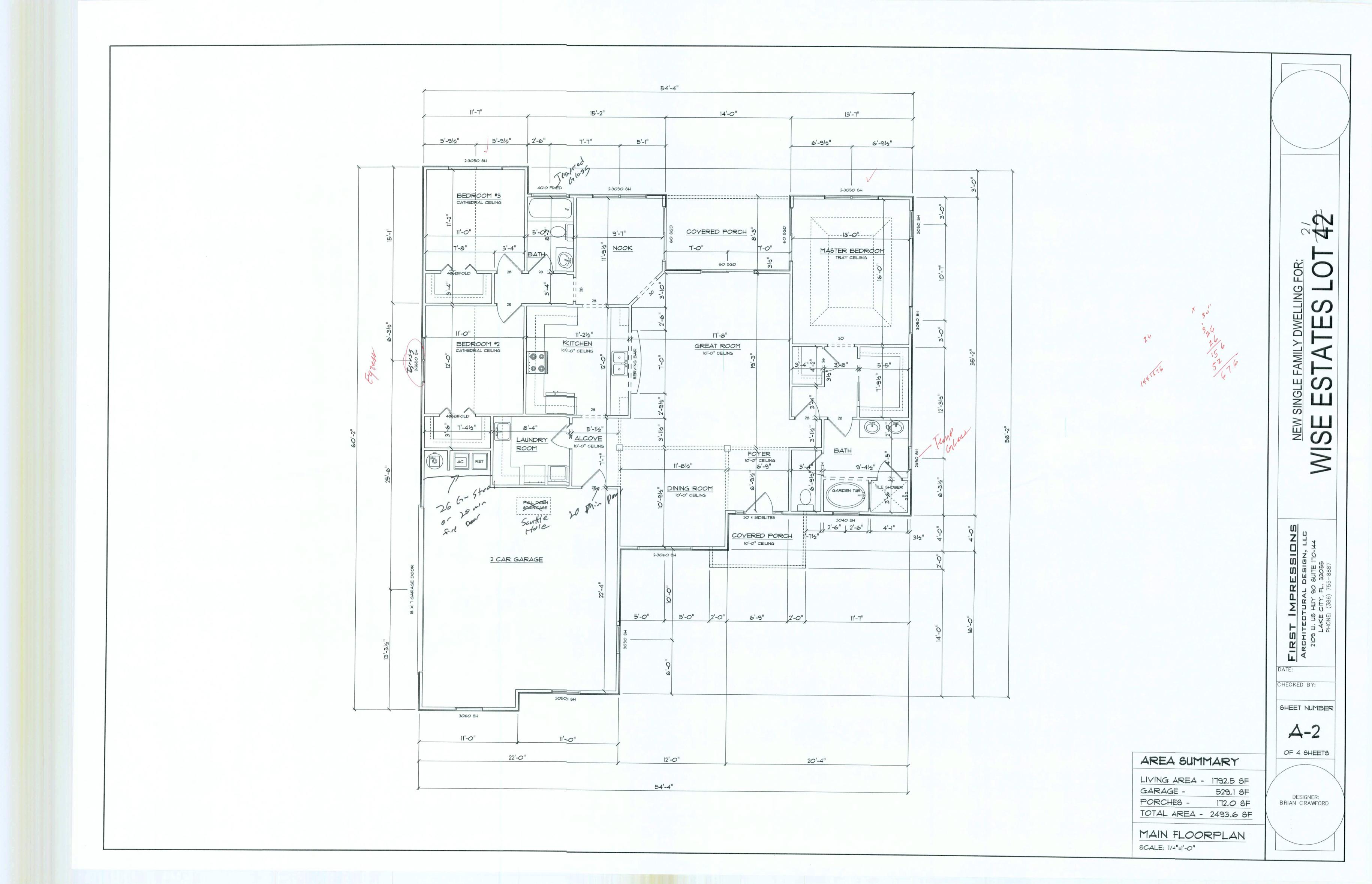
office





DATE:

CHECKED BY:

SHEET NUMBER

OF 4 SHEETS

DESIGNER: BRIAN CRAWFORD

ROOF PLAN SCALE: 1/4"=1'-0"

GARAGE -

PORCHES -

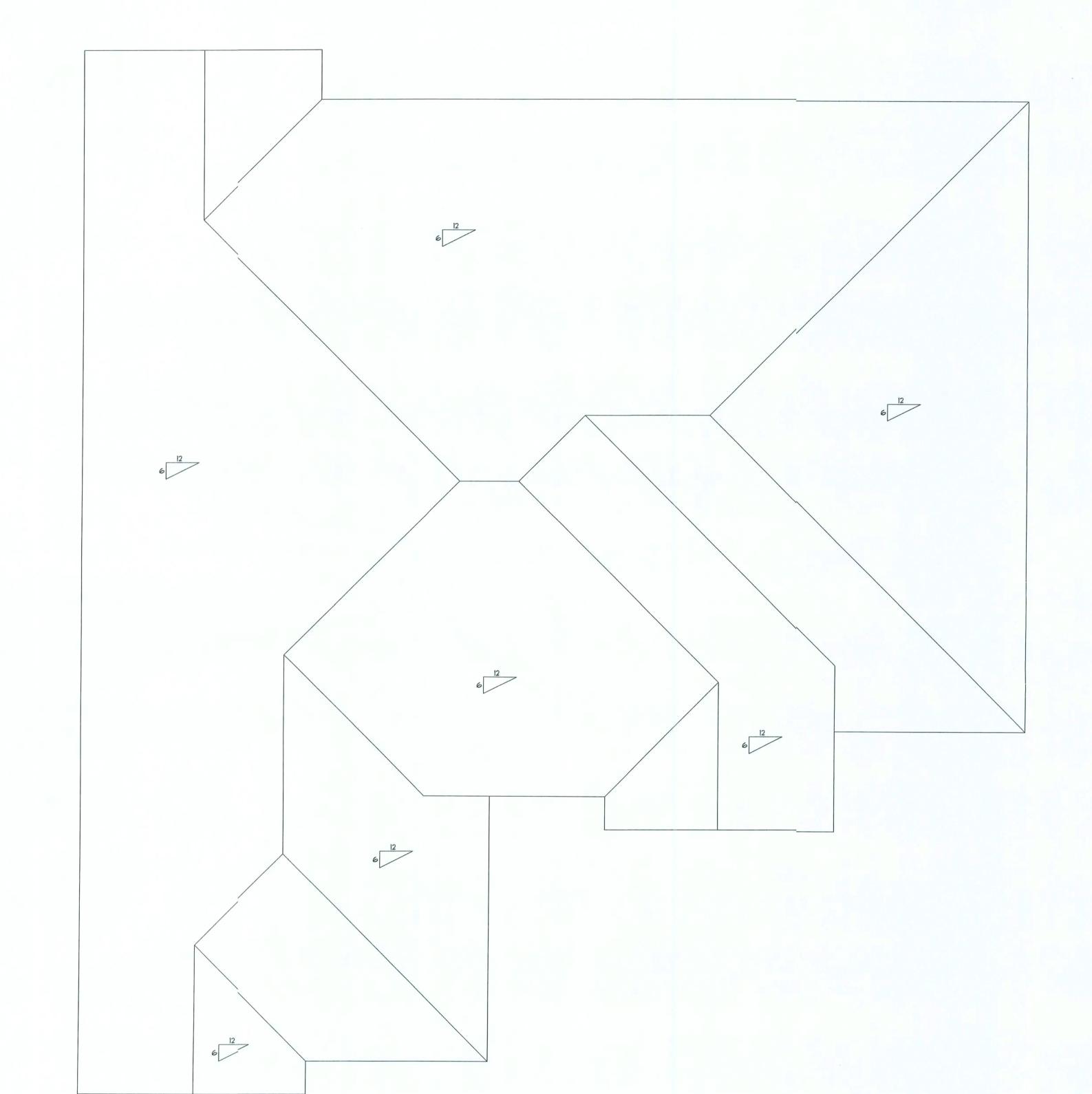
AREA SUMMARY

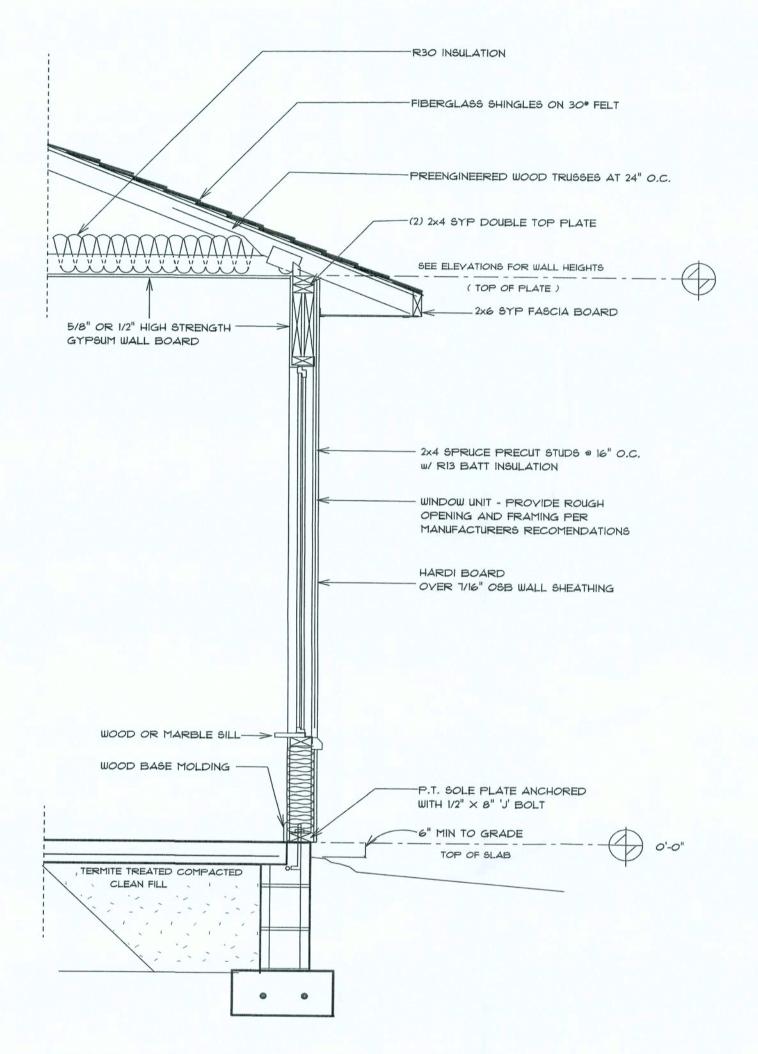
LIVING AREA - 1792,5 SF

TOTAL AREA - 2515.6 SF

551.1 SF

172.0 SF





TYPICAL WALL SECTION

2 × 4 STUD WALL W/ SIDING

ELECTRICAL PLAN SCALE: 1/4"=1'-0"

TOTAL AREA - 2515.6 SF

AREA SUMMARY

GARAGE -

PORCHES -

LIVING AREA - 1792.5 SF

172.0 SF

ELECTRICAL PLAN NOTES

ALL RECEPTICALS IN ALL BEDROOMS SHALL BE AFIC CIRCUITS

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

INSTALLATION SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECTING MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE 4-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

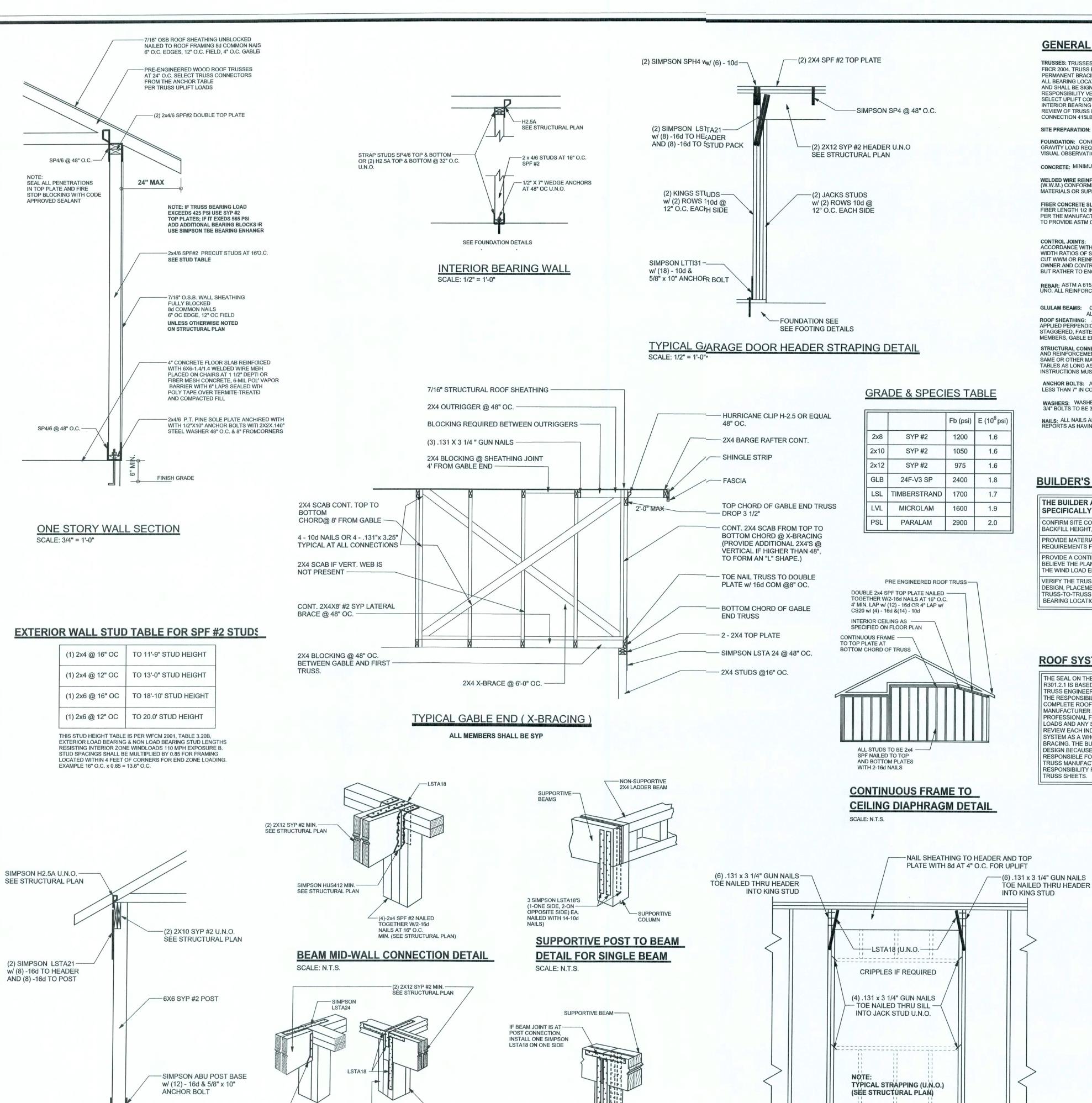
TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ELECTRICAL CONT'R SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION & BRKR, SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING/DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.

CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY.

	I	
ELECTRICAL	COUNT	SYMBOL
ceiling fan	4	
can light	10	
chandelier	1	900 900 900
double spotlight	2	₫₽
fluorescent fixture	5	
vanity bar light	2	88888
wall mount 1	2	Q
electrical panel	1	11
cable tv outlet	4	TV
fan with light	2	⊕
light	9	
outlet	23	Ф
outlet 220v	12	#
outlet gfi	6	⊕ cen
smoke detector	4	•
switch	28	\$
switch 3 way	16	\$3
telephone	4	∇

\$\$\$



4-SIMPSON LSTA18 ---

SUPPORTIVE CENTER POST TO BEAM DETFAIL

(2-ONE SIDE 2-ON

OTHER SIDE)

BEAM W/4-16d

BEAM MAY BE ATTACHED IN

EITHER METHOD SHOWN ABOVE

BEAM CORNER CONNECTION. DETAIL

SIMPSON HUS412 MIN.

SCALE: N.T.S.

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

SEE STRUCTURAL PLAN

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO. STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION

INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" \times 2" \times 9/64"; WITH 5/8" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 3/4" BOLTS TO BE 3" \times 3" \times 9/64"; WITH 7/8" BOLTS TO BE 3" \times 3" \times 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALLY	NOT PART OF T	HE WIND LO	AD ENGINEER	FOLLOWING, WHICE S SCOPE OF WORK	(.
CONFIRM SITE CON BACKFILL HEIGHT,	DITIONS, FOUNDAT WIND SPEED AND D	TION BEARING DEBRIS ZONE,	CAPACITY, GRAD AND FLOOD ZONE	E AND	
PROVIDE MATERIA REQUIREMENTS FO	LS AND CONSTRUC OR THE STATED WI	TION TECHNION TO VELOCITY	QUES, WHICH COM AND DESIGN PRES	IPLY WITH FBCR 2004 SSURES.	
BELIEVE THE PLAN	IUOUS LOAD PATH OMITS A CONTINUO IGINEER IMMEDIATE	OUS LOAD PA	ES TO FOUNDATIO TH CONNECTION, (N. IF YOU CALL	
DESIGN, PLACEME	MANUFACTURER'S NT PLANS, TEMPOR CONNECTIONS, ANI NS.	ARY AND PER	MANENT BRACING	DETAILS.	

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

Grout

CMU standard

Clay brick standard

Movement joints

Reinforcing bars, #3 - #11

ACI530.1-02 Section

IN WRITING.

3.3.E.7

-SP4 OR (2) H2.5A OR (2) SSP----

ALL OPENINGS (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

Coating for corrosion protection | Anchors, sheet metal ties completely

Coating for corrosion protection | Joint reinforcement in walls exposed to

3.3.E.2 | Pipes, conduits, and accessories | Any not shown on the project drawings

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

medium surface finish, 8"x8"x16" running

ASTM C 90-02, Normal weight, Hollow,

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

splices min 48 bar dia. (30" for #5)

bond and 12"x12" or 16"x16" column

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 950

< 745

< 1465

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*

H5

H2.5

H2.5A

H6

H14-1

H14-2

H16-1

H16-2

MTS24C

HTS24

2 - HTS24

LGT2

HEAVY GIRDER TIEDOWN

MGT

HGT-2

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SP6

SPH6

LSTA18

CS20

CS16

STUD ANCHORS

LTT19

LTTI31

HD2A

HTT16

PAHD42

ABU44

ABU66

LSTA21

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 6485

< 9035

< 9250

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TO PLATES TO RAFTERIRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 11/2'

12-8d, 1 1/2"

12-8d, 1 /2"

6-10d

7-10d 1 12"

12-10d 1 /2"

14 -160

22 -10c

16 -10c

16 -10c

16 -10c

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

6-10d

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

1 -10d

6-10d

2-10d

14-10d

16-10d

18-8d

28-8d

TO STUDS

18-10d, 1 1/2'

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

8-8d, 1 1/2" 8-8d, 1 1/2"

10-10d, 1 1/2" 2-10d, 1 /2"

10-10d, 1 1/2" | 2-10d, 1 /2"

TO STUDS

TO FOUNDATION

I-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROL

12" EMBEDMENT

TO STUDS

4-10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

04 RESIDENTIAL, \$ECTION R301.2.1
ITH FLAT, HIPPEC, OR GABLE ROOFS; IORIZONTAL DIMENSION OR 60 FT; NOT FT IN EXP. B, 30FTIN EXP. C AND >10% C HEIGHT OR 1 MILE WHICHEVER IS LESS.

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

BASIC WIND SPEED = 110 MPH

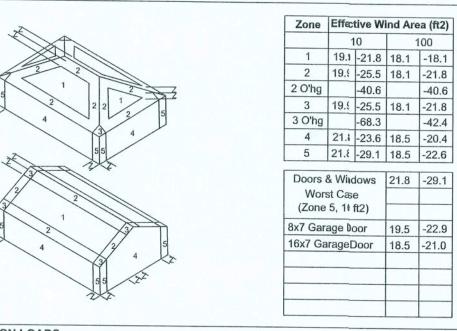
2.) WIND EXPOSURE = B WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES) MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



ESIGN	LOADS	
LOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	

12 PSF (12:12 AND GREATER)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOIL BEARING CAPACITY 1000PSF

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

David Disosway

JOB NUMBER: 602203 DRAWING NUMBER

OF 3 SHEETS

REVISIONS

VINDLOAD ENGINEER: Mark Disosway. PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 tated dimensions supercede scaled imensions. Refer all questions to Mark Disosway, P.E. for resolution o not proceed without clarification.

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CERTIFICATION: I hereby certify that I have amined this plan, and that the applicable portions of the plan, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one

building, at specified location.

MARK DISOSWAY P.E. 53915

> Concept Construction Of North Florida, Inc.

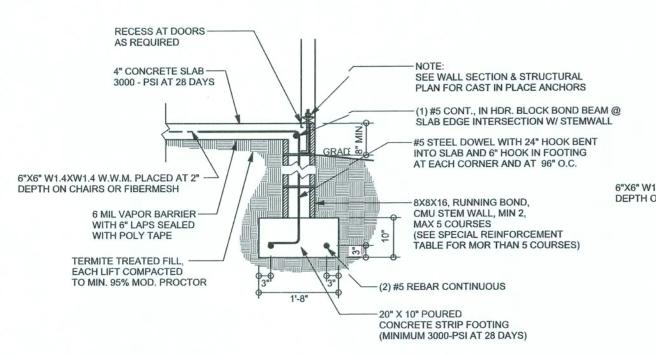
Spec House Lot 21 Wise Estates S/D

> ADDRESS: Lot 21 Wise Estates S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March 01, 2006 DRAWN BY: CHECKED BY

FINALS DATE: 01 / Mar / 06



—(1) #5 CONT., IN HDR. BLOCK BYOND BEAM @ SLAB EDGE INTERSECTION W// STEMWALL -#5 STEEL DOWEL WITH 24" HOLOK BENT INTO SLAB AND 6" HOOK IN FOLOTING AT EACH CORNER AND AT 96" (O.C. 6"X6" W1.4XW1.4 W.W.M. PLACED AT 2" ---DEPTH ON CHAIRS OR FIBERMESH -8X8X16, RUNNING BOND, 6 MIL VAPOR BARRIER — WITH 6" LAPS SEALED WITH POLY TAPE CMU STEM WALL, MIN 2, MAX 5 COURSES TERMITE TREATED FILL. EACH LIFT COMPACTED TO MIN. 95% MOD. PROCTOR (2) #5 REBAR CONTINOUS -20" X 10" POURED CONCRETE STRIP FOOTING (MINIMUM 3000-PSI AT 28 DAYS)

HOUSE SLAB

4" CONCRETE SLAB -

3000 - PSI AT 28 DAYS

ALT. STEM WALL PORCH FOOTING SCALE: 1/2" = 1'-0"

- PORCH POST SEE

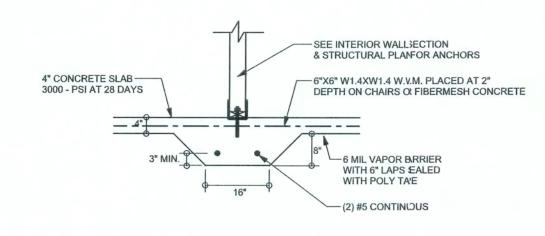
STRUCTURAL PLAN

SEE STRUCTURAL

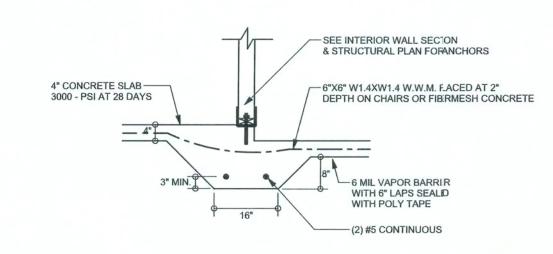
PLAN FOR CAST IN

PLACE ANCHORS

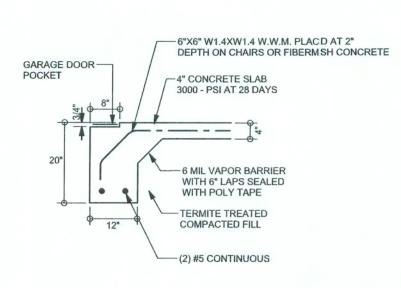
F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



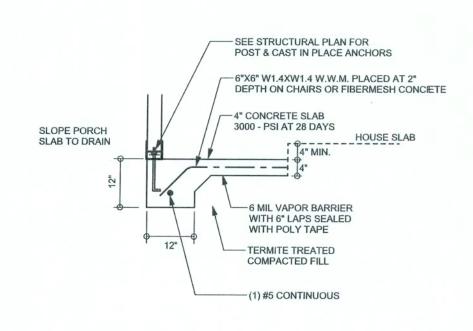
F2 INTERIOR BEARING FOOTING S-2 | SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING S-2 SCALE: 1/2" = 1'-0"



F4 GARAGE DOOR FOOTING S-2 SCALE: 1/2" = 1'-0"

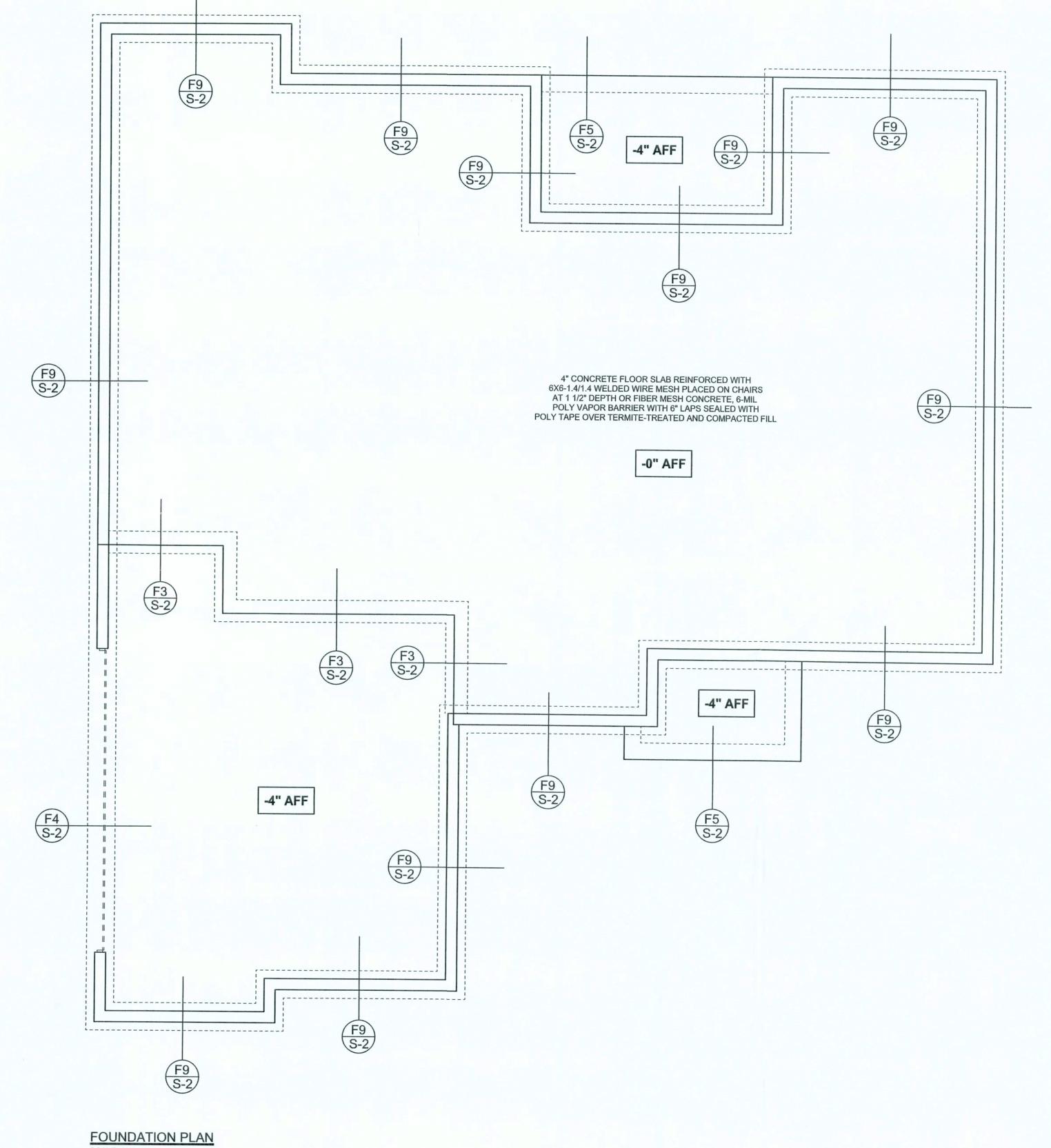


PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL UNBALANCED HEIGHT BACKFILL (FEET) HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
	#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



SCALE: 1/4" = 1'-0" DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

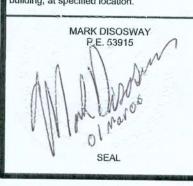
REVISIONS

VINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled

dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly resen its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any

form or manner without first the express writter permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Concept Construction Of North Florida, Inc

Spec House Lot 21 Wise Estates S/D

> ADDRESS: Lot 21 Wise Estates S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: March 01, 2006 DRAWN BY: CHECKED BY

David Disosway

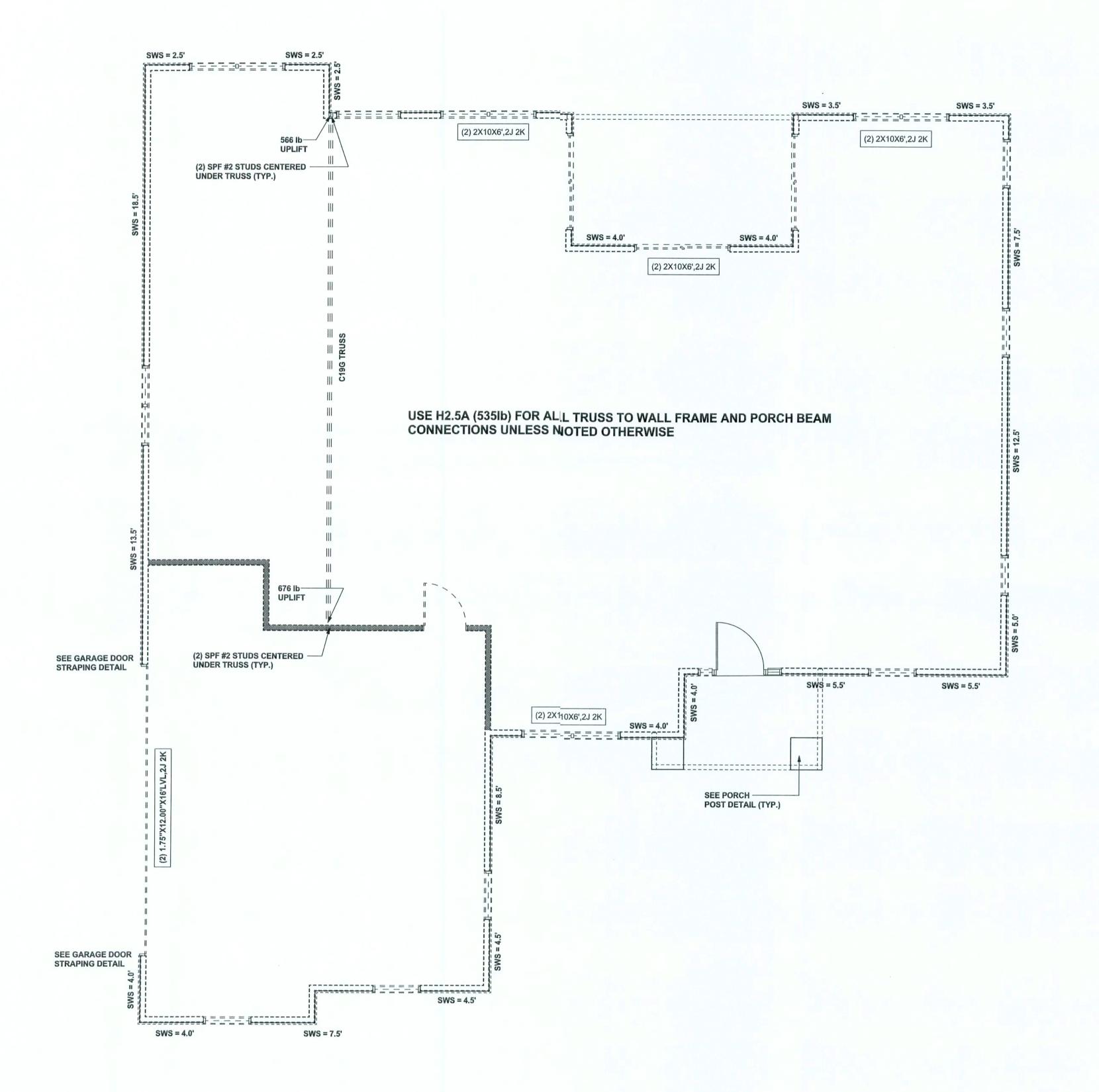
FINALS DATE: 01 / Mar / 06

JOB NUMBER: 602203 DRAWING NUMBER

> **S-2** OF 3 SHEETS

REVISIONS

SOFTPIAN ASSUMENTATION



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4

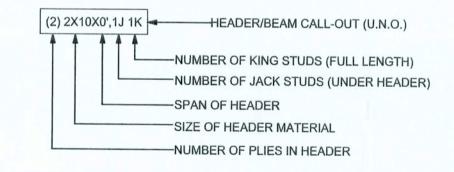
PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3. ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

sms = 0.0,	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

- 0.0 INDICATES	S SHEAR WA	ILL SEGMEN	
	REQUIRED	ACTUAL	
TRANSVERSE	34.5'	80.5'	
LONGITUDINAL	29.6'	51.5'	

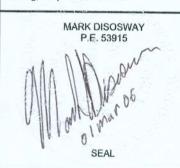
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Concept Construction Of North Florida, Inc.

Spec House Lot 21 Wise Estates S/D

ADDRESS: Lot 21 Wise Estates S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
March 01, 2006

DRAWN BY: CHECKED BY:
David Disosway

FINALS DATE: 01 / Mar / 06

JOB NUMBER: 602203 DRAWING NUMBER

> S-3 OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #3219