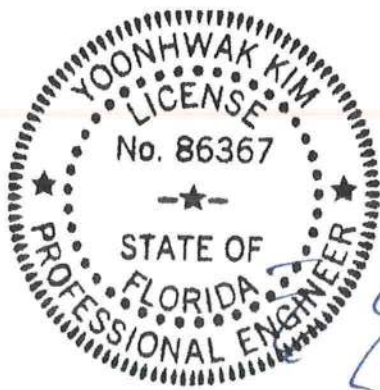
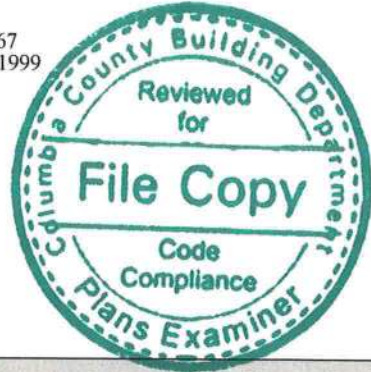




This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.



FL REG# 278, Yoonhwak Kim, FL PE #86367
Florida Certificate of Product Approval #FL 1999
02/01/2023



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Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8548B
Job Description: Ricky Mershon	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.01 through 22.02.00 JRef #: 1XMU2150006
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 7 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	032.23.0922.57020	A01
3	032.23.0923.01870	A03
5	032.23.0923.49583	B01
7	032.23.0925.23730	C02
9	CNNAILSP1014	
11	GBLLETIN0118	

Item	Drawing Number	Truss
2	032.23.0923.49645	A02
4	032.23.0923.49599	A04
6	032.23.0925.04480	C01
8	BRCLBSUB0119	
10	A14030ENC160118	
12	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

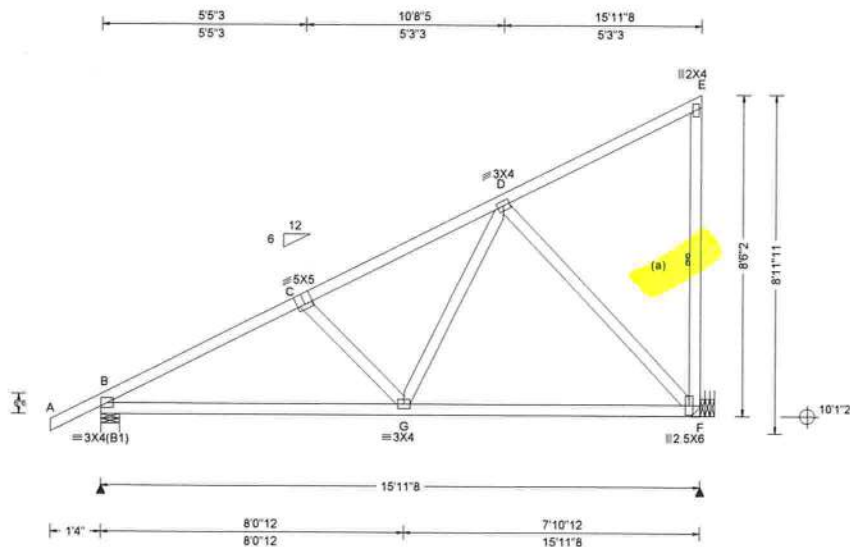
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)						
TCLL:	20.00	Wind Std:	ASCE 7-16	Pg: NA	Ct: NA	CAT: NA	PP Deflection in loc L/defl L/#	Gravity		Non-Gravity				
TCDL:	10.00	Speed:	130 mph	Pf: NA	Ce: NA		VERT(LL): 0.029 C 999 240	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
BCLL:	0.00	Enclosure:	Closed	Lu: NA	Cs: NA		VERT(CL): 0.056 C 999 180	B	781	/-	/-	/493	/63	/300
BCDL:	10.00	Risk Category:	II	Snow Duration:	NA		HORZ(LL): -0.012 E - -	F	710	/-	/-	/464	/185	/-
Des Ld:	40.00	EXP: C	Kzt: NA				HORZ(TL): 0.023 E - -	Wind reactions based on MWFRS						
NCBCLL:	10.00	Mean Height:	15.00 ft	Building Code:	FBC 7th Ed. 2020 Res.	Creep Factor:	2.0	B	Brg Wid = 6.0	Min Req = 1.5 (Truss)				
Soffit:	2.00	TCDL:	5.0 psf	TPI Std:	2014	Max TC CSI:	0.464	F	Brg Wid = -	Min Req = -				
Load Duration:	1.25	BCDL:	5.0 psf	Rep Fac:	Yes	Max BC CSI:	0.750	Bearing B is a rigid surface.						
Spacing:	24.0 "	MWFRS Parallel Dist:	0 to h/2	FT/RT:	20(0)/10(0)	Max Web CSI:	0.748	Members not listed have forces less than 375#						
		C&C Dist a:	3.00 ft	Plate Type(s):		VIEW Ver:	22.02.00.0914.12	Maximum Top Chord Forces Per Ply (lbs)						
		Loc. from endwall:	Any	WAVE				Chords	Tens.Comp.	Chords	Tens. Comp.			
		GCpi:	0.18					B - C	225	- 1049	C - D	163	- 825	
		Wind Duration:	1.60											

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

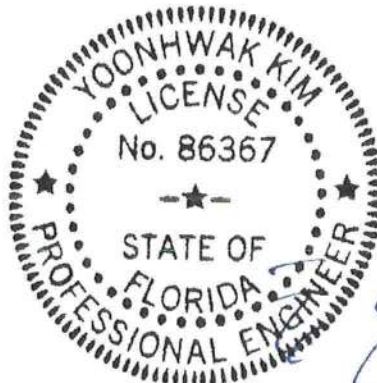
Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8'-6"-2.



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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

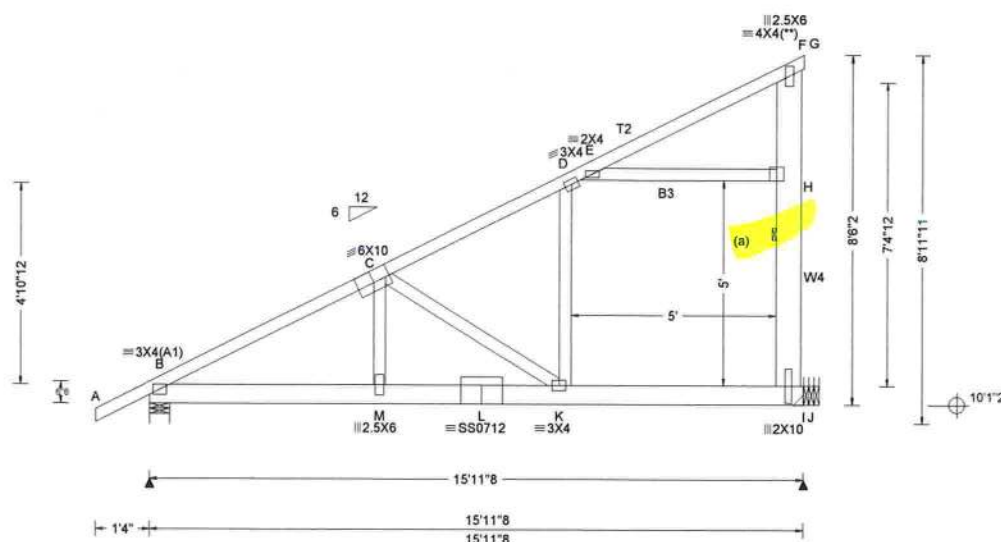
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCEA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCEA: sbceacomponents.com; ICC: iccsafe.org; AWC: awc.org



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Glenview, IL 60025



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.316 K 598 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.637 K 296 180	B 910 /- /- /494 /64 /300
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.151 D - -	I 1288 /- /0 /463 /184 /0
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.306 D - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	B Brg Wid = 6.0 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.375	I Brg Wid = - Min Req = -
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.499	Bearing B is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.902	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	Plate Type(s):		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE, 18SS	VIEW Ver: 21.02.01.1216.14	B - C 233 - 1354 E - F 1201 - 540
	Wind Duration: 1.60			C - D 0 - 715

Lumber

Top chord: 2x4 SP #2; T2 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B3 2x4 SP #2;
Webs: 2x4 SP #3; W4 2x8 SP 2400f-2.0E;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 10-3-8 to 15-3-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-6-2.



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(4) 0.148"x3" nails into supported member.



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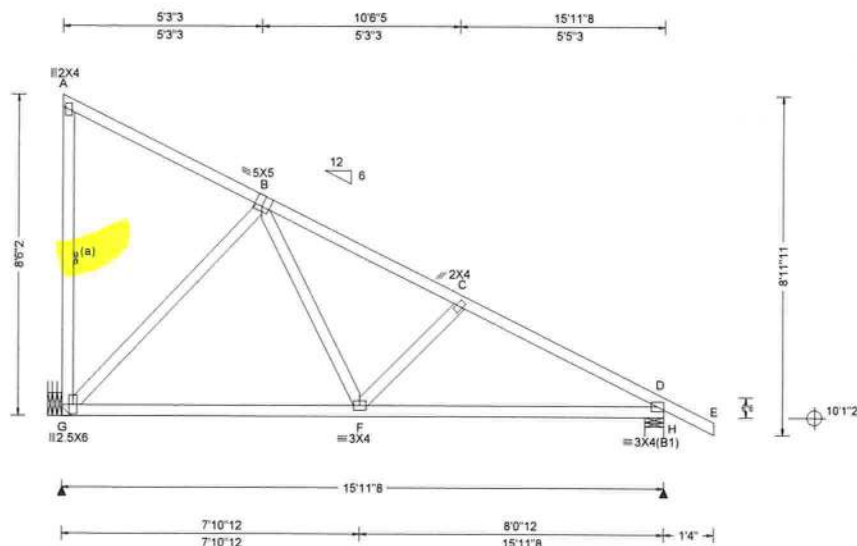
SEQN: 690151
FROM: CDM

MONO

Ply: 1
Qty: 2

Job Number: 22-8548B
Ricky Mershon
Truss Label: A03

Cust: R 215 JRef: 1XMU2150006 T4
DrwNo: 032.23.0923.01870
SSB / YK 02/01/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/def L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.024 C 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.049 C 999 180	G 649 /- /- /466 /186 /305
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.018 A - -	H 754 /- /- /466 /71 /-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.037 A - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	G Brg Wid = - Min Req = -
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.441	H Brg Wid = 6.0 Min Req = 1.5 (Truss)
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.750	Bearing H is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.692	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12	Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	WAVE		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18			B - C 176 -755 C - D 241 -981
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8'-6-2.



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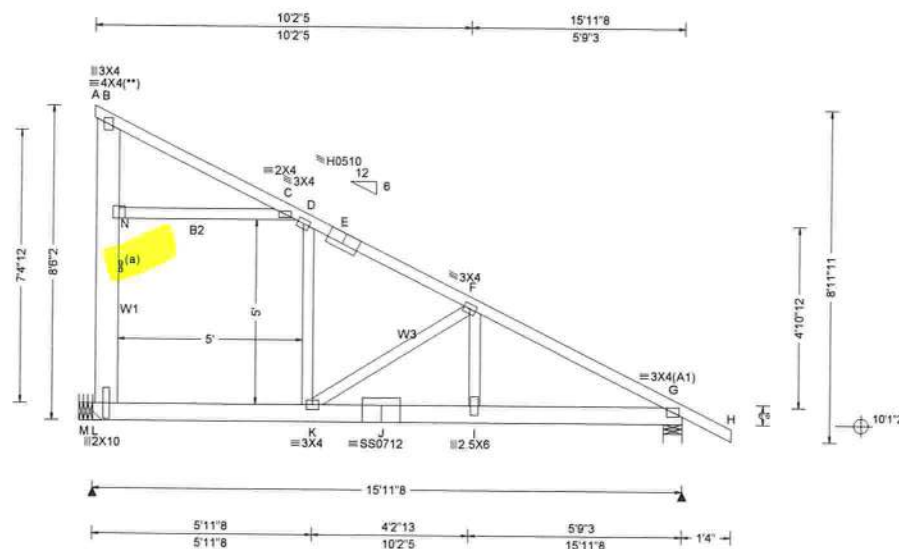
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Cs: NA Ce: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.315 K 599 240 VERT(CL): 0.635 K 297 180 HORZ(LL): -0.151 D - - HORZ(TL): 0.304 D - - Creep Factor: 2.0 Max TC CSI: 0.363 Max BC CSI: 0.500 Max Web CSI: 0.902 VIEW Ver: 22.02.00.0914.12	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 1288 /- /0 /465 /185 /305 G 910 /- /- /466 /72 /- Wind reactions based on MWFRS M Brg Wid = - Min Req = - G Brg Wid = 6.0 Min Req = 1.5 (Truss) Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 1201 -555 E - F 11 -716 D - E 16 -620 F - G 269 -1364

Lumber

Top chord: 2x4 SP M-31;
Bot chord: 2x6 SP 2400f-2.0E; B2 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x8 SP 2400f-2.0E;
W3 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Attic room loading from 0-8-0 to 5-8-0: Live Load: 40 PSF, Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 8-6-2.



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Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
L - K	528 0	J - I	1152 -85
K - J	1152 -85	I - G	1160 -88

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - N	388 -681	D - K	552 -22
L - N	403 -760	K - F	489 -749
N - C	608 -1534		

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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location $x=1''8$ uses the following support conditions: 1''8

Bearing M (1''8, 10'1''2) HUS26

Supporting Member: (3)2x6 SP 2400f-2.0E

(14) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



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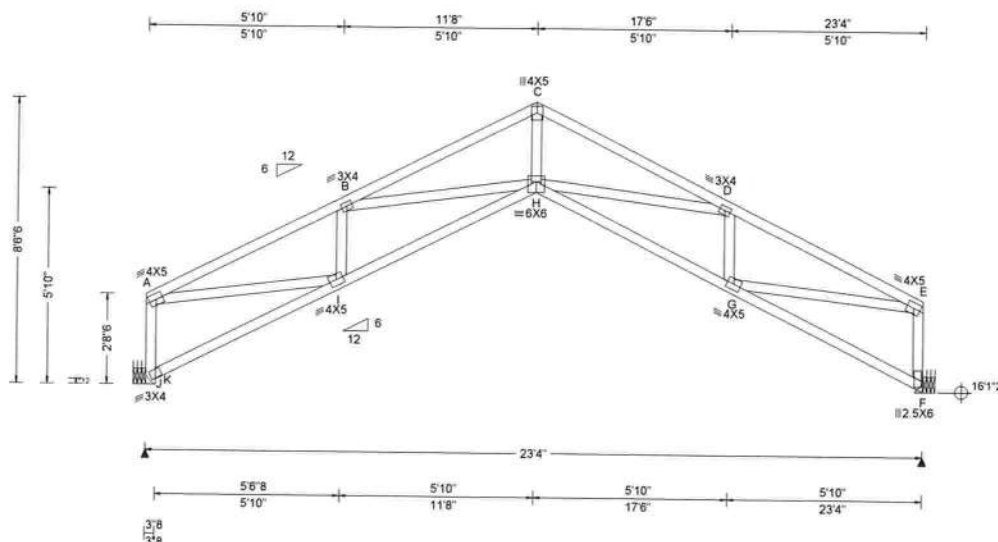
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Loading Criteria (psf)		Wind Criteria		Snow Criteria (Pg,Pf in PSF)		Defl/CSI Criteria		▲ Maximum Reactions (lbs)					
TCLL: 20.00		Wind Std: ASCE 7-16		Pg: NA Ct: NA CAT: NA		PP Deflection in loc L/defl L/#		Gravity		Non-Gravity			
TCDL: 10.00		Speed: 130 mph		Pf: NA Ce: NA		VERT(LL): 0.176 H 999 240		Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
BCLL: 0.00		Enclosure: Closed		Lu: NA Cs: NA		VERT(CL): 0.371 H 750 180		K 988	/- /-	/	/550	/259	/147
BCDL: 10.00		Risk Category: II		Snow Duration: NA		HORZ(LL): 0.203 F - -		F 982	/- /-	/	/550	/253	/-
Des Ld: 40.00		EXP: C Kzt: NA		Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE		HORZ(TL): 0.428 F - -		Wind reactions based on MWFRS					
NCBCLL: 10.00		Mean Height: 21.71 ft				Creep Factor: 2.0		K Brg Wid = - Min Req = -					
Soffit: 2.00		TCDL: 5.0 psf				Max TC CSI: 0.545		F Brg Wid = - Min Req = -					
Load Duration: 1.25		BCDL: 5.0 psf				Max BC CSI: 0.593		Members not listed have forces less than 375#					
Spacing: 24.0 "		MWFRS Parallel Dist: 0 to h/2				Max Web CSI: 0.810		Maximum Top Chord Forces Per Ply (lbs)					
		C&C Dist a: 3.00 ft						Chords Tens.Comp. Chords Tens. Comp.					
		Loc. from endwall: Any						A - B 954 -2141 C - D 1195 -2833					
		GCpi: 0.18						B - C 1167 -2833 D - E 946 -2177					
		Wind Duration: 1.60											

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties

(J) Hanger Support Required, by others

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes

Shim all supports to solid bearing.
The overall height of this truss excluding overhang is 8'-6".

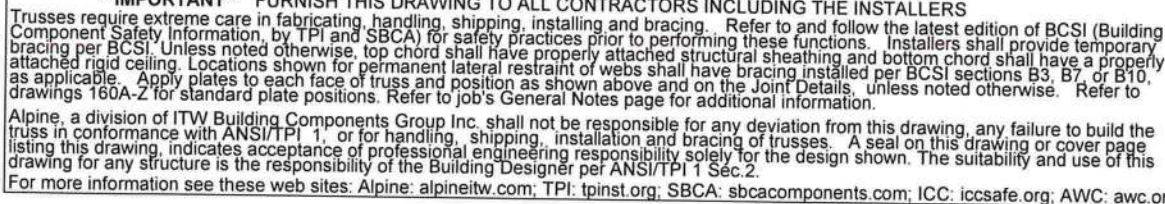


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Maximum Bot Chord Forces Per Ply (lbs)			
Chords	Tens.Comp.	Chords	Tens. Comp.
I - H	2146 -950	H - G	2181 -898

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
A - J	454 -918	H - D	551 -137
A - I	1897 -788	C - H	2127 -751
J - K	433 -986	D - G	400 -648
I - B	414 -661	G - E	1870 -754
B - H	563 0	E - F	457 -936

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Bearing Block(s)

Brg blocks: 0.131"x3", min. nails

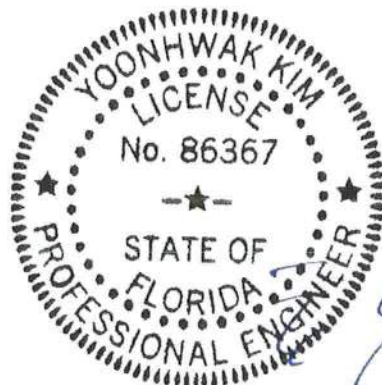
brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	0.000'	2	15"	21	Rigid Surface
2	35.375'	2	15"	21	Rigid Surface

Brg block to be same size and species as chord.

Refer to drawing CNNAILSP1014 for more information.

It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).



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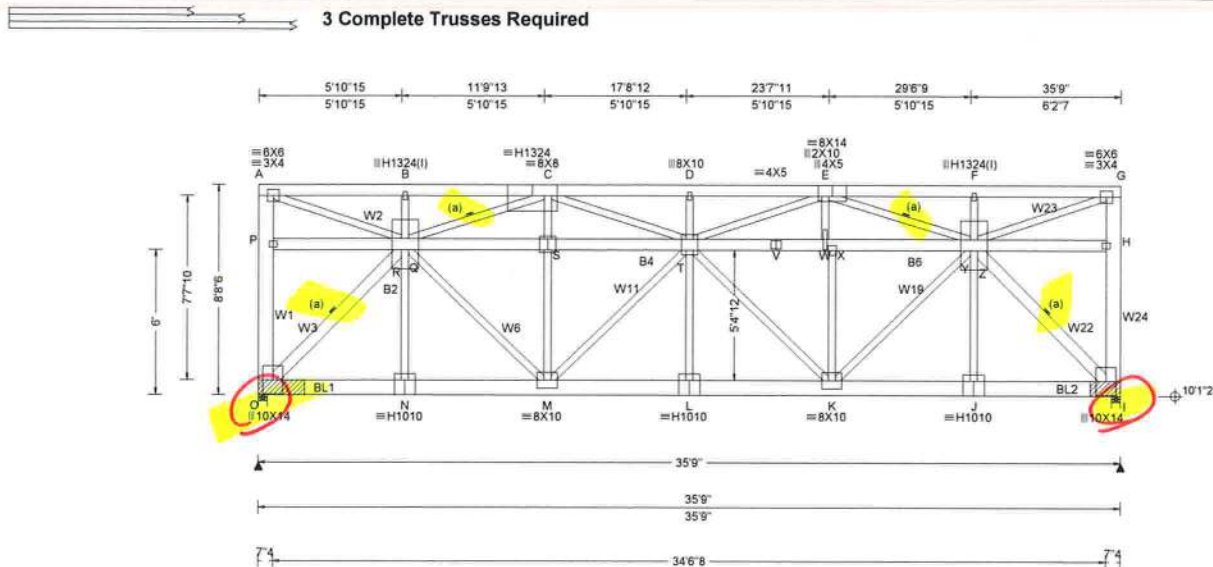
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg.Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.198 T 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.396 T 999 180	O 21533 -/- -/- -/- /4258 -/-
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.065 A - -	I 21823 -/- -/- -/- /4359 -/-
Des Ld: 40.00	EXP: C Kzt: NA	Building Code:	HORZ(TL): 0.129 A - -	Wind reactions based on MWFRS
NCBCLL: 0.00	Mean Height: 18.79 ft	FBC 7th Ed. 2020 Res.	Creep Factor: 2.0	O Brg Wid = 4.5 Min Req = -
Soffit: 2.00	TCDL: 5.0 psf	TPI Std: 2014	Max TC CSI: 0.276	I Brg Wid = 4.5 Min Req = -
Load Duration: 1.25	BCDL: 5.0 psf	Rep Fac: Yes	Max BC CSI: 0.534	Bearings O & I are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	FT/RT:20(0)/10(0)	Max Web CSI: 0.949	Members not listed have forces less than 375#
	C&C Dist a: 3.58 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12	Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 12.50 ft	WAVE, HS		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18			A - B 504 -2595 D - E 1030 -5307
	Wind Duration: 1.60			B - C 504 -2595 E - F 504 -2595
				C - D 1030 -5307 F - G 504 -2595

Lumber
 Top chord: 2x6 SP #2;
 Bot chord: 2x8 SP 2400f-2.0E; B2,B4,
 B6 2x6 SP #2;
 Webs: 2x4 SP #3; W1,W24 2x8 SP #2; W2,W6,W11,W19,
 W23 2x4 SP #2; W3,W22 2x6 SP #2;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Nailnote
 Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 2.50" o.c. (Each Row)
 Webs: 1 Row @ 4" o.c.
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
 -----(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
 TC: From 60 plf at 0.00 to 60 plf at 17.94
 TC: From 30 plf at 17.94 to 30 plf at 35.75
 BC: From 20 plf at 0.00 to 20 plf at 17.88
 BC: From 10 plf at 17.88 to 10 plf at 35.15
 BC: From 20 plf at 35.15 to 20 plf at 35.75
 BC: 982 lb Conc. Load at 0.44
 BC: 2270 lb Conc. Load at 1.94, 3.94, 5.94, 7.94
 9.94, 11.94, 13.94, 15.94, 17.94, 19.81, 21.81, 23.81
 25.81, 27.81, 29.81, 31.81, 33.81
 BC: 1632 lb Conc. Load at 35.31

Plating Notes
 All plates are 2X4 except as noted.

Wind
 Wind loads and reactions based on MWFRS.
 End verticals not exposed to wind pressure.

Bearing Block(s)
 Brg blocks: 0.131"x3", min. nails
 brg x-loc #blocks length 1/4" min. nails 1/4" min. nails
 1 0.000' 1 1/4" min. nails 1/4" min. nails
 2 35.375' 2 1/4" min. nails 1/4" min. nails
 Brg block to be same size and species as chord.
 Refer to drawing 160A-Z for more information.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
O - N	5316 -1038	L - K	8896 -1734
N - M	5316 -1038	K - J	5252 -1024
M - L	8896 -1734	J - I	5252 -1024

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
A - P	212 -1021	T - K	311 -1588
A - R	2739 -530	V - W	463 -2393
O - P	213 -1029	E - W	968 -173
O - Q	1452 -7438	E - Z	581 -2975
Q - N	2489 -452	W - X	463 -2393
Q - C	619 -3157	K - Y	3594 -694
Q - M	3500 -674	X - K	935 -167
Q - S	430 -2232	X - Y	463 -2393
C - S	1093 -178	Y - J	2492 -470
M - T	312 -1614	Y - G	2743 -532
S - M	1056 -171	Y - I	1433 -7348
S - T	430 -2232	H - G	200 -996
T - L	2154 -396	H - I	201 -999
T - V	463 -2393		

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Additional Notes

* WARNING* A reaction exceeds 20000 lbs.
 Truss must be installed as shown with top chord up.
 The overall height of this truss excluding overhang is 8-8-6.
 Laterally brace top chord below filler and bottom chord above filler at 24" o.c., including a lateral brace at chord ends (If no rigid diaphragm exists at that point).
 It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.



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CLR Reinforcing

Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4⊗
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6⊗

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

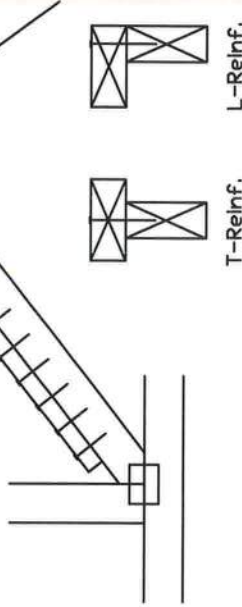
⊗ Center scab on wide face of web. Apply (1) scab to each face of web.

T-Reinforcement

or

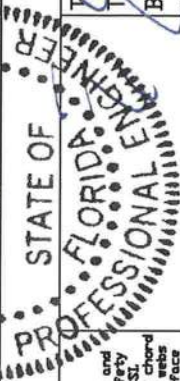
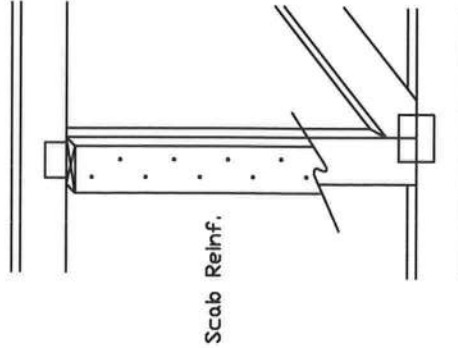
L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0", min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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For more information see this job's general notes page and these web pages:
ALPINE: www.alpine.com TPI: www.tpi.org SCSA: www.scsa.com ID: www.id.com

PSF REF CLR Subst.

PSF DATE 01/02/19

PSF DRWG BRCLBSUB0119

PSF

PSF

PSF

PSF

PSF

TC LL

TC DL

BC DL

BC LL

TOT. L.D.

DUR. FAC.

SPACING



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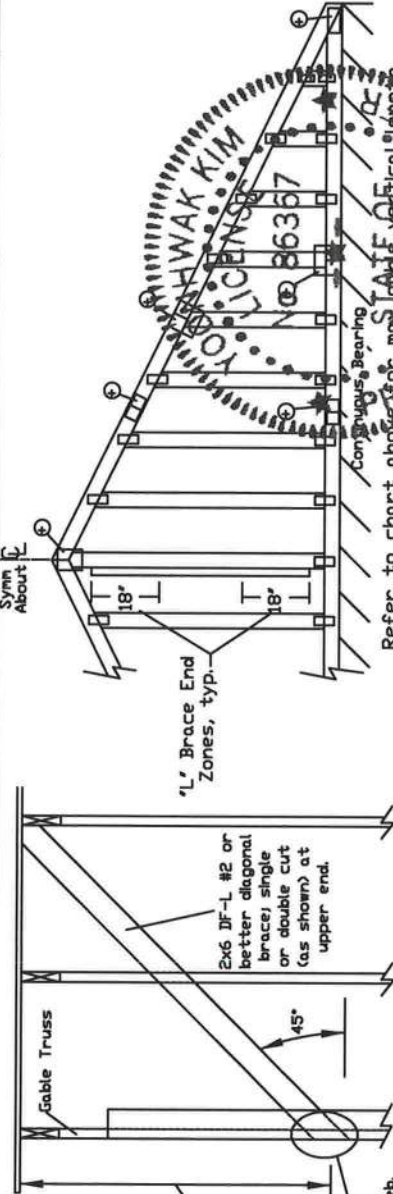
Dr	120 mph	Wind Speed, 30'	Mean Height, Exposure C, Kzt = 1.00
Dr	120 mph	Wind Speed, 30'	Mean Height, Exposure D, Kzt = 1.00
Dr	100 mph	wind speed, 30'	Mean Height, Exposure D, Kzt = 1.00

2x4 Gable Vertical Spacing		Brace		No Braces	(1) 1x4 "L" Brace ■ (1) 2x4 "L" Brace ■ (2) 2x4 "L" Brace ■■ (1) 2x6 "L" Brace ■■ (2) 2x6 "L" Brace ■■											
		Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B		
Max Gable Vertical Length	24" o.c.	SPF	#1 / #2	4' 1'	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"		
		HF	Stud	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"		
				3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"		
		Standard	#1	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"		
				4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"		
	16" o.c.	SP	#2	4' 1'	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	12' 10"	13' 4"	14' 0"	14' 0"			
		DFL	#3	4' 0"	5' 7"	5' 11"	7' 11"	7' 5"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"		
				4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"		
		Standard	#1 / #2	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"		
				4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"		
12" o.c.	16" o.c.	SPF	#3	4' 5'	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"			
		HF	Stud	4' 5'	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"			
				4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"			
		SP	#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"			
				4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"			
	12" o.c.	DFL	Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"			
		Standard	#1 / #2	4' 5'	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"		
				5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"			
		SPF	#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"			
				4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"			
12" o.c.	HF	Stud	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 7"	12' 8"	14' 0"	14' 0"	14' 0"				
	Standard	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"				
			5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"				
	SP	#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"				
			5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"				
DFL	Stud	4' 10"	6' 11"	7' 4"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"					

Diagonal brace options: vertical length may be doubled when diagonal brace is used. Connected diagonal brace for 5250 at each end. Max web total length is 14'.

Vertical length shown
In table above.

Connect diagonal at midpoint of vertical web.

Refer to chart change for
State of Alaska
Federal Reserve Bank of San Francisco
San Francisco, California
March 1968

◆IMPORTANT—READ AND FELLOW ALL NOTES ON THIS INVOICE. ◆IMPORTANT—FURNISH THIS INVOICE TO ALL CONTRACTORS INSTALLING THE INSTALLERS. require extreme care in fabricating, handling, shipping, installing and testing. Refer to and the latest edition of ESSI Guiding Component Safety Information, by TPI and ESSI, for safety prior to performing these functions. Installers shall provide temporary bracing per ESSI. used otherwise; top chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing. The top chord shall be permanently braced back to the building frame for permanent lateral restraint of webs. Backing installed per ESSI 35.7.1 shall be used to provide lateral restraint to each face and position as shown above and on the TPI details, unless noted otherwise.

division of ITV Building Components Group Inc. shall not be responsible for any deviation from the drawings or specifications. The user shall hold the trust in conformance with ANSI/TPI 1, or for handling, shipping, or any other use of the drawings or specifications. This drawing or cover page indicates acceptance of professional responsibility solely for the design shown. The user assumes the responsibility and use of this drawing structure is the responsibility of the Building Designer per ANSI/TPI 1, Sec.2.

For more information see this job's general notes page and these web sites:
 PTD: www.alpha-beta.com TPI: www.tpi.net or SBCA: www.sbcacompounds.com ICD: www.icdusa.com
 02/01/2023 2:78

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

MAX. TDT. LD. 60 PSF

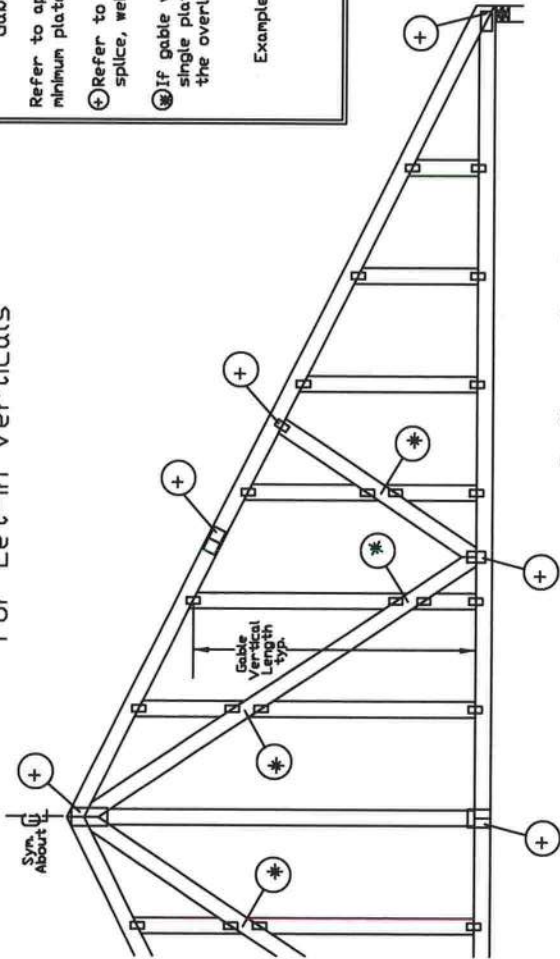
MAX. SPACING	24.0"
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Yoonhwak Kim, FL PE #86367

For more information see this job's general notes page and these web sites:
PTSD: www.alphahta.com TPI: www.tbstat.org SBCEA: www.sbccomponents.com IDC: www.idc.com
02/01/2023

Glenview, IL 60025

Gable Detail For Let-In Verticals

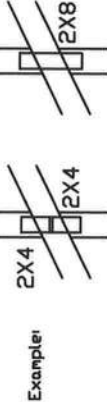


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

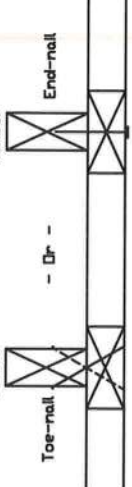
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



Example

T Reinforcement Attachment Detail
T Reinforcing Member



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.
'T' reinforcing member material must match size, species, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

T Reinf.	*T* Mbr. Size	Increase
2x4	2x6	30 %
2x6	2x8	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

- End Driven Nails:
- 10d Common (0.148"x 3", min) Nails at 4' o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:

- 10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

- A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

- A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A22015ENC100118, A24015ENC100118, A26015ENC100118, A28015ENC100118, A30015ENC100118, A32015ENC100118, A34015ENC100118, A36015ENC100118, A38015ENC100118, A40015ENC100118, A42015ENC100118, A44015ENC100118, A46015ENC100118, A48015ENC100118, A50015ENC100118, A52015ENC100118, A54015ENC100118, A56015ENC100118, A58015ENC100118, A60015ENC100118, A62015ENC100118, A64015ENC100118, A66015ENC100118, A68015ENC100118, A70015ENC100118, A72015ENC100118, A74015ENC100118, A76015ENC100118, A78015ENC100118, A80015ENC100118, A82015ENC100118, A84015ENC100118, A86015ENC100118, A88015ENC100118, A90015ENC100118, A92015ENC100118, A94015ENC100118, A96015ENC100118, A98015ENC100118, A100015ENC100118, A102015ENC100118, A104015ENC100118, A106015ENC100118, A108015ENC100118, A110015ENC100118, A112015ENC100118, A114015ENC100118, A116015ENC100118, A118015ENC100118, A120015ENC100118, A122015ENC100118, A124015ENC100118, 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Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

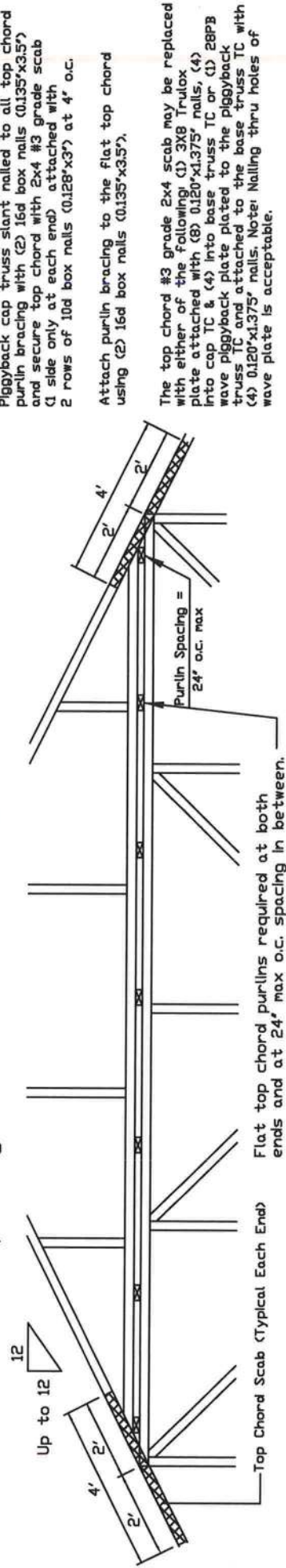
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 50 psf (min), Kzt=1.0, Dr 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 50 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

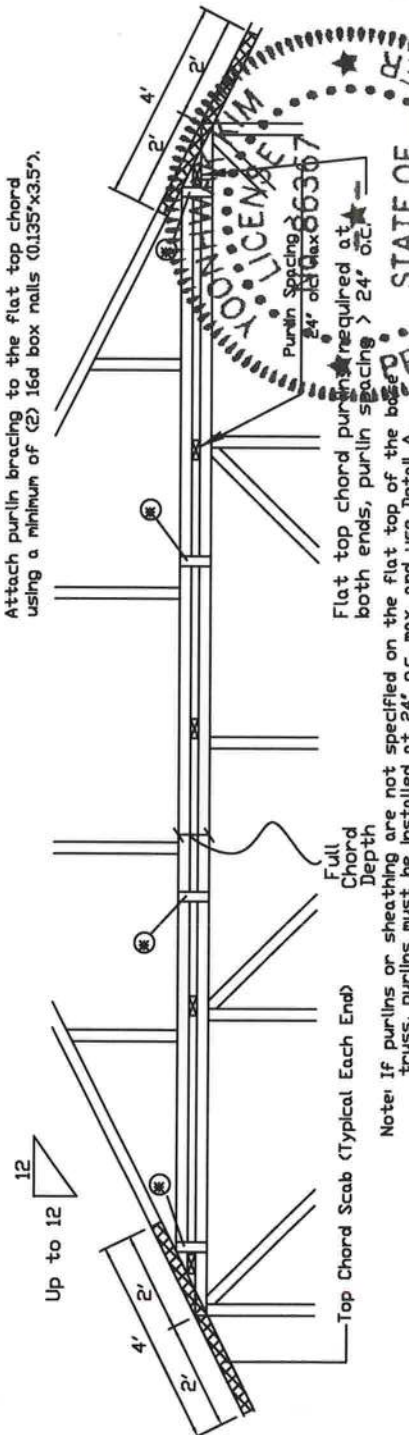
Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.

In addition, provide connection with one of the following methods:	
Trulux	Use 3X8 Trulux plates for 2x4 chord member, and 3X10 Trulux plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulux plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset	8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
2x4 Vertical Scabs	2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
28P3 Wave Piggyback Plate	One 28P3 wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication, attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.



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IMPORTANT! FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.
Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and SCSA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have properly attached structural sheathing. Trusses shall be braced in all directions. Refer to drawings 150A-2 for standard plate positions.
Alpha, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2.
For more information see this job's general notes page and these web sites:
ALPINE: www.alpineinc.com TPI: www.tpi.org SCSA: www.scsacomponents.com BCSI: www.bcsicomponents.com

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118
SPACING	24.0"