30779

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1UUJ487-Z0114095420

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

Truss Fabricator:	Anderson Truss Company
Job Identification:	13-029B--Doug Edgley Durham Res II -- Lake City, FL
Truss Count:	45
Model Code:	Florida Building Code 2010
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: 12015EC1-GBLLETIN-GABRST10-BRCLBSUB-12030EC1-PB16010-

#	Ref	Description	Drawing#	Date
1	67194-A	37'6" Stepdown	13073025	03/14/13
2	67195-A1	37'6" Stepdow	13073006	03/14/13
3	67196-A2	37'6" Stepdow	13073003	03/14/13
4	67197-A3	37'6" Stepdow	13073002	03/14/13
5	67198-A4	37'6" Stepdow	13073030	03/14/13
6	67199-A5	37'6" Stepdow	13073005	03/14/13
7	67200-H11AM	37'6" Step	13073001	03/14/13
8	67201-H15AM	37'6" Step	13073002	03/14/13
9	67202-H17AM	37'6" Step	13073003	03/14/13
10	67203-H19AM	37'6" Step	13073004	03/14/13
11	67204-H21AM	37'6" Mono	13073005	03/14/13
12	67205-H7AM	37'6" Stepd	13073014	03/14/13
13	67206-H9AM	37'6" Stepd	13073006	03/14/13
14	67207-H13AM	37'6" Step	13073007	03/14/13
15	67208-H11A	43' Stepdow	13073008	03/14/13
16	67209-H13A	43' Stepdow	13073009	03/14/13
17	67210-H15A	40' Stepdow	13073010	03/14/13
18	67211-H17A	40' Stepdow	13073011	03/14/13
19	67212-H19A	40' Stepdow	13073012	03/14/13
20	67213-H21A	37'6" Stepd	13073014	03/14/13
21	67214-H7A	43' Stepdown	13073015	03/14/13
22	67215-H9A	43' Stepdown	13073013	03/14/13
23	67216-B	44'4" Stepdown	13073019	03/14/13
24	67217-B1	44'2" Stepdow	13073020	03/14/13
25	67218-B2	36'10" Stepdo	13073028	03/14/13
26	67219--BDG	44'4" Gable	13073029	03/14/13
27	67220--C	24' Common	13073015	03/14/13
28	67221--CDG	24' Gable	13073013	03/14/13
29	67222--CP	8'3"10 Gable	13073016	03/14/13
30	67223--CJ1	1' Jack	13073009	03/14/13
31	67224--CJ3	3' Jack	13073010	03/14/13
32	67225--CJ3A	3' Jack	13073026	03/14/13
33	67226--CJ5	5' Jack	13073001	03/14/13
34	67227--CJ5A	5' Jack	13073024	03/14/13
35	67228--EJ7	7' End Jack	13073011	03/14/13
36	67229--EJ7	7' End Jack	13073023	03/14/13

#	Ref	Description	Drawing#	Date
37	67230-EJ7A	7' End Jack	13073027	03/14/13
38	67231-HJ	9'10"13 Hip J	13073012	03/14/13
39	67232-HJ1A	9'10"13 Hip	13073007	03/14/13
40	67233-BPB	5'8"10 Comm	13073008	03/14/13
41	67234-BPB	6'0"10 Comm	13073021	03/14/13
42	67235-BP1	8'11"10 Gabl	13073022	03/14/13
43	67236-APB	4'11"14 Comm	13073017	03/14/13
44	67237-APB1	4'11"14 Ste	13073004	03/14/13
45	67238-APB2	18'5"15 Mon	13073018	03/14/13

Replacement Set

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(c) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

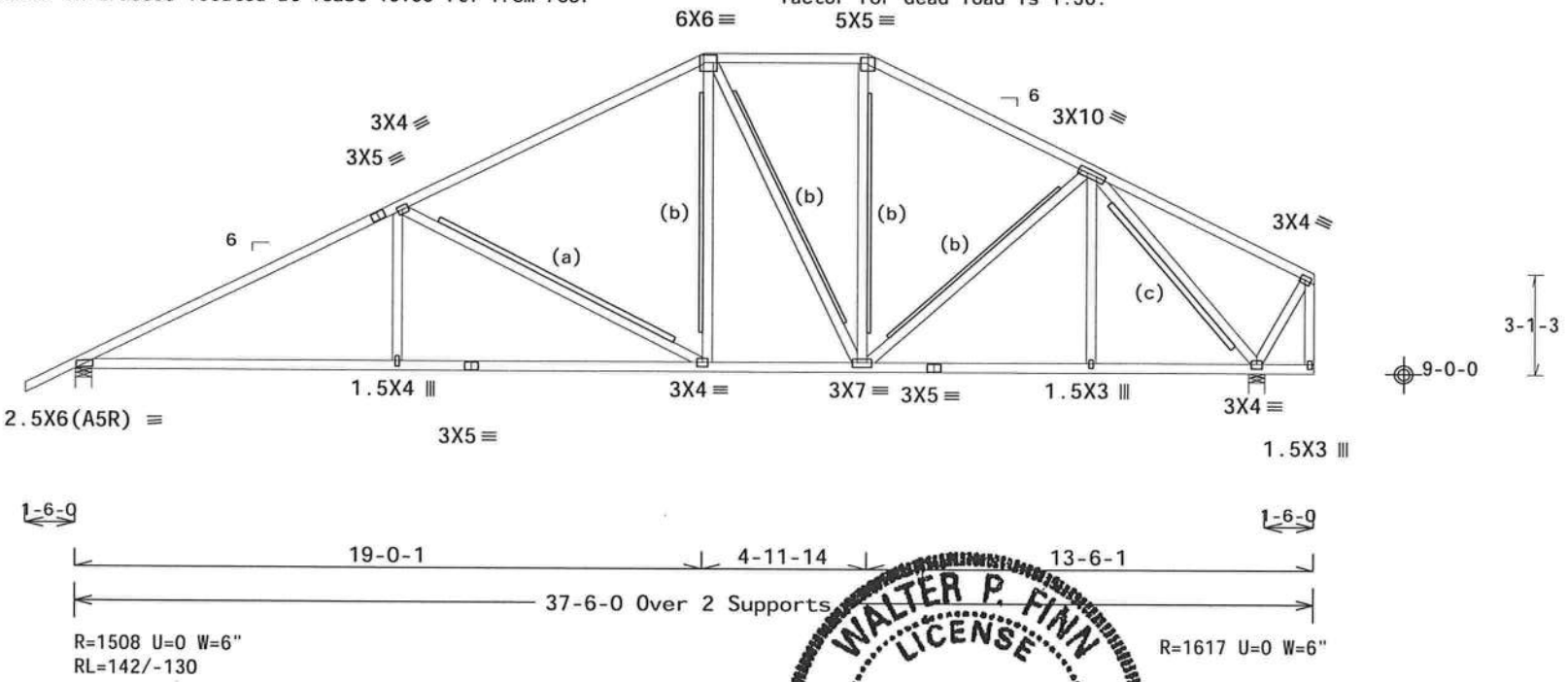
Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

0.03.11.0209.19 QTY:

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER 03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for standard practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page: ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	20.0 PSF	REF	R487-- 67194
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073025
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	66015
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A1 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

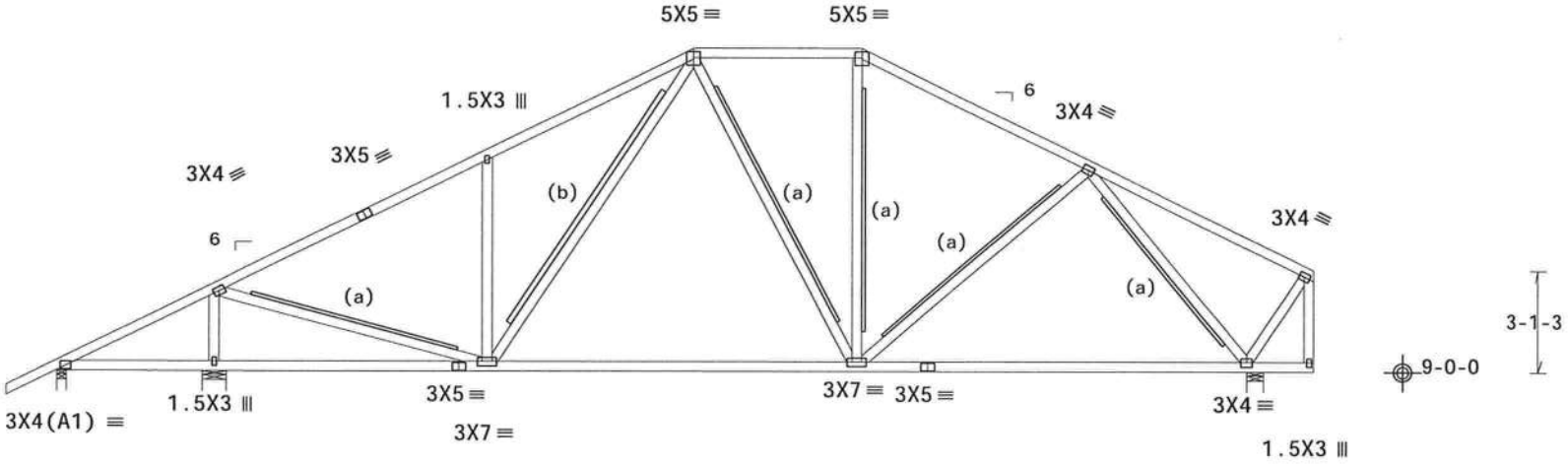
Wind loads and reactions based on MWFRS with additional C&C member design.

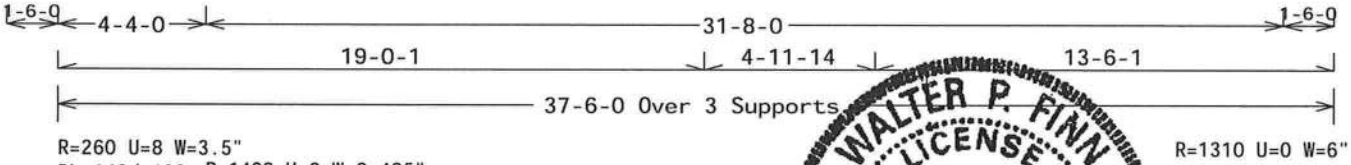
Right end vertical not exposed to wind pressure.

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=260 U=8 W=3.5"
RL=142/-130 R=1408 U=0 W=8.485"

R=1310 U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0809.19

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for general practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

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TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 67195
DATE	03/14/13
DRW	HCUSR487 13073006
HC-ENG	WHK/WHK
SEQN-	65978
JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A2 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

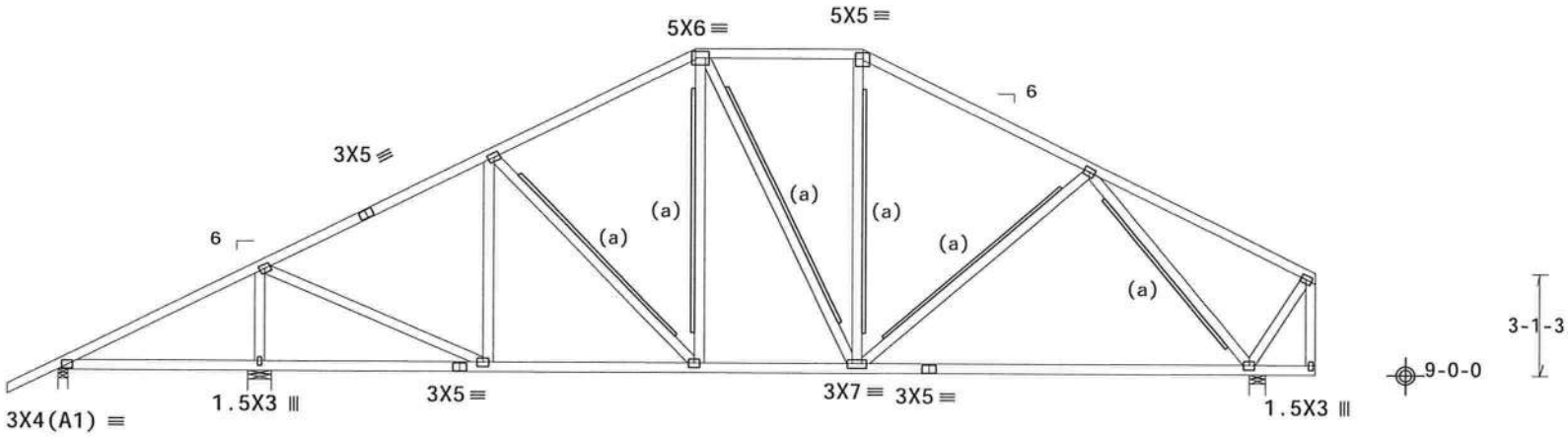
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

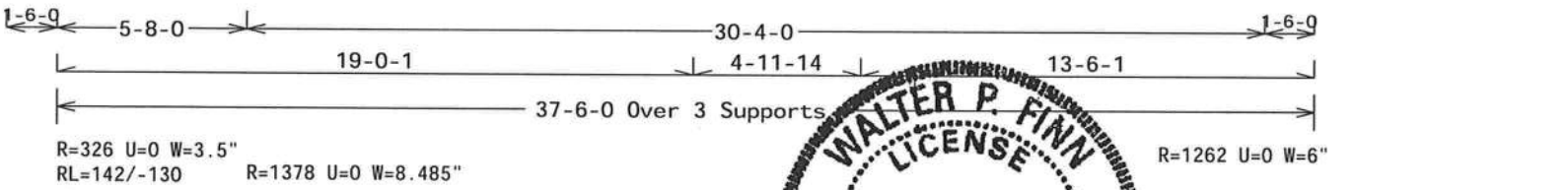
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=326 U=0 W=3.5"
RL=142/-130
R=1378 U=0 W=8.485"
R=1262 U=0 W=6"

Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 67196
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073003
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65971
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

ALPINE
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(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A3 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B3 2x4 SP M-30:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

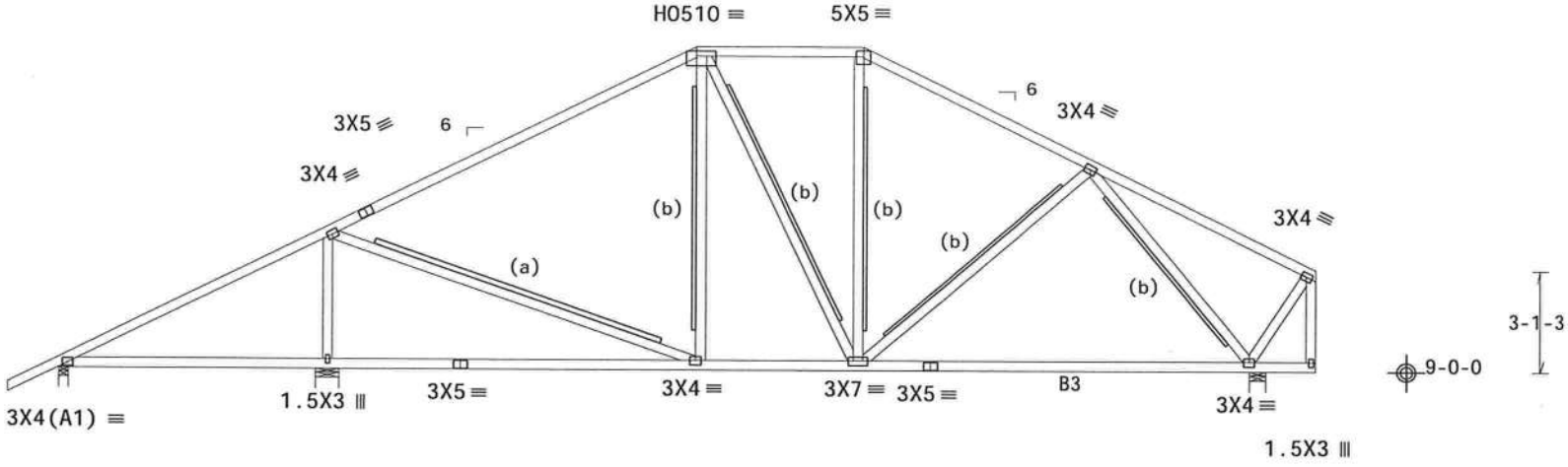
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 10.03.11.0309.19 QTY FL/-/5/-/-/R/- Scale =.1875"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67197
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65964
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A4 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

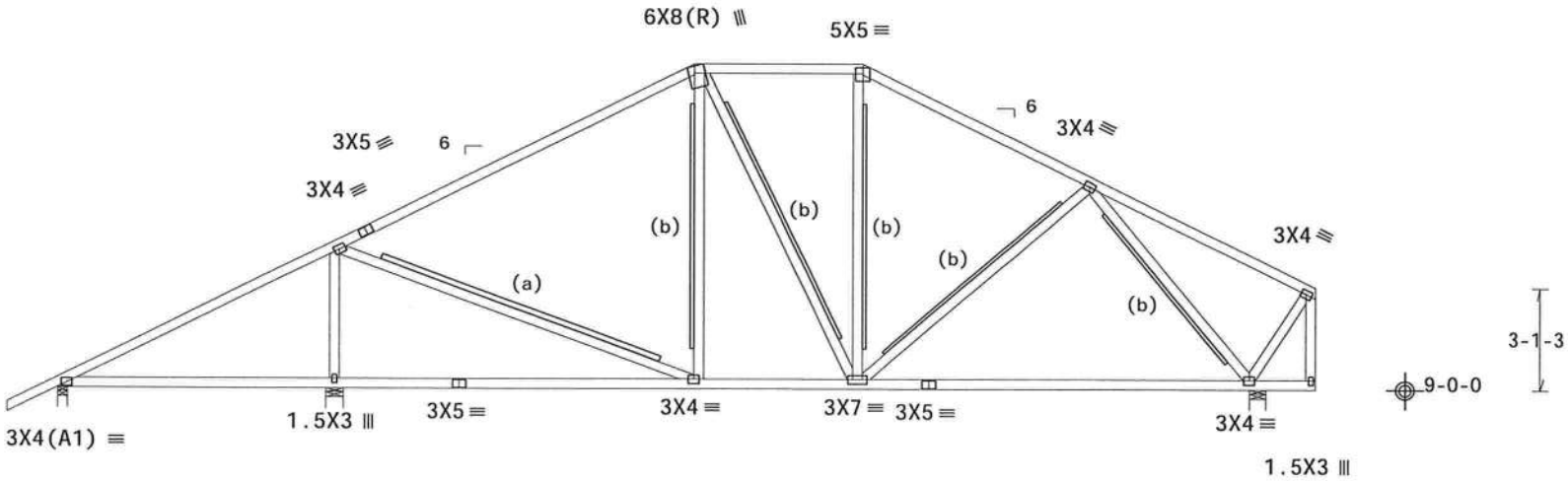
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

C: 03.11.0309.19

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE

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TC LL	20.0 PSF	REF	R487-- 67198
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073030
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65952
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - A5 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

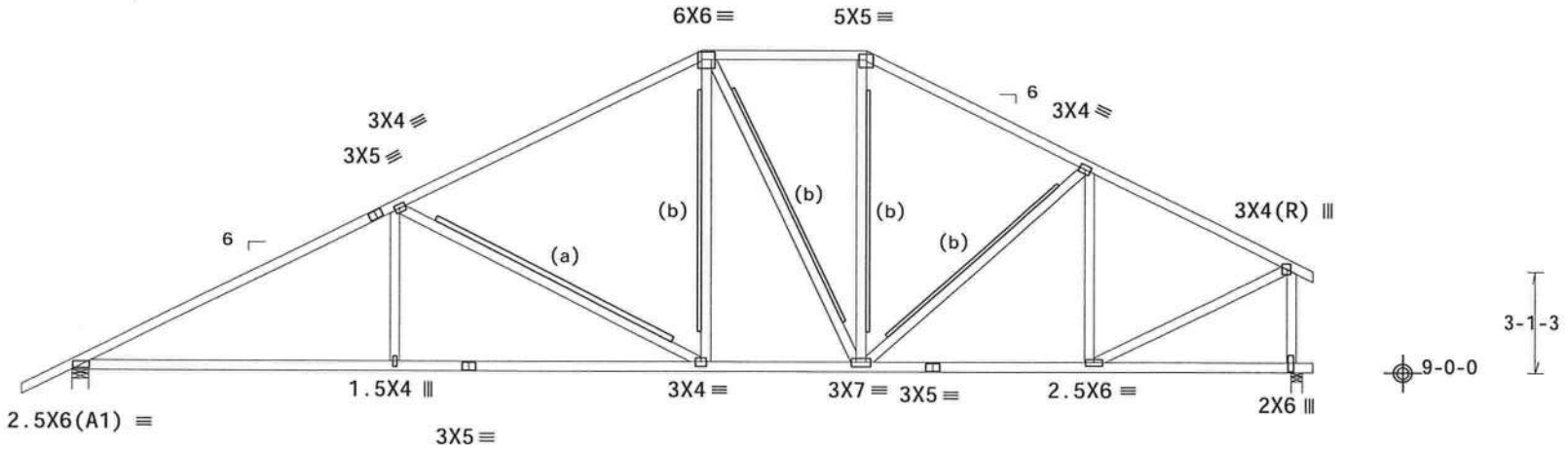
Wind loads and reactions based on MWFRS with additional C&C member design.

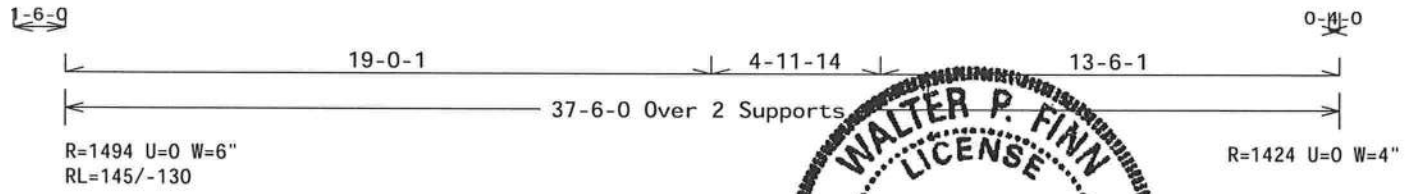
Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11 0809.19

QTY 1

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 67199
DATE	03/14/13
DRW	HCUSR487 13073005
HC-ENG	WHK/WHK
SEQN-	66025
JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H11AM 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A :T1 2x4 SP M-30:
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W1 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

Calculated horizontal deflection is 0.11" due to live load and 0.15" due to dead load.

(a) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

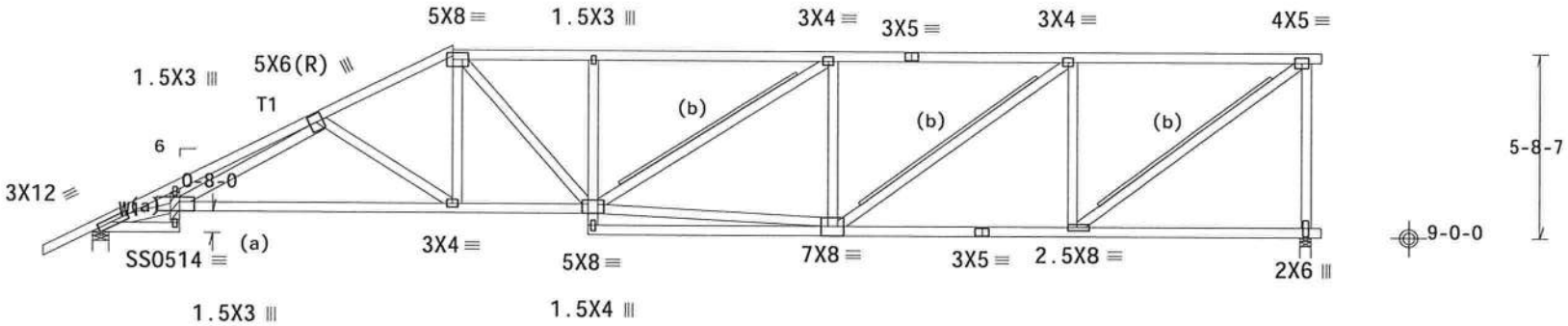
Wind loads and reactions based on MWFRS with additional C&C member design.

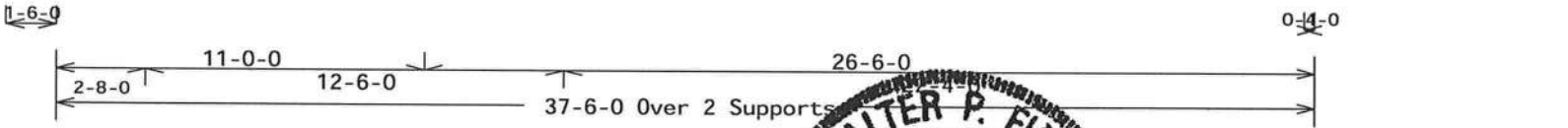
Right end vertical not exposed to wind pressure.

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=1517 U=60 W=6"
RL=103/-38

R=1405 U=70 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209 19 QTY: 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67200
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66897
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H15AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

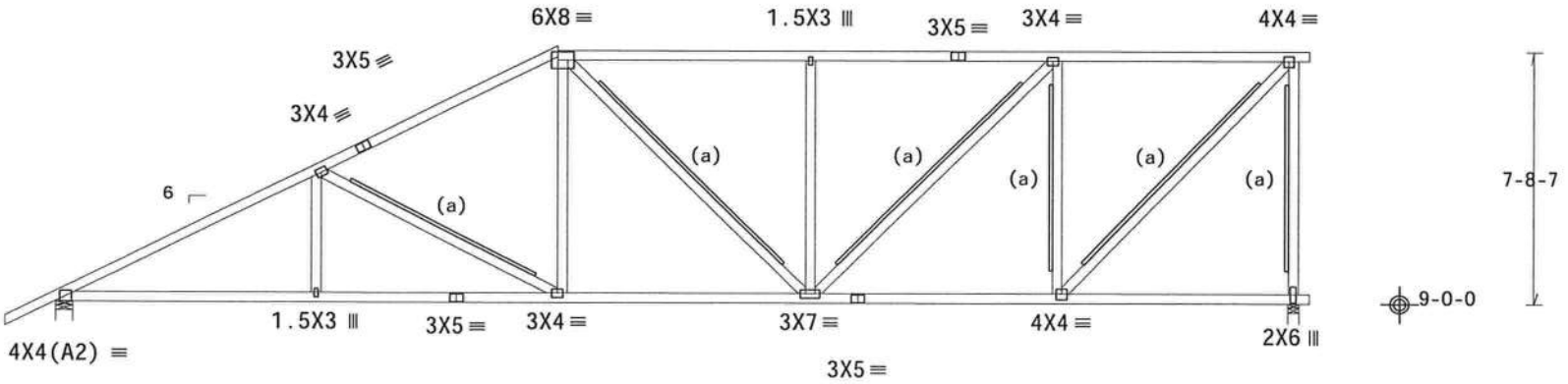
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

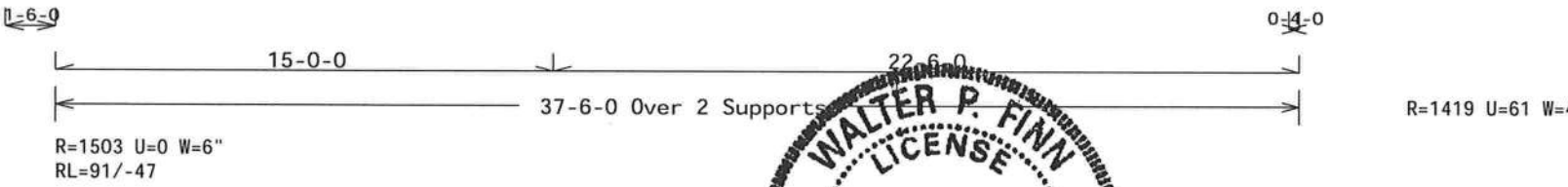
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) ★10.03.11 0809.19 ★ QTY

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF	REF	R487-- 67201
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073002
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66910
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H17AM 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

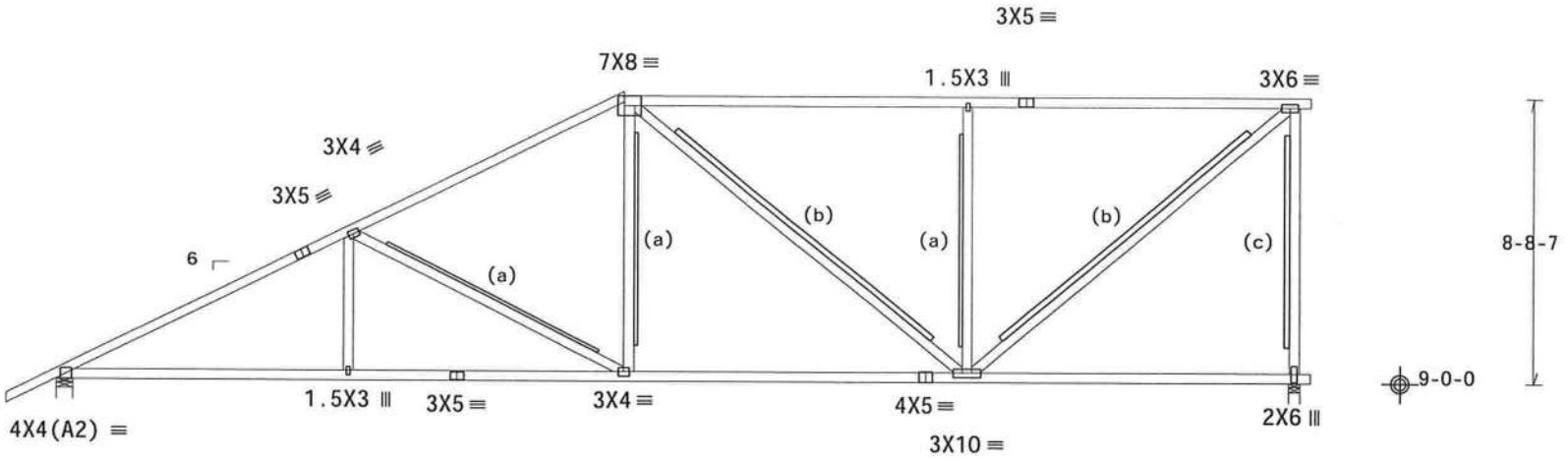
Wind loads and reactions based on MWFRS with additional C&C member design.

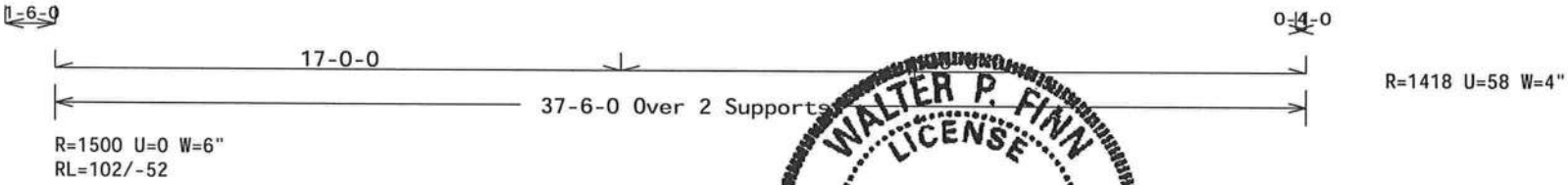
Right end vertical not exposed to wind pressure.

(c) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD
FT/RT=10%(0%)/0(0)
10.03.11.0259.19 QTY: 1

PLT TYP. Wave

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67202
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073003
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66919
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H19AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(c) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

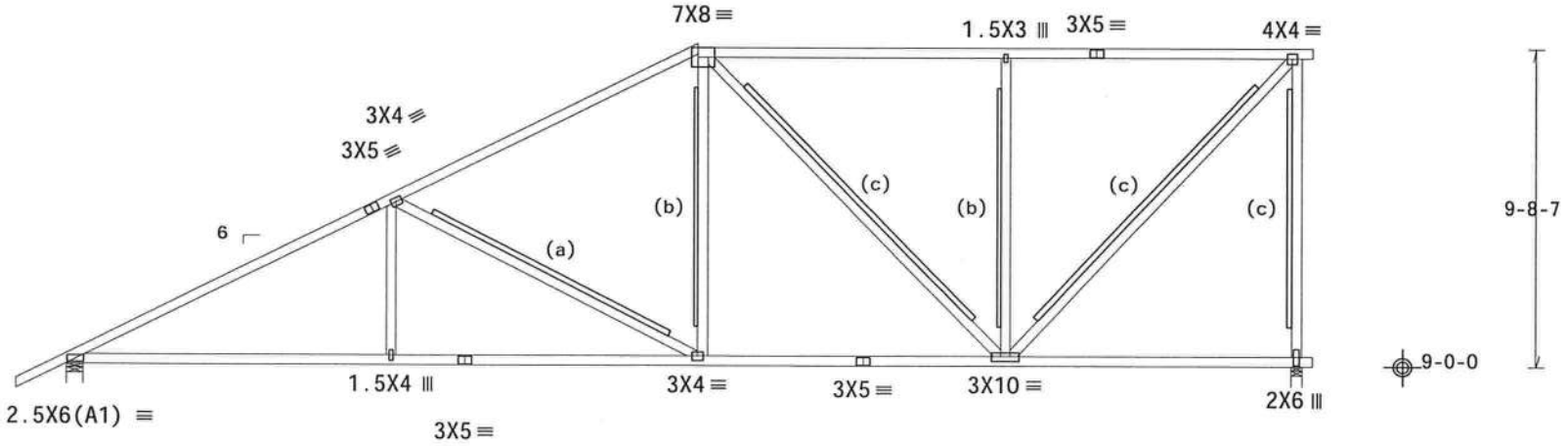
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11 0209.19 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67203
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073004
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66927
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H21AM 37'6" Mono Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B3 2x4 SP M-30:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(d) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

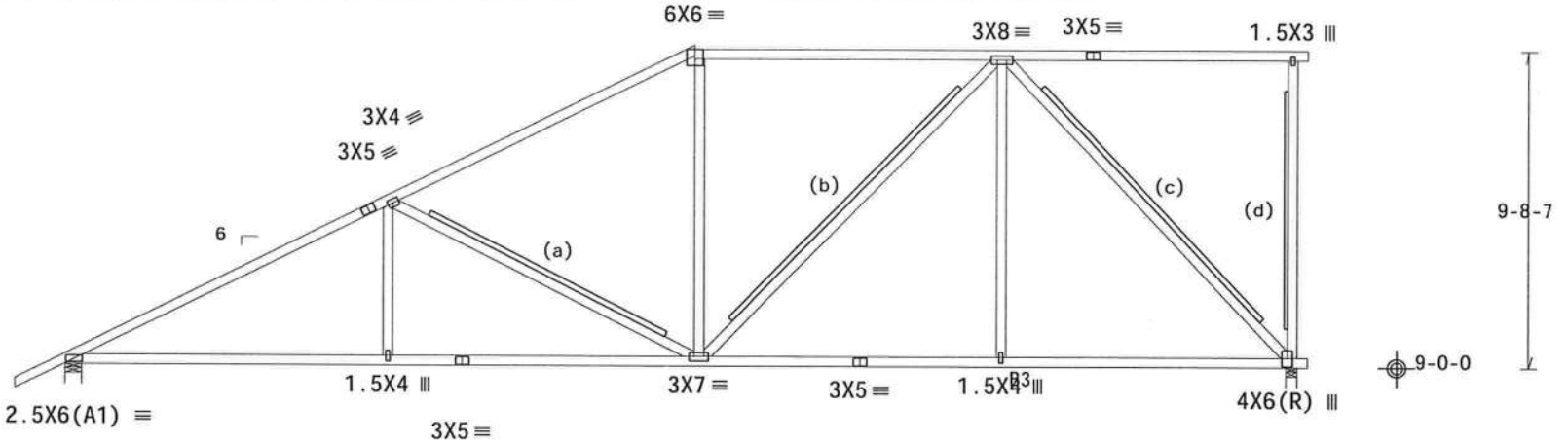
Right end vertical not exposed to wind pressure.

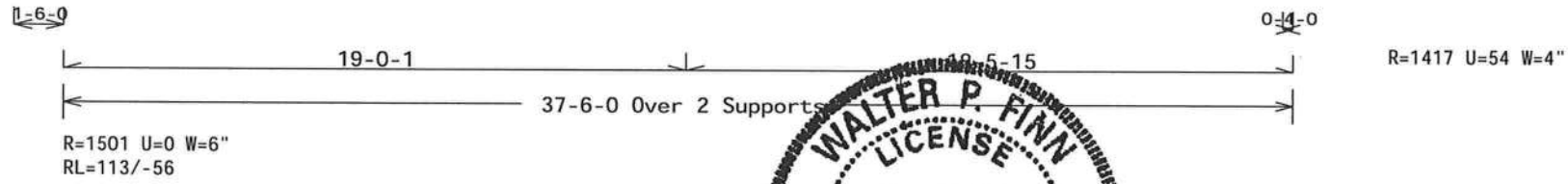
(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

(c) 2x8 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0809.19

QTY:

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF	REF	R487-- 67204
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073005
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66942
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H7AM 37'6" Stepdown Hip Girder)

Top chord 2x4 SP M-30 :T1 2x4 SP_#1_12A:
Bot chord 2x4 SP_#1_12A :B4 2x4 SP M-30:
Webs 2x4 SP_#3__12A
:W1, W2, W8, W9, W14 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -1.50 to 56 plf at 7.00
TC- From 28 plf at 7.00 to 28 plf at 21.00
TC- From 28 plf at 21.00 to 28 plf at 29.00
TC- From 28 plf at 29.00 to 28 plf at 37.50
BC- From 4 plf at -1.50 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 2.67
BC- From 20 plf at 2.67 to 20 plf at 7.03
BC- From 10 plf at 7.03 to 10 plf at 15.17
BC- From 10 plf at 15.17 to 10 plf at 27.17
BC- From 10 plf at 27.17 to 10 plf at 37.17
BC- From 4 plf at 37.17 to 4 plf at 37.50
TC- 227.25 lb Conc. Load at 7.02
TC- 123.55 lb Conc. Load at 9.06,11.06,13.06,15.06
TC- 167.95 lb Conc. Load at 17.06,19.06,21.06,21.94
23.94,25.94,27.94,29.94,31.94,33.94,35.97
BC- 375.59 lb Conc. Load at 7.03
BC- 144.42 lb Conc. Load at 9.06,11.06,13.06,15.06
BC- 126.35 lb Conc. Load at 17.06,19.06,21.06,21.94
23.94,25.94,27.94,29.94,31.94,33.94,35.97

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

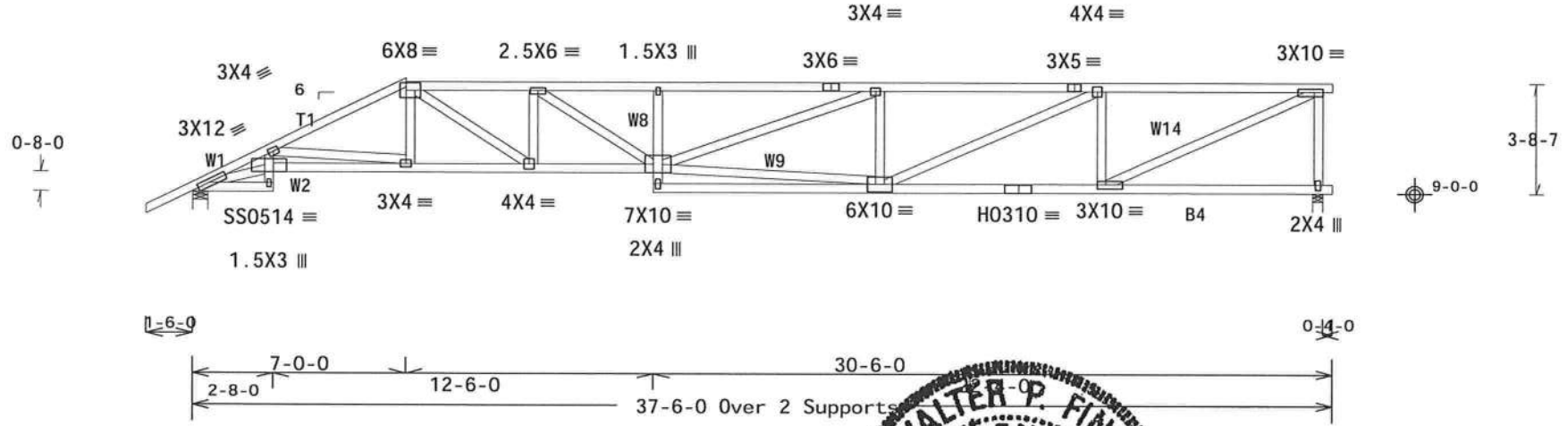
Right cantilever is exposed to wind

Calculated horizontal deflection is 0.13" due to live load and 0.16" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Calculated vertical deflection is 0.50" due to live load and 0.64" due to dead load at X = 15-3-12.



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)
C.03.11.0809.19
QTY:
FL/-/5/-/-/R/-
Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF	R487-- 67205
DATE	03/14/13
DRW	HCUSR487 13073014
HC-ENG	JB/WPF
SEQN-	66879
JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H9AM 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W1, W2, W7 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

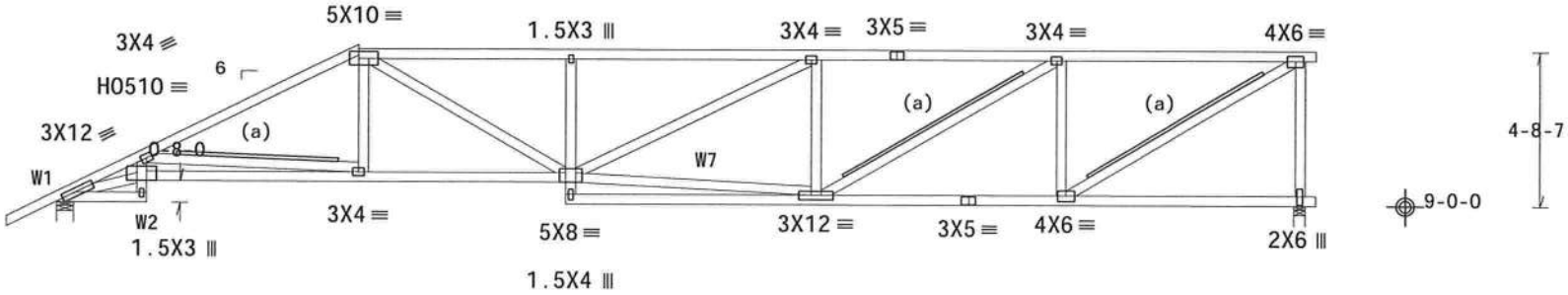
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1517 U=64 W=6"
RL=86/-34

R=1405 U=68 W=4"

Design Crit: FBC2010Res/TPI-2007(STD

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

GTI

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67206
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073006
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66889
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H13AM 37'6" Stepdown Hip)

Top chord 2x4 SP_#1_12A :T1 2x4 SP M-30:
Bot chord 2x4 SP_#1_12A :B2 2x4 SP M-30:
Webs 2x4 SP_#3__12A :W1, W2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

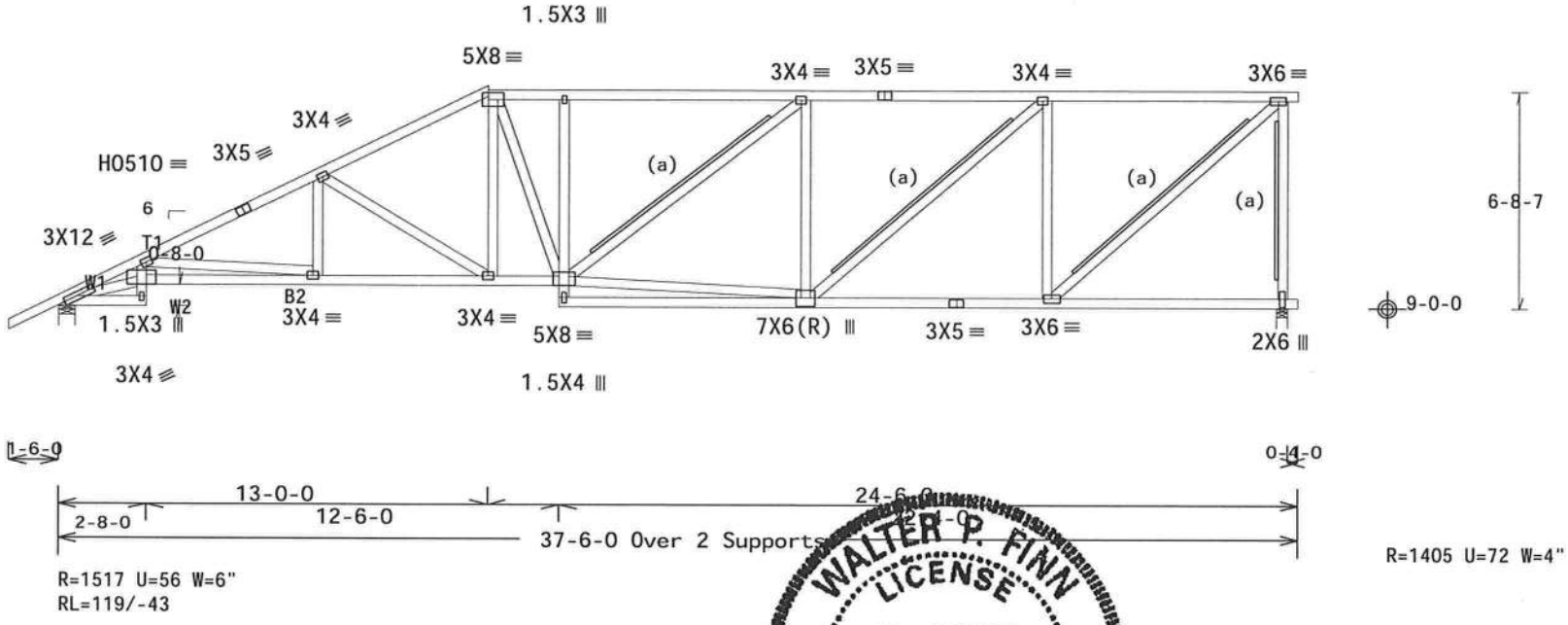
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.11.0909.19

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

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03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67207
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073007
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66905
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H11A 43' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

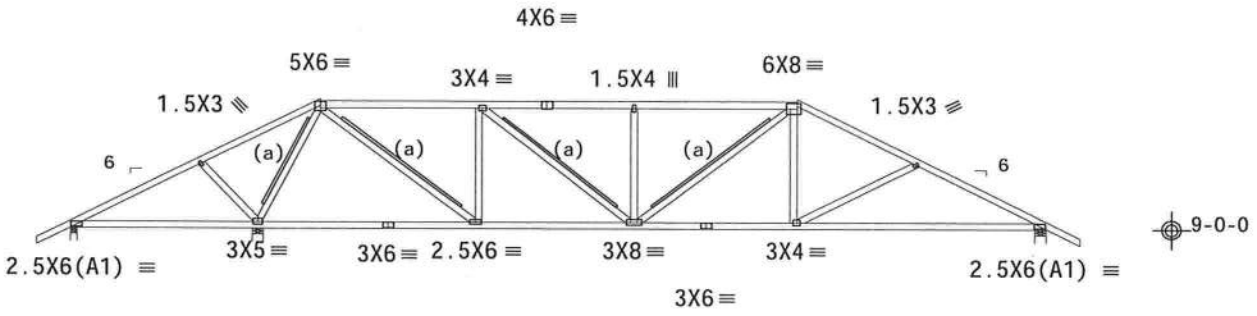
Negative reaction(s) of -213# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=219/-214 U=35 W=3.5"
RL=109/-109

R=2109 U=86 W=6"

R=1320 U=59 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) ★C.03.11.0809.19 ★DT★

FL/-/5/-/-/R/-

Scale =.125"/Ft.

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03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67208
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073008
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66849
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H13A 43' Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

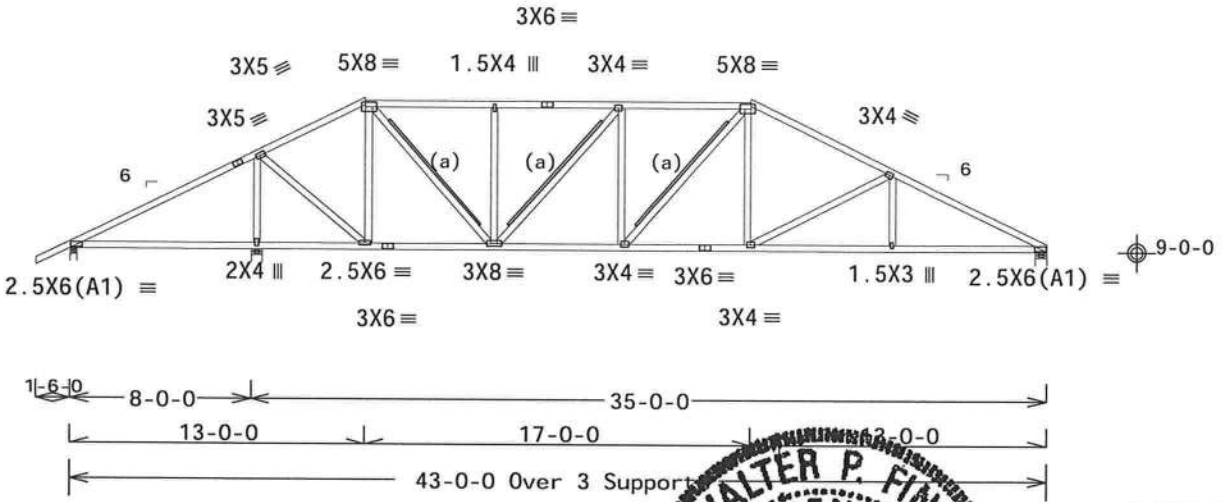
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=351 U=21 W=3.5"
RL=110/-117

R=1865 U=65 W=6"

R=1271 U=48 W=6"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

FL/-/5/-/-/R/-

Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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WALTER P. FINN
LICENSE
No.22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF	REF	R487-- 67209
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073009
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66856
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H15A 40' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

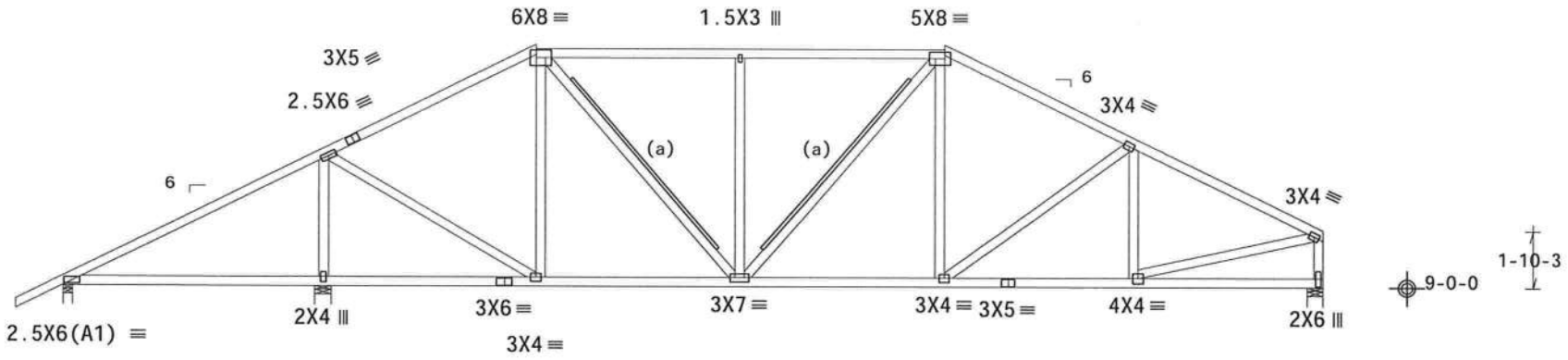
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

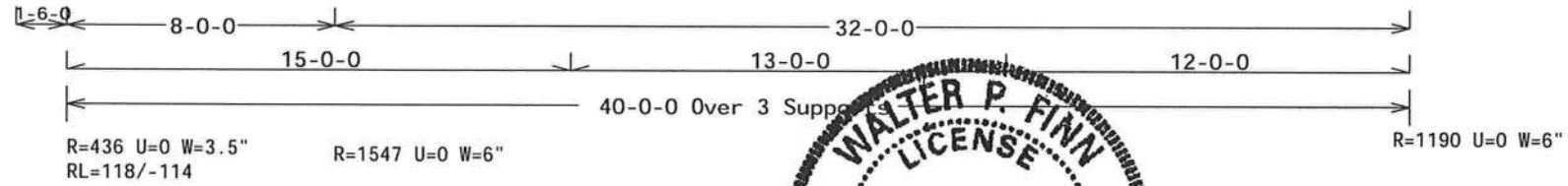
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0809.19

QTY:

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67210
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073010
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66860
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H17A 40' Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

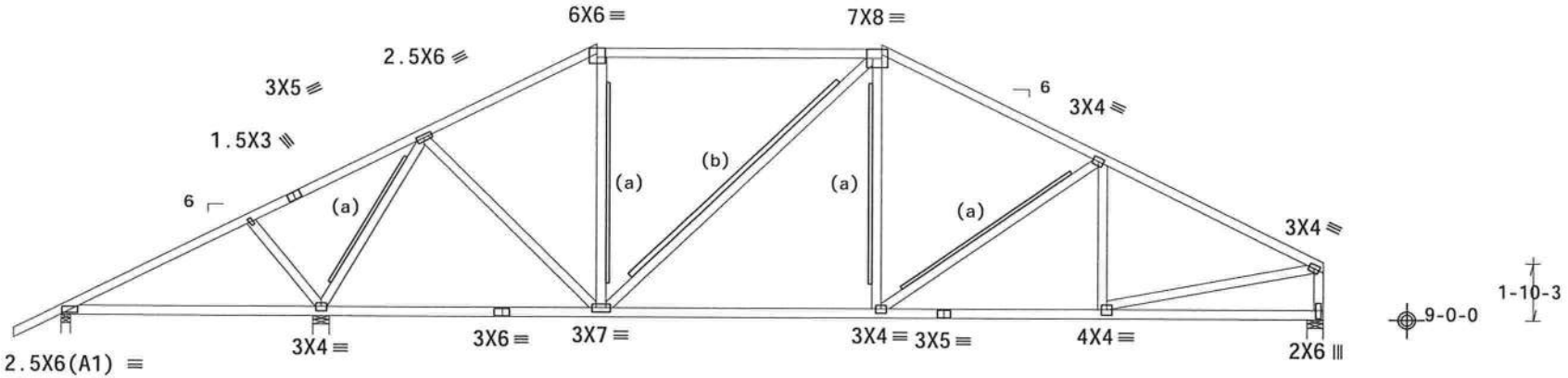
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

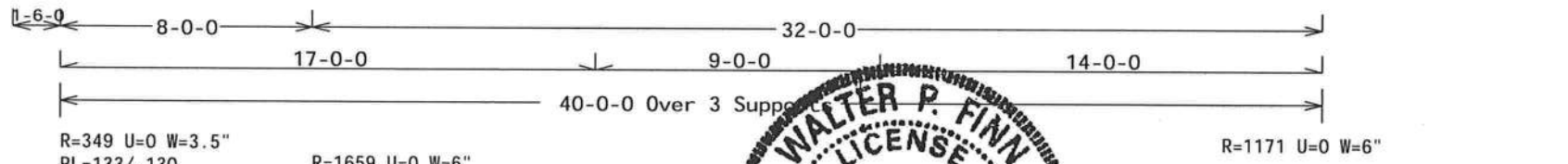
Wind loads and reactions based on MWFRS with additional C&C member design.

(b) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) ★10.03.11 0809.19 QTY 1 FL/-/5/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF	R487-- 67211
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073011
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66864
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H19A 40' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

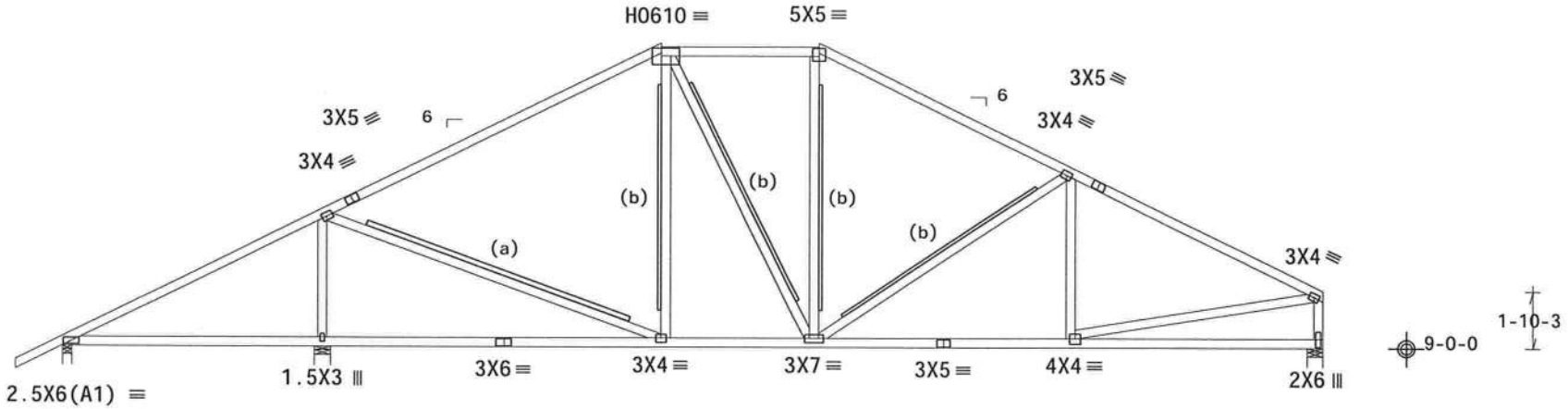
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

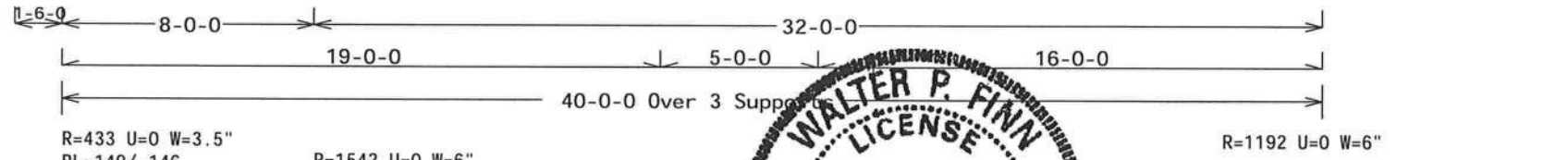
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=10%(0%)/0(0) 40 03 17 0289 19 QTY: FL/-/5/-/-/R/- Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67212
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073012
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66868
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H21A 37'6" Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A :B3 2x4 SP M-30:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Right cantilever is exposed to wind

(b) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

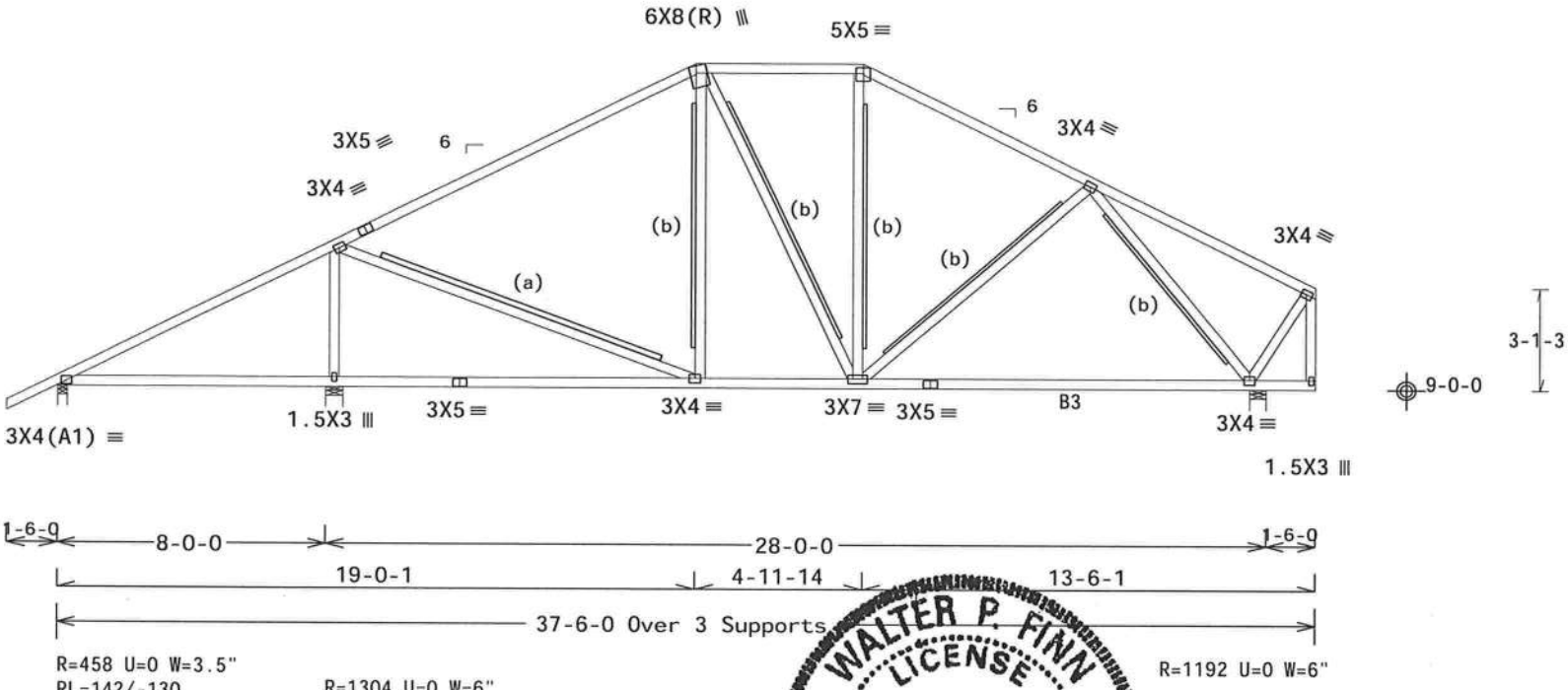
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	37.0 PSF
DUR.FAC.	1.25
SPACING	24.0"

REF R487-- 67213
DATE 03/14/13
DRW HCUSR487 13073014
HC-ENG WHK/WHK
SEQN- 65945
JREF- 1UUJ487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H7A 43' Stepdown Hip Girder)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x6 SP_SS_12A :T1 2x4 SP M-30:
:T5 2x4 SP_#1_12A:
Bot chord 2x6 SP_#1_Dense_12A :B3 2x6 SP_SS_12A:
Webs 2x4 SP_#3__12A :W4, W6, W10 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Left cantilever is exposed to wind

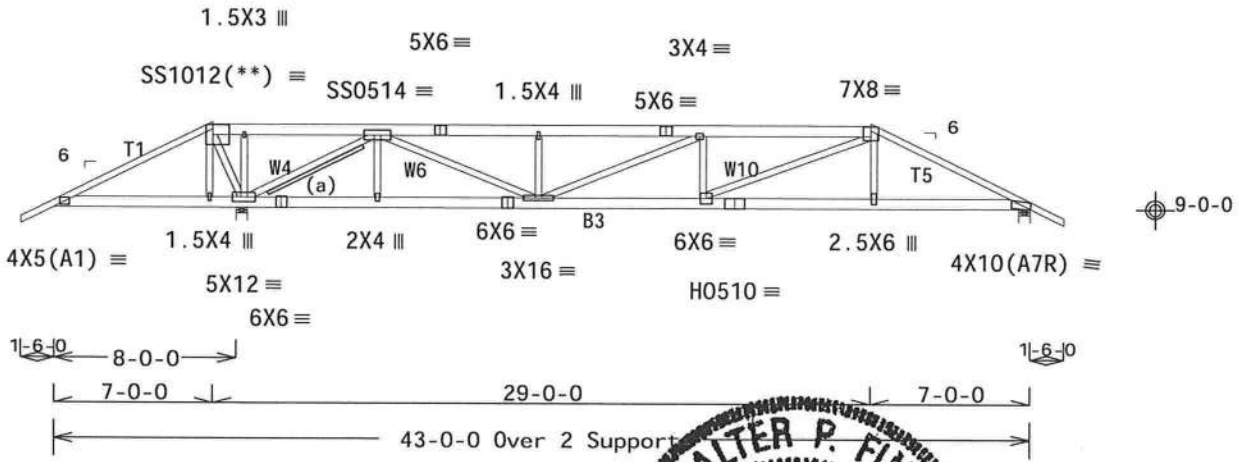
(a) 2x8 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -1.50 to 56 plf at 7.00
TC- From 28 plf at 7.00 to 28 plf at 36.00
TC- From 56 plf at 36.00 to 56 plf at 44.50
BC- From 4 plf at -1.50 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.03
BC- From 10 plf at 7.03 to 10 plf at 35.97
BC- From 20 plf at 35.97 to 20 plf at 43.00
BC- From 4 plf at 43.00 to 4 plf at 44.50
TC- 242.01 lb Conc. Load at 7.03,35.97
TC- 167.95 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,21.94,23.94,25.94,27.94,29.94,31.94 33.94
BC- 443.53 lb Conc. Load at 7.03,35.97
BC- 126.35 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,21.94,23.94,25.94,27.94,29.94,31.94 33.94

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. 20 Gauge HS,18 Gauge HS, Wave

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0)

10.03.11 0809.19 QTY 1

FL/-/5/-/-/R/-

Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

TC LL	20.0 PSF	REF	R487-- 67214
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073015
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66839
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - H9A 43' Stepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

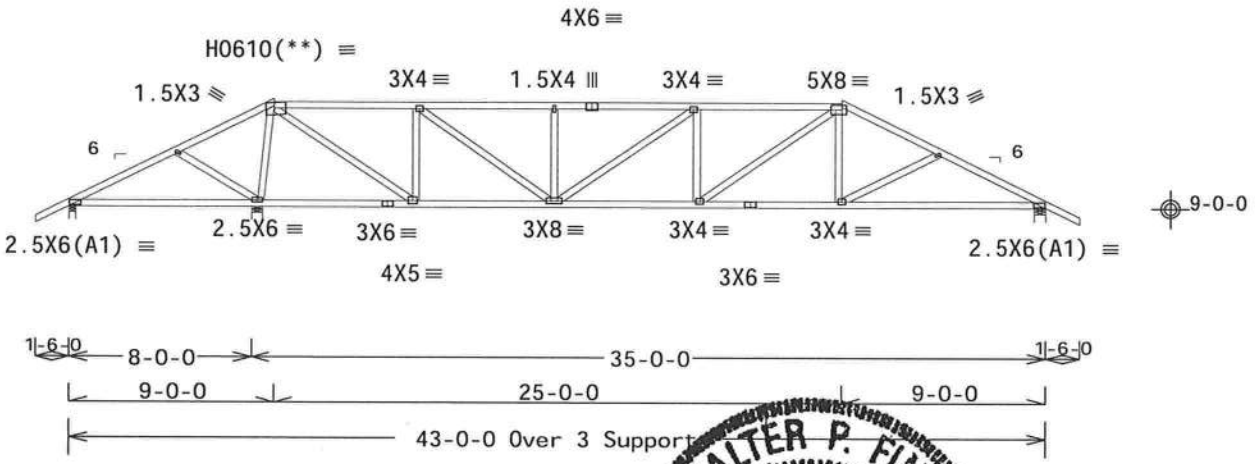
*** Negative reaction(s) of -400# MAX. (See below) from a non-wind load case requires uplift connection.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



*** R=121/-400 U=122 W=3.5"
RL=94/-94

R=2351 U=102 W=6"

R=1276 U=58 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 20 Gauge HS,Wave

FT/RT=10%(0%)/0(0)

10.03.11 0209.19

QTY:1

FL/-/5/-/-/R/-

Scale =.125"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67215
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073013
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	37.0 PSF	SEQN-	66844
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

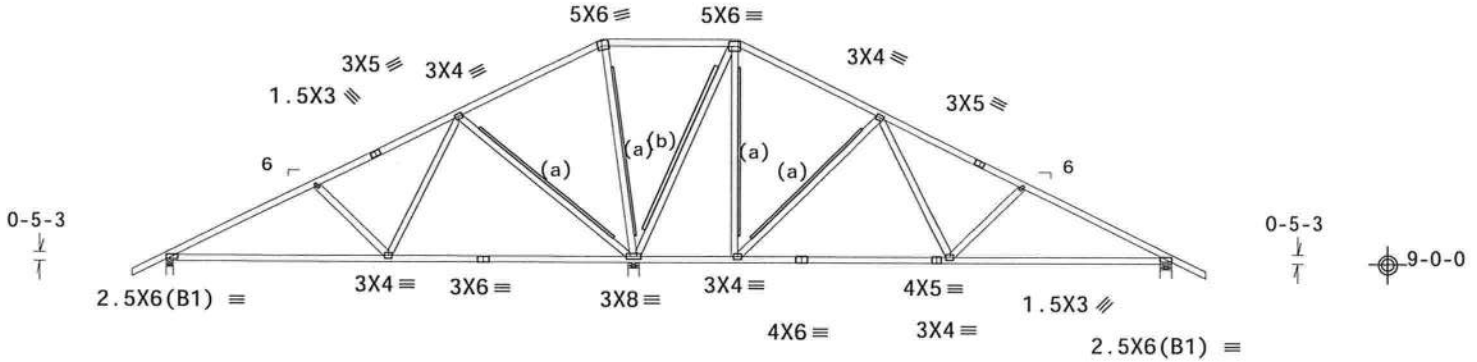
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 6.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

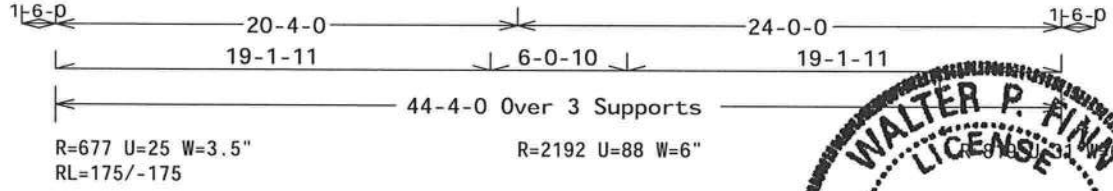
Wind loads and reactions based on MWFRS with additional C&C member design.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) ★10.03.11.0209.19 QTY FL/-/5/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF	REF	R487-- 67216
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073019
BC LL	0.0 PSF		HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65910
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP M-30 :B3, B4 2x4 SP_#1_12A:
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

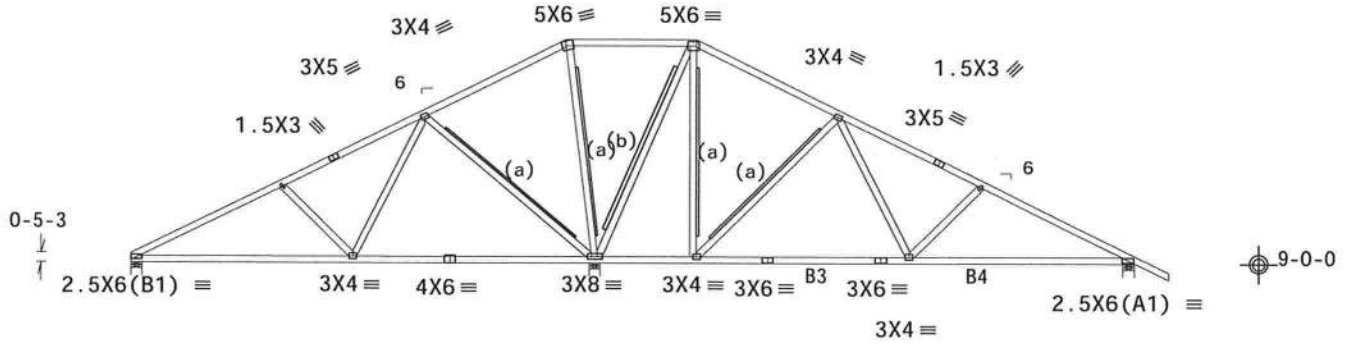
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 13.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



20-2-0 | 24-0-0 | 1-6-0

19-1-11 | 5-8-10 | 19-3-11

44-2-0 Over 3 Supports

R=570 U=0 W=6"
RL=167/-161

R=2576 U=0 W=6

U=0 W=6"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0909.19

FL/-/5/-/-/R/-

Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

TC LL	20.0 PSF	REF R487-- 67217
TC DL	7.0 PSF	DATE 03/14/13
BC DL	10.0 PSF	DRW HCUSR487 13073020
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 65914
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1UUJ487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

(b) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

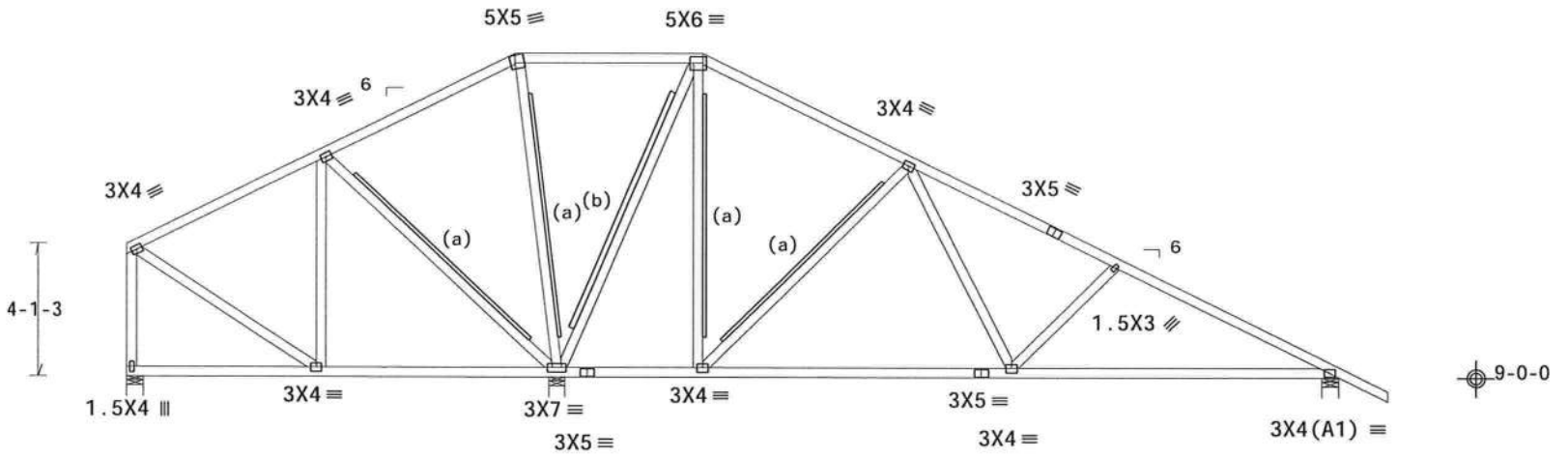
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0809.19

QTY 2

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67218
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073028
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65918
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - BDG 44'4" Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP M-30
Bot chord 2x4 SP M-30
Webs 2x4 SP_#3__12A
:W2, W6, M1, M19, M20 2x4 SP_#1_12A:
:Stack Chord SC1 2x4 SP M-30::Stack Chord SC2 2x4 SP M-30:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

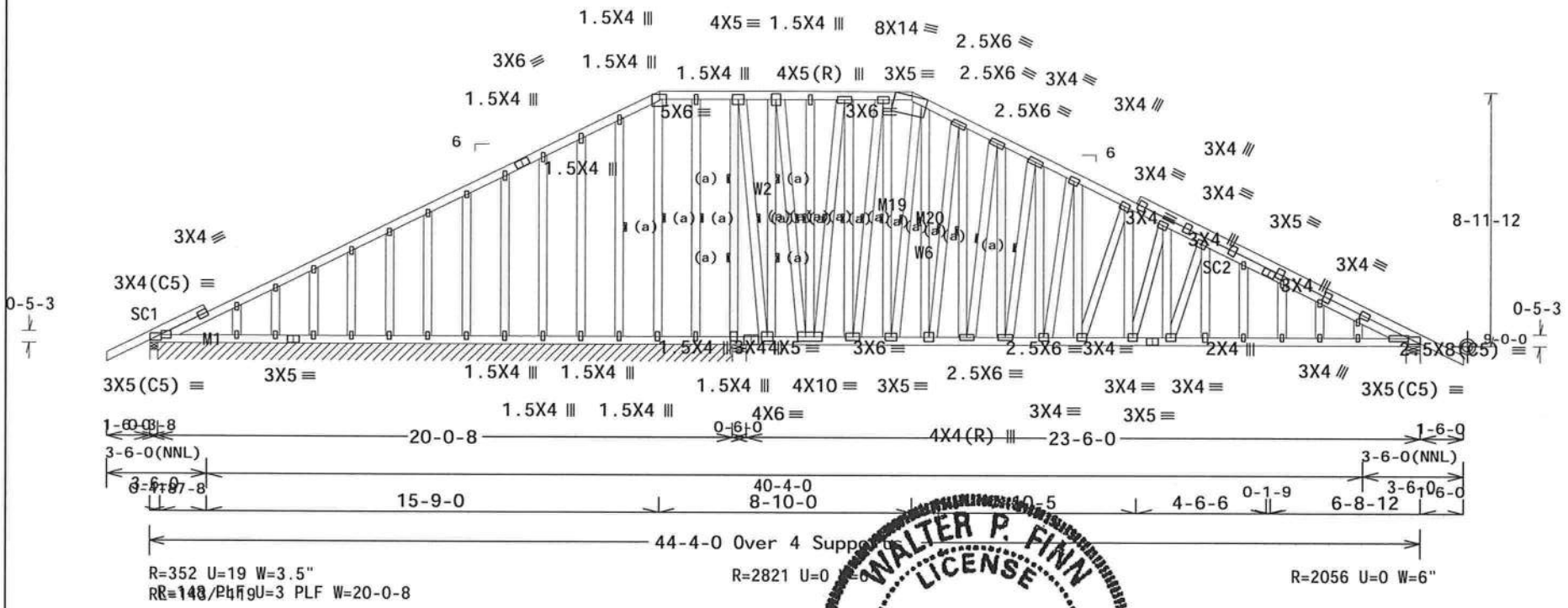
Gable end supports 8" max rake overhang.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

FL/-/5/-/-/R/-

Scale =.1875"/Ft.

TC LL	20.0 PSF	REF	R487-- 67219
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073029
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47692
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - C 24' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

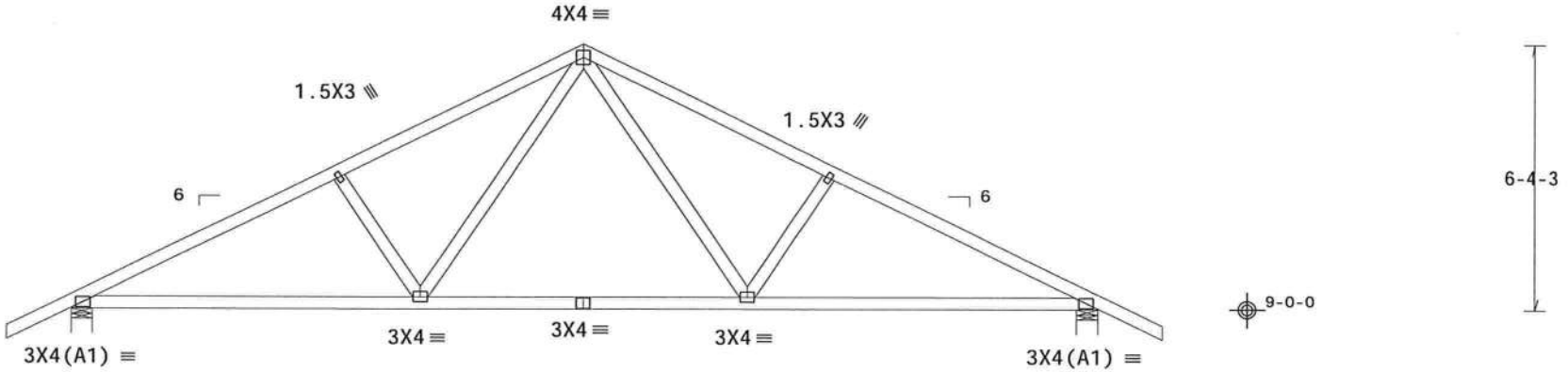
Bottom chord checked for 10.00 psf non-concurrent live load.

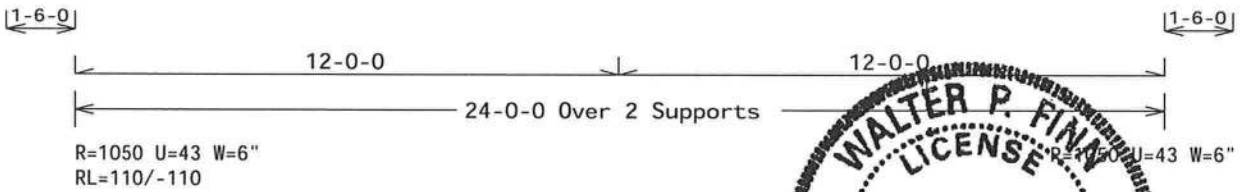
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.09.19

FL/-/5/-/-/R/-

Scale =.25"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67220
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073015
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296321
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - CDG 24' Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

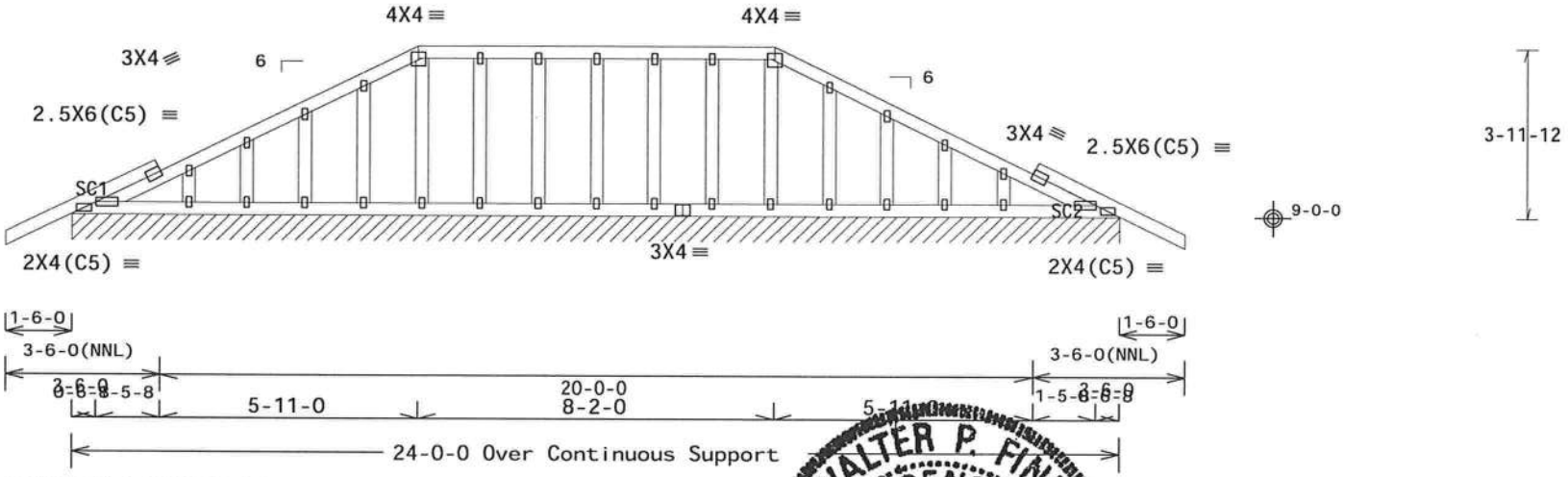
Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=164 PLF U=4 PLF W=24-0-0
RL=3/-3 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

0.03.11.0209.79 0T★

FL/-/5/-/-/R/-

Scale =.25"/Ft.

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03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67221
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073013
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	47638
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

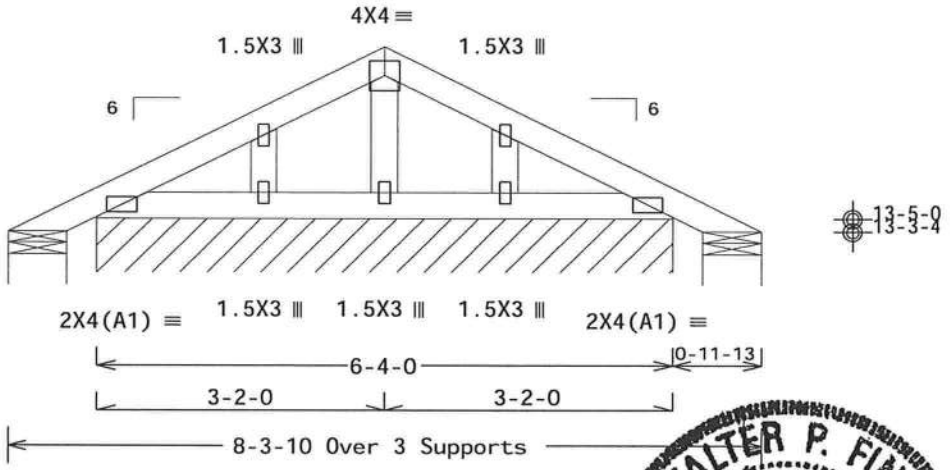
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at 0.00 to 56 plf at 4.15
TC- From 56 plf at 4.15 to 56 plf at 8.30
BC- From 4 plf at 0.00 to 4 plf at 8.30

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=17 U=11 W=7.826"
RL=29/-29 R=67 PLF U=10 PLF W=6-4-0

R=17 U=11 W=7.826"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) ★10.03.11.0209.19 QTY

FL/-/5/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67222
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073016
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	47758
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - CJ1 1' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

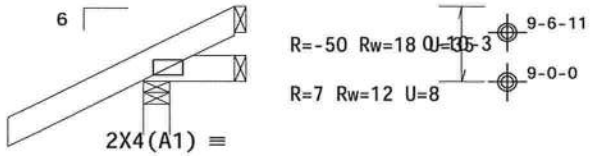
Bottom chord checked for 10.00 psf non-concurrent live load.

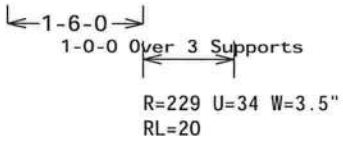
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0809.19 QTY 6 FL/-/5/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67223
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073009
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296095
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - CJ3 3' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

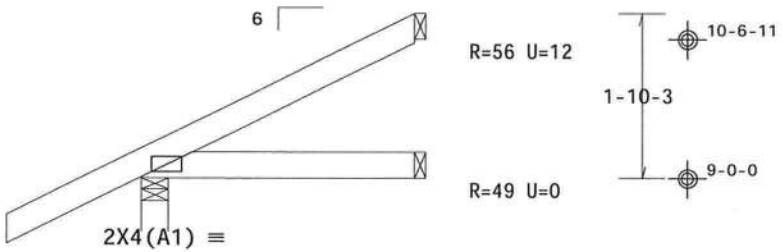
Bottom chord checked for 10.00 psf non-concurrent live load.

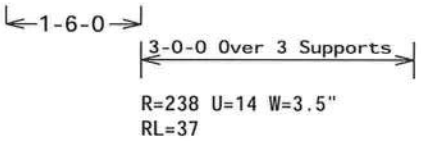
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=238 U=14 W=3.5"
RL=37

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY 4 FL/-/5/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67224
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073010
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296096
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - CJ3A 3' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

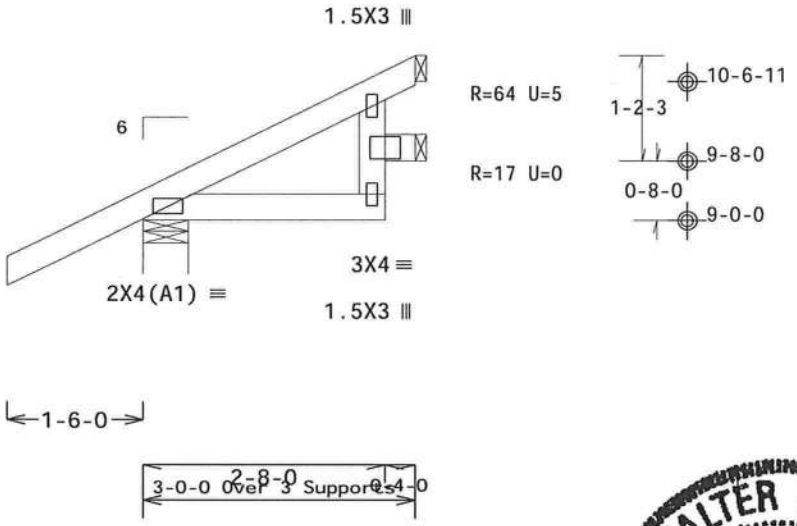
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11 0809.19 QTY 2 FL/-/5/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67225
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073026
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65859
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - CJ5 5' Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

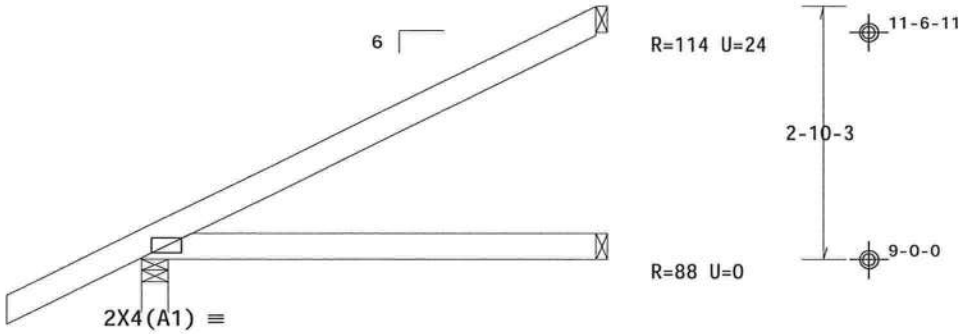
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) ★10.03.11.0309.19 QTY 1 FL/-/5/-/-/R/- Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67226
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073001
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296097
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

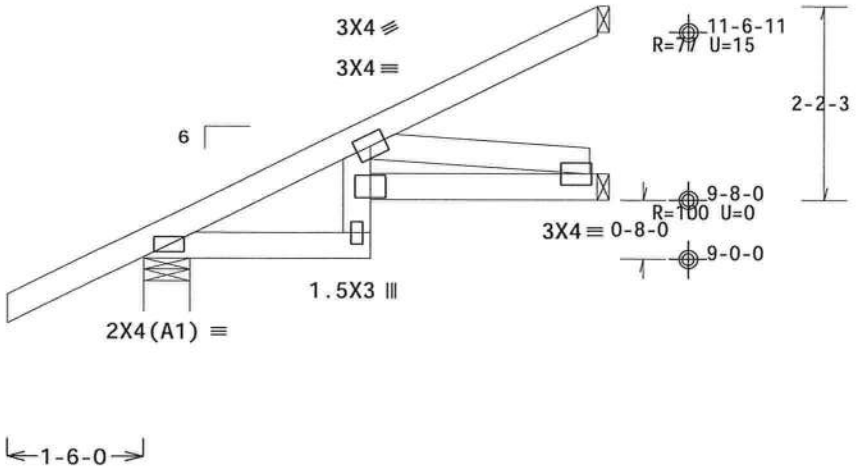
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11 0809.19

QTY 2

FL/-/5/-/-/R/-

Scale =.5"/Ft.

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67227
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073024
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65865
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - EJ7 7' End Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

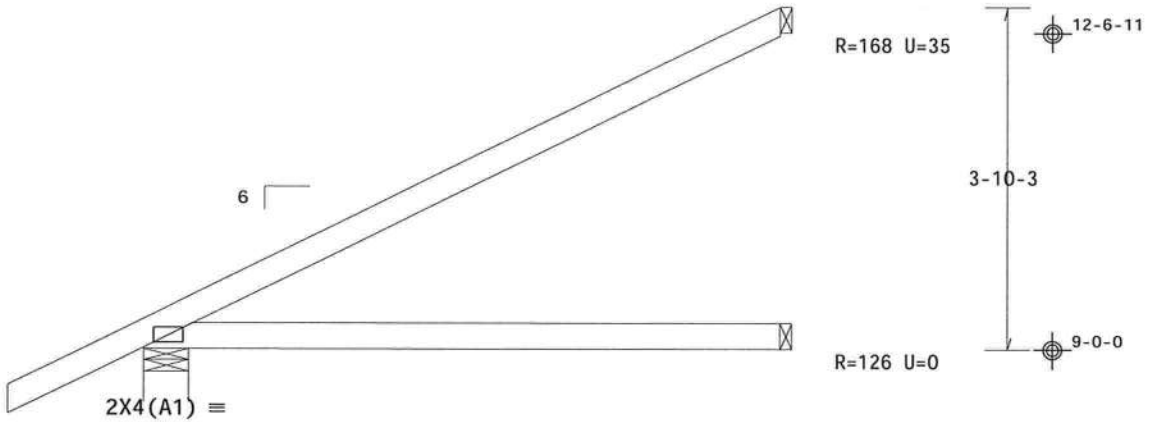
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



1-6-0

7-0-0 Over 3 Supports

R=372 U=8 W=6"
RL=70/-27

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

0.03 11.0269.79 CT: 27 FL/-/5/-/-/R/-

Scale =.5"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

ALPINE
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Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R487-- 67228
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296112
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - EJ7 7' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

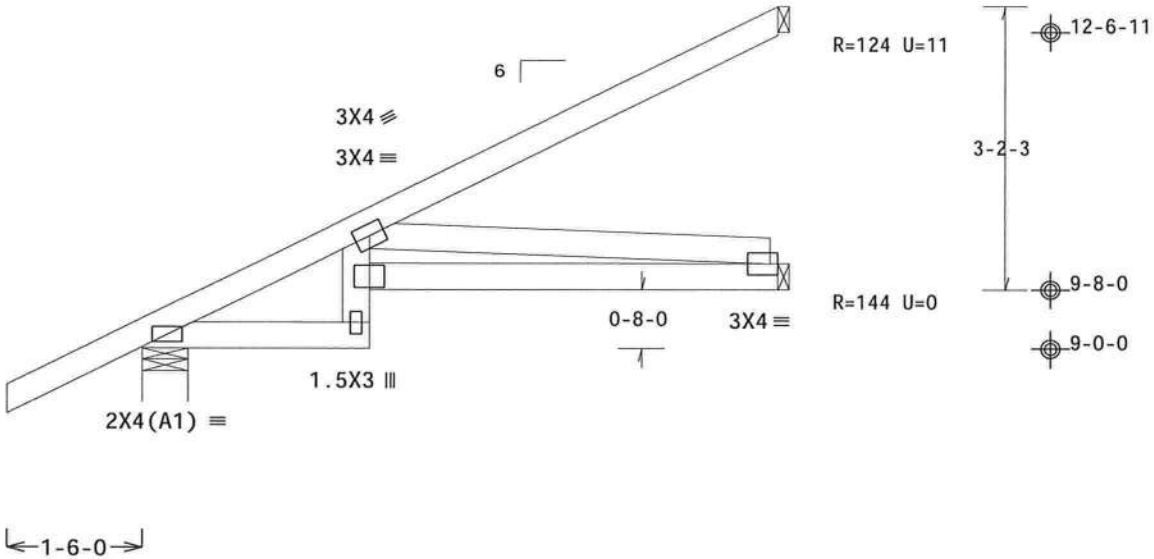
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11 0209.19 QTY 11 FL/-/5/-/-/R/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67229
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073023
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65863
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - EJ7A 7' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(T1) = (J) Tie not calculated Unavailable

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

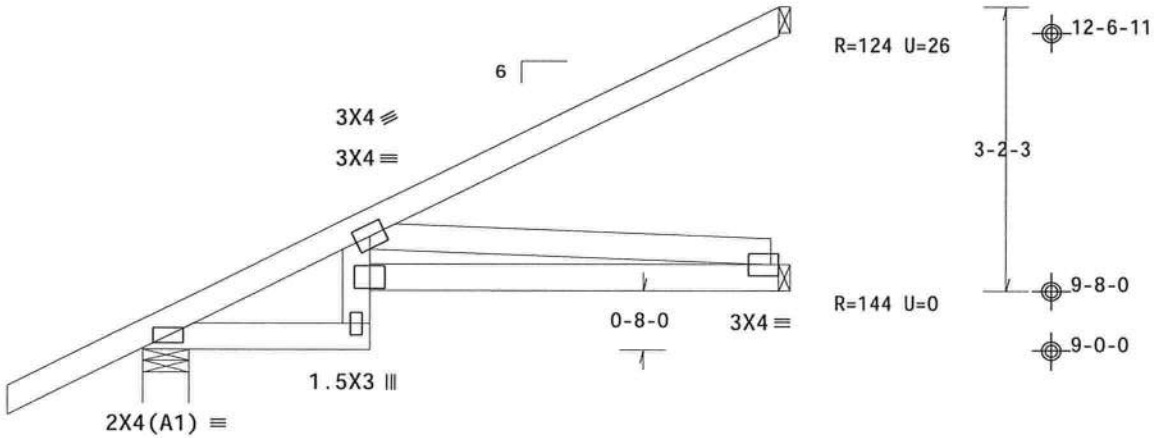
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

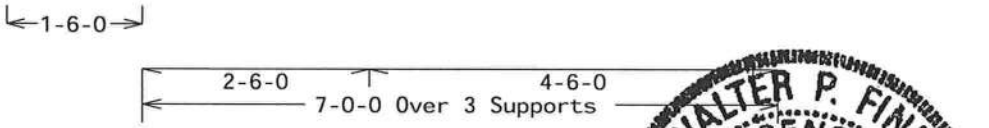
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.





Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0289.79 QTY:

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

FL/-/5/-/-/R/-

Scale =.5"/Ft.

03/14/2013

ALPINE
ITW Building Components Group Inc.
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TC LL	20.0 PSF	REF	R487-- 67230
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65871
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - HJ 9'10"13 Hip Jack Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

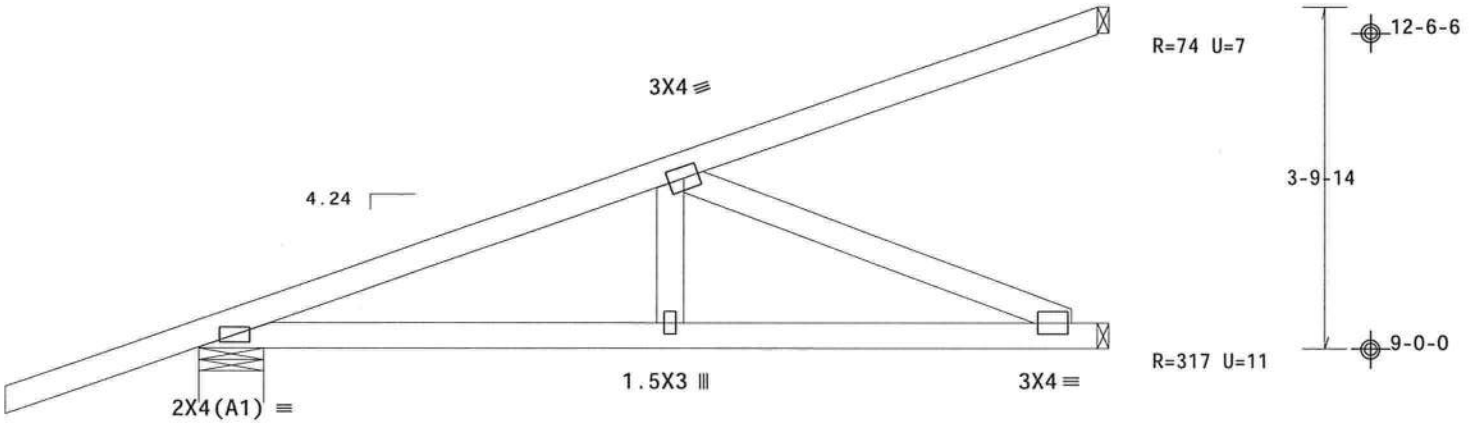
Wind loads and reactions based on MWFRS.

Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.12 to 55 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.90
BC- From 0 plf at -2.12 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.90
TC- -31.38 lb Conc. Load at 1.48
TC- 111.08 lb Conc. Load at 4.31
TC- 228.42 lb Conc. Load at 7.13
BC- 14.23 lb Conc. Load at 1.48
BC- 98.68 lb Conc. Load at 4.31
BC- 176.50 lb Conc. Load at 7.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(ST
FT/RT=10%(0%)/0(0)
0.03.15.0809.19
QTY:

FL/-/5/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF	R487-- 67231
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073012
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	296134
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#1_12A :W2 2x4 SP_#3__12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

(T1) = (J) Tie not calculated Unavailable

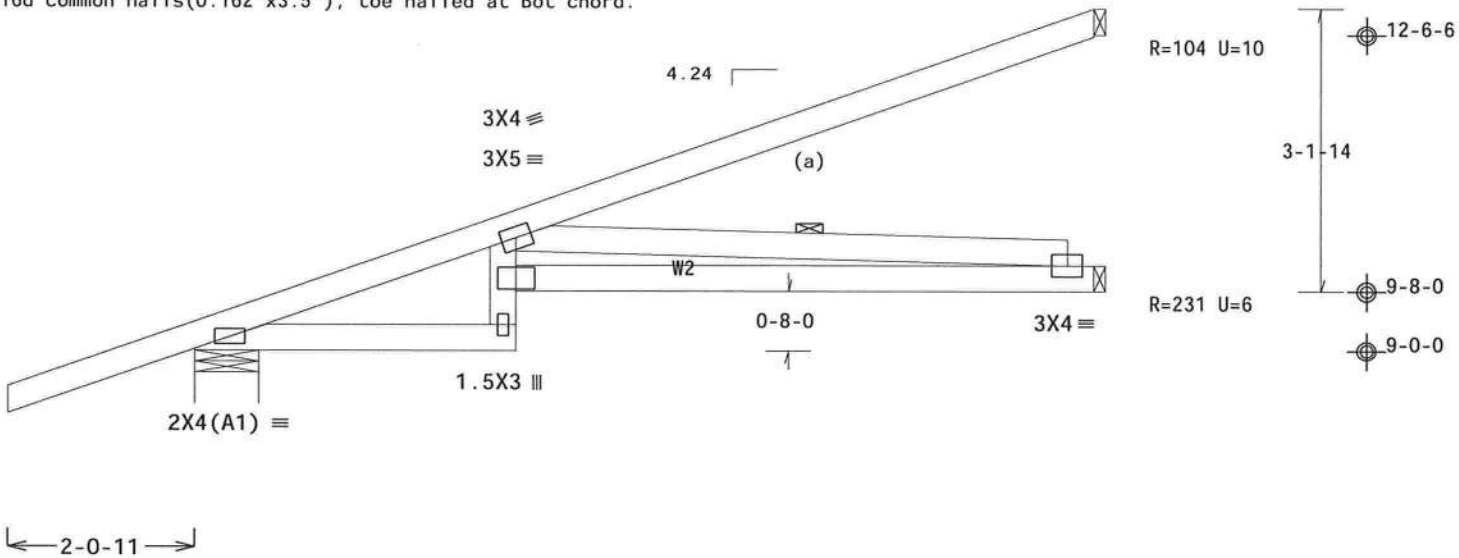
Bottom chord checked for 10.00 psf non-concurrent live load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -2.06 to 55 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.90
BC- From 0 plf at -2.06 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 3.54
BC- From 2 plf at 3.54 to 2 plf at 9.90
TC- -31.38 lb Conc. Load at 1.48
TC- 127.81 lb Conc. Load at 4.31
TC- 154.02 lb Conc. Load at 7.13
BC- 14.23 lb Conc. Load at 1.48
BC- 34.02 lb Conc. Load at 4.31
BC- 200.35 lb Conc. Load at 7.13

(a) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

★10.03.11.0309.19 ★

FL/-/5/-/-/R/-

Scale =.5"/Ft.

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WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

TC LL	20.0 PSF	REF	R487-- 67232
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65877
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - BPB 5'8"10 Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

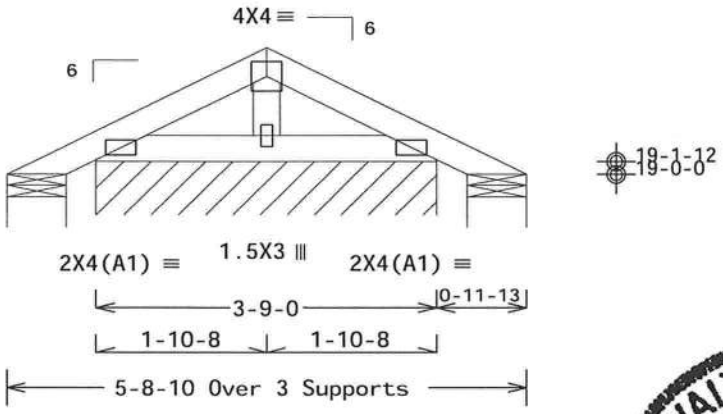
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 9.86 ft. from roof edge.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at 0.00 to 56 plf at 2.86
TC- From 56 plf at 2.86 to 56 plf at 5.72
BC- From 4 plf at 0.00 to 4 plf at 5.72

120 mph wind, 19.72 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

C.03.11.0809.19

DTX:

FL/-/5/-/-/R/-

Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67233
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65906
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - BPB 6'0"10 Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

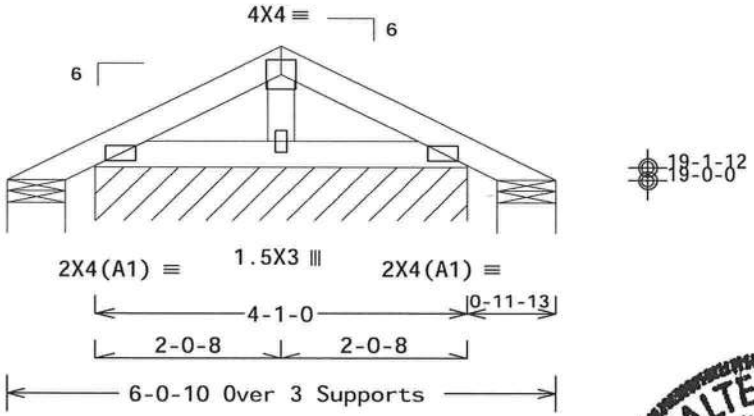
Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at 0.00 to 56 plf at 3.03
TC- From 56 plf at 3.03 to 56 plf at 6.05
BC- From 4 plf at 0.00 to 4 plf at 6.05

120 mph wind, 19.76 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

10.03.11.0809.19 QTY 1

FL/-/5/-/-/R/-

Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67234
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073021
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	65905
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - BP1 8'11"10 Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

Gable end supports 8" max rake overhang.

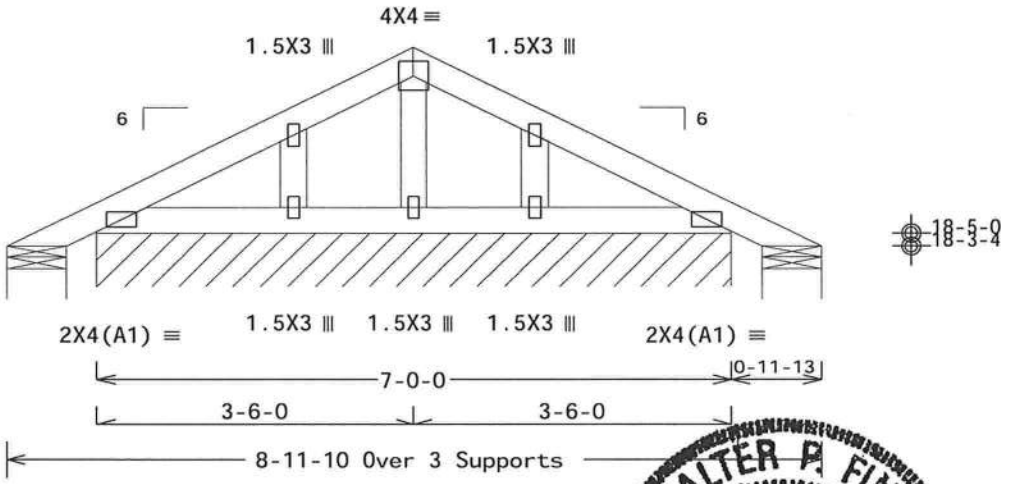
In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at 0.00 to 56 plf at 4.49
TC- From 56 plf at 4.49 to 56 plf at 8.97
BC- From 4 plf at 0.00 to 4 plf at 8.97

120 mph wind, 19.39 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=13 Rw=14 U=13 W=7.826"
RL=31/-3R=67 PLF U=14 PLF W=7-0-0

WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

Design Crit: FBC2010Res/TPI-2007(STD)
PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11 0209.19 QTY 11 FL/-/5/-/-/R/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67235
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073022
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	47756
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - APB 4'11"14 Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

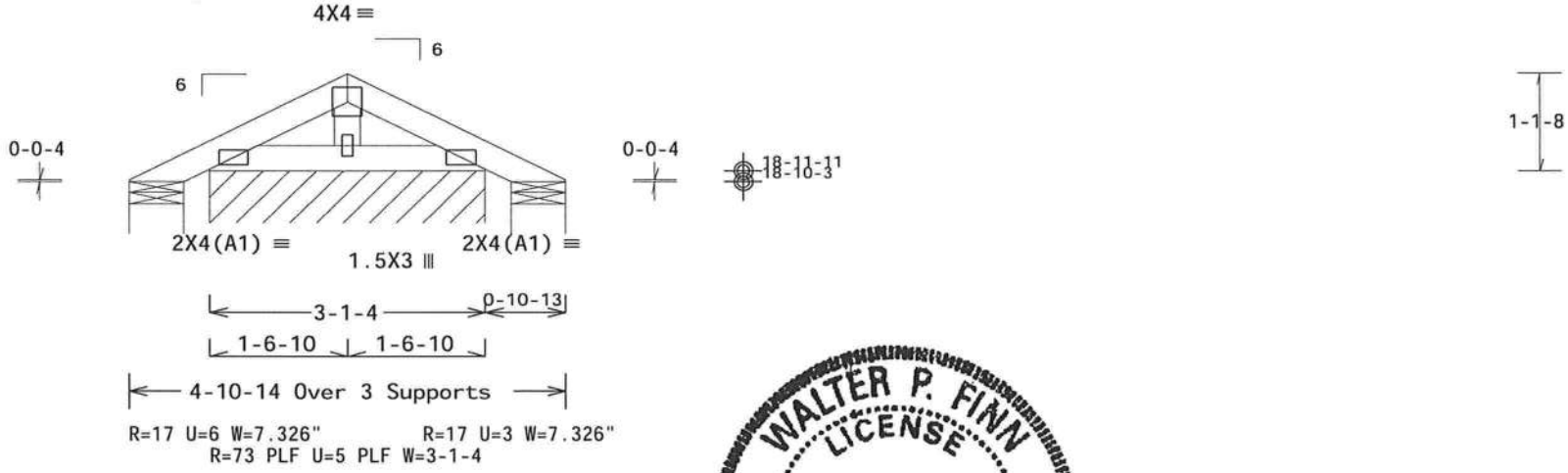
MWFRS loads based on trusses located at least 19.48 ft. from roof edge.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.55
TC- From 56 plf at 1.55 to 56 plf at 4.01
BC- From 4 plf at -0.90 to 4 plf at 4.01

120 mph wind, 19.48 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf.
GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) ★16.03.11.0309.19 QTY ★ FL/-/5/-/-/R/- Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67236
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073017
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48805
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - APB1 4'11"14 Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 19.36 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf.
GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

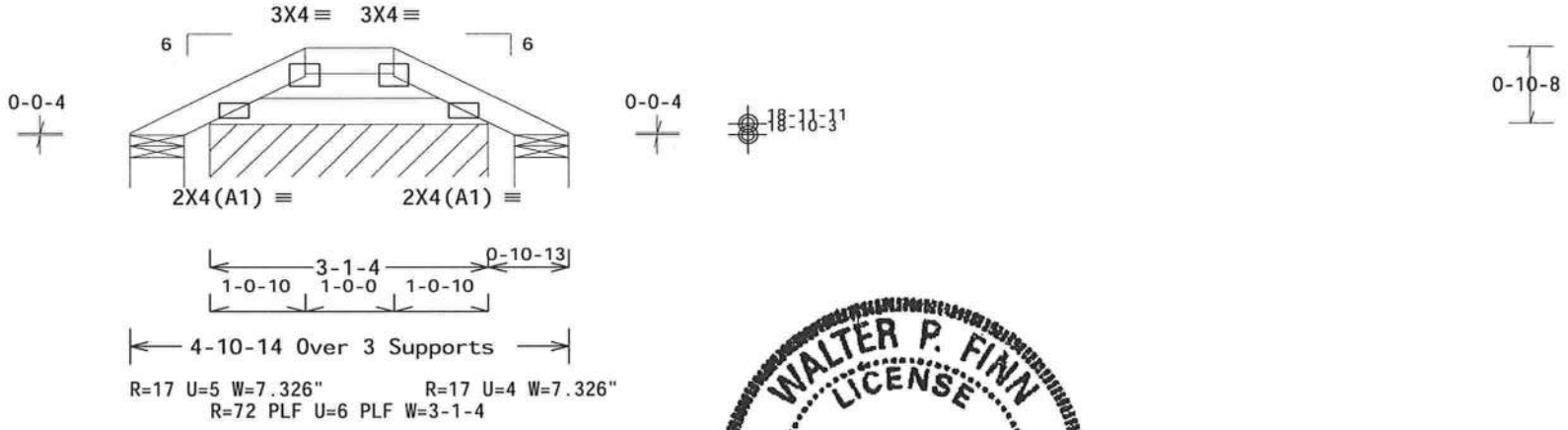
MWFRS loads based on trusses located at least 19.36 ft. from roof edge.

Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.05
TC- From 56 plf at 1.05 to 56 plf at 2.05
TC- From 56 plf at 2.05 to 56 plf at 4.01
BC- From 4 plf at -0.90 to 4 plf at 4.01

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

★10.03.11.0809.19 OT★

FL/-/5/-/-/R/-

Scale =.5"/Ft.

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TC LL	20.0 PSF	REF	R487-- 67237
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073004
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48804
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(13-029B--Doug Edgley Durham Res II -- Lake City, FL - APB2 18'5"15 Mono Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

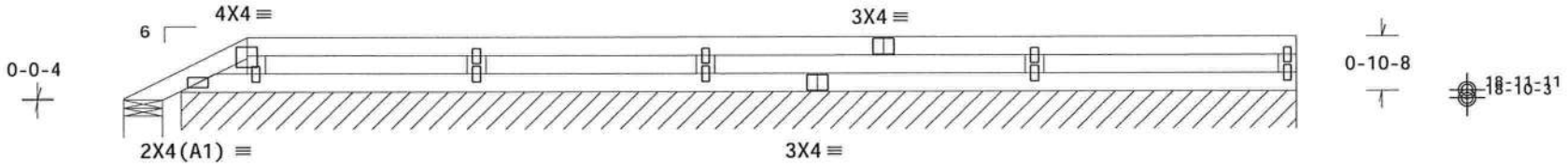
MWFRS loads based on trusses located at least 19.36 ft. from roof edge.

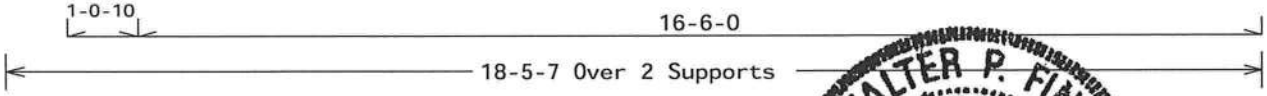
Refer to DWG PB160100212 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 56 plf at -0.90 to 56 plf at 1.05
TC- From 56 plf at 1.05 to 56 plf at 17.55
BC- From 4 plf at -0.90 to 4 plf at 17.55

120 mph wind, 19.36 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=2.0 psf. GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=20 U=0 W=7.326"
R=61 PLF U=13 PLF W=17-6-10

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) ★10.03.11.0809.19 QTY 1 FL/-/5/-/-/R/- Scale =.375"/Ft.

WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER
03/14/2013

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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TC LL	20.0 PSF	REF	R487-- 67238
TC DL	7.0 PSF	DATE	03/14/13
BC DL	10.0 PSF	DRW	HCUSR487 13073018
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	48839
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1UUJ487_Z01

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"		14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

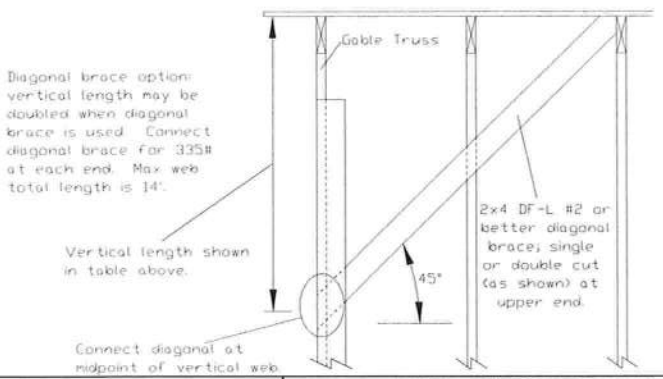
"L" bracing must be a minimum of 80% of web member length.

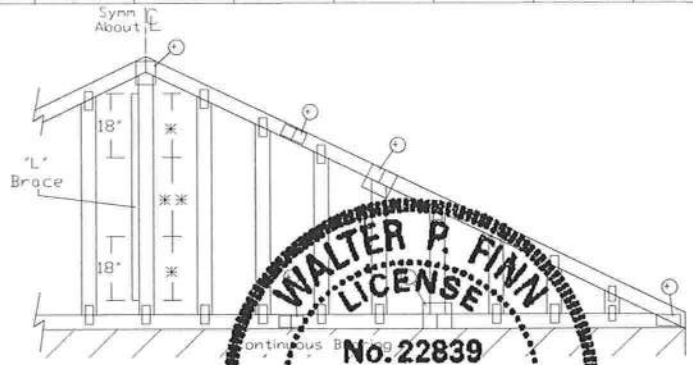
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greather than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





WALTER P. FINN LICENSE No.22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

Mar 14 '13

TW Building Components Group Inc.

Earth City, MO 63045

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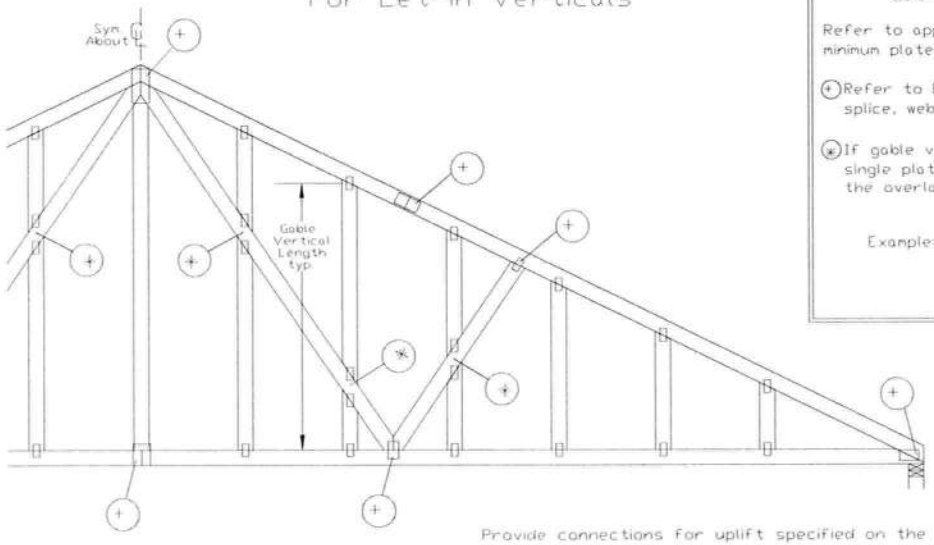
MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-10-GAB12015
DATE 2/14/12
DRWG A12015ENC100212

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

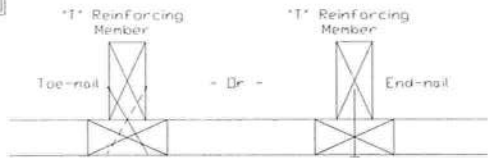
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:

"T" Reinforcement Attachment Detail

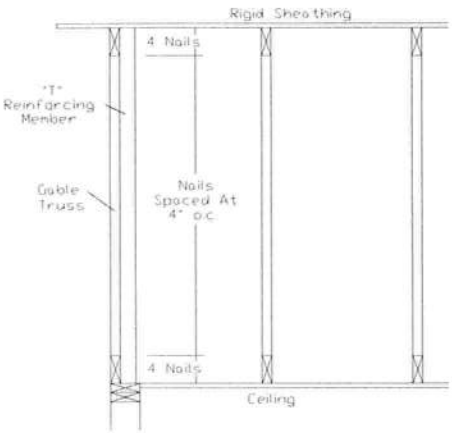


Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.



This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A1001505[illegible]
A13030050109, A12030050109, A11030050109, A1[illegible]

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A1401[illegible]
A18015ENC100212, A20015ENC100212, A200[illegible]20015PED[illegible]
A11530ENC100212, A12030ENC100212, A14[illegible]0ENC[illegible]212, A16030ENC100212,
A18030ENC100212, A20030ENC100212, A2[illegible]30EN[illegible]

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

WALTER P. FINN
LICENSE
No.22839
STATE OF FLORIDA
PROFESSIONAL ENGINEER
03/14/2013

Mar 14 '13

Building Components Group Inc.

Earth City, MO 63045

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MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

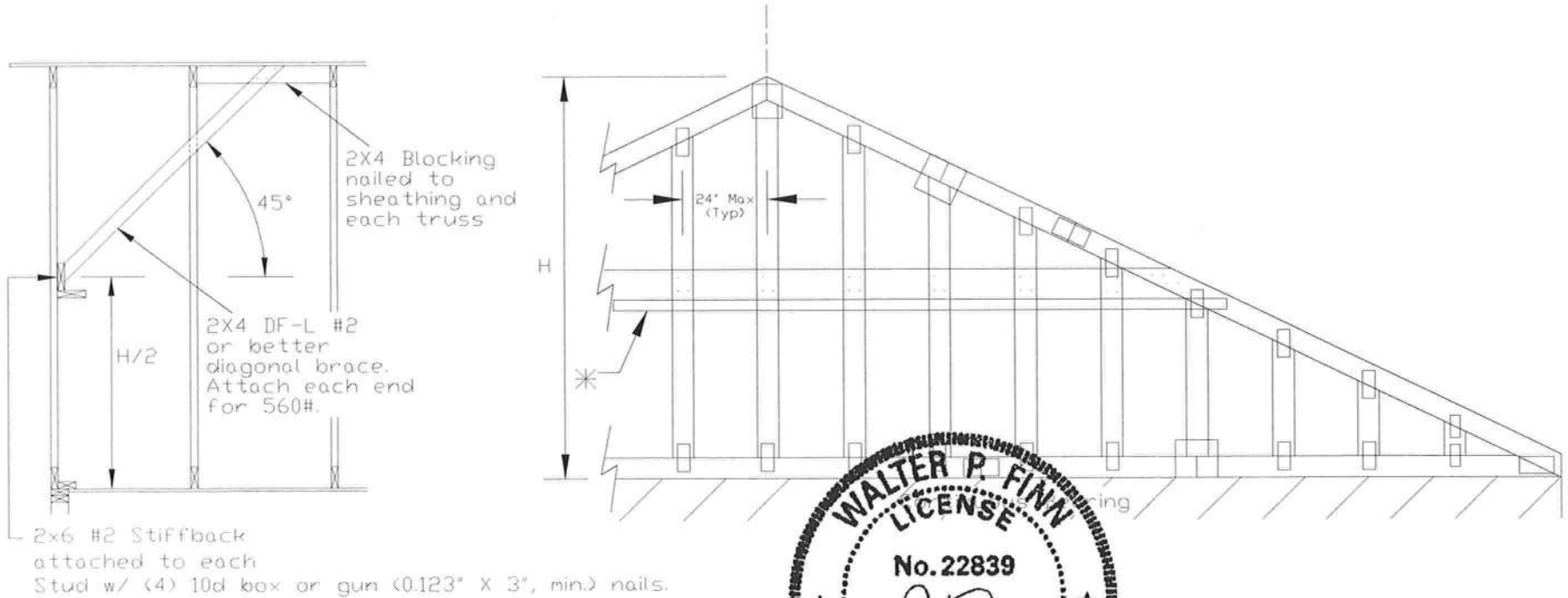
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max: provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



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Earth City, MO 63045

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Mar 14 '13

MAX. TOT. L.D. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

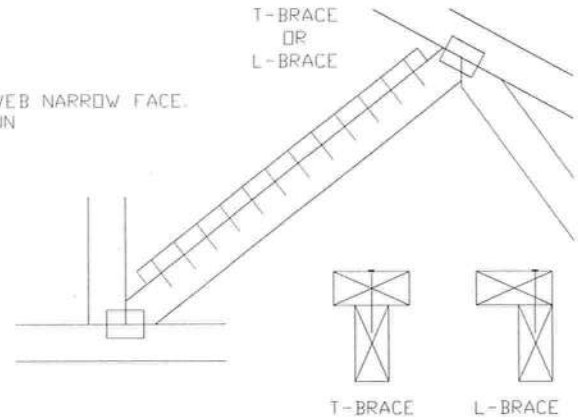
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

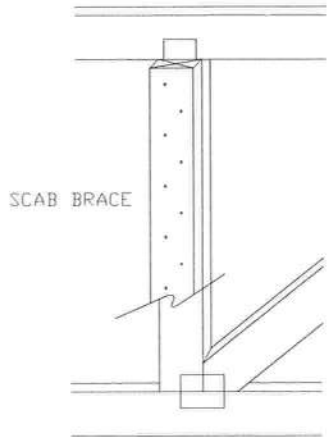
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



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LICENSE
No. 22839
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03/14/2013

Mar 14 '13

Building Components Group Inc.

Earth City, MO 63045

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 4"	7' 6"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 10"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Stud	4' 4"	6' 5"	6' 10"	8' 7"	9' 2"	10' 10"	11' 4"	13' 5"	14' 0"	14' 0"	14' 0"
		Standard	4' 3"	5' 7"	5' 11"	7' 5"	7' 11"	10' 0"	10' 9"	11' 8"	12' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	9' 0"	9' 4"	10' 7"	11' 0"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 0"	6' 10"	7' 3"	9' 1"	9' 8"	12' 4"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 11"	9' 11"	10' 3"	11' 8"	12' 2"	13' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 6"	9' 1"	9' 8"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 6"	7' 10"	8' 4"	10' 6"	11' 3"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

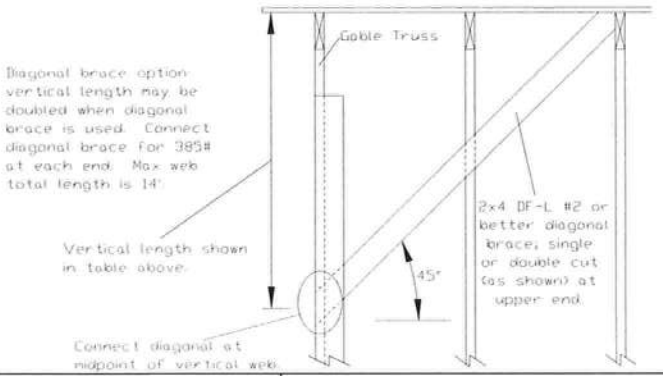
** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimun of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greather than 11' 6"	2.5X4

* Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.



WALTER P. FINN LICENSE No. 22839 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/14/2013

Mar 14 '13

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to, and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the trusses in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see this job's general notes page and these web sites: ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-10-GAB12030

DATE 2/14/12

DRWG A12030ENC100212

Piggyback Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

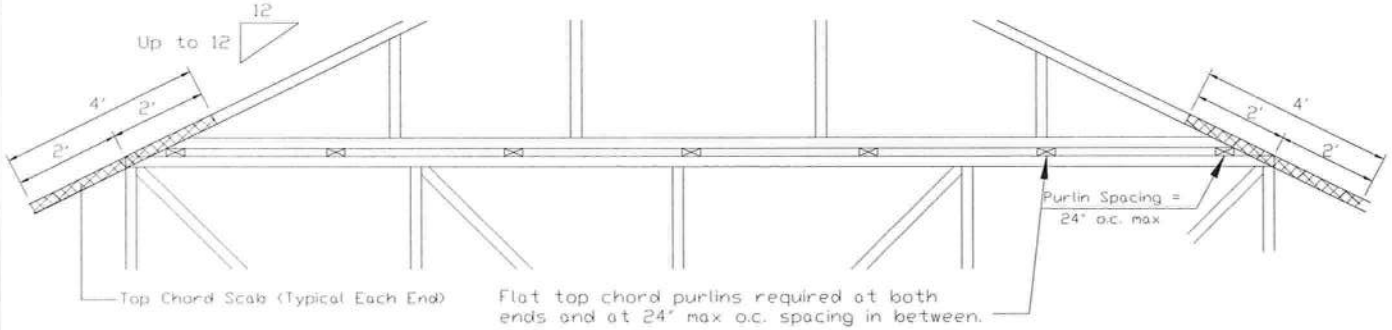
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-10, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

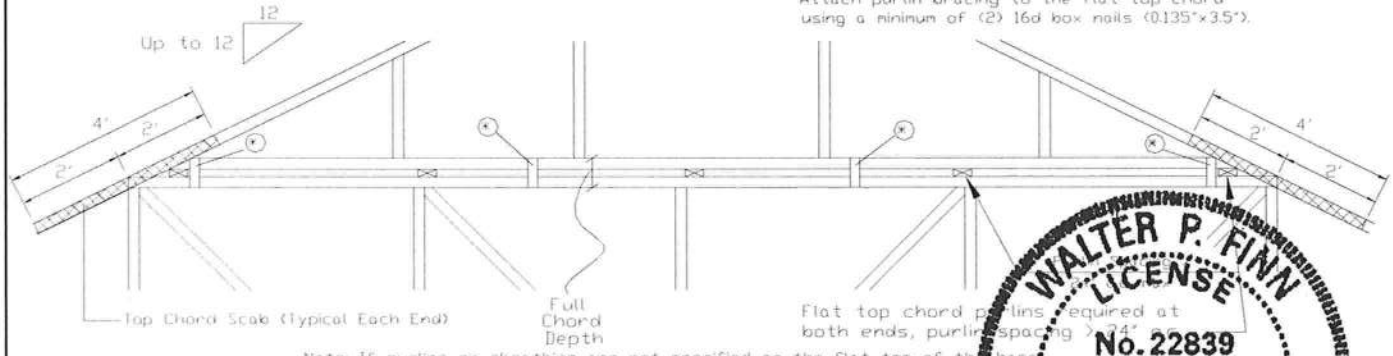
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8" o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4" o.c. front to back faces.

APA Rated Gusset
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8" o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4" o.c. front to back faces.

2x4 Vertical Scabs
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8" o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4" o.c. front to back faces.

28PB Wave Piggyback Plate
One 28PB wave piggyback plate to each face @ 8" o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4" o.c. front to back faces.

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REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212
SPACING	24.0"