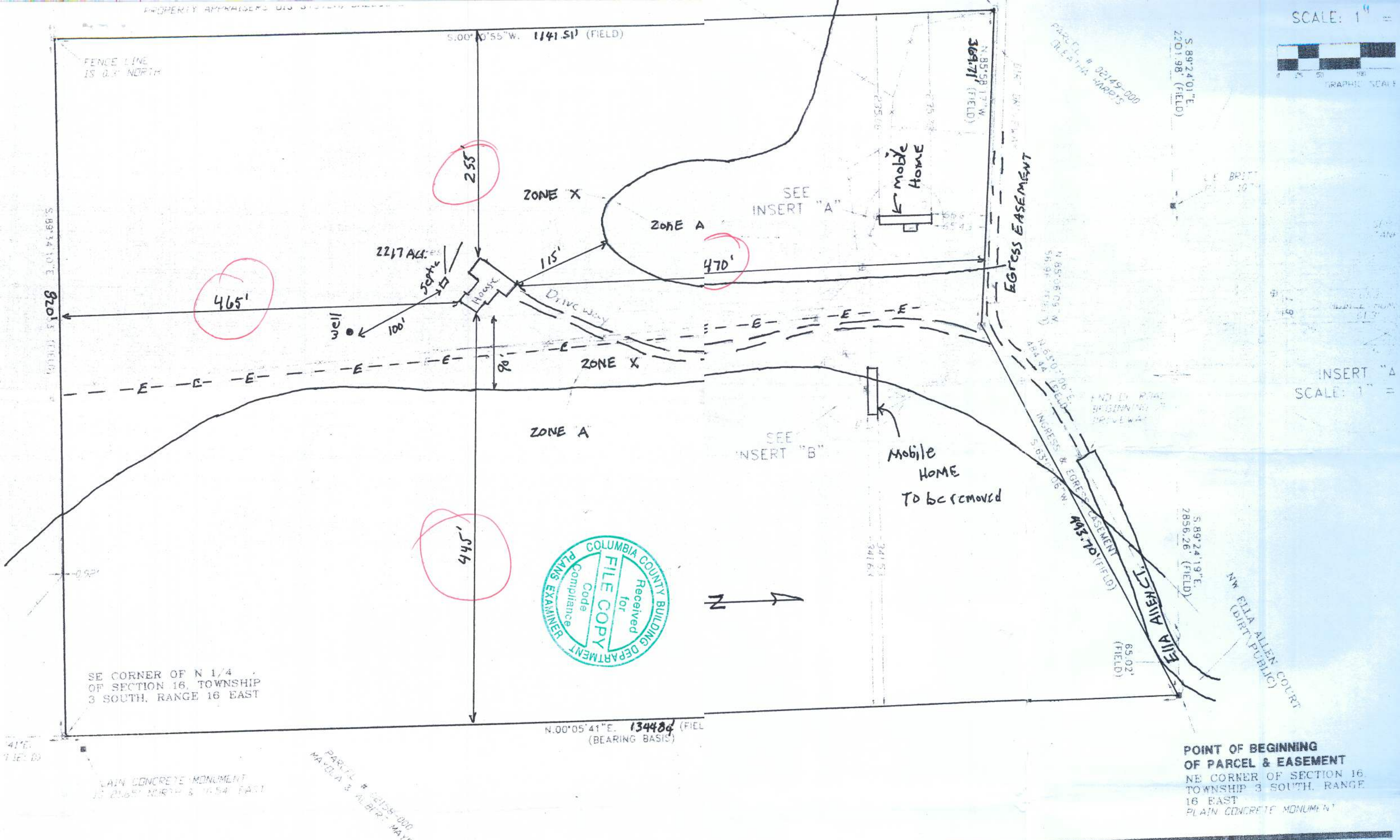


SCALE: 1" =



POINT OF BEGINNING
OF PARCEL & EASEMENT
NE CORNER OF SECTION 16,
TOWNSHIP 3 SOUTH, RANGE
16 EAST
PLAIN CONCRETE MONUMENT

BRITT
& ASSOC

GRIP'S CERTIFICATION

MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM

EASTERLY
RIGHT-OF-WAY
LINE OF I-75

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF A PREVIOUS SECTION BREAKDOWN BY THIS OFFICE.
2. BEARINGS ARE BASED ON AN ASSUMED BEARING OF N00°05' FOR THE EAST LINE OF SAID SECTION 16.
3. IT IS APPARENT THAT SOME PORTIONS OF THIS PARCEL ARE IN ZONE "A" AND MAY BE SUBJECT TO FLOODING. HOWEVER, NO BASE FLOOD ELEVATION HAS BEEN DETERMINED FOR ZONE "A". SOME PORTIONS OF THIS PARCEL ARE IN ZONE "A" AND ARE DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD INSURANCE RATE MAP, DATED 4 FEBRUARY, 2009, FIRM PANEL NO 12023C0280C. HOWEVER, FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY WING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF TITLE COMMITMENT OR A TITLE POLICY.
7. DIMENSIONS SHOWN HEREON ARE IN FEET AND DECIMAL PART THEREOF.
8. THIS SURVEY DOES NOT REFLECT OR DETERMINE OWNERSHIP.
9. THE ADJACENT OWNERSHIP INFORMATION AS SHOWN HEREON IS BASED ON THE COUNTY PROPERTY APPRAISERS GIS SYSTEM, UNLESS OTHERWISE NOTED.

DESCRIPTION:

BEGIN AT THE NE CORNER OF SECTION 16, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN S.63°07'06"W, 493.70 FEET; THENCE N.85°58'17"W, 369.71 FEET; THENCE S.00°40'55"W, 1141.51 FEET TO THE SOUTH LINE OF THE NORTH 1/4 OF SAID SECTION 16; THENCE S.89°34'10"E, ALONG SAID SOUTH LINE, 820.53 FEET TO THE SE CORNER OF SAID NORTH 1/4; THENCE N.00°05'41"E, ALONG THE EAST LINE OF SAID SECTION 16, 1344.84 FEET TO THE POINT OF BEGINNING, CONTAINING 22.17 ACRES, MORE OR LESS.

TOGETHER WITH AN EASEMENT FOR INGRESS AND EGRESS BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:
BEGIN AT THE NE CORNER OF SECTION 16, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN S.63°07'06"W, 493.70 FEET; THENCE N.85°58'17"W, 369.71 FEET; THENCE S.00°40'55"W, 1141.51 FEET TO THE NORTH LINE OF SAID SECTION 16; THENCE S.89°34'10"E, ALONG SAID NORTH LINE, 65.02 FEET TO THE POINT OF BEGINNING.

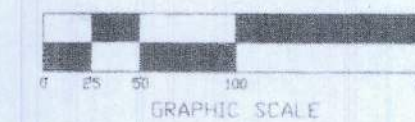
A BOUNDARY SURVEY IN SECTION 16, TOWNSHIP 3 SOUTH,
RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA

NW CORNER OF SECTION 16,
TOWNSHIP 3 SOUTH, RANGE
16 EAST
M.C. HALE, P.L.S. 1519

SYMBOL LEGEND:

- 4"x4" CONCRETE MONUMENT FOUND
- 4"x4" CONCRETE MONUMENT SET
- IRON PIPE FOUND
- IRON PIN AND CAP SET
- "X" CUT IN PAVEMENT
- CALCULATED PROPERTY CORNER
- NAI & DISK
- POWER POLE
- SIGN POST
- WATER METER
- UTILITY BOX
- WELL
- SANITARY MANHOLE
- CENTERLINE
- SECTION LINE
- ELECTRIC LINES
- WIRE FENCE
- CHAIN LINK FENCE
- WOODEN FENCE
- (PLAT) AS PER A PLAT OF RECORD
- (DEED) AS PER A DEED OF RECORD
- (CALC) AS PER CALCULATIONS
- (FIELD) AS PER FIELD MEASUREMENTS
- P.R.M. PERMANENT REFERENCE MARKER
- P.C.P. PERMANENT CONTROL POINT

SCALE: 1" = 100'



INSERT "A"
SCALE: 1" = 30'

INSERT "B"
SCALE: 1" = 30'

POINT OF BEGINNING
NE CORNER OF SECTION 16,
TOWNSHIP 3 SOUTH, RANGE
16 EAST
PLAIN CONCRETE MONUMENT

FENCE LINE
IS 0.3' NORTH

S.00°5'W, 1141.51' (FIELD)

FENCE CORNER
IS 0.57' SOUTH
& 0.38' EAST

Mobile Home
To be removed

SEE
INSERT "B"

SE CORNER OF N 1/4
OF SECTION 16, TOWNSHIP
3 SOUTH, RANGE 16 EAST

PLAIN CONCRETE MONUMENT
IS 21.65' NORTH & 16.54' EAST

CERTIFIED TO:

VICKIE HARRIS

SURVEYOR'S CERTIFICATION

HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM ETHICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS' CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 478.027, FLORIDA STATUTES.

2/21/11
SURVEY DATE

01/25/11
DRAWING DATE

Britt
L.E. BRITT, P.E.
CERTIFICATION # 5757

UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAY THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

BRITT

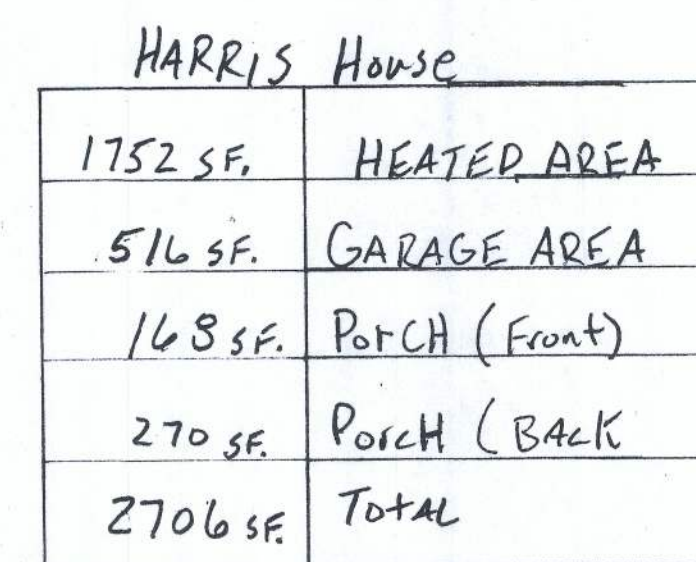
www.brittsurveying.com
TELEPHONE (386) 752-7163 FAX (386) 752-5573

BRITT SURVEYING
& ASSOCIATES, INC.

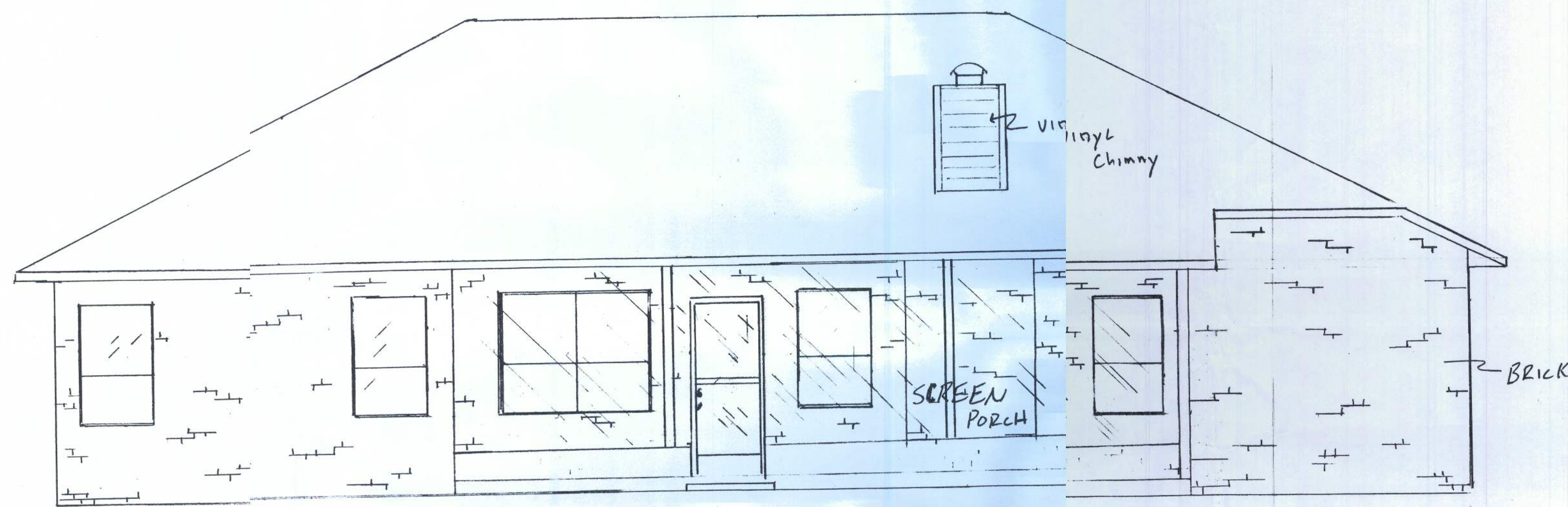
LAND SURVEYORS AND MAPPERS, L.B. # 7593
830 WEST DUVAL STREET
LAKE CITY, FLORIDA 32055

WORK ORDER # L-20740

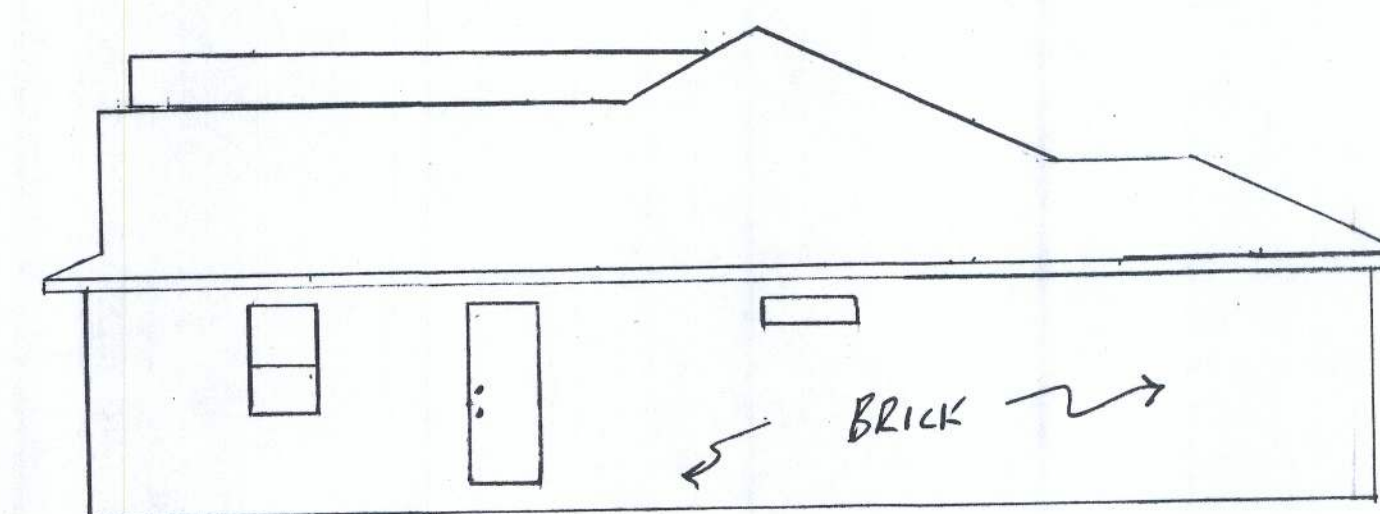
FIELD BOOK SEE PAGE(S) FILE



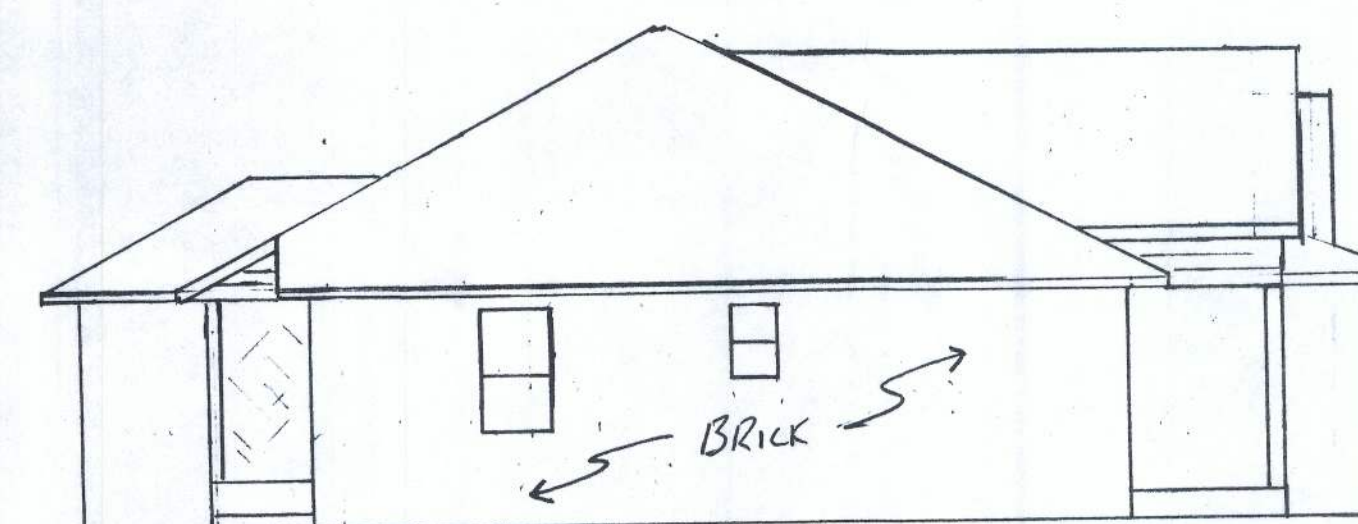
HARRIS HOUSE
MILTON BUILDERS LLC



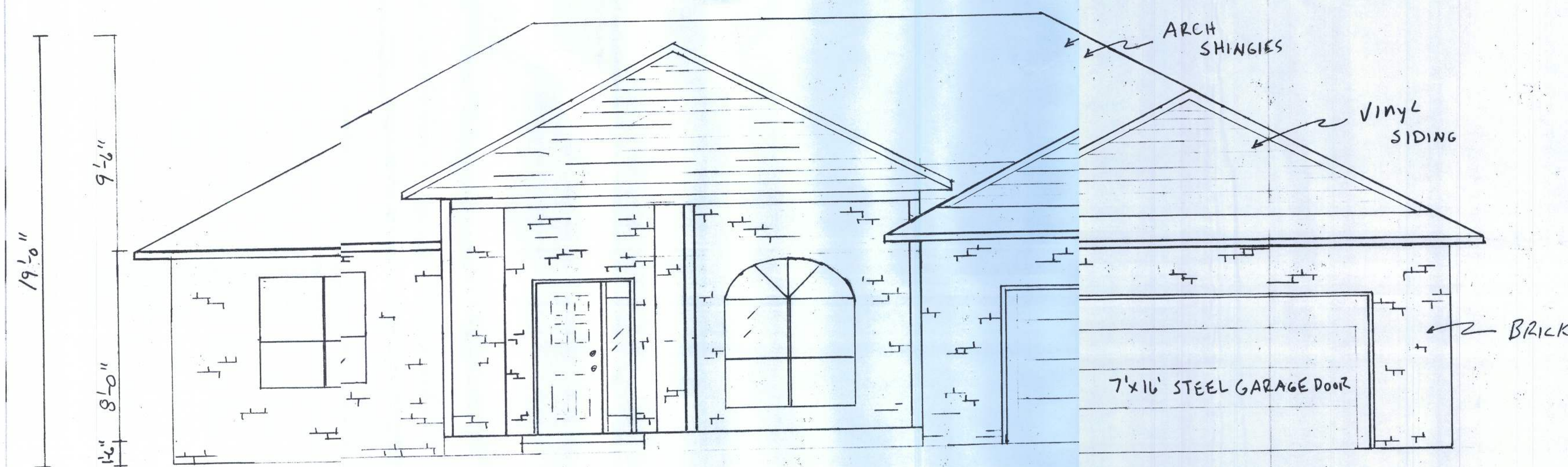
REAR ELEVATION



RIGHT SIDE ELEVATION

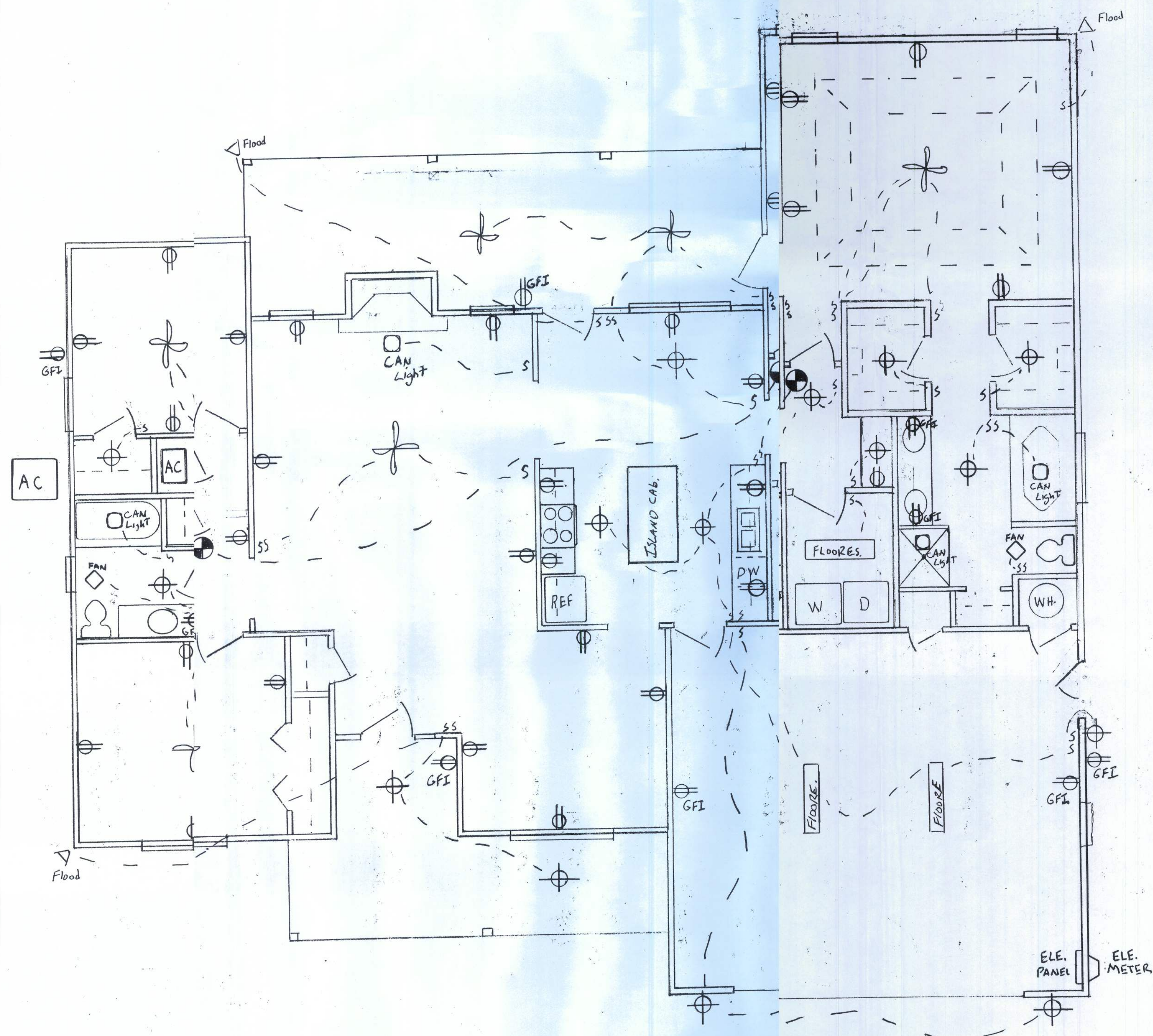


LEFT SIDE ELEVATION



FRONT ELEVATION 1"0" = 1/4"

HARRIS IS HOUSE
MILTON N BUILDERS LLC.

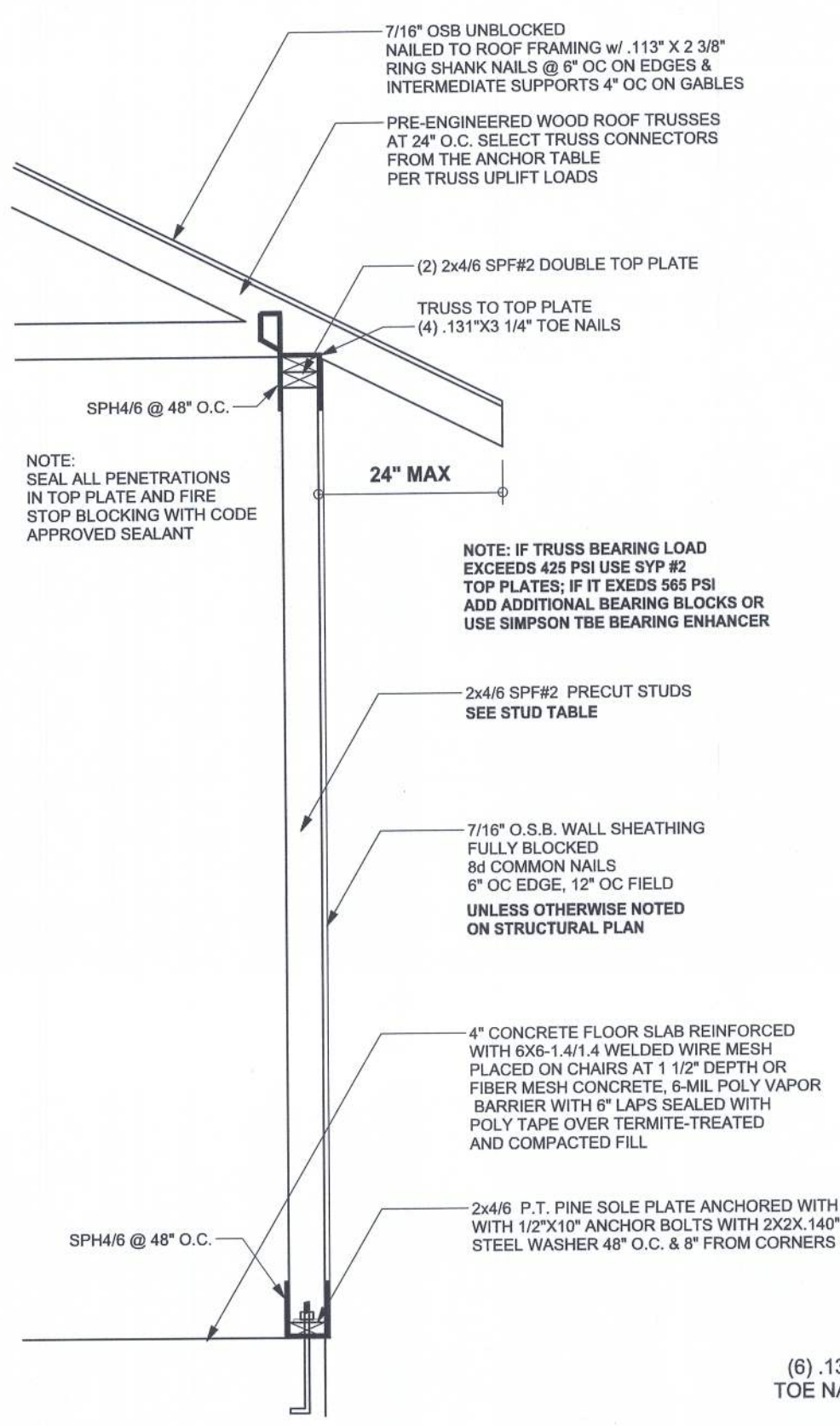


ELECTRICAL

- 110 VAC Plug
- GROUND FAULT Plug
- Light Fix.
- SMOKE DETECTOR
- CAN LIGHT
- FAN Light
- ARC Vault Breaker

NOTE All Plugs in house child protected.

ELECTRICAL PLAN 1"=1/4"

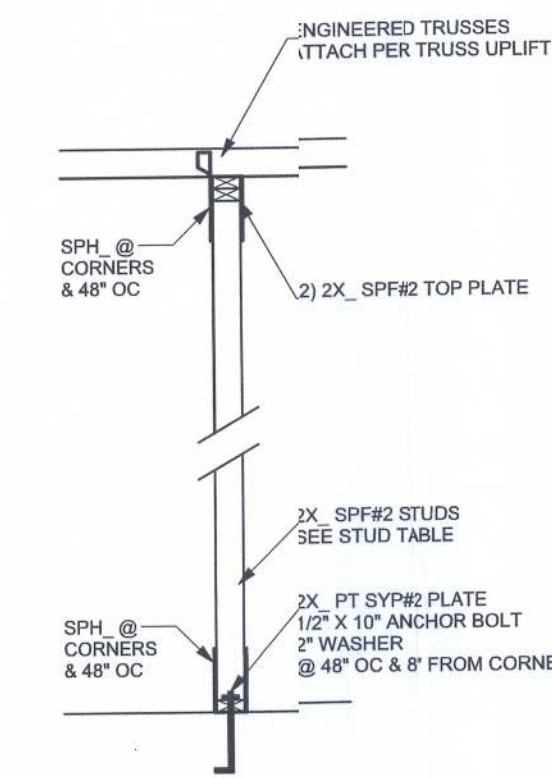


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

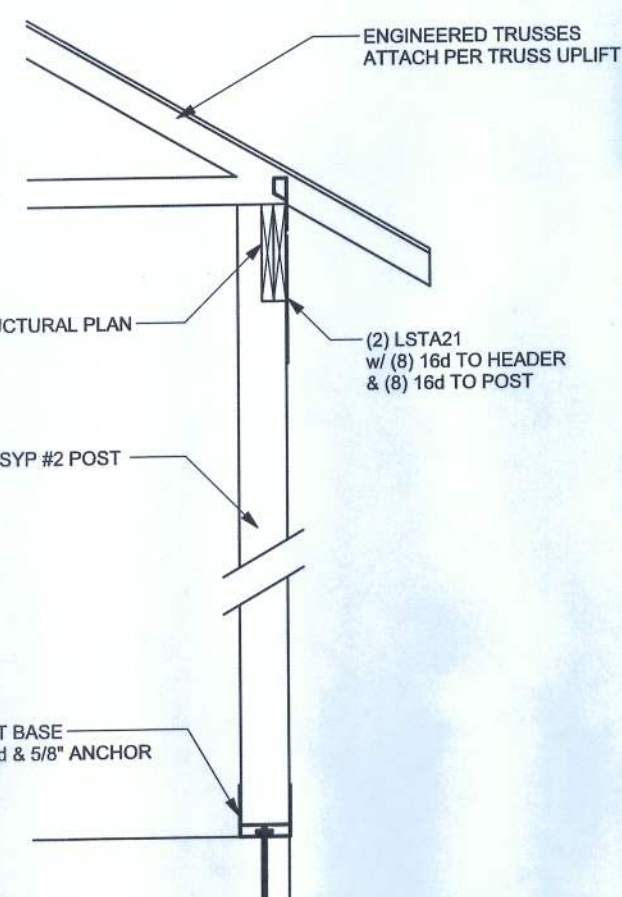
EXTERIOR WALL STUD TABLE
FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-6" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-7" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 16'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 18'-7" STUD HEIGHT

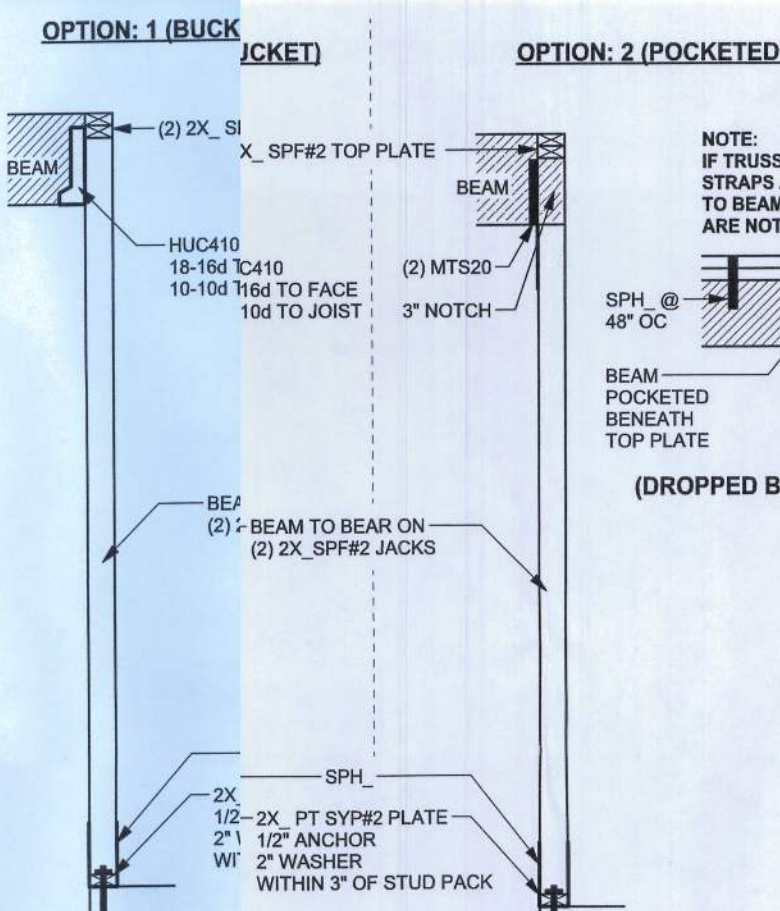
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE C. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME WITH STRAPS & ANCHORS



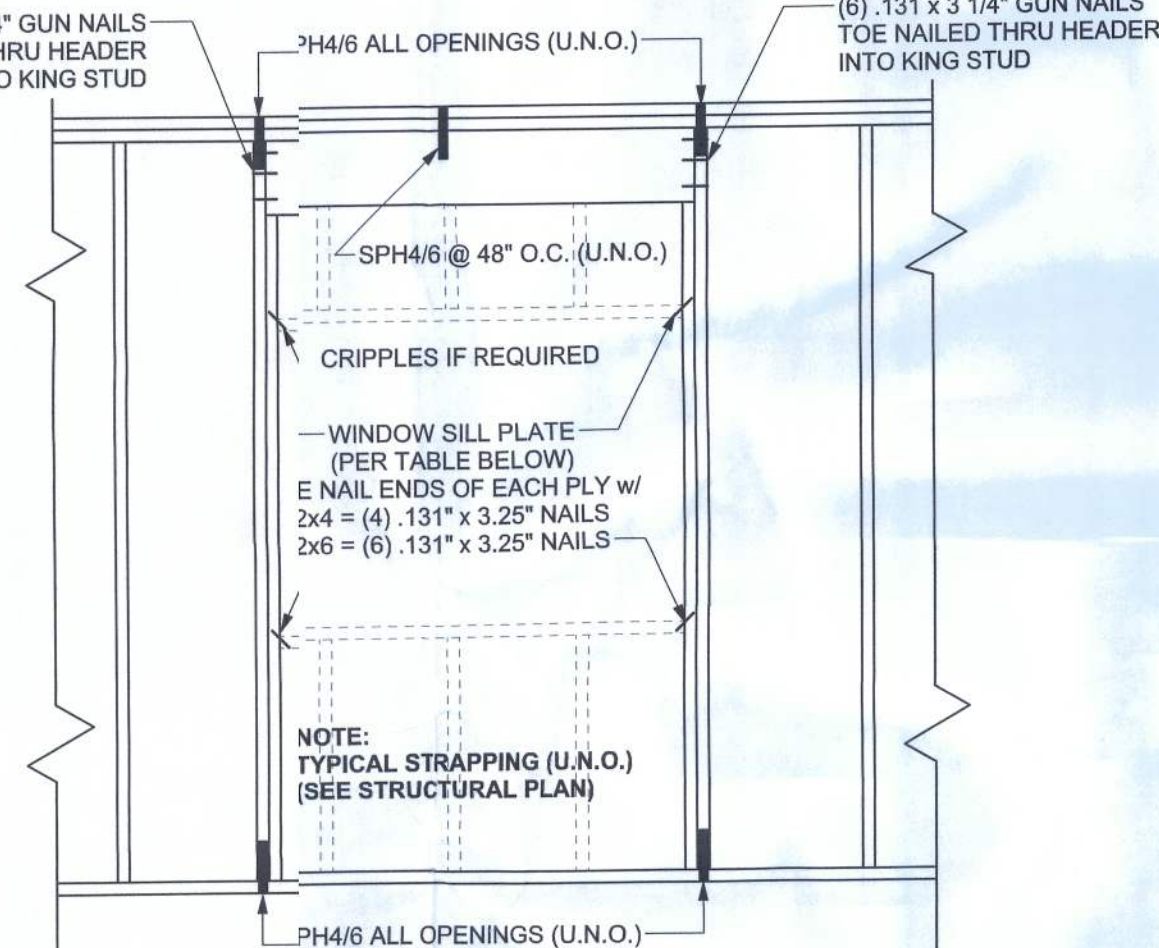
(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME WITH STRAPS & ANCHORS

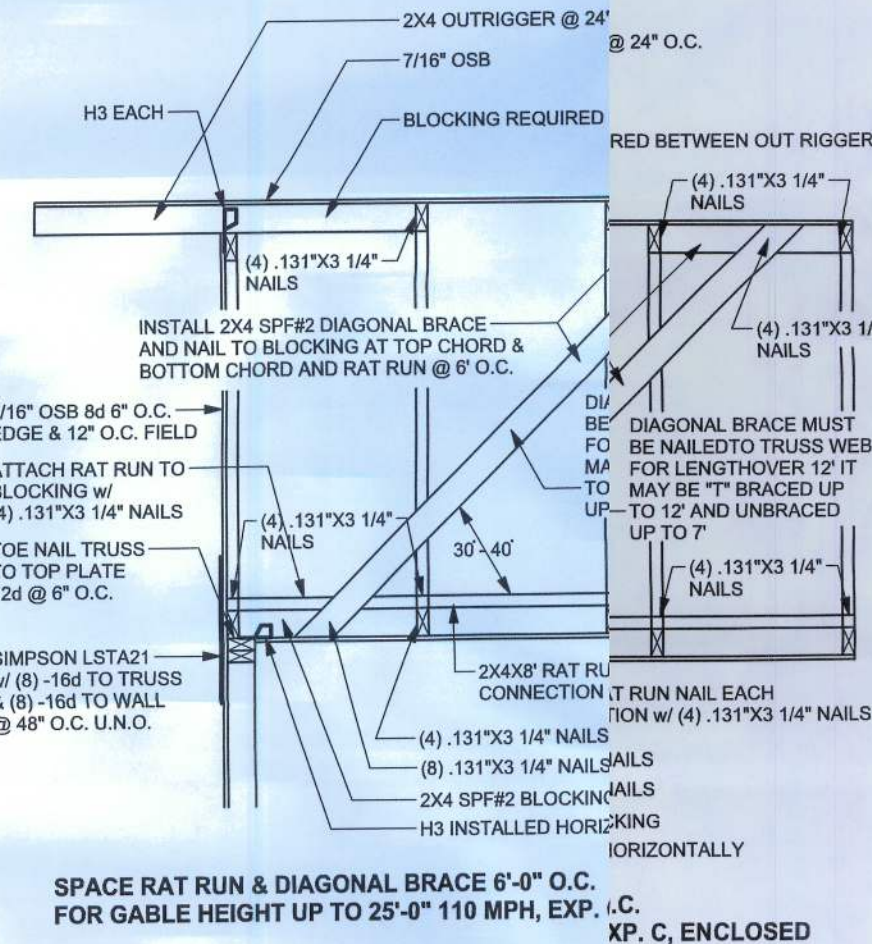
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE 3.20B
90-100 MPH	4' (2) 2x4	7'-0" 7'-0" 11'-4"
110-120 MPH	3' 6"	6'-5" 9'-6"
130 MPH	2' 6"	5'-11" 8'-9"

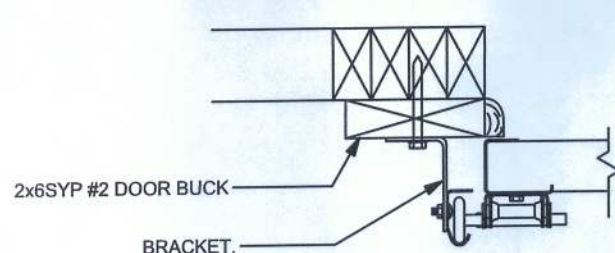
TYPIC HEADER STRAPPING DETAIL
SCALE: 1/2" = 0'-0"



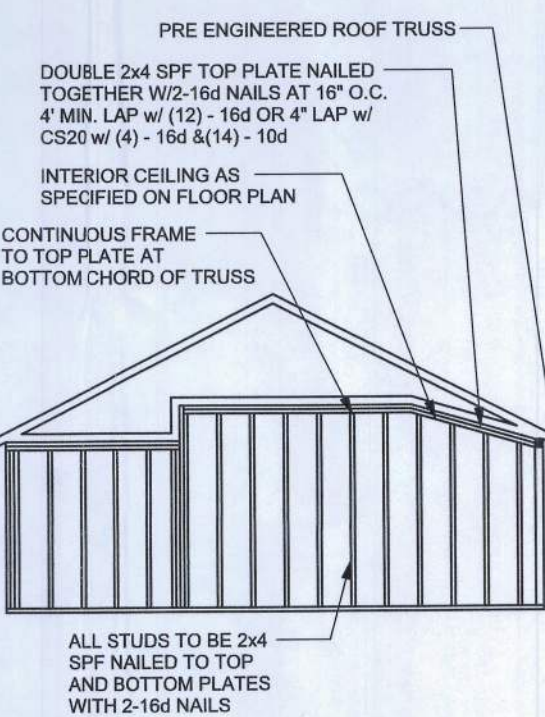
(TYP.) GABLE BRACING DETAIL
WOOD FRAME

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131 x 3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF 131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 10FT. DO NOT CUT WWI OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2400$ psi, $E = 1800$ ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 6d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 1" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 9/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL, WIND, HEIGHT AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMBT A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

1.4A	Compressive strength	8" block bearing walls $F'_m = 1500$ psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	Cmu standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5A	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d, 1 1/2"	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d, 1 1/2"	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1900	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG12	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*					
TO FOUNDATION					
< 3965	< 3330	MG1		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6 - 10d, 1 1/2"
< 1240	< 1065	SPH4			10 - 10d, 1 1/2"
< 885	< 760	SP6			6 - 10d, 1 1/2"
< 1240	< 1065	SPH6			10 - 10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
STUD ANCHORS*					
< 1350	< 1305	LTT19	8 - 16d		1/2" AB
< 2310	< 2310	LTT131	18 - 10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2 - 5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAND42	16 - 16d		
< 3335	< 3335	HPAH22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1/2" AB
< 2300	< 2300	ABU66	12 - 16d		1/2" AB
< 2320	< 2320	ABU88	16 - 16d		2 - 5/8" AB

DESIGN DATA

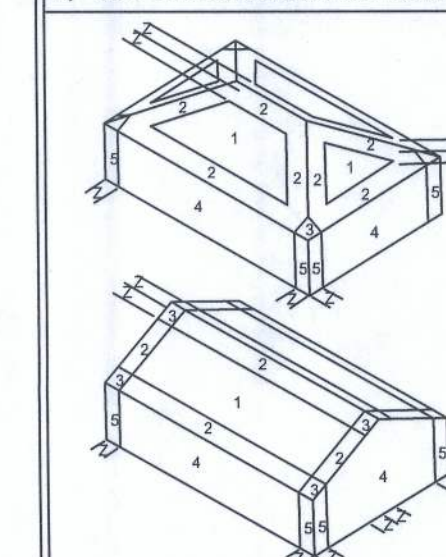
WINDLOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBTSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1) BASIC WIND SPEED = 110 MPH
- 2) WIND EXPOSURE = C
- 3) WIND IMPORTANCE FACTOR = 1.0
- 4) BUILDING CATEGORY = II
- 5) ROOF ANGLE = 10-45 DEGREES
- 6) MEAN ROOF HEIGHT = <30 FT
- 7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)	10	100
1	27.8	-30.5	25.3
2	27.8	-35.7	25.3
3	27.8	-35.7	25.3
4	30.5	-33.0	25.9
5	30.5	-40.7	25.9

Doors & Windows	30.5	-40.7
Worst Case (Zone 5, 10 ft)		
8x7 Garage Door	27.3	-32.0
16x7 Garage Door	25.9	-29.4

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)	
30 PSF (ATTICS WITH STORAGE)	
10 PSF (ATTICS WITHOUT STORAGE, <3.12)	
ROOF	20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)	
12 PSF (12:12 AND GREATER)	
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME

(TYP.) CORR FRAMING
WOOD FRAME

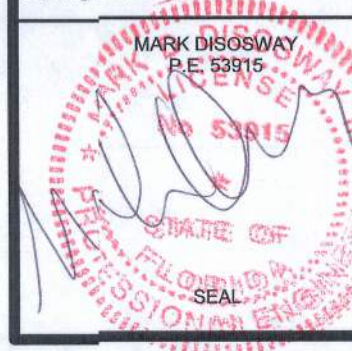
WINDLOAD ENGINEER: Mark Disoway, P.E. No. 0115, P.O. Box 868, Lake City, FL 32056, 38-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code regulation 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Milton Builders

Jarris Residence

ADDRESS:
Lake Jeffery Road
olumbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
March 14, 2011

DRAWN BY: STRUCTURAL BY:
David Disoway

FINISH DATE:
11Mar11

JOB NUMBER:
1103016

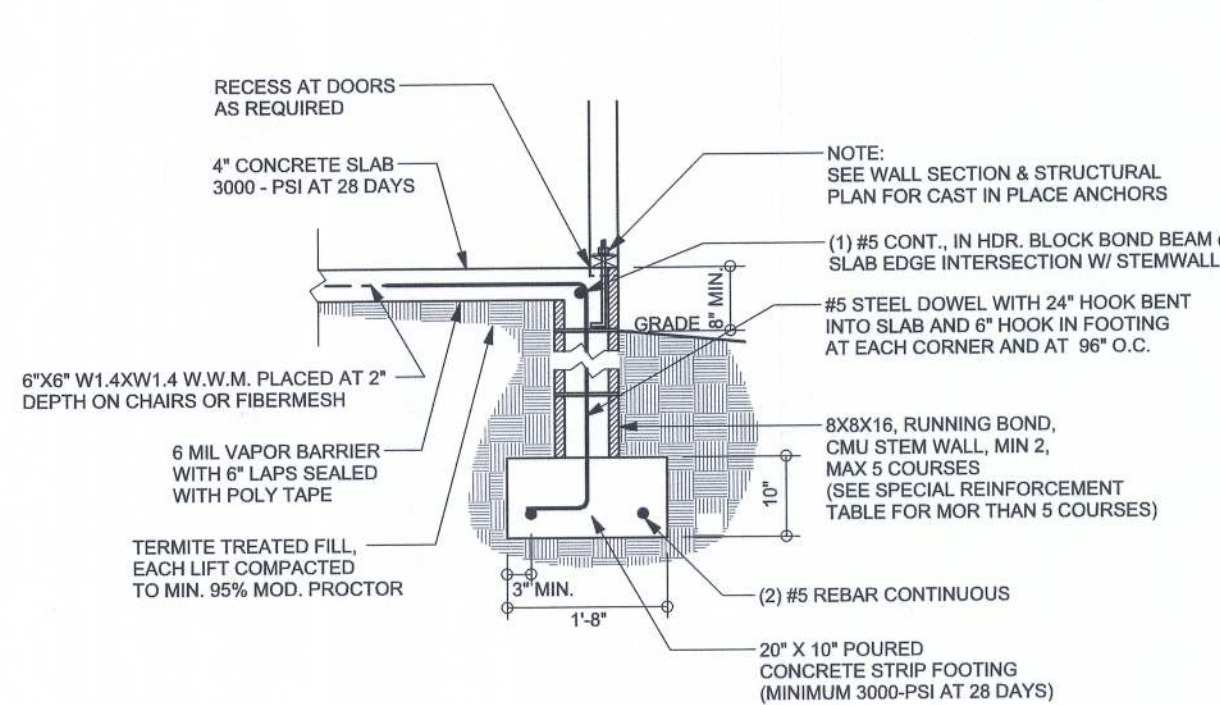
DRAWING NUMBER

S-1

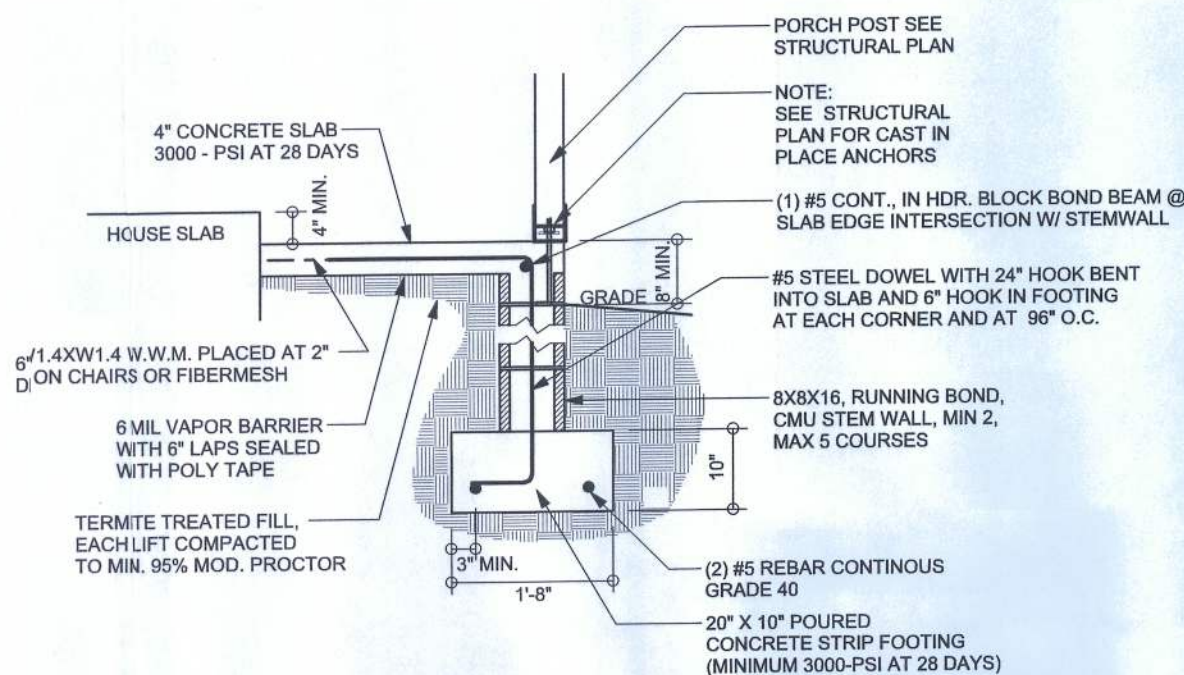
OF 3 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

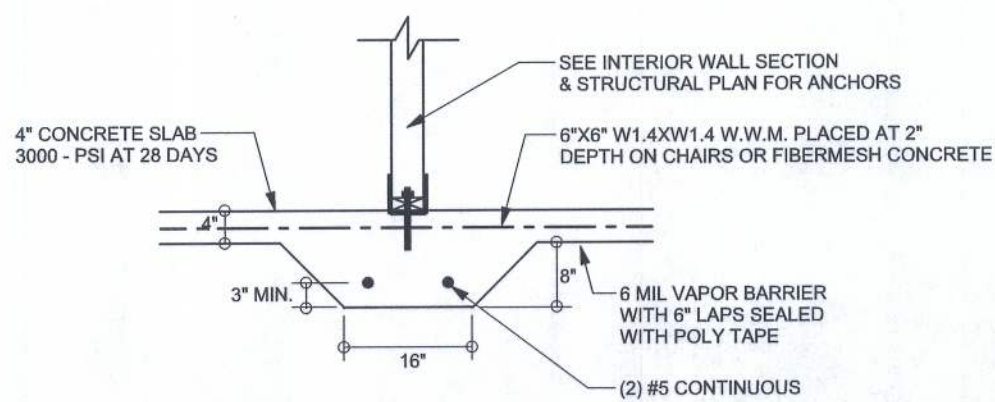


F12 S-2 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

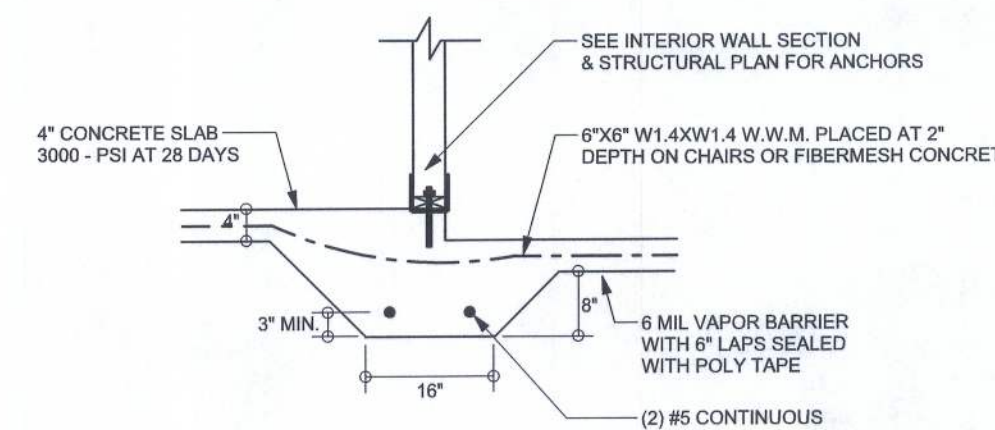
FALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diagonal ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall, 12" CMU may be used with reinforcement as shown in the table below.

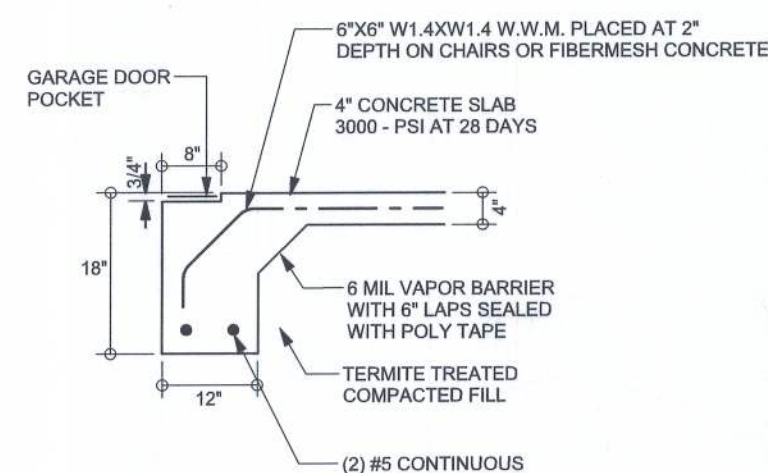
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



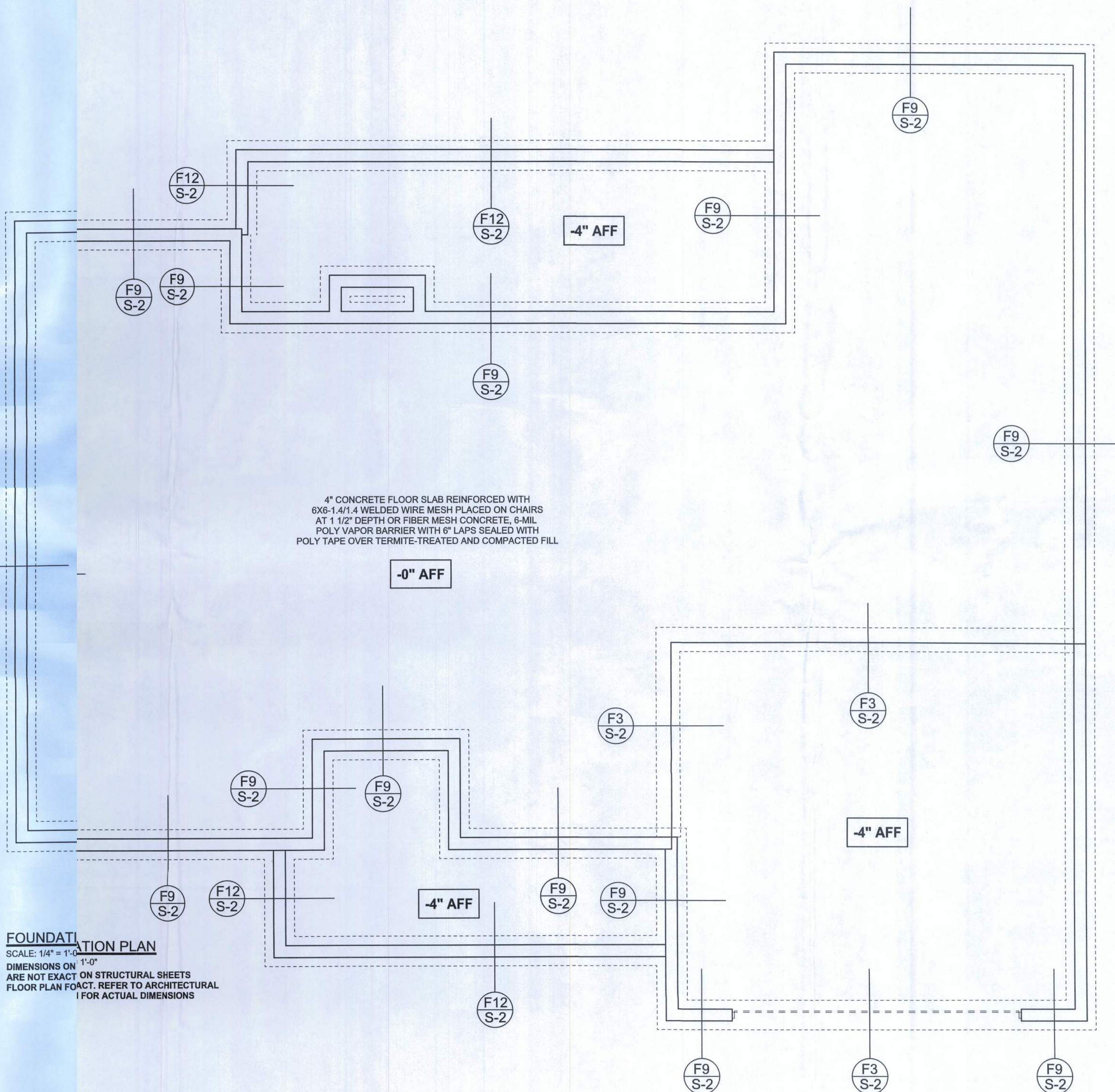
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"
DIMENSIONS ON ARE NOT EXACT ON STRUCTURAL SHEETS FLOOR PLAN FOACT. REFER TO ARCHITECTURAL FOR ACTUAL DIMENSIONS

WINDLOAD/ENGINEER: Mark Disoway, PE No.5395, PCB 866, Lake City, FL 32056, 386/754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined the plan, and that the applicable portions of the plan, relating to wind engineering comply with section F501.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, as detailed location.



Milton Builders

Harris Residence

ADDRESS:
Lake Jeffery Road
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 866
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
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DRAW BY: STRUCTURAL BY:
David Disoway

FINAL DATE:
11Mar11

JOB NUMBER:
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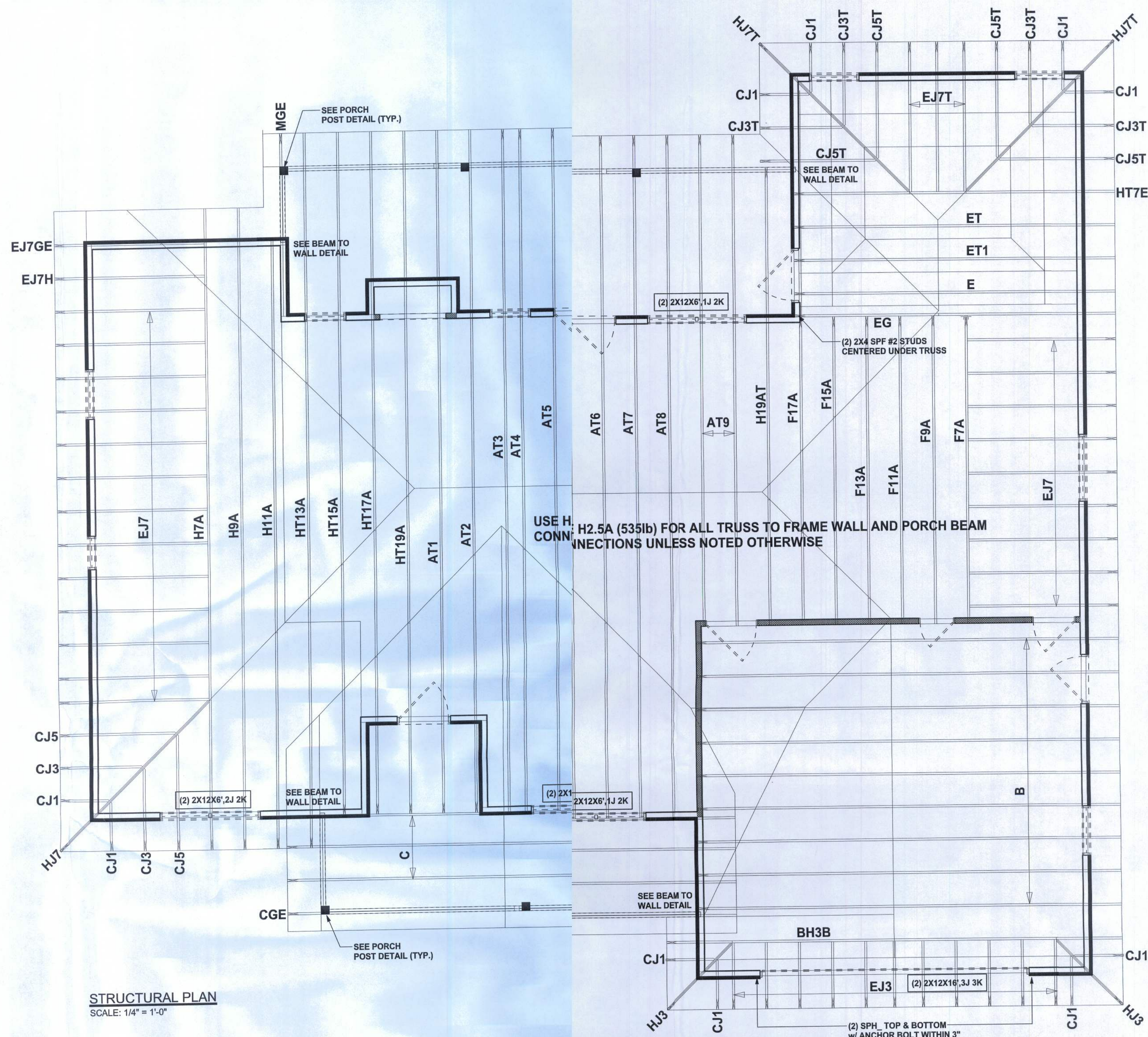
DRAWING NUMBER

S-2

OF 3 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER)
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	46.8'	98.1'
LONGITUDINAL	40.0'	67.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. JOB #11-043

WINLOAD ENGINEER: Mark Disoway, P.E. 53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stair dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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