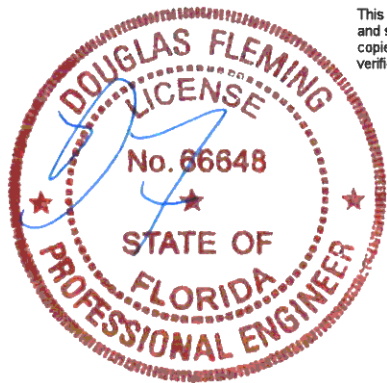




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08/19/2022

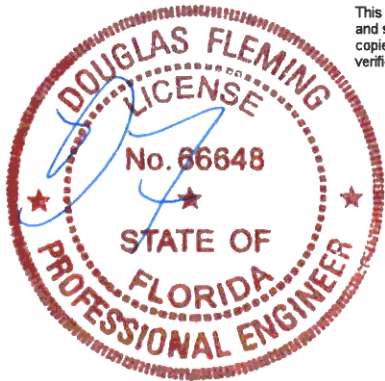
Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-5560
Job Description: Miller Res	
Address:	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 JRef #: 1X172150016
Wind Standard: ASCE 7-16 Wind Speed (mph): 120 Building Type: Closed	Design Loading (psf): 40.00, 55.00

This package contains general notes pages, 47 truss drawing(s) and 8 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	231.22.1320.30640	A01	2	231.22.1320.52367	A02
3	231.22.1320.54607	A03	4	231.22.1320.56817	A04
5	231.22.1321.10837	A05	6	231.22.1321.13670	A06
7	231.22.1321.16240	A07	8	231.22.1321.54427	A08
9	231.22.1321.56260	A09	10	231.22.1322.00107	A10
11	231.22.1322.15097	A11	12	231.22.1322.21807	A12
13	231.22.1322.24473	A13	14	231.22.1322.26287	A14
15	231.22.1322.29387	A15	16	231.22.1322.31243	B01
17	231.22.1322.32520	B02	18	231.22.1324.10600	B03
19	231.22.1324.13013	B04	20	231.22.1324.19303	B05
21	231.22.1326.05270	B06	22	231.22.1326.29323	B07
23	231.22.1326.30887	B08	24	231.22.1326.32440	B09
25	231.22.1326.43033	B10	26	231.22.1326.44767	C01
27	231.22.1326.46440	C02	28	231.22.1326.58870	C03
29	231.22.1327.35063	D01	30	231.22.1328.05960	D02
31	231.22.1328.17657	E01	32	231.22.1328.18903	E02
33	231.22.1328.26237	F01	34	231.22.1328.28177	F02
35	231.22.1328.31323	PB01	36	231.22.1328.33447	PB02
37	231.22.1328.38877	PB03	38	231.22.1328.59590	PB04
39	231.22.1329.03483	V01	40	231.22.1329.04660	V02
41	231.22.1329.05777	V03	42	231.22.1329.07077	V04
43	231.22.1329.09333	V05	44	231.22.1329.10680	V06
45	231.22.1329.11980	V07	46	231.22.1329.13857	V08
47	231.22.1329.16847	V09	48	A12030ENC160118	
49	BRCLBSUB0119		50	GABRST160118	



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North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 21-5560
Job Description: Miller Res	
Address:	

Item	Drawing Number	Truss
51	GBLLETIN0118	
53	CNNAILSP1014	
55	VALTN160118	

Item	Drawing Number	Truss
52	A12015ENC160118	
54	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

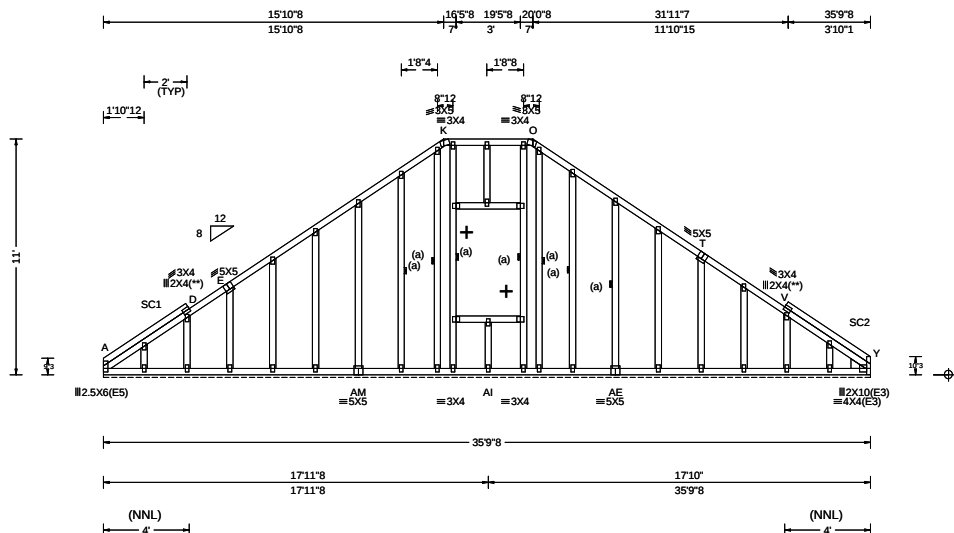
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 100071 FROM:	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A01	Cust: R 215 JRef: 1X172150016 T2 DrwNo: 231.22.1320.30640 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.58 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 M 999 240 VERT(CL): 0.011 M 999 180 HORZ(LL): 0.005 V - - HORZ(TL): 0.006 V - - Creep Factor: 2.0 Max TC CSI: 0.338 Max BC CSI: 0.039 Max Web CSI: 0.165 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Y* 84 /- /- /55 /- /7 Wind reactions based on MWFRS Y Brg Wid = 429 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals not exposed to wind pressure.

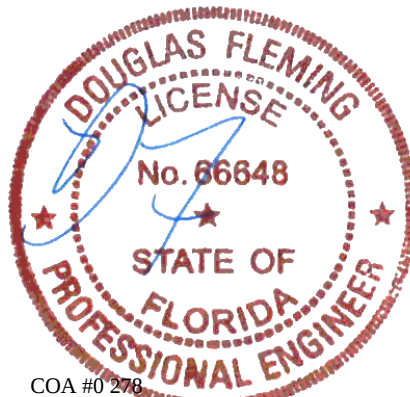
Wind loading based on both gable and hip roof types.

+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



COA #0 278

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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

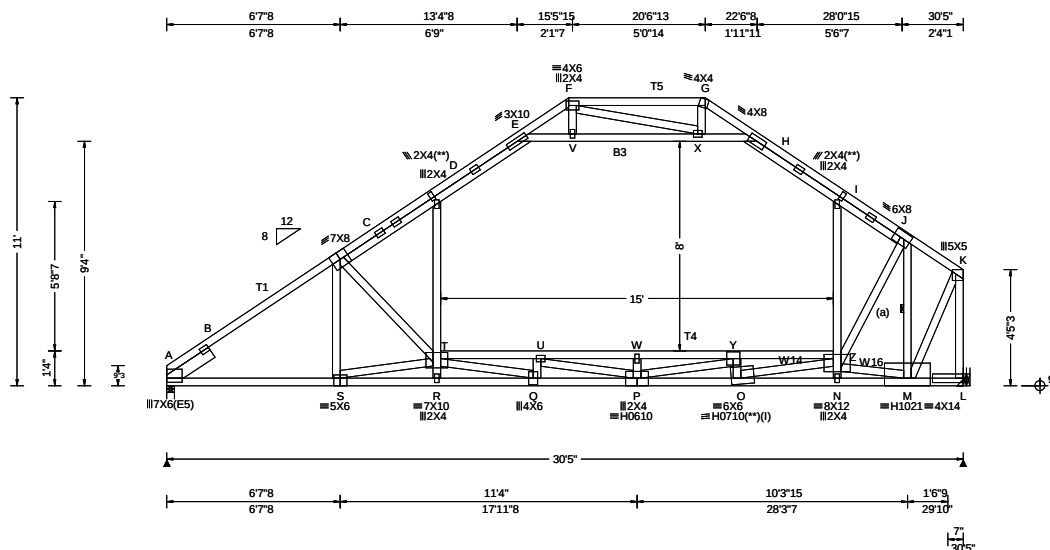
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100058 FROM:	SPEC Ply: 1 Qty: 9	Job Number: 21-5560 Miller Res Truss Label: A02	Cust: R 215 JRef: 1X172150016 T43 DrwNo: 231.22.1320.52367 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.04 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.462 Q 789 240 VERT(CL): 0.905 Q 403 180 HORZ(LL): 0.257 T - - HORZ(TL): 0.516 T - - Creep Factor: 2.0 Max TC CSI: 0.792 Max BC CSI: 0.562 Max Web CSI: 0.896 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 2069 - / - / /732 - / /161 L 2410 - / - / /687 - / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.7 (Truss) L Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP M-31; T1, T4, T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP M-31; W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.972'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.
(l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

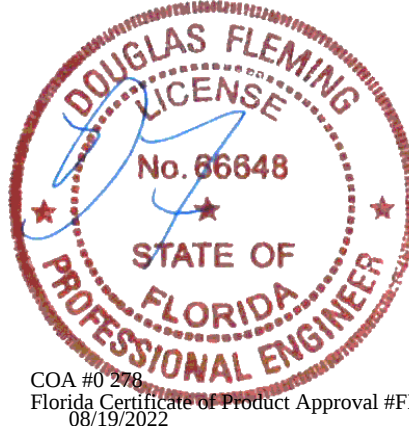
Loading
Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Chords	Tens.Comp.	Chords	Tens. Comp.
A - S	2498 -38	P - O	2696 0
S - R	4309 -147	O - N	189 -3058
R - Q	4394 -151	N - M	187 -2944
Q - P	5031 0		

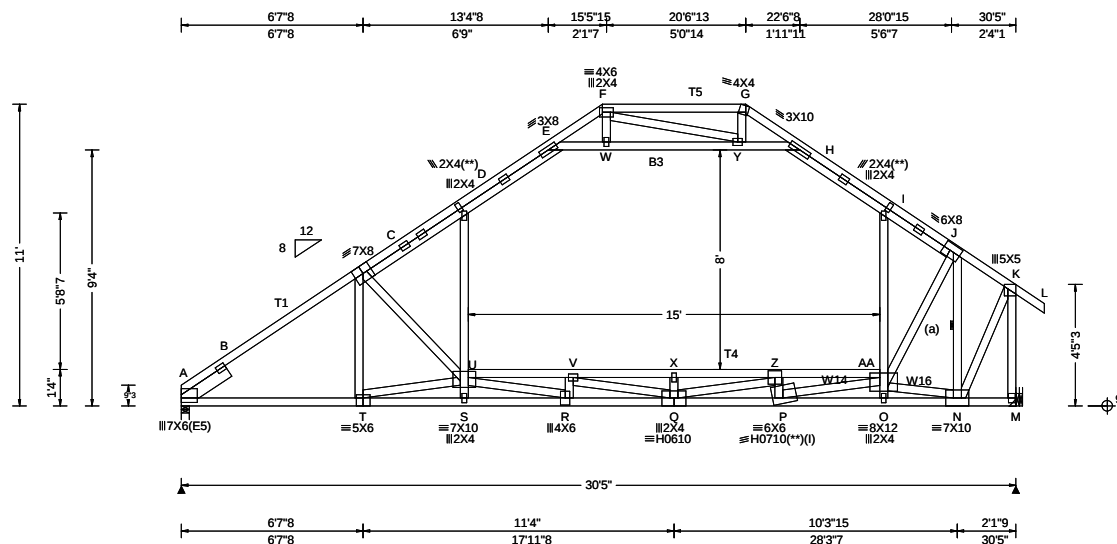
Webs	Tens.Comp.	Webs	Tens. Comp.
S - T	192 -1990	W - Y	0 -2812
C - S	545 0	X - H	156 -1708
C - T	129 -1161	Y - O	13 -1055
T - Q	1507 -369	Y - Z	277 -999
T - U	0 -3460	O - Z	5122 0
D - T	1159 0	Z - N	400 0
E - V	151 -1445	Z - I	1184 0
U - P	132 -674	Z - J	1676 0
U - W	0 -2812	Z - M	3890 -97
F - X	53 -384	J - M	0 -2741
V - X	151 -1437	M - K	1936 0
P - Y	2215 0	K - L	0 -2367



****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100046 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A03	Cust: R 215 JRef: 1X172150016 T19 DrwNo: 231.22.1320.54607 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.04 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.462 R 789 240 VERT(CL): 0.905 R 403 180 HORZ(LL): 0.257 U - - HORZ(TL): 0.516 U - - Creep Factor: 2.0 Max TC CSI: 0.792 Max BC CSI: 0.562 Max Web CSI: 0.896 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 2069 - / - / 732 - / 161 M 2410 - / - / 687 - / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.7 (Truss) M Brg Wid = - Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 3713 -82 A - B 198 -3171 D - E 1591 -3098 B - C 14 -3109 H - I 2260 -72 E - F 0 -436 I - J 872 -3096 J - K 14 -1101

Lumber
Top chord: 2x4 SP M-31; T1, T4, T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14 2x4 SP M-31; W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400F-2.0E; block length = 1.972'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties
(J) Hanger Support Required, by others

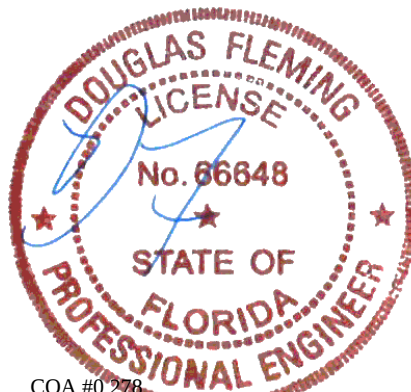
Loading
Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

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Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Chords	Tens.Comp.	Chords	Tens. Comp.
A - T	2498 -38	Q - P	2696 0
T - S	4309 -147	P - O	189 -3058
S - R	4394 -151	O - N	187 -2944
R - Q	5031 0		

Webs	Tens.Comp.	Webs	Tens. Comp.
T - U	192 -1990	X - Z	0 -2812
C - T	545 0	Y - H	156 -1708
C - U	129 -1161	Z - P	13 -1055
U - R	1507 -369	Z - AA	277 -999
U - V	0 -3460	P - AA	5122 0
D - U	1159 0	AA - O	400 0
E - W	151 -1445	AA - I	1184 0
V - Q	132 -674	AA - J	1676 0
V - X	0 -2812	AA - N	3890 -97
F - Y	53 -384	J - N	0 -2741
W - Y	151 -1437	N - K	1936 0
Q - Z	2215 0	K - M	0 -2367

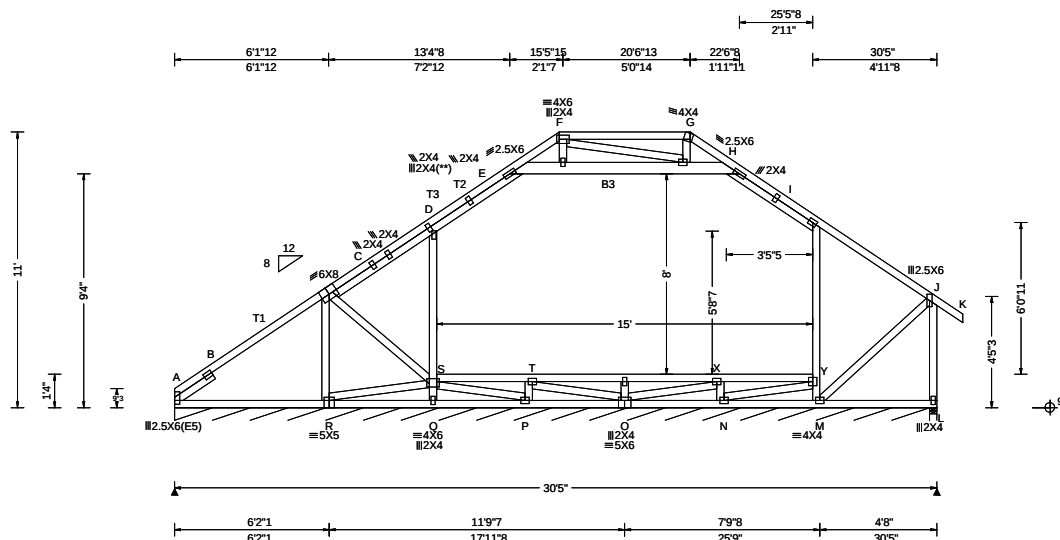


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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100134 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A04	Cust: R 215 JRef: 1X172150016 T49 DrwNo: 231.22.1320.56817 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.059 H 999 240 VERT(CL): 0.136 H 999 180 HORZ(LL): 0.029 B - - HORZ(TL): 0.060 B - - Creep Factor: 2.0 Max TC CSI: 0.594 Max BC CSI: 0.340 Max Web CSI: 0.474 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 149 -/- /- /5 -/- /- L 1288 -/- /- /- /135 /- Wind reactions based on MWFRS A Brg Wid = 361 Min Req = - L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. C - D 90 -534 F - G 65 -543 A - B 153 -839 G - H 78 -623 B - C 85 -716 H - I 307 -992 D - E 74 -573 I - J 106 -895 E - F 83 -646

Lumber

Top chord: 2x4 SP #2; T1,T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP #2; B3 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.836'

Plating Notes

All plates are 3X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

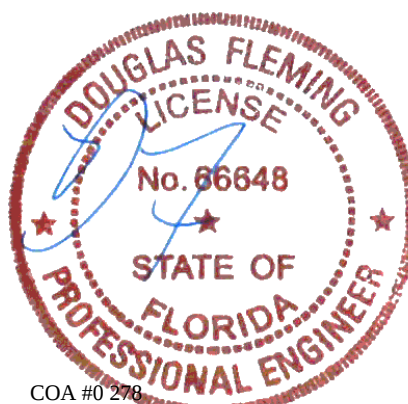
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - R	549 -62	O - N	569 -56
P - O	458 -44	N - M	674 -77

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - R	78 -574	X - N	3 -472
D - S	53 -375	Y - I	56 -482
S - Q	42 -492	Y - M	50 -564
S - T	389 -38	M - J	921 -105
P - T	7 -506	J - L	154 -1253

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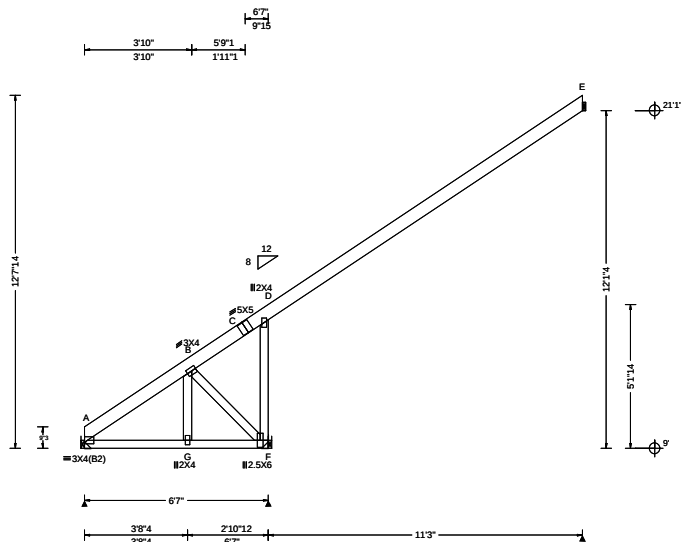
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100105 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A05	Cust: R 215 JRef: 1X172150016 T38 DrwNo: 231.22.1321.10837 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.71 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 D 999 240 VERT(CL): 0.006 D 999 180 HORZ(LL): 0.004 D - - HORZ(TL): 0.007 D - - Creep Factor: 2.0 Max TC CSI: 0.379 Max BC CSI: 0.118 Max Web CSI: 0.345 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 233 /- /- /148 /7 /178 F 824 /- /- /508 /90 /- E 291 /- /- /168 /45 /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - F Brg Wid = - Min Req = - E Brg Wid = 1.5 Min Req = - Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. D - F 228 -804

Lumber

Top chord: 2x6 SP 2400F-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

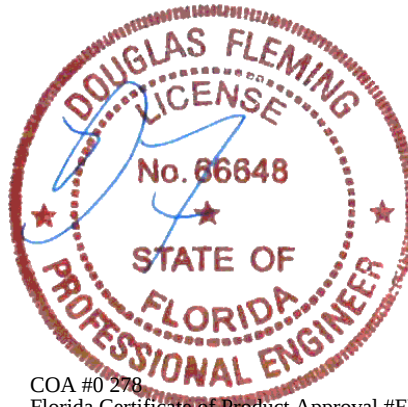
Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



COA #0 278
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08/19/2022

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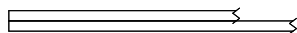
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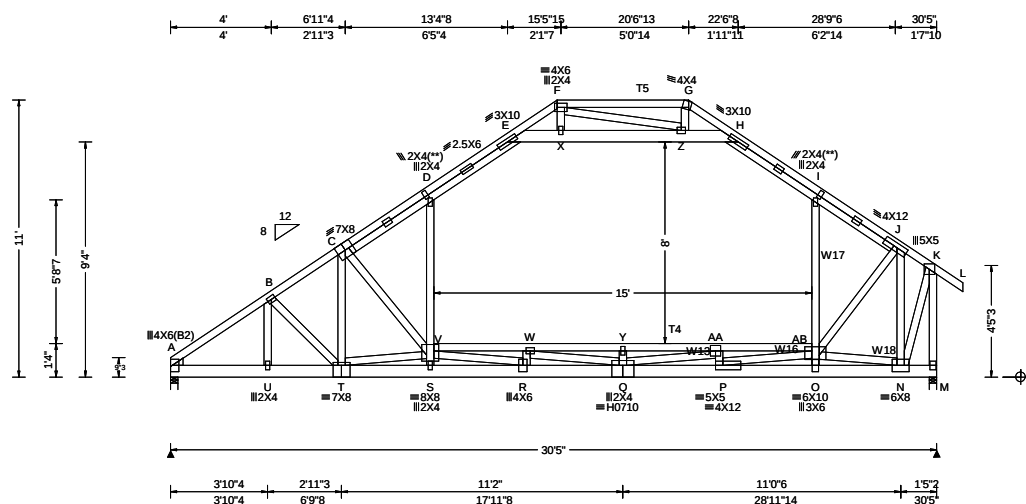
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100136 FROM:	SPEC Ply: 2 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A06	Cust: R 215 JRef: 1X172150016 T24 DrwNo: 231.22.1321.13670 KD / DF 08/19/2022
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.399 Y 913 240 VERT(CL): 0.779 Y 467 180 HORZ(LL): 0.185 D - - HORZ(TL): 0.360 D - - Creep Factor: 2.0 Max TC CSI: 0.807 Max BC CSI: 0.448 Max Web CSI: 0.879 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 3336 - / - / - /16 /85 - / - M 4252 - / - / - /6 /85 - / - Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) M Brg Wid = 3.5 Min Req = 1.8 (Truss) Bearings A & M are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
				D - E 1201 -2503 C - D 1228 -3351 I - J 999 -3058 H - I 437 -1813 A - B 69 -2436 J - K 14 -662 B - C 72 -2539

Lumber

Top chord: 2x4 SP M-31; T4,T5 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W13 2x4 SP #2; W16,W17,
W18 2x4 SP M-31;
Lt Wedge: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Plating Notes

All plates are 3X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to
scaled plate plot details for special positioning
requirements.

Loading

Attic room loading from 10-5-8 to 25-5-8: Live Load: 40
PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls:
10 PSF

Purlins

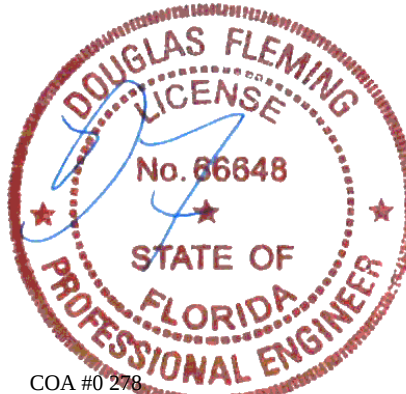
In lieu of structural panels use purlins to brace all flat
TC @ 24" oc.

Collar-tie braced with continuous lateral bracing at 24"
oc. or rigid ceiling.

THE LOADING SHOWN MUST BE APPROVED BY
THE BUILDING DESIGNER.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



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08/19/2022

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - U	1955 -52	R - Q	4424 -42
U - T	1960 -52	Q - P	2993 0
T - S	2739 -97	P - O	65 -1698
S - R	2780 -98	O - N	62 -1611

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
T - V	41 -1011	Z - H	6 -1780
C - V	27 -983	AA - P	24 -770
D - V	1314 0	AA - AB	0 -1265
V - R	1593 0	P - AB	4552 -29
V - W	4 -2842	AB - O	831 0
E - X	0 -1652	AB - I	1162 0
W - Q	788 -389	AB - J	1456 -48
W - Y	0 -3675	AB - N	2275 -73
X - Z	0 -1653	J - N	58 -2435
Q - AA	2266 -69	N - K	1658 -26
Y - AA	0 -3675	K - M	42 -1959

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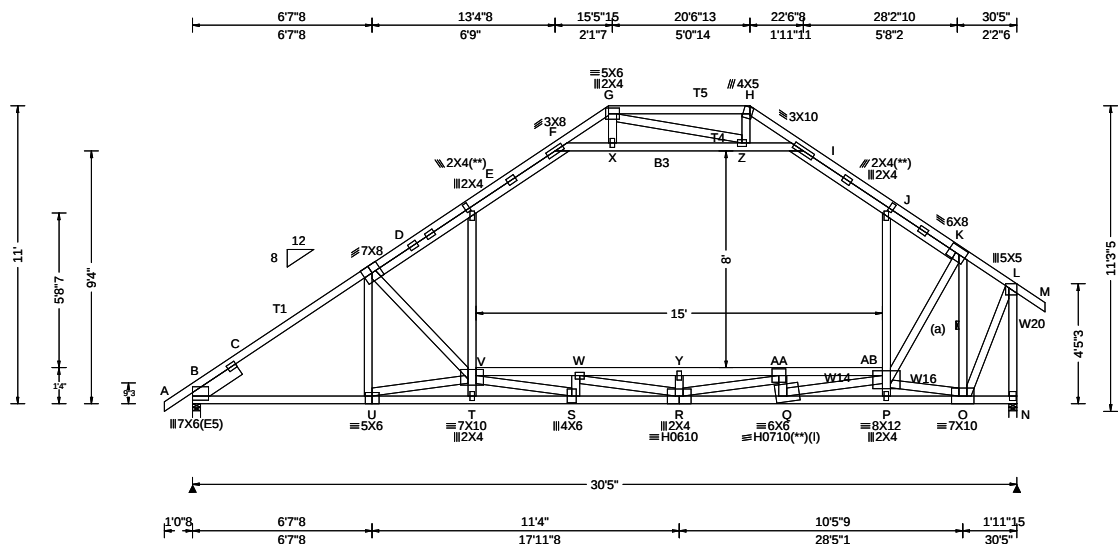
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100086 FROM:	SPEC Ply: 1 Qty: 3	Job Number: 21-5560 Miller Res Truss Label: A07	Cust: R 215 JRef: 1X172150016 T47 DrwNo: 231.22.1321.16240 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.460 S 792 240 VERT(CL): 0.896 S 407 180 HORZ(LL): 0.255 V - - HORZ(TL): 0.510 V - - Creep Factor: 2.0 Max TC CSI: 0.792 Max BC CSI: 0.559 Max Web CSI: 0.858 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 2136 - / - / - / 771 - / 181 N 2478 - / - / - / 722 - / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.8 (Truss) N Brg Wid = 3.5 Min Req = 2.1 (Truss) Bearings B & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. D - E 3711 0 B - C 191 -3162 E - F 1607 -3031 C - D 86 -3100 I - J 2271 0 F - G 202 -434 J - K 841 -3054 K - L 65 -1050

Lumber
Top chord: 2x4 SP M-31; T1,T4,T5 2x4 SP #2;
Bot chord: 2x4 SP M-31; B3 2x4 SP #2;
Webs: 2x4 SP #3; W14,W20 2x4 SP M-31;
W16 2x4 SP #2;
Lt Slider: 2x6 SP 2400F-2.0E; block length = 1.972'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.
(l) - plates so marked were sized using 0% Fabrication Tolerance, 0 degrees Rotational Tolerance, and/or zero Positioning Tolerance.
(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

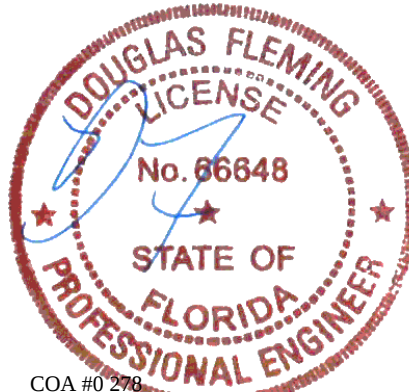
Loading
Attic room loading from 10-5-8 to 25-5-8: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.
Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.
B - U 2488 -70 R - Q 2668 0
U - T 4263 -193 Q - P 214 -3006
T - S 4347 -197 P - O 211 -2897
S - R 4993 0

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.
U - V 206 -1952 Y - AA 0 -2772
D - U 533 -1 Z - I 10 -1711
D - V 65 -1144 AA - Q 19 -1059
V - S 1508 -362 AA - AB 285 -967
V - W 0 -3418 Q - AB 5039 0
E - V 1158 0 AB - P 387 0
F - X 2 -1450 AB - J 1157 0
W - R 146 -673 AB - K 1683 -27
W - Y 0 -2772 AB - O 3796 -144
G - Z 55 -381 K - O 45 -2707
X - Z 2 -1442 O - L 1932 -28
R - AA 2211 0 L - N 84 -2435

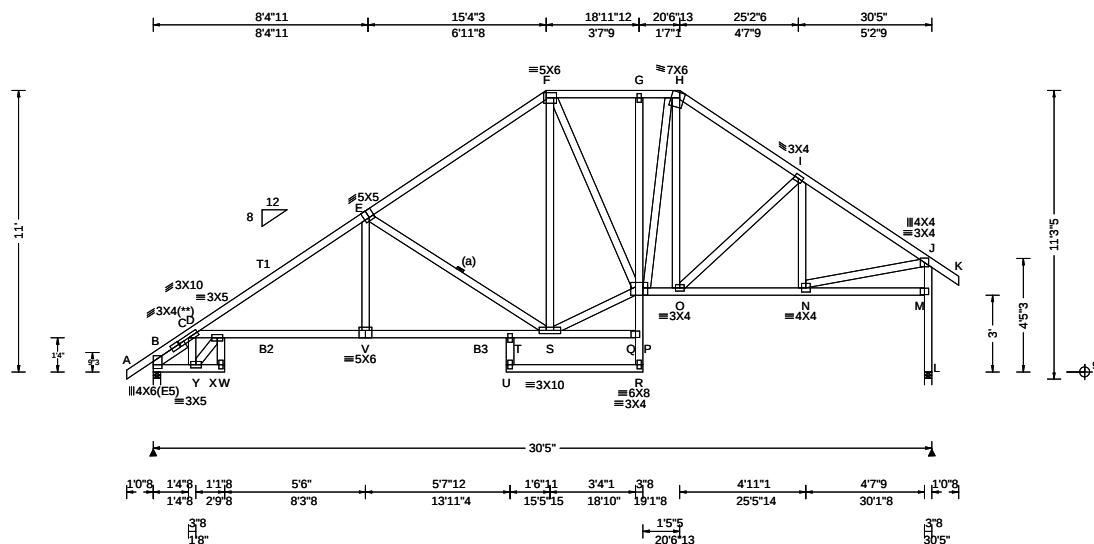


COA #0 278
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08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100072 FROM:	SPEC Ply: 1 Qty: 3	Job Number: 21-5560 Miller Res Truss Label: A08	Cust: R 215 JRef: 1X172150016 T41 DrwNo: 231.22.1321.54427 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.434 W 837 240 VERT(CL): 0.895 W 405 180 HORZ(LL): 0.208 R - - HORZ(TL): 0.429 R - - Creep Factor: 2.0 Max TC CSI: 0.800 Max BC CSI: 0.902 Max Web CSI: 0.842 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1338 -/- /- /768 -/- /181 L 1345 -/- /- /720 -/- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings B & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 62 -1433 F - G 166 -1188 C - D 55 -1383 G - H 166 -1189 D - E 117 -2238 H - I 166 -1415 E - F 165 -1376 I - J 105 -1509

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2, B3 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

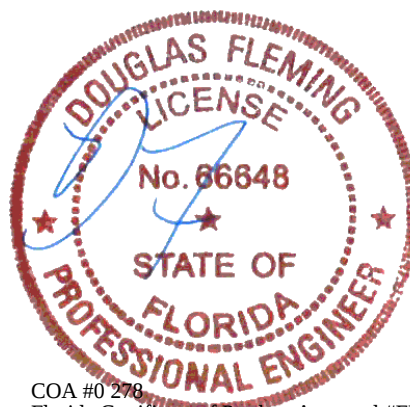
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C. Max. including a lateral brace at chord ends.

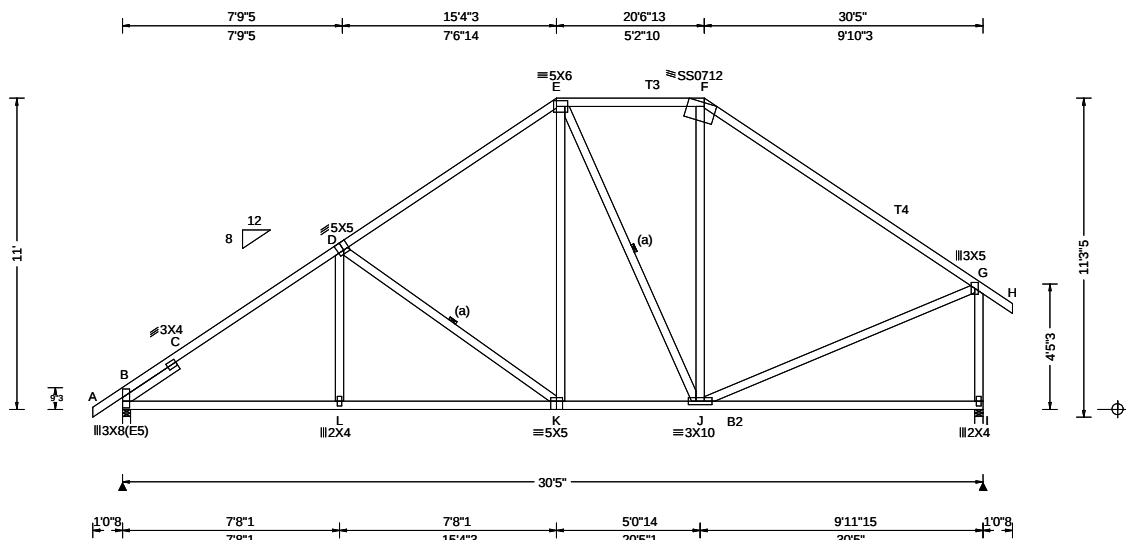


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08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99855 FROM:	SPEC Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: A09	Cust: R 215 JRef: 1X172150016 T12 DrwNo: 231.22.1321.56260 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/def L/# VERT(LL): 0.100 C 999 240 VERT(CL): 0.187 C 999 180 HORZ(LL): 0.070 C - - HORZ(TL): 0.131 C - - Creep Factor: 2.0 Max TC CSI: 0.801 Max BC CSI: 0.864 Max Web CSI: 0.522 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1540 - / - / 771 - / 180 I 1458 - / - / 721 - / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.8 (Truss) I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 210 - 2172 E - F 160 - 967 C - D 85 - 2074 F - G 135 - 1311 D - E 159 - 1436

Lumber

Top chord: 2x4 SP #2; T3, T4 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 2.331'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins

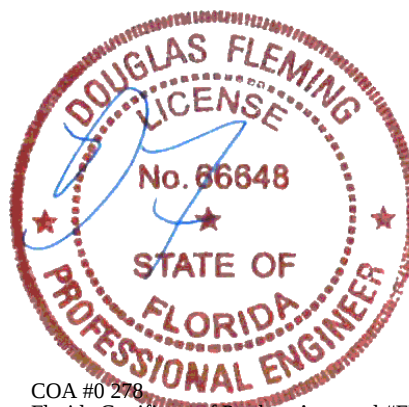
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

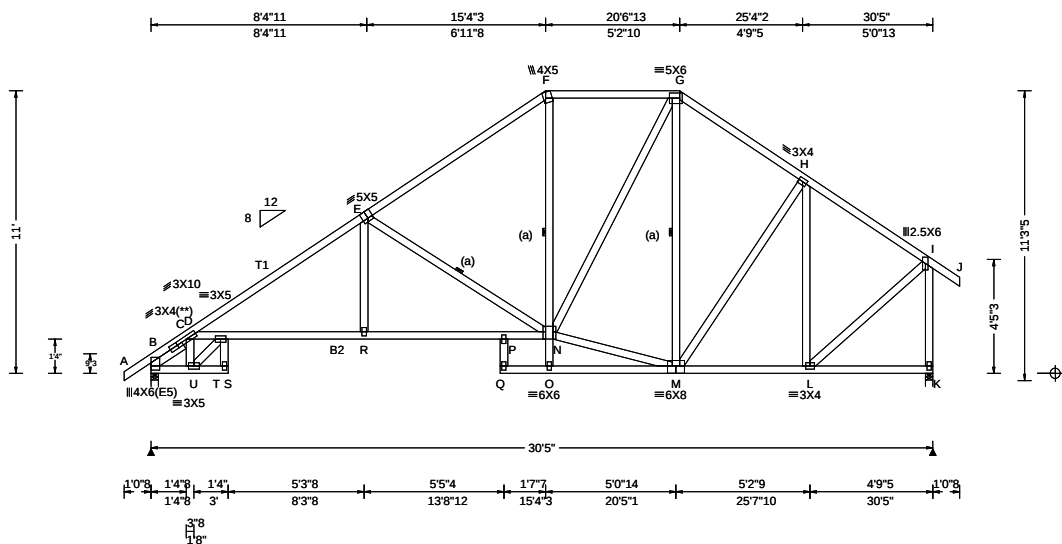


COA #0 278
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08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100073 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A10	Cust: R 215 JRef: 1X172150016 T18 DrwNo: 231.22.1322.00107 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.449 S 813 240 VERT(CL): 0.929 S 393 180 HORZ(LL): 0.218 L - - HORZ(TL): 0.451 L - - Creep Factor: 2.0 Max TC CSI: 0.777 Max BC CSI: 0.899 Max Web CSI: 0.560 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1345 -/- /- /771 -/- /181 K 1345 -/- /- /722 -/- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 64 -1457 F - G 165 -1050 C - D 58 -1406 G - H 167 -1067 D - E 118 -2242 H - I 96 -943 E - F 163 -1403

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

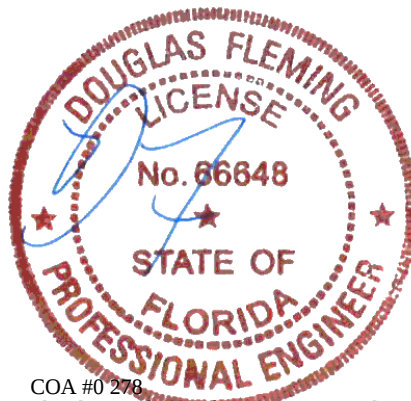
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C. Max. including a lateral brace at chord ends.

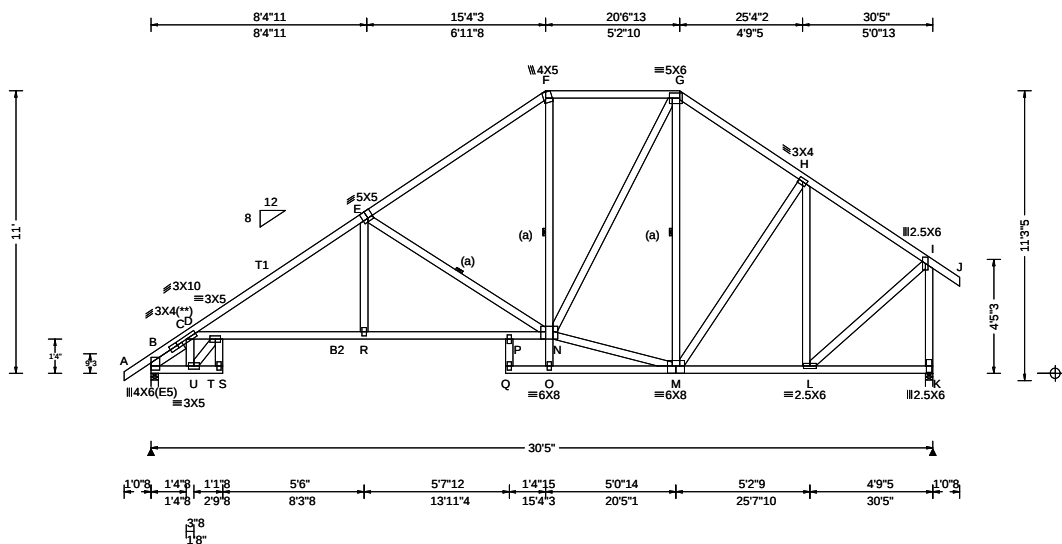


COA #0278
Florida Certificate of Product Approval #FL1999
08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100074 FROM:	SPEC Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: A11	Cust: R 215 JRef: 1X172150016 T40 DrwNo: 231.22.1322.15097 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.464 S 786 240 VERT(CL): 0.928 S 393 180 HORZ(LL): 0.241 L - - HORZ(TL): 0.482 L - - Creep Factor: 2.0 Max TC CSI: 0.795 Max BC CSI: 0.919 Max Web CSI: 0.528 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1363 -/- /- /771 -/- /181 K 1417 -/- /- /722 -/- /- Non-Gravity Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = 3.5 Min Req = 1.7 (Truss) Bearings B & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 63 -1457 F - G 165 -1080 C - D 56 -1406 G - H 167 -1111 D - E 117 -2280 H - I 96 -1005 E - F 163 -1440

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

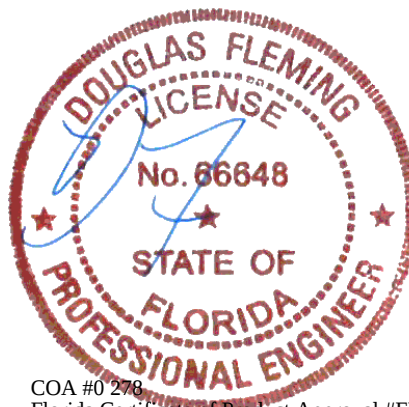
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.

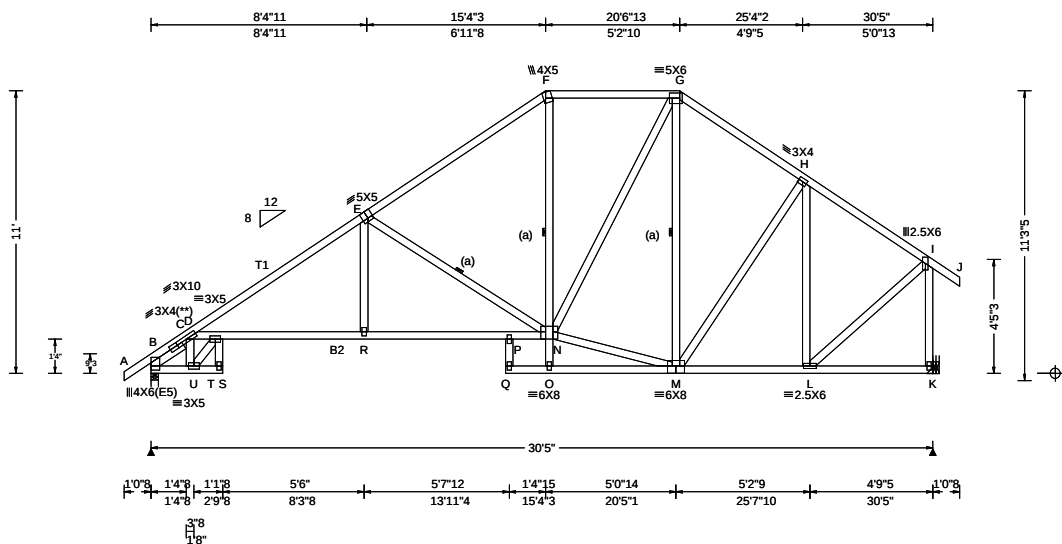


COA #0 278
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08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100075 FROM:	COMN	Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: A12	Cust: R 215 JRef: 1X172150016 T17 DrwNo: 231.22.1322.21807 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.04 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.464 S 786 240 VERT(CL): 0.928 S 393 180 HORZ(LL): 0.241 L - - HORZ(TL): 0.482 L - - Creep Factor: 2.0 Max TC CSI: 0.795 Max BC CSI: 0.919 Max Web CSI: 0.528 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1363 -/- /- /771 -/- /181 K 1417 -/- /- /722 -/- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.6 (Truss) K Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 63 -1457 F - G 165 -1080 C - D 56 -1406 G - H 167 -1111 D - E 117 -2280 H - I 96 -1005 E - F 163 -1440

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

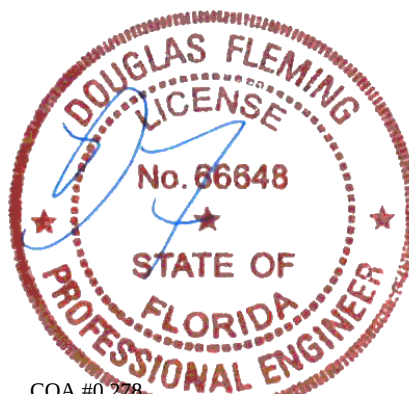
Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Note: Laterally brace bottom chord above filler at 20" O.C.Max. including a lateral brace at chord ends.

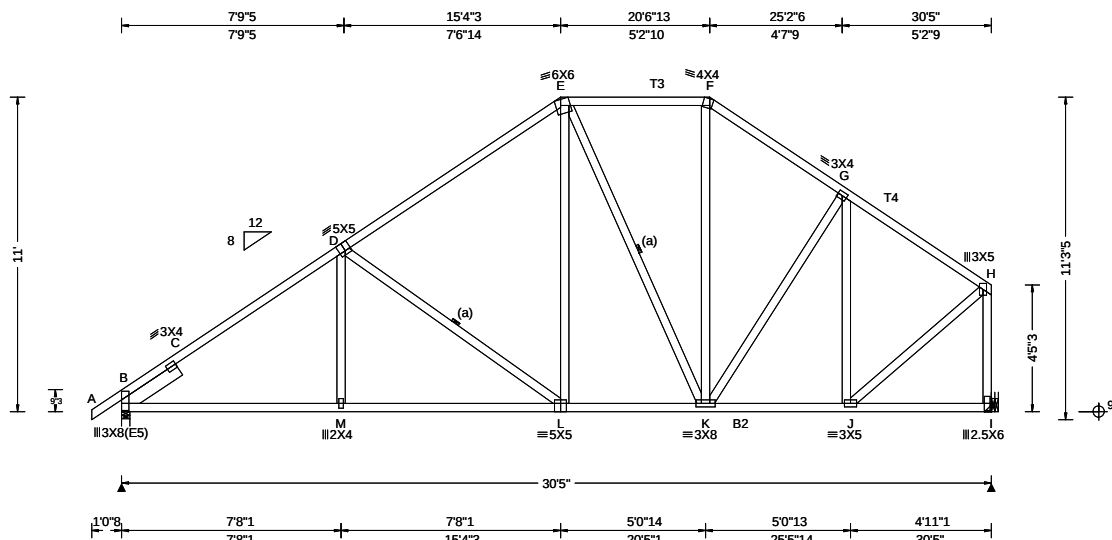


COA #0 278
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100076 FROM:	COMN Ply: 1 Qty: 9	Job Number: 21-5560 Miller Res Truss Label: A13	Cust: R 215 JRef: 1X172150016 T9 DrwNo: 231.22.1322.24473 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.04 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.075 C 999 240 VERT(CL): 0.138 C 999 180 HORZ(LL): 0.053 C - - HORZ(TL): 0.097 C - - Creep Factor: 2.0 Max TC CSI: 0.427 Max BC CSI: 0.426 Max Web CSI: 0.604 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1560 - / - / 772 - / 167 I 1461 - / - / 678 - / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) I Brg Wid = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 297 -2230 E - F 204 -958 C - D 157 -2106 F - G 215 -1239 D - E 213 -1472 G - H 125 -1109

Lumber
Top chord: 2x4 SP M-31; T3, T4 2x4 SP #2;
Bot chord: 2x4 SP M-31; B2 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 2.331'

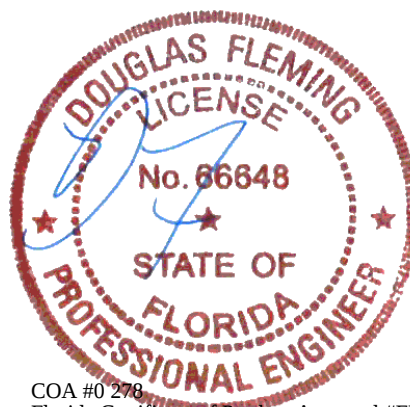
Bracing
(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties
(J) Hanger Support Required, by others

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Purlins
In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

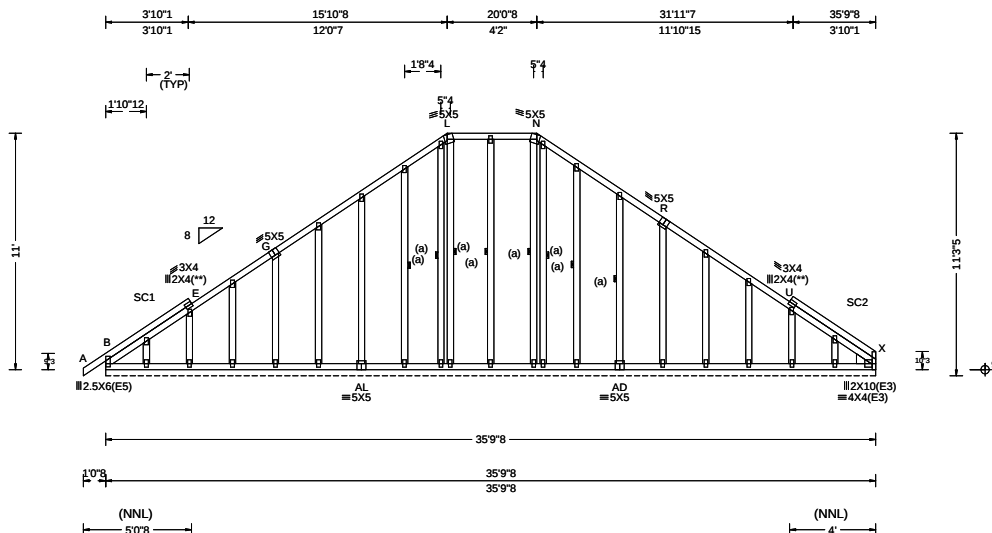


COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100077 FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A14	Cust: R 215 JRef: 1X172150016 T6 DrwNo: 231.22.1322.26287 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.58 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 M 999 240 VERT(CL): 0.003 M 999 180 HORZ(LL): 0.005 U - - HORZ(TL): 0.007 U - - Creep Factor: 2.0 Max TC CSI: 0.338 Max BC CSI: 0.037 Max Web CSI: 0.163 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity X* 86 /- /- /54 /1 /8 Wind reactions based on MWFRS X Brg Wid = 429 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

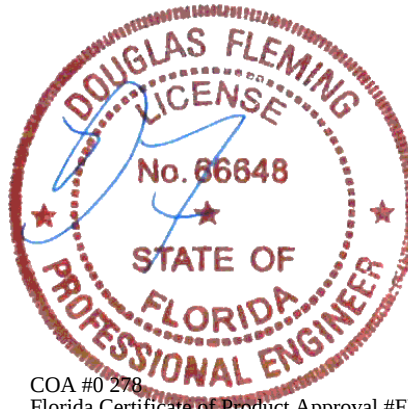
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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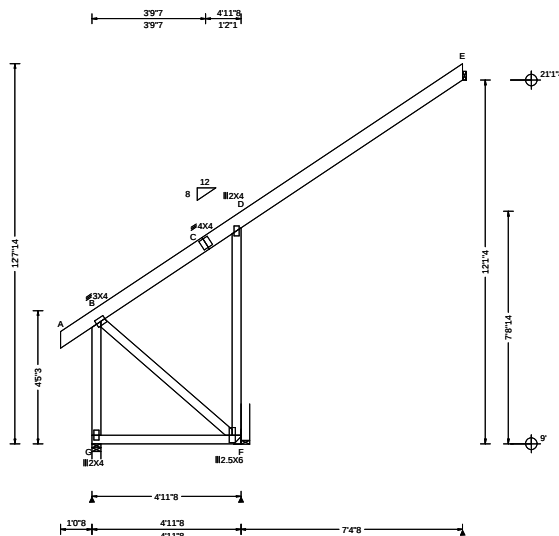
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100052 FROM:	MONO Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: A15	Cust: R 215 JRef: 1X172150016 T10 DrwNo: 231.22.1322.29387 KD / DF 08/19/2022
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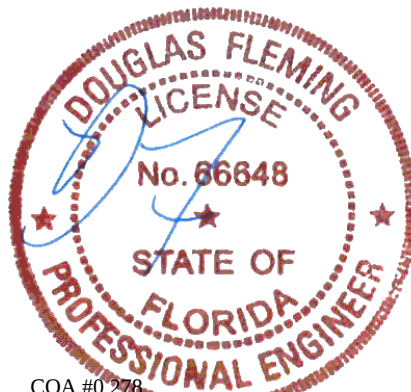


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 17.20 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 D 999 240 VERT(CL): 0.006 D 999 180 HORZ(LL): -0.006 D - - HORZ(TL): 0.007 D - - Creep Factor: 2.0 Max TC CSI: 0.142 Max BC CSI: 0.292 Max Web CSI: 0.495 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 249 -/- /- /254 /22 /180 F 544 -/- /- /406 /259 -/- E 199 -/- /- /112 /54 -/- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = - Min Req = - E Brg Wid = 1.5 Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp.

Lumber
Top chord: 2x6 SP 2400F-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

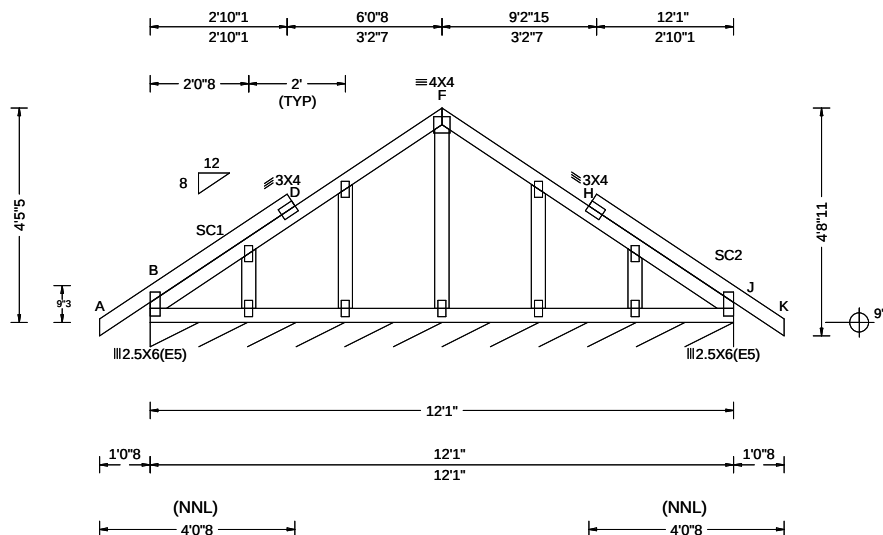


COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100000 FROM:	GABL Qty: 2	Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: B01	Cust: R 215 JRef: 1X172150016 T8 DrwNo: 231.22.1322.31243 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.002 H 999 240 VERT(CL): 0.004 H 999 180 HORZ(LL): -0.001 H - - HORZ(TL): 0.002 H - - Creep Factor: 2.0 Max TC CSI: 0.140 Max BC CSI: 0.032 Max Web CSI: 0.041 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J* 95 /- /- /49 /- /7 Wind reactions based on MWFRS J Brg Wid = 145 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

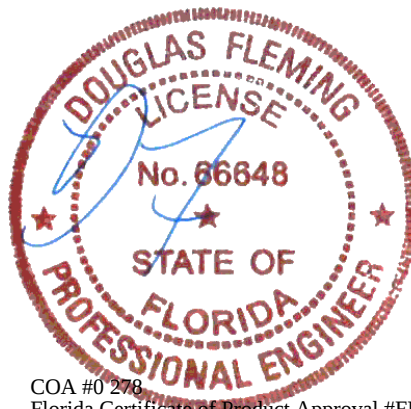
End verticals not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.



COA #0 278
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08/19/2022

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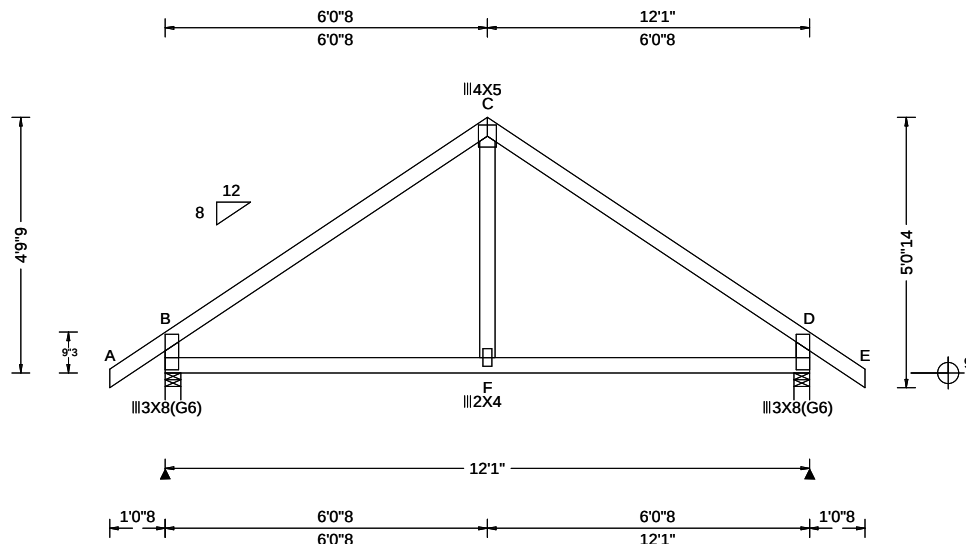
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99787 FROM:	COMN Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: B02	Cust: R 215 JRef: 1X172150016 T22 DrwNo: 231.22.1322.32520 KD / DF 08/19/2022
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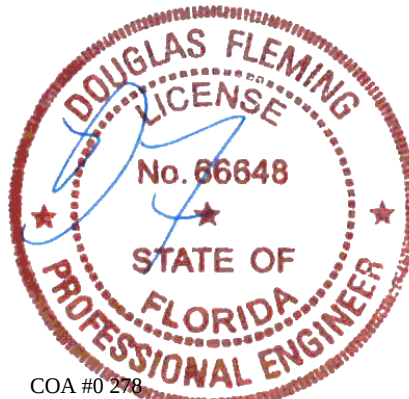
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 F 999 240 VERT(CL): 0.008 F 999 180 HORZ(LL): 0.004 D - - HORZ(TL): 0.008 D - - Creep Factor: 2.0 Max TC CSI: 0.365 Max BC CSI: 0.334 Max Web CSI: 0.099 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 574 /- /- /329 /- /85 D 574 /- /- /291 /- /- Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 139 -578 C - D 139 -578

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3; Rt Stub Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

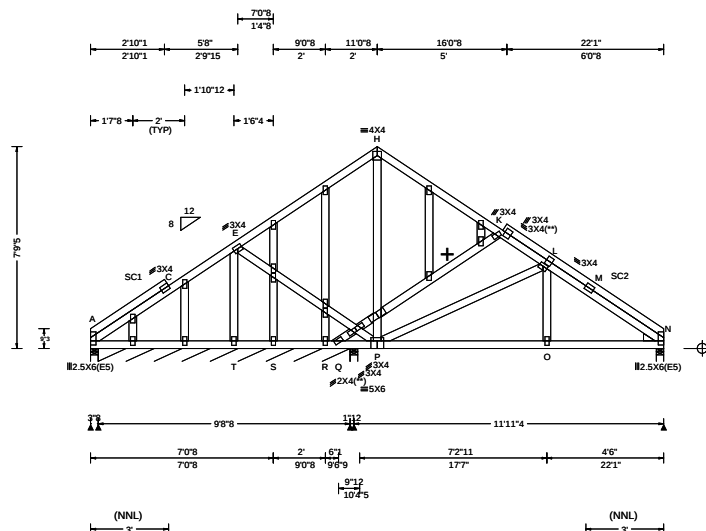


COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100009 FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B03	Cust: R 215 JRef: 1X172150016 T7 DrwNo: 231.22.1324.10600 KD / DF 08/19/2022
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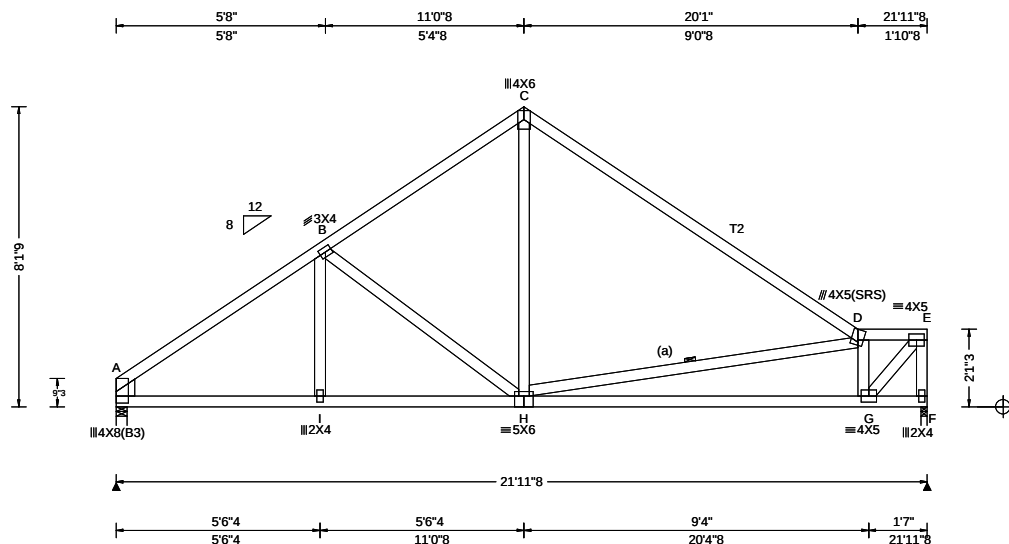
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.029 M 999 240 VERT(CL): 0.060 M 999 180 HORZ(LL): -0.013 M - - HORZ(TL): 0.027 M - - Creep Factor: 2.0 Max TC CSI: 0.329 Max BC CSI: 0.373 Max Web CSI: 0.315 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 69 /-30 /- /69 /28 /226 A* 110 /- /- /106 /11 /- Q 188 /- /- /160 /- /- N 563 /- /- /505 /- /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) A Brg Wid = 116 Min Req = - Q Brg Wid = 3.5 Min Req = 1.5 (Truss) N Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A, A, Q, & N are a rigid surface. Members not listed have forces less than 375#

Lumber	Additional Notes	Maximum Top Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2; Rt Stub Wedge: 2x4 SP #3;	See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements. Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.	Chords Tens.Comp. Chords Tens. Comp. K - L 24 -386 M - N 0 -769 L - M 0 -666
Plating Notes	Maximum Bot Chord Forces Per Ply (lbs)	Maximum Gable Forces Per Ply (lbs)
All plates are 2X4 except as noted. (**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.	Chords Tens.Comp. Chords Tens. Comp. P - O 582 0 O - N 586 0	Gables Tens.Comp. T - E 0 -404

Loading	Wind	Professional Engineer Seal
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.	Wind loads based on MWFRS with additional C&C member design. Wind loading based on both gable and hip roof types. + Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.	 COA #0 278 Florida Certificate of Product Approval #FL1999 08/19/2022

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org	ALPINE AN ITW COMPANY 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025
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SEQN: 99798 FROM:	COMN Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B04	Cust: R 215 JRef: 1X172150016 T1 DrwNo: 231.22.1324.13013 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.033 H 999 240 VERT(CL): 0.068 H 999 180 HORZ(LL): 0.016 G - - HORZ(TL): 0.034 G - - Creep Factor: 2.0 Max TC CSI: 0.631 Max BC CSI: 0.731 Max Web CSI: 0.551 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 923 - / - /516 - /116 F 923 - / - /493 - /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 2.0 Min Req = 1.5 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 114 -1250 C - D 131 -1007 B - C 150 -922 D - E 93 -954

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

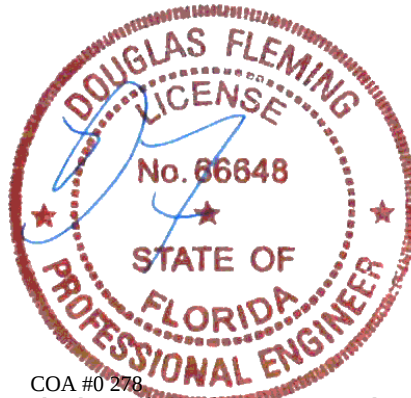
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

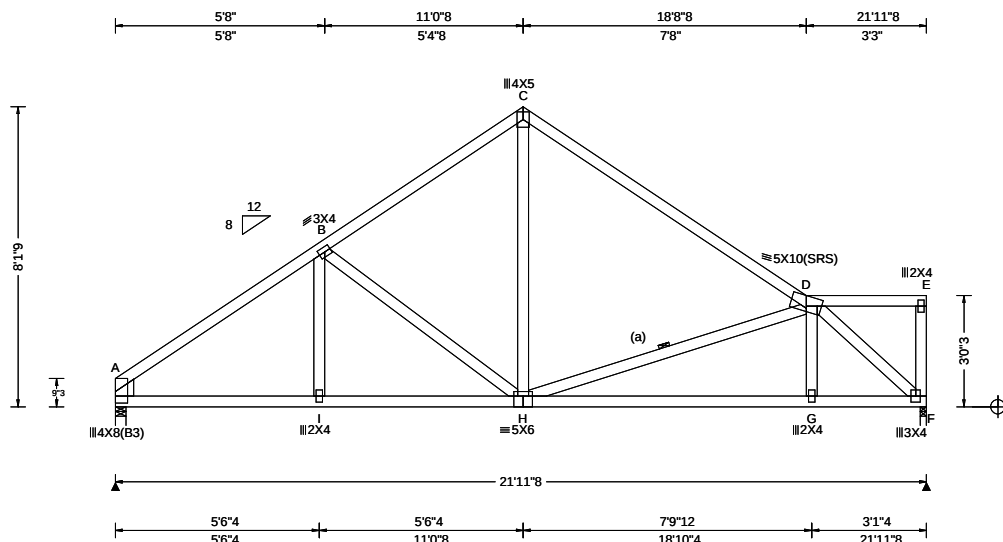


COA #0 278
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AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99801 FROM:	SPEC	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B05	Cust: R 215 JRef: 1X172150016 T11 DrwNo: 231.22.1324.19303 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.035 H 999 240 VERT(CL): 0.073 H 999 180 HORZ(LL): 0.017 F - - HORZ(TL): 0.035 F - - Creep Factor: 2.0 Max TC CSI: 0.670 Max BC CSI: 0.555 Max Web CSI: 0.447 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 923 - / - / 517 - / 115 F 923 - / - / 482 - / - Non-Gravity Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 2.0 Min Req = 1.5 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 24 - 1255 C - D 67 - 966 B - C 74 - 916

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

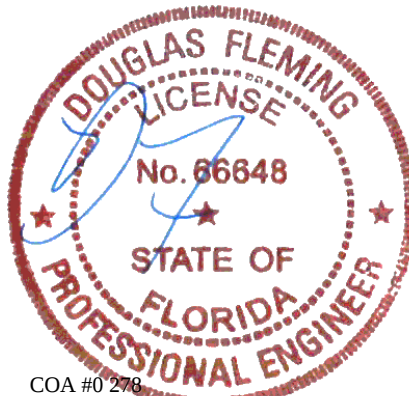
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



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08/19/2022

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - I	950 -19	H - G	1044 -52
I - H	948 -20	G - F	1050 -48

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - H	531 0	D - F	61 -1367

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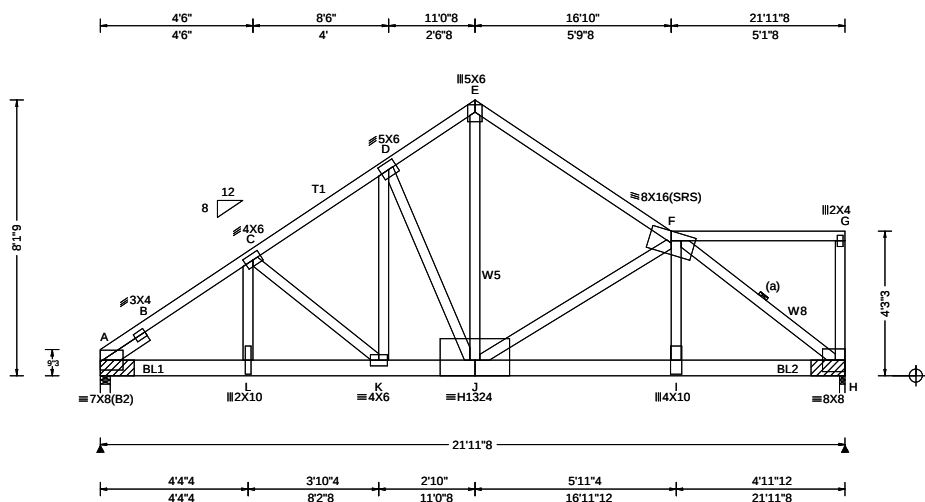
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.116 K 999 240 VERT(CL): 0.230 K 999 180 HORZ(LL): 0.041 C - - HORZ(TL): 0.081 C - - Creep Factor: 2.0 Max TC CSI: 0.565 Max BC CSI: 0.651 Max Web CSI: 0.868 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 9178 -/- /- /- /574 -/ H 8295 -/- /- /- /1104 -/ Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = - H Brg Wid = 2.0 Min Req = - Bearings A & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 632 -5704 D - E 501 -3869 B - C 627 -5673 E - F 506 -3910 C - D 588 -4645

Lumber

Top chord: 2x4 SP #2; T1 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5,W8 2x4 SP M-31;
Lt Slider: 2x4 SP #3; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 4.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at 0.00 to 64 plf at 21.96
BC: From 10 plf at 0.00 to 10 plf at 21.96
BC: 1348 lb Conc. Load at 0.35, 2.35
BC: 1461 lb Conc. Load at 4.35, 6.35, 8.35, 10.35
12.35, 14.35, 16.35, 18.35, 20.02

Purlins

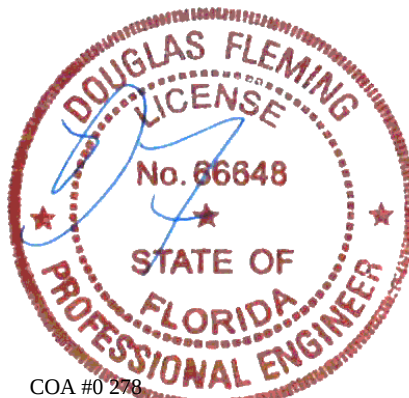
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Bearing Block(s)

Brg blocks: 0.131"x3", min. nails
brg x-loc #blocks length/blk #nails/blk wall plate
1 0.000' 1 12" 6 Rigid Surface
2 21.792' 2 12" 14 Rigid Surface
Brg block to be same size and species as chord.
Refer to drawing C9NAILSP1014 for more information.



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08/19/2022

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - L	4670 -509	J - I	4685 -623
L - K	4592 -506	I - H	4771 -635
K - J	3782 -480		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
L - C	1241 -43	E - J	4096 -530
C - K	26 -999	J - F	248 -1764
K - D	1623 -181	F - I	2279 -319
D - J	158 -1399	F - H	794 -5967

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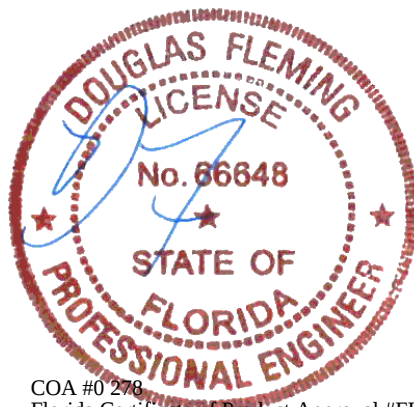


155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100040	SPEC	Ply: 2	Job Number: 21-5560	Cust: R 215 JRef: 1X172150016 T26
FROM:		Qty: 1	Miller Res	DrwNo: 231.22.1326.05270
Page 2 of 2			Truss Label: B06	KD / DF 08/19/2022

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)



COA #0 278
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08/19/2022

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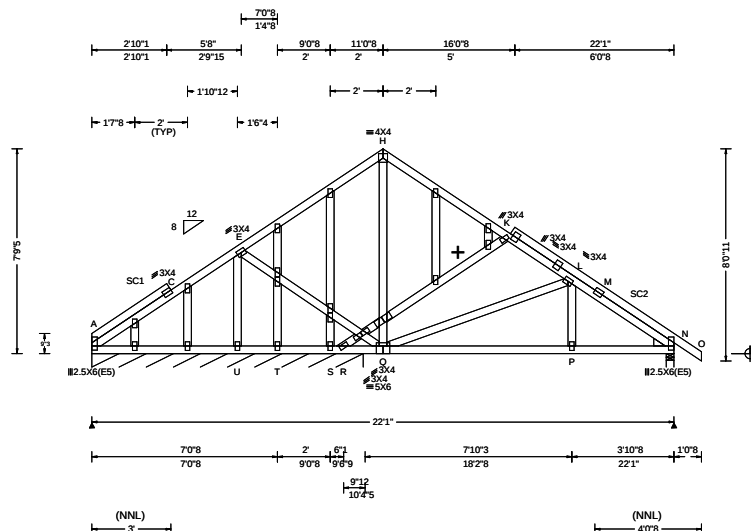
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99998 FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B07	Cust: R 215 JRef: 1X172150016 T5 DrwNo: 231.22.1326.29323 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.023 999 240 VERT(CL): 0.047 999 180 HORZ(LL): -0.006 - - HORZ(TL): 0.012 - - Creep Factor: 2.0 Max TC CSI: 0.209 Max BC CSI: 0.427 Max Web CSI: 0.337 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 124 /- /- /98 /- /15 N 647 /- /- /387 /- /- Wind reactions based on MWFRS A Brg Wid = 123 Min Req = - N Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. K - L 25 -445 M - N 0 -812 L - M 0 -709

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;
Rt Stub Wedge: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notched area using 3x6.

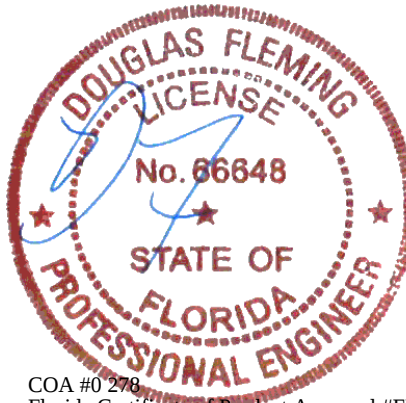
+ Member to be laterally braced for horizontal wind loads. bracing system to be designed and furnished by others.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
Q - P	621	0	P - N 625 0

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
U - E	0 -436



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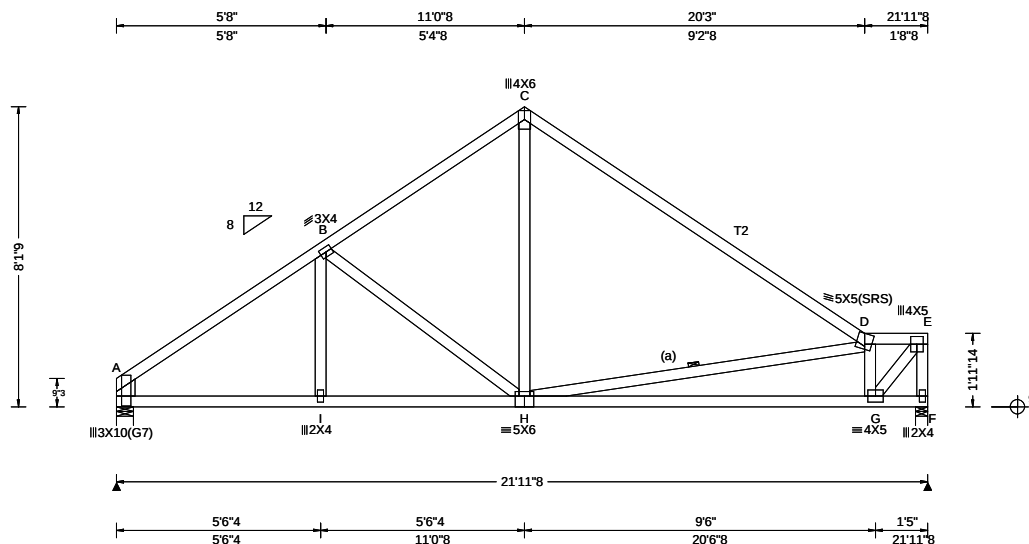
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99776 FROM:	COMN Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B08	Cust: R 215 JRef: 1X172150016 T13 DrwNo: 231.22.1326.30887 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.032 H 999 240 VERT(CL): 0.067 H 999 180 HORZ(LL): 0.016 G - - HORZ(TL): 0.034 G - - Creep Factor: 2.0 Max TC CSI: 0.663 Max BC CSI: 0.756 Max Web CSI: 0.563 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 923 - / - /516 - /116 F 923 - / - /494 - /- Non-Gravity Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 114 -1250 C - D 130 -1013 B - C 150 -923 D - E 91 -948

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

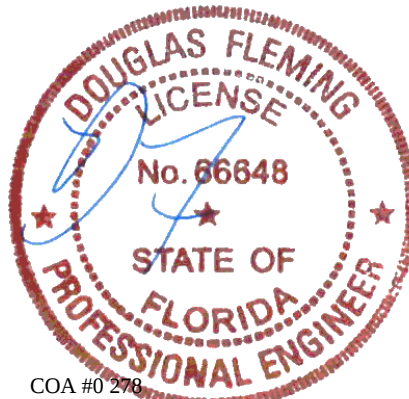
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



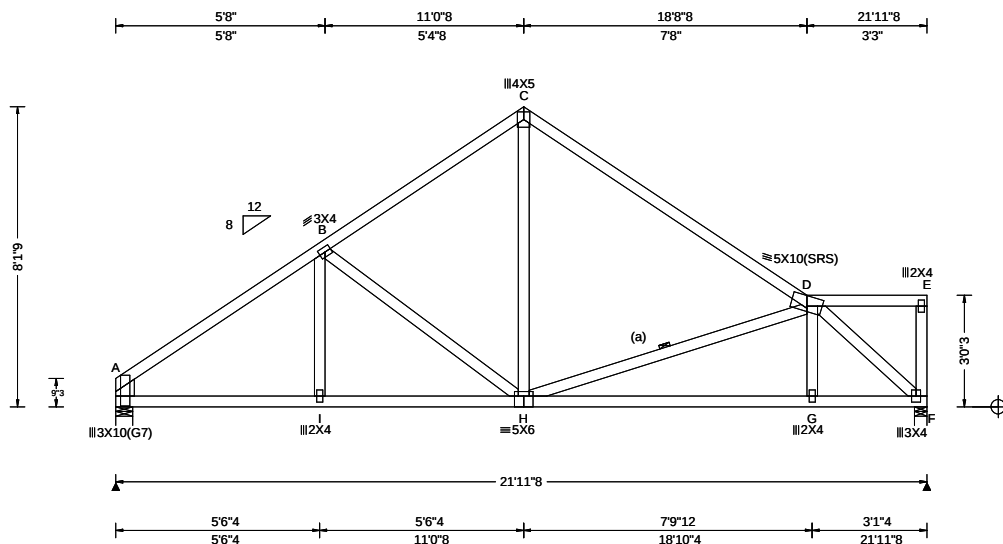
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99781 FROM:	SPEC	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: B09	Cust: R 215 JRef: 1X172150016 T14 DrwNo: 231.22.1326.32440 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.035 H 999 240 VERT(CL): 0.073 H 999 180 HORZ(LL): 0.017 F - - HORZ(TL): 0.035 F - - Creep Factor: 2.0 Max TC CSI: 0.670 Max BC CSI: 0.555 Max Web CSI: 0.447 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 923 - / - / 517 - / 115 F 923 - / - / 482 - / - Non-Gravity Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings A & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 24 - 1255 C - D 67 - 966 B - C 74 - 916

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x6 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Purlins

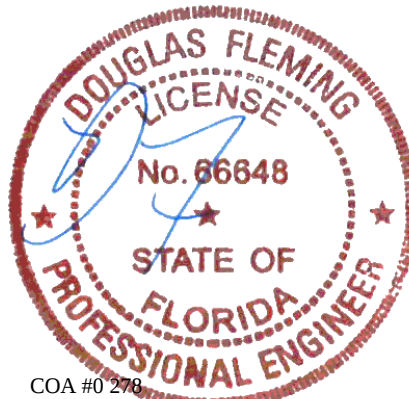
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



COA #0 278

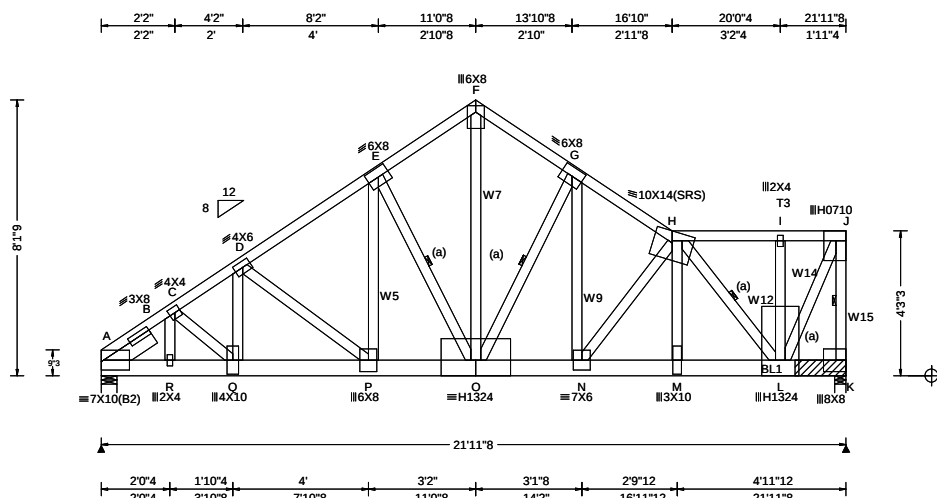
Florida Certificate of Product Approval #FL1999
08/19/2022

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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/def L/# VERT(LL): 0.181 N 999 240 VERT(CL): 0.360 N 724 180 HORZ(LL): 0.062 E - - HORZ(TL): 0.124 E - - Creep Factor: 2.0 Max TC CSI: 0.608 Max BC CSI: 0.802 Max Web CSI: 0.870 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL A 12954 /- /- /345 /- /- K 12775 /- /- /341 /- /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 5.4 (Truss) K Brg Wid = 4.0 Min Req = - Bearings A & K are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP M-31; T3 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W5,W9,W12,W15 2x4 SP #2; W7,
W14 2x4 SP M-31;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.605'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Nailnote
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 3.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
-----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at 0.00 to 64 plf at 21.96
BC: From 10 plf at 0.00 to 10 plf at 21.96
BC: 2410 lb Conc. Load at 2.02, 4.02, 6.02, 8.02
10.02,12.02,14.02,16.02,18.02,20.02

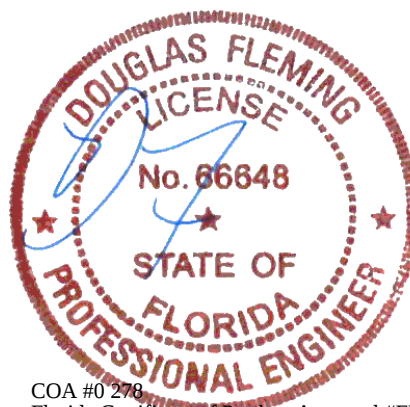
Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Bearing Block(s)
Brg blocks: 0.131"x3", min. nails
brg x-loc #blocks length/blk #nails/blk wall plate
2 21.625' 1 18" 24 Rigid Surface
Brg block to be same size and species as chord.
Refer to drawing CENNAISP1014 for more information.

Chords	Tens.Comp.	Chords	Tens. Comp.
A - R	6502 0	O - N	6060 0
R - Q	6431 0	N - M	6946 0
Q - P	7171 0	M - L	7008 0
P - O	6083 0		

Webs	Tens.Comp.	Webs	Tens. Comp.
C - Q	1154 0	G - N	2910 0
Q - D	1251 0	N - H	0 - 1377
D - P	0 - 1263	H - M	1647 0
P - E	2960 0	H - L	0 - 6062
E - O	0 - 2437	L - J	7070 0
F - O	6400 0	J - K	0 - 6355
O - G	0 - 2424		



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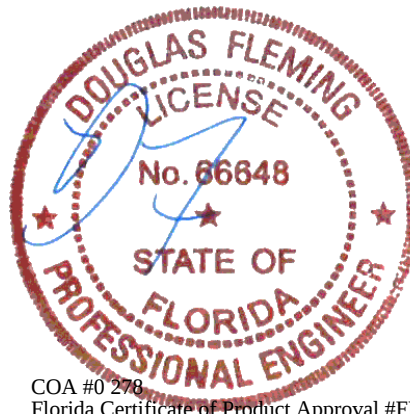
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100048	SPEC	Ply: 2	Job Number: 21-5560	Cust: R 215 JRef: 1X172150016 T31
FROM:		Qty: 1	Miller Res	DrwNo: 231.22.1326.43033
Page 2 of 2			Truss Label: B10	KD / DF 08/19/2022

Blocking

Apply additional nailing over the following bearings with fasteners at 4" oc both perpendicular and parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)



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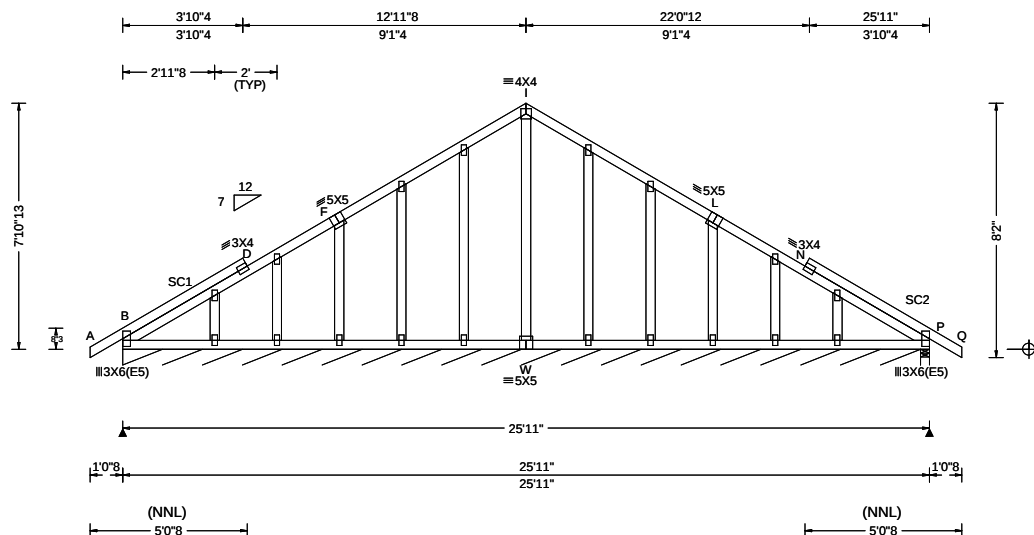
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99768 FROM:	GABL Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: C01	Cust: R 215 JRef: 1X172150016 T15 DrwNo: 231.22.1326.44767 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 N 999 240 VERT(CL): 0.006 N 999 180 HORZ(LL): 0.002 N - - HORZ(TL): 0.003 D - - Creep Factor: 2.0 Max TC CSI: 0.293 Max BC CSI: 0.056 Max Web CSI: 0.116 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 80 - / - / 42 - / 5 P 231 - / - / 121 - / - Wind reactions based on MWFRS B Brg Wid = 307 Min Req = - P Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & P are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

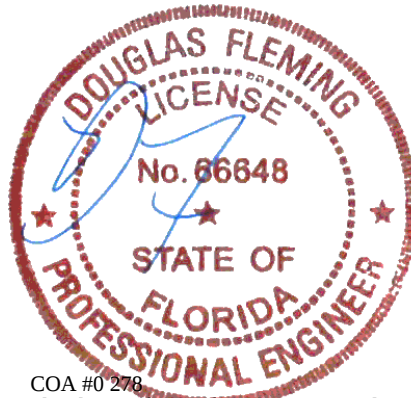
Wind

Wind loads based on MWFRS with additional C&C member design.
Left end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.



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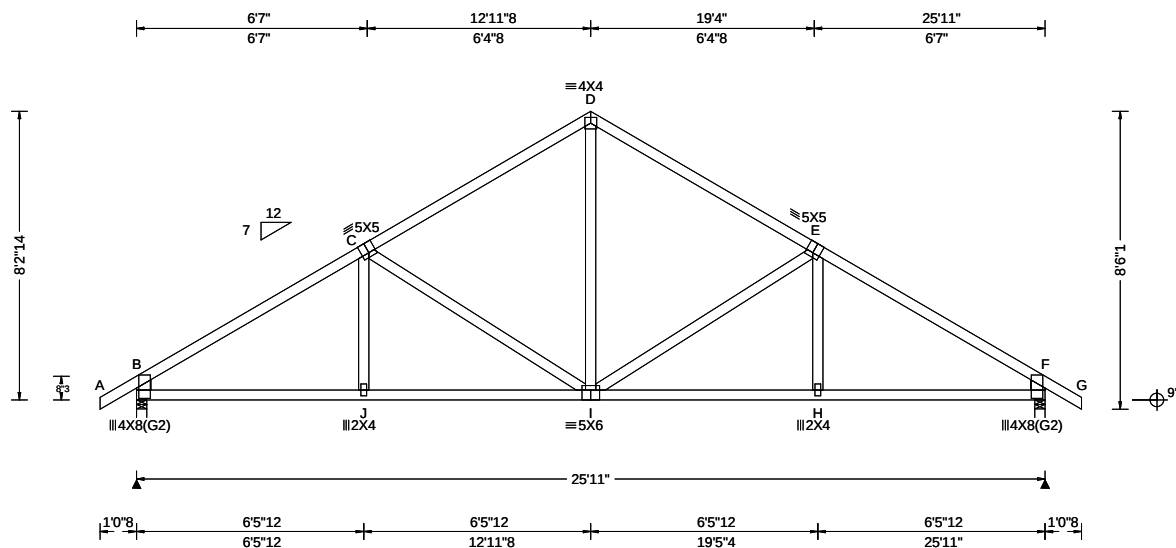
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99764 FROM:	COMN	Ply: 1 Qty: 6	Job Number: 21-5560 Miller Res Truss Label: C02	Cust: R 215 JRef: 1X172150016 T28 DrwNo: 231.22.1326.46440 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.053 I 999 240 VERT(CL): 0.108 I 999 180 HORZ(LL): 0.026 F - - HORZ(TL): 0.053 F - - Creep Factor: 2.0 Max TC CSI: 0.438 Max BC CSI: 0.479 Max Web CSI: 0.546 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1143 - / - / - / 636 - / 130 F 1143 - / - / - / 636 - / - Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 111 -1641 D - E 143 -1157 C - D 142 -1157 E - F 111 -1641

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3; Rt Stub Wedge: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

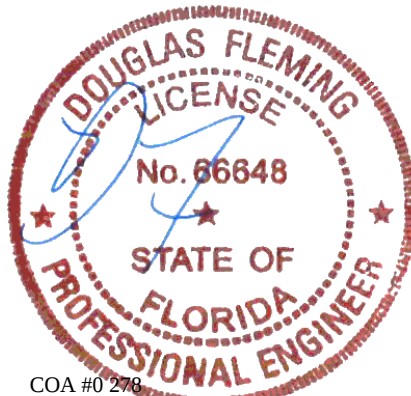
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	1319 -13	I - H	1316 -18
J - I	1316 -14	H - F	1319 -18

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	91 -486	I - E	91 -486
D - I	664 -32		



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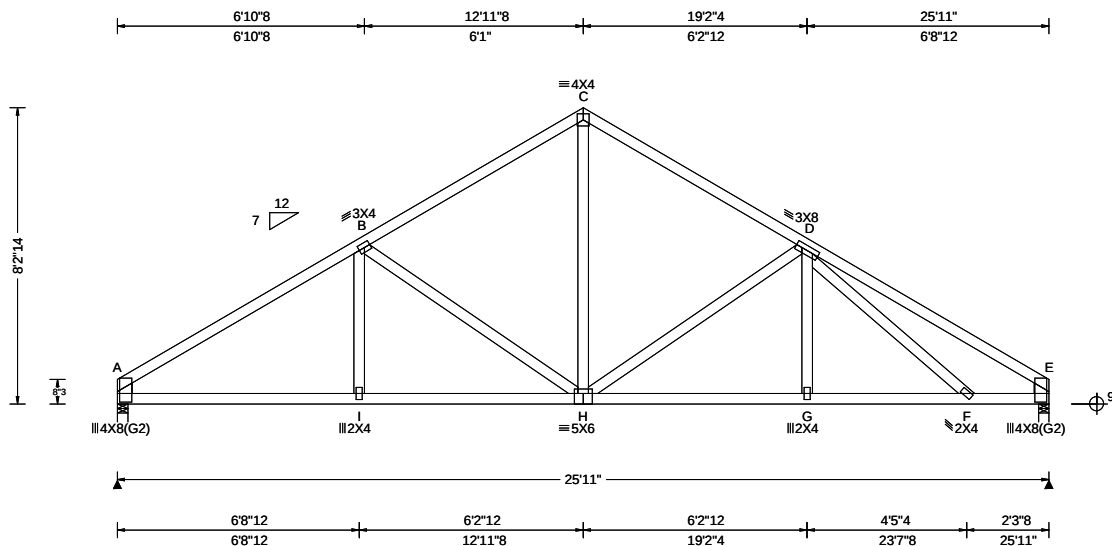
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100111 FROM:	COMN Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: C03	Cust: R 215 JRef: 1X172150016 T42 DrwNo: 231.22.1326.58870 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.049 H 999 240 VERT(CL): 0.102 H 999 180 HORZ(LL): 0.019 E - - HORZ(TL): 0.039 E - - Creep Factor: 2.0 Max TC CSI: 0.552 Max BC CSI: 0.214 Max Web CSI: 0.616 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 1098 /- /- /14 /- /- E 1290 /- /- /9 /- /- Wind reactions based on MWFRS A Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 0 -1679 C - D 1 -1196 B - C 0 -1194 D - E 0 -1942

Lumber

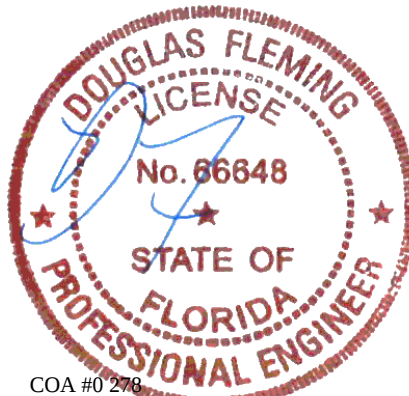
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Lt Stub Wedge: 2x4 SP #3;Rt Stub Wedge: 2x4 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 63 plf at 0.00 to 63 plf at 25.92
BC: From 20 plf at 0.00 to 20 plf at 25.92
BC: 233 lb Conc. Load at 23.69

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.
THIS TRUSS MUST BE INSTALLED AS SHOWN
AND NOT END FOR END.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.		Chords	Tens. Comp.	
A - I	1350	0	G - F	1411	0
I - H	1347	0	F - E	1580	0
H - G	1413	0			

Maximum Web Forces Per Ply (lbs)

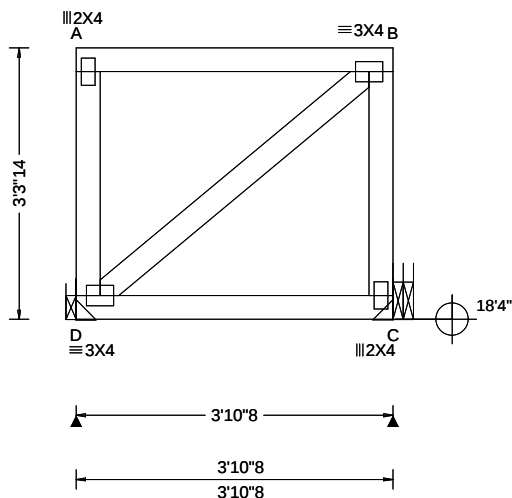
Webs	Tens.Comp.	Webs	Tens. Comp.
B - H	0 -494	H - D	0 -567
C - H	725 0		

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100132 FROM:	FLAT Ply: 2 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: D01	Cust: R 215 JRef: 1X172150016 T39 DrwNo: 231.22.1327.35063 KD / DF 08/19/2022
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2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.66 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 B 999 240 VERT(CL): 0.003 B 999 180 HORZ(LL): 0.001 A - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.488 Max BC CSI: 0.043 Max Web CSI: 0.205 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 1062 /- /- /79 /83 /- C 1123 /- /- /79 /88 /- Wind reactions based on MWFRS D Brg Wid = - Min Req = - C Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - D 69 -512 B - C 69 -542

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @ 5.75" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.13 to 60 plf at 0.25
TC: From 560 plf at 0.25 to 560 plf at 4.00
BC: From 20 plf at 0.13 to 20 plf at 4.00

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

The TC of this truss shall be braced with attached
spans at 24" oc in lieu of structural sheathing.

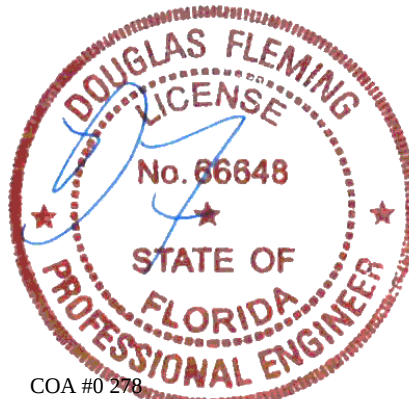
Wind

Wind loads based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up.

It is the responsibility of the Building Designer and
Truss Fabricator to review this drawing prior to
cutting lumber to verify that all data, including
dimensions and loads, conform to the architectural
plans/specifications and fabricators truss layout.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

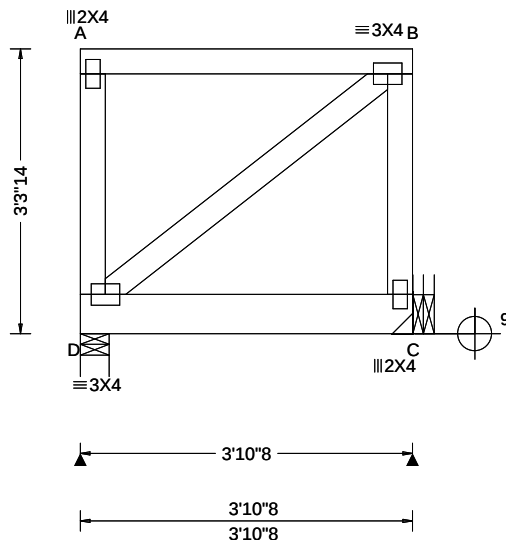
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100126 FROM:	FLAT Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: D02	Cust: R 215 JRef: 1X172150016 T23 DrwNo: 231.22.1328.05960 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 10.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): 0.000 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.263 Max BC CSI: 0.372 Max Web CSI: 0.063 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 567 /- /- /- /46 /- C 567 /- /- /- /46 /- Wind reactions based on MWFRS D Brg Wid = 4.0 Min Req = 1.5 (Truss) C Brg Wid = - Min Req = - Bearing D is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Special Loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 60 plf at 0.00 to 60 plf at 3.88
BC: From 20 plf at 0.00 to 20 plf at 3.88
BC: 824 lb Conc. Load at 1.94

Hangers / Ties

(J) Hanger Support Required, by others

Purlins

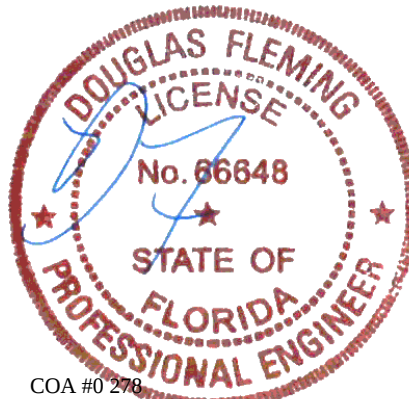
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Wind

Wind loads and reactions based on MWFRS.
End verticals not exposed to wind pressure.

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

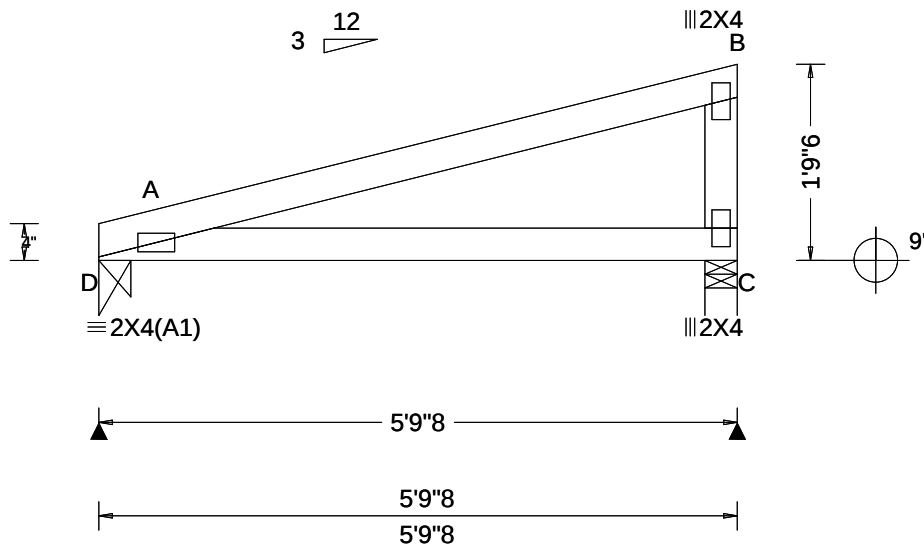
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99785 FROM:	MONO Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: E01	Cust: R 215 JRef: 1X172150016 T20 DrwNo: 231.22.1328.17657 KD / DF 08/19/2022
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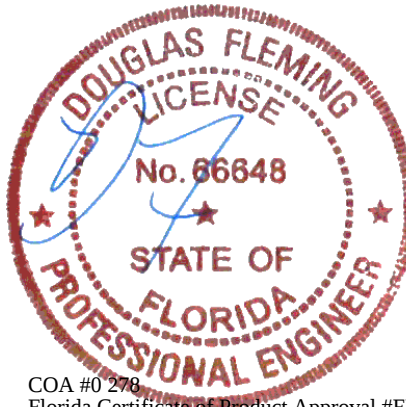
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.009 A - - HORZ(TL): 0.017 A - - Creep Factor: 2.0 Max TC CSI: 0.415 Max BC CSI: 0.331 Max Web CSI: 0.125 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D 239 /- /- /122 /- /25 C 227 /- /- /116 /- /- Wind reactions based on MWFRS D Brg Wid = 3.5 Min Req = 1.5 (Truss) C Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings D & C are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
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08/19/2022

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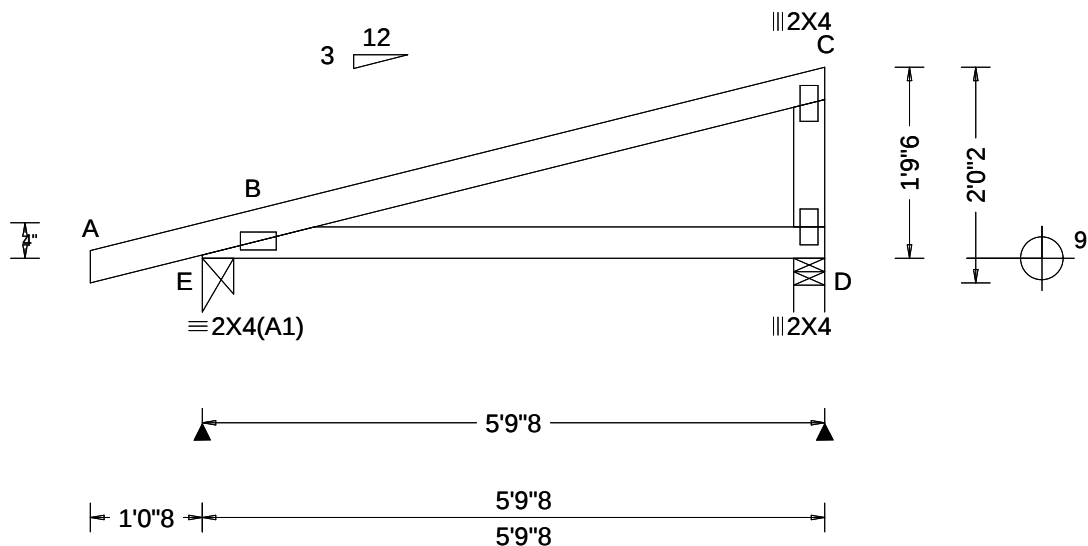
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99783 FROM:	MONO Ply: 1 Qty: 13	Job Number: 21-5560 Miller Res Truss Label: E02	Cust: R 215 JRef: 1X172150016 T16 DrwNo: 231.22.1328.18903 KD / DF 08/19/2022
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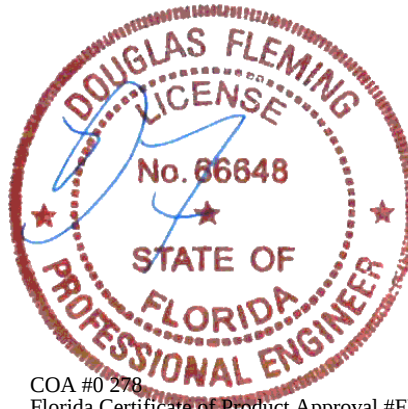
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.008 B - - HORZ(TL): 0.015 B - - Creep Factor: 2.0 Max TC CSI: 0.394 Max BC CSI: 0.297 Max Web CSI: 0.118 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 310 /- /- /160 /8 /32 D 220 /- /- /114 /- /- Wind reactions based on MWFRS E Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings E & D are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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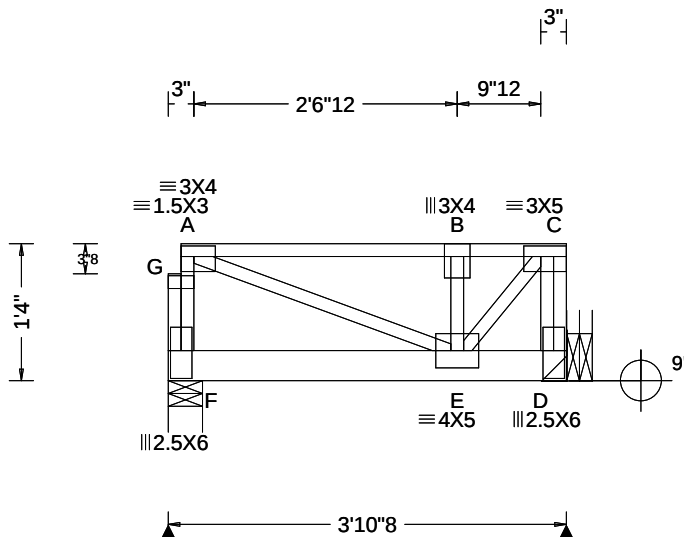
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100130 FROM:	SY42 Qty: 1	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: F01	Cust: R 215 JRef: 1X172150016 T3 DrwNo: 231.22.1328.26237 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.015 B 999 480 VERT(CL): 0.021 B 999 360 HORZ(LL): -0.004 C - - HORZ(TL): 0.006 C - - Creep Factor: 2.0 Max TC CSI: 0.697 Max BC CSI: 0.160 Max Web CSI: 0.296 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL F 483 -/- /- /- /- /- D 475 -/- /0 /- /- /- F Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. G - F 0 -413 B - E 0 -894 A - G 0 -402 E - C 621 0 A - E 393 0

Lumber

Top chord: 4x2 SP M-31;
Bot chord: 4x4 SP #2;
Webs: 4x2 SP #3;

Special Loads

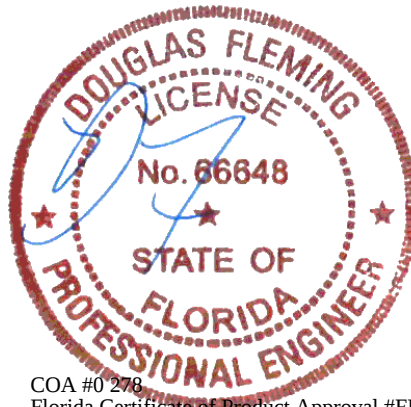
----- (Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 100 plf at 0.12 to 100 plf at 3.88
BC: From 10 plf at 0.00 to 10 plf at 3.88
TC: 544 lb Conc. Load at 1.81

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
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08/19/2022

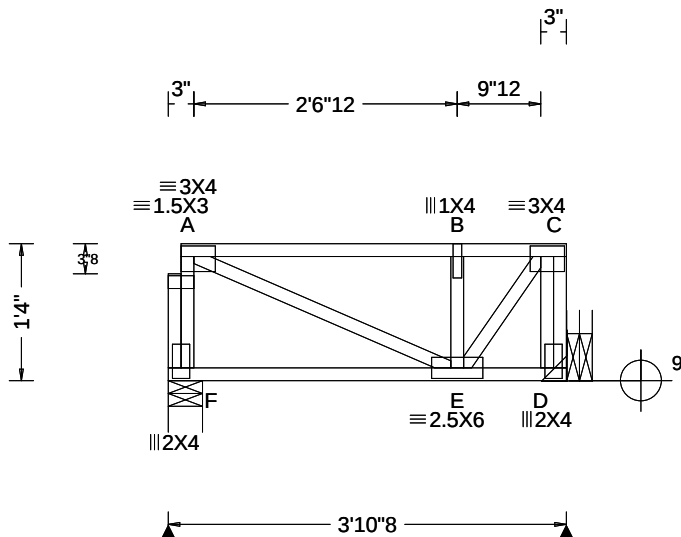
****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100128 FROM:	SY42 Ply: 1 Qty: 4	Job Number: 21-5560 Miller Res Truss Label: F02	Cust: R 215 JRef: 1X172150016 T27 DrwNo: 231.22.1328.28177 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 40.00 TCDL: 10.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 55.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.00 Spacing: 24.0 "	Wind Std: NA Speed: NA mph Enclosure: NA Category: NA EXP: NA Kzt: NA Mean Height: NA ft TCDL: NA psf BCDL: NA psf MWFRS Parallel Dist: NA C&C Dist a: NA ft Loc. from endwall: NA I: NA GCpi: NA Wind Duration: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 12(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.006 B 999 480 VERT(CL): 0.009 B 999 360 HORZ(LL): -0.002 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.095 Max Web CSI: 0.124 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 196 /- /- /- /- /- D 217 /- /- /- /- /- F Brg Wid = 4.0 Min Req = 1.5 (Truss) D Brg Wid = - Min Req = - Bearing F is a rigid surface. Members not listed have forces less than 375#

Lumber

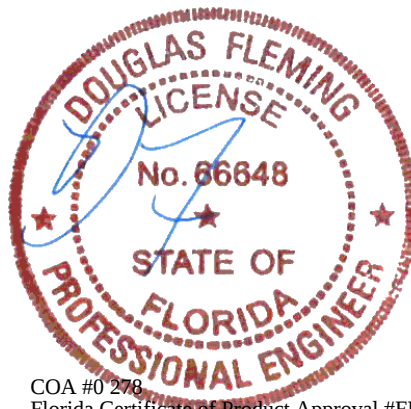
Top chord: 4x2 SP #2;
Bot chord: 4x2 SP #2;
Webs: 4x2 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Additional Notes

Truss must be installed as shown with top chord up.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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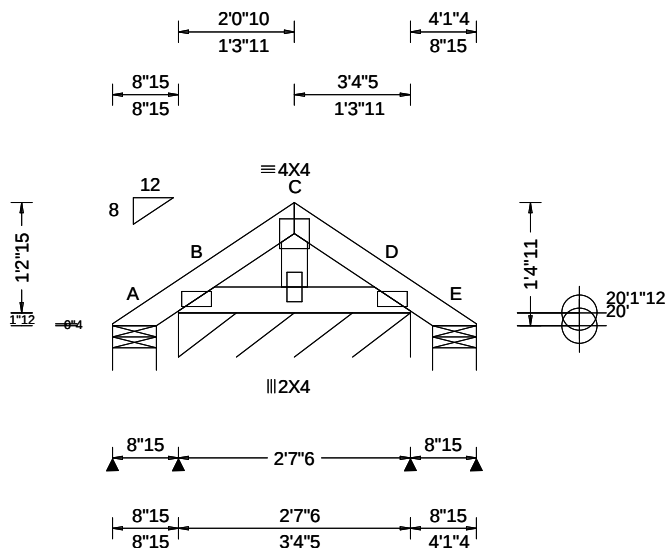
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100078 FROM:	GABL Ply: 1 Qty: 2	Job Number: 21-5560 Miller Res Truss Label: PB01	Cust: R 215 JRef: 1X172150016 T21 DrwNo: 231.22.1328.31323 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.70 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.013 Max BC CSI: 0.009 Max Web CSI: 0.008 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 17 /- /- /21 /7 /20 B* 82 /- /- /58 /5 /- E 17 /- /- /14 /2 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 31.4 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

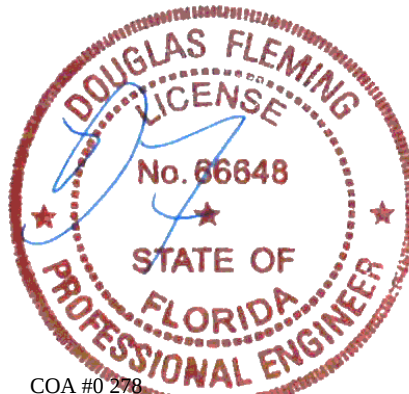
Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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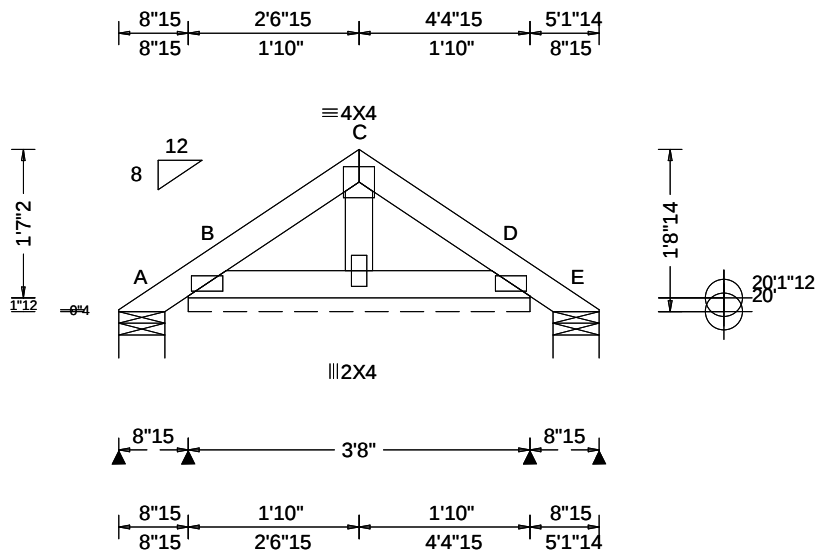
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100079 FROM:	SPEC Ply: 1 Qty: 32	Job Number: 21-5560 Miller Res Truss Label: PB02	Cust: R 215 JRef: 1X172150016 T32 DrwNo: 231.22.1328.33447 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.88 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.030 Max BC CSI: 0.016 Max Web CSI: 0.011 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 9 /- /- /22 /12 /25 B* 82 /- /- /57 /6 /- E 9 /- /- /8 /- /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 44.0 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

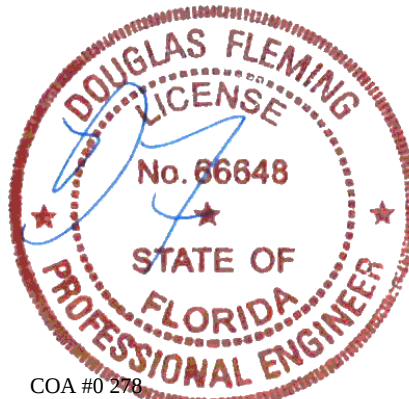
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

Refer to DWG PB160160118 for piggyback details.



COA #0 278

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08/19/2022

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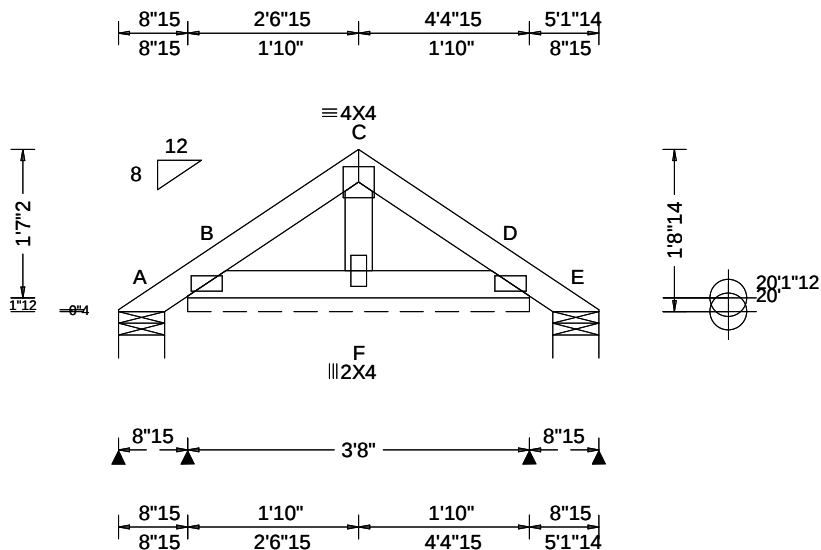
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100028 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: PB03	Cust: R 215 JRef: 1X172150016 T4 DrwNo: 231.22.1328.38877 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.88 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.030 Max BC CSI: 0.016 Max Web CSI: 0.011 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 8 /- /- /17 /14 /27 B* 82 /- /- /55 /1 /- E 8 /- /- /2 /0 /- Wind reactions based on MWFRS A Brg Wid = 5.9 Min Req = 1.5 (Truss) B Brg Wid = 44.0 Min Req = - E Brg Wid = 5.9 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

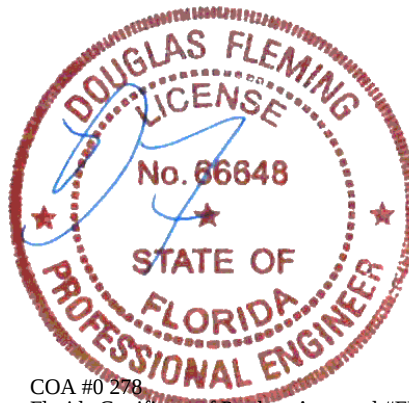
Wind

Wind loads based on MWFRS.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.



COA #0 278
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08/19/2022

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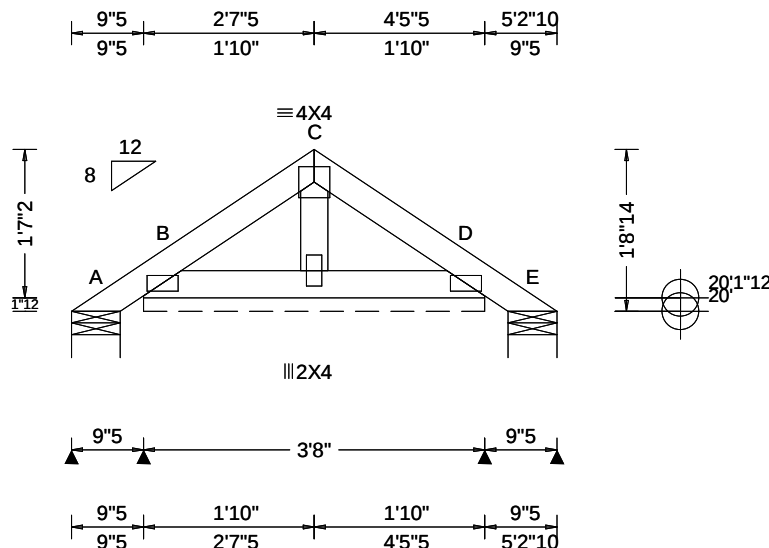
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 100080 FROM:	SPEC Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: PB04	Cust: R 215 JRef: 1X172150016 T44 DrwNo: 231.22.1328.59590 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TCDL: 5.0 psf BCDL: 2.0 psf MWFRS Parallel Dist: > 2h C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 B 999 240 VERT(CL): 0.000 B 999 180 HORZ(LL): 0.000 B - - HORZ(TL): 0.000 B - - Creep Factor: 2.0 Max TC CSI: 0.030 Max BC CSI: 0.016 Max Web CSI: 0.011 VIEW Ver: 21.02.00.1005.17	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 9 /- /- /22 /12 /25 B* 82 /- /- /57 /6 /- E 9 /- /- /8 /- /- Wind reactions based on MWFRS A Brg Wid = 6.3 Min Req = 1.5 (Truss) B Brg Wid = 44.0 Min Req = - E Brg Wid = 6.3 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

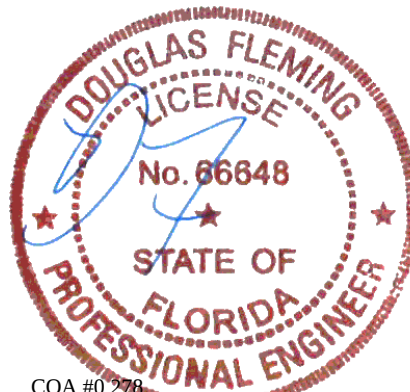
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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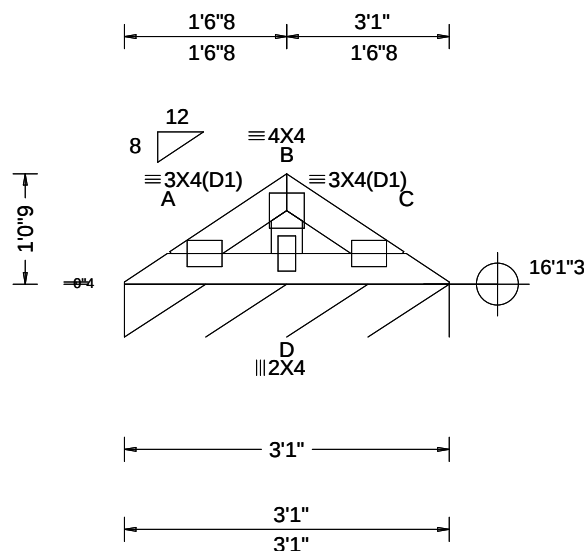
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99970 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V01	Cust: R 215 JRef: 1X172150016 T45 DrwNo: 231.22.1329.03483 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.78 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.019 Max BC CSI: 0.016 Max Web CSI: 0.015 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /38 /- /4 Wind reactions based on MWFRS C Brg Wid = 37.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

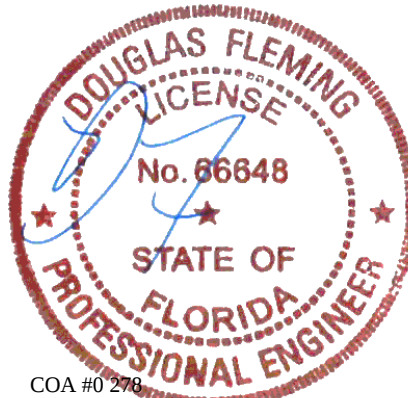
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



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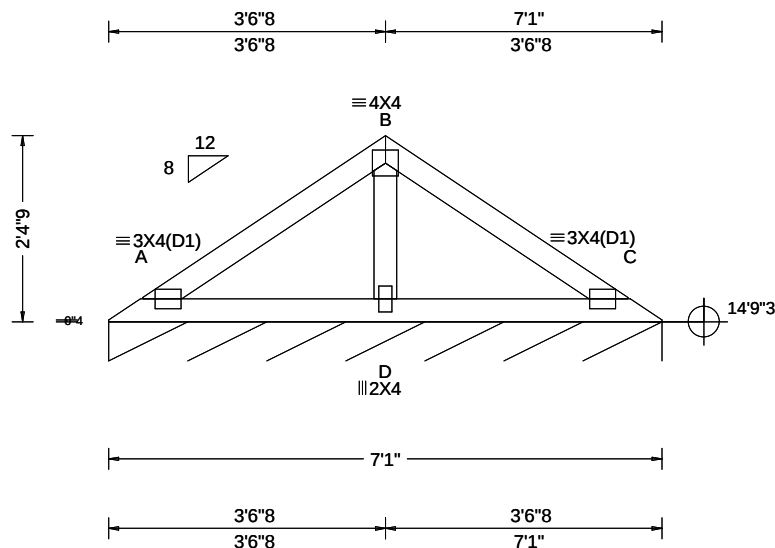
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99972 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V02	Cust: R 215 JRef: 1X172150016 T46 DrwNo: 231.22.1329.04660 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.11 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCp: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.009 A 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.154 Max BC CSI: 0.133 Max Web CSI: 0.062 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 84 /- /- /42 /- /5 Wind reactions based on MWFRS C Brg Wid = 85.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

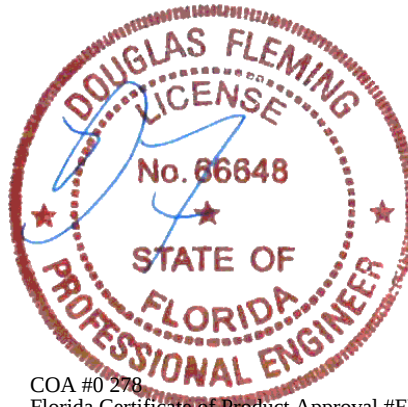
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

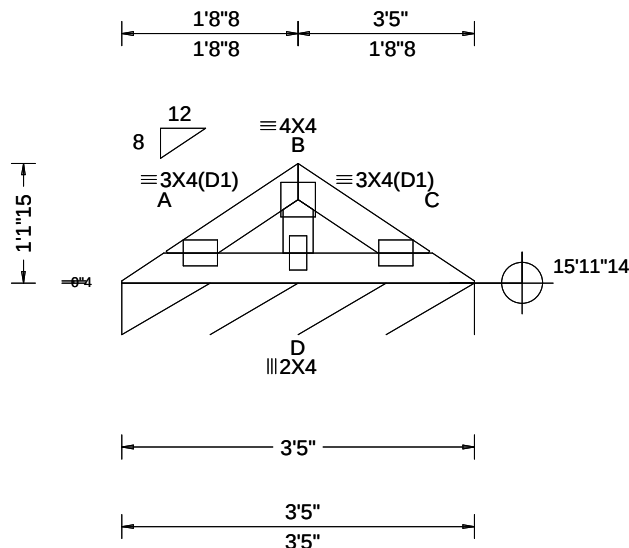
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SEQN: 99974 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V03	Cust: R 215 JRef: 1X172150016 T33 DrwNo: 231.22.1329.05777 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.72 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.000 A 999 240 VERT(CL): 0.001 A 999 180 HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.026 Max BC CSI: 0.021 Max Web CSI: 0.018 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /39 /- /4 Wind reactions based on MWFRS C Brg Wid = 41.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

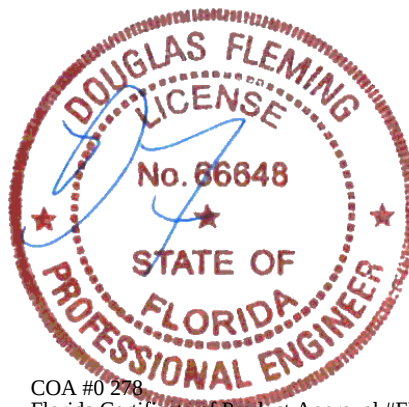
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
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08/19/2022

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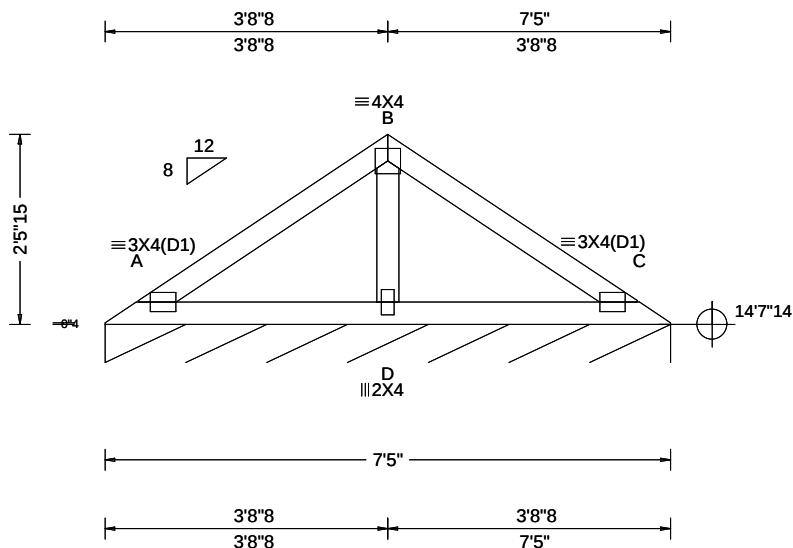
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99976 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V04	Cust: R 215 JRef: 1X172150016 T25 DrwNo: 231.22.1329.07077 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.06 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 A 999 240 VERT(CL): 0.010 A 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.172 Max BC CSI: 0.148 Max Web CSI: 0.067 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 84 /- /- /42 /- /5 Wind reactions based on MWFRS C Brg Wid = 89.0 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

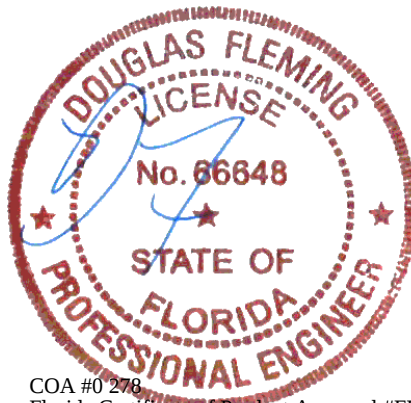
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278

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08/19/2022

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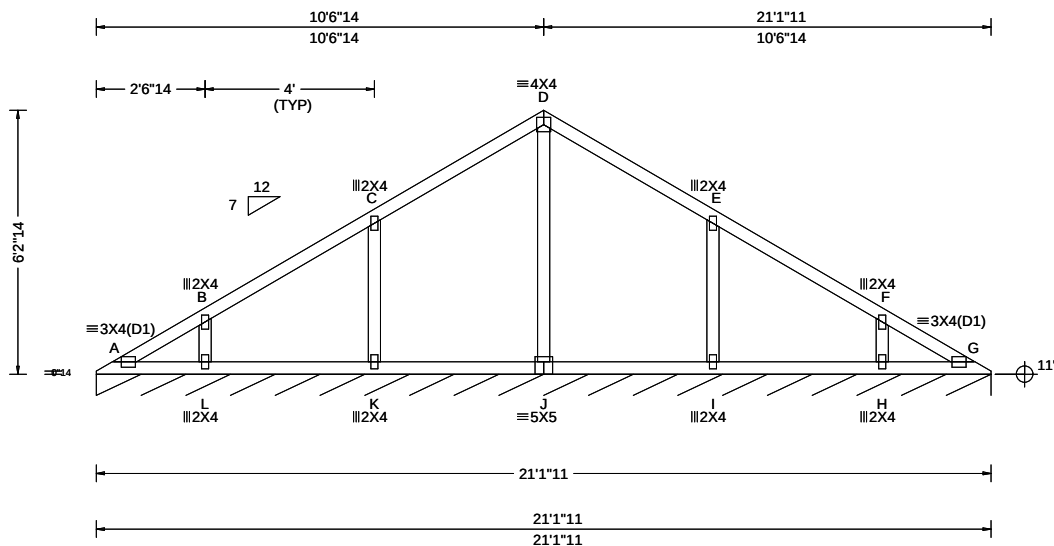
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99978 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V05	Cust: R 215 JRef: 1X172150016 T36 DrwNo: 231.22.1329.09333 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.002 D 999 180 HORZ(LL): 0.001 E - - HORZ(TL): 0.001 E - - Creep Factor: 2.0 Max TC CSI: 0.214 Max BC CSI: 0.109 Max Web CSI: 0.129 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G* 83 /- /- /43 /- /4 Wind reactions based on MWFRS G Brg Wid = 253 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

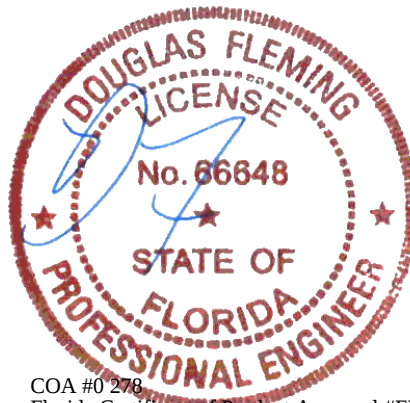
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278

Florida Certificate of Product Approval #FL1999
08/19/2022

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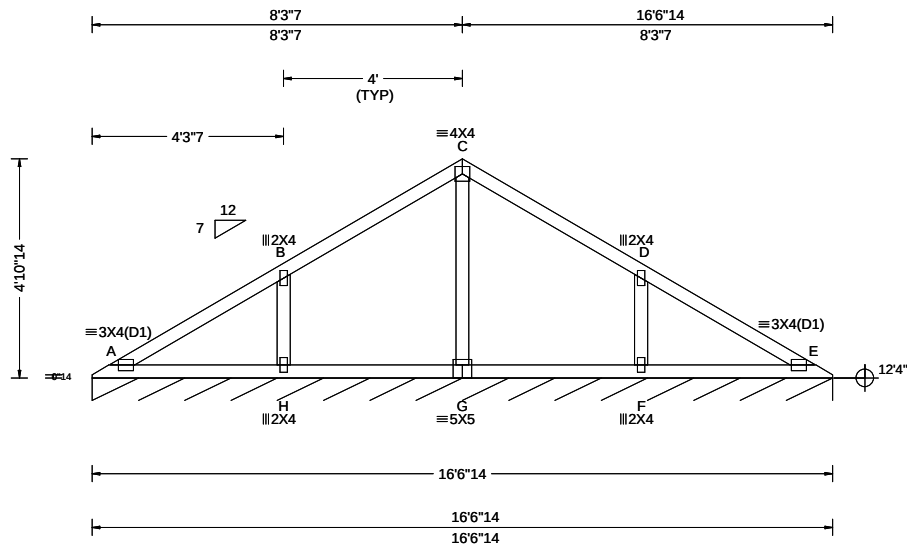
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99980 FROM:	VAL Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V06	Cust: R 215 JRef: 1X172150016 T34 DrwNo: 231.22.1329.10680 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 E 999 240 VERT(CL): 0.012 E 999 180 HORZ(LL): -0.002 E - - HORZ(TL): 0.005 E - - Creep Factor: 2.0 Max TC CSI: 0.305 Max BC CSI: 0.160 Max Web CSI: 0.106 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 83 /- /- /43 /- /4 Wind reactions based on MWFRS E Brg Wid = 198 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

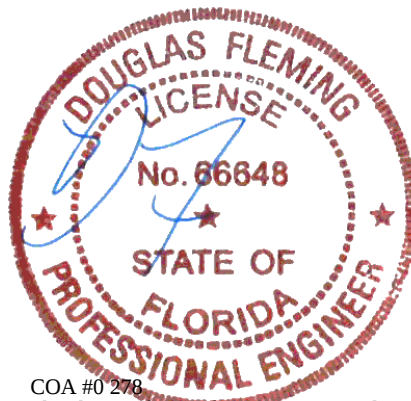
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0278
Florida Certificate of Product Approval #FL1999
08/19/2022

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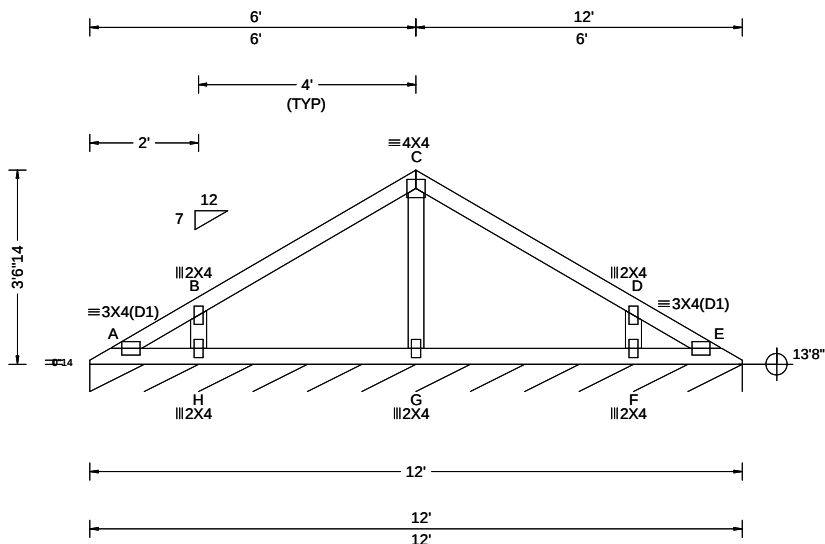
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155 Harlem Ave
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Glenview, IL 60025

SEQN: 99984 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V07	Cust: R 215 JRef: 1X172150016 T37 DrwNo: 231.22.1329.11980 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.61 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.204 Max BC CSI: 0.117 Max Web CSI: 0.048 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 83 /- /- /42 /- /4 Wind reactions based on MWFRS E Brg Wid = 144 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

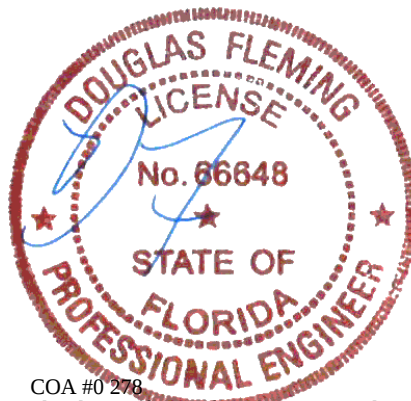
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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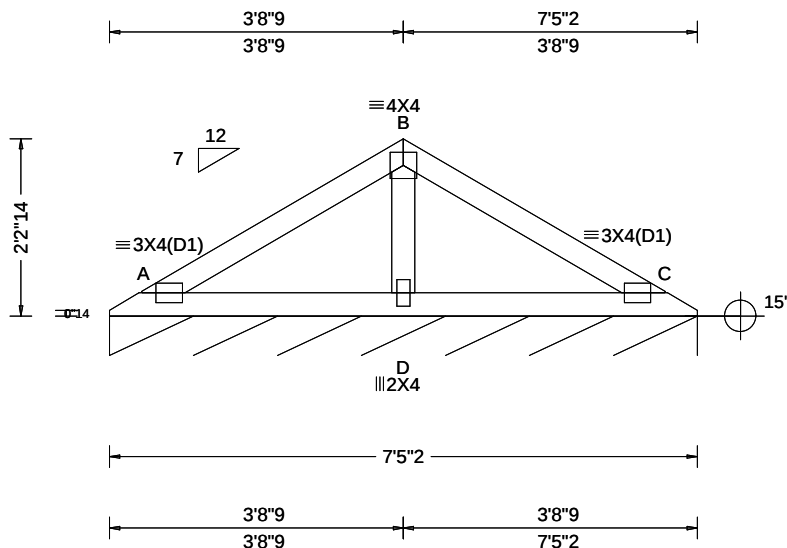
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North Building, 4th Floor
Glenview, IL 60025

SEQN: 99986 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V08	Cust: R 215 JRef: 1X172150016 T35 DrwNo: 231.22.1329.13857 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.27 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.011 C 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.168 Max BC CSI: 0.151 Max Web CSI: 0.059 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /41 /- /4 Wind reactions based on MWFRS C Brg Wid = 89.1 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

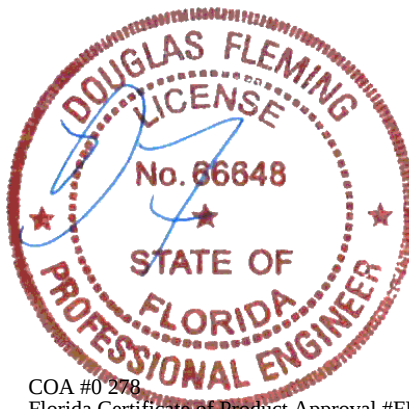
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
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08/19/2022

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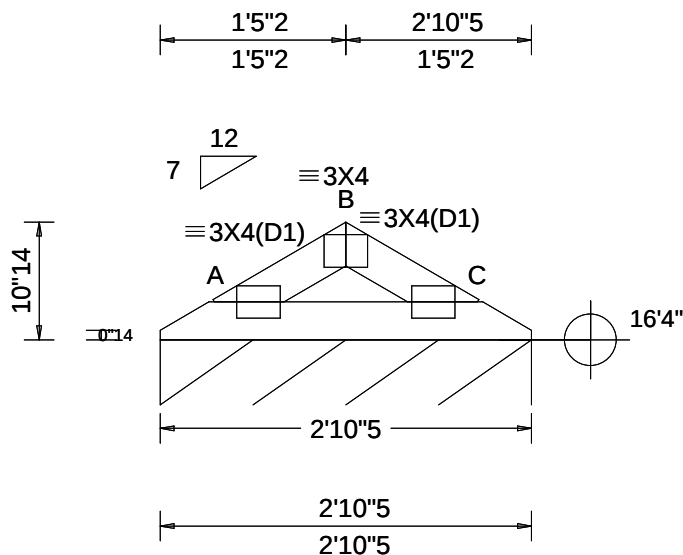
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 99988 FROM:	VAL	Ply: 1 Qty: 1	Job Number: 21-5560 Miller Res Truss Label: V09	Cust: R 215 JRef: 1X172150016 T30 DrwNo: 231.22.1329.16847 KD / DF 08/19/2022
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 16.94 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.002 A 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.033 Max BC CSI: 0.052 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 82 /- /- /37 /- /3 Wind reactions based on MWFRS C Brg Wid = 34.3 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;

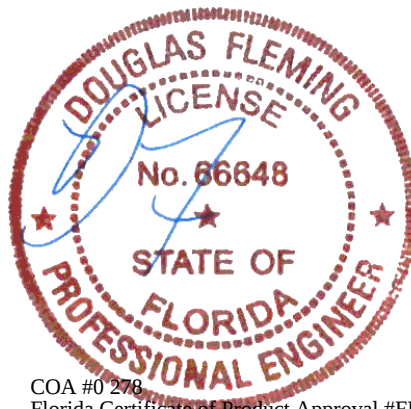
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



COA #0 278
Florida Certificate of Product Approval #FL1999
08/19/2022

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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

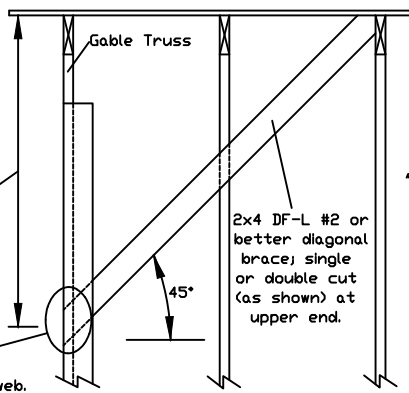
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
	SP	DFL	Stud	4' 6"	6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"
			Standard	4' 4"	5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	12' 10"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 6"	9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#2	#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 6"	8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

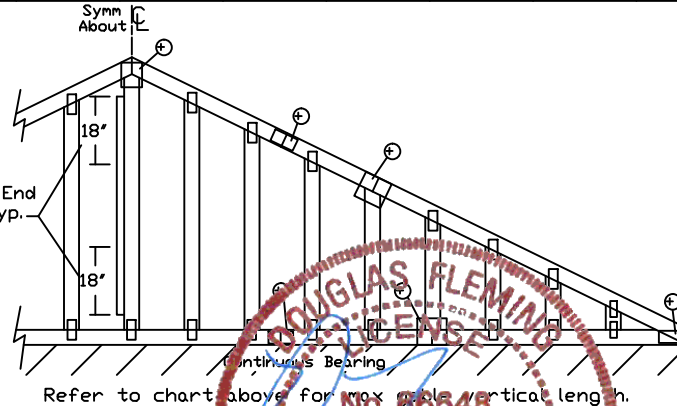
Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 385# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



'L' Brace End Zones, typ.



Refer to chart above for max gable vertical length.

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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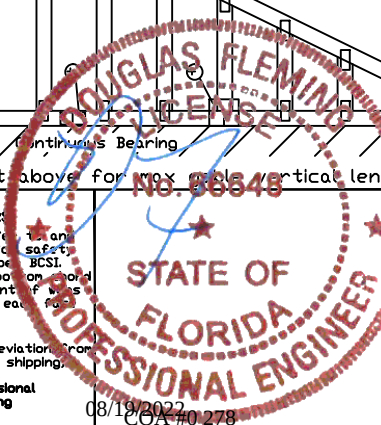
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



08/19/2022
 COA #0278

REF ASCE7-16-GAB12030

DATE 01/26/2018

DRWG A12030ENC160118

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

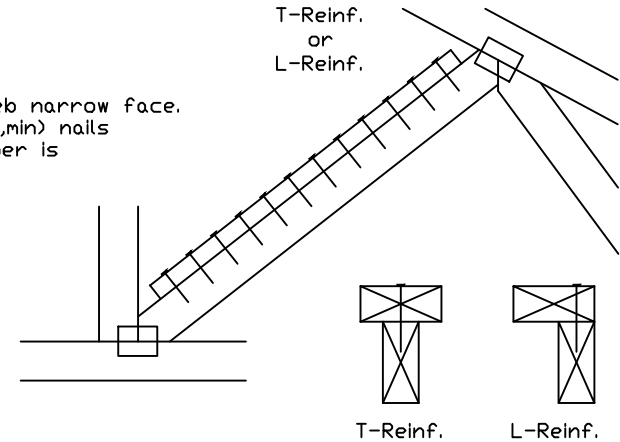
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

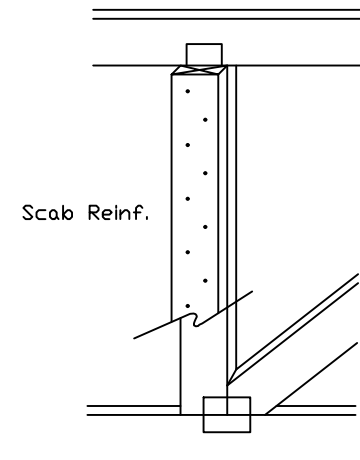
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



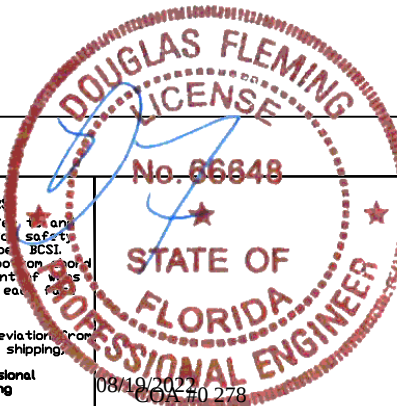
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements

Top: Continuous roof sheathing

Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

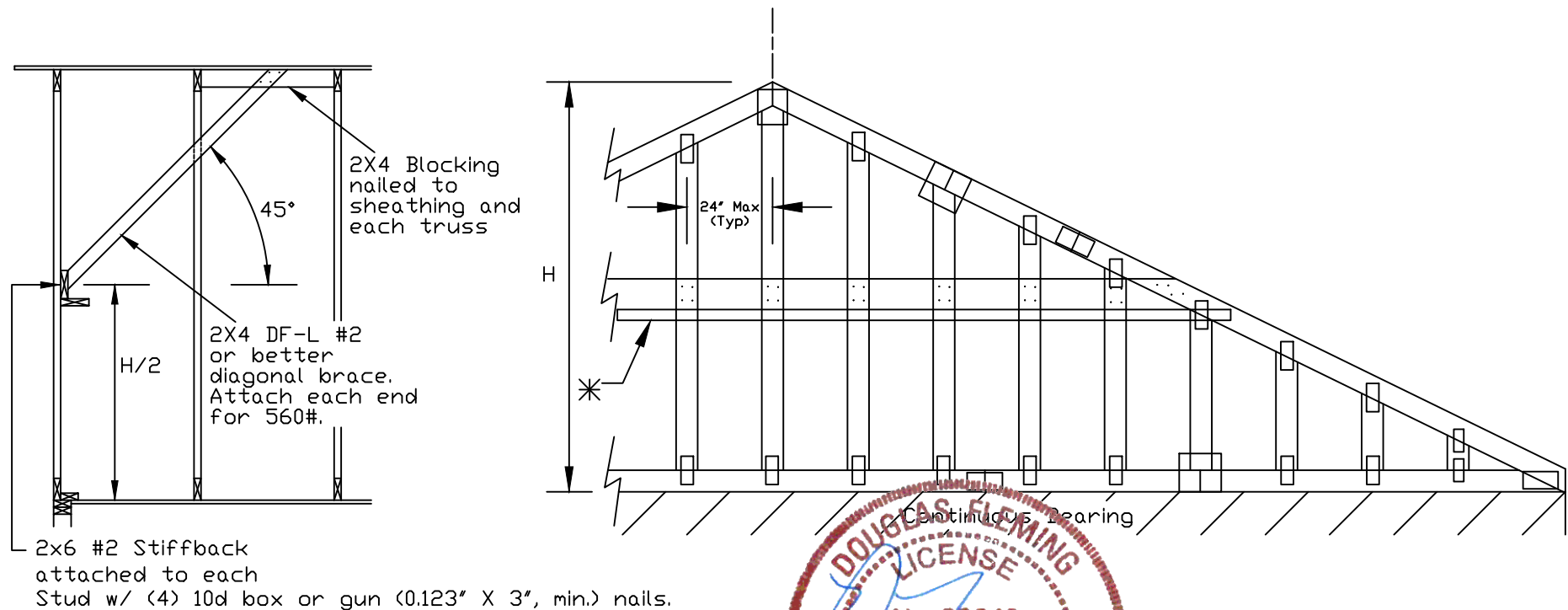
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



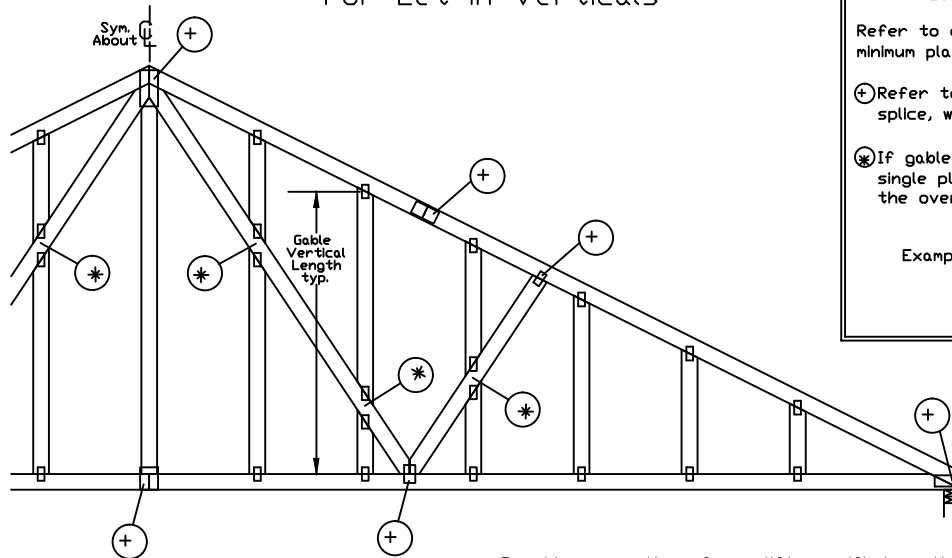
155 Harlem Ave
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REF	GE WHALER
DATE	01/02/2018
DRWG	GABRST160118
MAX. TOT. LD.	60 PSF
MAX. SPACING	

Gable Detail For Let-in Verticals

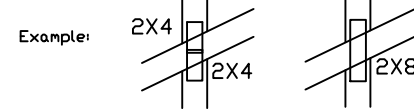


Gable Truss Plate Sizes

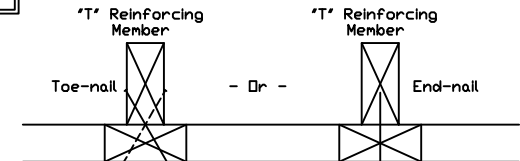
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(X) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

"T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

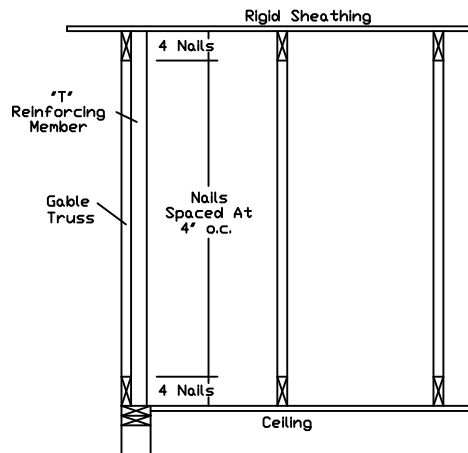
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum reinforced gable vertical length.



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ALPINE
AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

No. 66848

STATE OF

FLORIDA

PROFESSIONAL ENGINEER

08/19/2022
COA #0278

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

DUR. FAC. ANY

MAX. SPACING 24.0"

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	7' 8"	8' 2"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 7"	6' 7"	7' 0"	8' 10"	9' 5"	11' 5"	11' 10"	13' 10"	14' 0"	14' 0"	14' 0"
			#2	5' 0"	8' 4"	8' 7"	9' 10"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
			Standard	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"
		Standard	#1	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	#1	5' 9"	9' 6"	9' 10"	11' 3"	11' 8"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 3"	8' 1"	8' 7"	10' 10"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 9"	9' 6"	9' 10"	11' 3"	11' 8"	13' 4"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	#1	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 9"	9' 4"	9' 11"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

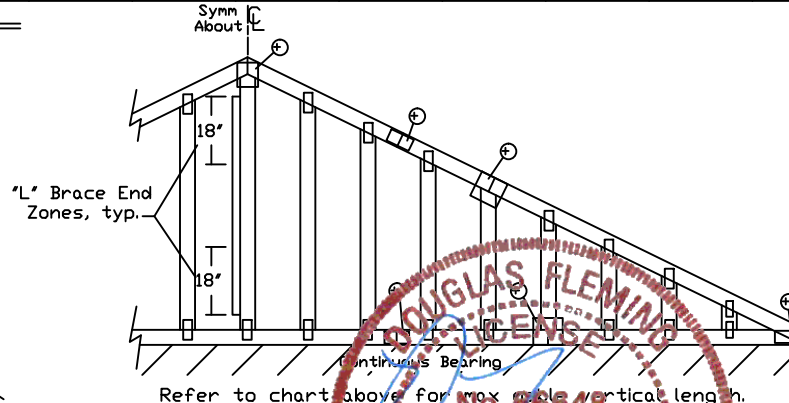
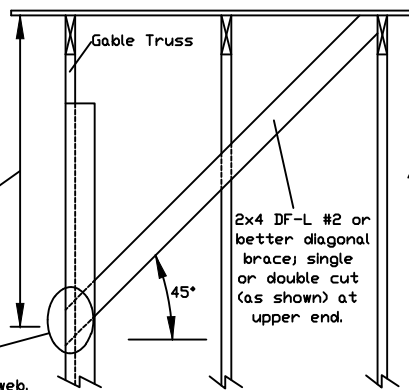
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Refer to chart above for max gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
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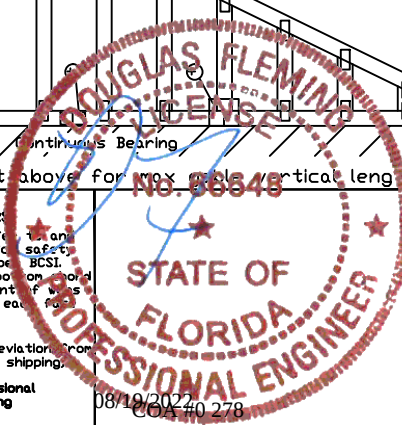
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF ASCE7-16-GAB12015

DATE 01/26/2018

DRWG A12015ENC160118

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

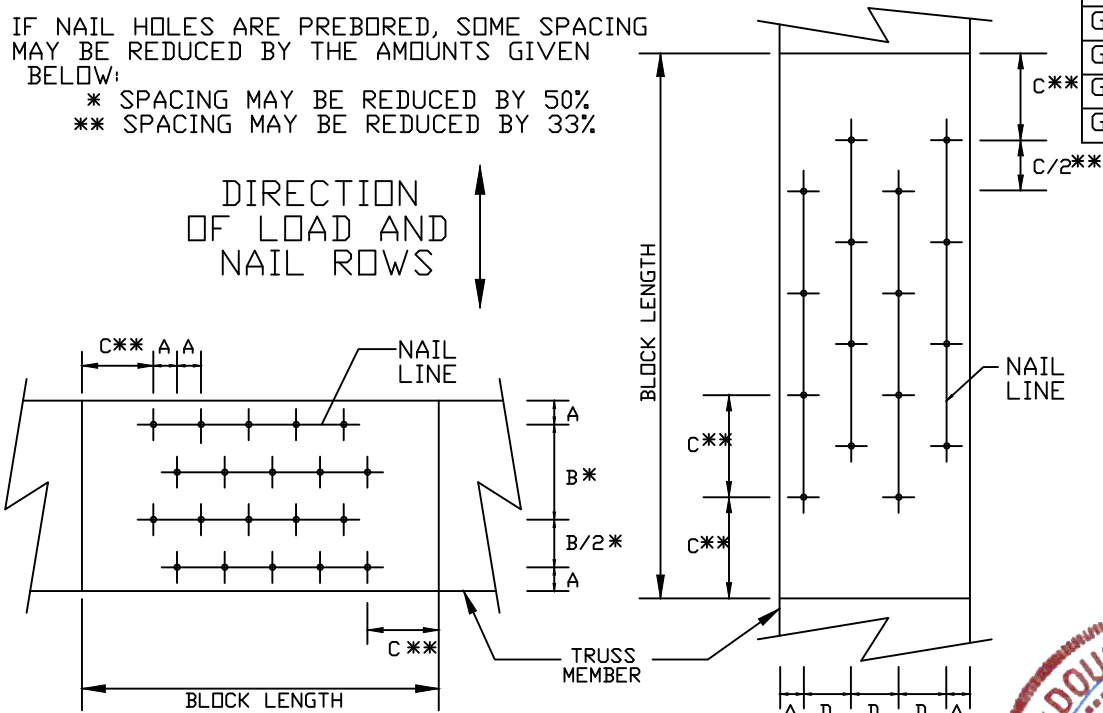
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

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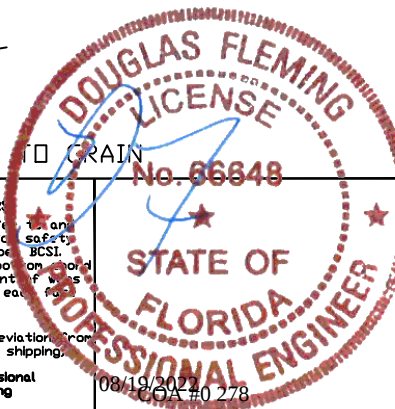
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025



REF NAIL SPACE
 DATE 10/01/14
 DRWG CNNAILSP1014

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

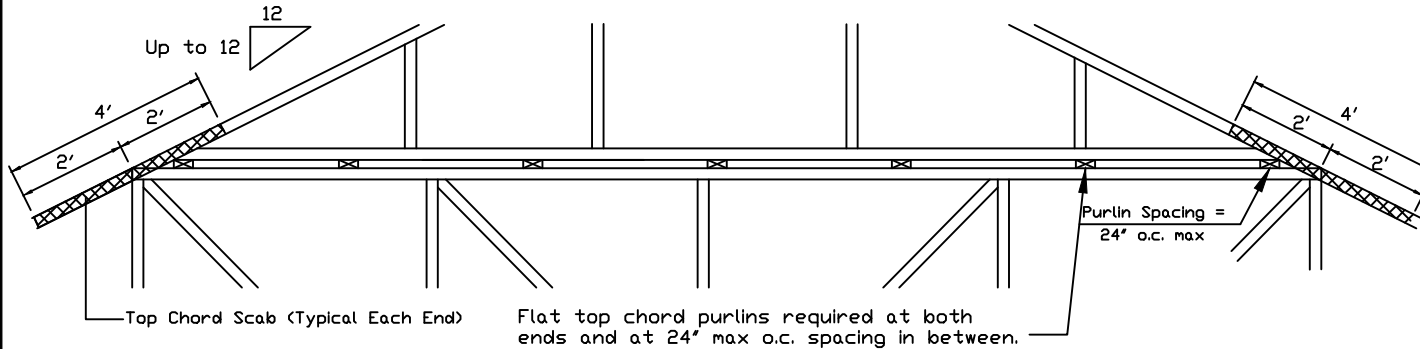
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

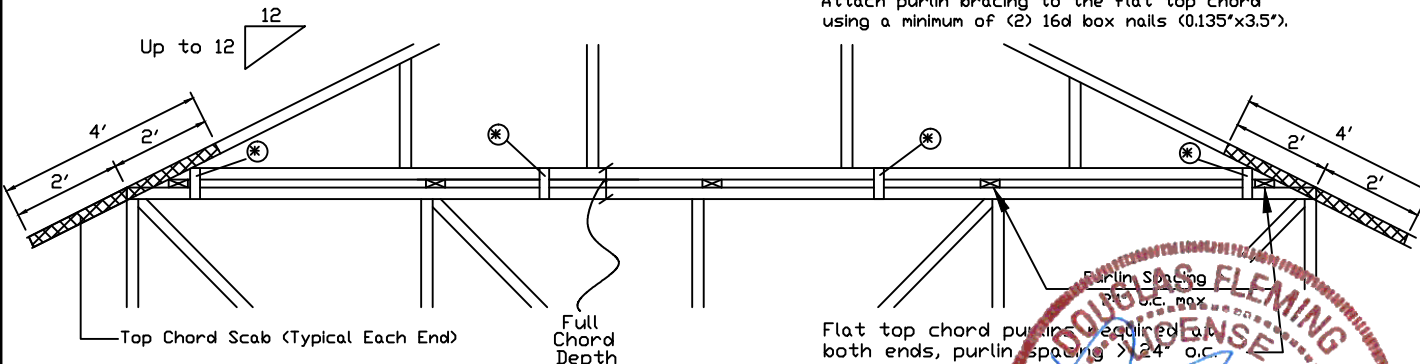


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

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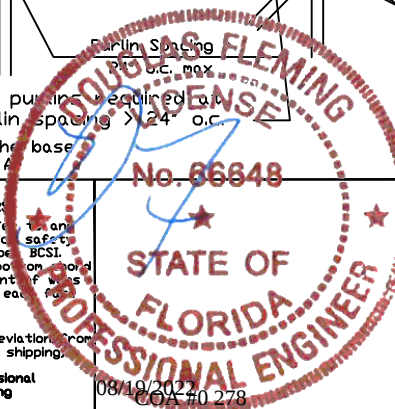
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155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025



REF PIGGYBACK

DATE 01/02/2018

DRWG PB160160118

SPACING

24.0"

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

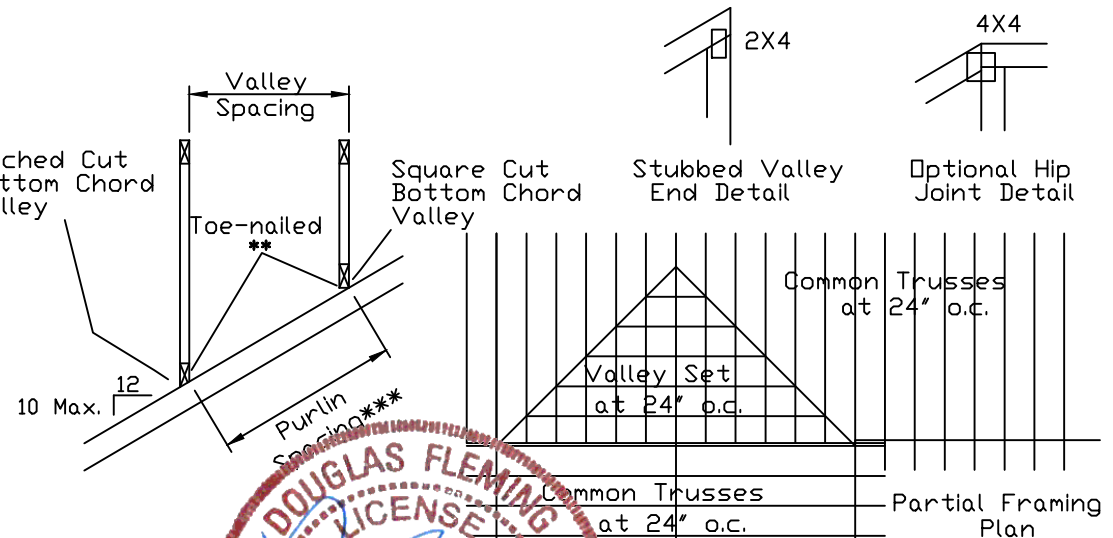
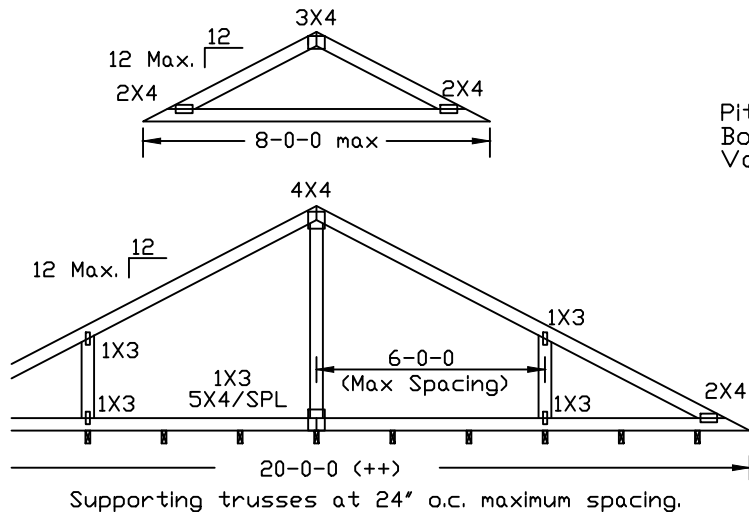
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

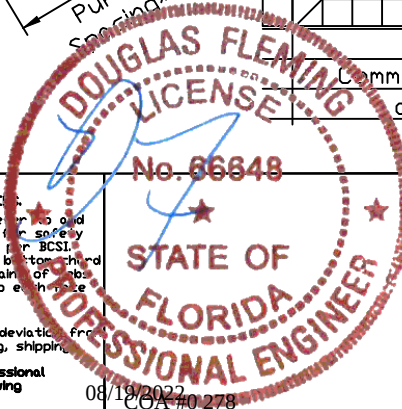
++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



ALPINE
 AN ITW COMPANY

155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

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 Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord
 shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
 shall have bracing installed per BCSI sections 83, 87 or 810, as applicable. Apply plates to each side
 of truss and position as shown above and on the Joint Details, unless noted otherwise.
 Refer to drawings 160A-Z for standard plate positions.
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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7 PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VALTN160118
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
DUR.FAC.	1.25/1.33	1.15	1.15		
SPACING	24.0"				