES AS FOLLOWS, UNLESS NOTED OTHERWISE:
   A. STUDS: CONSTRUCTION GRADE SPRUCE–PINE–FIR (SPF) AS SPECIFIED IN FRAMING DETAILS.
   B. JOISTS, HEADERS, BEAMS, BRACING: NO. 2 SOUTHERN PINE (SP).
   C. MISCELLANEOUS: SPRUCE–PINE–FIR (SPF).
   D. POSTS: NO. 2 SOUTHERN PINE (SP).

3. STRUCTURAL WOOD PANELS: MARK ALL PANELS WITH APA CERTIFICATION. PROVIDE THE FOLLOWING PANEL TYPE, THICKNESS, AND NAILING, UNLESS OTHERWISE NOTED.
   A. ROOF SHEATHING: ORIENTED STRAND BOARD (OSB), SHEATHING GRADE, OR APA RATED CDX PLYWOOD SHEATHING. FASTEN PER TYPICAL ROOF SHEATHING DETAIL.
   B. WALL SHEATHING: ORIENTED STRAND BOARD (OSB), SHEATHING GRADE, OR APA RATED CDX PLYWOOD SHEATHING. FASTEN PER TYPICAL EXTERIOR BEARING WALL DETAIL.

4. ENGINEERED WOOD PRODUCTS:
   A. LAMINATED VENEER LUMBER (LVL): "GP–LAM", BY GEORGIA PACIFIC, Fb = 2,900 PSI, Fv = 285 PSI, E = 2,000,000 PSI.

5. WOOD TRUSSES: ROOF AND FLOOR TRUSS MANUFACTURER TO ENGINEER TRUSSES TO SUPPORT DEAD, LIVE, AND WIND LOADS PER FLORIDA BUILDING CODE AND ASCE 7. TRUSS ENGINEERING TO BE SIGNED AND SEALED BY PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA.
   A. COMPLY WITH ANSI/TPI 1 "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED TRUSS CONSTRUCTION."
   B. COMPLY WITH BUILDING COMPONENT SAFETY INFORMATION, BCSI: GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED TRUSSES.
   C. COMPLY WITH TPI DSB "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
   D. COMPLY WITH BUILDING COMPONENT SAFETY INFORMATION, BCSI–B3: PERMANENT RESTRAINT/BRACING OF CHORDS AND WEB MEMBERS AND/OR BCSI–B7: TEMPORARY AND PERMANENT RESTRAINT/BRACING OF PARALLEL CHORD TRUSSES.

6. WOOD FASTENERS, ANCHORS & CONNECTORS:
   A. UNLESS OTHERWISE NOTED, USE FASTENERS AS SPECIFIED IN FBC TABLE 2304.9.1 "FASTENING SCHEDULE."
   B. ANCHOR BOLTS: ASTM A 307 OR A 36, 36 KSI YIELD STRENGTH.
   C. BOLTS: ASTM A 307.
   D. NAILS: COMMON, MIL GALVANIZED, UNLESS NOTED OTHERWISE.
   E. SCREWS: SELF–DRILLING WOOD SCREWS.
   F. CONNECTORS, ANCHORS, STRAPS & TIES: SIMPSON STRONG–TIE OR EQUIVALENT. REFER TO ANCHOR SCHEDULE.
   G. CORROSION PROTECTION: PROVIDE HOT–DIP GALVANIZED ANCHOR BOLTS, BOLTS, NUTS, WASHERS, NAILS, SCREWS, ANCHORS, CONNECTORS AND OTHER HARDWARE WHEN EXPOSED TO EXTERIOR WEATHER OR IN CONTACT WITH PRESERVATIVE TREATED LUMBER. COMPLY WITH ASTM A 153 FOR HOT–DIP FASTENER PRODUCTS, OR ASTM A 653, "G–185" COATING FOR HOT–DIP CONNECTOR AND SHEET PRODUCTS. USE SIMPSON "ZMAX" COATING OR EQUIVALENT.

7. WOOD TREATMENT:
   PROVIDE PRESERVATIVE TREATED WOOD FOR ALL LOCATIONS WHERE WOOD IS IN CONTACT WITH SOIL, CONCRETE OR MASONRY, OR IS PERMANENTLY EXPOSED TO EXTERIOR WEATHER. DO NOT USE PRESERVATIVES CONTAINING CCA PRODUCTS.

8. TESTING & INSPECTION: INSPECT SHEATHING FOR TYPE, GRADE, THICKNESS AND FASTENING. INSPECT FRAMING FOR SIZE, SPECIES, GRADE, SPACING AND CONNECTORS.

Drawing Status:
For Permit & Construction

Rev. No.:	Date:	Description:

WAYLAND
STRUCTURAL ENGINEERING
2801 SW 81st Street Gainesville, FL 32608
Phone: 352–317–4120
Email: waylandgs@aol.com
Florida COA: #8236

G.S. WAYLAND
LICENSE
No. 54396
STATE OF FLORIDA
PROFESSIONAL ENGINEER

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Gregory S Wayland
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Client:
Robinson Renovation & Custom Homes, Inc
6910 W. University Avenue Gainesville, FL 32607

Project Name:
Rendek Residence
SW Mapleton Street Fort White, FL

Drawing Name:
Structural Notes

Drawn by: GS WAYLAND

Date: SEPTEMBER 10, 2020

WSE Project No.: **20087**

Drawing No. **S2**