

2802399 RE:

SIMQUE - LOT 54 PLL

BUILDING

Tampa, FL 33610-4115

MiTek USA, Inc. 6904 Parke East Blvd. Received

ō

Site Information:

Customer: Aaron Simque Homes Project Name: 2802399 Lot/Block: 54 Model: 2281

Address: TBD

Subdivision: The Preserve at Laurel Lake State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special City: Columbia Cty

Design Program: MiTek 20/20 8.4 Design Code: FBC2020/TPI2014 Loading Conditions):

ASCE 7-16

Wind Code:

Roof Load: 37.0 psf

EXAMIN

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Wind Speed: 130 mph Floor Load: N/A psf

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet This package includes 37 individual, dated Truss Design Drawings and 0 Additional Drawings.

confc	31G15-31.003	section 5 of the	Florida Board o	of Profe	essional Engineers R	ules.	
No.		Iruss Name	Date	SO.	Seal#	I russ Name	Date
_		CJ01	5/24/2021	21	T24044005	T08A	5/24/2021
7	T24043986	CJ01P	5/24/2021	22	T24044006	T08B	5/24/2021
က		CJ03	5/24/2021	23	T24044007	T09	5/24/2021
4	T24043988	CJ03A	5/24/2021	24	T24044008	T10	5/24/2021
5		CJ05	5/24/2021	25	T24044009	T11	5/24/2021
9		CJ05A	5/24/2021	26	T24044010	T12	5/24/2021
7		EJ01	5/24/2021	27	T24044011	T13	5/24/2021
8	T24043992	EJ02	5/24/2021	28	T24044012	T14	5/24/2021
6		EJ03	5/24/2021	29	T24044013	T15	5/24/2021
10		HJ05	5/24/2021	30	T24044014	T16	5/24/2021
7		HJ10	5/24/2021	31	T24044015	T17	5/24/2021
12		HJ10A	5/24/2021	32	T24044016	T18	5/24/2021
13		T01	5/24/2021	33	T24044017	T19	5/24/2021
14	T24043998	T02	5/24/2021	34	T24044018	T20	5/24/2021
15		T03	5/24/2021	35	T24044019	T21	5/24/2021
16		T04	5/24/2021	36	T24044020	T22	5/24/2021
17	T24044001	T05	5/24/2021	37	T24044021	T23	5/24/2021
18	T24044002	T06	5/24/2021				
19	T24044003	T07	5/24/2021				
20	T24044004	T08	5/24/2021				

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc under my direct supervision

based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2023. Florida COA: 6634

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek. Any project specific information included is for MITek customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

No 68182

No 68182

No 68182

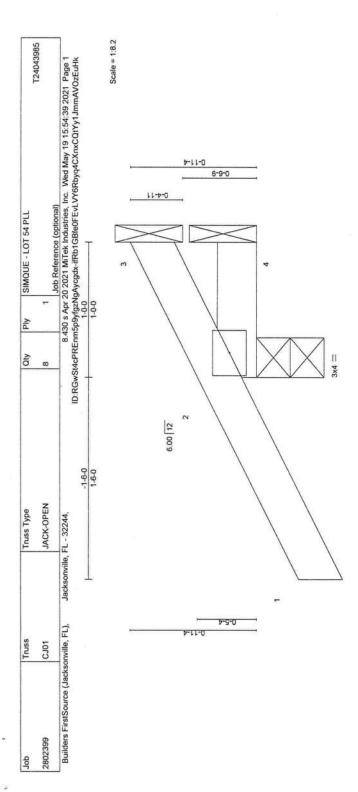
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STATE OF WELL

SONAL EVO Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

May 24, 2021

Velez, Joaquin



		GRIP	244/190			FT = 20%	oc purlins.
		PLATES	MT20			Weight: 6 lb	Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.
	T	2	240	180	n/a		heathing dire
		l/defi	>999	>999	n/a		I wood s ing direc
1-0-0	1-0-0	(loc)	7	1	2		structura tigid ceil
		.⊑	0.00	0.00	00.0		
		DEFL.	Ver(LL)	Vert(CT)	Horz(CT)		BRACING- TOP CHORD BOT CHORD
			0.20	BC 0.04	0.00	x-MP	
		CSI.	7	BC	WB	Matri	
		2-0-0	1.25	1.25	YES	PI2014	
		SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/T	0.2
		st)	20.0	0.	. 0.0	10.0	2x4 SP No.2 2x4 SP No.2
		ADING (ps	20	7	0	10	LUMBER- TOP CHORD BOT CHORD
		LOAD	TCLL	TCDL	BCLL	BCDL	LUMBER- TOP CHOI BOT CHOI

2x4 SP No.2 2x4 SP No.2 TOP CHORD BOT CHORD

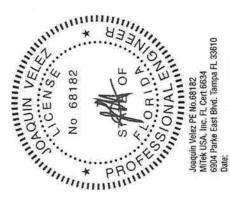
(size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=57(LC 12) Max Upliff 3=-6(LC 1), 2=-106(LC 12), 4=-19(LC 1) Max Grav 3=9(LC 8), 2=179(LC 1), 4=25(LC 16) REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 \$Refer to girder(s) for truss to truss connections.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (||=|b|) 2=106.



May 22,2021

besign valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building especial connectors. This design is based only upon parameters shown, and is for an individual building especial building design real vertices. The building design into the overall using design. Blacing independ is to prevent bucking of individual truss web andior chord members only. Additional temporary and permanent bracing a sharys required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the abordance is a constructed and bracing of trusses and fruss systems, see

ANSIMPI Quality Criteria, DSB-88 and BCSI Building Component after Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waidorf, MD 20601



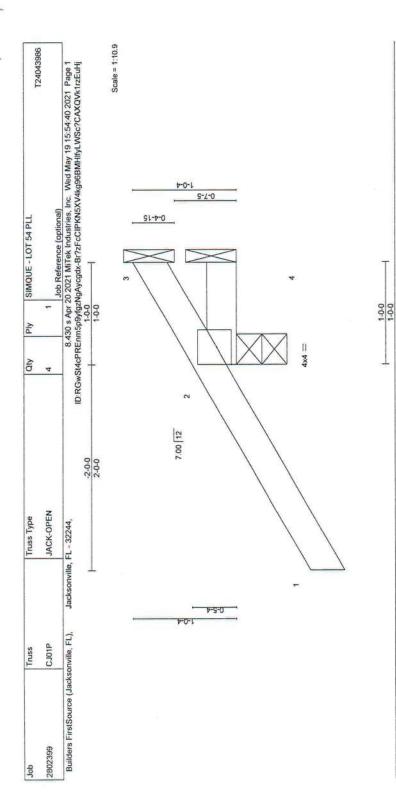


Plate Offsets (X,Y)- [2:Edge,0	- [2:Edge,0-1-12]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	P/I	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.43			-	240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25	BC 0.12		0.00	>999	180			
BCLL 0.0	Rep Stress Incr	YES	WB 0.00		00	n/a	n/a			
BCDL 10.0	Code FBC2020/TI	PI2014	Matrix-MP					Weight: 7 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4	2x4 SP No.2 2x4 SP No.2			BRACING- TOP CHORD BOT CHORD	Structi Rigid o	rral wood	sheathing din	Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.	oc purlins.	

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=78(LC 12) Max Upliff 3=-27(LC 1), 2=-166(LC 12), 4=-52(LC 19) Max Grav 3=25(LC 16), 2=254(LC 1), 4=47(LC 16)

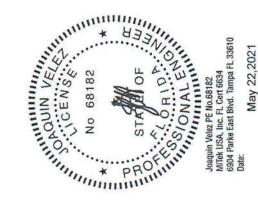
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 to the use of this truss component.
 - 3) This truss has been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.

 4) This truss has been designed for a live load of 20.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 5) Refer to girder(s) for truss to truss connections.

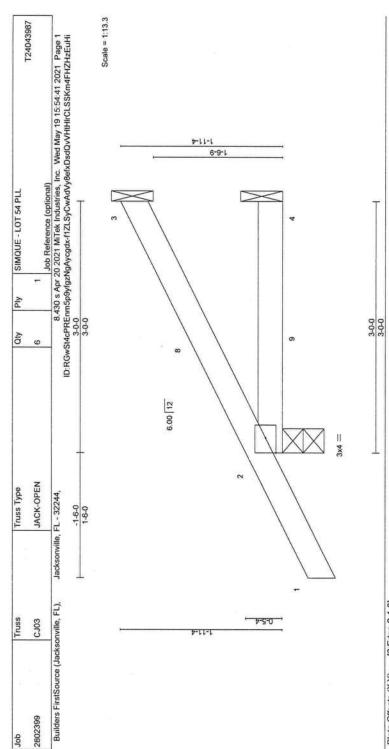
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (||=|b|) 2=166.



May WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with Mitek® connectors. This design is based only upon parameters show, and its for an individual building component, not a trust so system. Before use, the building design arrameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent outsking of individual inuss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and properly damage. For general guidance regarding the fabrication is not prevent colleges with possible personal injury and properly damage. For general guidance regarding the fabrication and bracing of trusses and truss systems, see





	GRIP	244/190			FT = 20%	oc purlins.
	PLATES	MT20			Weight: 12 lb	Structural wood sheathing directly applied or 3-0-0 oc purlins Rigid ceiling directly applied or 10-0-0 oc bracing.
	5	240	180	n/a	All and a second	heathing dire
	l/defl	>999	>999	n/a		al wood s ling direc
	(loc)	4-7	4-7	3		structura tigid cei
	.⊑	0.01	-0.01	-0.00		NOTES TO A
	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)		BRACING- TOP CHORD BOT CHORD
	CSI.	TC 0.20	BC 0.15	WB 0.00	Matrix-MP	=
	2-0-0	1.25	1.25	YES	12014	11
[2:Edge,0-1-8]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TF	No.2 No.2
	(Jsd	20.0	7.0	. 0.0	0.01	D 2x4 SP No.2 D 2x4 SP No.2
Plate Offsets (X,Y)-	LOADING (TCLL	TCDL	BCLL		LUMBER- TOP CHORD BOT CHORD
- 4140						

Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=105(LC 12) Max Uplift 3=-58(LC 12), 2=-98(LC 12), 4=-27(LC 9) Max Grav 3=60(LC 1), 2=210(LC 1), 4=50(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 2) Building Designer | Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 5) Refer to girder(s) for truss to truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb upliff at joint(s) 3, 2, 4.

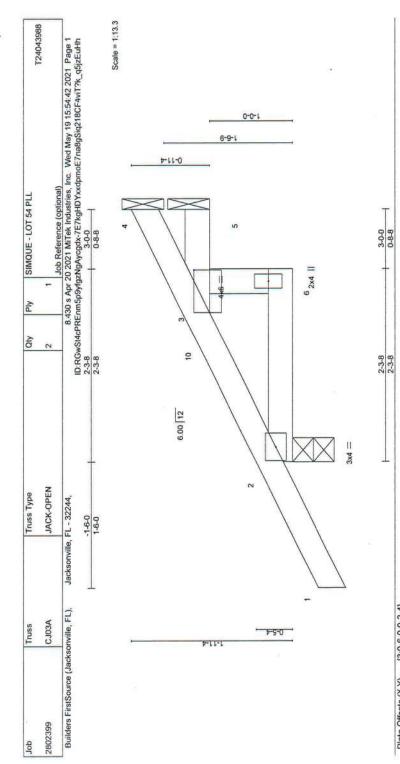


y design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., \$19,2020 BEFORE USE.

With With MIR Ride Sonnectors. This design is based only upon parameters shown, and its for an individuab building component, not rere use, the building designer mast verify the applicability of design parameters and property incorporate this design into the overall acing individual truss were undor chord members only. Additional temporary and permanent bracing individual truss were undor chord members only. Additional temporary and permanent bracing the stability and property dramage. For general guidance regarding the delivery, erection and bracing thus and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Con available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 is and READ NOTES ON THIS AREA
Connectors. This design is based only upon parameters are
ng designer must verify her applicability of design parameter
in designer must verify her and/or chord me
are and/or chord me
are some injury and property sign valid for use only with MITek® uss system. Before use, the buildin





/										П
LOADING (psf)	SPACING-	2-0-0	CSI.		2	(loc) I/defi		PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.20	Vert(LL)	0.01		240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25	BC 0.10	10	100	566< 9				
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00		100	5 n/s	n/a			
BCDL 10.0	Code FBC2020/TPI201	PI2014	Matrix-MR					Weight: 14 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 'Except'	SP No.2 SP No.2 "Except"			BRACING- TOP CHORD BOT CHORD	Stru	ictural woo	d sheathing d rectly applied	Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.	oc purlins.	1

2x4 SP No.2 "Except" 3x6 SP No.2 "Except" 3x6: 2x4 SP No.3 REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=105(LC 12) Max Upliff 4=-38(LC 12), 2=-97(LC 12), 5=-19(LC 12) Max Grav 4=49(LC 11), 2=-211(LC 11), 5=-45(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

NOTES-

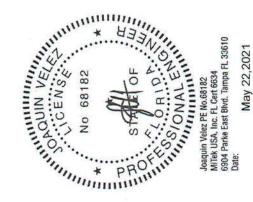
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific

- to the use of this truss component.

 3) This truss has been designed for a 10.0 pst bottom chord live load shown covers rain loading requirements specific to the use of this truss bas been designed for a 10.0 pst bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0 pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 5) Refer to girder(s) for truss to truss connections.

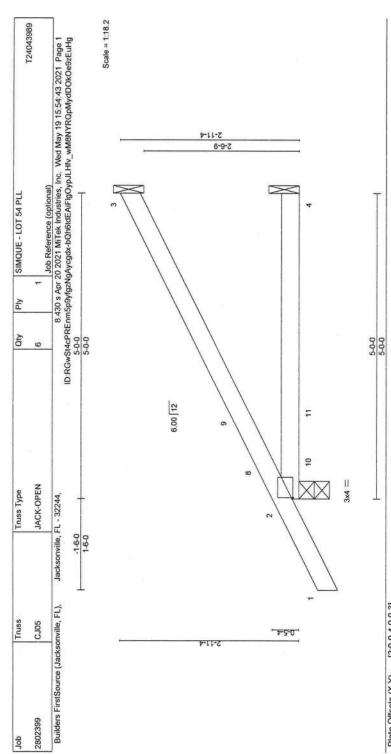


May WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev., 5/19/2020 BEFORE USE.

Design wall for use only with MTek80 connectors. This design is based only upon parameters and nis for an individual building designer must verify the applicability of design parameters and properly incorporate his design into the overall building design. Bracing indicated is to prevent obtained of individual truss web androit choird members only. Additional temporary and permanent bracing is always required for stability and to prevent obtained of individual truss web androit choird members only. Additional temporary and permanent bracing is always required for stability and to prevent obtained with the second injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





					9	
	GRIP	244/190			FT = 20%	oc purlins.
	PLATES	MT20			Weight: 18 lb	thing directly applied or 5-0-0 oc purlins. applied or 10-0-0 oc bracing.
	3	240	180	n/a		
	l/defl	>473	>518	n/a		Structural wood shea
	(loc)	4-7	4-7	က		Structura Rigid ceil
	.⊆	0.13	0.11	-0.01		0712
	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	-	BRACING- TOP CHORD BOT CHORD
	CSI.	TC 0.55	BC 0.55	WB 0.00	Matrix-MP	
	2-0-0	1.25	1.25	YES	12014	
Plate Offsets (A, 1) - [2.0-0-4,0-0-3]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TF	No.2 No.2
ets (A, Y)-	(bsd)	20.0	7.0	• 0.0	10.0	LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2
Plate Oilse	LOADING	TCLL	TCDL	BCLL	BCDL 10.0	LUMBER- TOP CHOR BOT CHOR

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=155(LC 12) Max Uplift 3=-107(LC 12), 2=-114(LC 12), 4=-48(LC 9) Max Grav 3=114(LC 1), 2=276(LC 1), 4=89(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20h; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(ZE) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this fruss component.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

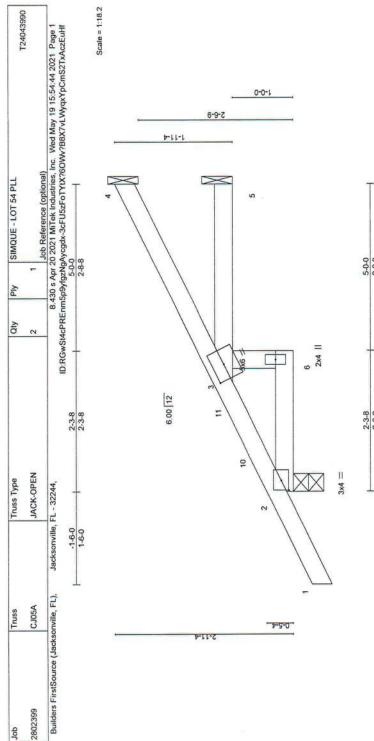
 5) Refer to girder(s) for truss to truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (|i=b)



NG -Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
If for use only with Mither& connections. This design is based only upon parameters shown, and is for an individual building component, not
leave use, the building designer must verify the applicability of design parameters shown, and properly incropate this design into the overall
sign. Biracing indicated is to prevent bucking of individual truss web androff or hord members only. Additional temporary and permanent tracing
squired for stability and prevent ordispse with possible personal injury and property damage. For general guidance regarding the
ANSIGTPH Country, erection and bracing of fursses and truss systems, see
ANSIGTPH Country Criteria, DSB-89 and BCSI Building Component
remation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Weldorf, MD 20601





2-3-8		DEFL.	>731 240 MT20	Vert(CT) -0.08 6 >728	Horz(CT) 0.05 5 n/a n/a	
	2-4]	2-0-0 CSI.		1.25 BC		2014 Matr
	2:0-1-15,0-1-8], [3:0-3-0,0-2	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TPI2
	Plate Offsets (X,Y)-	LOADING (psf)	TCLL 20.0			BCDL 10.0

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

BRACING-TOP CHORD BOT CHORD

2x4 SP No.2 2x4 SP No.2 "Except" 3-6: 2x4 SP No.3 REACTIONS.

TOP CHORD BOT CHORD

LUMBER-

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=155(LC 12) Max Uplift 4=-87(LC 12), 2=-114(LC 12), 5=-24(LC 12) Max Grav 4=102(LC 1), 2=278(LC 1), 5=83(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E)-1-6-0 to 1-6-0. Interior(1) 1-6-0 to 4-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

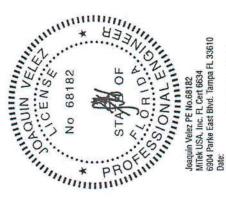
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will between the bottom chord and any other members.

 5) Refer to girder(s) for truss to truss connections.

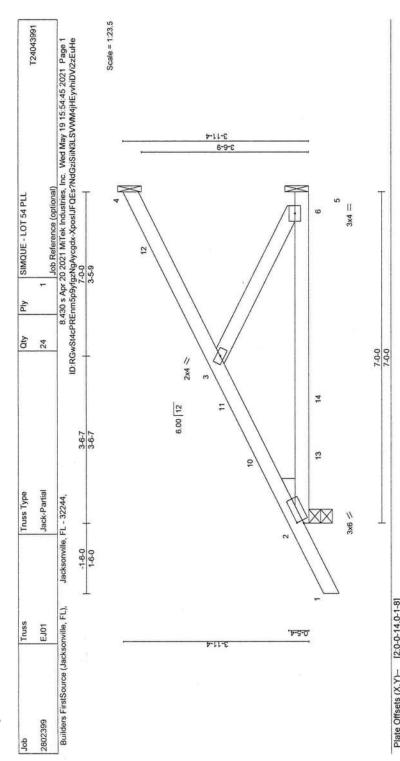
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (il=1b) 2=114.



May 22,2021

Design valid for use only with Marke connectors. This design is based only upon parameters show, and is for an individual building component, not a trust saysfam. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building to the parameters and properly incorporate this design into the overall is a ways required to stability and to prevent collapse with possible personal injury and properly demande. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see a ANSITPR Quality Criteria, DSB-89 and BCSI Building Component Sarety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20001





LOADING (psf) SPACING- 2- TCLL 20.0 Plate Grip DOL 1								
Plate Grip DOL		190	1	.5	10011	Habit	7	DI ATEC
Plate Grip DOL	2-2-2	So	DEFL.	=	(loc)	l/dell		PLAIES
	1.25	TC 0.51	Vert(LL)	0.25	6-9	>329	240	MT20
Lumber DOL	1.25	BC 0.75	Vert(CT)	0.23	6-9	>364	180	
Rep Stress Incr	YES	WB 0.16	Horz(CT)	-0.01	2	n/a	n/a	
1	014	Matrix-MS						Weight: 31 lb

FT = 20%

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 5-2-14 oc bracing.

BRACING-TOP CHORD BOT CHORD

GRIP 244/190

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 LUMBER-TOP CHORD BOT CHORD WEBS

Left: 2x4 SP No.3

REACTIONS.

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=197(LC 12) Max Uplift 4=-64(LC 12), 2=-137(LC 12), 5=-125(LC 9) Max Grav 4=74(LC 1), 2=346(LC 1), 5=175(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. D 2-3=-268/243 D 2-6=-471/230 3-6=-261/536 FORCES.

TOP CHORD BOT CHORD WEBS

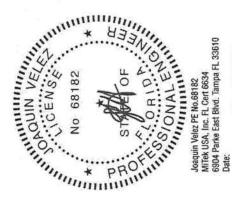
NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
- to the use of this truss component.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 5) Refer to girder(s) for truss to truss connections.

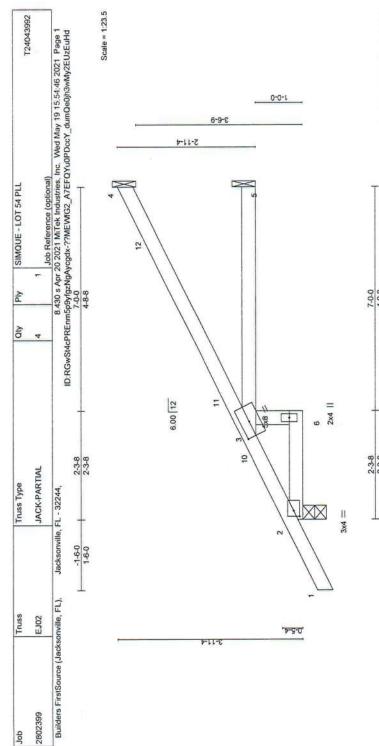
 6) Provide mechanical connection (by others) of tru 2=137, 5=125



May 22,2021

is and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE FAGE MII-7473 rev. 5/19/2020 BEFORE USE. connectors. This design is based only upon parameters shown, and it for an individual building component, not glo designer must verify the applicability of design parameters and property incoporate his design into the overall op prevent bucking of individual trues web andfor chord members only. Additional temporary and permanent bracing trevent collapses with possible personal injury and property damage. For general guidance regarding the trevent collapses with possible personal injury and property damage. For general guidance regarding the ANSITTPI Quality Criteria, DSB-89 and BCSI Building Com uss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 hanger in MITek® connectors are, the building designer ing indicated is to prevent call cantility and to prevent coll





					Weight: 27 lb FT = 20%
	P/I	240	180	n/a	
	l/defi	>363	>316	n/a	
	(loc)	3-5	3-5	2	
	.⊑	0.23	-0.26	0.13	
	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
	CSI.	TC 0.66	BC 0.65	WB 0.00	Matrix-MR
2-4]	2-0-0	1.25	1.25	YES	12014
[2:0-1-7,0-1-8], [3:0-4-0,0	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TF
Plate Offsets (X,Y)-	LOADING (psf)	TCLL 20.0	TCDL 7.0	BCLL 0.0 *	BCDL 10.0
	Plate Offsets (X,Y)- [2:0-1-7,0-1-8], [3:0-4-0,0-2-4]	2-0-0 CSI. DEFL. in (loc) lidefi Lid PLATES	Offsets (X,Y)- [2:0-1-7,0-1-8], [3:0-4-0,0-2-4] CSI. DEFL. in (loc) i/defl NING (psf) SPACING- 2-0-0 CSI. TC 0.66 Vert(LL) 0.23 3-5 >363	Offsets (X,Y)- [2:0-1-7,0-1-8], [3:0-4-0,0-2-4] ING (psf)	2-0-0 CSI. in (loc) l/defl L/d PLATES GR 1.25 TC 0.66 Vert(LI) 0.23 3-5 >363 240 MT20 24 1.25 BC 0.65 Vert(CT) -0.26 3-5 >316 180 YES WB 0.00 Horz(CT) 0.13 5 n/a n/a

2x4 SP No.2 2x4 SP No.2 *Except* 3-6: 2x4 SP No.3 TOP CHORD BOT CHORD

LUMBER-

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

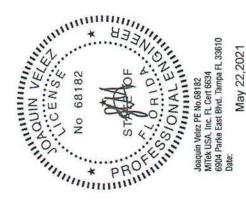
BRACING-TOP CHORD BOT CHORD

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical Max Horz 2=197(LC 12) Max Uplift 4=-118(LC 12), 2=-136(LC 12), 5=-27(LC 12) Max Grav 4=153(LC 1), 2=348(LC 1), 5=120(LC 3) REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown FORCES.

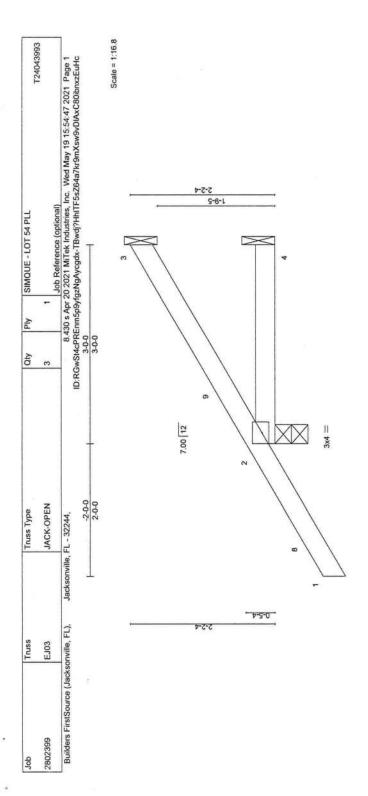
NOTES-

- 1) What ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) Building Designer P Poject engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 3) This truss has been designed for a 10-0 psf bottom chord live load nonconcurrent with any other live loads.
 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 5) Refer to girder(s) for truss connection.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (it=lb) 4=118, 2=136.



Design valid for use only with Mittels connections. This design is based only upon parameters shown, and is for an individual building component, not a trust system. Before use, the building designer must verify the applicability of design parameters and properly incorporate its design in to the overall building designer use, the which the applicability of design parameters only. Additional temporary and permanent building design and to prevent Collapse with possible personal injury and properly demangers only. Additional temporary and permanent bracing is always required for stability and to prevent Collapse with possible personal injury and properly damage. For general guidance regarding the tability and properly demanger delivery, erection and bracing of frusses and truss systems, see a MNSITPPII Quality Criteria, DSB-89 and BCSI Building Component Safery Information available from Truss Plate Institute. 2010 Crain Highway, Suite 203 Waldorf, MD 20601 5/19/2020 BEFORE USE and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473





						9	2-0-0					
Plate Offsets (X,Y)- [2:Edge,)- [2:Edge,0-1-4]											
LOADING (psf)	SPACING-	1G-	2-0-0	CSI.						PLATES	GRIP	
TCLL 20.0	Plate Gr.	rip DOL	1.25	TC	0.39	Vert(LL)	-0.01	4-7 >9	>999 240	MT20	244/190	
TCDL 7.0	Lumber	Lumber DOL	1.25	BC 0.11	0.11		-0.01	4-7 >9				
BCLL 0.0	* Rep Stre	ess Incr	YES	WB	0.00		0.00	3	n/a n/a			
BCDL 10.0	Code FI	Code FBC2020/TP	12014	Matrix	·MP					Weight: 13 lb	lb FT = 20%	9
LUMBER- TOP CHORD 29 BOT CHORD 29	2x4 SP No.2 2x4 SP No.2					BRACING- TOP CHORD BOT CHORD		uctural w	ood sheathii g directly app	Structural wood sheathing directly applied or 3-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.	3-0 oc purlins. g.	195

3=Mechanical, 2=0-3-8, 4=Mechanical (size) REACTIONS.

Max Horz 2=135(LC 12) Max Uplift 3=-57(LC 12), 2=-123(LC 12) Max Grav 3=63(LC 19), 2=253(LC 1), 4=48(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 2-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific

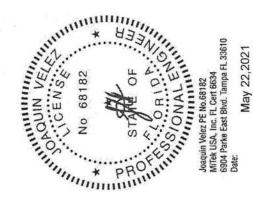
- to the use of this truss component.

 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) *This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

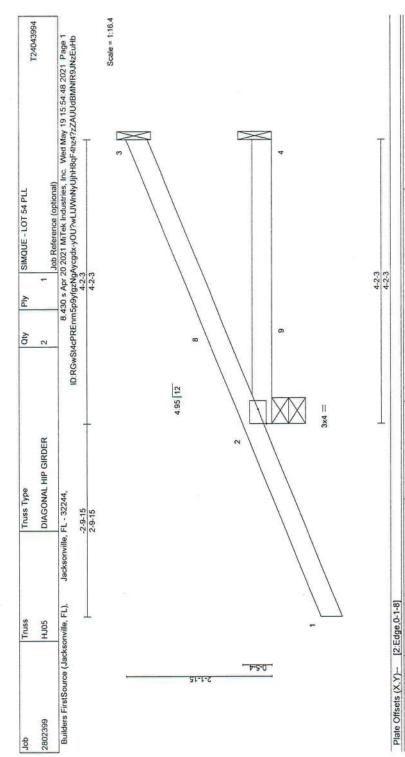
 5) Refer to girder(s) for truss to truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (i=lb) 2=123.



b and/or chord members only. Additional temporary and permanent bracing njury and propenty damage. For general guidance regarding the ANSITPI Quality Criteria, DSB-89 and BCSI Building Component Suite 203 Waldorf, MD 20601 and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-473 rev. 6192020 BEFORE USE onnectors. This design is based only upon parameters shown, and is for an individual building component, not designer must verify the applicability of design parameters and properly incorporate this design into the overall





	PLATES GRIP				eight: 18 lb FT = 20%
	7	M			Wei
	P/I	240	180	n/a	
	l/defi	>893	>951	n/a	
	(loc)	4-7	4-7	8	
	⊒.	-0.06	0.05	0.01	
	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	80
	CSI.	0.56	0.43	0.00	rix-MP
	CSI	TC	BC	WB	Mat
	2-0-0	1.25	1.25	ON ON	PI2014
[2:Edge,0-1-8]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/T
Plate Offsets (X,Y)-	LOADING (psf)	TCLL 20.0	TCDL 7.0	BCLL 0.0 •	BCDL 10.0

2x4 SP No.2 2x4 SP No.2 LUMBER-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-3 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BRACING-TOP CHORD BOT CHORD

(size) 3=Mechanical, 2=0.4-9, 4=Mechanical Max Horz 2=134(LC 8) Max Uplift 3=-77(LC 8), 2=-281(LC 4), 4=-46(LC 16) Max Grav 3=54(LC 1), 2=282(LC 1), 4=67(LC 30) REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. FORCES.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
- will fit between the bottom chord and any other members.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) 3, 4 except (il=lb)
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 103 lb up at 1-6-1, and 85 lb down and 103 lb up at 1-6-1 on top chord, and 72 lb down and 74 lb up at 1-6-1, and 82 lb down and 103 lb up at 1-6-1 on top chord, and 72 lb down and 74 lb up at 1-6-1 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). 2=281.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)
Vert: 1-3=-54, 4-5=-20
Concentrated Loads (lb)
Vert: 8=51(F=25, B=25) 9=69(F=34, B=34)

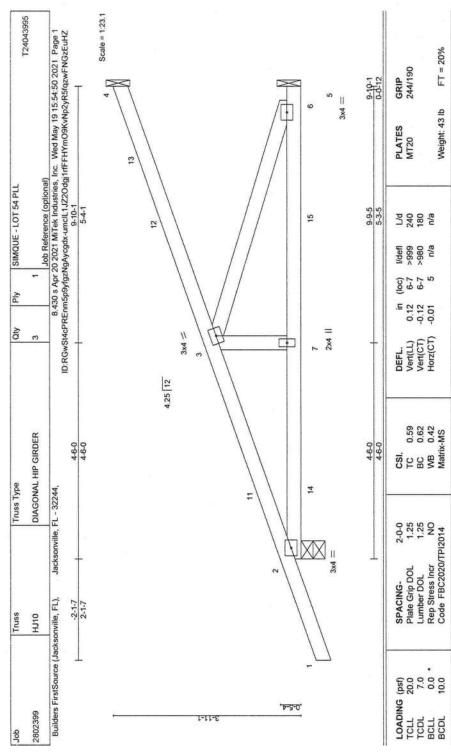
No 68182

No 68182

STATE OF ELEVANTER OF EL Joaquin Velez PE No. 68182 Mitek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date: May 22,2021

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not the trust system, be building designer must verify the applicability of design parameters and properly incorporate this design into the everall building design. Bacing indicated is to prevent buckling of individual fruss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see AMSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety information available from Truss Plate institute, 2870 Coain Highway, suite 203 Waldoot, MID 20601 MII-7473 rev. 5/19/2020 BEFORE USE and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE





2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD LUMBER-WEBS

4=Mechanical, 2=0-4-9, 5=Mechanical

(size)

REACTIONS.

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-8-13 oc bracing.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. D 2-3=-765/594 D 2-7=-663/692, 6-7=-663/692 3-7=-143/276, 3-6=-732/701 Max Horz 2=215(LC 4) Max Uplift 4=-126(LC 4), 2=-477(LC 4), 5=-276(LC 4) Max Grav 4=151(LC 1), 2=526(LC 1), 5=297(LC 1) FORCES.

TOP CHORD BOT CHORD WEBS

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 5) Refer to girder(s) for truss to truss connections.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) except (it=lb) 4-126, 2-477, 5=276.
- 46 4=126, 2=477, 5=276.

 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 87 lb down and 77 lb up at 1-6-1, 28 lb down and 57 lb up at 4-4-0, 28 lb down and 57 lb up at 4-4-0, and 52 lb down and 115 lb up at 7-1-15, and 52 lb down and 46 lb up at 1-6-1, 57 lb down and 46 lb up at 1-6-1, 57 lb down and 48 lb up at 1-6-1, 20 lb down and 35 lb up at 4-4-0, and 41 lb down and 63 lb up at 1-1-15 and 41 lb down and 63 lb up at 7-1-15 and 41 lb down and 63 lb up at 7-1-15 and 41 lb down and 63 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

No 68182

No 68182

STATE OF

RESIDENCE OF

RESIDENCE

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-4=-54, 5-8=-20

Concentrated Loads (Ib)

Vert: 7=-4(F=-2, B=-2) 12=-74(F=-37, B=-37) 15=-57(F=-29, B=-29)

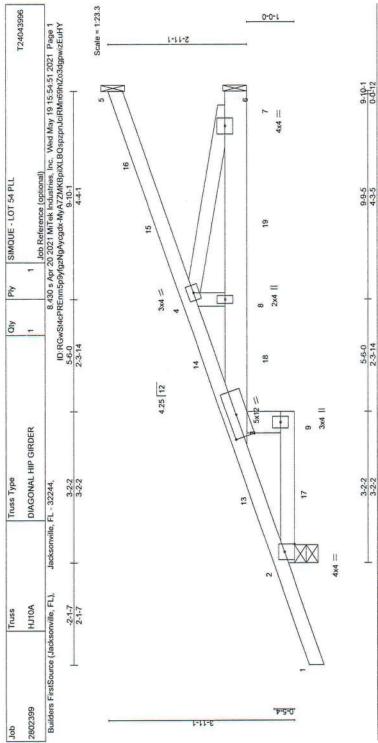
May 22,2021 MiTek

6904 Parke East Blvd. Tampa, FL 36610

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

rem. before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not assign parameters and properly incorporate this design into the overall sign. The action indicated is to prevent collapse with possible personal right of members only. Additional temporary and permanent bracing squited for stability and to prevent collapse with possible personal right and property damage. For general guidance regarding the stones and truss systems, see

ANSITPH Quality Criteria, DSB-49 and BCSI Building Component margin available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 s and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473



		5	7-7		200				0	2		
		3.	3-2-2		2-3-14				4-3-5	-0	0-0-12	
Plate Offsets (X,Y)-	[2:0-2-0,Edge], [3:0-6-0,0-2	2-2]										
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	.5	(loc)	l/defi	P/I	PLATES	GRIP	1
TCLL 20.0	Plate Grip DOL	1.25	75	080	Vert(LL)	0.33	e	>356	240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25	BC	0.82	Vert(CT)	-0.37	က	>320	180			
BCLL 0.0 *	Rep Stress Incr	ON ON	WB	0.47	Horz(CT)	0.16	9	n/a	n/a			
BCDL 10.0	Code FBC2020/TF	12014	Matrix-	MS						Weight: 50 lb	FT = 20%	

TOP CHORD BOT CHORD LUMBER-

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 7-0-8 oc bracing.

BRACING-TOP CHORD BOT CHORD

2x4 SP M 31 2x6 SP No.2 "Except" 2-9: 2x4 SP No.2 2x4 SP No.3

WEBS

REACTIONS.

(size) 5=Mechanical, 2=0.4-9, 6=Mechanical Max Horz 2=215(LC 4) Max Upliff 5=-82(LC 4), 2=-392(LC 4), 6=-219(LC 8) Max Grav 5=111(LC 1), 2=542(LC 1), 6=351(LC 1)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. D 3-11=-268/88, 3-4=-1277/818 D 3-8=-885/1214, 7-8=-893/1225 4-8=-208/373, 4-7=-1262/920

FORCES. (lb) TOP CHORD BOT CHORD WEBS

NOTES

- Monta: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., 10xnd: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., 10xnd: Ascendible of specific and a specific

- 2=392, 6=219.
- 2–394, 0–219.

 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 87 lb down and 77 lb up at 1-6-1, 28 lb down and 36 lb up at 4-4-0, 28 lb down and 36 lb up at 4-4-0, and 51 lb down and 94 lb up at 7-1-15, and 51 lb down and 94 lb up at 7-1-15, and 51 lb down and 94 lb up at 7-1-15 on top chord, and 24 lb down and 46 lb up at 1-6-1, 24 lb down and 28 lb up at 4-4-0, 24 lb down and 28 lb up at 4-4-0, and 41 lb down and 41 lb up at 7-1-15, and 41 lb down and 41 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

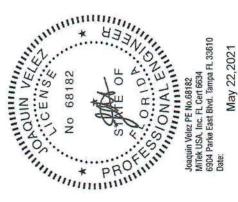
 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (pif) Vert: 1-3=54, 3-5=54, 9-10=20, 3-6=-20

Concentrated Loads (Ib)

Vert: 14=-3(F=-2, B=-2) 15=-51(F=-26, B=-26) 18=-31(F=-15, B=-15) 19=-83(F=-41, B=-41)

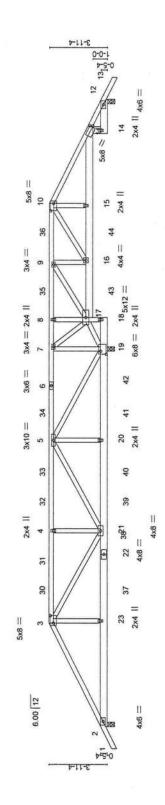


and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 rev. 519/2020 BEFORE USE



T24043997 | A | Job Reference (optional) | R430 s Apr 20 2021 MTek Industries, Inc. Wed May 19 15:54:58 2021 Page 1 ID:RGwSt4cPREnm5p9/gzN4p3cyc4/J5nrmPaAsexhGSCjxRG5dJecbwqxtqgDShgozEuHR 25:5-12 | 314-1 | 35-4.0 | 40.0-8 | 42.40 43.10-9 | 6-1-13 | 2-1-12 | 3-8.8 | 3-11-15 | 4-8.8 | 2-3.8 | 1-6-0 SIMQUE - LOT 54 PLL 2 Ply aty HIP GIRDER Truss Type Jacksonville, FL - 32244 Builders FirstSource (Jacksonville, FL), 10 2802399

= 1:75.1



7-0-0	0-0-1	13-2-14	19-3-15	25-5-12	27-7-8	314-1	35-4-0	40-0-8	42-4-0
Oct (V V) atombo otolo		6-2-14	6-1-1	6-1-13	2-1-12	3-8-8	3-11-15	4-8-8	2-3-8
riale Oilsets (A, 1)- [3.0	[3:0-6-0,0-2-8], [10:0-6-0,0-2-8], [1:0-0-14,0-2-4], [19:0-2-8,0-4-8]						
OADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)		97	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.25		Vert(LL)	0.13 21-23	>999	240	MT20	244/190
CDL 7.0	Lumber DOL	1.25	BC 0.42	Vert(CT)	-0.16 11-15		180		
CLL 0.0 •	Rep Stress Incr	ON.	WB 0.88	Horz(CT)	0.09 12	n/a	n/a		
BCDL 10.0	Code FBC2020/T	PI2014	Matrix-MS					Weight: 503 lb	FT = 20%
UMBER- OP CHORD 2x4 SP No.2 SOT CHORD 2x6 SP No.2 *Exce	2x4 SP No.2 2x6 SP No.2 "Except" 8-18: 2x4 SD No.3			BRACING- TOP CHORD BOT CHORD	CHANGE.	rral wood ceiling dire	Structural wood sheathing directly ap Rigid ceiling directly applied or 10-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-	Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid celling directly applied or 0-0-0 oc bracing. Except: 6-00 oc bracing. Except:	oc purlins. Except:

2x6 SP No.2 *Except* 8-18: 2x4 SP No.3 2x4 SP No.3

REACTIONS.

WEBS

(size) 2=0-3-8, 12=0-3-8, 19=0-3-8 Max Horz 2=92(LC 27) Max Uplift 2=-1172(LC 5), 12=-279(LC 9), 19=-3286(LC 4) Max Grav 2=1480(LC 19), 12=514(LC 20), 19=4727(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2639/2284, 3-4=-2408/2359, 4-5=-2408/2359, 5-7=-1671/3040, 7-8=-1652/2912,

8-9=-1629/2879, 9-10=-418/758, 10-11=-654/333, 11-12=-426/246

BOT CHORD

2-23=-2005/2289, 21-23=-2025/2312, 20-21=-1208/781, 19-20=-1208/781,

18-19=-321/165, 16-7=-758/609, 15-16=-189/594, 11-15=-180/548, 11-14=-141/369

3-23=-495/697, 3-21=-41/1153, 4-21=-709/581, 5-21=-1359/1980, 5-20=-265/539, 5-19=-4121/3140, 7-19=-818/565, 17-19=-3040/1894, 7-17=-92/255, 9-17=-2546/1472, 9-16=-473/972, 10-16=-1591/919, 10-15=-357/742

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2x4 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2x6 2 rows staggered at 0-9-0 oc, 2x4 1 row at 0-9-0 oc.
 Webs connected as follows: 2x4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.

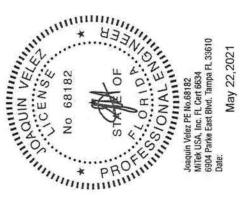
 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.
- GCpi=0.18; MWFRS (envelope) gable end zone, porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60 5) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- to the use of mis truss component.

 5) Provide adequate drainage to prevent water ponding.

 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 8) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
 - will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 6
 - 2=1172, 12=279, 19=3286



Continued on page 2

a fruss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. The Tabling design and permanent bracing is always required to stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the relationship and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safey Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 NCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE and is for an individual building component, not do properly incorporate this design into the overall sons. Additional some IN READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 WARNING - Verify design paramete sign valid for use only with MiTek® russ system. Before use, the buildin



T24043997		C 0 4000 0000 0000 00000
SIMQUE - LOT 54 PLL	Job Reference (optional)	Company of the Compan
Ply	7	
aty	-	
Truss Type	HIP GIRDER	
Truss	TOT	
Job	2802399	

Builders FirstSource (Jacksonville, FL),

8,430 s Apr 20 2021 MTek Industries, Inc. Wed May 19 15:54:59 2021 Page 2 ID:RGwSt4CPREnm5p9yfgzNgAyg3dx-7V/9E5QCx9mOIP1OHeyfel1pM?w9ZO7zvtcECEzEuHQ

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 138 lb up at 7-0-0, 110 lb down and 138 lb up at 17-0-12, 110 lb down and 138 lb up at 17-0-12, 110 lb down and 138 lb up at 17-0-12, 110 lb down and 138 lb up at 13-0-12, 110 lb down and 138 lb up at 13-3-4, 110 lb down and 138 lb up at 23-3-4, 110 lb down and 138 lb up at 23-3-4, 110 lb down and 138 lb up at 27-3-4, 99 lb down and 118 lb up at 25-3-4, 110 lb down and 118 lb up at 35-4.0 on top chord, and 332 lb down and 389 lb up at 70-0, 86 lb down and 88 lb up at 13-0-12, 86 lb down and 88 lb up at 13-0-12, 86 lb down and 88 lb up at 13-0-12, 86 lb down and 88 lb up at 13-0-12, 86 lb down and 88 lb up at 13-0-12, 86 lb down and 88 lb up at 13-3-4, and 89 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 88 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 80 lb up at 13-3-4, 80 lb down and 8

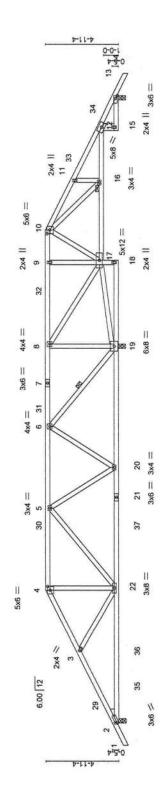
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert. 1-54, 3-10=54, 10-11=-54, 11-13=-54, 18-24=-20, 11-17=-20, 14-27=-20
Concentrated Loads (lb)
Vert. 3=-110(F) 6=-110(F) 10=-99(F) 18=-63(F) 8=-110(F) 23=-332(F) 21=-63(F) 35=-99(F) 35=-99

May Waranno - Verify design parameters and READ NOTES ON 1HIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rav. 5119:2020 BEFORE USE.
Design what ide rous eonly with Mithe& connections is based only upon parameters abow, and its for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent obtaining of individual usax web androt chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obtaining of individual usax web androt do the unity and properly damage. For general guidance regarding the labitation of the component contraction, storage, delivery, erection and bracing of fusses and truss systems, see Anstitret to using Component Anstitret our available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 2060 Labitation and 203 Waldorf, MD 2060 Labitation and 203 May 2010 Labitation and 203 Waldorf, MD 2060 Labitation and 203 May 2010 Labitation and



| Job Reference (optional)
| Job Reference (optional)
| B 430 s Apr 20 2021 MiTek Industries, Inc. Wed May 19 15:55:01 2021 Page 1
| D:RGwSl4cPREnm5p9ygzNBAycgdx-3umvinSTSnG6YjBn03 7jj6BGpXf1HJGMBSLG7ZEJHO
| 25-512 | 33-40 | 33-44 | 42-40 | 410-04
| 5-5-12 | 5-9-12 | 2-0-8 | 34-4 | 3-44 | 1-6-0 SIMQUE - LOT 54 PLL Ply Qty Truss Type Jacksonville, FL - 32244, 로 9-0-0 Builders FirstSource (Jacksonville, FL), T02 2802399 Job

Scale = 1:75.1



-		0-0-6	17-3-11		25-5-12	31-3-8	89	36-8-4	, 40-0-8	42-4-0	
L		0-0-6	8-3-11		8-2-1	5-9-12	2	5-4-12		2-3-8	
Plate Offsets	(X,Y)-	Plate Offsets (X,Y)- [2:0-0-14,0-1-8], [4:0-3-0,0-2-0], [10:0-3-0,0-2-0]	0-2-0], [10:0-3-0	,0-2-0]							
LOADING (psf)	Sf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defi	3	PLATES	GRIP	
TCLL 20	20.0	Plate Grip DOL 1.25	1.25	TC 0.65	Vert(LL)	0.37 22-25	>818	240	MT20	244/190	
TCDL 7	0.7	Lumber DOL	1.25	BC 0.74	Vert(CT)	0.33 22-25	>925	180			
	. 0.0	Rep Stress Incr	YES		Horz(CT)	0.07 13	n/a	n/a			
BCDL 10	10.01	Code FBC2020/TI	PI2014	Matrix-MS					Weight: 233 lb	FT = 20%	
LUMBER-					BRACING-						
TOP CHORD	2x4 SP	. No.2			TOP CHORD		al wood sh	neathing directly	Structural wood sheathing directly applied or 5-2-1 oc purlins.	oc purlins.	
BOT CHORD	2x4 SP	BOT CHORD 2x4 SP No.2 "Except"			BOT CHORD		iling direct	Rigid ceiling directly applied or 3-3-5 oc bracing.	3-5 oc bracing.		
	9-18: 2	9-18: 2x4 SP No.3, 12-15: 2x6 SP No.2	P No.2		WEBS	1 Row a	Row at midpt	6-19			
WEBS	2x4 SP No.3				JOINTS	1 Brace	Brace at Jt(s): 16	9			
WEDGE											
Left: 2x4 SP No.3	No.3										

REACTIONS.

(size) 2=0-3-8, 13=0-3-8, 19=0-3-8 Max Horz 2=113(LC 16) Max Upliff 2=-568(LC 9), 13=-194(LC 13), 19=-1166(LC 8) Max Grav 2=829(LC 23), 13=304(LC 24), 19=2264(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1233/1853, 3-4=-976/1697, 4-5=-829/1568, 5-6=-396/1211, 6-8=-596/1482, 8-9=-306/594, 9-10=-319/008

BOT CHORD 2-22=-1579/1060, 20-22=-1161/669, 19-20=-587/406, 16-17=-323/384, 12-16=-25/262

WEBS 3-22=-28/21317, 4-22=-655/204, 5-22=-223/347, 5-20=-640/401, 6-20=-1049/823, 6-19=-1498/1601, 8-19=-875/477, 17-19=-1410/712, 8-17=-429/1030, 10-17=-554/295, 10-16=-315/572, 11-16=-265/236

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(12E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 33-4-0, Exterior(2R) 33-4-0 to 37-6-15, Interior(1) 37-6-15 to 43-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1, 50 plate grip DOL=1,60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- b. This truss has been designed for a 10.0 pet bottom chord live load nonconcurrent with any other live loads.
 f) This truss has been designed for a 10.0 pet bottom chord live load nonconcurrent with any other live loads.
 f) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 g) * This truss has been designed for a live load of 20.0psf on the bottom chord and any other members.
 - Will itt between ure barton under and end verse, morrows.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=568, 13=194, 19=1166.

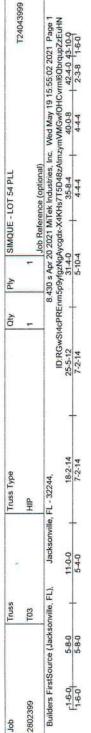
No 68182

STATE OF E Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

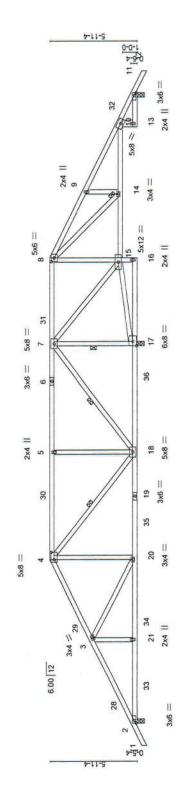
May 22,2021

ign parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19:2020 BEFORE USE. With MTREGK connectors. This design is based only upon parameters shown, and is for an individuab building component, not set, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall indicated is to prevent bucking of individual into: web and/or chord members only. Additional temporary and permanent braining billy and to prevent bucking of individual into: web and/or chord members only. Additional temporary and permanent braining billy and to prevent collapse with possible personal injury and properly diamage. For general guidance regarding the ery, erection and braining of trusses and truss systems; see





= 1:75.1



	5-8-0	11-0-0	1	18-2-14		25-5-12	31-3-8	8	35-8-4	40-0-8	42-4-0	
L	5-8-0	5-4-0	-	7-2-14		7-2-14	5-9-1	2	4-4-12	444	2-3-8	
Plate Offsets (x,Y)- [4:0-	Plate Offsets (X,Y)- [4:0-6-0,0-2-8], [8:0-3-0,0-2-4], [17:0-3-8,0-3-0]	1-2-4], [17:0-	3-8,0-3-0]								
LOADING (psf)	0	SPACING-	2-0-0	CSI			in (loc)	l/defl	P/I	PLATES	GRIP	
TCLL 20.	0	Plate Grip DOL	1.25	70	0.57		0.19 18-20	666<	240	MT20	244/190	
TCDL 7.	0	Lumber DOL	1.25	BC	BC 0.62	Vert(CT)	-0.19 10-14	>999	180			
BCLL 0.	. 0	Rep Stress Incr YE	YES	WB	0.87		0.07	n/a	n/a			
BCDL 10.0	0	Code FBC2020/T	PI2014	Mat	rix-MS					Weight: 239 lb	FT = 20%	
LUMBER-						BRACING-						
TOP CHORD	2x4 SP No.2	2				TOP CHORD		al wood s	sheathing direct	Structural wood sheathing directly applied or 5-0-7 oc purlins.	c purlins.	
BOT CHORD		2x4 SP No.2 *Except* 8-16: 2x4 SP No.3 10-13: 2x6 SP No.2	No 2			MERS		eiling dire	ctly applied or 4	Rigid ceiling directly applied or 4-3-13 oc bracing. Row at midot		
MEDO	DAY CD No 3	3				STAICI	4 Drace	Dence of 11/01: 44				

1 11

2=0-3-8, 11=0-3-8, 17=0-3-8 (size) REACTIONS.

Max Horz 2=-136(LC 17) Max Uplift 2=-571(LC 9), 11=-174(LC 13), 17=-1078(LC 8) Max Grav 2=864(LC 25), 11=276(LC 24), 17=2518(LC 2)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. TOP CHORD 2-3=-1323/2005, 3-4=-872/1479, 4-5=-357/1016, 5-7=-357/1016, 7-8=-176/603, 8-9=-49/282

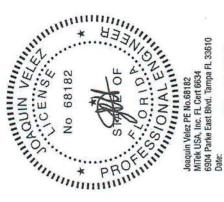
2-21=-1683/1139, 20-21=-1683/1139, 18-20=-1111/741, 17-18=-1374/667, 8-15=-740/364, 14-15=-599/431 BOT CHORD WEBS

3-21=-360/214, 3-20=-475/672, 4-20=-755/481, 4-18=-739/487, 5-18=426/326, 7-18=-1728/1723, 7-17=-1990/1460, 15-17=-1357/610, 7-15=-308/996, 8-14=-390/690,

9-14=-308/286

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(ZE) -1-6-0 to 1-6-0, interior(1) 1-6-0 to 11-0-0. Exterior(ZR) 11-0-0 to 15-2-15, Interior(1) 15-2-15 to 31-2-6. Exterior(2R) 31-2-6 to 35-8-4, Interior(1) 38-4 to 43-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown. Lumber DOL=1, 60 plate grip DOL=1,60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)
 - 2=571, 11=174, 17=1078



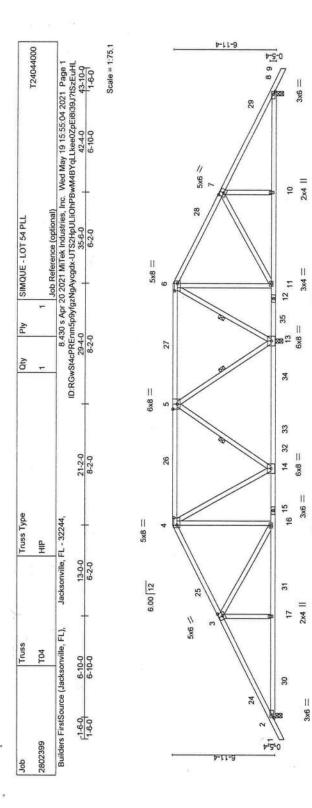
May 22,2021

icing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing activities as the stability and properly damage. For general guidance regarding the delivery, erection and bracing of trusses and truss systems, see

ANSITPI Quality Criteria, DSB-89 and BCSI Building Coavailable from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL.7473 rev. ectors. This design is based only upon parameter igner must verify the applicability of design parar and buckling of individual truss web and/or chord



6904 Parke East Blvd. Tampa, FL 36610



	6-10-0	6-2-0	3-10-4	8-7-8	3.10-4	6-2-0	6-10-0)
late Offsets (X,Y)- [3:0-3-0,0-3	[3:0-3-0,0-3-0], [4:0-6-0,0	1-2-8], [5:0-4-0,E	dge], [6:0-6-0,0-	3-0], [4:0-6-0,0-2-8], [5:0-4-0,Edge], [6:0-6-0,0-2-8], [7:0-3-0,0-3-0]				
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.			PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25					MT20	244/190
	Lumber DOL	1.25	BC 0.71		-0.33 13-14 >	>940 180		
· 0.0 ·	Rep Stress Incr	YES		4 Horz(CT)	0.03 13	n/a n/a		
	Code FBC2020/T	PI2014	Matrix-MS				Weight: 230 lb	FT = 20%
UMBER- OP CHORD 2x4 S OT CHORD 2x4 S WEBS 2x4 S	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD WEBS		wood sheathing ig directly applie	Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 4-5-5 oc bracing.	oc purlins.

3x6 ==

3x4 ==

REACTIONS.

(size) 2=0-3-8, 13=0-3-8, 8=0-3-8 Max Horz 2=-156(LC 13) Max Uplift 2=-496(LC 9), 13=-1018(LC 9), 8=-288(LC 13) Max Grav 2=912(LC 25), 13=2197(LC 2), 8=512(LC 20)

-- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1383/1775, 3-4=-799/1094, 4-5=-457/807, 5-6=-527/731, 6-7=-420/417, 7-8=-541/317 (P) FORCES.

TOP CHORD

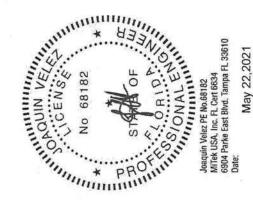
2-17=-14681186, 16-17=-1472/1188, 14-16=-733/681, 11-13=-280/621, 10-11=-157/456, 8-10=-158/454 8-10=-158/454 3-17=-488/290, 3-16=-611/870, 4-16=-690/410, 4-14=-466/479, 5-14=-1045/793, 5-13=-1286/1396, 6-13=-945/453, 6-11=-153/444, 7-11=-644/378, 7-10=0/303 **BOT CHORD**

NOTES.

WEBS

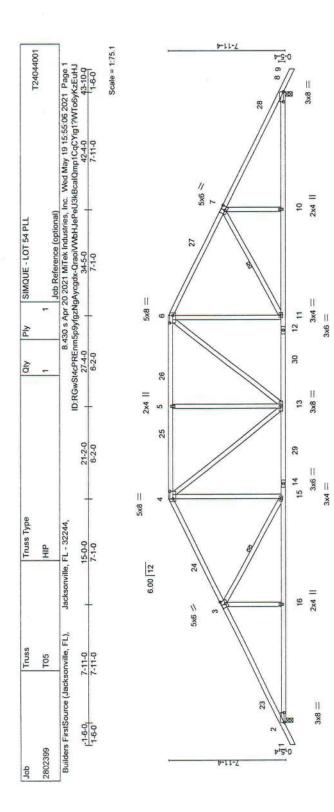
- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16, Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(ZE) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 13-0-0, Exterior(2R) 13-0-0 to 17-2-15, Interior(1) 17-2-15, Interior(1) 17-2-15, Interior(1) 17-2-15, Interior(1) 17-2-15, Interior(1) 17-2-16 to 29-4-0, Exterior(2R) 29-4-0 to 33-6-15, Interior(1) 33-6-15 to 43-10-0 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (#=lb)



use, the building designer mast verify the applicability of design parameters and properly incorporate his design into the overall strateging designer must verify the applicability of the control of th 13. This design is based only upon parameters shown, and is for an individual or must verify the applicability of design managers. and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE





-	7-11-0	15-0-0	21-2-0	, 27-4-0	9	34-5-0	9.0	42-4-0	
	7-11-0	7-1-0	6-2-0	6-2-0	. 0	7-1	0-	7-11-0	
Plate Offsets (X,Y)-	[2:0-8-0,0-0-1], [3:0-3-0,	,0-3-4], [4:0-6-0,0-	1], [3:0-3-0,0-3-4], [4:0-6-0,0-2-8], [6:0-6-0,0-2-8], [7:0-3-0,0-3-4], [8:0-8-0,0-0-1]	0-3-0,0-3-4], [8:0-8-0	0,0-0-1]				
LOADING (psf)	SPACING-		CSI.		in (loc)	l/defl	P	PLATES	GRIP
TCLL 20.0	Plate Grip DOL				0.26 11-13	666<	240	MT20	244/190
TCDL 7.0	Lumber DOL		BC 0.89	Vert(CT)	-0.43 11-13	666<	180		
BCLL 0.0 *	Rep Stress Incr	YES			0.17 8	n/a	n/a		
BCDL 10.0	Code FBC2020/TPI2014	TPI2014	Matrix-MS					Weight: 232 lb	FT = 20%
LUMBER-				BRACING-					
TOP CHORD 2x4 S	2x4 SP No.2			TOP CHORD		ral wood shi	eathing dire	Structural wood sheathing directly applied.	
	2x4 SP No.3			WEBS		Row at midpt	y applied of	3-15, 7-11	

(size) 2=0-3-8 Max Horz 2=178(LC 12) Max Uplift 2=-672(LC 12), 8=-672(LC 13) Max Grav 2=1783(LC 2), 8=1783(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-3195/1116, 3-4=-2563/918, 4-5=-2439/948, 5-6=-2439/948, 6-7=-2563/918,

7-8=-3195/1117

BOT CHORD

2-16=-1034/2794, 15-16=-1034/2794, 13-15=-662/2232, 11-13=-548/2232,

10-11=-856/2794, 8-10=-857/2794

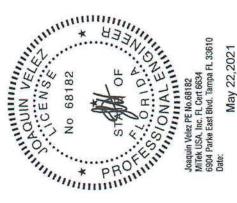
WEBS

3-16=0/304, 3-15=-673/431, 4-15=-170/599, 4-13=-234/450, 5-13=-373/291,

6-13=-234/450, 6-11=-170/599, 7-11=-673/431, 7-10=0/304

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWRSX (envelope) gable end zone and C-C Exterior(2E)-1-6-0 to 1-6-0, interior(1) 1-6-0 to 15-0-0. Exterior(2R) 17-0-0 to 19-2-15, Interior(1) 19-2-15 to 27-4-0, Exterior(2R) 27-4-0 to 31-6-15 to 43-10-0 zone; C-C for members and forces & MWRRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

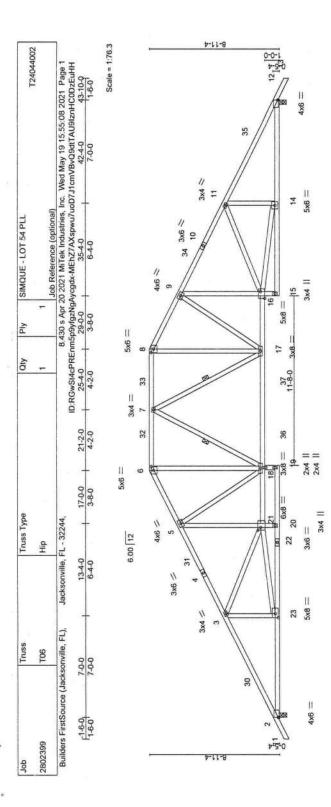
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb)
 - 2=672, 8=672



MANABURIO - Varify design parameters and READ NOTES ON THIS AND INCLUDED MILLER REPERTURE FINDER INVESTIGATION OF COMPONENT, and Exponent, not Design valid for use only with MTREGES connectors. This design is based only upon parameters starts and properly incorporate this design into the overall at truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify that applicability of design parameters and properly admission inforced to permanent bracing is always required for stability and to prevent colapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and facing of trusses and truss systems, see

ANSITPIT Quality Criteria, DSB-89 and BCSI Building CS
Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	7-0-0	13-4-0	, 17-0-0 17-4-0	25-4-0	29-0-0	. 0	35-4-0	42-4-0	
	7-0-0	6-4-0	3-8-0 0-4-0	8-0-0	3-8-0		64-0	2-0-0	
Plate Offsets (X,Y)- [2:0-0-0,0-0-1	[2:0-0-0,0-0-11], [6:0	13-0,0-2-0], [8:0-3-0	11], [6:0-3-0,0-2-0], [8:0-3-0,0-2-0], [12:0-0-0,0-0-11], [16:0-5-12,Edge], [21:0-2-4,0-2-4], [23:0-3-8,0-2-8]], [16:0-5-12,Edge],	21:0-2-4,0	-2-4], [23:C)-3-8,0-2-8]		
OADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	ol) ni	(loc) (/defi	3	PLATES	GRIP
CLL 20.0	Plate Grip DOL		TC 0.48	Vert(LL)	-0.52	19 >971	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.92	Vert(CT)	-1.02	19 >499	180		
CLL 0.0 *	Rep Stress Incr	or YES	WB 0.83	Horz(CT)	0.21	12 n/a	n/a		
ICDL 10.0	Code FBC20	FBC2020/TPI2014	Matrix-MS					Weight: 277 lb	FT = 20%
UMBER-				BRACING-					
TOP CHORD 2x4 S	2x4 SP No.2			TOP CHORD	550	ctural wood	1 sheathing direct	Structural wood sheathing directly applied or 2-9-15 oc purlins.	oc purlins.
3OT CHORD 2x4 S	2x4 SP No.2 *Except* 5-20,9-15; 2x4 SP No.3, 16	ot* 40.3, 16-21; 2x4 SP M 31		BOT CHORD		Rigid ceiling directly app 0-0-0 oc bracing: 20-21	rectly applied or ng: 20-21	Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 10-0-0 oc bracing: 20-21	cept:
MEBS 2x4 S	2x4 SP No.3 *Except* 21-23,14-16: 2x4 SP No.2			WEBS	1 R	Row at midpt		7-18, 7-17	

WEBS

8-9=-2748(995, 9-11=-3227/1101, 11-12=-3305/1100 2-23=-1038/2963, 20-21=0/314, 5-21=-183/756, 18-21=-854/2982, 17-18=-602/2514, 16-17=-750/2836, 9-16=-197/577, 12-14=-852/2895 3-23=-328/192, 21-23=-965/2928, 5-18=-902/422, 6-18=-323/1090, 7-18=-263/217, 7-17=-330/196, 8-17=-329/1064, 9-17=-741/434, 14-16=-827/2698, 11-14=-285/178

Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3380/1086, 3-5=-3391/1068, 5-6=-2809/981, 6-7=-2490/912, 7-8=-2435/925,

(Q)

TOP CHORD BOT CHORD

(size) 2=0-3-8, 12=0-3-8 Max Horz 2=199(LC 12) Max Uplift 2=-652(LC 12), 12=-659(LC 13) Max Grav 2=1859(LC 2), 12=1818(LC 2)

REACTIONS.

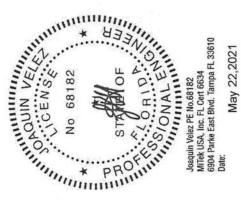
- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; WWFRS (envelope) gable end zone and CCC Exterior(2E) -1-6-0 to 2-8-13, Interior(1) 2-8-13 to 17-0-0, Exterior(2R) 17-0-0 to 22-11-13, Interior(1) 22-11-13 to 25-4-0, Exterior(2R) 22-4-0 to 31-3-13, Interior(1) 31-3-13 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3

- Provide adequate drainage to prevent water ponding.

 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 600
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=652, 12=659

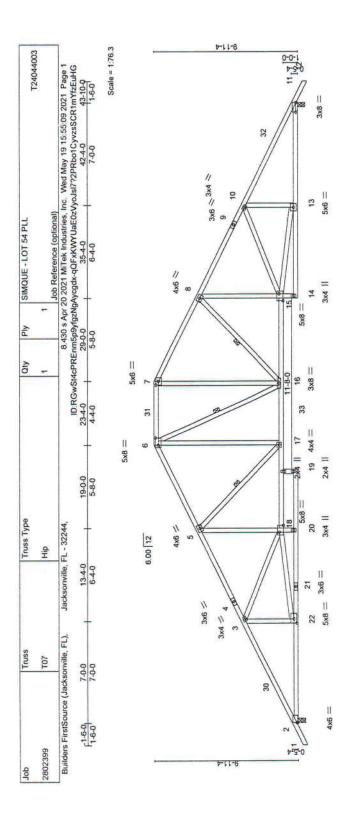


leally design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL1473 rev. 5/19/2020 BEFORE USE.

Be condy with Mirke's connectors. This design is based only upon parameters as shown, and its ora individual building component, not all absorber use, the building designer must verify the applicability of design parameters and properly incoporate this design into the overall Bracing indicated is to prevent obtaining of individual this web and/or chord members only. Additional temporary and permanent bracing for standing the prevent busing of individual parameters and proper to a prevent or proper and the property of individual transports and permanent bracing age, delivery, erection and bracing of frustses and furuss, systems, see

AMSATP1 Quality Criteria, DSB-99 and BCSI Building Co available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601. is always req fabrication, sl Safety Inform





		7-0-0		13-4-0	8 ·	17-4-0	,19-0-0,	23-4-0	29-	29-0-0	-	35-4-0	50	42-4-0	-
L		7-0-0		6-4-0	-	4-0-0	1-8-0	4-4-0	5-8	5-8-0	-	6-4-0	-	2-0-0	Γ
Plate Offsets (-(Y,X	Plate Offsets (X,Y)- [2:0-0-0,0-0-11], [6:0-6-0,0-2-8], [7:0-3-0,0-2-0], [11:0-8-0,0-0-1], [15:0-5-12,0-3-4], [18:0-5-12,Edge], [22:0-3-8,0-2-8]	[6:0-6-0	1,0-2-8], [7:0-3-0	0,0-2-0	1), [11:0-8	3-0,0-0-1].	[15:0-5-12,0-3	-4], [18:0-	5-12,Edg	e], [22:0	3-8,0-2-8			
LOADING (ps)	ç	SPACING		2-0-0		CSI.		DEFL		in (loc)	l/def	P/I		PLATES	GRIP
TCLL 20.0	0	Plate Grip DOL	DOL	1.25			0.49	Vert(L)	-) -0.53	3 19	>950	240		MT20	244/190
TCDL 7.	0	Lumber D	OL	1.25			76.0	Vert(CT)	T) -1.04	1 19	>487	180			
	. 0.0	Rep Stres.	s Incr	YES		WB	0.67	Horz(C	T) 0.23	3 11	n/a	n/a			
BCDL 10.0	0.	Code FBC2020/TPI2014	C2020/1	PI2014		Matrix-MS	MS							Weight: 273 lb FT = 20%	FT = 20%
LUMBER-								BRACING-	-97						
TOP CHORD	2x4 S	2x4 SP No.2						TOP CHORD	10RD	Structu	iral wool	d sheathing (directly a	Structural wood sheathing directly applied or 2-10-4 oc purlins.	oc purlins.
BOT CHORD		2x4 SP No.2 *Except*						BOT CHORD	HORD	Rigid c	eiling di	Rigid ceiling directly applied	1 or 5-8-8	Rigid ceiling directly applied or 5-8-8 oc bracing. Except:	cept:
WEBS	2x4 S	P No 3 *Excent*						WEBS		1 Row	Row at midpt	2	5-17.6-	5-17, 6-16, 8-16	
VVEDG	CATA	TOO LONGE						1		-	danill in			0 0	

2=0-3-8, 11=0-3-8 2x4 SP No.3 *Except* 18-22,13-15: 2x4 SP No.2 (size) REACTIONS. WEBS

Max Horz 2=221(LC 16) Max Upliff 2=-647(LC 12), 11=-654(LC 13) Max Grav 2=1848(LC 2), 11=1806(LC 2)

(b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
2-3=-3355/1074, 3-5=-3381/1060, 5-6=-2528/905, 6-7=-2190/870, 7-8=-2499/911,
8-10=-3213/1081, 10-11=-3277/1088
D 2-22=-1048/10240, 18-20=0/313, 5-18=-171/851, 17-18=-881/2985, 16-17=-531/2217, 15-18=-143/2835, 9-15=-182/670, 11-13=-840/2870
3-22=-334/192, 18-22=-968/2987, 5-17=-1087/491, 6-17=-310/956, 7-16=-261/885, 8-16=-915/501, 13-15=-807/2751, 10-13=-291/171 TOP CHORD BOT CHORD FORCES WEBS

NOTES-

- Unchainment of live loads have been considered for this design.

 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(ZE) -1-6-0 to 2-8-13, Interior(1) 2-8-13 to 19-0.0, Exterior(ZE) 19-0.0 to 23-4-0, Exterior(ZR) 23-4-0 to 29-1-12, Interior(1) 29-1-12 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

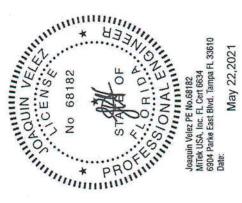
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 4) Provide adequate drainage to prevent water ponding.

 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

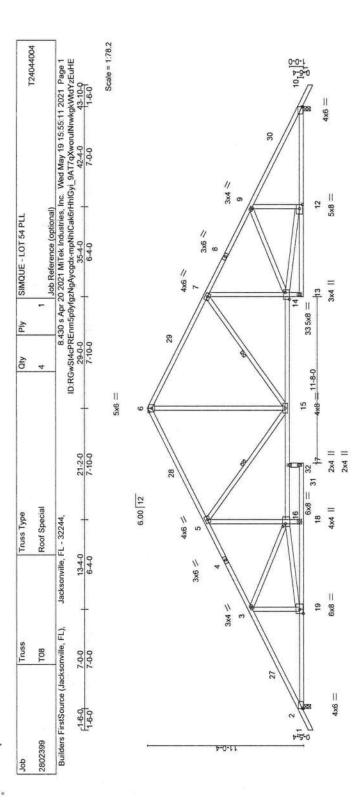
 5) This truss has been designed for a 10.0 psf bottom chord in the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2-647, 11=654.



M. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design void for use only with Mitthey connectors. This design is based only upon parameters show, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent Outside of individual truss whe andro closed members only. Additional temporary and permanent bracing is always required for stability and to prevent Colleges with possible personnel injury and properly damage. For general guidance regarding the individual trusses and truss systems; see
ANSITPH Coulify Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601.





			GRIP	244/190			FT = 20%
42-4-0	1 2-0-0		PLATES	MT20			Weight: 254 lb
35-4-0	6-4-0		2	240	180	n/a	
		-0]	l/defi	>847	>466	n/a	
		2-4], [19:0-3-8,0-3-0	(loc)	17	17	10	
29-0-0	10-0	4], [19:0	.⊆	-0.60	-1.09	0.22	
25	7-	[16:0-2-4,0-2-	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
21-2-0	10-0	2,Edge],					_
-	-	14:0-5-1		0.52	3C 0.98	0.83	-MS
17-4-0	4-0-0	0-2-8], [CSI.	2	BC	WB	Matrix
		2:0-3-8,					-
13-4-0	6-4-0	-0,0-0-11], [1	2-0-0	1.25	1.25	YES	rP12014
1-0-0	7-0-0	[2:0-0-0,0-0-11], [10:0-0	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/1
		ts (X,Y)-	(bsd)	20.0	7.0	. 0.0	10.0
		Plate Offse	LOADING	TCLL	TCDL	BCLL	BCDL

2x4 SP M 31 "Except" 1-4,8-11: 2x4 SP No.2 2x4 SP No.2 "Except" 2x4 SP No.3 "Except" 2x4 SP No.3 "Except" 16-19,12-14: 2x4 SP No.2 TOP CHORD вот сноя LUMBER WEBS

Structural wood sheathing directly applied or 2-9-3 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 10-0-0 oc bracing: 16-18 5-15, 7-15 BRACING-TOP CHORD BOT CHORD WEBS

(size) 2=0-3-8, 10=0-3-8 Max Horz 2=-244(LC 17) Max Uplift 2=-641(LC 12), 10=-648(LC 13) Max Grav 2=1898(LC 2), 10=1844(LC 2)

REACTIONS.

FORCES. (Ib

(Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.
2-3=3457/1058, 3-5=3529/1056, 5-6=2391/848, 6-7=2392/846, 7-9=-3322/1064, 9-10=-3356/1073
D-19=0-1055/3030, 16-18=0/372, 5-16=-158/951, 15-16=-915/3130, 14-15=-733/2946, 7-14=-167/725, 10-12=-825/2939
3-19=-331/188, 16-19=-958/3133, 5-15=-1312/588, 6-15=45/1/1735, 7-15=-1084/597, **BOT CHORD**

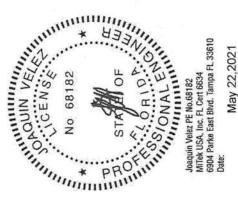
12-14=-776/2840, 9-12=-288/164

WEBS

NOTES-

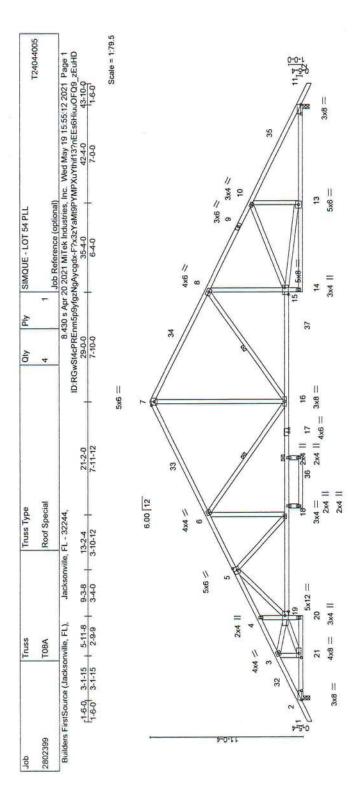
- Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-8-13, Interior(1) 2-8-13 to 21-2-0, Exterior(2R) 21-2-0 to 25-4-13, Interior(1) 25-4-13 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber 53
- DOL=1.60 plate grip DOL=1.60 Building Doser responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component. 3
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 ib uplift at joint(s) except (fi=lb) 2-641, 10-648.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. \$19,02020 BEFORE USE. Design void for use only with MITER& Commendate This design is based only upon parameters shown, and it for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Enabling facility and properly and properly and premarter the action is always equived for abbility and to prevent obtains with a page of individual truss what and/or chord members only. Additional temporary and permanent backing is always equived for abbility and to prevent obtains with a page of the seconal inquisity and properly damage. For general guidance regarding the abbridge delivery, erection and tracing of trusses and truss systems, see
ANSI/TPH Quality Criteria, DSB-99 and BCSI Building Co. Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			<u>a</u>	244/190			FT = 20%
42.4-0	1 2-0-0			MT20 244			Weight: 258 lb F
35-4-0	6-4-0		2	240	180	n/a	
35			Vdef	>999	>802	n/a	
		3,0-2-0]	(loc)	15-16	0.63 15-16	11	
202	10-0	21:0-3-8	⊒.	-0.36	-0.63	0.24	
- 28	7.	0-1], [15:0-5-12,0-3-4], [19:0-3-8,0-3-0], [21:0-3-8,0-2-0]	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
21-2-0	3-10-0	2,0-3-4], [19		15	96	98	"
7-4-0	8-8-8	15:0-5-1	SI.	C 0.5	BC 0.96	WB 0.8	Aatrix-M8
13-7-8 1	0-5-4 3	-0,0-0-1],	0	_	Ш	>	2
13-2-4	7-2-12	-3-0], [11:0-8-	2-0-0	1.25	1.25	YES	PI2014
		0-8-0,0-0-1], [5:0-3-0,0	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TI
3-1-15	3-1-15	x,Y)- [2:0	9	0	0	. 0	0
		Offsets (.	DING (ps	20.	7.	0	10.
		Plate	LOAD	TCLL	TCDL	BCLL	BCDL

LUMBER-		BRACING-
TOP CHORD	2x4 SP No.2 *Except*	TOP CHORD
	5-7: 2x4 SP M 31	BOT CHORD
BOT CHORD	2x4 SP No.2 "Except"	
	17-19,15-17: 2x4 SP M 31, 8-14,22-23: 2x4 SP No.3	WEBS
WEBS	2x4 SP No.3 *Except*	
	19-21,13-15: 2x4 SP No.2	

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 2-2-0 oc bracing. Except: 8-0-0 oc bracing: 16-18

REACTIONS.

(size) 2=0-3-8, 11=0-3-8 Max Horz 2=244(LC 12) Max Uplift 2=-657(LC 12), 11=-657(LC 13) Max Grav 2=1778(LC 2), 11=1778(LC 2)

(Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.
2-3=3219/1104, 3-4=4637/1679, 4-5=4724/1792, 5-6=3157/1119, 6-7=-2224/869,
7-8=-2226/868, 8-10=-3152/1086, 10-11=-3215/1092
2-21=-1417/2827, 18-19=-1218/3290, 16-18=-953/2806, 15-16=-754/2796, 8-15=-169/723,
11-13=842/2814
3-21=-1042/465, 19-21=-1109/2805, 3-19=477/1352, 5-18=-659/361, 6-18=-200/769,
6-16=-1096/611, 7-16=-474/1583, 8-16=-1082/598, 13-15=-787/2724, 10-13=-278/166,
5-19=-602/1368 TOP CHORD FORCES.

BOT CHORD WEBS

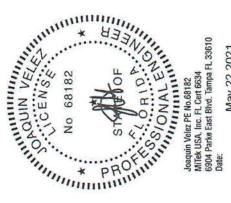
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-8-13; Interior(1) 2-8-13 to 21-2-0, Exterior(2R) 21-2-0 to 2-4-13. Interior(1) 25-4-13 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber 2014 and 2014 a DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=657, 11=657.



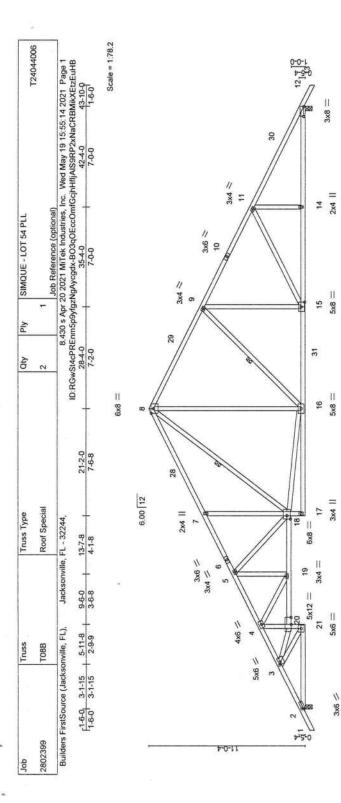
May 22,2021

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a tubulding designer must verify the applicability of design parameters and properly incorporate into the overall building design remarker in the sey that in other overall building design. Bracing indicated is to prevent bucking of individual furse web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ordingse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/FIT Quality Criteria, DSB-89 and BCSI Building Co.

Zafety information available from Truss Plate institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 rev. 5/19/2020 BEFORE USE. 14 READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473





	5-11-8 , 9-6-0	13-7-8	21-2-0	284-0		35	35-4-0	42-4-0		
		-	7-6-8	7-2-0		7-	0-0	1-0-0	Γ	
Plate Offsets (X,Y)- [2:0-0-14,0-	[2:0-0-14,0-1-8], [3:0-2-0	1,0-1-8], [12:0-8-1	J-1-8], [3:0-2-0,0-1-8], [12:0-8-0,0-0-1], [15:0-4-0,0-3-0], [18:0-2-12,0-3-0]	[18:0-2-12,0-3-0]						
LOADING (psf)	SPACING-	2-0-0	CSI.		in (loc)	l/defi	3	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.56	Vert(LL) -0.	-0.33 16-17	666<	240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25	BC 0.89		.61 16-17	>832	180			
BCLL 0.0 *	Rep Stress Incr	YES			.26 12	n/a	n/a			
BCDL 10.0	Code FBC2020/	rPI2014	Matrix-MS					Weight: 265 lb	FT = 20%	
LUMBER-				BRACING-						
TOP CHORD 2x4 SP M 31 "Except"	3P M 31 *Except*			TOP CHORD	Structur	al wood sh	eathing direct	Structural wood sheathing directly applied or 2-5-10 oc purlins.	oc purlins.	
1-6,1	0-13: 2x4 SP No.2			BOT CHORD	Rigid ce	illing directl	y applied or	Rigid ceiling directly applied or 5-5-12 oc bracing.		
BOT CHORD 2x4 S	2x4 SP No.2 "Except"			WEBS	1 Row a	Row at midpt	8-1	8-18, 9-16		

1 11

		1-6,10-13: 2x4 SP No.2	2x4 SP No.2 "Except"	4-21,18-20: 2x4 SP M 31, 7-17: 2x4 SP No.3	2x4 SP No.3 "Except"	3-20: 2x4 SP No.2	(size) 2=0-3-8, 12=0-3-8	Max Horz 2=244(LC 12)	Max Uplift 2=-657(LC 12), 12=-657(LC 13)
LOWIDEN-	TOP CHORD		BOT CHORD		WEBS		REACTIONS.		

Max Grav 2=1757(LC 2), 12=1770(LC 2)

FORCES. (Ib)

(b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
2.3=3186/1141, 3-4=-4354/1602, 4-5=-3655/1318, 5-7=-2976/1087, 7-8=-3015/1283, 8-9=-1999/840, 9-11=-2640/956, 11-12=-3202/1098
2.21=-1162/2782, 20-21=-552/1404, 4-20=-202/610, 19-20=-1611/4066, 18-19=-1191/3255, 7-18=-375/373, 15-16=607/2304, 14-15=-850/2803, 12-14=-850/2803, 3-21=-2619/1130, 3-20=-1389/3459, 4-16=-948/480, 5-16=-806/386, 16-18=-394/1538, 8-18=-813/1510, 8-16=-291/716, 9-16=-840/506, 9-15=-124/550, 11-15=-572/363, 11-14=0/270, 5-19=-238/704 BOT CHORD WEBS

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 3-0-1, Interior(1) 3-0-1 to 21-2-0, Exterior(2R) 21-2-0 to 25-4-13, Interior(1) 25-4-13 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate
 - grip DOL=1.60

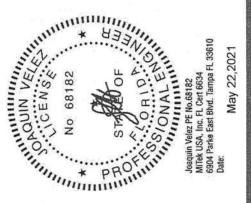
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb) 2=657, 12=657.

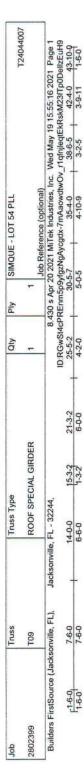
se and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE connectors. This design is based only upon parameters shown, and is for an individual building component, not glesigner must verify the applicability of design parameters and properly incorporate this design into the overall prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing sign valid for use only with MITek® uss system. Before use, the buildin

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design are to prevent building before the same and or chord members only. Additional transporary and appearanted bracing institutes to stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the stability and properly damage. For general guidance regarding the stability and possible personal results and truss systems, see MASITPHY quality Criteria, DSB-89 and BCSI Building CO. Safety Information available from Truss Palei Institute, 2570 Cain Highway, Suite 203 Waldod, MD 20001.

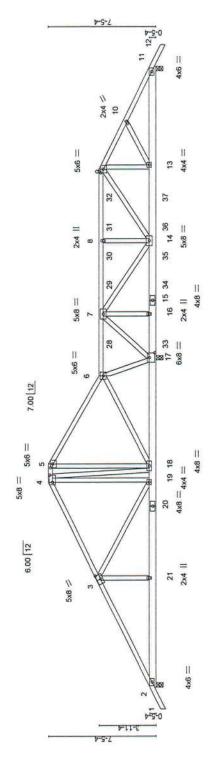




6904 Parke East Blvd. Tampa, FL 36610



Scale = 1:76.3



	7-6-0	14-0-0	15-3-2	22-7-8	25-5-2	The second second	30-5-7	Total Control	35-4-0	42-4	0
L	1-6-0	0-9-9	1-3-2	7-4-6	2-9-10		5-0-5		4-10-9	0-0-2	0
late Offsets (X,Y)-	[3:0-4-0,0-3-0], [4:0-6-0,0-	2-8], [5:0-3-0,0	1-12], [9:0-3-0,	0-2-0], [17:0-4-0,0-4	1-8]						
OADING (psf)	SPACING-	2-0-0	CSI.	DEF	i.	in (loc	1		200	PLATES	GRIP
CLL 20.0	Plate Grip DOL	1.25				12 13-1		3.5		MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.41	_	Vert(CT) -0.	0.12 13-14	4 >999	180			
CLL 0.0 •	Rep Stress Incr	ON		_	_	02					
ICDL 10.0	Code FBC2020/TF	712014	Matrix-MS	_						Weight: 276 lb	FT = 20%

1 11

2x4 SP No.2	2x6 SP No.2	2x4 SP No.3	
TOP CHORD			

REACTIONS.

Structural wood sheathing directly applied or 3-10-15 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

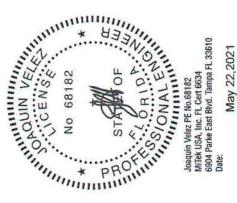
BRACING-TOP CHORD BOT CHORD

(size) 2=0-3-8, 17=0-3-8, 11=0-3-8 Max Horz 2=186(LC 7) Max Uplift 2=-419(LC 27), 17=-1816(LC 9), 11=-895(LC 9) Max Grav 2=724(LC 15), 17=2937(LC 1), 11=1245(LC 20)

(Q) **BOT CHORD** TOP CHORD FORCES. WEBS

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 - Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=419, 17=1816, 11=895.
- 23-3-4, 150 lb down and 138 lb up at 25-3-4, 150 lb down and 138 lb up at 27-3-4, 150 lb down and 138 lb up at 29-3-4, 150 lb down and 138 lb up at 31-3-4, and 150 lb down and 138 lb up at 31-3-4, and 150 lb down and 138 lb up at 35-3-4, 86 lb down and 88 lb up at 23-3-4, 86 lb down and 88 lb up at 27-3-4, 86 lb down and 88 lb up at 27-3-4, and 86 lb down and 88 lb up at 33-3-4, and 86 lb down and 88 lb up at 35-3-4. 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 150 lb down and 138 lb up at on bottom chord. The design/selection of such connection device(s) is the responsibility of others. In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)



LOAD CASE(S) Standard

6

Continued on page 2

or stability and to prevent collapse with possible personal injury and property damage. For general lemporary and permanent bracing delivery, erection and bracing of flusses and fluss systems, see

ANSI/IP1 Quality Criteria, DSB-89 and BCSI Building Collable from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 S and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL.7473 rav. 5/19/2020 BEFORE USE MiTek® connectors. This design is based only upon parameters the building designer many verify the applicability of design paramated is to prevent buckling of individual turss web and/or chord and to prevent collapse with possible personal injury and prope Design valid for use only with M a truss system. Before use, the building design. Bracing indical is always required for stability at fabrication, storage, delivery, et Safety Information available f WARNING - Verify design



6904 Parke East Blvd. Tampa, FL 36610

		T24044007		Job Reletine (opininal)
	SIMQUE - LOT 54 PLL		Manager of account of the	Job Releience (opinorial)
	Ply	7		000
10 mm	Qty	,	-	
	Truss Type	GOOD INICIAL STORY	NOOT STEVINE SINDEN	777000
				13
	Truss	100	60	
	Job	0000000	5002333	

Builders FirstSource (Jacksonville, FL),

8.430 s Apr 20 2021 MiTek Industries, Inc. Wed May 19 15;55:16 2021 Page 2 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-7mAaowdtwOv_11qfnjleqtEkRskM23fTp0DellzEuH9

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

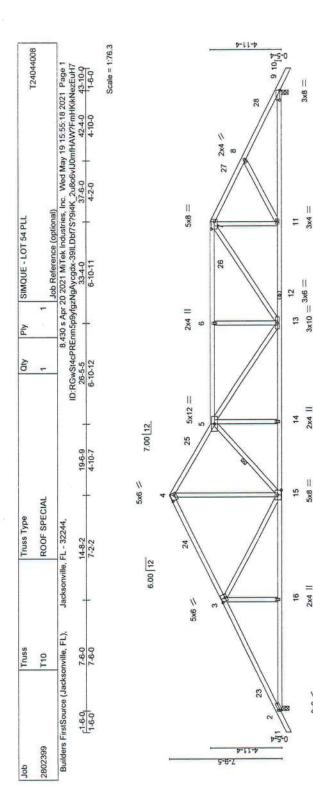
Vert: 1-4-54, 45-54, 5-6-54, 6-9-54, 9-12-54, 22-25-20

Concentrated Loads (lb)

Vert: 9--185(B) 7--110(B) 16--63(B) 13--332(B) 28--110(B) 30--110(B) 31--110(B) 32--110(B) 33--63(B) 34--63(B) 35--63(B) 36--63(B)

Vert: 9--185(B) 7--110(B) 16--63(B) 13--332(B) 28--110(B) 29--110(B) 31--110(B) 32--110(B) 33--63(B) 34--63(B) 35--63(B) 36--63(B)

MITEK 6904 Parke East Blvd. Tampa, FL 36610



	Section 1						
			GRIP	244/190			FT = 20%
42-4-0	0-0-6		PLATES	MT20			Weight; 224 lb
-					7.20		
33-4-0	111		2	240	180	n/a	
33	6-10		l/defl	>999	>752	n/a	
-	1	3-0]	(loc)	13-14	13-14	6	
wee		0-4-0'0-	. <u>u</u>	0.37	-0.68	0.19	
26-5-5	1 6-10-12	3-8-0,0-0-1], [15:	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
19-6-9	4-10-7	:0-6-0,0-2-8], [9:0		86.0	0.90	0.79	x-MS
1) -		0-3-0], [7:0	CSI.	TC	BC	WB	Matr
14-8-2	7-2-2	13-4], [4:0-4-2,	2-0-0	1.25	1.25	YES	12014
-		[3:0-3-0,0	ė	DOL	OC	ss Incr	C2020/TP
7-6-0	7-6-0	[2:0-0-14,0-1-8]	SPACING	Plate Grip DOL	Lumber	Rep Stre	Code FE
÷		ts (X,Y)-	(bst)	20.0	7.0	* 0.0	10.0
		Plate Offse	LOADING	TCLL 20.0	TCDL	BCLL	BCDL

3×10

2x4 II

5x8

2x4 II

3x6

LUMBER.		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 5-4-7 oc bracing.
WEBS 2x4 SP No.3	2x4 SP No.3	WEBS	1 Row at midpt 5-15
REACTIONS.	(size) 2=0-3-8, 9=0-3-8		

Max Horz 2=179(LC 11) Max Uplift 2=-592(LC 12), 9=-731(LC 13) Max Grav 2=1647(LC 1), 9=1647(LC 1)

(b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2938/1067, 3-4=-2361/961, 4-5=-2402/993, 5-6=-3405/1509, 6-7=-3405/1509, 7-8=-2743/1197, 8-9=-2969/1308 7-8=-2743/1197, 8-9=-2969/1308 10. 2-16=-889/2551, 15-16=-890/2548, 14-15=-1221/3557, 13-14=-1219/3560, 11-13=-863/2477, 9-11=-1059/2598 3-16=0/295, 3-15=-626/426, 4-15=-737/1876, 5-15=-2115/999, 5-13=-272/149, 6-13=-425/337, 7-13=-469/1192, 7-11=-55/362 FORCES. (Ib

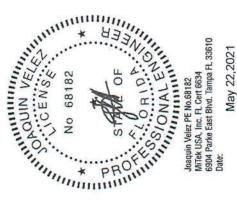
BOT CHORD

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.

 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf, h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 14-8-2, Exterior(2R) 14-8-2 to 17-8-2, Interior(1) 17-8-2 to 33-4-0, Exterior(2R) 33-4-0 to 36-4-0, Interior(1) 36-4-0 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component. 3
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|i=lb) 2=592, 9=731.



Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in 0the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for shallfy and to prevent collapse with possible personal lipiury and property damage. For general guidance regarding the labeling temporary and permanent bracing is always required for shallfy and to prevent collapse with possible personal lipiury and property damage. For general guidance regarding the labeling temporary solution, along the divery, erection and bracing of fuseses and truss systems, see a MASITPIT Quality Criteria, DSB-89 and BCSI Building Component Sareby Information available from Truss Plate institute. 2610 Crain Highway, Cuite in 203 Waldorf, MD 20601 MII-7473 rev. 5/19/2020 BEFORE USE and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE WARNING - Verify de



Job	Truss		Truss Type			Qty Ply		SIMQUE - LOT 54 PLL	PLL		
2802399	111		ROOF SPECIAL	AL.		554.6	-				T24044009
								Job Reference (optional)	ional)		
Builders FirstSource (Jacksonville,	cksonville, FL).	Jacksonville, FL - 32244	FL - 32244,			8.430	s Apr 20	2021 MiTek Indus	tries, Inc. Wed	May 19 15:55:19 2	021 Page 1
					ID:RGwS	ID:RGwSt4cPREnm5	Nzgly6do	gAycgdx-XLsjRxg	DJHZiUZESrJL	cPREnm5p9yfgzNgAycgdx-XLsjRxglDJHZiUZESrJLSWsEF3ggFRrwV SIv4zEuH6	SIv4zEuH6
r-1-6-0,	2-6-0	100	14-8-2	, 17-10-0	24-7-0		314		3644	42-4-0	43-10-0
1-6-0	7-6-0		7-2-2	3-1-14	0-6-9	-	6-9	0	5-0-4	5-11-12	1-6-0

Scale = 1:76.3

4-11-5 9 10_{TS} 3x4 // 27 5x8 == 2x4 || 5x12 == 7.00 12 2x6 // 1/ 9xg

5-6-Z

3x6 //

2x4 || =

12 3x4

13 3x6

14 3x8

=

2×4 15

5x8 == 16

2×4 || 17

3x6 //

	1-6-0	14-8-2	17-10-0	24-7-0	31	4-0	3644	42-4	9
	7-6-0	7-2-2	3-1-14	0-6-9	-9	0-6-9	5-0-4	5-11-12	.12
late Offsets (X,Y)-	[2:0-0-14,0-1-8], [3:0-3-0	0-3-4], [4:0-4-2,(7-3-0], [7:0-6-0,0-2-8], [9:0-0-14,0-1-8], [16:0-4-0,0-3-0	:0-4-0,0-3-0]				
OADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	Vdefi	P/	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.81	Vert(LL)	0.30 14-15	>666	240	MT20	244/190
CDL 7.0	Lumber DOL	1.25	BC 0.75	Vert(CT)	-0.56 14-15	>905	30		
. 0.0 · O.O	Rep Stress Incr	YES	WB 0.83	Horz(CT)	0.17 9	n/a	/a		
CDL 10.0	Code FBC2020/T	PI2014	Matrix-MS	11			L	Weight: 235 lb	FT = 20%

.UMBER-		BRACING-
OP CHORD	2x4 SP No.2	TOP CHORE
3OT CHORD	2x4 SP No.2	BOT CHORD
VEBS	2x4 SP No.3	WEBS

Structural wood sheathing directly applied.
Rigid ceiling directly applied or 5-10-9 oc bracing.
1 Row at midpt 5-16, 5-14

REACTIONS.

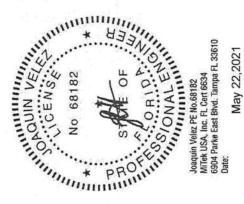
(size) 2=0-3-8, 9=0-3-8 Max Horz 2=174(LC 16) Max Uplift 2=-592(LC 12), 9=-731(LC 13) Max Grav 2=1647(LC 1), 9=1647(LC 1)

9 FORCES. (Ib

- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2938/1072, 3-4=-2360/960, 4-5=-2372/1011, 5-6=-2909/1314, 6-7=-2909/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 6-7=-2908/1314, 11-12=-1015/12587, 9-11=-1015/12587, 9-11=-1015/12587, 9-11=-1015/12587, 6-14=-420/328, 17-14=-333/823, 7-12=-112/381, 8-12=-380/275 BOT CHORD

WEBS

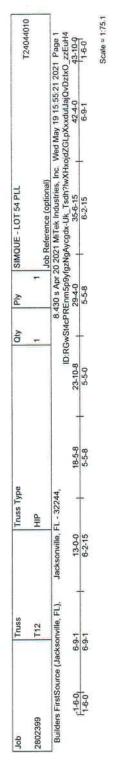
- NOTES—
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-16-0 to 1-6-0, interior(1) 17-10-0 to 31-4-0, to 34-4-0, interior(1) 34-4-0 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
 Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|i=lb)

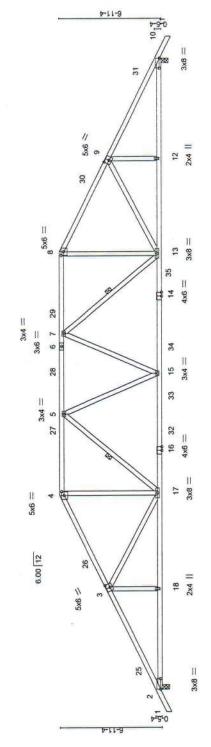


ore use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not acting individual building component, not acting individual building component, not acting individual building of admitting design into the overall acting individual building of individual building and personal injury and property damage. For general guidance regarding the debiewy, erection and bracing of trusses and fruss systems, see

ANSI/TPII Quality Criteria, DSB-89 and BCSI Building Component available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. S1922020 BEFORE USE: sed only upon parameters shown, and is for an individual building component, not ilicability of design parameters and properly incorporate this design into the overall.







							5.27
	-		GRIP	244/190			FT = 20%
42-4-0	1-6-9-1		PLATES	MT20			Weight: 230 lb
35-6-15	6-2-15		P/1	240	180	n/a	
			l/defi	>999	>941	n/a	
		-1]	(loc)	15-17	15-17	10	
29-4-0	8-2-0	0-0'0-8-0	.⊑	-0.31	-0.54	0.17	
		1-3-0,0-3-0J, [10:C	DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
21-2-0	8-2-0	3-0,0-2-0], [9:0		TC 0.61	0.92	0.54	ix-MS
		0-2-0], [8:0-	CSI	TC	BC	WB	Matr
13-0-0	6-2-15	-3-0], [4:0-3-0,0	2-0-0	1.25	1.25	YES	P12014
		0-1], [3:0-3-0,0	ACING-	ate Grip DOL	nber DOL	o Stress Incr	Je FBC2020/T
6-9-1	6-9-1	[2:0-8-0,0	SP	Plate	Lur	Rep	Š
		sets (X,Y)-	(pst)	20.0	7.0	0.0	10.0
		Plate Offs	LOADING	TCLL	TCDL	BCLL	BCDL

LUMBER-	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2 "Except"
	14-16: 2x4 SP M 31
WEBS	2x4 SP No 3

Structural wood sheathing directly applied or 2-10-15 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing. 1 Row at midpt 5-17, 7-13

BRACING-TOP CHORD BOT CHORD WEBS

Max Horz 2=-156(LC 13) Max Uplift 2=-675(LC 12), 10=-675(LC 13) Max Grav 2=1786(LC 2), 10=1786(LC 2) 2=0-3-8, 10=0-3-8 (size) REACTIONS.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3241/1143, 3-4=-2730/979, 4-5=-2402/938, 5-7=-2814/1012, 7-8=-2402/938,

8-9=-2730/979, 9-10=-3241/1144

BOT CHORD 2-18=-1050/2840, 17-18=-1051/2837, 15-17=-812/2741, 13-15=-780/2741,

12-13=-895/2837, 11-12=-894/2840

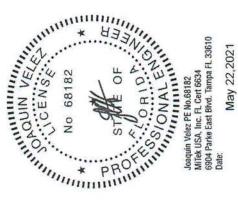
3-17=-542/363, 4-17=-243/956, 5-17=-627/322, 5-15=-100/290, 7-15=-99/290,

7-13=-627/322, 8-13=-242/956, 9-13=-543/363

NOTES-

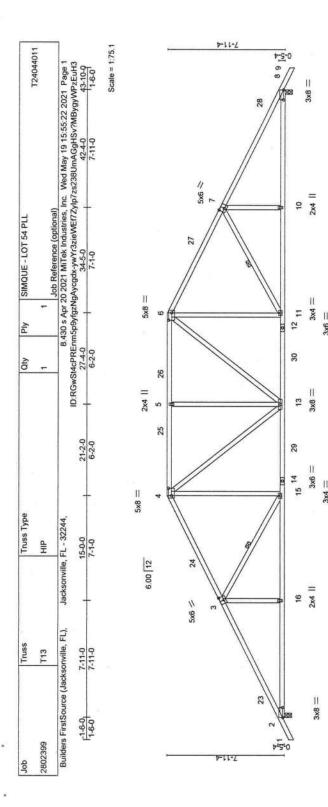
- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MM*RS (envelope) gable end zone and C-C Exterior(2E)-1-6-0 to 1-6-0, Interior(1) 1-6-0 to 13-0-0. Exterior(2R) 13-0-0 to 17-2-15. Interior(1) 17-2-15 to 29-4-0, Exterior(2R) 29-4-0 to 33-6-15, Interior(1) 33-6-15 to 43-10-0 zone; C-C for members and forces & MW*RRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 4) Provide adequate drainage to prevent water ponding.
 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) **This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb)

- 2=675, 10=675.



a truss system. Before use, the building designer must verify the applicability of design parameters shown, and its for an individual building component, nod a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate bits design into the overall building design. Bracing indicated is to prevent bucking of individual truss web androt chord members only. Additional temporary and permanent bracing is always required for siballing and to prevent bucking of individual truss web androt driver the manual transparent behaviors to state the prevent bucking of individual trusses and truss systems; see ANTIFPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 5/19/2020 BEFORE USE IA READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 READ NOTES on serious only upon paractors. This design is based only upon paractors. This design is based only upon parameters and only the applicability of design.





12.08-0.0-1], [3:0-3-0,0-3-4], [4:0-6.0,0-2-8], [6:0-6-0,0-3-4], [8:0-8-0,0-1] 12.08-0,0-1]		7-11-0	15-0-0	21-2-0	27-4-0	-	34-5-0	42.40	T	
SPACING- 2-0-0 CSI. DEFL. in Plate Grip Dol. 1.25 TC 0.76 Vert(LL) -0.26	Plate Offsets (X,Y)-		-3-4], [4:0-6-0,0-	2-8], [6:0-6-0,0-2-8], [7:0-	3-0,0-3-4], [8:0-8-0	,0-0-1]	0-1-1	P-11-7	1	11
Plate Grip DOL 1.25 TC 0.76 Vert(LL) -0.26 Lumber DOL 1.25 BC 0.89 Vert(CT) -0.43 Rep Stress Incr YES WB 0.42 Horz(CT) 0.17 Code FBC2020/TPI2014 Matrix-MS BRACING-17 PROP STRESS Incr ASP No.2 WB No.2 WEBS 1	LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl L/d	PLATES	GRIP	1
- Lumber DOL 1.25 BC 0.89 Vert(CT) -0.43 - Rep Stress Incr YES WB 0.42 Horz(CT) 0.17 - Code FBC2020/TPI2014 Matrix-MS - ASP No.2 - ASP No.2 - ASP No.2 - WB COMB CHORD STRESS INCR ASP No.2 - WB COMB CHORD STRESS INCR ASP No.3 - WB COMB CHORD STRESS INCR ASP No.3 - WB COMB CHORD STRESS INCR ASP No.3	TCLL 20.0	Plate Grip DOL	1.25			0.26 11-13		MT20	244/190	
* Rep Stress Incr YES WB 0.42 Horz(CT) 0.17 Code FBC2020/TPI2014 Matrix-MS 44 SP No.2 TOP CHORD 8 44 SP No.2 Horz(CT) 0.17 **ASP No.2 TOP CHORD 8 **ASP No.2 HORZ TOP CHORD 8 **ASP No.2 TOP CHORD 8 **ASP No.2 HORZ TOP CHORD 8 **ASP No.2 TOP CHORD 8 **ASP No.3 TOP CHORD 8 **	TCDL 7.0	Lumber DOL	1.25			0.43 11-13				
Code FBC2020/TPI2014 Matrix-MS BRACING- 44 SP No.2 TOP CHORD 8 44 SP No.2 WEBS 1	BCLL 0.0 •	Rep Stress Incr	YES		_	0.17 8	n/a n/a			
2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3	BCDL 10.0	Code FBC2020/T	PI2014	Matrix-MS	66 55			Weight: 232 lb	FT = 20%	
2x4 SP No.2 TOP CHORD \$ 2x4 SP No.2 BOT CHORD F 2x4 SP No.3 WEBS 1	LUMBER-				BRACING-					
2x4 SP No.2 BOT CHORD F 2x4 SP No.3 WEBS 1		No.2			TOP CHORD	Structura	al wood sheathing	g directly applied.		
ZX4 SP NO.3		No.2			BOT CHORD	Rigid ce	ling directly appli	ed or 5-8-0 oc bracing.		
		No.3			WEBS	1 Kow a	t midpt	3-15, 7-11		

REACTIONS.

(size) 2=0-3-8, 8=0-3-8 Max Horz 2=178(LC 12) Max Uplift 2=-672(LC 12), 8=-672(LC 13) Max Grav 2=1783(LC 2), 8=1783(LC 2)

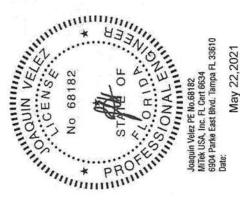
Q FORCES. (Ib

- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3195/1116, 3-4=-2563/918, 4-5=-2439/948, 5-6=-2439/948, 6-7=-2563/918, 7-8=-3195/1117 7-8=-3195/1117 7-11=-343/2794, 13-15=-662/2232, 11-13=-548/2232, 10-11=-856/2794, 8-10=-857/2794 3-16=0/304, 3-15=-673/431, 4-15=-170/599, 4-13=-234/450, 5-13=-373/291, 6-13=-234/450, 6-11=-170/599, 7-11=-673/431, 7-10=0/304 BOT CHORD WEBS

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, interior(1) 1-6-0 to 15-0-0. Exterior(2R) 15-0-0 to 19-2-15, Interior(1) 192-15, to 27-4-0. Exterior(2R) 27-4-0 to 31-6-15, Interior(1) 31-6-15 to 27-4-0. Exterior(2R) 27-4-0 to 31-6-15, Interior(1) 31-6-15 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

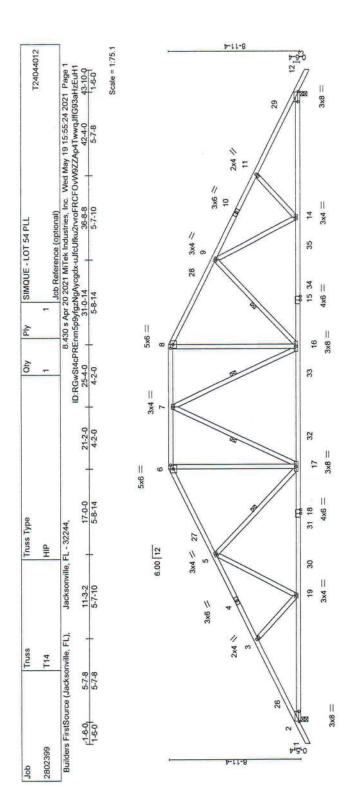
- 4) Provide adequate drainage to prevent water ponding.
 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|=|b|)



Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fusion system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building designs must verify the applicability of design parameter with Additional temporary and permanent braining is always required for stability and to prevent cubishes with possible personal injury and properly damage. For general guidance regarding the training the ability of the section and braining for trustees and truss systems, see

ANSITTPI Quality Criteria, DSB-89 and BCSI Building Con Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE





(E.	8-5-6	17-0-		25-4-0	-	33-10-1	0	42-4-0	
	8-5-6	8-6-10		8-4-0	-	8-6-10		8-5-6	
Plate Offsets (X,Y)- [2:0-8-4,0-0-5	[2:0-8-4,0-0-5], [6:0-3-0,	0-2-0], [8:0-3-0,0	3, [6:0-3-0,0-2-0], [8:0-3-0,0-2-0], [12:0-8-4,0-0-5]						
LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defi	P/I	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.43	Vert(LL)	-0.29 14-16		240	MT20	244/190
TCDL 7.0	Lumber DOL		BC 0.43	Vert(CT)	-0.49 14-16		180		
BCLL 0.0 *	Rep Stress Incr			Horz(CT)	0.13 12		n/a		
BCDL 10.0	Code FBC2020/TPI2014	TPI2014	Matrix-MS					Weight: 243 lb	FT = 20%
LUMBER-				BRACING-					
	2x4 SP No.2			TOP CHORD		real wood	sheathing dir.	Structural wood sheathing directly applied or 3-0-6 oc purlins.	c purlins.
BOT CHORD 2x4 \$	2x4 SP M 31			BOT CHOR		seiling dire	ctly applied o	Rigid ceiling directly applied or 7-4-12 oc bracing.	
						The second second	-	4 4 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	

2x4 SP No.2 2x4 SP M 31 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

Structural wood sheathing directly applied or 3-0-6 oc purlins. Rigid ceiling directly applied or 7-4-12 oc bracing. 1 Row at midpt 5-17, 7-17, 7-16, 9-16

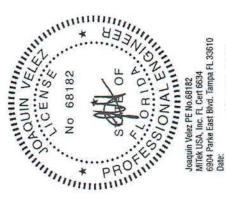
(size) 2=0-3-8, 12=0-3-8 Max Horz 2=199(LC 16) Max Uplift 2=-667(LC 12), 12=-667(LC 13) Max Grav 2=1810(LC 2), 12=1810(LC 2) REACTIONS.

TOP CHORD FORCES.

BOT CHORD

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=3296/1157, 3-5=-3129/1093, 5-6=2419/893, 6-7=-2119/851, 7-8=-2119/851, 2-3=-3296/1157, 3-5=-3129/1093, 11-12=-3296/1157 (lb) 2-19=-1119/2998, 17-19=-869/2531, 16-17=-552/2161, 14-16=-700/2531, 12-14=-319/2898 (lb) 2-19=-141/516, 5-17=-599/414, 6-17=-257/854, 7-17=-287/194, 7-16=-287/193, 8-16=-257/854, 9-16=-599/414, 9-14=-141/516, 11-14=-256/263 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; WWFRS (envelope) gable end Cor. Exterior(2R) 17-6-0 to 1-6-0 to 17-0.0, interior(1) 1-6-0 to 17-0.0, Exterior(2R) 17-0-0 to 21-2.0, Interior(1) 21-2-0 to 25-4-0, Exterior(2R) 25-4-0 to 29-6-15, Interior(1) 29-6-15 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 4) Provide adequate drainage to prevent water ponding.
 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL= 10.0psf.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=b) 2-667, 12-667.

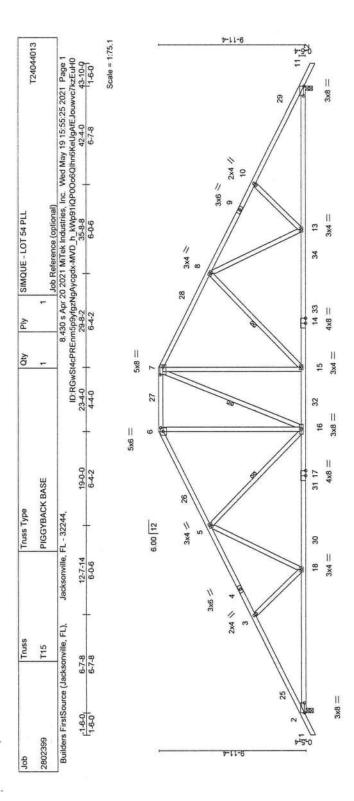


May 22,2021

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fluxs system. Before use, the building designer marks with the applicability of design parameters and properly incorporate this design info the overall building design. Bracing indicated is to prevent bucking of individual truss well and or chord members only. Additional temporary and permanent bracing its always required for stability and to prevent collapses with possible personal injury and properly damage. For general guidance regarding the institute of inside and truss systems, see

ANSI/IPHI Quality Criteria, DSB-89 and BCSI Building Compor AND INCLUDED MITER REFERENCE PAGE MII-7473 I READ NOTES ON

6904 Parke East Blvd Tampa, FL 36610 MiTek



	9-8-1		19-0-0	23-4-0	32-	32-7-15		42-4-0		
	9-8-1	-	9-3-15	4-4-0	9-6	9-3-15		9-8-1		
Plate Offsets (X,Y)- [2:0-8-0,0-0-1	[2:0-8-0,0-0-1], [6:0-3-0,0	2-0], [7:0-6-0,0-2	1], [6:0-3-0,0-2-0], [7:0-6-0,0-2-8], [11:0-8-0,0-0-1]							
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)		3	PLATES	GRIP	
TCLL 20.0	Plate Grip DOL	1.25	TC 0.49	Vert(LL)	-0.38 13-15	>999	240	MT20	244/190	
TCDL 7.0	Lumber DOL	1.25		Vert(CT)	-0.65 13-15		180			
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.51	Horz(CT)	0.13 11	n/a	n/a			
BCDL 10.0	Code FBC2020/TPI2014	912014	Matrix-MS					Weight: 240 lb	FT = 20%	
LUMBER- TOP CHORD 2x4 8 BOT CHORD 2x4 9 14-11	2x4 SP No.2 2x4 SP M 31 "Except" 14-17: 2x4 SP No.2 2x4 SP No.3	7		BRACING- TOP CHORD BOT CHORD WEBS	13/17/4/8	Structural wood s Rigid ceiling direc Row at midpt	heathing dire	Structural wood sheathing directly applied or 2-11-9 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing. 1 Row at midpt 5-16, 7-16, 8-15	oc purlins.	

(size) 2=0-3-8, 11=0-3-8 Max Horz 2=-221(LC 17) Max Uplift 2=-663(LC 12), 11=-663(LC 13) Max Grav 2=1810(LC 2), 11=1811(LC 2) REACTIONS.

(Q) TOP CHORD FORCES.

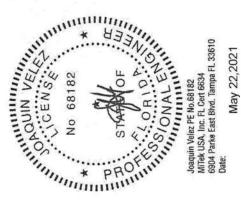
- Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3256/1125, 3-5=-3066/1063, 5-6=-2237/861, 6-7=-1948/827, 7-8=-2239/861, 8-10=-3070/1064, 10-11=-3259/1125
2-18=-1098/2857, 16-18=-821/2430, 15-16=-438/1950, 13-15=-658/2433, BOT CHORD

11-13=-878/2860 3-18=-302/300, 5-18=-176/631, 5-16=-698/461, 6-16=-241/752, 7-15=-286/760, 8-15=-699/462, 8-13=-176/632, 10-13=-302/300

WEBS

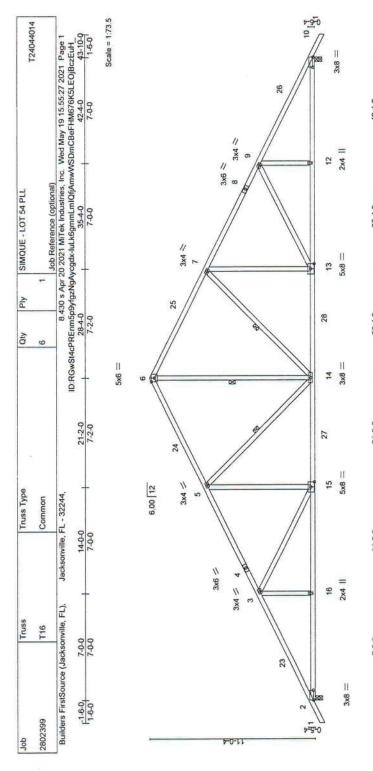
NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0,18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, interior(1) 1-6-0 to 19-0-0. Exterior(2E) 19-0-0 to 23-4-0, Exterior(2R) 23-4-0, Exterior(2R) 23-4-0, Exterior(1) 1-6-0 to 27-6-15, interior(1) 27-6-15 to 43-10-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
 4) Provide adequate drainage to prevent water ponding.
 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) This truss has been designed for a live load of 20.0 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing shawlys required for stability and to prevent lostseave they possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see AMSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 ins and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII.-7473 rev. 5:19:2020 BEFORE USE. to connectors. This design is based only upon parameters shown, and is for an infiviliable building component, not and designer must verify the applicability of design parameters and properly incorporate this design into the overall operent bucking of individual horses web, and or chord members only. Additional temporary and permanent backing to prevent bucking of individual horses.





			GRIP	244/190			FT = 20%
42.4-0	7-0-0		PLATES	MT20			Weight: 236 lb
35-4-0	7-0-0		P/I	240	180	n/a	700000
			l/defi	>999	>999	n/a	
			(loc)	14-15	14-15	10	
28-4-0	7-2-0		in (loc)	-0.29	-0.49	0.18	
			DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
21-2-0	7-2-0	0-4-0,0-3-0]		0.61	0.91	0.67	-MS
		,0-3-0], [15:0	CSI.	TC 0.61	BC	WB	Matrix
14-0-0	7-0-0	0-0-1], [13:0-4-0	2-0-0	1.25	1.25	YES	PI2014
	-	2:0-8-0,0-0-1], [10:0-8-0,	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/T
7-0-0	7-0-0						
1		fsets (X,Y)-	G (psf)	20.0	7.0	. 0.0	10.0
		Plate Of	LOADIN	TCLL	TCDL	BCLL	BCDL

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

Structural wood sheathing directly applied or 2-11-8 oc purlins. Rigid ceiling directly applied or 5-7-4 oc bracing. 1 Row at midpt 6-14, 7-14, 5-14

BRACING-TOP CHORD BOT CHORD WEBS

(size) 2=0-3-8, 10=0-3-8 Max Horz 2=244(LC 12) Max Uplift 2=-657(LC 12), 10=-657(LC 13) Max Grav 2=1788(LC 2), 10=1788(LC 2)

REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-3241/1098, 3-5=-2681/956, 5-6=-2030/837, 6-7=-2030/837, 7-9=-2681/956, 9-10=-3241/1099 (lb) - 2-16=-1093/2839, 15-16=-1093/2839, 14-15=-771/2341, 13-14=-606/2341, 12-13=-851/2839, 10-12=-851/2839, 10-12=-851/2839, 10-13=-124/556, 9-13=-570/364, 9-12=0/268, 5-14=-479/1459, 7-14=-827655, 3-15=-570/363, 3-16=0/268 TOP CHORD FORCES.

BOT CHORD

WEBS

- 1) Unbalanced roof live loads have been considered for this design.

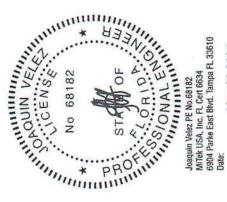
 2) Wind: ASCE 74:6; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MYREX (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 2-8-13, Interior(1) 2-8-13 to 21-2-0, Exterior(2R) 21-2-0 to 25-4-13, Interior(1) 25-4-13 to 43-10-0 zone; C-C for members and forces & MWVERS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 plate grip DOL=1.60

 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

 4) This trus has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will it between the bottom chord and any other members, with BCDL = 10.0psf.

 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2-657, 10-657.

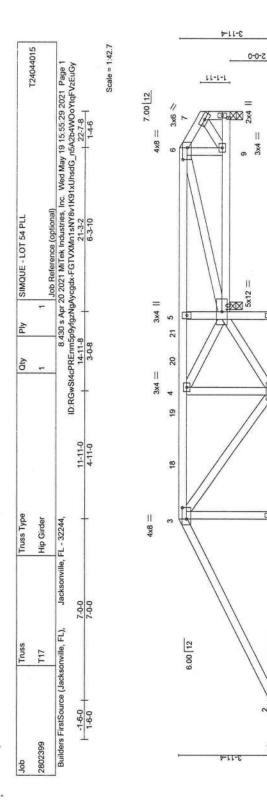


May 22,2021

NARAING - Venity design paramaters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. \$5.09.2020 BEFORE USE.

Subsign veil for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fusus system. Before use, the building designer must verify the applicability of design parameters and properly incroprorate this design into the overall building designer must verify the applicability of design parameters and properly incroprorate this design into the overall building designer must verify the applicability of design parameters only. Additional temporary and permanent bracking building designer and to prevent doubts with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and hadron fusus systems, saw AANSTIPM Quality Criteria, DSB-89 and BCSI Building Component Sarkey information available from Truss Plate Institute, 2810 Calai Highway, Suite 203 Waldorf, MD 20601





2-0-0 2-0-3-7,0-1-8], 3-0-2-0], 6:0-5-8,0-2-0] 4-11 13-0-5-4,0-2-0], 6:0-5-8,0-2-0] 4-11 13-0-5-4,0-2-0], 6:0-5-8,0-2-0] 5-0-0 5-0-0 1-2-0	11-11-0	14-11-8	15-40	21-3-2	1 22-	1-8-1
NG (pst) SPACING - 2.0-3-7,0-1-8], [3:0-5-4,0-2-0], [6:0-5-8,0-2-0] SPACING - 2-0-0 CSI. 2.0-0 TC 0 CSI. C	4-11-0	3-0-8	048	5-11-2	1	14-6
ING (pst) SPACING- 2-0-0 20.0 Plate Grip DOL 1.25 7.0 Lumber DOL 1.25 0.0 Rep Stress Incr NO 10.0 Code FBC2020/TPI2014						
20.0 Plate Grip DOL 1.25 7.0 Lumber DOL 1.25 0.0 • Rep Stress Incr NO 10.0 Code FBC2020/TPI2014	DEFI		(loc)	2	PLATES	GRIP
7.0 Lumber DOL 1.25 0.0 * Rep Stress Incr NO 10.0 Code FBC2020/TPI2014	0.61 Vert(LL)	LL) 0.06 1	14-17 >999	240	MT20	244/190
0.0 * Rep Stress Incr NO 10.0 Code FBC2020/TPI2014			9-10	180		
10.0 Code FBC2020/TPI2014			8	n/a		
	_			Carriera	Weight: 130 lb	FT = 20%
UMBER-	BRAC	BRACING-	Oferiodisco.	de de la constante de la const	A O A so bellean of	1
OF CHOCK LAT OF NO.2		מאטנוי	all control woon sile	a streaming affects	Siluciulal wood sheathing directly applied of 4-0-15 oc purifies	oc burins,

2x4 ||

Ξ

24

12 3x8

23

22

4x6 13

3x6

2x4 || 4

0

5-11: 2x4 SP No.3, 8-10: 2x4 SP No.2 2x4 SP No.3 REACTIONS. WEBS

Rigid ceiling directly applied or 6-0-0 oc bracing.

BOT CHORD

(size) 2=0-3-8, 8=0-3-8, 10=0-3-8 Max Horz 2=186(LC 8) Max Uplift 2=645(LC 8), 8=204(LC 23), 10=-1303(LC 5) Max Grav 2=962(LC 1), 8=223(LC 20), 10=1700(LC 1) 2=0-3-8, 8=0-3-8, 10=0-3-8 2=186(LC 8)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
D 2-3=-1495/1084, 3-4=-748/613, 7-8=-252/234
D 2-14=-1011/1265, 12-14=-1028/1288, 5-10=-404/319
3-14=-418/660, 3-12=-667/538, 4-12=-69/259, 10-12=-702/893, 4-10=-1116/905, 6-10=-369/286, 7-9=-265/282 ТОР СНОRD ВОТ СНОRD WEBS

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific
 - to the use of this truss component.

 - Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Will it between the bottom around any others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (fi=lb) 2=645, 8=204, 10=1303.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 138 lb up at 7-0-0, 110 lb down and 138 lb up at 13-0-12, and 110 lb down and 138 lb up at 13-0-12, and 110 lb down and 138 lb up at 13-0-12, and 110 lb down and 136 lb up at 14-10-12 on top chord, and 332 lb down and 399 lb up at 7-0-0, 86 lb down and 88 lb up at 14-0-12, and 86 lb down and 88 lb up at 14-0-12, and 86 lb down and 88 lb up at 14-9-12 on bottom
 - chord. The design/selection of such connection device(s) is the responsibility of others.

 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-54, 3-5=-54, 6-7=-54, 11-15=-20, 8-10=-20

Continued on page 2

NAMANINO - Verity design perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overal building designer must verify the applicability of design parameters and properly incorporate this design into the overal building design. Brazing indicated is to prevent bucking of individual tures web and/or chord members only. Additional temporary and permanent brazing is always required for stability and to prevent oldingse with possible personal injury and properly damage. For general guidance regarding the tablication, storage, delivery, erection and brazing of trusses and truss systems, see — AMSI/IPH Quality Criteria, DSB-89 and BCSI Building Con Safey Information available from Truss Plate Institute, 2610 Crain Highway, Suite 203 Waldorf, MD 20601

No 68182

STATEDOF

RENOVELLENS

NO 68182

STATEDOF

RENOVELLENS

RENO Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 May 22,2021



6904 Parke East Blvd. Tampa, FL. 36610

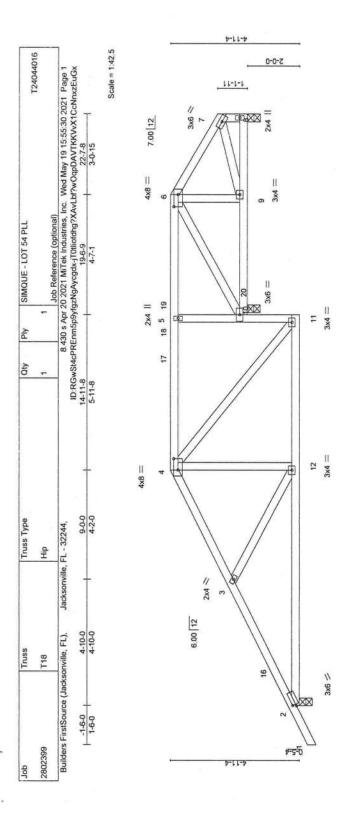
Job	Truss	Truss Type	αtγ	Ply	SIMQUE - LOT 54 PLL T24044015
2802399	117	Hip Girder	T	-	Job Reference (optional)
Builders FirstSource (Jacks	onville, FL),	Jacksonville, FL - 32244,	B ID:RGwSt4cPREnm	430 s Apr 5p9yfgzNg	8.430 s Apr 20 2021 MiTek Industries, Inc. Wed May 19 15:55:29 2021 Page 2 ID:RGwSI4cPREnm5p9yfgzNgAycgdx-FGTVXMn1sNY8v1K91xUhsdG n5A2b4WOoY1gFVzEuGy

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 3=-110(B) 11=-63(B) 5=-110(B) 14=-332(B) 18=-110(B) 19=-110(B) 20=-110(B) 22=-63(B) 23=-63(B) 24=-63(B)

Machania - Vanify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5192020 BEFORE USE.

Design valid for use only with MITer® connectors. This design is based only upon parameters shown, and is for an individual building component, not a tube system. Before use, the building designer must verify the applicabilities of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent obtaining of individual trassive web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obtaining of individual trassive bear of control in an appropriate of stability and permanent bracing its prevent obtaining of trusses and truss systems, see ANSITRI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501

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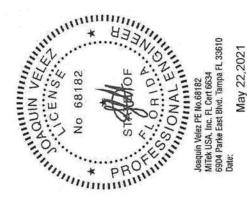
	0-0-6				14-11-8		0-4-61	8-0-8	8-1-77	
	0-0-6				5-11-8		0-4-8	4-2-9	3-0-15	
Plate Offsets (X	Plate Offsets (X,Y)- [2:0-0-14,0-1-8], [4:0-5-4,0-2-0], [6:0-5-8,0-2-0]	0], [6:0-5-8,0-2	-0]							
LOADING (psf)	SPACING-	0-0	CSI.			in (loc		2	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	2	0.38	Vert(LL)	-0.13 12-15		240	MT20	244/190
TCDL 7.0		1.25	BC	0.62		-0.27 12-1	5 >653	180		
BCLL 0.0		res	WB	0.56		0.01	8 n/a	n/a		
BCDL 10.0	Code FBC2020/TPI20	14	Matrix-MS	-MS	ST N				Weight: 119 lb	FT = 20%
LUMBER-					BRACING-					
TOP CHORD	2x4 SP No.2				TOP CHORD		ctural wood	sheathing dire	Structural wood sheathing directly applied or 6-0-0 oc purlins,	c purlins,
BOT CHORD	2x4 SP No.2 *Except* 5-11: 2x4 SP No.3				ROTCHORD		except end verticals.	cals,	except end verticals. Rigid ceiling directly applied or 9-0-0 oc bracing	
WEBS	2x4 SP No.3								.n	

REACTIONS.

(size) 2=0-3-8, 10=0-3-8, 8=0-3-8 Max Horz 2=208(LC 12) Max Uplift 2=-284(LC 12), 10=-467(LC 9), 8=-156(LC 8) Max Grav 2=617(LC 23), 10=876(LC 1), 8=254(LC 24)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2.3=-778/360, 3-4=-517/229, 6-7=-253/407, 7-8=-231/339 3.212=-256/260, 4-12=-203/420, 10-11=-159/381, 5-10=-337/258, 9-10=-317/188 3.12=-256/260, 4-12=-78/404, 4-11=-551/274, 6-10=-238/387, 6-9=-253/137, 7-9=-270/162 FORCES. (Ib) TOP CHORD BOT CHORD WEBS

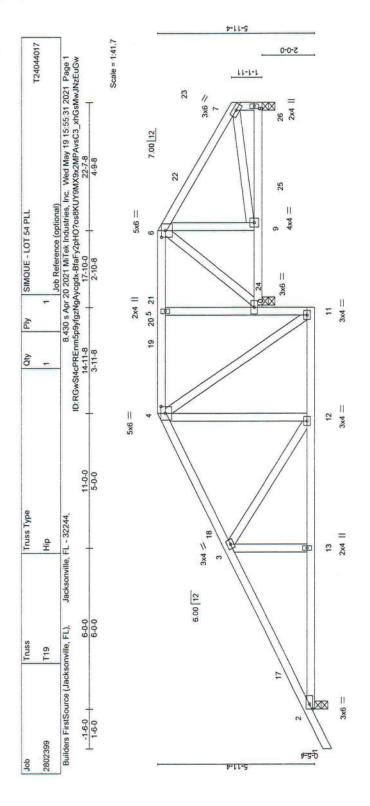
- NOTES1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; WMFRS (envelope) gable end zone and C-C Exterior(12E) -1-6-0 to 1-6-0, interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-6-9, Exterior(2E) -1-5-0 to 1-6-0, interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-6-9, Exterior(2E) -1-6-0 to 1-6-0, interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-6-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-6-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-6-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 13-2-15 to 19-0-0, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 13-2-15 to 19-0-0, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 13-2-15 to 19-0-0, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 13-2-15 to 19-0-0, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 19-0-0, Interior(1) 13-2-15 to 19-0-0, Exterior(2R) 9-0-0 to 19-0-0, Exterior(2R) 9-0-0 to 19-0-0, Exterior(2R) 9-0-0 to 19-0-0, Exterior(2R) 9-0-0 to 19-0-0 to 19-0
- 2=284, 10=467, 8=156.



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NARAING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE.

Begin valid for use only with Mattek connections. This design is based only upon parameters shown, and is for an inclividual building component, not a trust system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building of individual truss web and ord chord members only. Additional temporary and permanent bracing the swarp required for stability and to prevent collapse with possible perconal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see AMXITPH Quality Criteria, DSB-99 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



	0-0-0		0-0-1			11-11	-	0-1-77	
	0-0-9		5-0-0	3-11-8		0-4-8 2-6-0	. 0	4-9-8	Γ
Plate Offsets (X,Y)- [4:0-3-0	[4:0-3-0,0-2-0], [6:0-3-0,0-1-12]	-1-12]							
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defi	P/I	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.30	Vert(LL)	0.04 13-16	666<	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC 0.32	Vert(CT)	-0.07 13-16	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.44	Horz(CT)	0.00	n/a	n/a		
BCDL 10.0	Code FBC2020/T	P12014	Matrix-MS					Weight: 130 lb FT = 20%	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2	5P No.2			BRACING- TOP CHORD		ral wood s	heathing dire	Structural wood sheathing directly applied or 6-0-0 oc purlins,	purlins,

17.10.0

15 4.0

11.0.0

except end verticals.

Rigid ceiling directly applied or 9-1-5 oc bracing.

BOT CHORD

2x4 SP No.2 *Except* 5-11: 2x4 SP No.3 2x4 SP No.3 REACTIONS.

TOP CHORD BOT CHORD

2=0-3-8, 10=0-3-8, 8=0-3-8 (size)

Max Horz 2=229(LC 12) Max Uplitt 2=-286(LC 12), 10=-388(LC 9), 8=-158(LC 8) Max Grav 2=624(LC 23), 10=855(LC 1), 8=269(LC 24)

(b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-804/327, 3-4=-378/199, 6-7=-234/340, 7-8=-226/279 2-13=-405/664, 12-13=-405/664, 11-12=-138/266, 10-11=-210/443 3-12=-463/314, 4-12=-139/363, 4-11=-494/264, 6-10=-246/356, 6-9=-299/163 FORCES.

TOP CHORD BOT CHORD

NOTES-

WEBS

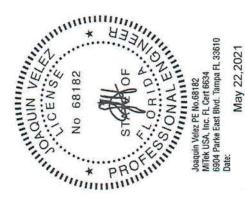
- Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl.,
 GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-0-0. Exterior(2R) 11-0-0
 to 15-2-15, Interior(1) 15-2-15 to 17-10-0, Exterior(2R) 17-10-0 to 22-0-15, Interior(1) 22-0-15 to 22-0-12 zone; porch right
 exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1-60 plate grip DOL=1-60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific 50
 - to the use of this truss component. 3
- Provide adequate drainage to prevent water ponding.

 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

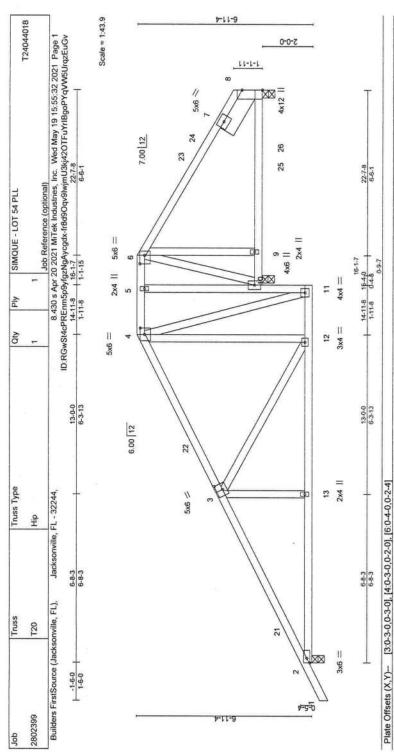
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 6 5 4
 - will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (i=lb) 2=286, 10=388, 8=158.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED mitter Recentaries. The common and is for an individual building component, not a trust system. Before use, the building design is based only upon parameters and properly incorporate his design into the overall activities as system. Before use, the building design in the overall properly incorporate his design parameters and properly incorporate his design parameters and properly incorporate his design into the overall building design. Bracing indicated is to prevent loutishing of individual trusts web androt chord members only. Additional temporary and permanent bracing is sharps required for stability and prevent obsidies with possible personal injury and properly damage. For general guidance regarding the labrication, storage, delivery, erection and bracing of trusses and truss systems. See ANSITFH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





240 240 180 1/a //defl >873 >999 n/a (loc) 9-16 13-20 10 0.11 0.09 0.03 BRACING-TOP CHORD BOT CHORD DEFL. Vert(LL) Vert(CT) Horz(CT) TC 0.43 BC 0.44 WB 0.55 Matrix-MS
 SPACING 2-0-0

 Plate Grip DOL
 1.25

 Lumber DOL
 1.25

 Rep Stress Incr
 YES

 Code FBC2020/TPI2014
 (psf) 20.0 7.0 0.0 LOADING LUMBER-TCDL BCDL

FT = 20%

q

Weight: 137

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 8-9-2 oc bracing.

GRIP 244/190

PLATES MT20

2x4 SP No.2 2x4 SP No.2 "Except" 5-11: 2x4 SP No.3 2x4 SP No.3 Right 2x8 SP 2400F 2.0E -1 1-11-8 REACTIONS.

SLIDER

WEBS

TOP CHORD BOT CHORD

2=0-3-8, 10=0-3-8

Max Horz 2=248(LC 12) Max Upitit 8=-224(LC 13), 2=-299(LC 12), 10=-298(LC 9) Max Grav 8=333(LC 24), 2=654(LC 1), 10=773(LC 1)

FORCES.

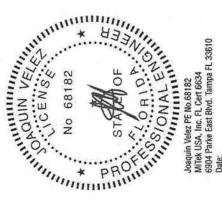
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
2-3=844393, 3-4=302/274, 4-5=-106/255, 5-6=-109/264, 6-8=-242/434
2-13=-429/693, 12-13=-430/688, 10-11=-282/520, 9-10=-261/171, 8-9=-279/178
3-13=0/280, 3-12=-575/387, 4-12=-144/391, 4-11=-528/275, 6-10=-407/633, 6-9=-562/275 TOP CHORD BOT CHORD WEBS

NOTES

- Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-16; Vutt=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ff; Cat. II; Exp C; Encl.,
 GCpi=0.18; MWFRS (envelope) gabbe end zone and C-C Exterior(2E) -14-0 to 1-6-0, Interior(1) 1-6-0 to 13-0-0. Exterior(2E) 13-0-0
 to 16-1-7, Exterior(2R) 16-1-7 to 20-4-5, Interior(1) 20-4-5 to 22-7-8 zone; porch right exposed; C-C for members and forces &
 MWFRS for reactions shown; Lumber DQL=1, 60 plate grip DQL=1,60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific 53
 - to the use of this truss component. 3
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

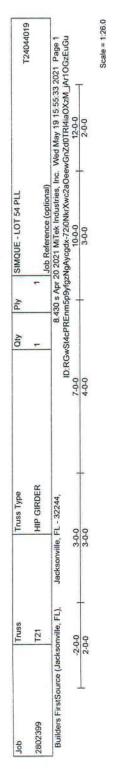
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 8=224, 2=299, 10=298.

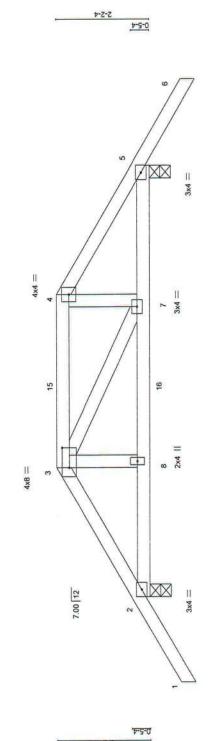


May 22,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 rev. 5192020 BEFORE USE.
Design vival for use only with MITE&Connection is based only upper angents shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design into the overall sale and the page of the page of







			PLATES GRIP				Weight: 49 lb FT = 20%
						_	
0-0-0	3-0-0		2	240	180	n/a	
			l/defl	×999	>999	n/a	
			(loc)	7-8	7-8	2	
			.⊑	0.03	-0.02	-0.01	
7-0-0	4-0-0		DEFL.	Vert(LL)	Vert(CT)	Horz(CT)	
			CSI.	0.27	0.21	90.0	x-MS
			CSI.	10	BC	WB	Matri
3-0-0	3-0-0		2-0-0	1.25	1.25	ON	9 2014
	_	[3:0-5-8,0-2-0]	SPACING-	Plate Grip DOL	Lumber DOL	Rep Stress Incr	Code FBC2020/TF
		late Offsets (X,Y)-	(Jsd)	20.0	7.0	. 0.0	10.0
		Plate Offs	LOADING	TCLL 20.0	TCDL	BCLL	BCDL

1 11

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 TOP CHORD BOT CHORD WEBS LUMBER-

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 8-11-0 oc bracing.

BRACING-TOP CHORD BOT CHORD

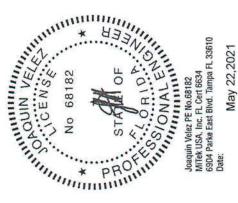
(size) 2=0-3-8, 5=0-3-8 Max Horz 2=-94(LC 6) Max Uplift 2=-342(LC 5), 5=-343(LC 9) Max Grav 2=521(LC 19), 5=521(LC 20) REACTIONS.

(ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. D 2-3=-554/489, 3-4=-454/441, 4-5=-554/488 D 2-8=-428/519, 7-8=-433/526, 5-7=-406/504 FORCES.

TOP CHORD BOT CHORD

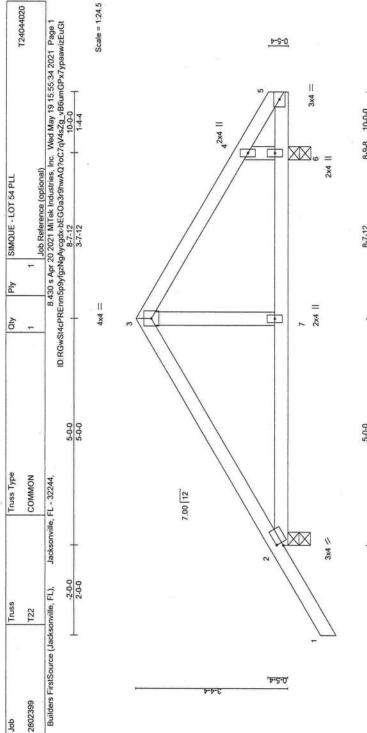
- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 133 lb down and 143 lb up at 3.0-0, and 93 lb down and 49 lb up at 4-11-4, and 133 lb down and 143 lb up at 7-0-0 on top chord, and 144 lb down and 48 lb up at 3.0-0, and 53 lb down at 4-11-4, and 144 lb down and 48 lb up at 6-11-4 on bottom chord. The design/selection of such 2=342, 5=343.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B) connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pf)
Vert : 13=54, 34=54, 4.6=54, 9-12=-20
Concentrated Loads (lb)
Vert : 3=-15(F) 4=-15(F) 8=-17(F) 7=-17(F) 15=-6(F) 16=-8(F)



Design valid for use only with MITeMS connectors. This design is based only upon parameters shown, and is for an individual building component, not a trust very fire papticability of design parameters and properly incorporate this design in the new overall building designer must verify the applicability of design parameters and properly incorporate this design in the new overall building design. Bracing inciderated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the facing temporary calculated and bracing of trusses and truss systems, see ANS/ITPHI Quality Criteria, DSB-89 and BCSI Building Component Sareby Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 5/19/2020 BEFORE USE NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE





DEFL. in (loc) I/defl L/d PLATES GRIP Vert(LL) 0.08 7-13 >999 240 MT20 244/190 Vert(CT) 0.07 7-13 >999 180 HOD MICH MICH MICH MICH MICH MICH MICH MICH	in (loc) l/defl L/d MT20 240 0.08 7-13 >999 240 0.07 7-13 >999 180 -0.01 2 n/a n/a Weight: 41 lb RD Structural wood sheathing directly applied or 6-0-0 oc p	5-0-0	5-0-0		Ī		8 6	3-7-12	0-1-12 1-2-8	_
in (loc) I/defl L/d MT20 244 0.08 7-13 >999 240 MT20 244 0.07 7-13 >999 180 -0.01 2 n/a n/a Weight: 41 lb	DEFL. in (loc)	late Offsets (X,Y)- [2:0-0-15,0-1-8]								
Vert(LL) 0.08 7-13 >999 240 MT20 24 Vert(CT) 0.07 7-13 >999 180 Horz(CT) -0.01 2 n/a n/a Weight: 41 lb	Vert(LL)	2-0-0	CSI.		DEFL.	.⊑			PLATES	GRIP
Vert(CT) 0.07 7-13 >999 180 Horz(CT) -0.01 2 n/a n/a Weight: 41 lb	Vert(CT)	1.25	2	0.38	Vert(LL)				MT20	244/190
Horz(CT) -0.01 2 n/a n/a Weight: 41 lb	Horz(CT) -0.01 2 n/a n/a Weight: 41 lb BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc p BOT CHORD Rigid ceiling directly applied or 9-5-5 oc bracing.		BC	0.33	Vert(CT)					
Weight: 41 lb	BRACING- TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc p	YES	WB	0.11	Horz(CT)					
	L DESCRIPTION	Code FBC2020/TPI2014 Matrix	Matri	k-MS					Weight: 41 lb	FT = 20%
		2x4 SP No.2			BOT CHORD		gid ceiling	directly applie	d or 9-5-5 oc bracing.	

2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 LUMBER-TOP CHORD BOT CHORD WEBS

(size) 2=0-3-8, 6=0-3-8 Max Horz 2=117(LC 9) Max Uplift 2=-202(LC 12), 6=-158(LC 13) Max Grav 2=433(LC 1), 6=415(LC 1)

REACTIONS.

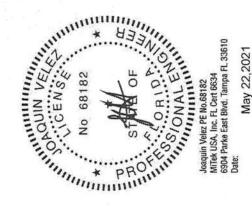
FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-306/468, 3-4=-302/509, 4-5=-194/315 D 2-7=-307/205, 6-7=-307/205, 5-6=-307/205 3-7=-306/151, 4-6=-264/354

TOP CHORD BOT CHORD WEBS

NOTES-

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 5-0-0, Exterior(2R) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 10-0-0 zone; cantilever right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1:60 plate grip DOL=1:60
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide
 - will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (|t=lb| 2=202, 6=158.



s and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIN-1473 rev. 5:19:2020 BEFORE USE, connectors. This design is based only upon parameters as shown, and its for an individual building component, not it designer must verify the applicability of design parameters and property incoporate this design into the overall prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing event collopses with possible personal injury and property damage. For general guidance regarding the and bracing of trusses and truss systems, see AMSI/TPI Quality Criteria, DSB-89 and BCSI Building Co. SP Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



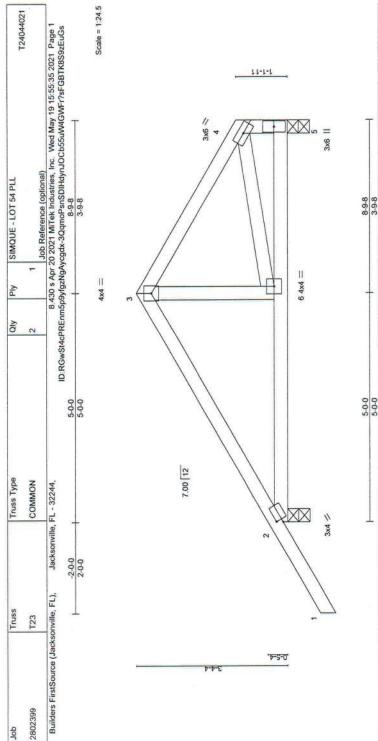


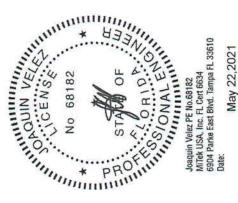
Plate Offsets (X,Y)- [2:0-0-1:	(,Y)- [2:0-0)-15,0-1-8]											1 1
LOADING (psf.		SPACING-	2-0-0	CSI.		DEFL.	572		l/defl	PΛ	PLATES	GRIP	
TCLL 20.0	-	Plate Grip DOL	1.25	7C	TC 0.36	Vert(LL)	0.05	6-9	>999	240	MT20	244/190	
TCDL 7.0	-	Lumber DOL	1.25	BC	0.28	Vert(CT)			666	180			
BCLL 0.0	• •	Rep Stress Incr	YES	WB	0.10	Horz(CT)			n/a	n/a			
BCDL 10.0	_	Code FBC2020/TPI2014	12014	Matri	c-MS						Weight: 42 lb	FT = 20%	
LUMBER-						BRACING-							
TOP CHORD	2x4 SP No.2					TOP CHORD		nctural	s poom	heathing dir	Structural wood sheathing directly applied or 6-0-0 oc purlins,	oc purlins,	
BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3					BOT CHORD	-	cept en	except end verticals. Rigid ceiling directly	als. tly applied o	except end verticals. Sigid ceiling directly applied or 8-4-7 oc bracing.		

(size) 2=0-3-8, 5=0-3-8 Max Horz 2=135(LC 12) Max Uplift 2=-202(LC 12), 5=-108(LC 13) Max Grav 2=440(LC 1), 5=307(LC 1) REACTIONS.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. (D 2-3=-342/559, 3-4=-322/576, 4-5=-280/465 (D 2-6=-430/238 3-6=-324/155, 4-6=-365/199 TOP CHORD BOT CHORD WEBS FORCES.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MVFRS (envelope) gable end zone and C-C Exterior(ZE) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 5-0-0. Exterior(2R) 5-0-0 to 1-0-0, Interior(1) 1-0-0 to 5-0-0, Interior(1) 1-0-0 to 1-0-0, Interior(1) 1-0-0, Interio



NARAINO - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-1473 rev. 519220 BEFORE USE.
Design varied for use only with Mittee Month and the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate his design into the overall building design. Bearing indicated is to prevent bucking of individual into use when and or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent obligate with possible personal injury and properly damage. For general guidance regarding the abhications, and pracing of trusses and truss systems; see ANSTIFFI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

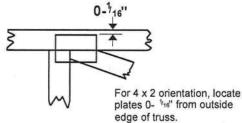


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

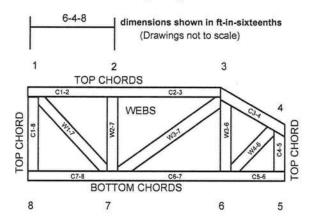
Design Standard for Bracing.

DSB-89: BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- 10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design,
- 14. Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.

AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

Page 1 of 1

MiTek USA, Inc.

MiTek USA, Inc.

Mitek USA, Inc.

Mitek USA, Inc.

Mitek Alliabe

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

	Brace Size for One-Ply Truss	Specified Continuous Rows of Lateral Bracing	2	2x4 I-Brace	2x6 I-Brace	2x8 I-Brace
	Brace for One-	Specified C Rows of Lat	-	2x4 T-Brace	2x6 T-Brace	2x8 T-Brace
ps.			Web Size	2x3 or 2x4	2x6	2x8

Nail Spacing

Nailing Pattern

Nail Size

T-Brace size

6" o.c.

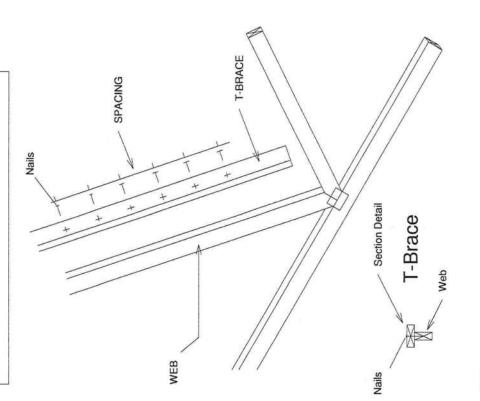
10d (0.131" X 3")

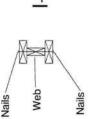
2x4 or 2x6 or 2x8

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

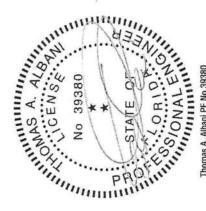
Brace Size for Two-Ply Truss Specified Continuous Rows of Lateral Bracing	2	2x4 T-Brace 2x4 I-Brace	2x6 T-Brace 2x6 I-Brace	2x8 T-Brace 2x8 I-Brace
, o c	Web Size	2x3 or 2x4 2x4	2x6 2x6	2x8 2x8

T-Brace / I-Brace must be same species and grade (or better) as web member.





I-Brace



Thomas A, Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610

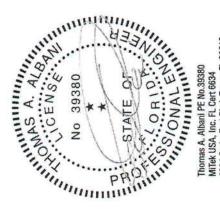
MII-SCAB-BRACE MiTek USA, Inc. *** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED. Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is Scab must cover full length of web +/- 6" SCAB-BRACE DETAIL impractical. (E) 1,2016 FINGWEERED BY MiTek USA, Inc. AUGUST

APPLY 2x SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.

Page 1 of 1

MAXIMUM WEB AXIAL FORCE = 2500 lbs MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE MINIMUM WEB GRADE OF #3 Section Detail Scab-Brace Web SCAB BRACE Nails

Scab-Brace must be same species grade (or better) as web member.



Thomas A. Albani PE No.39380 Milek USA, Inc. FI. Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

AUGUST 1, 2016

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

Page 1 of 1

MiTek USA, Inc.



THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE RECUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING

MiTek USA, Inc.

THE LOADS INDICATED.

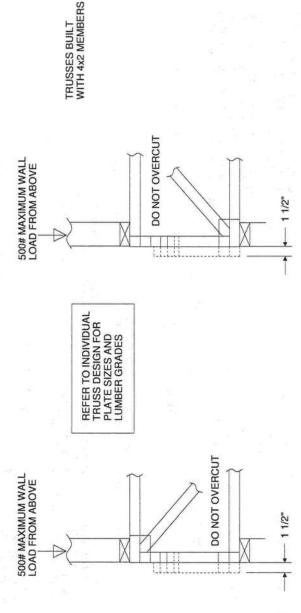
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
APPLYING REPAIR.

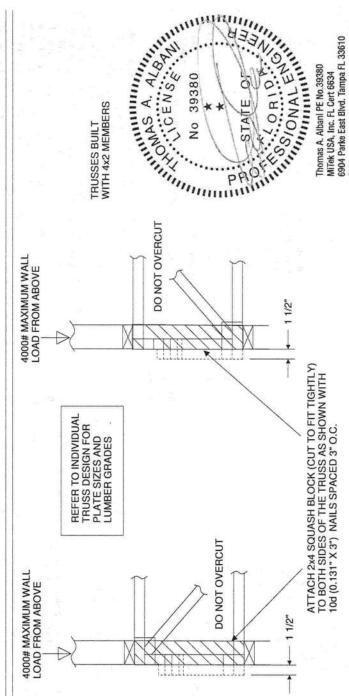
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
SUCH AS TO AVOID SPLITTING OF THE WOOD.

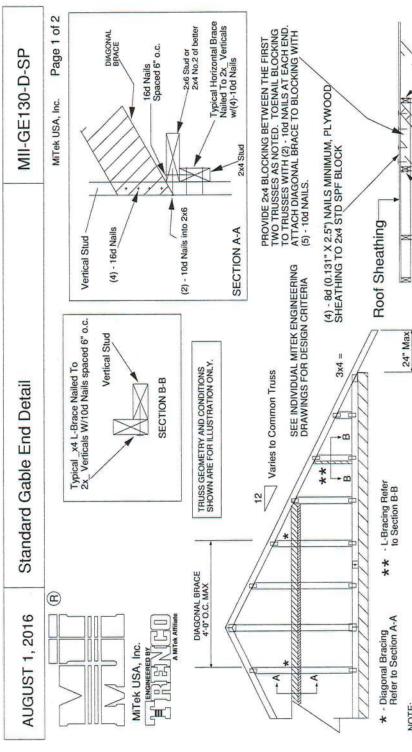
4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE AX. ORIENTATION ONLY.

6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.







Trusses @ 24" o.c.

2% DIÀGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

at 1/3 points Diag. Brace

if needed

HORIZONTAL BRACE (SEE SECTION A-A)

End Wall

- 10d NAILS

(2)

(2) - 10d NAILS

Max. 1-3

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SNB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A.A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS U240.
8. THIS DEFAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
 NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

2 DIAGONAL BRACES AT 1/3 POINTS 10-3-13 11-5-7 8-5-1 DIAGONAL 6-10-8 7-1-3 9-7-9 Maximum Stud Length 2x4 L-Brace 9-6-9 5-0-5 4-1-1 L-Brace 2-10-11 3-6-8 4-1-1 1x4 Without Brace 2-9-11 3-9-13 3-5-4 Spacing 2x4 SP No. 3 / Stud 16" O.C. 12" O.C. 2x4 SP No. 3 / Stud 24" O.C. 2x4 SP No. 3 / Stud Minimum Stud Size Species and Grade

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

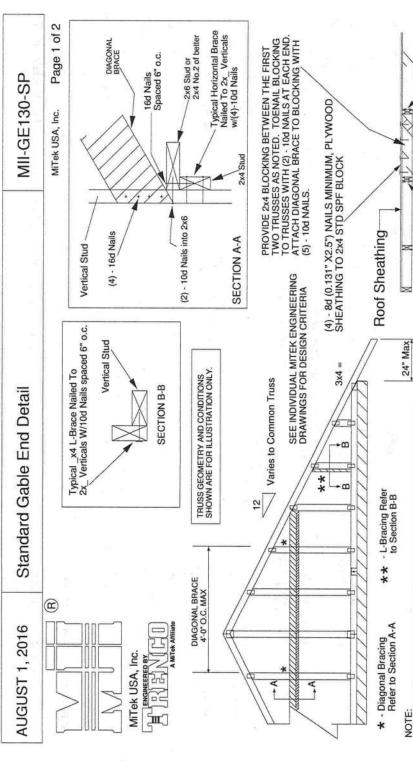
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE D
ASCE 7-02, ASCE 7-05 130 MPH.
ASCE 7-10 160 MPH
DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

No 39380

STATE OF LE

MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Thomas A. Albani PE No.39380



24" o.c.

Trusses @

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE (SEE SECTION A-A)

End Wall

at 1/3 points if needed Diag. Brace

- 10d NAILS

3

(2) - 10d NAILS

Max. 1-3

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

2 DIAGONAL BRACES AT 1/3 POINTS 12-1-6 11-0-11 DIAGONAL 8-0-15 6-1-5 7-4-1 Maximum Stud Length 2x4 L-Brace 6-3-8 5-5-6 4-5-6 1x4 L-Brace 3-10-4 3-1-12 4-5-6 Without Brace 3-0-10 3-8-0 4-0-7 Spacing Spacing 2x4 SP No. 3 / Stud 12" O.C. 16" O.C. 2x4 SP No. 3 / Stud 24" O.C. 2x4 SP No. 3 / Stud Minimum Stud Size and Grade Species

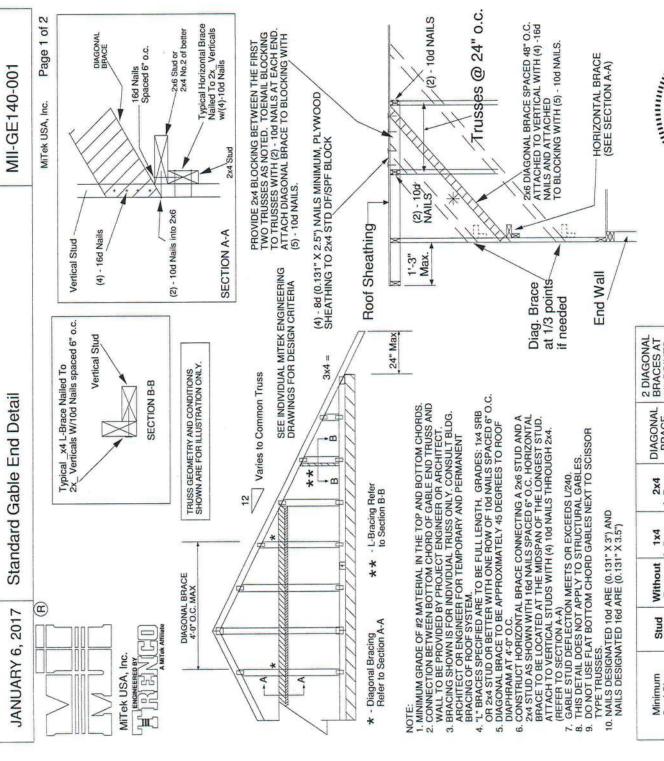
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE BO OF
ASCE 7-198, ASCE 7-02, ASCE 7-05 130 MPH
ASCE 7-10 160 MPH
STONARTION OF LOAD INCREASE: 1.60
CO

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

No 39380 STATE OF THE SOUND STATE OF THE STATE OF THE SOUND STATE OF THE SOUND STATE OF THE SOUND SOUN STATE STATE

Thomas A. Albani PE No.39380 Milek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

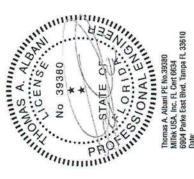


Minimum Stud Size	Stud Spacing	Stud Without 1x4 pacing Brace L-Brac	a)	2x4 DIA L-Brace Bi	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
species and Grade			Maximur	Maximum Stud Length	ngth	
2x4 DF/SPF Std/Stud 12" O.C. 3-10-1 3-11-7 5-7-2	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 DF/SPF Std/Stud 16" O.C. 3-3-14	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 DF/SPF Std/Stud 24" O.C. 2-8-9	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

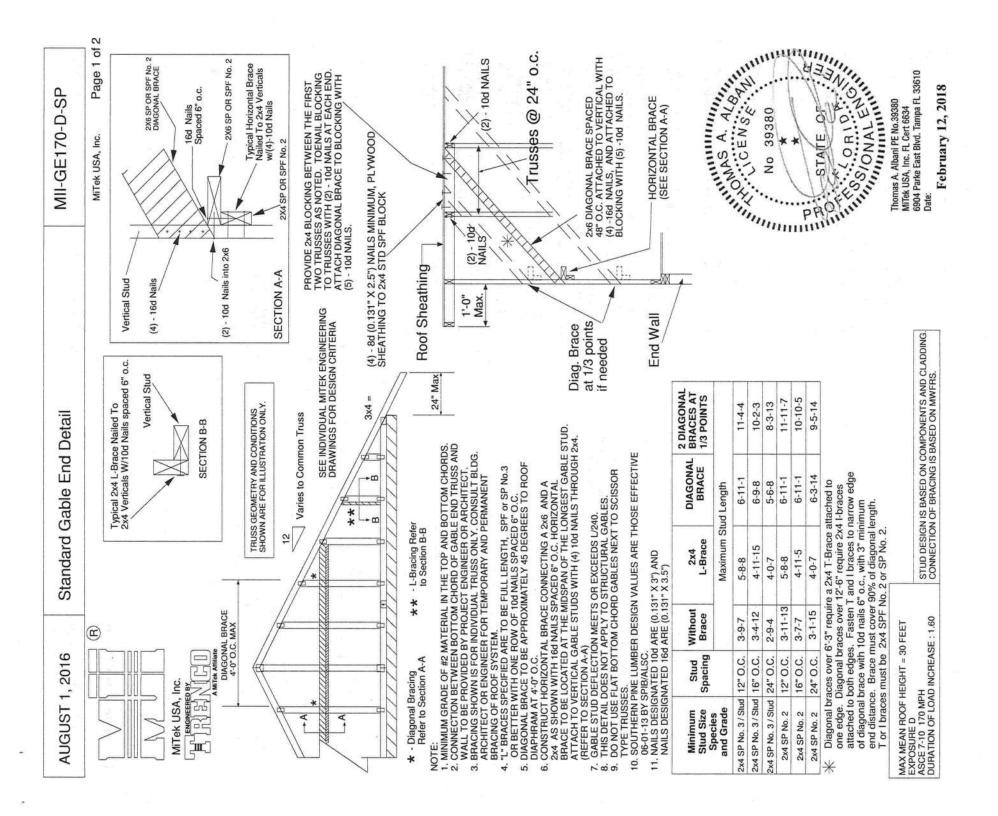
Diagonal braces over 6-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

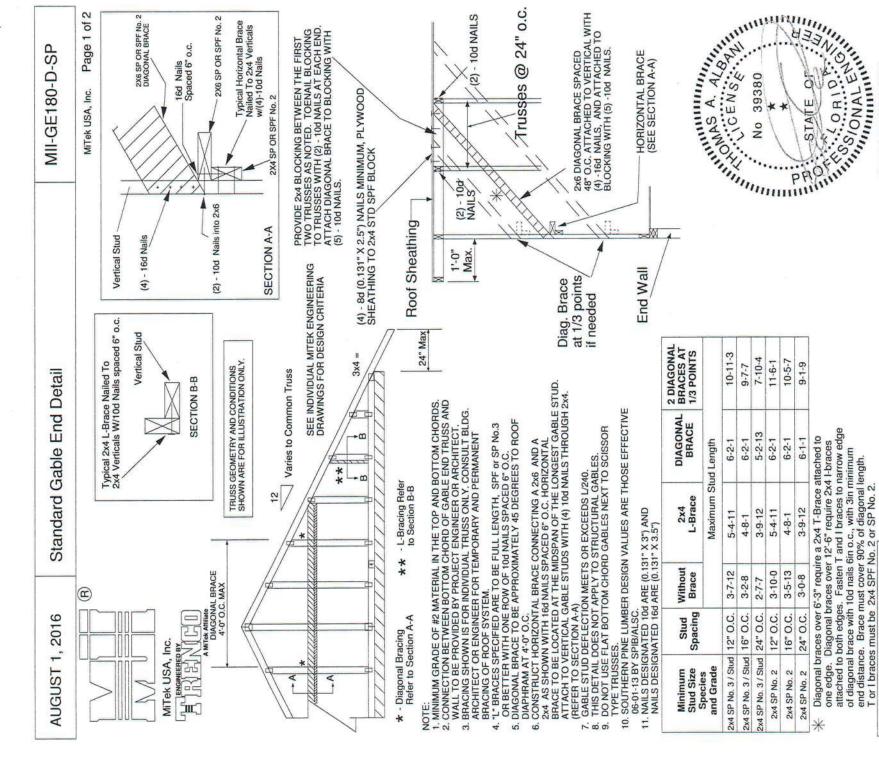
MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



January 19, 2018





MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

Thomas A. Albani PE No.39380

February 12, 2018

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH DURATION OF LOAD INCREASE: 1.60

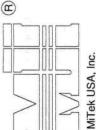
STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-10

MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24 " O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). DISTITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

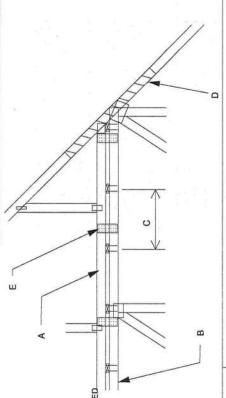


F R S S S

- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

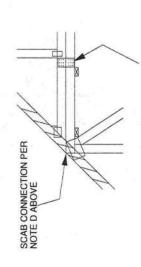
 E FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. WI (4) (1) 13" 1.5" JN IAILS PER MEMBERS. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE.

 (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PUBLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PUBLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA NAI-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.

This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint

and diagonal bracing requirements.

FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

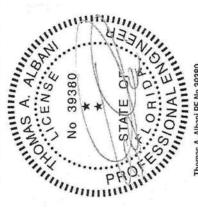
 2) ATTACH 2 x x 4-0° SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131° X 3°) NAILS SPACED 4° O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

 (MINIMUM 2X4)

 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS. GREATER THAN 4000 LBS.

 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. SOUCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

X



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610

2016 AUGUST 1,

TRUSS CONNECTION DETAIL STANDARD PIGGYBACK

MII-PIGGY-ALT 7-10

MiTek USA, Inc.

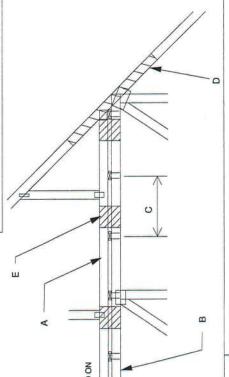
MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10
DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

(E)

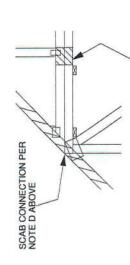
MiTek USA, Inc.

SEL PASSING

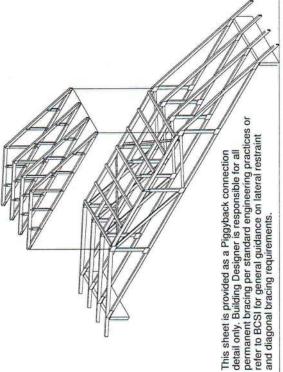


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

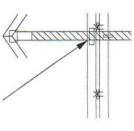
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSET'S AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

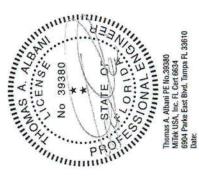
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

 2) ATTACH 2 x x 4-t° SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

 (MINIMUM 2X4)

 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

 4) FOR PIGGYBACK TRUSS TO MATCH BASE TRUSS. ONUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.



January 19, 2018

AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

Page 1 of

MiTek USA, Inc.



OTAL NI	TOTAL NUMBER OF			MAX	IMUM FO	RCE (lbs)	MAXIMUM FORCE (Ibs) 15% LOAD DURATION	D DURAT	NOI	
NAILS E	OF BREAK	X INCHES	S	SP	_	DF	S	SPF	_	生
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
56	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	22	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	99	48"	3657	5485	3346	5019	2829	4243	2898	4347

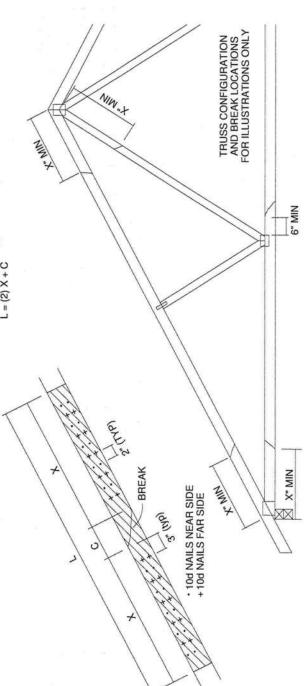
* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x. SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS)

THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:

L = (2) X + C



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- NOT THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED. THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING THE LOADS INDICATED.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 4. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 22, ORIENTATION ONLY.

 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

No 39380 STATE OF STA Thomas A. Albani PE No. 39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018

(B) 2016 AUGUST 1

LATERAL TOE-NAIL DETAIL

SP MII-TOENAIL

MiTek USA, Inc.

Page 1 of

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT THE BACK CORNER OF THE MEMBER END AS SHOWN.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

SPF-S

SPF 68.4

生

H

SP

DIAM.

OE-NAIL

63.4 73.8

72.6

85.6 80.6

> 135 162

3'2" LONG

9.66

108.8

84.5

50.3

59.0 57.6

> 60.3 64.6

75.9

81.4

148

3.25" LONG

58.9

67.9 69.5 74.5

74.2

.128 131

52.5 51.1

63.2

59.7

6.69 74.2 86.4

88.0 93.5

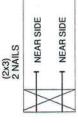
131

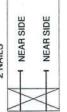
SINGLE SHEAR VALUES PER NDS 2001 (Ib/nail)

ENGINEERED BY
AMTOK AMT

MiTek USA, Inc.

SIDE VIEW

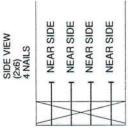




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,	NEAR SIDE	NEAR SIDE	NEAR SIDE
SIDE VIEW (2x4) 3 NAILS	I NEA	- NEA	I NEA
0000	>	*	



VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

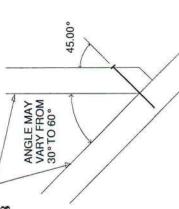
EXAMPLE:

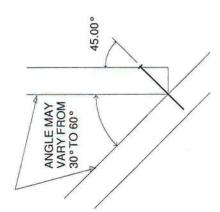
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD For load duration increase of 1.15: 3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

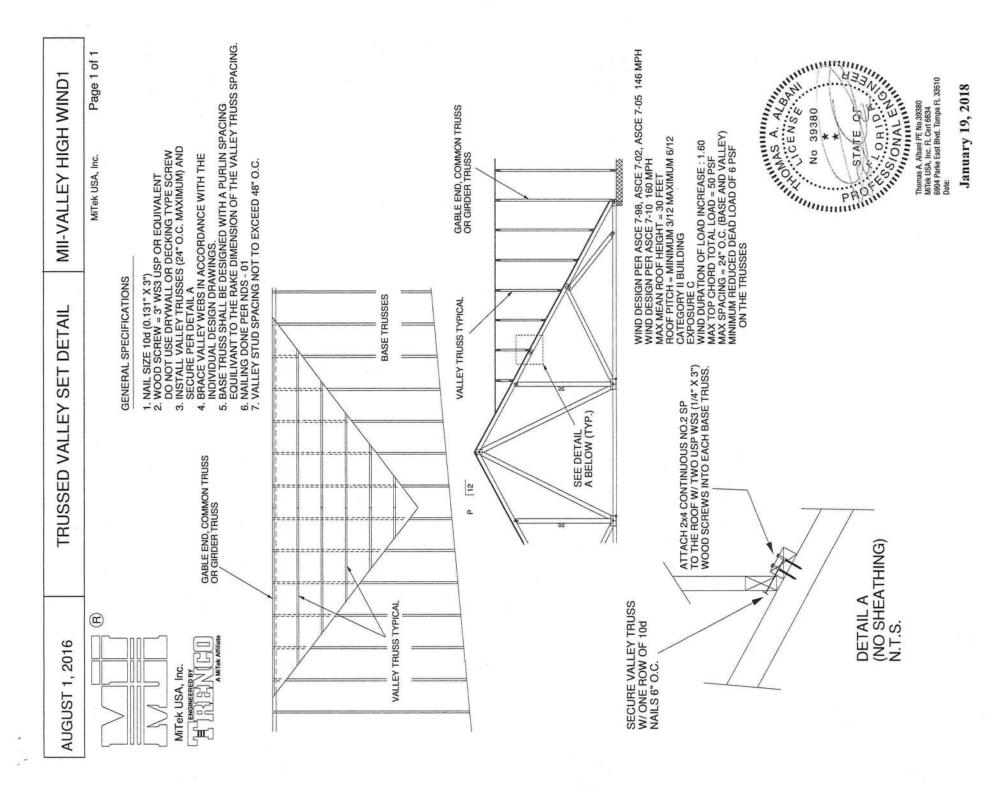


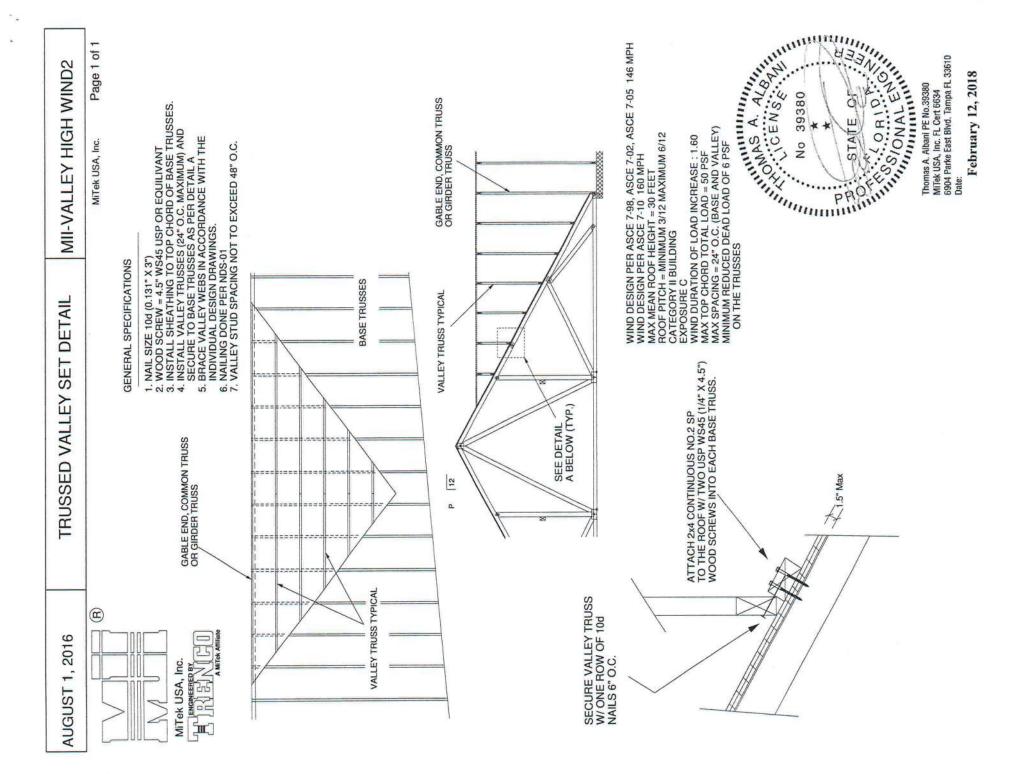
Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

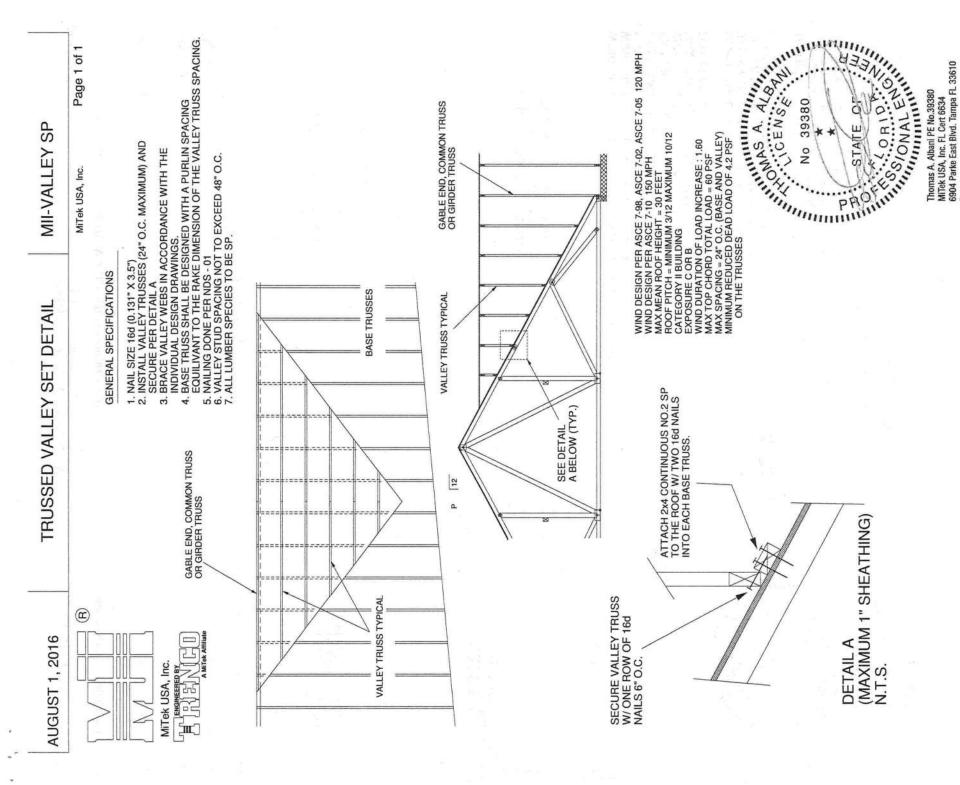
January 19, 2018 45.00° ANGLE MAY VARY FROM 30° TO 60°

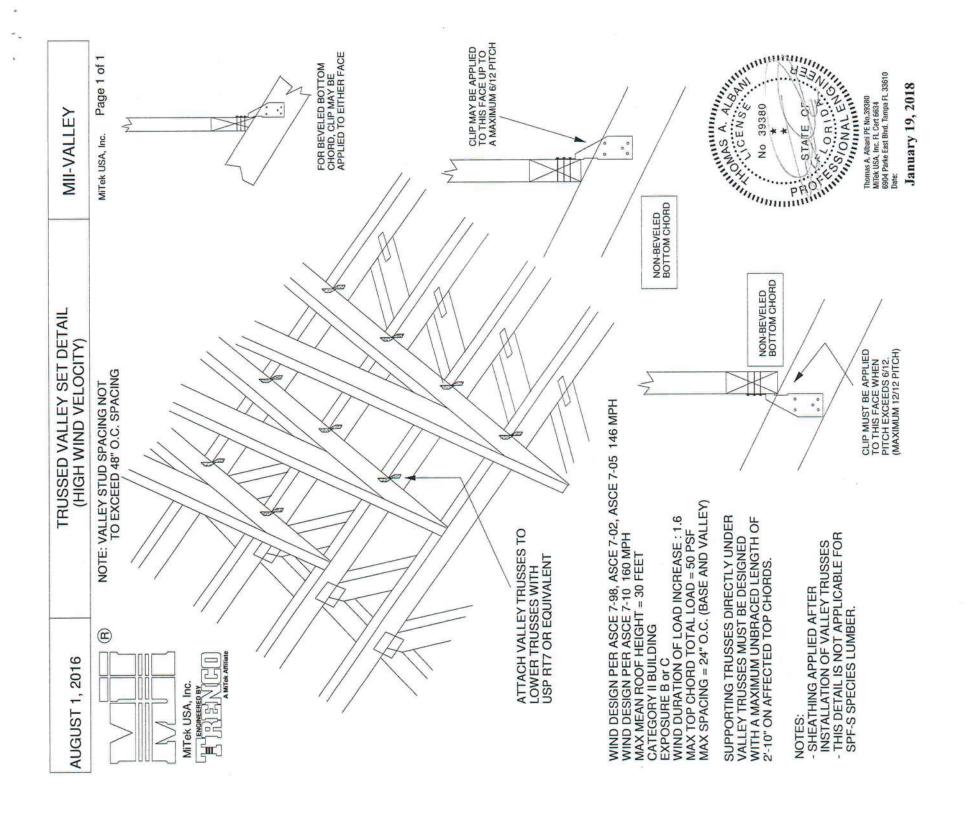


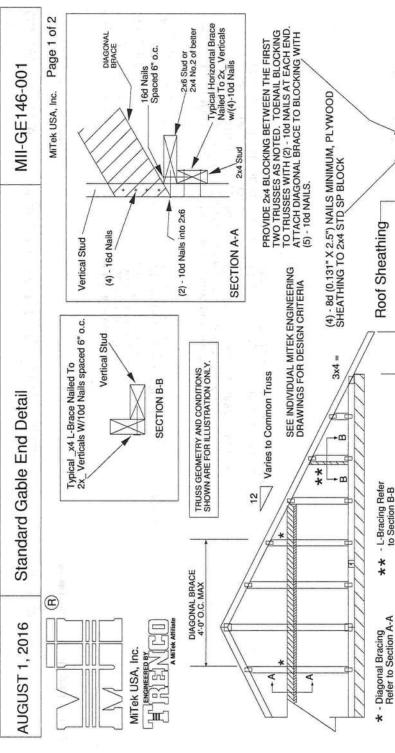












NOTE:

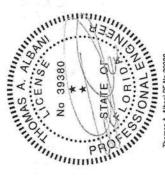
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER DO TRUSS AND WALL.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, GRADES:
2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. DIAPHRAM AT 4-0" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4.
(REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.
10. NAILS DESIGNATED 16d ARE (0.131" X 3.5")
NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species and Grade	Stud	Without Brace Maxim	hout 2x4 DIAGG ace L-Brace BRA Maximum Stud Length	DIAGONAL BRACE ength	2 DIAGONAL BRACES AT 1/3 POINTS
2x4 SP No 3/Stud 12" O.C. 3-11-3	12" O.C.	3-11-3	0-8-9		11-9-10
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11
2x4 SP No 3/Stud 24" O.C.	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7

Diagonal braces over 6:-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. *

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS. MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
CATEGORY II BUILDING
ASCE 7-03, ASCE 7-05
DURATION OF LOAD INCREASE: 1.60

Trusses @ 24" o.c. - 10d NAILS 2x6 DIÀGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS. HORIZONTAL BRACE (SEE SECTION A-A) (2) (2) - 10d NAILS Max. 1-3 End Wall Diag. Brace at 1/3 points if needed 24" Max



Thomas A. Albani PE No.39380 Milek U.SA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33510 Date:

January 19, 2018

OCTOBER 5, 2016

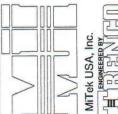
B

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1

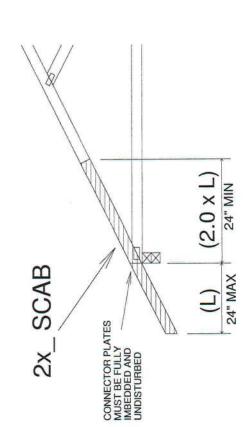


ENGINEERED BY

NOTES

TRUSS CRITERIA:
LOADING: 40-10-0-10
DURATION FACTOR: 1.15
SPACING: 24" O.C.
TOP CHORD: 2x4 OR 2x6
PITCH: 4/12 - 12/12
HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL
END BEARING CONDITION

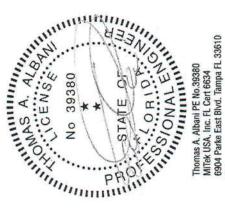
1. ATTACH 2x. SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES

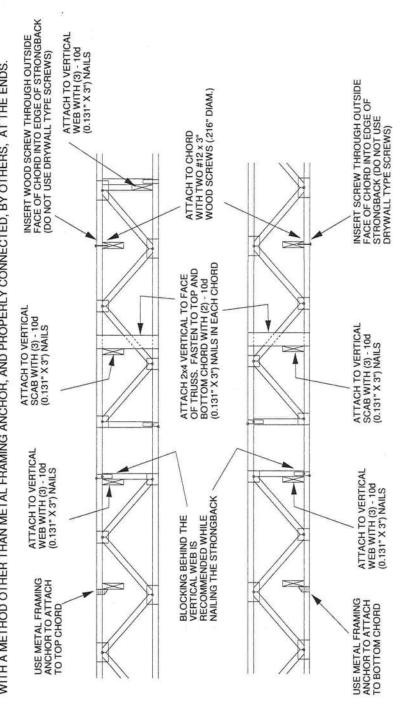


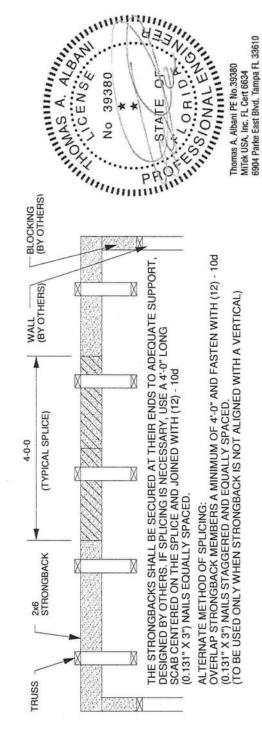
MII-STRGBCK TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET MiTek USA, Inc. RECOMMENDATIONS ALONG A FLOOR TRUSS LATERAL BRACING (H) 2016 ENGINEERED BY FINE STATE A MITTER ATTI MiTek USA, Inc. , AUGUST

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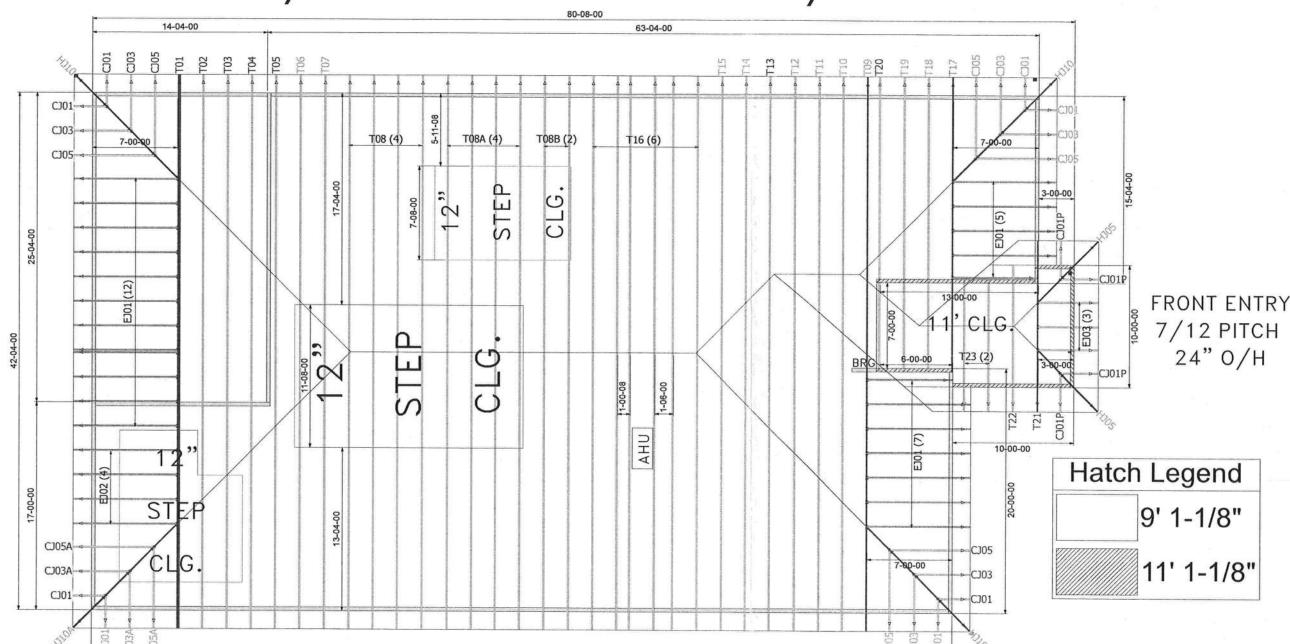
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NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW. NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.

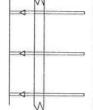




6/12 PITCH - 18" 0/H



THE ARROW HEAD AT THE END OF THE TRUSS ON THE TRUSS PLACEMENT PLAN (LAYOUT) CORRESPONDS WITH THE LEFT SIDE OF THE INDIVIDUAL TRUSS DRAWING, USE THIS AS AN DRIENTATION GUIDE WHEN SETTING THE TRUSSES ON THE STRUCTURE.



General Notes:

· Per ANSI/TPI 1-2002 all " Truss to Wall" connection are the responsibility of the Building Designer, not the Fruss Manufacturer.

Use Manufacturer's specifications for all hanger onnections unless noted otherwise.

· Trusses are to be 24" o.c. U.N.O.

· All hangers are to be Simpson or equivalent U.N.O. Use 10d x 1 1/2" Nails in hanger connections to single ply

Trusses are not designed to support brick U.N.O.
 Dimensions are Feet-Inches- Sixteenths

No back charges will be accepted by Builders FirstSource unless approved in writing first. 850-835-4541

ACQ lumber is corrisive to truss plates. Any ACQ lumber that comes in contact with truss plates (i.e. scabbed on tails) must have an approved barrier applied first.

Refer to BCSI-B1 Summary Sheet-Guide for handling, Installing and Bracing of Metal Plate Connected Wood Truss prior to and during truss installation.

It is the responsibility of the Contractor to ensure of the proper orientation of the truss placement plans as to the construction documents and field conditions of the structure orientation. If a reversed or flipped layout is required, it will be supplied at no extra cost by Builders

It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbing drops, can lights, ect..., so the trusses do not interfere with these

All common framed roof or floor systems must be designed as to NOT impose any loads on the floor trusses below. The floor trusses have not been designed to carry any additional loads from above

This truss placement plan was not created by an engineer, but rather by the Builders FirstSource staff and is solely to be used as an installation guide and does not require a seal. Complete truss engineering and analysis can be found on the truss design drawings which may be sealed by the truss design engineer

Gable end trusses require continuous bottom chord bearing. Refer to local codes for wall framing

Although all attempts have been made to do so, trusses may not be designed symmetrically. Please refer to the individual truss drawings and truss placement plans for proper orientation and placement.



Lake City PHONE: 386-755-6894 FAX: 386-755-7973

Jacksonville PHONE: 904-772-6100 FAX: 904-772-1973

Tallahassee PHONE: 850-576-5177

AARON SIMQUE HOMES

Lot 54 Preserve

Model:	
2281	
Date:	

Drawn By:	Original Ref#:
KLH	2802399
Floor 2 Job#:	Roof Job #:
N/A	2802399
	KLH Floor 2 Job#:

MITEK PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2

70-08-00