

8'-0"
2'-0"
6'-7 1/2"
3'-1 1/4"



20'-1 3/4"



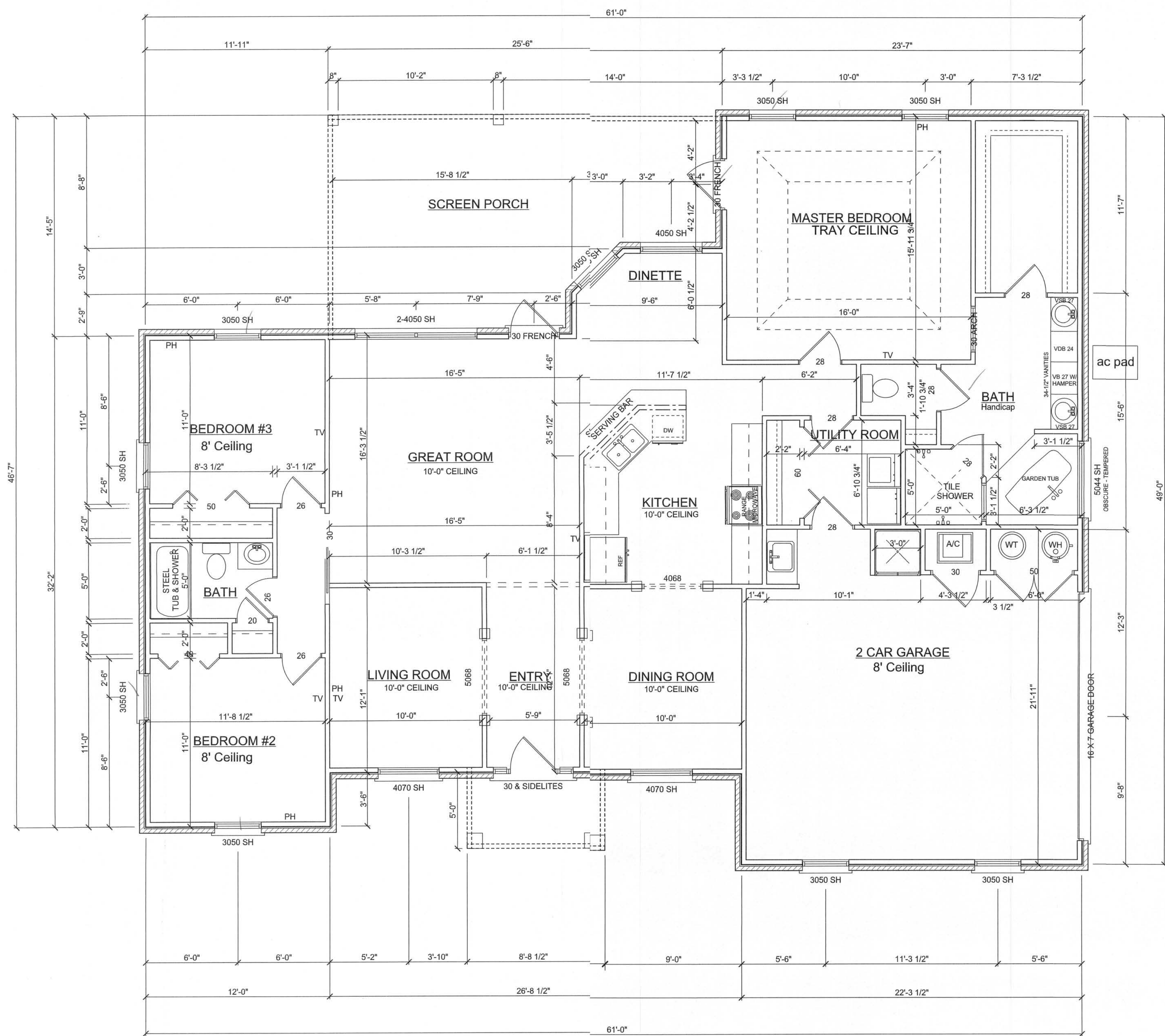
AREA SUMMARY	
LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

ELEVATION PLAN
SCALE: 1/4"=1'-0"

NEW CUSTOM HOME FOR:
HILLENDALE FARMS, LLC

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (366) 755-8887

DATE:
CHECKED BY:
SHEET NUMBER
A-1
OF 4 SHEETS



AREA SUMMARY

LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

MAIN FLOORPLAN

SCALE: 1/4"=1'-0"

NEW CUSTOM HOME FOR:
HILLENDALE FARMS, LLC

BRIAN S. CRAWFORD

ARCHITECTURAL DESIGN

DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

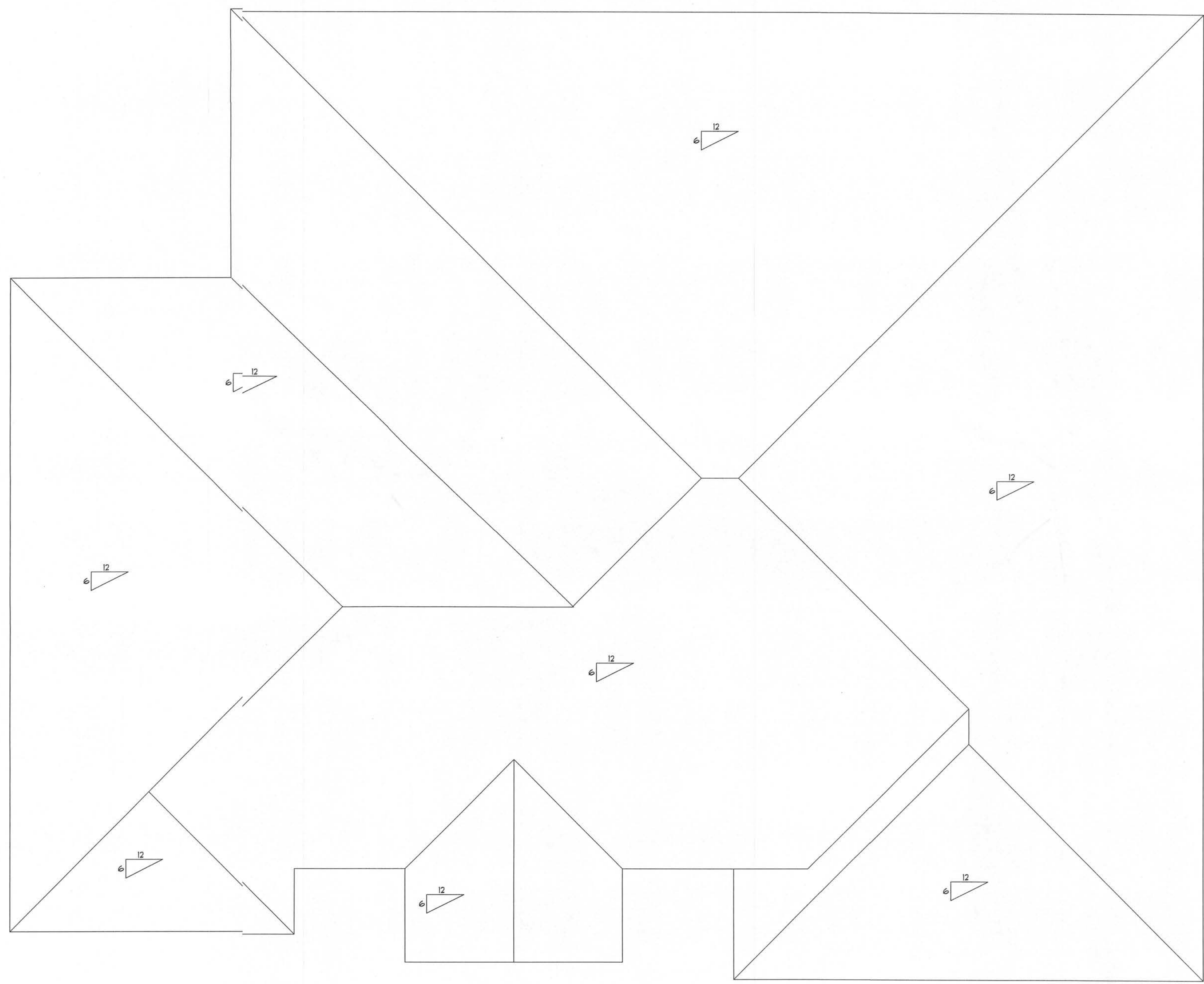
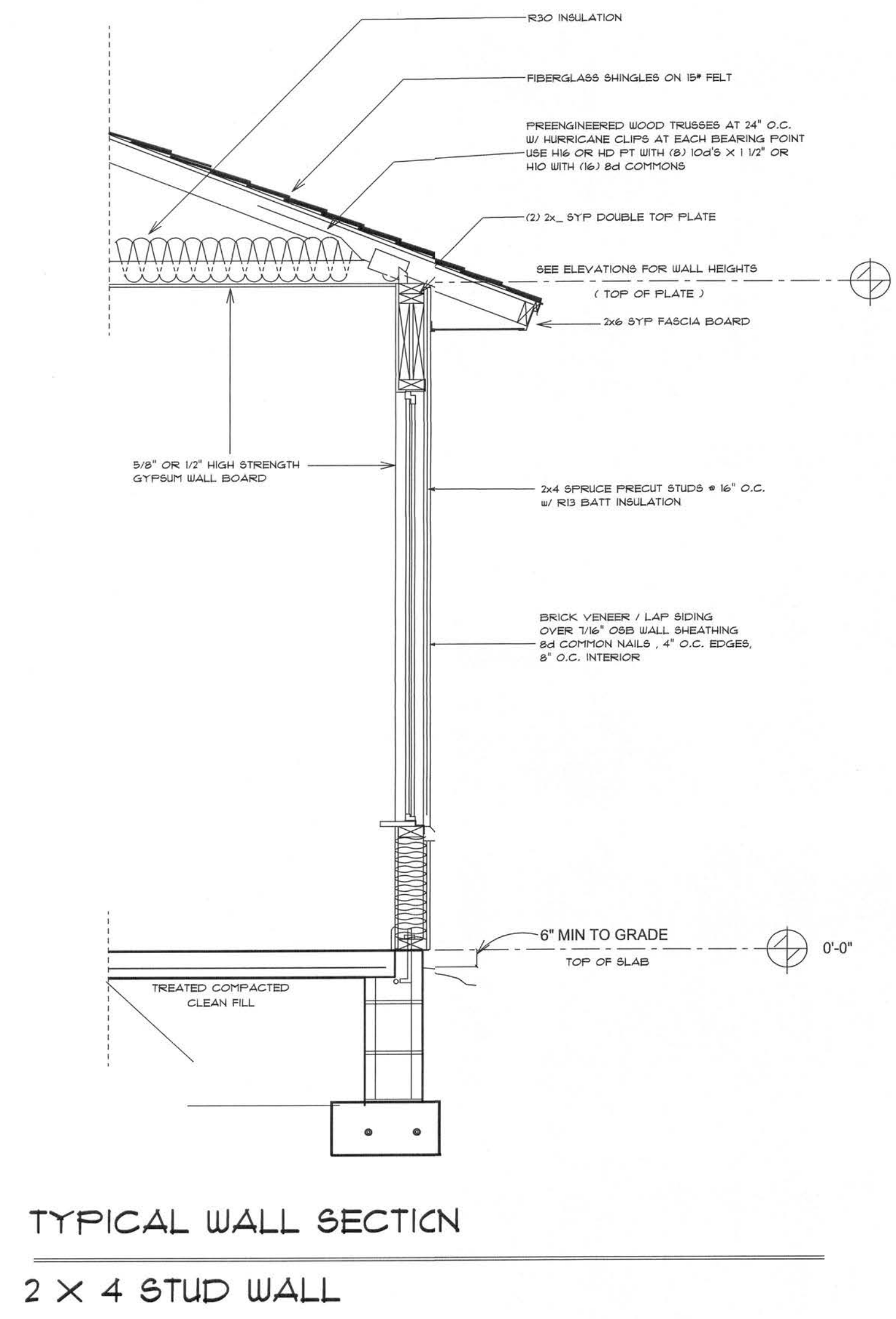
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SHEET NUMBER

A-2

OF 4 SHEETS



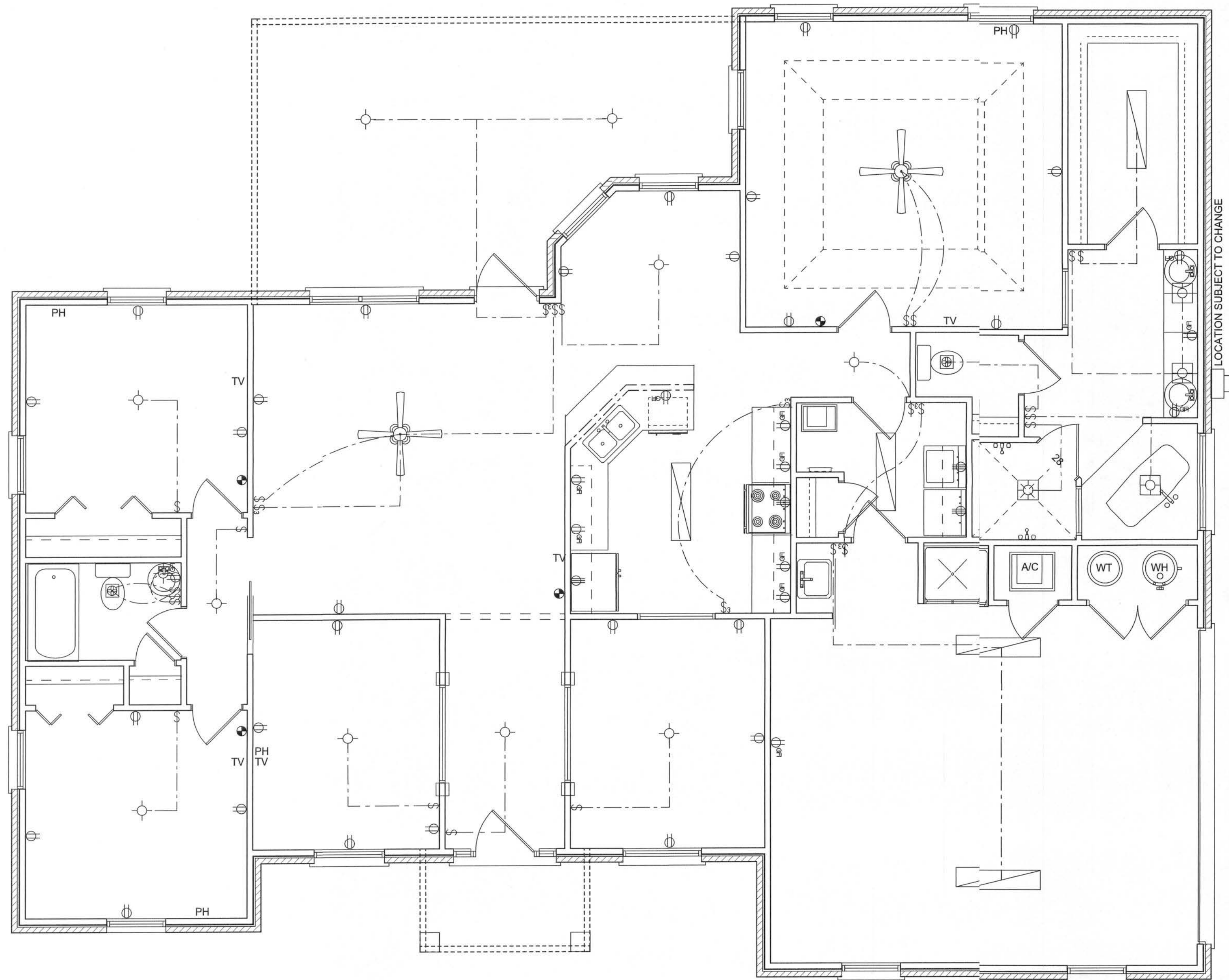
AREA SUMMARY	
LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

ROOF PLAN
 SCALE: 1/4"=1'-0"

NEW CUSTOM HOME FOR:
HILLENDALE FARMS, LLC

BRIAN S. CRAWFORD
 ARCHITECTURAL DESIGN
 DESIGNER: BRIAN CRAWFORD
 PHONE: (386) 755-8887

DATE:
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 SHEET NUMBER
A-3
 OF 4 SHEETS



ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	2	
can light	4	
fluorescent fixture	5	
electrical panel	1	
meter	1	
fan with light	2	
light	11	
outlet	30	
outlet 220v	4	
outlet gfi	13	
smoke detector	4	
switch	21	
switch 3 way	6	

ELECTRICAL PLAN NOTES

ALL RECEPTALS IN ALL BEDROOMS SHALL BE AFCI CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECTING MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE 4-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONTR SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR. SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE/W RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

AREA SUMMARY

LIVING AREA	1840.8 SF
GARAGE	478.7 SF
PORCHES	361.0 SF
TOTAL	2680.5 SF

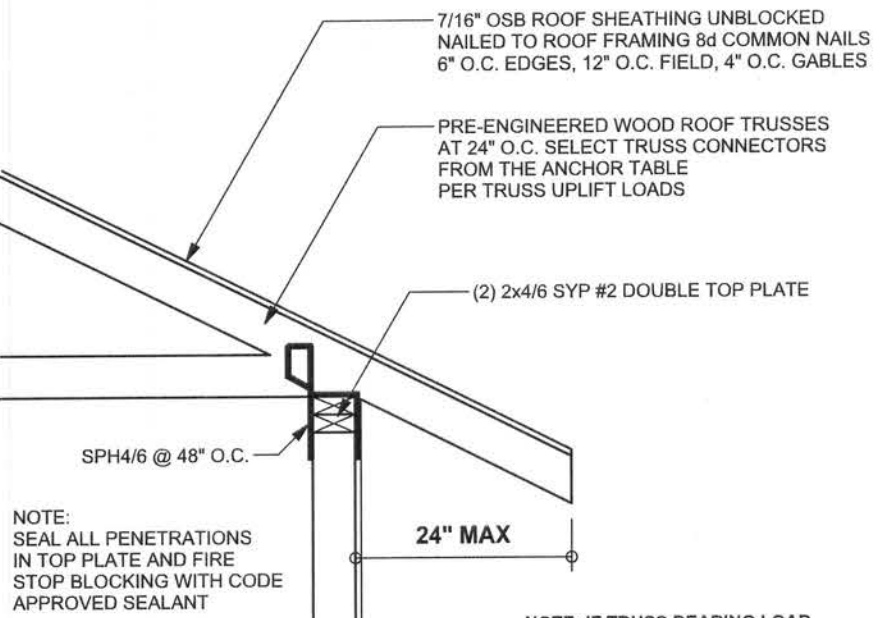
ELECTRICAL PLAN

SCALE: 1/4"=1'-0"

NEW CUSTOM HOME FOR:
HILLENDALE FARMS, LLC

BRIAN S. CRAWFORD
ARCHITECTURAL DESIGN
DESIGNER: BRIAN CRAWFORD
PHONE: (386) 755-8887

DATE:
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SHEET NUMBER
A-4
OF 4 SHEETS



NOTE: TRUSS BEARING LOAD EXCEEDS 425 PSF USE SYP #2 TOP PLATES; IF IT EXCEEDS 565 PSF ADD ADDITIONAL BEARING BLOCKS OR USE SIMPSON TBE BEARING ENHANCER

2x4x6 SPF#2 PRECUT STUDS AT 16\"/>

7/16\"/>

4\"/>

2x4x6 P.T. PINE SOLE PLATE ANCHORED WITH 1/2\"/>

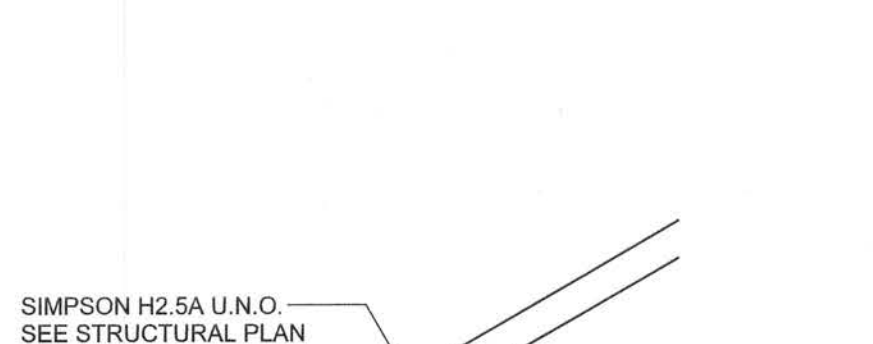
ONE STORY WALL SECTION

SCALE: 3/4\"/>

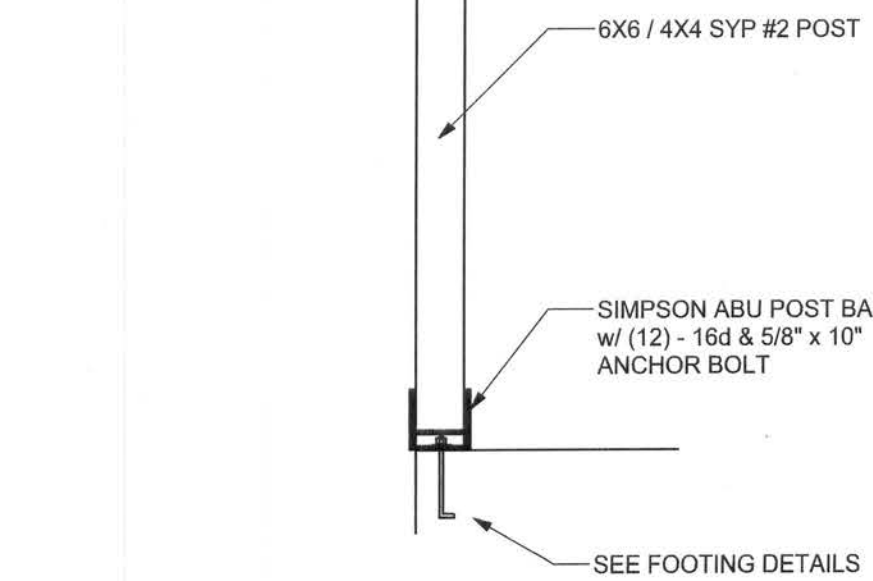
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16\"/>

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.208, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16\"/>

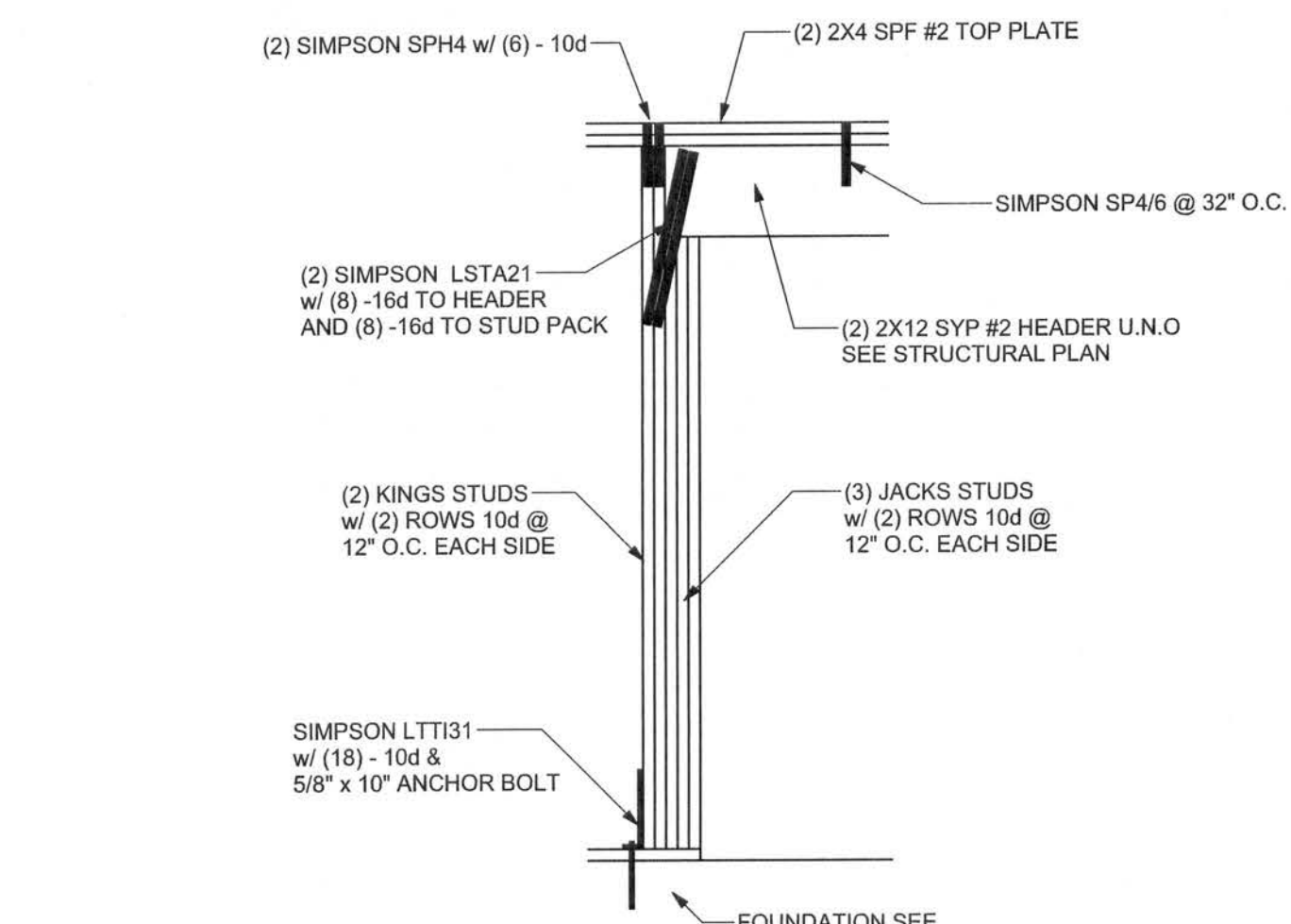


(2) SIMPSON LSTA21 w/ (8) -16d TO HEADER AND (8) -16d TO POST



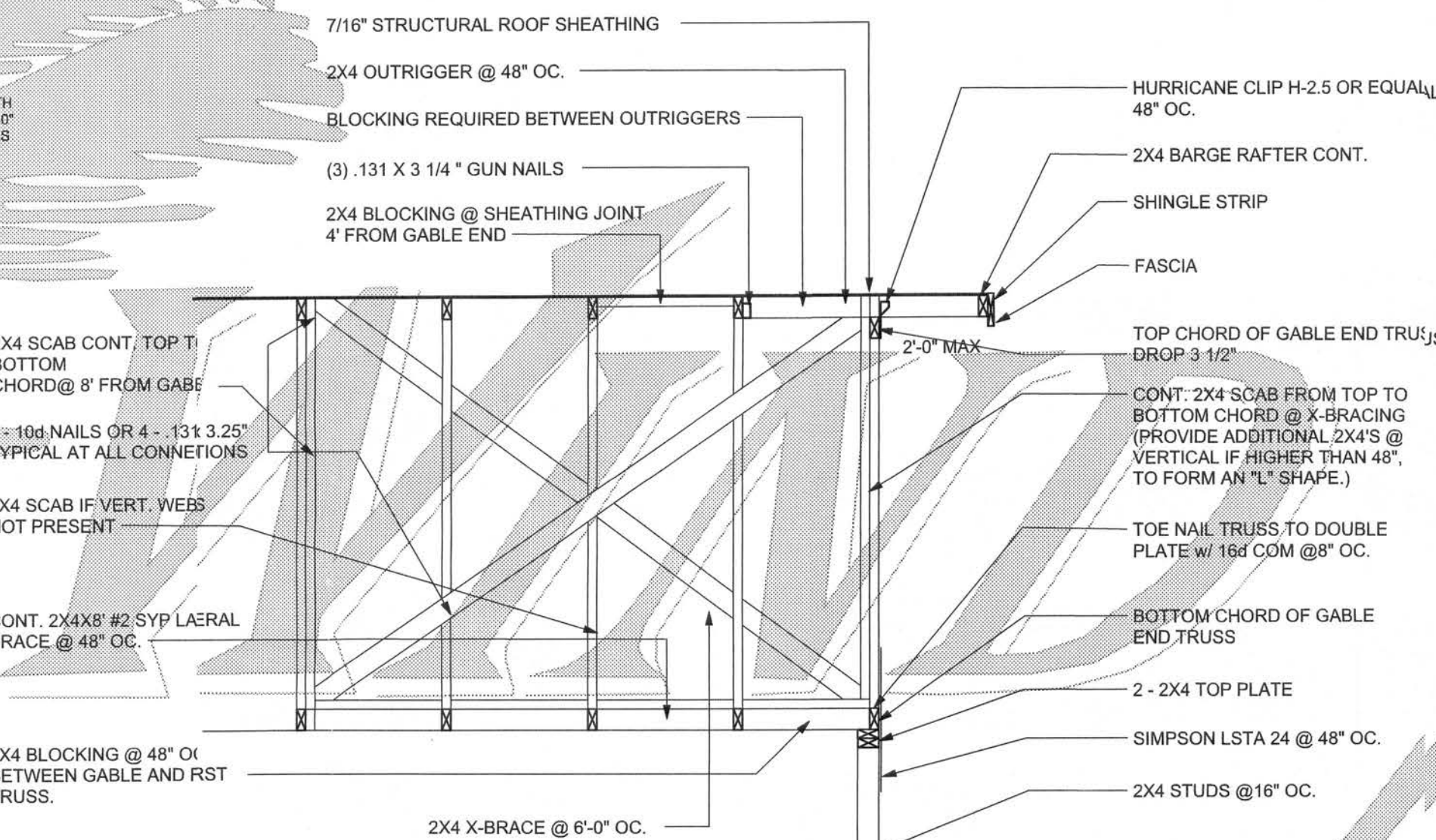
4X4 / 6X6 PORCH POST DETAIL

SCALE: 1/2\"/>



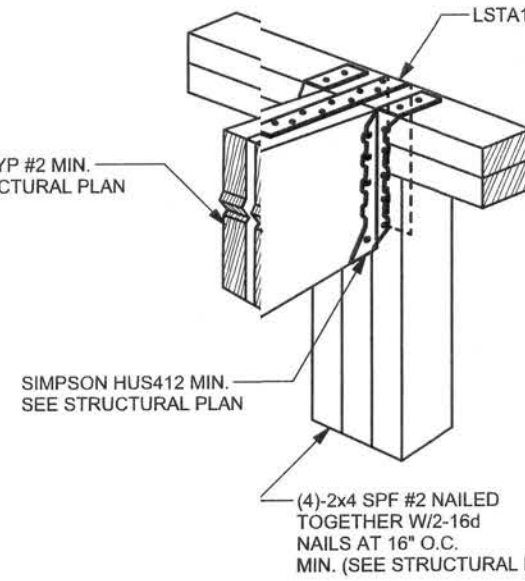
TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

SCALE: 1/2\"/>



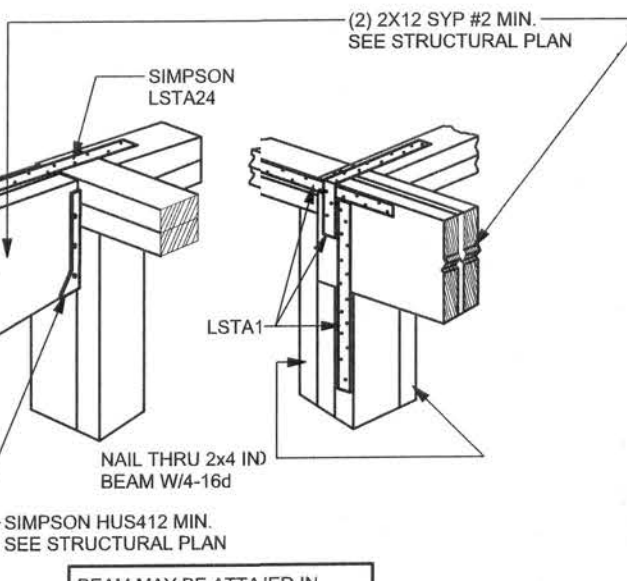
TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



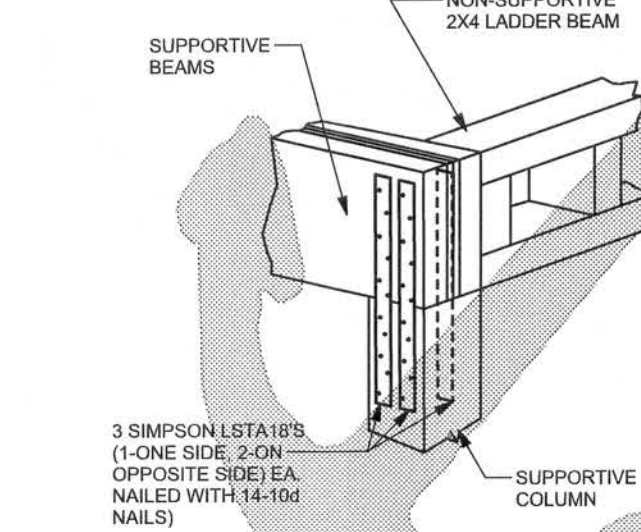
BEAM MID-WALL CONNECTION DETAIL

SCALE: N.T.S.



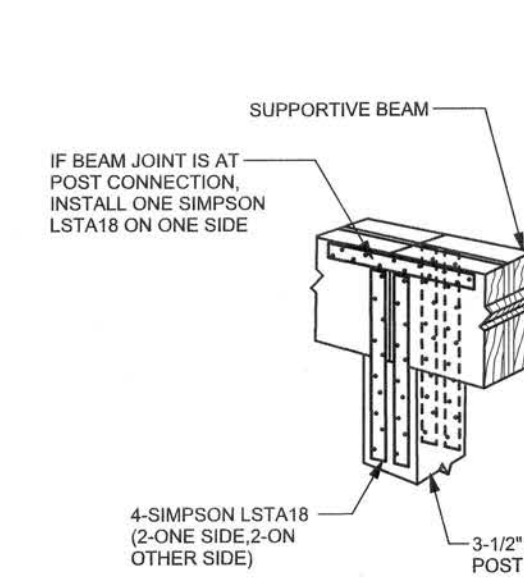
BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

SCALE: N.T.S.

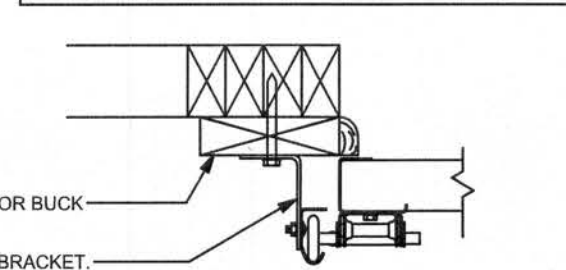


SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8\"/>

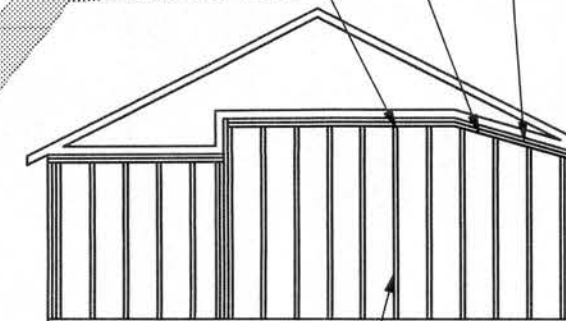
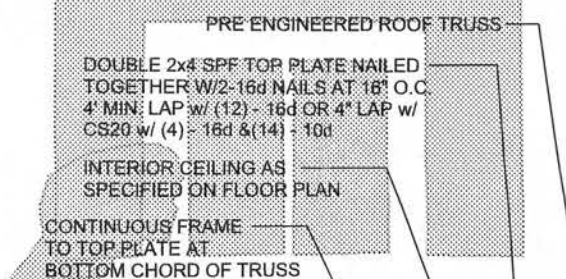


GARAGE DOOR BUCK INSTALLATION DETAIL

SCALE: N.T.S.

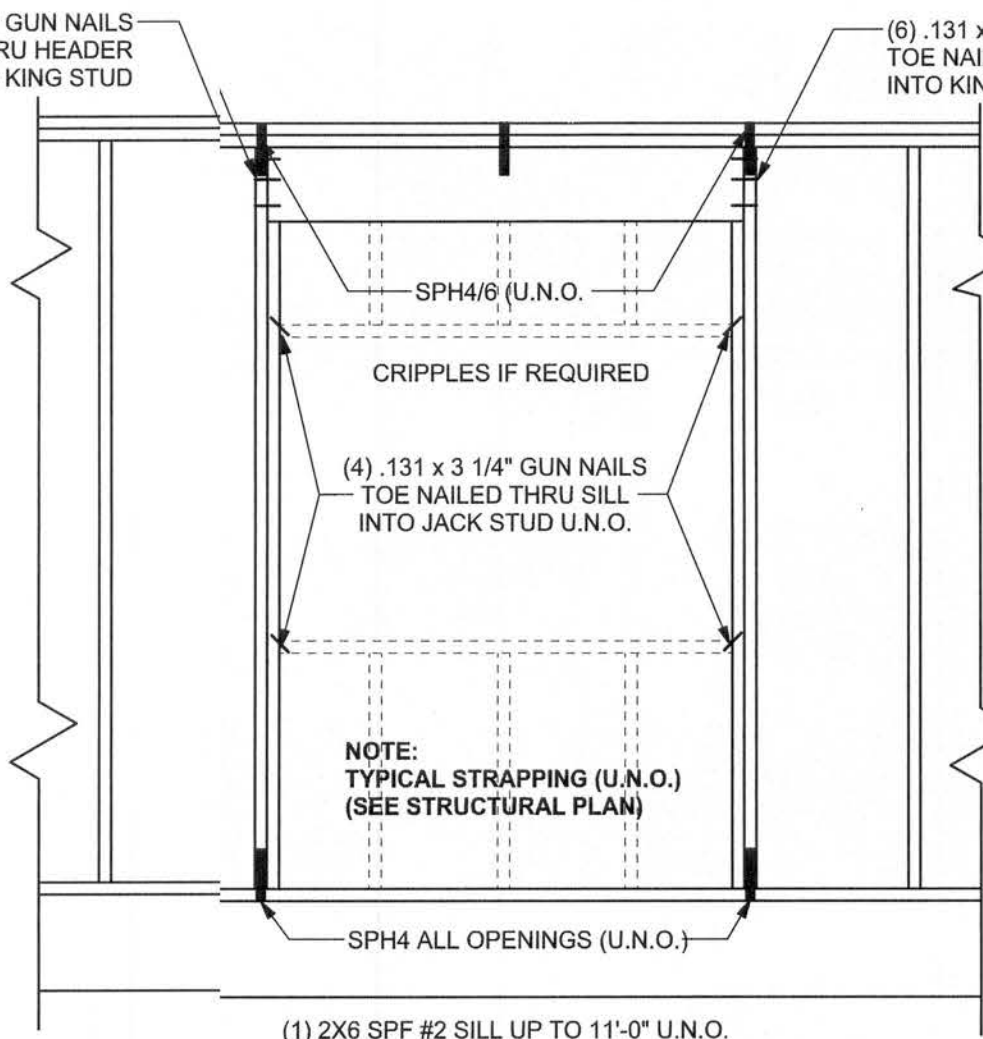
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1800	1.9
PSL	PARALAM	2800	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2\"/>

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 1/4x1/8 EACH END, 2X6 RAFTERS 7/16\"/>

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6\"/>

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.7 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 40\"/>

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,400, E = 1,800,000. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN USING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16\"/>

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7\"/>

WASHERS: WASHERS USED WITH 1/2\"/>

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8\"/>
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8\"/>
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5\"/>
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30\"/>
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class 60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 380	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 780	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SP4H			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SP6H			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LT119	8-16d		1/2" AB
< 2310	< 2310	LT1131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3895	HT16	18-16d		5/8" AB
< 1400	< 1400	HPAD42	16-16d		
< 5335	< 3335	HPAD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

DESIGN DATA

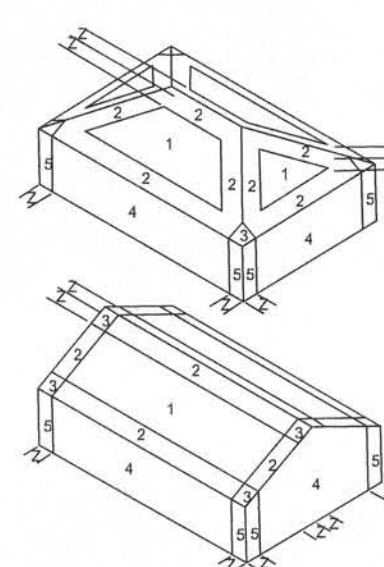
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT. NOT ON UPPER HALF OF HILL OR ESCARPMENT 60\"/>

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = < 30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 On g	40.6	-40.6	
3	19.9 - 25.5	18.1	-21.8
3 On g	68.3	-42.4	
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS

- FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
- 30 PSF (SLEEPING ROOMS)
- 30 PSF (ATTICS WITH STORAGE)
- 10 PSF (ATTICS WITHOUT STORAGE, < 3:12)
- ROOF 20 PSF (FLAT OR < 4:12)
- 16 PSF (4:12 TO < 12:12)
- 12 PSF (12:12 AND GREATER)
- STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
- SOIL BEARING CAPACITY 1000PSF
- NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 888, Lake City, FL 32066, 386-754-0419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915
20 June 07
SEAL

Stanley Crawford
Construction

Hillendale Farms, LLC

ADDRESS:
343 NW Hillendale Glen
Lake City FL 32055

Mark Disoway P.E.
P.O. Box 888
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
June 19, 2007

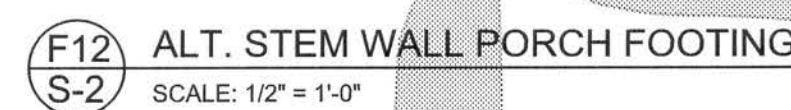
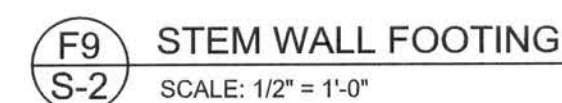
DRAWN BY: Ben Sparks
CHECKED BY:

FINALS DATE:
15 / Jun / 07

JOB NUMBER:
704123

DRAWING NUMBER
S-1

OF 3 SHEETS

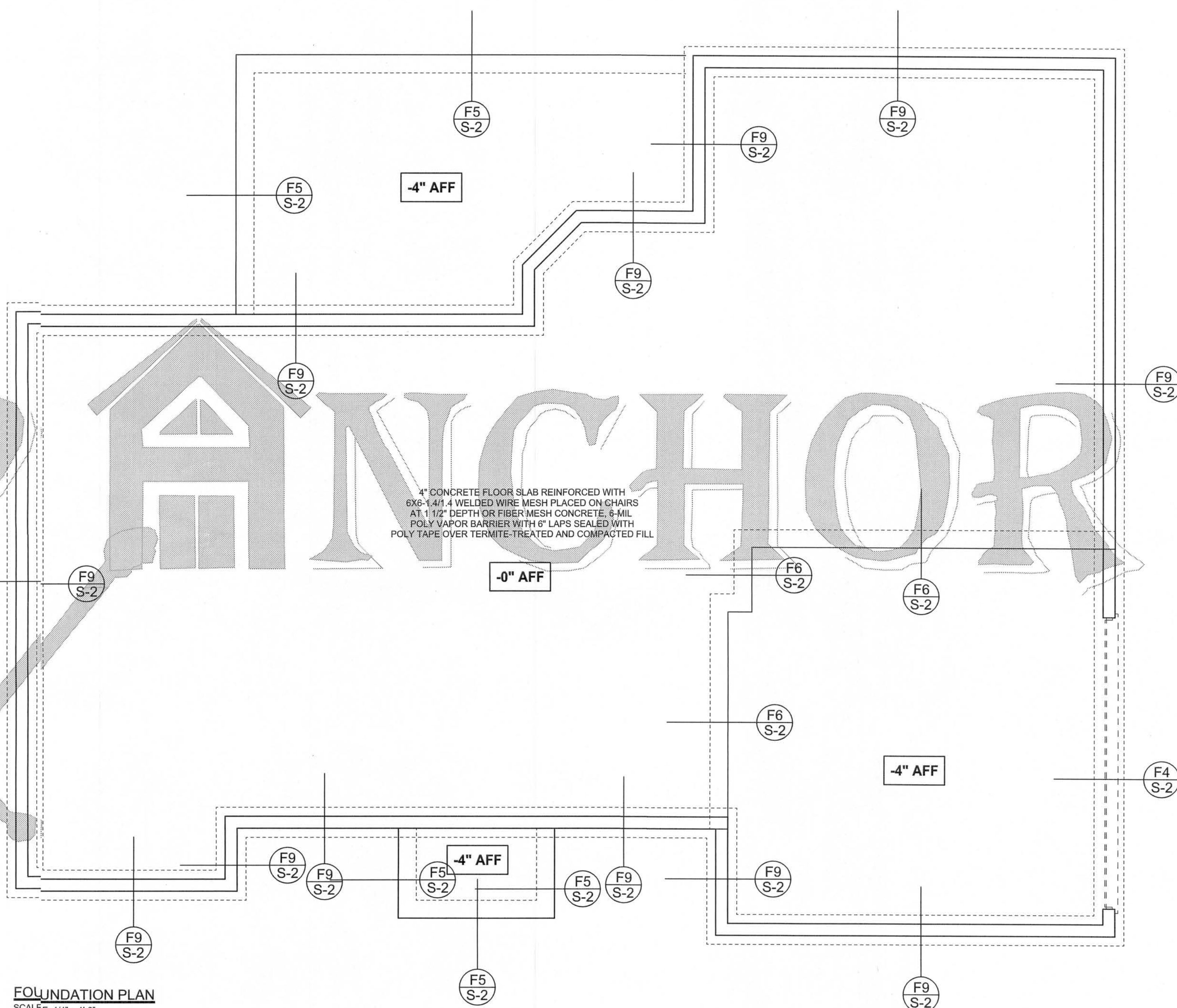


The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMMALL HEIGHT (FEET)	UNBALANCED BACKLIFT HEIGHT	VERTICAL REINFORCEMENT FOR 7" CMU STEMMALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMMALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

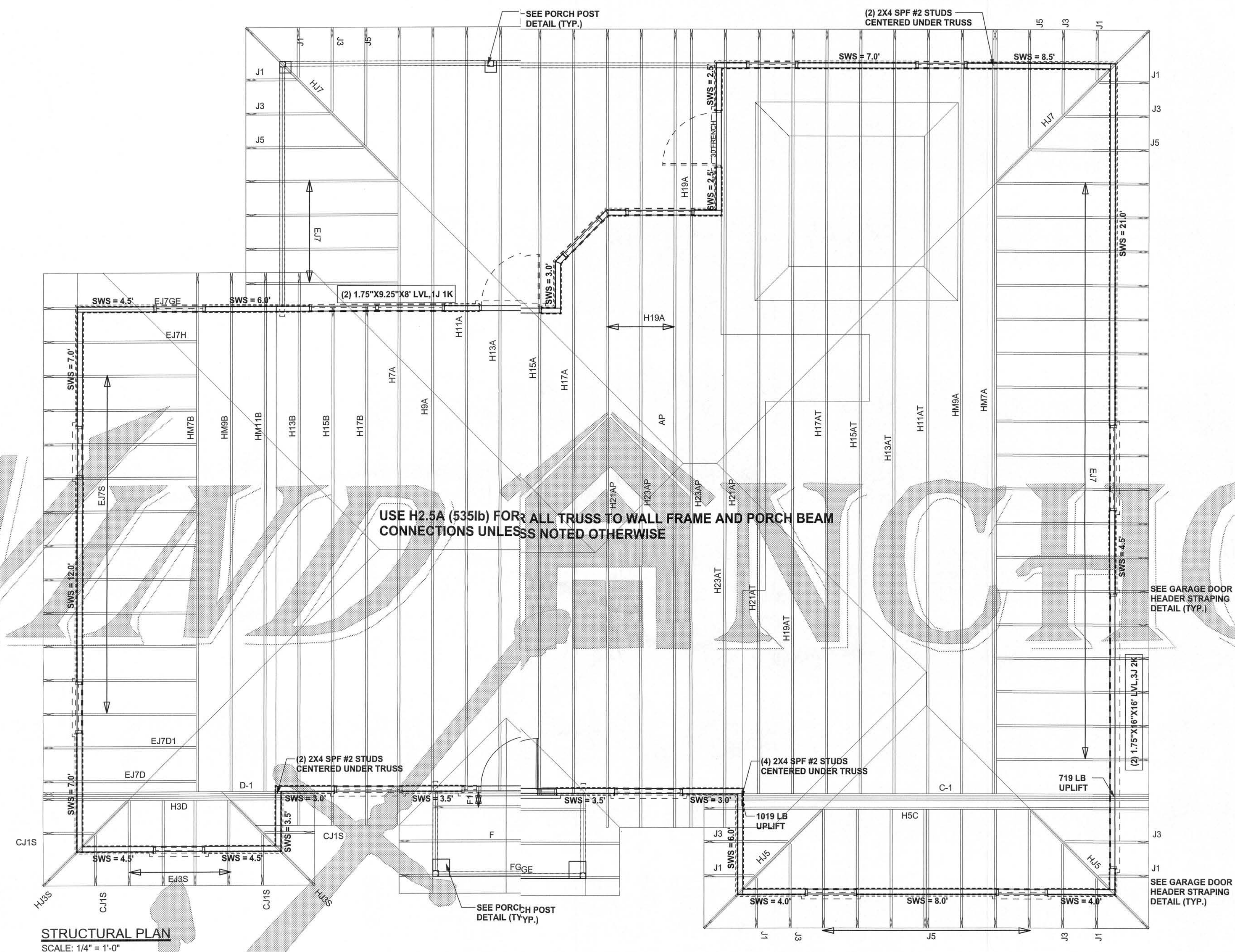
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS



1950
SEAL

OF 3 SHEETS



STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND

(2) 2X12X0', 1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
↑	NUMBER OF KING STUDS (FULL LENGTH)
↑	NUMBER OF JACK STUDS (UNDER HEADER)
↑	SPAN OF HEADER
↑	SIZE OF HEADER MATERIAL
↑	NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	29.0'	69.0'
LONGITUDINAL	16.0'	64.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS
JOB #7-180

WINDOW ENGINEER: Mark Disoway,
PE No. 3815, P.O. Box 868, Lake City, FL
32056-3867-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not brocade without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portion of the plan, relating to wind engineering, complies with section RS01.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 3815

Mark Disoway
200707
SEAL

Stanley Crawford
Construction

Hilendale Farms, LLC

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PRINTED DATE:
June 19, 2007

DRAWN BY:
BerSparks

CHECKED BY:

FINIS DATE:
15 Jun / 07

JOB NUMBER:
704123

DRAWING NUMBER

S-3

OF 3 SHEETS