

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 2253489 - SIMQUE - 1995 MODEL

MiTek USA, Inc.

Site Information:

6904 Parke East Blvd. Tampa, FL 33610-4115

Customer Info: Aaron Simque Homes Project Name: Spec House Model: 1995

Subdivision: The Preserve at Laurel Lake

Lot/Block: 134 Address: TBD, TBD City: Columbia Cty

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Truss Name Date

Wind Code: ASCE 7-10 Roof Load: 37.0 psf

Wind Speed: 130 mph Floor Load: N/A psf

This package includes 23 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

T19767913 T10

Seal#

No.

23

No.	Seal#	Truss Name	Date
123456789111234567890	T19767891 T19767892 T19767893 T19767895 T19767896 T19767897 T19767898 T19767890 T19767900 T19767901 T19767903 T19767905 T19767905 T19767907 T19767907 T19767907 T19767907 T19767909 T19767909 T19767909 T19767909 T19767909	CJ01 CJ03 CJ05 EJ01 EJ02 EJ03 HJ08 HJ10 PB01 PB02 PB02G T01 T02 T03 T04 T05 T06 T07 T08 T08G	3/23/20 3/23/20
21 22	T19767911 T19767912	T09 T09G	3/23/20 3/23/20



3/23/20

The truss drawing(s) referenced above have been prepared by MiTek USA. Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

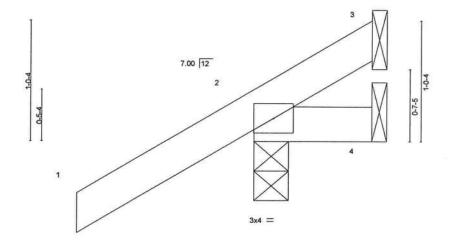
March 23,2020



Job Truss Truss Type Qty Ply SIMOUE - 1995 MODEL T19767891 2253489 **CJ01** Jack-Open 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:22 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-460wh4cXU8Fad7gwpZdMaLgMJP6dKArapLvd8YzY167

1-0-0

Scale = 1:9.4



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.00	7	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.04	Vert(CT)	0.00	7	>999	180	VIII.	
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MP			_	3,111		Weight: 6 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=64(LC 12)

Max Uplift 3=-5(LC 1), 2=-105(LC 12), 4=-25(LC 19) Max Grav 3=8(LC 16), 2=179(LC 1), 4=26(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=105.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

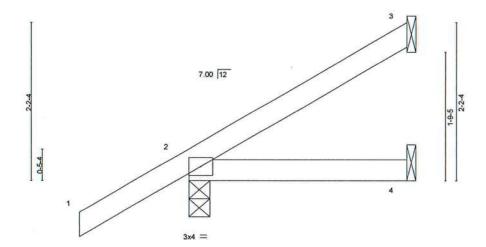
ANSITYPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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-1-6-0 1-6-0

Scale = 1:15.3



LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	0.01	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.11	Vert(CT)	-0.01	4-7	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MP						Weight: 12 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

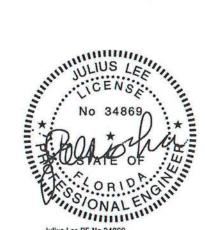
Max Horz 2=120(LC 12)

Max Uplift 3=-62(LC 12), 2=-89(LC 12), 4=-26(LC 9) Max Grav 3=66(LC 19), 2=210(LC 1), 4=51(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

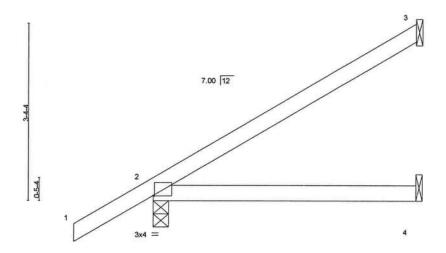
March 23,2020

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Composately Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type SIMQUE - 1995 MODEL Qty Ply T19767893 2253489 **CJ05** Jack-Open 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:23 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-YJalvQcAFSNREHF6NH8b6ZCUKoM_3d5j2?eAg_zY166

Scale = 1:21.0



5-0-0

LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	0.08	4-7	>711	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.35	Vert(CT)	0.07	4-7	>820	180	SAMETOR	(50) (100 (100 (100 (100 (100 (100 (100 (1
BCLL	0.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MP	100000000000000000000000000000000000000		1.50	200000	153033	Weight: 19 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

(size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=177(LC 12)

Max Uplift 3=-113(LC 12), 2=-100(LC 12), 4=-47(LC 9) Max Grav 3=124(LC 19), 2=276(LC 1), 4=89(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (jt=lb) 3=113

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Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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March 23,2020

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ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply SIMQUE - 1995 MODEL T19767894 Jack-Partial 2253489 EJ01 10 Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244. 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:24 2020 Page 1 $ID: RGwSt4cPREnm5p9yfgzNgAycgdx-0V7g6mdo0IVIsRqJx_fqfmIY1Cc1o4LtGfOjCQzY165$ 7-0-0 Scale = 1:26.8 7.00 12 0-5-4 4x4 = 7-0-0 LOADING (psf) SPACING-2-0-0 DEFL. (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 0.79 Vert(LL) 0.33 >251 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.75 Vert(CT) 0.29 >292 180 0.0 BCLL Rep Stress Incr YES WB 0.00 -0.02 Horz(CT) n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 25 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

> (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=161(LC 12)

Max Uplift 3=-106(LC 12), 2=-98(LC 9), 4=-64(LC 9)

Max Grav 3=172(LC 19), 2=346(LC 1), 4=127(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4 except (telb) 3=106.



Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.



Job Truss Truss Type Qty SIMOUE - 1995 MODEL Ply T19767895 2253489 **EJ02** Jack-Partial 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:25 2020 Page 1 ID:RGwSI4cPREnm5p9yfgzNgAycgdx-Uhh2K6eQn3d9UbPVUiA3B_lkwczcXXb0VJ7HkszY164 Scale = 1:26.8 7.00 12 0-5-4 -6 2x4 || 3x4 = 4-8-8 Plate Offsets (X,Y)-[3:0-4-0,0-2-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in l/defl PLATES GRIP (loc) 1/d TCLL 20.0 Plate Grip DOL 1.25 TC 0.72 Vert(LL) 0.17 3-5 >485 240 244/190 MT20 TCDL 7.0 1.25 Lumber DOL BC 0.66 -0.30 Vert(CT) 3-5 >279 180 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.17 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MR Weight: 27 lb FT = 20%LUMBER-BRACING-TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS.

BOT CHORD

(size) 4=Mechanical, 2=0-3-8, 5=Mechanical

Max Horz 2=161(LC 12)

2x4 SP No.2 *Except*

3-6: 2x4 SP No.3

Max Uplift 4=-92(LC 12), 2=-55(LC 12), 5=-13(LC 12)

Max Grav 4=168(LC 19), 2=351(LC 1), 5=123(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2, 5.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty SIMQUE - 1995 MODEL T19767896 2253489 EJ03 Jack-Open Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:26 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-yuFRXSf2YNl?5k_h2PhlkBq060QLG_DAkztqHJzY163 2x4 || Scale = 1:21.0 7.00 12 0-1-12 0.54 6 3x4 2x4 || LOADING (psf) SPACING-2-0-0 CSI. DEFL PLATES in (loc) I/defi L/d GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.31 -0.03 Vert(LL) >999 240 244/190 6-9 MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.25 Vert(CT) -0.05 6-9 >999 180 0.0 BCLL Rep Stress Incr YES WB 0.04 Horz(CT) 0.01 n/a n/a Code FBC2017/TPI2014

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

10.0

REACTIONS. (size) 2=0-3-8, 6=Mechanical

Max Horz 2=177(LC 12)

Max Uplift 2=-94(LC 12), 6=-125(LC 12)

Max Grav 2=268(LC 1), 6=194(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-MP

- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 6=125.



Weight: 23 lb

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

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Job Truss Truss Type Qty SIMQUE - 1995 MODEL T19767897 2253489 HJ08 Diagonal Hip Girder Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:27 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-Q4ppknfgJgtsjuYtc7CXHPN8GQku?QxJzdcOplzY162 Scale = 1:20.7 2x4 || 4.95 12 12 2x4 > 3 0-3-8 0-5-4 13 3x6 = 3x4 = 7-0-2 7-0-2 LOADING (psf) SPACING-CSI. DEFL. in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.25 TC 0.48 Vert(LL) 0.06 7-10 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 0.36 BC Vert(CT) -0 10 7-10 >828 180 0.0 BCLL Rep Stress Incr NO WB 0.07 Horz(CT) 0.00 2 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 34 lb FT = 20% LUMBER-BRACING-

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-4-9, 6=Mechanical

Max Horz 2=176(LC 8)

Max Uplift 2=-264(LC 4), 6=-201(LC 8) Max Grav 2=390(LC 1), 6=245(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-334/171 BOT CHORD 2-7=-258/252

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 84 lb down and 76 lb up at 1-6-1, 84 lb down and 76 lb up at 1-6-1, and 103 lb down and 52 lb up at 4-4-0, and 103 lb down and 52 lb up at 4-4-0 on top chord and 26 lb down and 54 lb up at 1-6-1, 26 lb down and 54 lb up at 1-6-1, and 18 lb down and 34 lb up at 4-4-0, and 18 lb down and 34 lb up at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 6-8=-20

Concentrated Loads (lb)

Vert: 14=-4(F=-2, B=-2)

No 34869 AND JONA

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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March 23,2020

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty SIMQUE - 1995 MODEL T19767898 2253489 **HJ10** Diagonal Hip Girder 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:28 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-uGNBy7gI4_0jL274AqimpcwHFp0LkoHSBHMxLBzY161 9-10-1 Scale = 1:26.3 4.95 12 3x4 = 3 0-5-4 13 6 7 2x4 || 3x4 = 53x4 = 4-6-0 LOADING (psf) SPACING-CSI DEFL. PLATES GRIP 2-0-0 I/defl L/d in (loc) Plate Grip DOL 0.59 244/190 TCLL 20.0 TC Vert(LL) 0.11 >999 1.25 6-7 240 MT20 TCDL 7.0 1.25 BC Lumber DOL 0.60 Vert(CT) -0.126-7 >999 180 BCLL 0.0 Rep Stress Incr WB 0.39 NO Horz(CT) -0.01 5 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 44 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

REACTIONS.

(size) 4=Mechanical, 2=0-4-9, 5=Mechanical Max Horz 2=233(LC 8)

Max Uplift 4=-151(LC 8), 2=-410(LC 4), 5=-280(LC 5)

Max Grav 4=150(LC 1), 2=526(LC 1), 5=298(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-728/530

BOT CHORD 2-7=-607/607, 6-7=-607/607 WEBS 3-7=-143/280, 3-6=-652/652

NOTES-

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=151, 2=410, 5=280.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 84 lb down and 76 lb up at 1-6-1, 84 lb down and 76 lb up at 1-6-1, 103 lb down and 52 lb up at 4-4-0, 103 lb down and 52 lb up at 4-4-0, and 135 lb down and 113 lb up at 7-1-15, and 135 lb down and 113 lb up at 7-1-15 on top chord, and 58 lb down and 54 lb up at 1-6-1, 58 lb down and 54 lb up at 1-6-1, 20 lb down and 34 lb up at 4-4-0, 20 lb down and 34 lb up at 4-4-0, and 42 lb down and 62 lb up at 7-1-15, and 42 lb down and 62 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

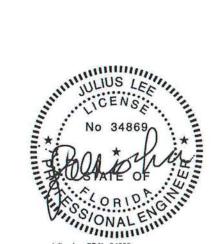
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=-4(F=-2, B=-2) 12=-74(F=-37, B=-37) 14=-57(F=-29, B=-29)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-11-13 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020



Truss Truss Type Qty SIMQUE - 1995 MODEL T19767899 2253489 **PB01** Piggyback 1 Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:29 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244. ID:RGwSt4cPREnm5p9yfgzNgAycgdx-NTxZ9Thwrl8ayCiGjYF?MqSZ4DUHTL5cQx5UtezY160 Scale = 1:13.8 TOP CHORD UNDER PIGGYBACKS TO BE LATERALLY BRACED BY PURLINS AT 2-0-0 OC. MAX. **TYPICAL** 4x4 = 4x4 = 3 7.00 12 0-4-5 2x4 = 2x4 || 2x4 || 2x4 =

TCLL 20.0 Plate Grip DOL 1.25 TC 0.14 Vert(LL) 0.00 TCDL 7.0 Lumber DOL 1.25 BC 0.04 Vert(CT) 0.00 BCLL 0.0 * Rep Stress Incr YES WB 0.03 Horz(CT) 0.00						7-0-0 7-0-0						4	
BCDL 10.0 Code FBC2017/TPI2014 Matrix-P	Pla Lur Re	Plate Grip DOL umber DOL tep Stress Incr	1.25 1.25 YES	TC BC WB	0.04 0.03	Vert(LL)	0.00	(loc) 5 5 5	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 20%	

LUMBER-

Job

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 5-3-11.

(lb) - Max Horz 2=25(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 5, 8, 7 Max Grav All reactions 250 lb or less at joint(s) 2, 5, 8, 7

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5, 8, 7.
- 8) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

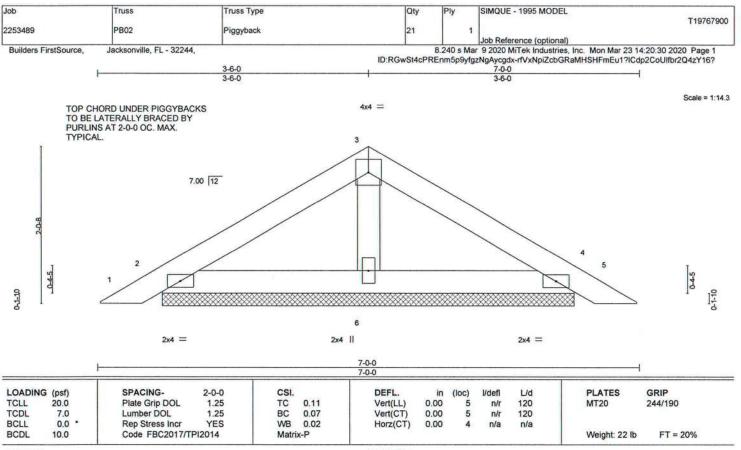


Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 2=5-3-11, 4=5-3-11, 6=5-3-11

Max Horz 2=-46(LC 10)

Max Uplift 2=-43(LC 12), 4=-47(LC 13), 6=-16(LC 12) Max Grav 2=134(LC 1), 4=134(LC 1), 6=184(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

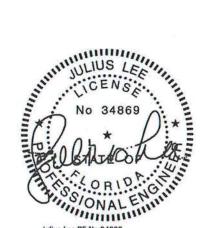
NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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March 23,2020

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Qty Job Truss Truss Type SIMQUE - 1995 MODEL T19767901 2253489 PB02G GABLE 1 Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:31 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-Jr3Ka9jBNvOICWsfryHURFYwd1AjxFqvtFabyWzY16_ 2-11-1 5-10-2 Scale = 1:12.5 4x4 = 3 7.00 12 2 0-4-5 9-1-1p 6 2x4 = 2x4 11 2x4 = 5-10-2 LOADING (psf) SPACING-2-0-0 CSI. DEFL **PLATES** GRIP (loc) I/defl

Vert(LL)

Vert(CT)

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.00

0.00

0.00

120

120

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/r

n/r

n/a

5

MT20

Structural wood sheathing directly applied or 5-10-2 oc purlins.

Weight: 18 lb

244/190

FT = 20%

LUMBER-

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

20.0

70

0.0

10.0

OTHERS 2x4 SP No.3

REACTIONS. (size) 2=

(size) 2=4-1-13, 4=4-1-13, 6=4-1-13

Plate Grip DOL

Rep Stress Incr

Code FBC2017/TPI2014

Lumber DOL

Max Horz 2=38(LC 11)

Max Uplift 2=-36(LC 12), 4=-40(LC 13), 6=-11(LC 12) Max Grav 2=112(LC 1), 4=112(LC 1), 6=141(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1.25

1 25

YES

TC 0.07

BC

WB 0.02

Matrix-P

0.04

- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4, 6.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

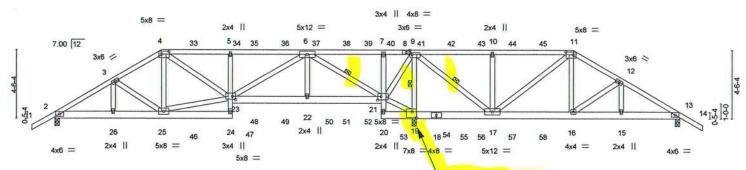
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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss		Truss Type	, , , , , , , , , , , , , , , , , , , 	Qty	Ply	SIMQUE - 1995 MODE	EL		52-00 San Mar Stee
2253489	T01		HIP GIRDER		1		1			T19767902
							Job Reference (options	BI)		
Builders FirstSource,	Jacksonville, FL	- 32244.				8.240 s M	ar 9 2020 MiTek Industrie	es, Inc. Mon Ma	ar 23 14:20:34	2020 Page 1
					ID:RGwSt4d		fgzNgAycgdx-jQkSDBl3fq			
_c 1-6-0 , 3-10-4	7-0-0	11-8-0	16-6-4	21-4-8	23-6-4	28-8-4	34-0-0	37-1-12	41-0-0	42-6-0,
1-6-0 3-10-4	3-1-12	4-8-0	4-10-4	4-10-4	2-1-12	5-2-0	5-3-12	3-1-12	3-10-4	1-6-0

Scale = 1:72.8



SUPPLEMENTARY BEARING PLATES, SPECIAL ANCHORAGE, OR OTHER MEANS TO ALLOW FOR THE MINIMUM REQUIRED SUPPORT WIDTH (SUCH AS COLUMN CAPS, BEARING BLOCKS, ETC.)
ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER OR THE BUILDING DESIGNER.

Structural wood sheathing directly applied or 3-4-2 oc purlins.

6-21, 9-19, 9-17

Rigid ceiling directly applied or 4-11-6 oc bracing.

1 Row at midpt

	3-10-4	7-0-0 1 1	1-8-0	16-6-4	21-4-8	23-6-4	28-8-4	1	34-0-0	37-1-12	41-0-0
	3-10-4	3-1-12	4-8-0	4-10-4	4-10-4	2-1-12	5-2-0		5-3-12	3-1-12	3-10-4
Plate Offse	ets (X,Y)-	[4:0-6-0,0-2-4], [8:0-2-1	12,0-1-8], [9:0-1	-12,0-1-8], [11	:0-6-0,0-2-4], [19	9:0-3-8,0-4-12	. [21:0-2-12,0-2	-12], [23	0-2-8,0-2-8]		
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L∕d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.78	Vert(LL)	0.13 22-23	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.20 22-23	>999	180	137110000000000000000000000000000000000	
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.91	Horz(CT)	0.06 19	n/a	n/a		
BCDL	10.0	Code FBC2017	/TPI2014	Matrix	x-MS	12. 12				Weight: 276	lb FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x6 SP No.2 *Except*

5-24,7-20: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 19=0-3-8 (req. 0-5-1), 13=0-3-8

Max Horz 2=124(LC 7)

Max Uplift 2=-647(LC 8), 19=-2416(LC 5), 13=-542(LC 4) Max Grav 2=1356(LC 19), 19=4316(LC 1), 13=773(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2195/1117, 3-4=-2040/1133, 4-5=-2315/1186, 5-6=-2331/1190, 6-7=-722/1259,

7-9=-746/1308, 9-10=0/295, 10-11=0/295, 11-12=899/864, 12-13=-1085/907 2-26=-981/1855, 25-26=-981/1855, 5-23=-531/312, 22-23=-488/1246, 21-22=-488/1246,

7-21=-386/227, 17-19=-1976/1049, 16-17=-672/767, 15-16=-732/898, 13-15=-732/898

3-25=-287/162, 4-25=-257/383, 23-25=-861/1554, 4-23=-263/743, 6-23=-820/1291,

6-22=-23/471, 6-21=-2932/1440, 19-21=-2054/1111, 9-21=-531/1256, 9-19=-2946/1635,

9-17=-1536/2394, 10-17=-631/379, 11-17=-1082/662, 11-16=-492/671

NOTES-

WEBS

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- WARNING: Required bearing size at joint(s) 19 greater than input bearing size.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=647, 19=2416, 13=542.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 116 lb up at 7-0-0, 126 lb down and 112 lb up at 9-0-12, 126 lb down and 112 lb up at 11-0-12, 132 lb down and 99 lb up at 13-0-12, 132 lb down and 99 lb up at 15-0-12, 132 lb down and 99 lb up at 17-0-12, 132 lb down and 99 lb up at 19-0-12, 132 lb down and 99 lb up at 20-6-0, 126 lb down and 112 lb up at 21-11-4, 126 lb down and 112 lb up at 23-11-4, 126 lb down and 112 lb up at 25-11-4, 126 lb down and 112 lb up at 27-11-4, 126 lb down and 112 lb up at 29-11-4, and 126 lb down and 112 lb up at 31-11-4, and 230 lb down and 284 lb up at 34-0-0 on top chord, and 333 lb down and 395 lb up at 7-0-0, 87 lb down and 84 lb up at 9-0-12, 87 lb down and 84 lb up at 11-0-12, 83 lb down and 33 lb up at 13-0-12, 83 lb down and 33 lb up at 15-0-12, 83 lb down and 33 lb up at 17-0-12, 83 lb down and 33 lb up at 19-0-12, 83 lb down and 33 lb and 84 lb up at 23-11-4, 87 lb down and 84 lb up at 25-11-4, 87 lb down and 84 lb up at 27-11-4, 87 lb down and 84 lb up at 29-11-4, and 87 lb down and 84 lb up at 31-11-4, and 333 lb down and 395 lb up at 33-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. Continued on page 2

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March 23,2020

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Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	SIMQUE - 1995 MODEL	720 72720 00
2253489	T01	HIP GIRDER	1	1		T19767902
		And the state of t			Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:34 2020 Page 2 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-jQkSDBl3fqmt3zbEW5qB3t9GjE5E8NaLaDpFYrzY15x

NOTES-

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)

Vert: 1-4=-54, 4-11=-54, 11-14=-54, 24-27=-20, 21-23=-20, 20-30=-20

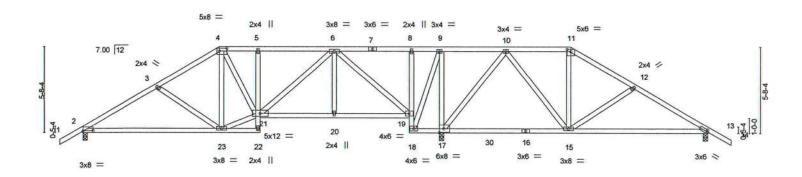
Concentrated Loads (lb)

Vert: 4=-110(B) 11=-183(B) 25=-333(B) 16=-333(B) 33=-110(B) 34=-110(B) 35=-100(B) 36=-100(B) 37=-100(B) 38=-100(B) 39=-100(B) 40=-110(B) 42=-110(B) 42=-110(B) 43=-110(B) 44=-110(B) 45=-110(B) 45=-11



Job		Trus	S		Truss T	ype				Qty	Ply	SIMQUE - 19	95 MODEL		200
2253489		T02			HIP					1		1			T19767903
												Job Reference	ce (optional)		
Builders FirstS	ource,	Jackso	nville, FL - 3	2244,						8	.240 s N	Mar 9 2020 MiTe	k Industries, Inc. Mor	Mar 23 14:20:3	5 2020 Page 1
									ID:RGwSt4	CPREnm	5p9yfgz	NgAycgdx-BclqC	XmhQ8ukg7AQ4oM	Qb5iYseNwtsDU	osYp4HzY15w
r1-6-0	4-11-4	1	9-0-0	11-8-0	9	16-6-4	1	21-4-8	, 23-6-4	27-	8-4	32-0-0	36-0-12	41-0-0	42-6-0
1-6-0	4-11-4		4-0-12	2-8-0		4-10-4	4	4-10-4	2-1-12	4-	2-0	4-3-12	4-0-12	4-11-4	1-6-0

Scale = 1:72.8



	1	9-0-0	11-8-0	16-6-4	21-4-8	, 23-6-4	32-0	0-0	¥	41-0-0	70
		9-0-0	2-8-0	4-10-4	4-10-4	2-1-12	8-5-	-12		9-0-0	
Plate Offse	ets (X,Y)-	[2:0-8-0,0-0-6], [4:0-6-0	,0-2-4], [11:0	-3-0,0-1-12], [13	:0-0-15,0-1-8],	17:0-3-8,0-3-0]					
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.37	Vert(LL)	0.20 15-29	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.67	Vert(CT)	-0.28 23-26	>995	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.82	Horz(CT)	0.02 17	n/a	n/a		
BCDL	10.0	Code FBC2017/	TPI2014	Matri	x-MS	5.00				Weight: 247 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

5-22,8-18: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS. (size) 2=0-3-8, 17=0-3-8, 13=0-3-8

Max Horz 2=-153(LC 10)

Max Uplift 2=-192(LC 12), 17=-581(LC 9), 13=-307(LC 8) Max Grav 2=848(LC 23), 17=1772(LC 1), 13=596(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1142/588, 3-4=-917/505, 4-5=-889/549, 5-6=-899/554, 9-10=-103/453, TOP CHORD

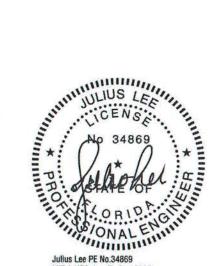
10-11=-360/642, 11-12=-437/693, 12-13=-664/801 2-23=-374/953, 20-21=-151/608, 19-20=-151/608, 18-19=-888/421, 17-18=-453/264, **BOT CHORD**

13-15=-600/544

WEBS 3-23=-345/233, 21-23=-174/791, 4-21=-144/329, 6-21=-150/394, 6-19=-990/422,

9-18=-307/868, 9-17=-969/398, 10-17=-770/629, 10-15=-384/451, 12-15=-341/299

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=192, 17=581, 13=307.



Structural wood sheathing directly applied or 5-3-11 oc purlins.

Rigid ceiling directly applied or 5-7-12 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020

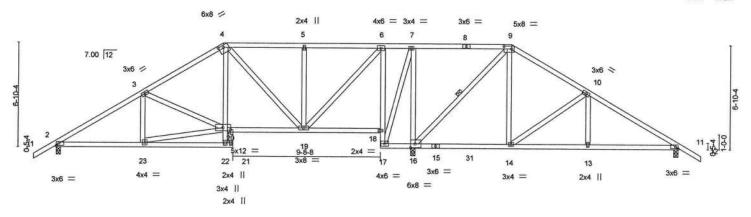
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTIEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord memors only. Additional temporary and permanent bracing is always required for stability and to prevent localizes with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITYPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandia, VA 22314.



Job		Truss		Tru	ss Type			Qty	Ply	SIMQUE - 1995 MODEL		
2253489		T03		HIP				1		1		T19767904
Builders FirstSc	ource.	Jacksonvill	e. FL - 32244.						8 240 s M	Job Reference (optional) ar 9 2020 MiTek Industries, Inc. M	Mon Mar 23 14:20:	37 2020 Page 1
								ID:RGwSt4cP		NgAycgdx-8?QbrCnyyl8SwRJoBD		
r1-6-0	5-8-8		11-0-0	1173-8	16-4-0	-	21-4-8	23-6-4	30-0-0	35-0-9	41-0-0	42-6-0
1-6-0	5-8-8	1	5-3-8	0-3-8	5-0-8	7,0%	5-0-8	2-1-12	6-5-12	5-0-9	5-11-7	1-6-0

Scale = 1:72.8



1	5-8-8	11-3-8	117870	16-4-0	21-4-8	23-6-4	30-0-0		35-0-9	, 41-0-0	
Dieta Officia (V	5-8-8	5-7-0	0-4-8	4-8-0	5-0-8	2-1-12	6-5-12		5-0-9	5-11-7	
Plate Offsets (X	(,1)- [4:0	0-4-0,0-1-11], [9:0-6-0	,0-2-4], [16:0-	3-8,0-3-0], [24	:0-2-0,0-1-8]						
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0)	Plate Grip DOL	1.25	TC	0.49	Vert(LL)	0.07 13-30	>999	240	MT20	244/190
TCDL 7.0)	Lumber DOL	1.25	BC	0.39	Vert(CT)	-0.09 19-20	>999	180		
BCLL 0.0	.	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.02 11	n/a	n/a		
BCDL 10.0)	Code FBC2017/T	PI2014	Matri	x-MS		9,0000 NE		13020	Weight: 258 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

4-22,6-17: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

BOT CHORD

(size) 2=0-3-8, 16=0-3-8, 11=0-3-8

Max Horz 2=182(LC 11)

Max Uplift 2=-206(LC 12), 16=-529(LC 9), 11=-314(LC 8) Max Grav 2=855(LC 23), 16=1778(LC 1), 11=603(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1188/574, 3-4=-912/510, 4-5=-559/436, 5-6=-559/436, 7-9=-64/383,

9-10=-320/562, 10-11=-696/862

2-23=-352/967, 4-20=-129/422, 19-20=-176/734, 17-18=-804/343, 6-18=-752/348, 16-17=-355/225, 13-14=-620/541, 11-13=-620/541

WEBS 3-20=-407/231, 4-19=-366/97, 5-19=-322/239, 6-19=-360/906, 7-17=-242/774,

7-16=-1079/472, 9-16=-742/607, 9-14=-500/400, 10-14=-481/518, 20-23=-346/911

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=206, 16=529, 11=314.



Structural wood sheathing directly applied or 5-2-4 oc purlins.

Rigid ceiling directly applied or 5-11-7 oc bracing. Except:

9-16

10-0-0 oc bracing: 20-22

1 Row at midpt

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

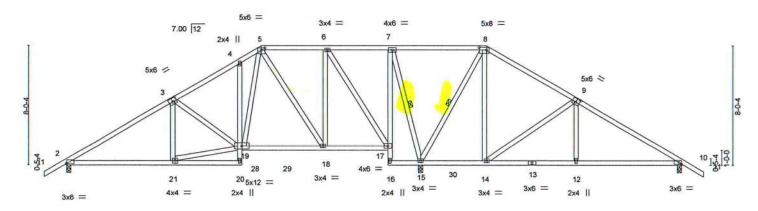
WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss systems. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss			Truss Type			Qty	Ply	SIMQUE - 1995 MODEL		
2253489	T04			HIP			1		1		T19767905
Part March Colored	i i i i i i i i i i i i i i i i i i i					ar transmitted and the same of the	2222		Job Reference (optional)		
Builders FirstSource,	Jacksonvil	lle, FL - 32	2244,					8.240 s M	ar 9 2020 MiTek Industries, Ir	nc. Mon Mar 23 14:20	0:38 2020 Page 1
							ID:RGwSt4c	PREnm5p9	yfgzNgAycgdx-cB_z2Yoaj3G.	IXau?lwv7DjK0KrSs4	BzxUqnThczY15t
r1-6-0	7-1-0	- (11-8-0	13-0-0	17-2-4	21-4-8	28-	0-0	33-11-0	41-0-0	42-6-0,
1-6-0	7-1-0		4-7-0	4-4-0	4-2-4	4-2-4	6-7	-8	5-11-0	7-1-0	1-6-0

Scale = 1:74.1



	T	7-1-0	11-8-0	17-2-4	21-4-8	23-6-4	28-0-0	1	33-11-0	41-0-0	
		7-1-0	4-7-0	5-6-4	4-2-4	2-1-12	4-5-12	1	5-11-0	7-1-0	1
Plate Offse	ets (X,Y)-	[2:0-0-0,0-0-2], [3:0-3-0	0-3-0], [5:0-3-0	,0-1-12], [8:0-6-	0,0-2-4], [9:0-3-	0,0-3-0]					
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L∕d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC C	0.49	Vert(LL)	-0.06 21-24	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC C	0.46	Vert(CT)	-0.14 21-24	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0	0.95	Horz(CT)	0.02 15	n/a	n/a		
BCDL	10.0	Code FBC2017/	TPI2014	Matrix-M	MS					Weight: 268 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

4-20,7-16: 2x4 SP No.3

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 15=0-3-8, 10=0-3-8

Max Horz 2=-211(LC 10)
Max Uplift 2=-215(LC 12), 15=-267(LC 9), 10=-222(LC 13) Max Grav 2=842(LC 23), 15=1797(LC 1), 10=623(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1110/582, 3-4=-847/550, 4-5=-886/614, 5-6=-466/421, 6-7=-144/276, 7-8=-66/412,

8-9=-278/282, 9-10=-692/384 **BOT CHORD**

2-21=-337/943, 18-19=-147/552, 17-18=-97/328, 7-17=-185/655, 12-14=-183/516,

10-12=-183/514

WEBS 19-21=-333/976, 3-19=-407/233, 5-19=-305/658, 5-18=-450/127, 6-18=-65/525, 6-17=-787/275, 8-15=-773/274, 8-14=-146/455, 9-14=-638/340, 9-12=0/292,

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=215, 15=267, 10=222.



Structural wood sheathing directly applied or 4-11-0 oc purlins.

8-15, 7-15

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt

6904 Parke East Blvd. Tampa FL 33610

March 23,2020

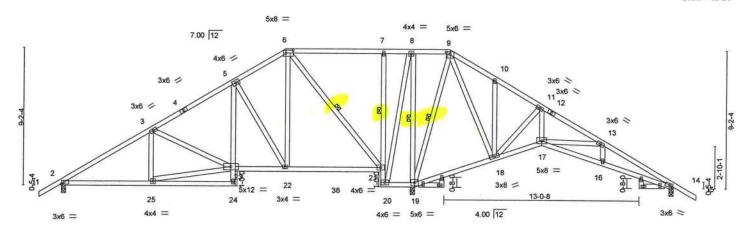
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design indicated is to prevent buckling of individual truss web and/or chord mabbers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **

SNSTPT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss		Truss Type			C	ty F	Ply	SIMQUE - 1995 N	ODEL	THE STATE	
2253489	T05		Hip			1		1				T19767906
Builders FirstSource,	Jacksonville	e. FL - 32244.					82		Job Reference (op 9 2020 MiTek Ind		Mar 22 14:20:4	0.2020 Bass 1
r1-6-0 ı	6-0-0	11-8-0	, 15-0-0		21-4-8	ID:RGwS			NgAycgdx-Ya5jTE	pqFgW0nu2NsL	xbl8PMqf9TY9T	Dy8GalVzY15r
1-6-0	6-0-0	5-8-0	3-4-0	-	6-4-8	2-3-8	2-4-0	3-0-0		36-3-0 4-0-12	41-0-0	1-6-0

Scale = 1:74.1



	6-0-0	11-8-0	15-0-0	21-4-8		u 25-8-0 u	29-0-0	32-2-4		36-3-0	1 38-8-8	41-0-0	
	6-0-0	5-8-0	3-4-0	6-4-8	2-1-12	1-12	3-4-0	3-2-4		4-0-12	2-5-8	2-3-8	ł
e Offsets (X,Y)-	[6:0-6-0,0-2-4]	[9:0-3-0,0-1-12], [1	14:0-2-5,0-1-8]		0-	1-12							_
	T								_	_			

LOADING TCLL TCDL BCLL	G (psf) 20.0 7.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC BC WB	0.49 0.39 0.69	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.14 0.05		l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190	7
BCDL	10.0	Code FBC2017/TF		200000000000000000000000000000000000000	x-MS	Holz(C1)	0.05	19	II/a	n/a	Weight: 297 lb	FT = 20%	

BRACING-

WEBS

TOP CHORD

BOT CHORD

1 Row at midpt

1 Row at midpt

LUMBER-

WEBS

Plate

BOT CHORD

REACTIONS.

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except*

5-24,7-20,26-27,29-30: 2x4 SP No.3

2x4 SP No.3

(size) 2=0-3-8, 19=0-3-8, 14=0-3-8

Max Horz 2=240(LC 11) Max Uplift 2=-209(LC 12), 19=-246(LC 12), 14=-190(LC 13) Max Grav 2=791(LC 23), 19=2075(LC 1), 14=442(LC 20)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

TOP CHORD 2-3=-1057/517, 3-5=-730/442, 5-6=-391/375, 6-7=0/410, 7-8=0/414, 8-9=0/633, 9-10=-84/456, 10-11=-80/423, 11-13=-279/334, 13-14=-829/428

2-25=-294/881, 5-23=-140/379, 22-23=-233/559, 21-22=-165/306, 20-21=-1151/547, **BOT CHORD**

7-21=-329/270, 19-20=-643/364, 18-19=-553/341, 17-18=-305/217, 16-17=-278/724,

14-16=-277/730

23-25=-299/826, 3-23=-456/247, 5-22=-582/326, 6-22=-231/673, 6-21=-951/303,

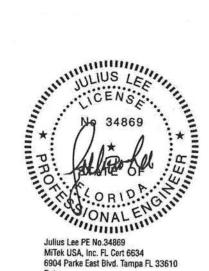
8-20=-443/1052, 8-19=-1080/435, 9-19=-727/232, 9-18=-226/507, 11-18=-477/222,

11-17=-89/317, 13-17=-652/331

NOTES-

WEBS

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 14 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=209, 19=246, 14=190.



Structural wood sheathing directly applied or 5-5-4 oc purlins.

Rigid ceiling directly applied or 4-11-5 oc bracing. Except:

7-21

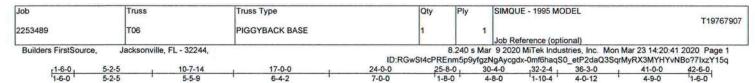
6-21, 8-19, 9-19

MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

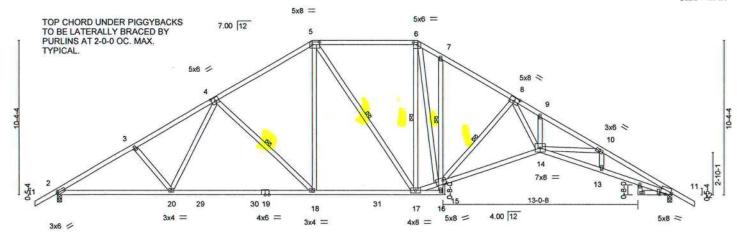
March 23,2020

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Scale = 1:74.1



	1	7-7-4	17-0-0	0		24-0-0	25-8-0	1	32-2-4		36-3-0	38-8-8	41-0-0
	1	7-7-4	9-4-12	2		7-0-0	1-8-0	1:	6-6-4	1	4-0-12	2-5-8	2-3-8
late Offse	ets (X,Y)-	[2:0-0-15,0-1-8], [4:0-3-0,	0-3-0], [5:0-6-0,	,0-2-4], [6:0-	3-8,0-2-0], [8:0-4-0,0-3-0], [11:	0-1-2,E	dge], [1	1:0-0-0,0-	4-12], [15:0	0-2-4,0-3-0]		
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PL	ATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.87	Vert(LL)	-0.40	14	>999	240	MT	20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.97	Vert(CT)	-0.79	14-15	>620	180			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.94	Horz(CT)	0.44	11	n/a	n/a			
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	25 52					We	ight: 271 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

1 Row at midpt

1 Row at midpt

LUMBER-TOP CHORD

2x4 SP No.2

2x4 SP No.2 *Except* **BOT CHORD**

7-16,11-21: 2x4 SP No.3, 11-14: 2x4 SP M 31

WEBS 2x4 SP No.3 *Except*

8-14: 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 11=0-3-8

Max Horz 2=-269(LC 10)

Max Uplift 2=-324(LC 12), 11=-324(LC 13) Max Grav 2=1598(LC 1), 11=1598(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2603/1140, 3-4=-2448/1130, 4-5=-1848/951, 5-6=-1529/896, 6-7=-1995/1111, TOP CHORD

7-8=-2071/1034, 8-9=-5018/2129, 9-10=-5054/2062, 10-11=-5050/2097 2-20=-854/2358, 18-20=-677/2027, 17-18=-403/1576, 14-15=-1110/3159,

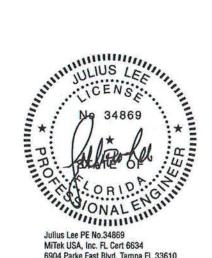
BOT CHORD 13-14=-1787/4534, 11-13=-1760/4463

3-20=-271/214, 4-20=-120/457, 4-18=-649/386, 5-18=-212/754, 6-17=-532/163, WEBS

15-17=-355/1568, 6-15=-544/1231, 8-15=-1891/825, 8-14=-1139/3025

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=324, 11=324.



Structural wood sheathing directly applied or 1-9-7 oc purlins.

7-15

4-18, 5-17, 6-17, 8-15

Rigid ceiling directly applied or 2-2-0 oc bracing. Except:

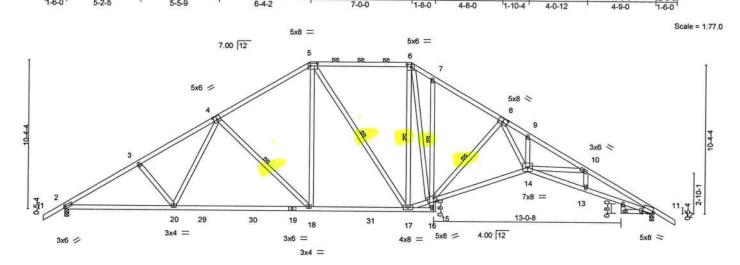
MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

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Job	Truss		Truss Type		Qty	Ply	1	SIMQUE - 1995 N	MODEL		
2253489	T07		PIGGYBACK BASE		16		1				T197679
Duildess CirclOs								Job Reference (o)			
Builders FirstSource,	Jacksonvil	le, FL - 32244,				8.240 s	Mar	9 2020 MiTek Ind	lustries, Inc. N	10n Mar 23 14:2	0:43 2020 Page 1
		10 (00000000000000000000000000000000000		ID:F	RGwSt4cPR	Enm5p	9yfgzt	NgAycgdx-y9ns50	GsiYbubeMnyY	YUVIwn1n1s4ilS	Vge6UEMgzY15o
[1-6-0]	5-2-5	10-7-14	17-0-0	24-0-0	25-8-		30-4-		36-3-0	41-0-0	42-6-0,
1-6-0	5-2-5	5-5-9	6-4-2	7-0-0	1-8-	0	4-8-0		4-0-12	4-9-0	1-6-0



 	7-7-4	17-0-0 9-4-12	7-0-0	25-8-0	32-2-4	36-3-0 38-8-	
Plate Offsets (X,Y)-		,0-3-0], [5:0-6-0,0-2-4], [6:0-3	-8,0-2-0], [8:0-4-0,0-3-0], [1	1:0-1-2,Edge], [6-6-4 [11:0-0-0,0-4-12],	4-0-12 2-5-8 [15:0-2-4,0-3-0]	3 2-3-8
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/T	1.25 BC YES WB	DEFL. 0.87 Vert(LL) 0.80 Vert(CT) 0.93 Horz(CT) MS	등	>999 240	PLATES MT20 Weight:	244/190

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP M 31 *Except*

7-16,11-21: 2x4 SP No.3 WEBS 2x4 SP No.3 *Except*

8-14: 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 11=0-3-8

Max Horz 2=-269(LC 10)

Max Uplift 2=-324(LC 12), 11=-324(LC 13)

Max Grav 2=1598(LC 1), 11=1598(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2603/1140, 3-4=-2448/1130, 4-5=-1847/951, 5-6=-1530/896, 6-7=-1994/1110,

7-8=-2070/1034, 8-9=-5015/2129, 9-10=-5052/2062, 10-11=-5051/2097

BOT CHORD 2-20=-855/2359, 18-20=-677/2026, 17-18=-403/1576, 14-15=-1110/3160,

13-14=-1787/4534, 11-13=-1760/4463 WEBS

3-20=-271/215, 4-20=-119/457, 4-18=-648/386, 5-18=-211/752, 6-17=-524/160,

15-17=-348/1549, 6-15=-541/1222, 8-15=-1893/826, 8-14=-1138/3022

NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=324, 11=324,
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 1-9-7 oc purlins, except

4-18, 5-17, 6-17, 8-15

Rigid ceiling directly applied or 5-9-0 oc bracing. Except:

7-15

2-0-0 oc purlins (3-1-0 max.): 5-6.

1 Row at midpt

1 Row at midpt

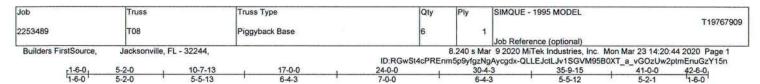
Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

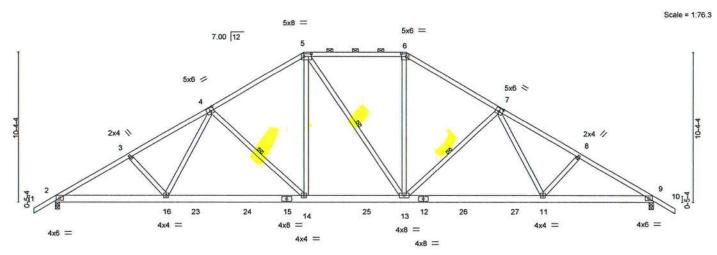
March 23,2020

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.







		1-1-3	1/	-0-0	1	24-0-0	-1		33-4-13		41-0-0	
		7-7-3	9-	4-13		7-0-0		701175	9-4-13		7-7-3	1
Plate Off	sets (X,Y)-	[4:0-3-0,0-3-0], [5:0-6-0,0	-2-4], [6:0-4-0,	0-2-4], [7:0-3	-0,0-3-0]							
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.74	Vert(LL)	-0.23	14-16	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.93	Vert(CT)	-0.45	14-16	>999	180	CAGGARGOO	
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.85	Horz(CT)	0.08	9	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS	CHAPTER STATE OF THE STATE OF T					Weight: 272 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2 *Except*

5-6: 2x4 SP M 31 2x6 SP M 26 "Except"

BOT CHORD 12-15: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 9=0-3-8

Max Horz 2=269(LC 11)

Max Uplift 2=-430(LC 12), 9=-370(LC 13) Max Grav 2=2020(LC 19), 9=1781(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-3557/1565, 3-4=-3421/1552, 4-5=-2364/1198, 5-6=-1832/1037, 6-7=-2195/1116,

7-8=-2881/1305, 8-9=-3020/1320

BOT CHORD 2-16=-1222/3201, 14-16=-967/2698, 13-14=-618/2084, 11-13=-829/2216,

9-11=-1023/2542 WEBS

3-16=-255/211, 4-16=-291/844, 4-14=-869/491, 5-14=-448/1219, 5-13=-443/220,

6-13=-300/830, 7-13=-665/391, 7-11=-126/493, 8-11=-270/218

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=430, 9=370,
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-5=-54, 5-6=-54, 6-10=-54, 16-17=-20, 14-16=-80(F=-60), 14-20=-20



Structural wood sheathing directly applied or 2-10-4 oc purlins,

4-14, 5-13, 7-13

2-0-0 oc purlins (5-3-11 max.): 5-6.

1 Row at midpt

Rigid ceiling directly applied or 7-6-5 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 23,2020

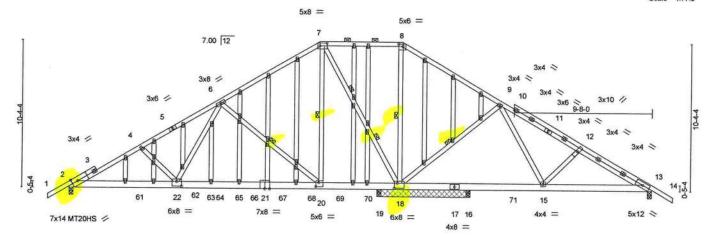
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designs, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occliapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty Ply SIMQUE - 1995 MODEL T19767910 2253489 T08G GABLE Job Reference (optional) 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:47 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-rw0NxdvDbqP17z4jnJZE4dCU1UTkhGAFZkSRVbzY15k Builders FirstSource. Jacksonville, FL - 32244.

20-6-0 23-5-1

Scale = 1:77.9



	7-7-3		17-6-15	22-0-0	23-5-1 25-0-0	28-3-8	33-4-13	41-0-0	10
Plate Offsets (X,Y)-	7-7-3 [2:0-4-15,Edge], [7:0-6-0	.0-2-4], [8:0-3-8	9-11-12 .0-2-0], [18:0-2-8.0-4-	4-5-1	1-5-1 1-6-15	3-3-8 21 [22:0-4	5-1-5	7-7-3	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/TI	2-0-0 1.25 1.25 NO	CSI. TC 0.81 BC 0.73 WB 0.95 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.28 20-22 -0.34 20-22 0.02 18	l/defl >940 >756	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 388 lb	GRIP 244/190 187/143 FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP M 26 *Except*

17-21: 2x6 SP No.2 2x4 SP No.3 *Except*

WEBS 7-18: 2x4 SP No.2

OTHERS 2x4 SP No.3

REACTIONS. All bearings 0-3-8 except (jt=length) 18=6-7-0.

(lb) - Max Horz 2=-269(LC 25)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-687(LC 8), 18=-1312(LC 8),

13=-352(LC 28), 19=-171(LC 8)

Max Grav All reactions 250 lb or less at joint(s) 16 except 2=1342(LC 19),

18=2913(LC 1), 13=609(LC 16), 19=322(LC 33)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-2178/1194, 4-6=-2036/1171, 6-7=-258/175, 7-8=-383/969, 8-9=-501/1165,

9-12=-655/648, 12-13=-711/587

BOT CHORD 2-22=-1159/2084, 20-22=-651/1154, 19-20=-276/375, 18-19=-276/375, 16-18=-672/491, 15-16=-672/491, 13-15=-488/590

WERS

4-22=-321/201, 6-22=-939/1545, 6-20=-1258/771, 7-20=-952/1709, 7-18=-2216/1158,

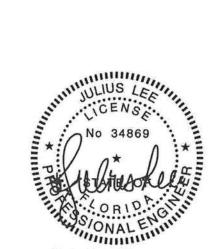
8-18=-860/335, 9-18=-613/289, 9-15=-100/421, 12-15=-270/154

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 687 lb uplift at joint 2, 1312 lb uplift at joint 18, 352 lb uplift at joint 13 and 171 lb uplift at joint 19.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied or 3-0-9 oc purlins, except

6-20, 7-20, 8-18, 9-18

2-0-0 oc purlins (9-2-11 max.): 7-8.

1 Row at midpt

2 Rows at 1/3 pts

Rigid ceiling directly applied or 6-0-0 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020

Continued on page 2

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ANSITPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	SIMQUE - 1995 MODEL	100000000000000000000000000000000000000
2253489	T08G	GABLE	1	1		T19767910
					Job Reference (optional)	

Builders FirstSource,

Jacksonville, FL - 32244,

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:48 2020 Page 2 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-J6al9zwrM7Xuk7fwK14TdqlfntoyQjQPoOC?11zY15j

NOTES-

- 12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 368 lb down and 374 lb up at 5-0-0, 163 lb down and 145 lb up at 7-0-12, 163 lb down and 145 lb up at 11-0-12, 163 lb down and 145 lb up at 13-0-12, 163 lb down and 145 lb up at 15-0-12, 163 lb down and 145 lb up at 17-0-12, and 163 lb down and 145 lb up at 17-0-12, and 163 lb down and 145 lb up at 17-0-12, and 163 lb down and 145 lb up at 17-0-12, and 163 lb down and 145 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-7=-54, 7-8=-54, 8-14=-54, 53-57=-20

Concentrated Loads (lb)

Vert: 61=-368(F) 62=-152(F) 63=-152(F) 65=-152(F) 66=-152(F) 67=-152(F) 68=-152(F) 69=-152(F) 70=-152(F)



Job Truss Truss Type SIMQUE - 1995 MODEL Qty T19767911 2253489 T09 Common Job Reference (optional) Builders FirstSource Jacksonville, FL - 32244, 8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Mar 23 14:20:48 2020 Page 1 ID:RGwSt4cPREnm5p9yfgzNgAycgdx-J6al9zwrM7Xuk7fwK14TdqllvttoQqOPoOC?11zY15j 19-10-0 4x6 || Scale = 1:41.0 7.00 12 2x4 📏 2x4 // 10 9 8 3x4 = 3x4 = 3x6 = 3x6 = 3x6 = 12-10-4 19-10-0 6-11-12 5-10-8 Plate Offsets (X,Y)-[2:0-0-0,0-0-2], [6:0-0-0,0-0-2] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) I/defi L/d **PLATES** GRIP TCLL 20.0 0.42 Plate Grip DOL 1.25 TC Vert(LL) 0.12 8-16 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.42 Vert(CT) -0.11 10-13 >999 180

LUMBER-

BCLL

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS

0.0

10.0

BRACING-

Horz(CT)

0.02

6

n/a

n/a

TOP CHORD **BOT CHORD**

Structural wood sheathing directly applied or 5-4-3 oc purlins. Rigid ceiling directly applied or 5-4-11 oc bracing.

Weight: 98 lb

FT = 20%

REACTIONS.

(size) 2=0-3-8, 6=0-3-8 Max Horz 2=207(LC 11)

Max Uplift 2=-320(LC 12), 6=-320(LC 13) Max Grav 2=815(LC 1), 6=815(LC 1)

Rep Stress Incr

Code FBC2017/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-1134/1303, 3-4=-966/1264, 4-5=-966/1264, 5-6=-1134/1303 TOP CHORD

BOT CHORD 2-10=-1029/939, 8-10=-582/615, 6-8=-1039/939

WEBS 4-8=-603/354, 5-8=-318/275, 4-10=-603/354, 3-10=-319/275

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

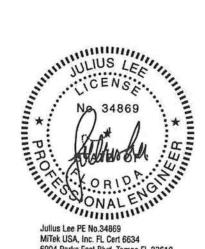
WB 0.51

Matrix-MS

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 2 and 320 lb uplift at joint 6.



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March 23,2020

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job	Truss	Truss Type	Qty	Ply	SIMQUE - 1995 MODEL		
2253489	T09G	GABLE	1	1	19		T19767912
					Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 32244,				r 9 2020 MiTek Industries, In		
			ID:RGwSt4	cPREnm5p	9yfgzNgAycgdx-FViVZfx5ulno	:_QpISS7xiFq2?	hZZuk6iFih56wzY15h
-1-6-0	4-6-7	9-11-0	-	15-3-9	1	19-10-0	, 21-4-0
1-6-0	4-6-7	5-4-9	,	5-4-9		4-6-7	1-6-0

Scale = 1:39.9

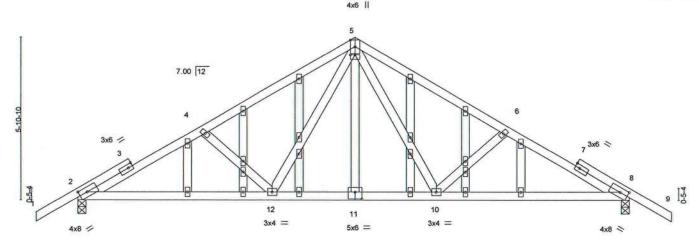


Plate Offsets (X,Y)-	[2:0-3-7,0-1-15], [5:0-2-0,	0-0-0], [8:0-3-7	7,0-1-15], [11:	0-3-0,0-3-0]							
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	0.11	10-35	>999	240	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.40	Vert(CT)	-0.10	12-31	>999	180	SECONDALIA.	
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.49	Horz(CT)	0.03	8	n/a	n/a		
BCDL 10.0	Code FBC2017/TF	PI2014	Matrix	c-MS	The restrictive Activity (F)					Weight: 136 lb	FT = 20%

12-10-4

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

2x4 SP No.3 **OTHERS**

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=-197(LC 10) Max Uplift 2=-323(LC 12), 8=-323(LC 13) Max Grav 2=812(LC 1), 8=812(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-4=-1168/1393, 4-5=-1012/1313, 5-6=-1012/1313, 6-8=-1168/1393 TOP CHORD

BOT CHORD 2-12=-1152/1033, 10-12=-607/629, 8-10=-1166/1033

WEBS 5-10=-643/382, 6-10=-376/353, 5-12=-643/382, 4-12=-376/353

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2 and 323 lb uplift at joint 8.



Structural wood sheathing directly applied or 4-11-9 oc purlins.

Rigid ceiling directly applied or 5-2-2 oc bracing.

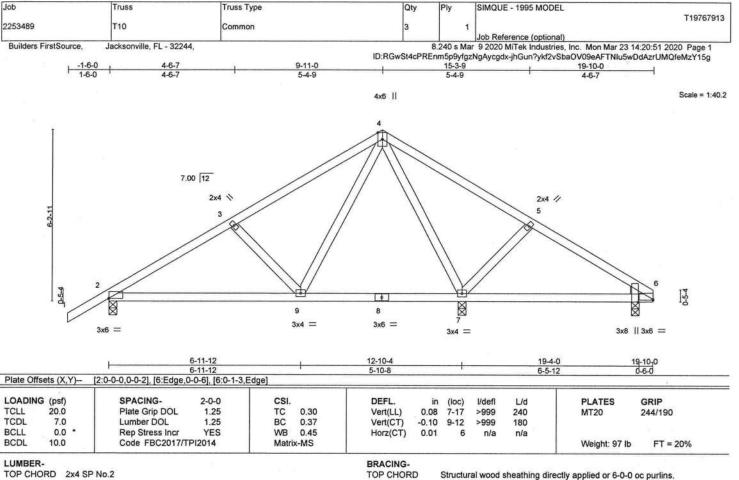
Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

March 23,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

WEBS 2x4 SP No.3

WEDGE

Right: 2x4 SP No.3

REACTIONS.

(size) 2=0-3-8, 7=0-3-8, 6=0-3-8

Max Horz 2=159(LC 9)

Max Uplift 2=-122(LC 12), 7=-165(LC 12), 6=-150(LC 8)

Max Grav 2=544(LC 1), 7=761(LC 1), 6=266(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-620/360, 3-4=-491/327, 4-5=-104/257, 5-6=-202/299 **BOT CHORD** 2-9=-247/544

4-7=-540/148, 5-7=-318/290, 4-9=-124/429, 3-9=-335/250 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 2, 165 lb uplift at joint 7 and 150 lb uplift at joint 6.



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March 23,2020

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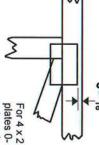
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

œ

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

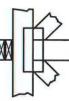
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

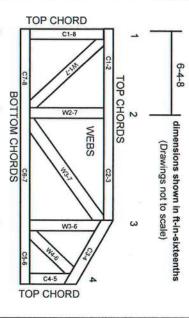
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89: Des BCSI: Buil

Design Standard for Bracing.
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

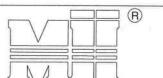
S

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks, Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

T-Brace size

2x4 or 2x6 or 2x8

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nail Spacing
6" o.c.

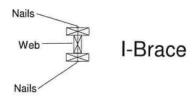
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

10d (0.131" X 3")

Nailing Pattern

Nail Size

Nails	
SPACING	
WEB HE A THE STATE OF THE STATE	
T-BRACE	A
Nails Section Detail T-Brace	
Web	



	1.0001.0000	ce Size -Ply Truss			
	Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

	10000000	e Size -Ply Truss		
	Specified Continuous Rows of Lateral Braci			
Web Size	1	2		
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace		
2x6	2x6 T-Brace	2x6 I-Brace		
2x8	2x8 T-Brace	2x8 I-Brace		

T-Brace / I-Brace must be same species and grade (or better) as web member.



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SCAB-BRACE DETAIL

MII-SCAB-BRACE

MiTek USA, Inc.

Page 1 of 1

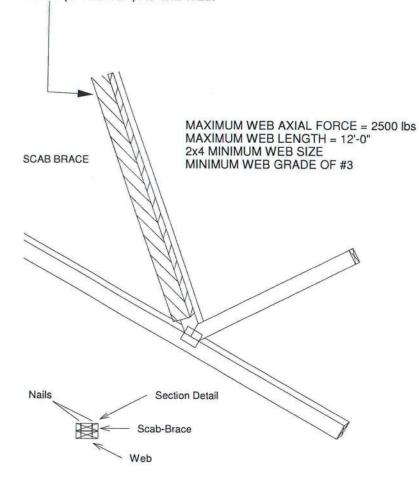


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.

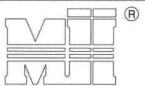


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STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc. ENGINEERED BY AL! 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED. THE LOADS INDICATED.

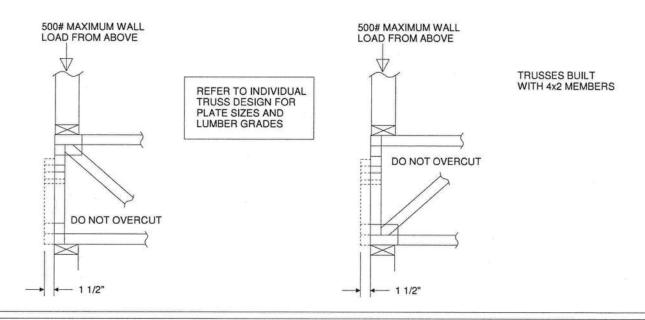
2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

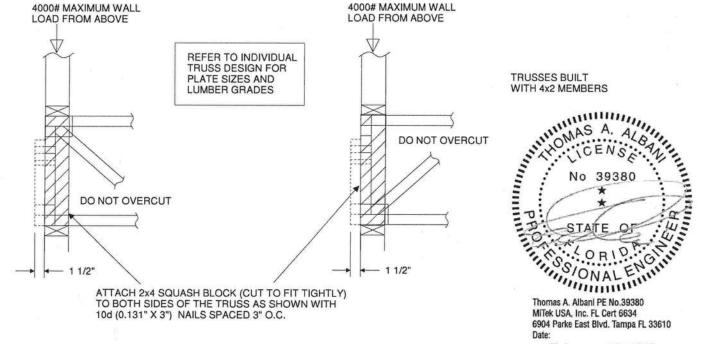
3. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.

6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.

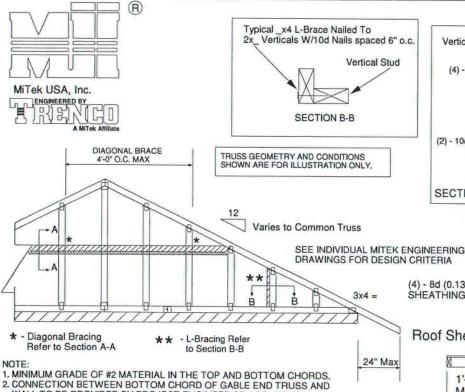






Standard Gable End Detail

MII-GE130-D-SP



MiTek USA, Inc. Page 1 of 2 Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A 2v4 Stud

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A CONSTRUCT HORIZONTAL BRACE CONNECTING A 2X0 STUD AND A 2X4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

1'-3" Max.	(2) - 10d NAILS (2) - 10d NAILS
1	Trusses @ 24" o.c.
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)
	The state of the s

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximu	m Stud Le	ngth	
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

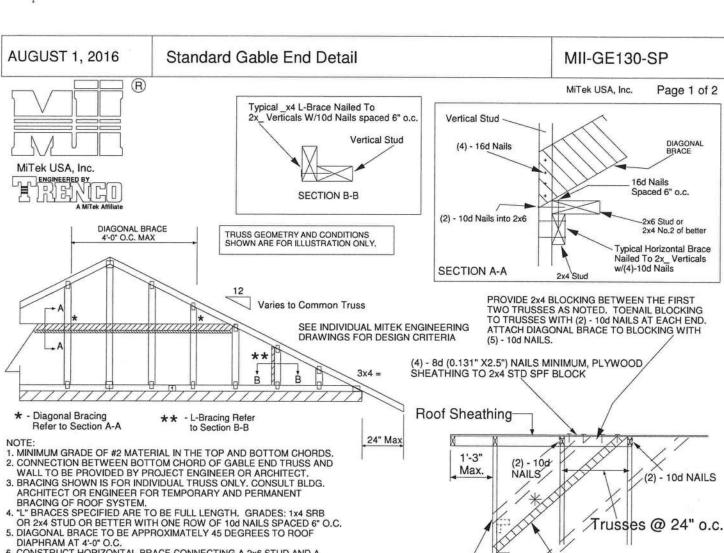
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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Diag. Brace

at 1/3 points

End Wall

if needed

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A) GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

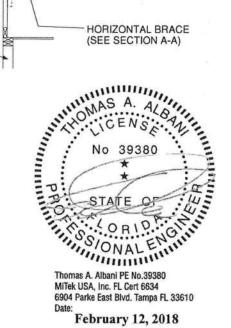
NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
and Grade		Maximum Stud Length					
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6	
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1	
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING CONNECTION OF BRACING IS BASED ON MWFRS.



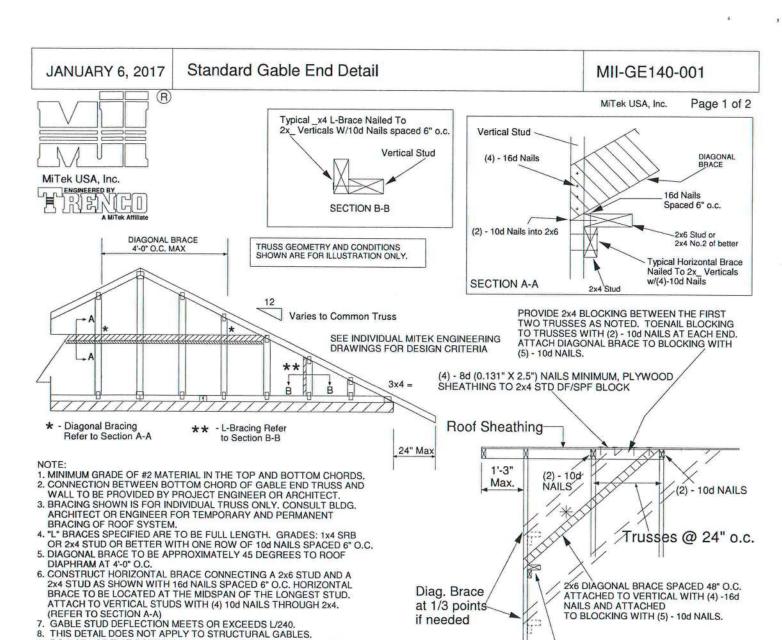
2x6 DIAGONAL BRACE SPACED 48" O.C.

ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

TO BLOCKING WITH (5) - 10d NAILS.

Page 1 of 2

DIAGONAL BRACE



End Wall

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade		Maximum Stud Length						
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4		
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11		
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60

TYPE TRUSSES.

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



HORIZONTAL BRACE

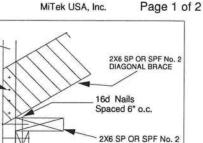
(SEE SECTION A-A)

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 19, 2018

Standard Gable End Detail

MII-GE170-D-SP



Typical Horizontal Brace

(2) - 10d NAILS

Trusses @ 24" o.c.

Nailed To 2x4 Verticals w/(4)-10d Nails

MiTek USA, Inc. 门别尉

DIAGONAL BRACE

4'-0" O.C. MAX

R

Typical 2x4 L-Brace Nailed To 2x4 Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

**

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

3x4 =

24" Max

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

2X4 SP OR SPF No. 2

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

(2) - 10d

NAILS

Vertical Stud

(4) - 16d Nails

(2) - 10d Nails into 2x6

SECTION A-A

Roof Sheathing

Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
 BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL

BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

,	1'-0" Max.	
Diag. Br at 1/3 po if neede	oints/	
End	Wall	

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

> HORIZONTAL BRACE (SEE SECTION A-A)

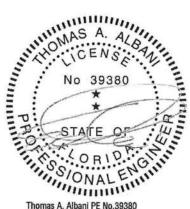
Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
and Grade			Maximum St	ud Length	
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



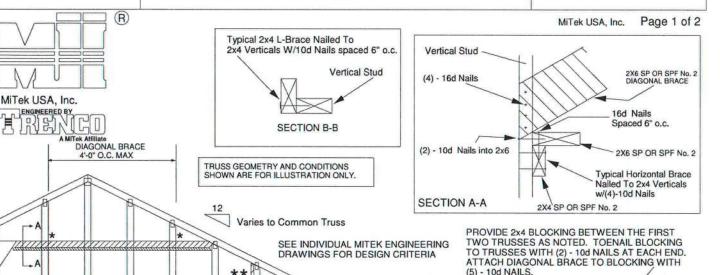
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

MII-GE180-D-SP

(2) - 10d NAILS

Trusses @ 24" o.c.



3x4 =

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

- Diagonal Bracing Refer to Section A-A - L-Bracing Refer to Section B-B

B

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG.

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3

OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES

SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

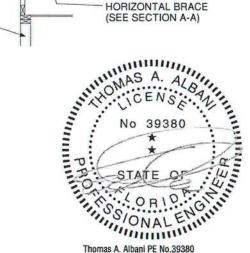
Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
Species and Grade			ud Length		
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH

DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH

(4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD

- 10d

NAILS

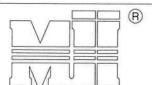
SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-0"

Max.

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



MiTek USA, Inc.

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING **EXPOSURE B or C**

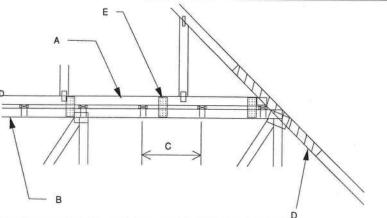
ASCE 7-10 DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131" X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
D - 2 X X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: DIRECTIONS AND:

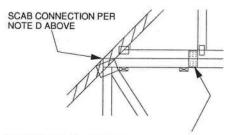
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM
 PIGGYBACK SPAN OF 12 ft.
 E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH

MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REO. REGARDLESS OF SPAN)

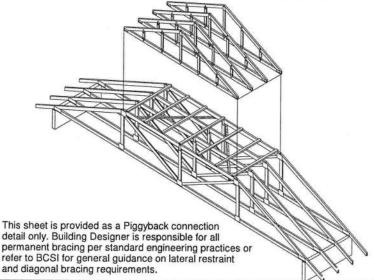


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

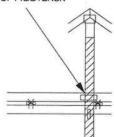
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH NAII-ON PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

 VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

2) ATTACH 2 x __ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131* X 3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24* O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131* X 3.5") NAILS EACH.
D - 2 X __ X 4*-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131* X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

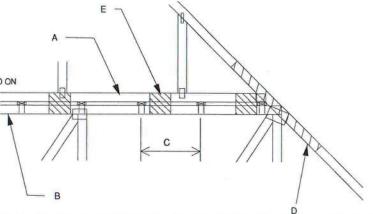
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12t.

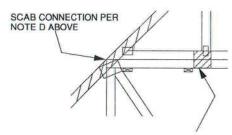
E - FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH

3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

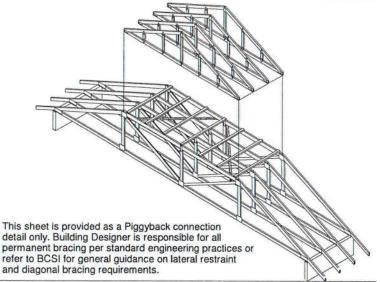


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

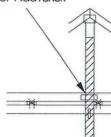
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DELTAIL.
ATTACH 2 X ___ x 4-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.

FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. CONCENTRATED LOAD MUST BE APPLIED TO BOTH

THE PIGGYBACK AND THE BASE TRUSS DESIGN.



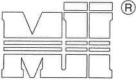
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc. ENGINEERED BY

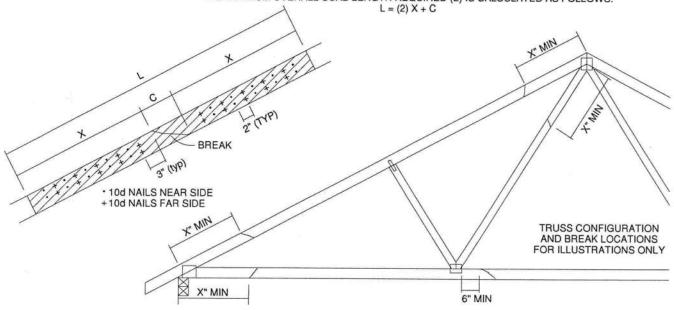
TOTAL NUMBER OF NAILS EACH SIDE		MAXIMUM FORCE (lbs) 15% LOAD DURATION								
	X INCHES	SP		DF		SPF		HF		
2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6	
30	24"	1706	2559	1561	2342	1320	1980	1352	2028	
39	30"	2194	3291	2007	3011	1697	2546	1738	2608	
48	36"	2681	4022	2454	3681	2074	3111	2125	3187	
57	42"	3169	4754	2900	4350	2451	3677	2511	3767	
66	48"	3657	5485	3346	5019	2829	4243	2898	4347	
	2x6 30 39 48 57	X INCHES 2x6 30 24" 39 30" 48 36" 57 42"	X INCHES 2x6 2x4 30 24" 1706 39 30" 2194 48 36" 2681 57 42" 3169	X INCHES X INCHES SP 2x6 2x4 2x6 30 24" 1706 2559 39 30" 2194 3291 48 36" 2681 4022 57 42" 3169 4754	X INCHES X INCHES SP INCHES 2x6 2x4 2x6 2x4 30 24" 1706 2559 1561 39 30" 2194 3291 2007 48 36" 2681 4022 2454 57 42" 3169 4754 2900	X INCHES SP DF 2x6 2x4 2x6 2x4 2x6 30 24" 1706 2559 1561 2342 39 30" 2194 3291 2007 3011 48 36" 2681 4022 2454 3681 57 42" 3169 4754 2900 4350	X INCHES SP DF S 2x6 2x4 2x6 2x4 2x6 2x4 30 24" 1706 2559 1561 2342 1320 39 30" 2194 3291 2007 3011 1697 48 36" 2681 4022 2454 3681 2074 57 42" 3169 4754 2900 4350 2451	X INCHES SP DF SPF 2x6 2x4 2x6 2x4 2x6 2x4 2x6 30 24" 1706 2559 1561 2342 1320 1980 39 30" 2194 3291 2007 3011 1697 2546 48 36" 2681 4022 2454 3681 2074 3111 57 42" 3169 4754 2900 4350 2451 3677	X INCHES SP DF SPF H 2x6 2x4 2x6	

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN.

STAGGER NAIL SPACING FROM FROM FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

NOTES:

- NOTES:

 1. THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES
 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS
 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
 AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 2. THE END DISTANCE FORE DISTANCE AND SPACING OF MAILS SHALL BE SUCH AS TO AVOID.
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
 THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



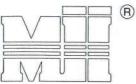
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LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

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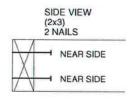
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES
- FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY



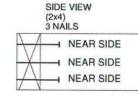
TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) DIAM. DF SPF-S HE SPF .131 88.0 80.6 69.9 LONG 68.4 59.7 .135 93.5 85.6 74.2 72.6 63.4 3.5" .162 108.8 99.6 86.4 84.5 73.8 LONG .128 74.2 57.6 67.9 58.9 50.3 .131 75.9 69.5 60.3 59.0 51.1 3.25" .148 81.4 74.5 64.6 63.2 52.5

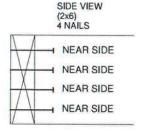
VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

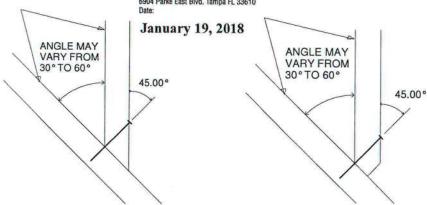
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

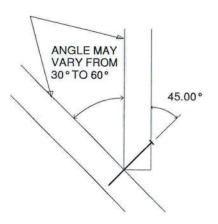






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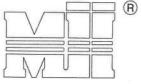


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

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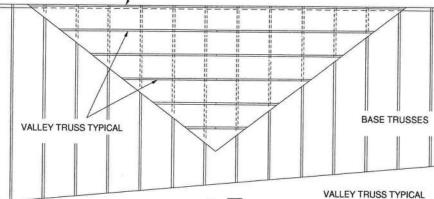


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GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE PER DETAIL A
- BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
- EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING. 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



VALLEY TRUSS TYPICAL GABLE END, COMMON TRUSS P 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d

NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS. DETAIL A (NO SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C

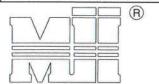
WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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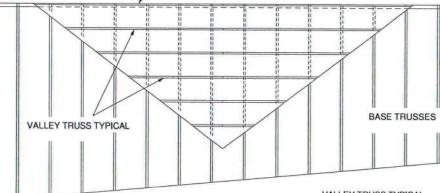


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GABLE END, COMMON TRUSS OR GIRDER TRUSS

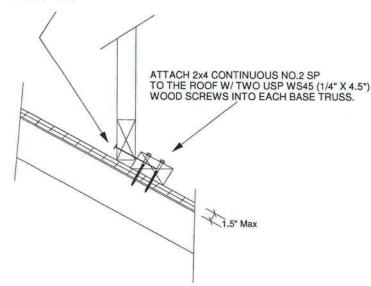
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3") 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
- 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
- SECURE TO BASE TRUSSES AS PER DETAIL A 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
- INDIVIDUAL DESIGN DRAWINGS.
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

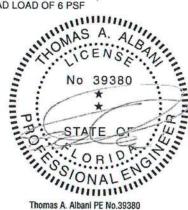


GABLE END, COMMON TRUSS OR GIRDER TRUSS VALLEY TRUSS TYPICAL 12 P SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



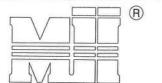
WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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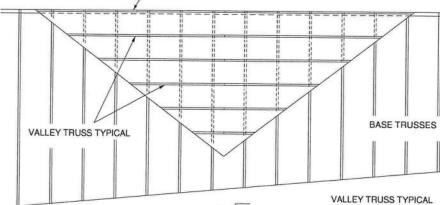


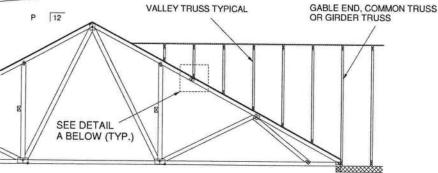
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GABLE END, COMMON TRUSS OR GIRDER TRUSS

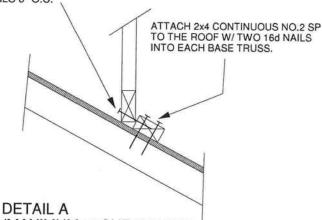
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 16d (0.131" X 3.5") 2. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 3. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C. 7. ALL LUMBER SPECIES TO BE SP.





SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C.



(MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF

ON THE TRUSSES

No 39380

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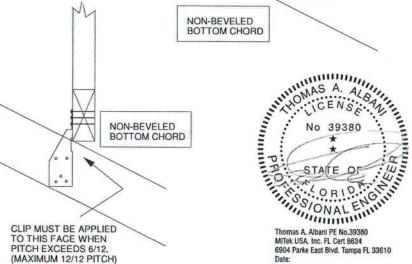
Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

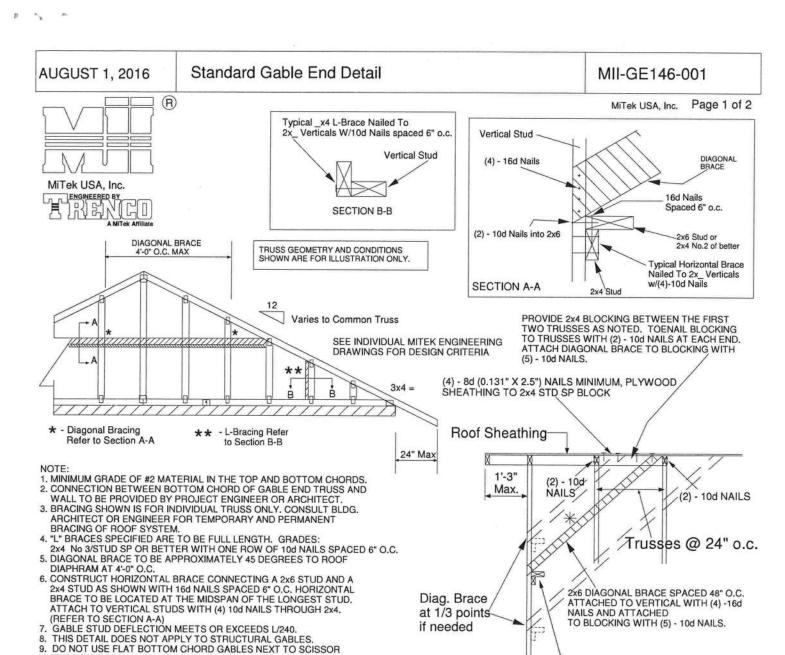
TRUSSED VALLEY SET DETAIL MII-VALLEY AUGUST 1, 2016 (HIGH WIND VELOCITY) R NOTE: VALLEY STUD SPACING NOT Page 1 of 1 MiTek USA, Inc. TO EXCEED 48" O.C. SPACING MiTek USA, Inc. ENGINEERED BY TREAL P FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED EXPOSURE B or C **BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY)

SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.





End Wall

Minimum Stud Size	Stud Spacing	Without 2x4 Brace L-Brace		DIAGONAL BRACE	2 DIAGONA BRACES AT 1/3 POINTS	
Species and Grade		Maxim	num Stud L	ength.		
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10	
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13 10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7	

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

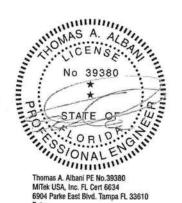
MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE: 1.60

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



HORIZONTAL BRACE

(SEE SECTION A-A)

REPLACE BROKEN OVERHANG

MII-REP13B

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(R)

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TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

END BEARING CONDITION

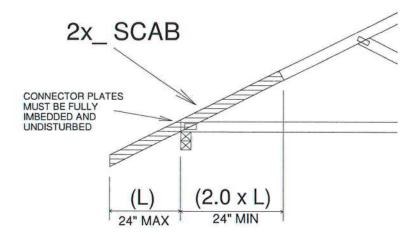
NOTES:

1. ATTACH 2x_ SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH

AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



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LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

MiTek USA, Inc.

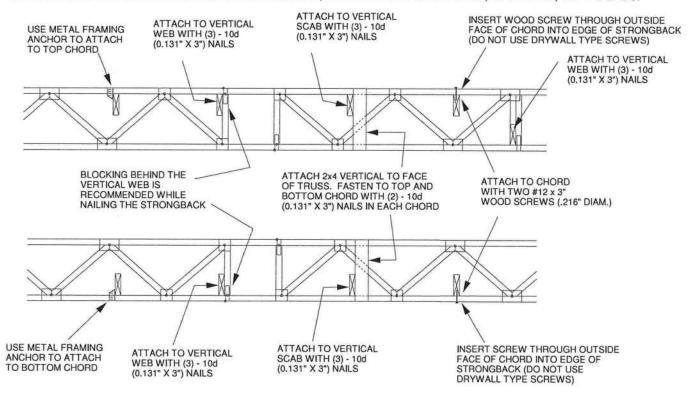
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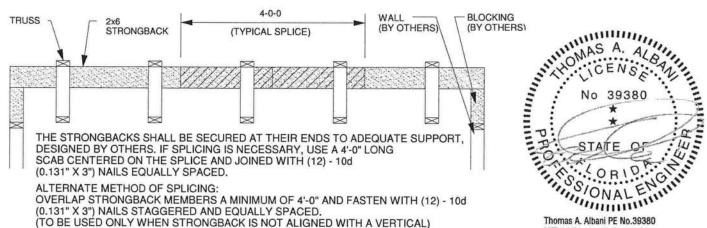


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS. AT THE ENDS.





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