

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

39052

RE: 2135432 - CHEMERY CONST. - LOT 7 FWS

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: Chemery's Const. Project Name: Spec House Model: Custom

Subdivision: Fort White Station

Lot/Block: 7 Address: N/A, N/A

State: FL

City: Columbia Cty

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014

Design Program: MiTek 20/20 8.2

Wind Code: ASCE 7-10

Wind Speed: 130 mph

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 21 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T19096838	CJ01	1/10/20
23456789	T19096839 T19096840	CJ03 CJ05	1/10/20 1/10/20
4	T19096841	EJ01	1/10/20
5	T19096842	EJ02	1/10/20
9	T19096843 T19096844	HJ06 HJ10	1/10/20 1/10/20
8	T19096845	T01	1/10/20
	T19096846	T02	1/10/20
10 11	T19096847 T19096848	T03 T04	1/10/20 1/10/20
12	T19096849	T04G	1/10/20
13	T19096850	T05	1/10/20
14 15	T19096851 T19096852	T05G T06	1/10/20 1/10/20
16	T19096853	T07	1/10/20
17 18	T19096854 T19096855	T08 T09	1/10/20
19	T19096856	T109	1/10/20 1/10/20
20	T19096857	Ť11	1/10/20
21	T19096858	T11G	1/10/20

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

Job Truss Truss Type Qty Ply CHEMERY CONST. - LOT 7 FWS T19096838 2135432 CJ01 JACK-OPEN Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:19 2020 Page 1 ID:5lppGi3mlXfNNmnlqREYFDz9Eln-yGhXv1DfERiiif0Z6UbtQHrN11fHhcPK432QBTzwqJ6 Scale = 1:9.5 6.00 12 948 1-0-0

Plate Offs	Plate Offsets (X,Y)— [2:0-1-4,0-1-9]											
LOADING TCLL	(psf) 20.0	SPACING- Plate Grip DOL	2-0-0 1.25	CSI.	0.32	DEFL. Vert(LL)	in 0.00	(loc)	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.07	Vert(CT)	0.00	7	>999	180		
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code FBC2017/T	YES Pi2014	WB Matri	0.00 x-MP	Horz(CT)	0.00	2	n/a	n/a	Weight: 7 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 BRACING. TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-27/Mechanical, 2=254/0-3-8, 4=-46/Mechanical

Max Horz 2=66(LC 12)

Max Uplift 3=-27(LC 1), 2=-162(LC 12), 4=-46(LC 1)

Max Grav 3=25(LC 16), 2=254(LC 1), 4=44(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (ft=lb) 2=162.



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January 10,2020

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6904 Parke East Blvd. Tampa, FL 36610

Job CHEMERY CONST. - LOT 7 FWS Truss Truss Type Qty T19096839 2135432 CJ03 JACK-OPEN 1 Job Reference (optional)

8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07;27;20 2020 Page 1
ID:5lppGi3mlXfNNmntqREYFDz9Eln-yGhXv1DfERiiif0Z6UbtQHrN11fBhcPK432QBSzwqJ6 Builders FirstSource Jacksonville, FL - 32244, Scale = 1:14.6 144 6.00 12 1-5-13 9 3-0-0 Plate Offsets (X,Y)-[2:0-0-3,0-0-5] LOADING (psf) SPACING-DEFL. **PLATES** CSI. (loc) l/defl L/d **GRIP** TCLL 20.0 Plate Grip DOL 1.25 TC 0.32 Vert(LL) -0.00 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.07 -0.01 >999 180 Vert(CT) BÇLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 BCDL 10.0 Code FBC2017/TPI2014 Matrix-MP Weight: 13 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 3=52/Mechanical, 2=253/0-3-8, 4=20/Mechanical

Max Horz 2=113(LC 12)

Max Uplift 3=-48(LC 12), 2=-126(LC 12) Max Grav 3=52(LC 1), 2=253(LC 1), 4=48(LC 3)

FORCES. (lb) - Max, Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=126.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd, Tampa FL 33610 Date:

January 10,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-1473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ______ATMITTHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Truss Type Qty CHEMERY CONST. - LOT 7 FWS Ply T19096840 2135432 CJ05 Jack-Open | Job Reference (optional) 8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:20 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:5lppGi3mlXfNNmnlqREYFDz9Eln-QTFv6NEI?lqZJoblgB66zUOYnRz?Q3eTJjnzjvzwqJ5 -2-0-0 2-0-0 5-0-0 Scale = 1:19.5 6.00 12 8 5-0-0 LOADING (psf) SPACING-CSI. DEFL. PLATES in **I/defi** L/d GRIP (loc) TCLL 20.0 Plate Grip DOL 1.25 TC 0.32 Vert(LL) 0.03 4-7 >999 240 244/190 **MT20 TCDL** 7.0 Lumber DOL 1.25 BC 0.23 Vert(CT) -0.05 4-7 >999 180

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

0.00

3

n/a

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-0-0 oc purlins.

Weight: 19 lb

FT = 20%

LUMBER-

BCLL

BCDL

2x4 SP No.2 TOP CHORD **BOT CHORD**

0.0 *

10.0

2x4 SP No.2

REACTIONS. (lb/size) 3=108/Mechanical, 2=313/0-3-8, 4=53/Mechanical

Max Horz 2=162(LC 12)

Max Uplift 3=98(LC 12), 2=137(LC 12), 4=1(LC 12) Max Grav 3=108(LC 1), 2=313(LC 1), 4=87(LC 3)

Code FBC2017/TPI2014

Rep Stress Incr

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

WB 0.00

Matrix-MP

- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb)



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ANSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd. Tampa, FL 36610

Job Truss Truss Type Qty Ply CHEMERY CONST. - LOT 7 FWS T19096841 2135432 EJ01 Jack-Partial 5 1 Job Reference (optional) 8,240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:21 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:5lppGi3mlXfNNmnlqREYFDz9Eln-vfpHKjFwl2yQxyAxEvdLViwe7rFw9WucXNXXFMzwqJ4 7-0-0 Scale: 1/2"=1" 6.00 12 A 7-0-0

Plate Offsets (X,Y) [2:0-1-13,0-1-8]											
E.OADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TP 2014	CSI. TC 0.67 BC 0.51 WB 0.00 Matrix-MS	DEFL. in (loc) I/defl L/d Vert(LL) 0.12 4-7 >670 240 Vert(CT) -0.21 4-7 >393 180 Horz(CT) 0.01 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 26 lb FT = 20%							

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 **BRACING-**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=160/Mechanical, 2=380/0-3-8, 4=81/Mechanical

Max Horz 2=144(LC 12)

Max Uplift 3=-94(LC 12), 2=-81(LC 12)

Max Grav 3=160(LC 1), 2=380(LC 1), 4=125(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2,



MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Type Truss Qty Ply CHEMERY CONST. - LOT 7 FWS T19096842 EJ02 2135432 Jack-Closed 11 Job Reference (optional)
8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:22 2020 Page 1 Builders FirstSource, Jacksonville, FL - 32244, ID:5lppGi3mlXfNNmnlqREYFDz9Eln-NrNgX3GYWM4HZ6l7oc8a2vTuHFgduz8mm1G4nozwqJ3 Scale = 1:17.0 2x4 || 3 6.00 12 6 2x4 || Plate Offsets (X,Y)-[2:0-1-4,0-1-9] LOADING (psf) SPACING-2-0-0 CSI. DEFL. **Vdefi PLATES GRIP** TC BC TCLL 20.0 Plate Grip DOL 1.25 0.32 Vert(LL) 0.02 6-9 >999 240 MT20 244/190 **TCDL** 7.0 Lumber DOL 1.25 0.16 Vert(CT) 0.02 6-9 >999 180 0.0 * **BCLL** Rep Stress Incr YES WB 0.00 Horz(CT) -0.00 **BCDL** 100 Code FBC2017/TPI2014 Matrix-MP Weight: 19 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2x4 SP No 2

2x4 SP No.3 WEBS

(lb/size) 6=118/Mechanical, 2=274/0-3-8

Max Horz 2=138(LC 12)

Max Uplift 6=-80(LC 9), 2=-125(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

REACTIONS.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (it=lb) 2=125.



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Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS
				1	T19096843
2135432	HJ06	Diagonal Hip Girder	1	1	
			1		Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s De	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:23 2020 Page 1
			ID:5lppGi3mtXfi	NmniqRE	YFDz9Ein-r2x2iPHAHgC8AGKKLKfpb700OfxrdQOv?h0dKEzwqJ2
	-2-9-15	1		5-7-2	
	2-9-15			5-7-2	

Scale = 1:17.8

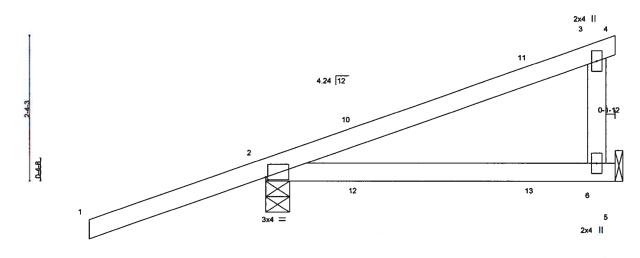


Plate Off	Plate Offsets (X,Y)— [2:0-0-6,0-0-8]												
LOADIN		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	Vdefl	L/d	PLATES	GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	- 0.10	6-9	>669	240	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.41	Vert(CT)	-0.10	6-9	>663	180			
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a			
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MP						Weight: 24 lb	FT = 20%	

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING-TOP CHORD

Structural wood sheathing directly applied or 5-7-2 oc purlins,

except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

5-7-2 5-7-2

REACTIONS. (lb/size) 6=128/Mechanical, 2=303/0-4-9 Max Horz 2=160(LC 4)

Max Uplift 6=-93(LC 5), 2=-234(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb)
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 83 lb down and 103 lb up at 1-6-1, and 83 lb down and 103 lb up at 1-6-1, and 25 lb down and 38 lb up at 4-4-0 on top chord, and 69 lb down and 74 lb up at 1-6-1, 69 lb down and 74 lb up at 1-6-1, and 56 lb down and 2 lb up at 4-4-0, and 56 lb down and 2 lb up at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-14, 5-7=-20

Concentrated Loads (lb)

Vert: 10=50(F=25, B=25) 12=70(F=35, B=35) 13=5(F=2, B=2)

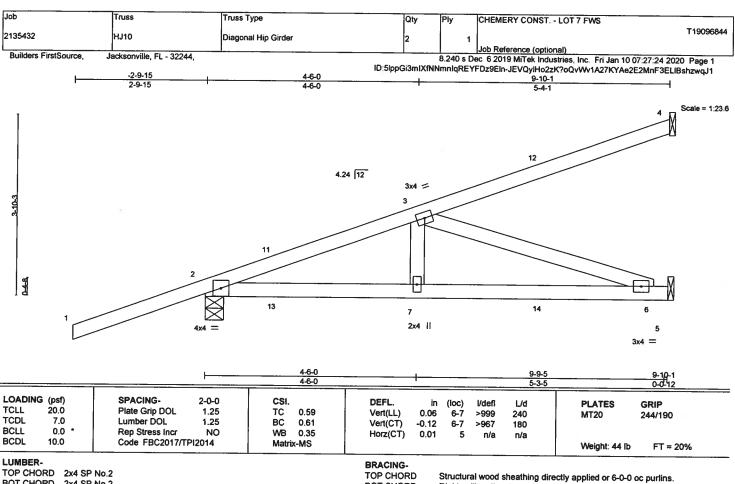


Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

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BOT CHORD

Rigid ceiling directly applied or 9-9-0 oc bracing.

TOP CHORD **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 **WEBS**

REACTIONS. (lb/size) 4=150/Mechanical, 2=463/0-4-9, 5=251/Mechanical

Max Horz 2=233(LC 22)

Max Uplift 4=-141(LC 4), 2=-264(LC 4), 5=-103(LC 8) Max Grav 4=150(LC 1), 2=463(LC 1), 5=266(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-628/252

BOT CHORD 2-7=-327/573, 6-7=-327/573

WEBS

3-6=-603/345

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. It; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=141, 2=264, 5=103.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 86 lb down and 103 lb up at 1-6-1, 86 lb down and 103 lb up at 1-6-1, 26 lb down and 38 lb up at 4-4-0, 26 lb down and 38 lb up at 4-4-0, and 50 lb down and 97 lb up at 7-1-15, and 50 lb down and 97 lb up at 7-1-15 on top chord, and 36 lb down and 74 lb up at 1-6-1, 36 lb down and 74 lb up at 1-6-1, 28 lb down and 2 lb up at 4-4-0, 28 lb down and 2 lb up at 4-4-0, and 44 lb down and 15 lb up at 7-1-15, and 44 lb down and 15 lb up at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 5-8=-20

Concentrated Loads (lb)

Vert: 7=5(F=2, B=2) 11=50(F=25, B=25) 12=-64(F=-32, B=-32) 13=70(F=35, B=35) 14=-49(F=-24, B=-24)



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

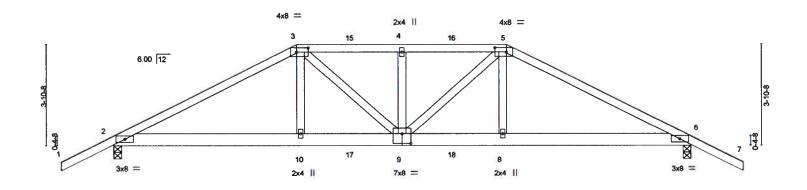
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6904 Parke East Blvd Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
		100 TO 100				T19096845
2135432	T01	Hip Girder	1	1	1	
1					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s D	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 0	7:27:25 2020 Page 1
			ID:5lppGi3mtXfNN	ImniqREY	FDz9Eln-nQ3oA5lQpHSsQaUiTkiHgY5GeSZr	5GBCS?VkO7zwqJ0
, -2-0-0	7-0-0	11-0-0	15-0-0		22-0-0	24-0-0
2-0-0	7-0-0	4-0-0	4-0-0	- '	7-0-0	2-0-0

Scale = 1:42.4



	-	7-0-0 7-0-0		+	11-0-0 4-0-0		-0-0 -0-0				22-0-0 7-0-0	
Plate Offse	ts (X,Y)-	[3:0-5-4,0-2-0], [5:0-5-4,0	-2-0], [9:0-4-0,	0-4-8]	4-0-0						7-0-0	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	20.0	Plate Grip DOL	1.25	TC	0.95	Vert(LL)	-0.11	9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.70	Vert(CT)	-0.20	9	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.24	Horz(CT)	0.06	6	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	-MS						Weight: 122 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied or 8-2-9 oc bracing.

LUMBER-

REACTIONS.

2x4 SP No.2 TOP CHORD

BOT CHORD 2x6 SP No.2 WEBS

2x4 SP No.3

(lb/size) 2=1586/0-3-8, 6=1613/0-3-8

Max Horz 2=-61(LC 6)

Max Uplift 2=-495(LC 8), 6=-534(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2831/898, 3-4=-2831/954, 4-5=-2831/954, 5-6=-2890/982

BOT CHORD 2-10=-752/2460, 9-10=-755/2482, 8-9=-800/2534, 6-8=-797/2513

3-10=-93/624, 3-9=-267/564, 4-9=-465/267, 5-9=-143/480, 5-8=-91/623 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (tl=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 125 lb down and 100 lb up at 7-0-0, 106 lb down and 100 lb up at 9-0-12, 106 lb down and 100 lb up at 11-0-0, and 106 lb down and 100 lb up at 12-11-4, and 227 lb down and 250 lb up at 15-0-0 on top chord, and 294 lb down and 131 lb up at 7-0-0, 85 lb down at 9-0-12, 85 lb down at 11-0-0, and 85 lb down at 12-11-4, and 294 lb down and 131 lb up at 14-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

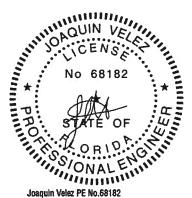
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-6=-20

Concentrated Loads (lb)

Vert: 3=-106(F) 5=-180(F) 10=-284(F) 9=-61(F) 4=-106(F) 8=-284(F) 15=-106(F) 16=-106(F) 17=-61(F) 18=-61(F)



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020

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Job	Truss	Truss Type		In:	In	
30D	iiuss	Truss Type		Qty	Ply	CHEMERY CONST LOT 7 FWS
2135432	T02	Hip		1	1	T19096846
		L		ı		Job Reference (optional)
Builders FirstSource, Ja	acksonville, FL - 32244.					
					0.240 5 De	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:26 2020 Page 1
200	400		ID:5	ppGi3mIX	fNNmnlqRE	REYFDz9Eln-FddANRJ2abai1j2v1SDWCldZhsv9qkoMhfElwZzwqJ?
-2-0-0	4-9-8	9-0-0 ,	13-0-0		17-2	2-8 22-0-0 24-0-0
2-0-0	4-9-8	4-2-8	4-0-0	_	4-2-	

Scale = 1 42.4

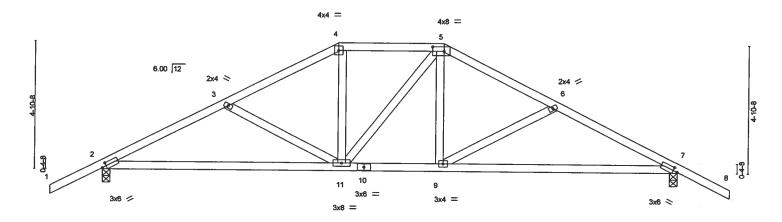


Plate Offsets (X,Y)—	9-0-0 [2:0-1-15,0-1-8], [5:0-5-4,0-2-0], [7:0-1-1	15 0-1-81	4-0-0	+ -		22-0-0 9-0-0			
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.42 BC 0.69 WB 0.15 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.16 9-17 -0.32 9-17 0.04	>999	L/d 240 180 n/a	PLATES MT20 Weight: 111 lb	GRIP 244/190 FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD**

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=922/0-3-8, 7=922/0-3-8 Max Horz 2=-75(LC 10)

Max Uplift 2=-199(LC 12), 7=-199(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-3=-1378/742, 3-4=-1115/596, 4-5=-952/583, 5-6=-1114/595, 6-7=-1378/742

BOT CHORD 2-11=-513/1207, 9-11=-294/951, 7-9=-539/1207 WEB\$

3-11=-300/281, 4-11=-90/318, 5-9=-97/318, 6-9=-300/281

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=199, 7=199.



Structural wood sheathing directly applied or 4-10-0 oc purlins.

Rigid ceiling directly applied or 7-11-15 oc bracing.

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020

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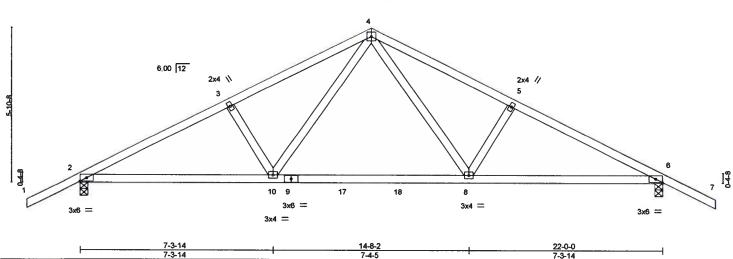


6904 Parke East Blvd. Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
2135432	тоз	Common	6	1		T19096847
					Job Reference (optional)	
Builders FirstSource,	Jacksonville, FL - 32244,			8,240 s D	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27	27 2020 Page 1
			ID:5lppG	3mlXfNNn	nniqREYFDz9Ein-jpAZbmKhLuiZftd5a9kltzAihGl3Z	_Lpwz?Tr_lwVv8
2-0-0	5-8-5	11-0-0	10	3-11	22-0-0	24-0-0
2-0-0	5-8-5	5-3-11	5	-3-11	5-8-5	2-0-0

4x4 =

Scale = 1:41,9



. 000	7-3-14		'		7-4-5					7-3-14	
ts (X,Y)	[6:0-2-15,Edge]				T						
(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
20.0	Plate Grip DOL	1.25	TC	0.47	Vert(LL)	0.22	8-10	>999	240	MT20	244/190
7.0	Lumber DOL	1.25	BC	0.46	Vert(CT)	-0.36	8-10	>742	180	E 100	
0.0 *	Rep Stress Incr	NO	WB	0.35	Horz(CT)	0.04	6	n/a	n/a		
10.0	Code FBC2017/TI	PI2014	Matrix	-MS	_ ` '					Weight: 105 lb	FT = 20%
-	20.0 7.0 0.0 *	(x,Y)- [6:0-2-15,Edge]	(s (X,Y)- [6:0-2-15,Edge]	(s (X,Y)- [6:0-2-15,Edge] (psf) SPACING- 2-0-0 CSI. (20.0 Plate Grip DOL 1.25 TC 7.0 Lumber DOL 1.25 BC (0.0 ° Rep Stress Incr NO WB	(s (X,Y)- [6:0-2-15,Edge] (psf) SPACING- 2-0-0 CSI. (20.0 Plate Grip DOL 1.25 TC 0.47 (7.0 Lumber DOL 1.25 BC 0.46 (0.0 ° Rep Stress Incr NO WB 0.35 (1.25 Rep Stress Incr NO WB 0.35 (1.25 Rep Stress Incr NO WB 0.3	S (X,Y)- [6:0-2-15,Edge]	S (X,Y)- [6:0-2-15,Edge]	S (X,Y)- [6:0-2-15,Edge]	S (X,Y)- [6:0-2-15,Edge]	S (X,Y)- [6:0-2-15,Edge]	S (X,Y)- [6:0-2-15,Edge]

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP M 31 WEBS

2x4 SP No.3

REACTIONS. (lb/size) 2=1143/0-3-8, 6=1143/0-3-8 Max Horz 2=88(LC 11)

Max Uplift 2=-267(LC 12), 6=-267(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1913/1009, 3-4=-1770/1009, 4-5=-1770/1009, 5-6=-1913/1009 BOT CHORD 2-10=-745/1655, 8-10=-396/1111, 6-8=-765/1655

WEB\$ 4-8=-406/757, 5-8=-268/285, 4-10=-406/757, 3-10=-268/285

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=267, 6=267.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=54, 4-7=54, 10-11=-20, 8-10=-80(F=-60), 8-14=-20



Structural wood sheathing directly applied or 4-0-8 oc purlins.

Rigid ceiling directly applied or 8-9-10 oc bracing.

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Job Truss Truss Type Qty CHEMERY CONST. - LOT 7 FWS Ply T19096848 2135432 T04 COMMON 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:28 2020 Page 1 ID:5lppGi3mlXfNNmnlqREYFDz9Eln-B?kxo6KJ6CqQH1CH8tF_IAjvXfgElf6e8yjO_Szwqlz 11-4-0 Scale = 1:22.4 4x6 = 6.00 12 5 2x4 || Plate Offsets (X,Y)-[4:0-2-15,Edge] LOADING (psf) SPACING-CSI. DEFL. (loc) l/defi L/d **PLATES GRIP** TCLL 20.0 Plate Grip DOL 1.25 TC 0.34 Vert(LL) 0.05 5-8 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 вс 0.33 Vert(CT) -0.075-8 >999 180 0.0 * **BCLL** Rep Stress Incr YES WB 0.10 Horz(CT) 0.01 n/a n/a BCDL 10.0 Code FBC2017/TPI2014 Matrix-MS Weight: 43 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3 **BRACING-**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purtins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 4=410/0-3-8, 2=537/0-3-8

Max Horz 2=101(LC 12)

Max Uplift 4=-154(LC 13), 2=-231(LC 12)

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown. TOP CHORD 2-3=-571/342, 3-4=-567/338

BOT CHORD

2-5=-201/455, 4-5=-201/455

WEBS

3-5=-25/255

1) Unbalanced roof live loads have been considered for this design.

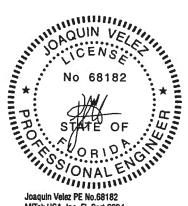
2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=154, 2=231.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

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ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Com, Sefety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd Tampa, FL 36610

)	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS	
35432	T04G	Common Supported Gable	1	1		T19096849
					Job Reference (optional)	
uilders FirstSource, Ja	cksonville, FL - 32244,			8.240 s De	ec 6 2019 MiTek Industries, Inc. Fri Jan 10	07:27:29 2020 Page 1
			ID:5lppGi3mlXfl	NmnlqRE'	YFDz9Eln-gCIJ?SLxtWzHuBnUiamDqOF5f3	34L172oNcTyWuzwqly
2-0-0		5-8-0			11-4-0	13-4-0
' 2-0-0	1	5-8-0	,		5-8-0	2-0-0
	35432 Builders FirstSource, Ja	35432 T04G Builders FirstSource, Jacksonville, FL - 32244,	35432 T04G Common Supported Gable Builders FirstSource, Jacksonville, FL - 32244, -2-0-0 5-8-0	35432 T04G Common Supported Gable 1 Builders FirstSource, Jacksonville, FL - 32244, ID:5lppGi3mlXff	35432 T04G Common Supported Gable 1 1 1 Builders FirstSource, Jacksonville, FL - 32244,	35432 T04G Common Supported Gable 1 1 1 Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, Builders FirstSource, FL - 32244, Builders FirstSource, FL - 32244, Builders FirstSource, FL - 32244, Builders Fi

Scale = 1:25.4

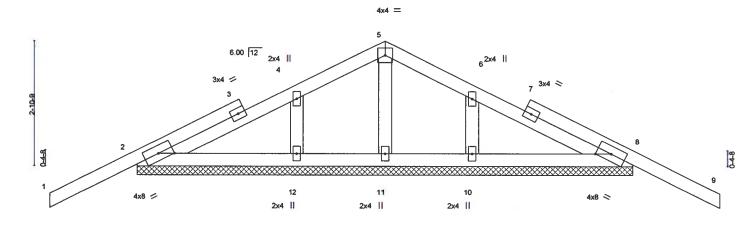


Plate Offsets (X,Y)-	Plate Offsets (X,Y)— [2:0-4-0,0-1-15], [8:0-4-0,0-1-15]												
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.31 BC 0.09 WB 0.05 Matrix-S	DEFL. in (loc) I/defl L/d Vert(LL) -0.02 9 n/r 120 Vert(CT) -0.02 9 n/r 120 Horz(CT) 0.00 8 n/a n/a	PLATES GRIP MT20 244/190 Weight: 56 lb FT = 20%									

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 11-4-0.

(lb) - Max Horz 2=47(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 12, 10

Max Grav All reactions 250 tb or less at joint(s) 11, 12, 10 except 2=258(LC 23), 8=258(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4,2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 12, 10.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

January 10,2020



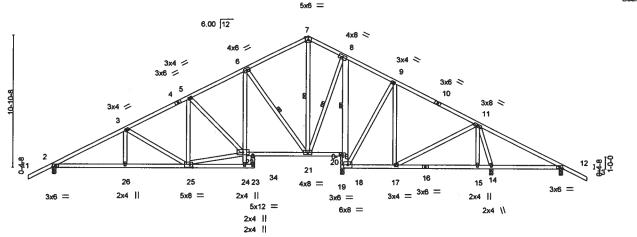
Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS
2135432	T05	Roof Special	1	1	T19096850
					Job Reference (optional)
Builders FirstSource,	Jacksonville, FL - 32244,			8.240 s D	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:31 2020 Page 1
			ID:5lppGi3mIXft	INmnloREY	FDz9Fln_ca0308NRP7D28Vysn2ohyol OFtd\\\\v\X50m3hozyyohy

28-1-12

35-0-0

21-0-0 5-0-0

Scale = 1:91.1



							23-10-4						
		6-0-0	11-2-4	16-0-0	_16-8-0	21-0-0	23-8-8	28-1-12		35-0-0	36-3-8	42-0-0	
		6-0-0	5-2-4	4-9-12	0 <u>1</u> 8-6	4-4-0	2-8-80-1-12	4-3-8		6-10-4	1-3-8	5-8-8	
Plate Offse	ets (X,Y)-	[12:0-2-15,Edge], [25:0-2-	12.0-3-01										
				<u> </u>									
LOADING	(nsf)	SPACING-	2-0-0	CSI.			DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
	. ,				0.50	- 1							
TCLL	20.0	Plate Grip DOL	1.25	TC	0.50	- 1	Vert(LL)	0.10	25-26	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	l BC	0.62		Vert(CT)	-0.10	25-26	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	l ws	0.57		Horz(CT)	0.02	12	n/a	n/a		
						- 1	11012(01)	0.02	12	100	11/4		
BCDL	10.0	Code FBC2017/TF	12014	Matri	x-MS							Weight: 291 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.2 *Except*

6-24: 2x4 SP No.3, 8-18: 2x6 SP No.2

WEBS 2x4 SP No.3

BRACING-

WEBS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-8-11 oc purlins. Rigid ceiling directly applied or 4-9-15 oc bracing. Except:

1 Row at midpt 8-20

6-0-0 oc bracing: 18-20

10-0-0 oc bracing: 22-24

1 Row at midpt

6-21, 7-21, 8-21

REACTIONS. All bearings 0-3-8.

(lb) - Max Horz 2=242(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-427(LC 9), 18=-527(LC 9), 14=-317(LC 13), 12=-189(LC

13

Max Grav All reactions 250 lb or less at joint(s) except 2=923(LC 1), 18=1593(LC 1), 14=589(LC 24), 12=321(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1389/1599, 3-5=-949/1181, 5-6=-636/889, 6-7=-175/426, 7-8=-172/441,

8-9=-32/395, 9-11=-202/278

2-26=-1369/1186, 25-26=-1369/1186, 6-22=-729/579, 21-22=-364/581, 20-21=-242/482,

18-20=-1181/1081, 8-20=-1138/1011, 17-18=-98/255

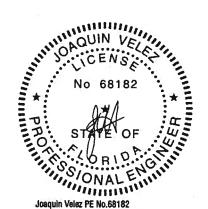
WEBS 3-25=-458/609, 5-25=-312/241, 22-25=-785/776, 5-22=-386/556, 6-21=-766/945,

8-21=-764/881, 9-18=-457/386, 9-17=-101/298, 11-14=-478/272

NOTES.

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 427 lb uplift at joint 2, 527 lb uplift at joint 18, 317 lb uplift at joint 14 and 189 lb uplift at joint 12.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

January 10,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and lot for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent localizes with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

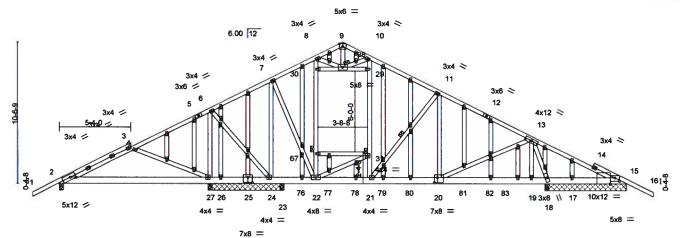
ANSI/TP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Job Ply CHEMERY CONST. - LOT 7 FWS Truss Truss Type T19096851 2135432 T05G GABLE Job Reference (optional) Builders FirstSource lacksonville FL - 32244 8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:35 2020 Page 1

ID:5lppGi3mIXfNNmnlqREYFDz9Eln-ULfaGWQiSMjRc6Ed2rtd4fV34U01ReygIYwGkYzwqls 21-0-0 23-0-0 25-10-14 2-0-0 2-0-0 2-10-14

Scale = 1:82.3



		5-8-0	11-0-8	11-2-4	16-4-8 15-10-4 19-0-	0 21-0-0 23-0-0	28-1-12	ř.	35-0-0	36-0-Q 42-0	0 ,
		5-8-0	5-4-8	0-1-12 2-8-0	2-0-0 0 6-4 2-7-4	2-0-0 2-0-0	5-1-12		6-10-4	100 60	0
Plate Offse	ts (X,Y)-	[2:0-4-12,Edge], [15:0-4-0	0,Edge], [15:0	-4-0,0-3-1], [2	0:0-2-12,0-4-8	3], [25:0-4-0,0-4-8	3], [39:0-1-13,0	-1-0]			
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	тс	0.57	Vert(LL)	0.12 20-21	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.52	Vert(CT)	-0.14 20-21	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.86	Horz(CT)	0.02 18	n/a	n/a		
BCDL	10.0	Code FBC2017/TI	PI2014	Matri	x-MS					Weight: 412 I	b FT = 20%

LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 4-7-8 oc purlins. **BOT CHORD** 2x6 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2x4 SP No.3 WEBS 6-0-0 oc bracing: 17-18,15-17. **OTHERS** 2x4 SP No.3 **WEBS** 1 Row at midet 11-21 1 Brace at Jt(s): 28, 31, 67 **JOINTS**

REACTIONS. All bearings 0-3-8 except (jt=length) 24=5-7-8, 27=5-7-8, 27=5-7-8, 17=6-0-0, 15=6-0-0, 26=5-7-8, 15=6-0-0

(lb) - Max Horz 2=235(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) except 2=-272(LC 4), 24=-542(LC 8), 27=-643(LC 5), 17=-151(LC

23), 15=-136(LC 24), 26=-423(LC 13), 23=-282(LC 20), 18=-1267(LC 9)

All reactions 250 lb or less at joint(s) 17, 15, 23, 15 except 2=653(LC 1), 24=846(LC 1), 27=1237(LC 1), 27=1237(LC 1), 26=373(LC 24), 18=2025(LC 1), 18=2025(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=780/693, 4-6=404/348, 6-7=-694/580, 7-8=-917/739, 8-9=-501/399,

9-10=-482/379, 10-11=-1043/751, 11-13=-1326/839, 13-15=-458/733

BOT CHORD 2-27=-372/706, 26-27=-85/336, 24-26=-85/336, 23-24=-135/579, 22-23=-135/579,

21-22=-297/916, 20-21=-461/1114, 17-18=-594/527, 15-17=-594/527

WEBS 9-28=-325/361, 11-21=-416/382, 13-20=-585/1097, 13-19=-188/293, 7-22=-263/567,

6-24--204/404, 6-27--734/412, 4-27--441/339, 7-24--807/478, 13-18--1958/1257 21-31=-307/409, 29-31=-245/339, 10-29=-246/339, 8-28=-266/280, 10-28=-647/542

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) na
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 272 lb uplift at joint 2, 542 lb uplift at joint 24, 643 lb uplift at joint 27, 151 lb uplift at joint 17, 136 lb uplift at joint 15, 423 lb uplift at joint 26, 282 lb uplift at joint 23, 1267 Ib uplift at joint 18 and 136 lb uplift at joint 15.



Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS
2135432	T05G	GABLE	1	1	T19096851
0.0			<u>'</u>	<u> </u>	Job Reference (optional)

Builders FirstSource

Jacksonville, FL - 32244.

8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:35 2020 Page 2 ID: 5lppGi3mlXfNNmnlqREYFDz9Eln-ULfaGWQiSMjRc6Ed2rtd4fV34U01ReyglYwGkYzwqls

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 98 lb down and 100 lb up at 11-10-3, 98 lb down and 100 lb up at 13-10-3, 98 lb down and 100 lb up at 15-10-3, 98 lb down and 100 lb up at 17-10-3, 98 lb down and 100 lb up at 19-10-3, 98 lb down and 100 lb up at 21-10-3, 98 lb down and 100 lb up at 23-10-3, 98 lb down and 100 lb up at 25-10-3, 98 lb down and 100 lb up at 27-10-3, 98 lb down and 100 lb up at 29-10-3, and 98 lb down and 100 lb up at 31-10-3, and 102 lb down and 128 lb up at 33-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=54, 3-4=54, 4-9=54, 9-16=-54, 2-75=-20

Concentrated Loads (lb)

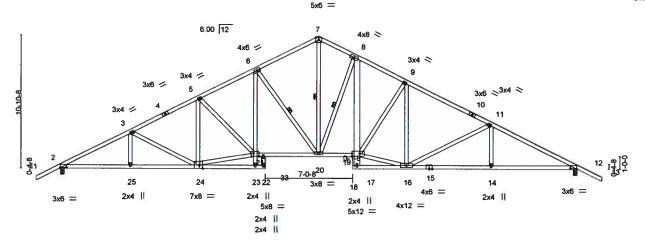
Vert: 20=-98(F) 24=-98(F) 26=-98(F) 25=-98(F) 75=-98(F) 77=-98(F) 78=-98(F) 79=-98(F) 80=-98(F) 81=-98(F) 82=-98(F) 83=-100(F)



Truss	Truss Type	Qty	Ply	CHEMERY CONST LOT 7 FWS
T06	Roof Special	3	1	T19096
	i i			Job Reference (optional)
Jacksonville, FL - 32244,			8,240 s D	ec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:37 2020 Page 1
	T06	T06 Roof Special	T06 Roof Special 3 Jacksonville, FL - 32244,	T06 Roof Special 3 1

IeMvYgzDsPNoQzwqlq 11-2-12 16-0-0 35-0-0 42-0-0 44-0-0 2-0-0

Scale = 1:90.4



		5-8-0	11-2-12 5-6-12	16-0-0 4-9-4	16-8-0 21-0-0 0-8-0 4-4-0	23-8-8	28-1-12 4-5-4	35-0-0 6-10-4		42-0-0 7-0-0	
Plate Offs	ets (X,Y)-	[2:0-6-0,0-0-3], [12:0-6-0						6-10-4		7-0-0	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc) I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	0.24 2	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	ВС	0.72	Vert(CT)	-0.44 20-21	>999	180	i	
BCLL	0.0 *	Rep Stress Incr	YES	₩8	0.84	Horz(CT)	0.18 1:	2 n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matr	ix-MS	. ,				Weight; 291 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

6-23; 2x4 SP No.3, 8-17; 2x6 SP No.2

WEBS 2x4 SP No 3

BRACING-TOP CHORD

WERS

BOT CHORD

Structural wood sheathing directly applied or 3-0-1 oc purlins. Rigid ceiling directly applied or 5-3-4 oc bracing. Except:

10-0-0 oc bracing: 21-23, 17-19

1 Row at midpt 6-20, 7-20, 8-20

REACTIONS. (lb/size) 2=1675/0-3-8, 12=1672/0-3-8

Max Horz 2=-242(LC 13)

Max Uplift 2=-645(LC 12), 12=-646(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-3037/1569, 3-5=-2617/1421, 5-6=-2541/1422, 6-7=-2000/1214, 7-8=-1954/1229, TOP CHORD

8-9=-2296/1349, 9-11=-2433/1347, 11-12=-2991/1550

BOT CHORD 2-25=-1234/2659, 24-25=-1234/2659, 6-21=-310/671, 20-21=-856/2228, 19-20=-714/2003, 8-19=-413/715, 14-16=-1233/2609, 12-14=-1233/2609

3-24-436/313, 21-24-946/2217, 6-20-840/552, 7-20-896/1493, 8-20-766/512,

WEBS 16-19=815/1976, 9-19=-276/297, 11-16=-583/417, 11-14=0/286

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 645 lb uplift at joint 2 and 646 lb uplift at joint 12



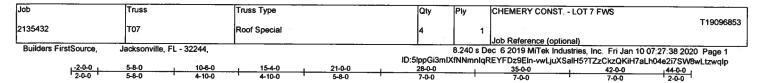
Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020

📤 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSITHE Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.





Scale = 1:82.2

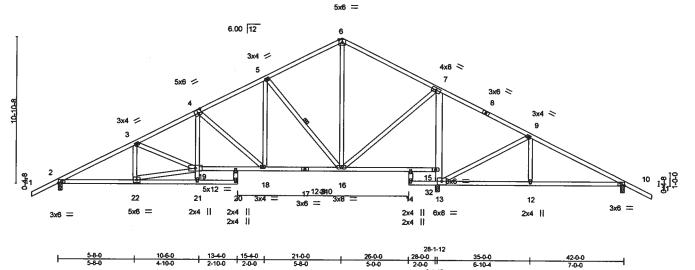


Plate Offsets (X,Y)-	[4:0-3-0,0-3-0], [10:0-2-15,Edge]		0-1-12	
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2017/TPI2014	CSI. TC 0.57 BC 0.62 WB 0.67 Matrix-MS	DEFL. in (loc) I/defl L/d Vert(LL) -0.13 20 >999 240 Vert(CT) -0.29 20 >999 180 Horz(CT) 0.03 13 n/a n/a	PLATES GRIP MT20 244/190 Weight: 264 lb FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

4-21: 2x4 SP No.3, 7-13: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=1103/0-3-8, 13=1883/0-3-8, 10=442/0-3-8

Max Horz 2=-156(LC 10) Max Uplift 2=-261(LC 12), 13=-262(LC 12), 10=-189(LC 13) Max Grav 2=1103(LC 1), 13=1883(LC 1), 10=503(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1784/958, 3-4=-1697/930, 4-5=-1124/740, 5-6=-551/534, 6-7=-578/521, 7-9=0/501,

9-10=-438/316

2-22=-680/1541, 4-19=-131/542, 13-15=-1504/681, 7-15=-1429/696, 12-13=-120/330, 10-12=-120/330, 18-19=-545/1481, 16-18=-250/969, 15-16=-369/297 19-22=-621/1512, 4-18=-685/391, 5-16=-826/522, 7-16=-261/1041, 9-13=-611/429, **BOT CHORD**

WEBS

9-12=0/307, 5-18=-237/620

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2, 262 lb uplift at joint 13 and 189 lb uplift at joint 10.



Structural wood sheathing directly applied or 4-2-2 oc purlins.

5-16

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

5-8-0 oc bracing: 13-15

10-0-0 oc bracing: 19-21

1 Row at midpt

Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

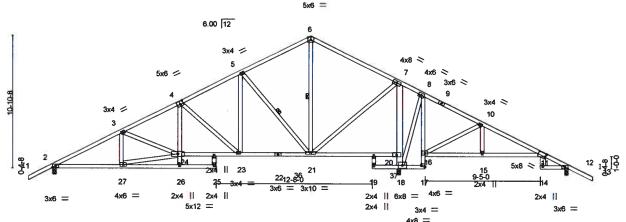
🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev., 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify for individual building designer must verify of a prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Saite 312, Alexandria, VA 22314.





Scale = 1:90.4



						20-1-12			
5-8-0	10-6-0	13-4-0	15-4-0	21-0-0	26-0-0	128-0-0 30-3-8	35-0-0	39-8-8	1 42-0-0
5-8-0	4-10-0	2-10-0	2-0-0	5-8-0	5-0-0	2-0-0 2-1-12	4-8-8	4-8-8	2-3-8
						0-1-12			
	5-8-0 5-8-0						5-8-0 10-6-0 13-4-0 15-4-0 21-0-0 26-0-0 128-0-0 130-3-6 15-8-0 5-0-0 20-0 128-0-0 128	5-8-0 10-6-0 13-40 15-40 21-00 26-00 128-00 30-36 35-00 5-8-0 4-10-0 2-10-0 2-0-0 5-8-0 5-00 2-0-0 2-1-12 4-8-8	5-8-0 10-6-0 13-4-0 15-4-0 21-0-0 26-0-0 28-0-0 30-3-8 35-0-0 39-8-8 5-8-0 4-10-0 2-10-0 2-0-0 5-8-0 5-0-0 2-0-0 2-1-12 4-8-8 4-8-8

LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	-0.13	,	>999	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.84	Vert(CT)	-0.28	25	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.56	Horz(CT)	0.03	18	n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matrix	x-MS						Weight: 279 lb	FT = 20%

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* BOT CHORD

4-26,8-17: 2x4 SP No.3, 7-18,11-14: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 2=1062/0-3-8, 18=2009/0-3-8, 12=361/0-3-8

Max Horz 2=156(LC 11)

Max Uplift 2=-264(LC 12), 18=-290(LC 13), 12=-155(LC 13) Max Grav 2=1081(LC 23), 18=2009(LC 1), 12=380(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1735/943, 3-4=-1637/912, 4-5=-1067/723, 5-6=-495/517, 6-7=-475/504, 7-8=0/627,

8-10=0/539, 10-11=-123/297

2-27-667/1498, 4-24-130/538, 18-20-1558/664, 7-20-1482/678, 17-18-403/271,

8-16-173/303, 23-24-511/1426, 21-23-212/930, 20-21-551/375

WEBS 24-27=-609/1473, 4-23=-680/391, 5-23=-236/616, 5-21=-825/521, 7-21=-325/1153,

8-18=-381/267, 10-16=-547/373

NOTES-

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 2, 290 lb uplift at joint 18 and 155 lb uplift at joint 12.



Structural wood sheathing directly applied or 4-2-14 oc purlins.

5-21, 6-21

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

5-5-0 oc bracing: 18-20

10-0-0 oc bracing: 24-26

1 Row at midpt

Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

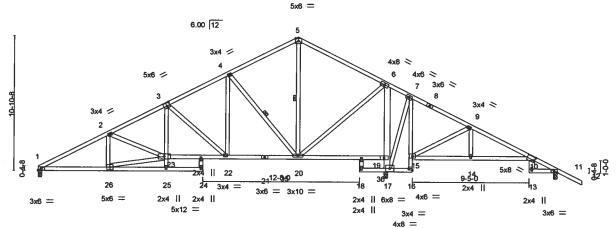
🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rey, 10/03/2015 BEFORE USE. Design valid for use only with MiTeN® connectors. This design is based only upon parameters shown, and is for an individual building component, not a lruss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Ì	Job	Truss	Truss Type	Qty Ply CHEMERY CONST LOT 7 FWS
	2135432	т09	Roof Special	T19096855
				Job Reference (optional)
	Builders FirstSource,	Jacksonville, FL - 32244,		8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:42 2020 Page 1
				ID:5lppGi3mlXfNNmnlqREYFDz9Eln-niaDkvV5pVbRyBHzzpVGs7lGkJJcas4iM868Uezwall
		5-8-0 10-6-0	15-4-0 21-0-0	28-0-0 30-3-8 35-0-0 39-8-8 42-0-0 44-0-0
		5-8-0 4-10-0	4-10-0 5-8-0	7-0-0 2-3-8 4-8-8 4-8-8 2-3-8 2-0-0

Scale = 1:89.7



						28-1-12			
	5-8-0	10-6-0	13-4-0	21-0-0	26-0-0	128-0-0 11 30-3-8 1	35-0-0	39-8-8	42-0-0
	5-8-0	4-10-0	2-10-0	7-8-0	5-0-0	2-0-0 2-1-12	4-8-8	4-8-8	2-3-8
						0-1-12			
Plate Offsets (X Y)-	I3∙ ቢ ዲብ ቢዲብ) [1	D-D_0.13 D.3_01	[11·0.2.15 E	Idaa) [16:Edaa 0 1 9]					

LOADING	· /	SPACING-	2-0-0	CSI.			DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP	ti
TCLL	20.0	Plate Grip DOL	1.25	TC	0.55	-	Vert(LL)	-0.13	10-14	>999	240	MT20	244/190	
TCDL	7.0	Lumber DOL	1.25	BC	0.84	100	Vert(CT)	-0.28	24	>999	180	1		
BCLL	0.0 *	Rep Stress Incr	YES	l WB	0.58		Horz(CT)	0.03	17	n/a	n/a			
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS							Weight: 275 lb	FT = 20%	

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

WEBS

TOP CHORD 2x4 SP No.2 **BOT CHORD**

2x4 SP No.2 *Except*

3-25,7-16: 2x4 SP No.3, 6-17,10-13: 2x6 SP No.2

2x4 SP No.3

REACTIONS. (lb/size) 1=950/0-3-8, 17=2013/0-3-8, 11=361/0-3-8

Max Horz 1=-159(LC 10) Max Uplift 1=-225(LC 12), 17=-291(LC 13), 11=-155(LC 13) Max Grav 1=969(LC 23), 17=2013(LC 1), 11=380(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-2=-1775/979, 2-3=-1655/927, 3-4=-1075/730, 4-5=-498/520, 5-6=-478/506, 6-7=0/628, TOP CHORD

7-9=0/539, 9-10=-123/296

BOT CHORD 1-26=-705/1538, 3-23=-144/542, 17-19=-1562/672, 6-19=-1486/685, 16-17=-404/272,

7-15=-173/303, 22-23=-526/1441, 20-22=-220/936, 19-20=-552/377

WEBS 23-26=-651/1517, 3-22=-690/400, 4-22=-243/618, 4-20=-832/528, 6-20=-333/1157,

7-17=-381/267, 9-15=-547/372

1) Unbalanced roof live loads have been considered for this design.

 Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 1, 291 lb uplift at joint 17 and 155 lb uplift at joint 11.



Structural wood sheathing directly applied or 4-1-4 oc purlins.

4-20, 5-20

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

5-5-0 oc bracing: 17-19

1 Row at midpt

10-0-0 oc bracing: 23-25

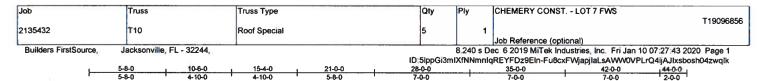
Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 10,2020

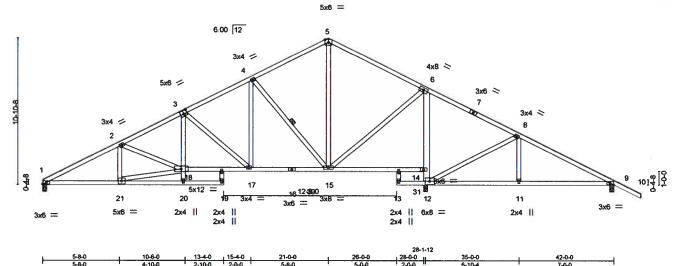
🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 19/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly manage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd



Scale = 1:81.8



Piate Offsets (X,Y)— [3:0-3-0,0-3-0], [9:0-2-15,Edge]													
LOADIN TCLL TCDL BCLL	20.ó 7.0 0.0 ◆	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC BC WB	0.57 0.63 0.67	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.13 -0.29 0.03	(loc) 19 19 12	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190	
BCDL	10.0	Code FBC2017/T	PI2014	Matri	x-MS						Weight; 260 lb	FT = 20%	

BRACING-

WEBS

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2 *Except*

3-20: 2x4 SP No.3, 6-12: 2x6 SP No.2

WEBS 2x4 SP No.3

REACTIONS. (lb/size) 1=991/0-3-8, 12=1888/0-3-8, 9=441/0-3-8

Max Horz 1=-159(LC 10)

Max Uplift 1=-222(LC 12), 12=-264(LC 12), 9=-189(LC 13) Max Grav 1=991(LC 1), 12=1888(LC 1), 9=503(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1823/993, 2-3=-1714/945, 3-4=-1131/747, 4-5=-553/536, 5-6=-580/523, 6-8=0/503, 8-9=-438/314

8-9=-438/314

BOT CHORD 1-21=-741/1580, 3-18=-146/546, 12-14=-1509/691, 6-14=-1435/705, 11-12=-116/330,

9-11=-116/330, 17-18=-569/1495, 15-17=-262/974, 14-15=-371/299

WEBS 18-21=-684/1556, 3-17=-695/402, 4-17=-245/622, 4-15=-833/530, 6-15=-270/1045,

8-12=-611/429, 8-11=0/307

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 1, 264 lb uplift at joint 12 and 189 lb uplift at joint 9.



Structural wood sheathing directly applied or 4-0-9 oc purlins.

4-15

Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

5-8-0 oc bracing: 12-14

1 Row at midpt

10-0-0 oc bracing: 18-20

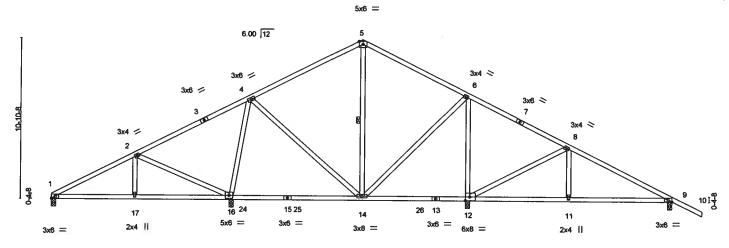
Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020



Job Truss Truss Type Qty Ply CHEMERY CONST - LOT 7 FWS T19096857 2135432 lT11 Common Job Reference (optional) Builders FirstSource, Jacksonville, FL - 32244, 8.240 s Dec 6 2019 MiTek Industries, Inc. Fri Jan 10 07:27:45 2020 Page 1 ID:5lppGi3mIXfNNmnlqREYFDz9Eln-CHGMMwYz6Q_0pe?Yex2zUmwk?WQEnBY936Lo4zzwqli 44-0-0 35-0-0 42-0-0 6-10-4

Scale = 1:74.9



	5-8-0 6-6	5-4	8-9-12	7-1-12	35-0-0 6-10-4	42-0-0 7-0-0
Plate Offsets (X,Y)-	[9:0-2-15,Edge], [12:0-3	3-8,0-3-0]			0-10-4	700
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2017/	2-0-0 1.25 1.25 YES TPI2014	CSI. TC 0.67 BC 0.52 WB 0.72 Matrix-MS	DEFL. in (loc) Verl(LL) -0.14 14-16 Verl(CT) -0.25 14-16 Horz(CT) 0.01 9	l/defl L/d >999 240 >756 180 n/a n/a	PLATES GRIP MT20 244/190 Weight: 230 lb FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING-

TOP CHORD BOT CHORD WEBS Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 5-14

REACTIONS. All bearings 0-3-8.

(lb) - Max Horz 1=-262(LC 17)

Max Uplift All uplift 100 lb or less at joint(s) except 1=-144(LC 9), 12=-475(LC 13), 16=-539(LC 12), 9=-272(LC

13)

Max Grav All reactions 250 lb or less at joint(s) except 1=355(LC 23), 12=1171(LC 1), 16=1154(LC 1), 9=551(LC 24)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2

1-2=-456/483, 2-4=-163/316, 4-5=-266/259, 8-9=-538/517

BOT CHORD 1-17=-298/380, 16-17=-298/380, 14-16=-92/361, 12-14=-147/424, 11-12=-317/420,

9-11=-317/420 WEBS 6-14=-75/348.0

6-14=-75/348, 6-12=-766/509, 8-12=-596/805, 8-11=-307/296, 4-16=-765/533,

2-16=-588/797, 2-17=-275/235

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 1, 475 lb uplift at joint 12, 539 lb uplift at joint 16 and 272 lb uplift at joint 9.



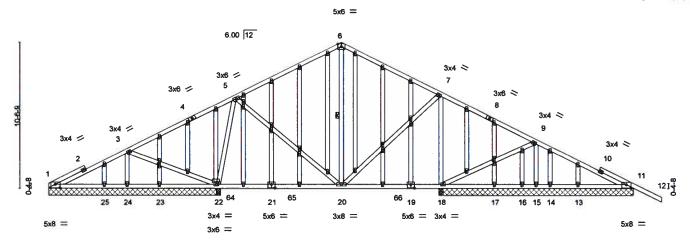
Joaquin Velez PE No.68182 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020



Job	Truss	Truss Type		Qty	Ply	CHEMERY CONST LOT 7	FWS	
				1				T19096858
2135432	T11G	GABLE		1	1 1			
						Job Reference (optional)		
Builders FirstSource,	Jacksonville, FL - 3224	4,			8.240 s D	ec 6 2019 MiTek Industries, I	nc. Fri Jan 10 07:27:	50 2020 Page 1
			ID	5lppGi3mIXfNNm	nIqREYFD	z9Eln-YE3FPeb6wzcJwPuWF	RUe8BpdbDX7GSUh	uCN2ZmAzwqld
	5-8-0	13-4-0	21-0-0	28-1-	12	35-0-0	42-0-0	44-0-0
	E 0 0	700	700	7.4	10	0.40.4	700	1000

Scale = 1:79.7



	1	5-8-0	12-2-4	12-4-0	21-0-0	- i//	28-0-0	28-1 ₋ 12	35-0-0	42-0-0	- 6
	,	5-8-0	6-6-4	0-1-12	8-8-0		7-0-0	0-1-12	6-10-4	7-0-0	
Plate Offse	ets (X,Y)-	[1:0-4-0,0-3-1], [11:0-4-0	,0-3-1], [19:0	0-3-0,0-3-0], [2	1:0-3-0,0-3-0]	, [22:0-2-4,0-0-4],	[30:0-1-14,	0-1-0], [41:0	-1-12,0-1-0]		
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (oc) l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.64	Vert(LL)	-0.12 20	,	240	MT20	244/190
CDL	7.0	Lumber DOL	1.25	BC	0.53	Vert(CT)	-0.23 20	-22 >831	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.60	Horz(CT)	0.01	11 n/a	n/a		
BCDL	10.0	Code FBC2017/T	PI2014	Matr	ix-MS					Weight: 359 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS

BRACING-

TOP CHORD **BOT CHORD** WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Row at midpt 6-20

REACTIONS. All bearings 14-0-0 except (jt=length) 1=12-4-0, 24=12-4-0, 22=12-4-0, 22=12-4-0, 25=12-4-0, 23=12-4-0,

1=12-4-0

Max Horz 1=-256(LC 13) (lb) -

All uplift 100 lb or less at joint(s) 1, 14, 16, 17, 25, 23 except 24=-232(LC 12), 22=-359(LC 12), 18=-318(LC 13), 15=-329(LC 13), 11=-122(LC 13), 13=-114(LC 8) Max Uplift

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 14, 16, 17, 25, 23, 1 except 24=375(LC 23),

22=847(LC 2), 22=829(LC 1), 18=767(LC 2), 18=762(LC 1), 15=512(LC 1), 11=254(LC 1), 11=254(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 5-6=-370/345, 6-7=-361/348

BOT CHORD 18-20=-63/252

WEBS 3-24=-364/287, 5-22=-677/436, 7-20=-56/298, 7-18=-700/457, 9-15=-464/376

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 14, 16, 17, 25, 23, 1 except (jt=lb) 24=232, 22=359, 18=318, 15=329, 11=122, 13=114, 11=122.



Joaquin Velez PE No.68182 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 10,2020

🛕 WARNING - Vertly design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters and individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual it russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual it russ web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of truss systems, see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



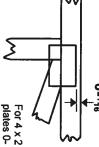
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/18" from outside edge of truss.

This symbol indicates the

required direction of slots in connector plates.

* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 X 4 LATERAL

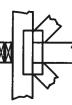
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



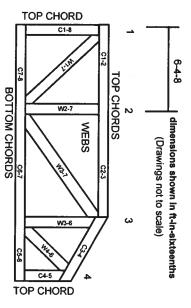
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards: ANSI/TPI1: National E

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing.
Building Component Safety Information, Guide to Good Practice for Handling,

Installing & Bracing of Metal Plate

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.

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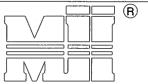
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- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2

MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

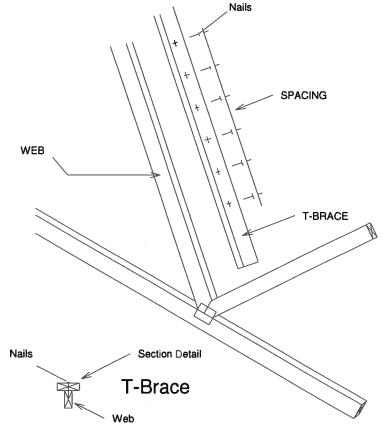
Nailing Pattern									
T-Brace size	Nail Size	Nail Spacing							
2x4 or 2x6 or 2x8	10d (0.131" X 3")	6" o.c.							
1									

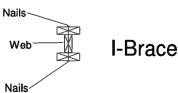
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

		Brace Size for One-Ply Truss				
	Specified Rows of La	Continuous Iteral Bracing				
Web Size	1	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

		Brace Size for Two-Ply Truss					
	Specified Rows of La	Continuous Iteral Bracing					
Web Size	1	2					
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace					
2x6	2x6 T-Brace	2x6 I-Brace					
2x8	2x8 T-Brace	2x8 I-Brace					

T-Brace / I-Brace must be same species and grade (or better) as web member.







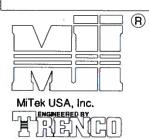
Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

SCAB-BRACE DETAIL

MII-SCAB-BRACE

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Page 1 of 1

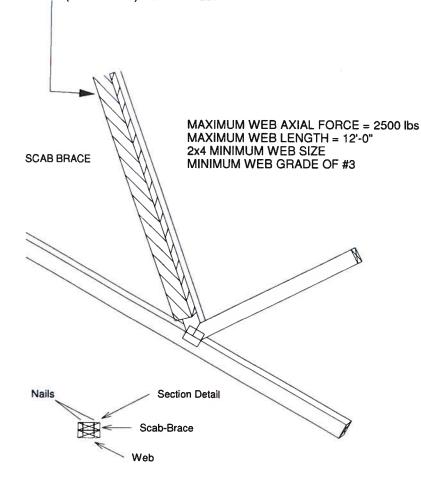


Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical.

Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

APPLY 2x___ SCAB TO ONE FACE OF WEB WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

STANDARD REPAIR TO REMOVE END **VERTICAL (RIBBON NOTCH VERTICAL)**

MII-REP05

MiTek USA, Inc. Page 1 of 1

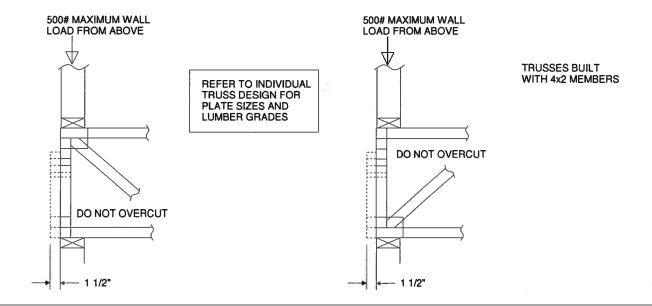


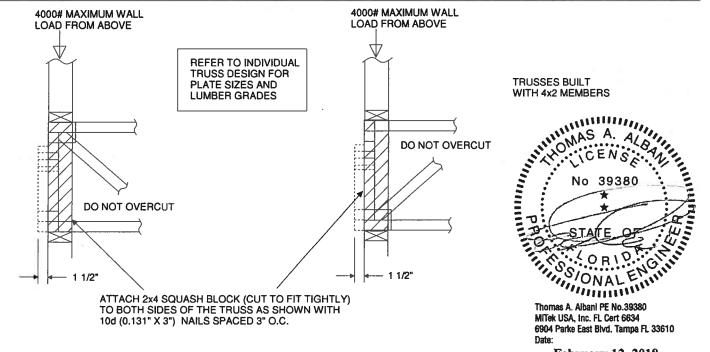
MiTek USA, Inc.

ENGINEERED BY HI

- 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
- ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE
 APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.
 THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE
 SUCH AS TO AVOID SPLITTING OF THE WOOD.
 LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ORIENTATION ONLY.
 CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.

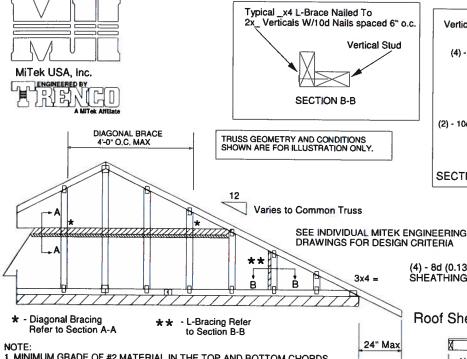




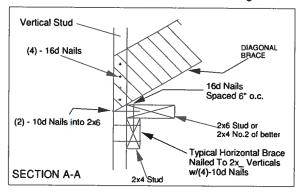
(R)

Standard Gable End Detail

MII-GE130-D-SP



MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. 2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO HOOP DIAPHRAM AT 4-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTIÓN MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND

NAILS DESIGNATED 16d ARE (0.131" X 3.5")

1'-3" Max.	(2) - 10d NAILS
	Trusses @ 24" o.c
Diag. Brace at 1/3 points if needed	2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.
End Wall	HORIZONTAL BRACE (SEE SECTION A-A)
	1

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7			
2x4 SP No. 3 / Stud		3-5-4	3-6-8	5-0-2	6-10-8	10-3-13			
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



o.c.

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:



Standard Gable End Detail

MII-GE130-SP

Page 1 of 2

(2) - 10d NAILS

∕Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

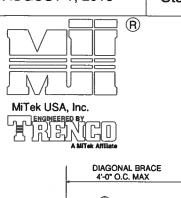
ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

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Typical _x4 L-Brace Nailed To 2x_Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

3x4 =

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

Vertical Stud DIAGONAL BRACE (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

10¢

NAILS

Roof Sheathing

1'-3"

Max.

* - Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

NOTE:

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM. 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, GRADES: 1x4 SRB

OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

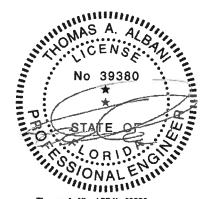
	Minimum						2 DIAGON
	NAILS DESIG	NATED 16d	ARE (0.131	I" X 3.5")			
1	. NAILS DESIGN	NATED 10d	ARE (0.131	I" X 3") AND)		
	06-01-13 BY S	PIB/ALSC.					
0	, SOUTHERN P	INE LUMBE	R DESIGN	VALUES A	RE THOSE	EFFECTIVE	
	TYPE TRUSSE						
	DO NOT USE I						
	THIS DETAIL D					S.	
	GABLE STUD			OR EXCEE	DS L/240.		
	(REFER TO SE			. ()			
	ATTACH TO VE						
	BRACE TO BE						
	2x4 STUD AS S						
	CONSTRUCT H		IL BRACE (CONNECTI	NG A 2x6 S	TUD AND A	
	DIAPHRAM AT	4'-0" O C					

Minimum Stud Size Species	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length							
2x4 SP No. 3 / Stud	12" O.C.	4-0-7	4-5-6	6-3-8	8-0-15	12-1-6			
2x4 SP No. 3 / Stud	16" O.C.	3-8-0	3-10-4	5-5-6	7-4-1	11-0-1			
2x4 SP No. 3 / Stud	24" O.C.	3-0-10	3-1-12	4-5-6	6-1-5	9-1-15			

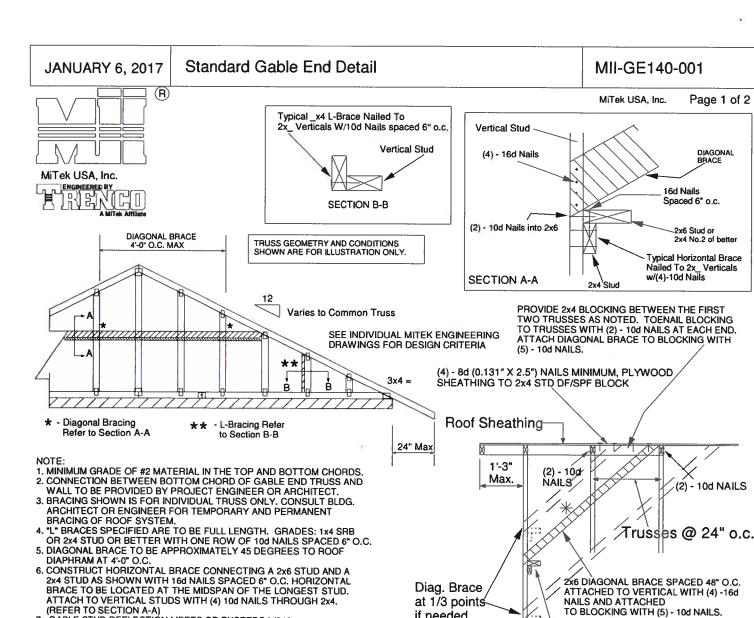
Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten Tard Traces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET **CATEGORY II BUILDING** EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10 160 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610



at 1/3 points

End Wall

if needed

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
		Maximum Stud Length						
2x4 DF/SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4		
2x4 DF/SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11		
2x4 DF/SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

MAXIMUM WIND SPEED = 140 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C ASCE 7-98, ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE: 1.60

TYPE TRUSSES.

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE

(SEE SECTION A-A)

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

January 19, 2018

Standard Gable End Detail

MII-GE170-D-SP

(5) - 10d NAILS.

Roof Sheathing

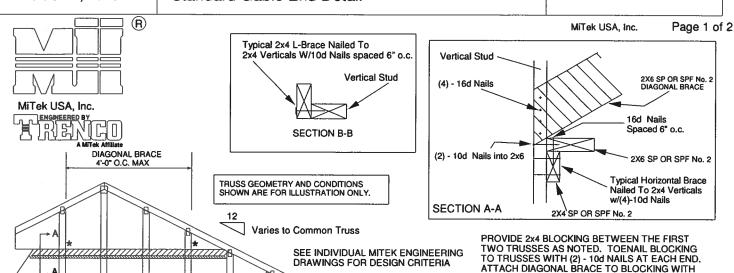
1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD, SHEATHING TO 2x4 STD SPF BLOCK

- 10d

NÁILS'



3x4 =

24" Max

Diag. Brace

at 1/3 points

if needed

Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

NOTE:

Minimum

Stud Size

Species

and Grade 2x4 SP No. 3 / Stud

2x4 SP No. 2

2x4 SP No. 2

2x4 SP No. 2

2x4 SP No. 3 / Stud 16" O.C.

2x4 SP No. 3 / Stud | 24" O.C.

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

"L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

4-0-7

Without

Brace

3-9-7

3-4-12

3-11-13

3-1-15

2-9-4

3-7-7

TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AN NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Stud

Spacing

12" O.C.

12" O.C.

16" O.C.

ADELS NEXT	10 301330N		
UES ARE TH	OSE EFFECTIVE		
3") AND 3.5")			End Wall
2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS	
Maximum St	tud Length		
5-8-8	6-11-1	11-4-4	
4-11-15	6-9-8	10-2-3	
4-0-7	5-6-8	8-3-13	
5-8-8	6-11-1	11-11-7	
4-11-5	6-11-1	10-10-5	

24" O.C. Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 170 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS

9-5-14



(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH
(4) -16d NAILS, AND ATTACHED TO

BLOCKING WITH (5) -10d NAILS.

HORIZONTAL BRACE

(SEE SECTION A-A)

Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

Standard Gable End Detail

MII-GE180-D-SP

(5) - 10d NAILS.

Roof Sheathing

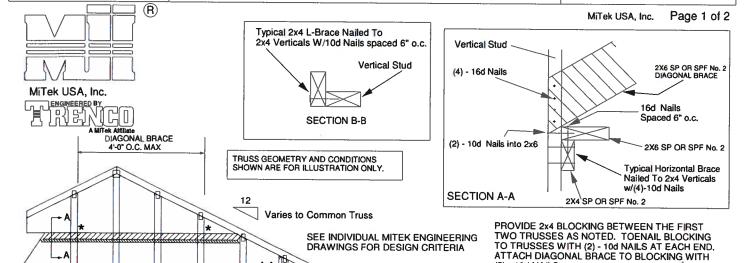
1'-0"

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

10¢

NAILS



3x4 =

24" Max

Diag. Brace at 1/3 points

End Wall

if needed

 Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

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1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3
OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

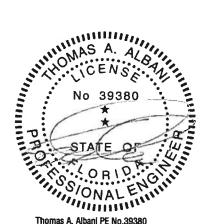
11. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			
2x4 SP No. 2	16" O.C.	3-5-13	4-8-1	6-2-1	10-5-7			
2x4 SP No. 2	24" O.C.	3-0-8	3-9-12	6-1-1	9-1-9			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10 180 MPH **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



2x6 DIAGONAL BRACE SPACED 48° O.C. ATTACHED TO VERTICAL WITH

BLOCKING WITH (5) -10d NAILS.

(4) -16d NAILS, AND ATTACHED TO

HORIZONTAL BRACE

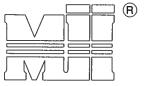
(SEE SECTION A-A)

(2) - 10d NAILS

Trusses @ 24" o.c.

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MiTek USA, Inc. Page 1 of 1



MiTek USA, Inc.



MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 ° O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-10

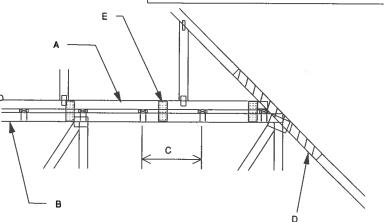
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES).
ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

DURATION OF LOAD INCREASE: 1.60

A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) (0.131* X 3.5*) TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24* O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131* X 3.5*) NAILS EACH.
D - 2 X __ X 4*-0* SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED.
ON INTERSECTION, WITH (2) ROWS OF (0.131* X 3.7*) NAILS @ 4* O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND: DIRECTIONS AND

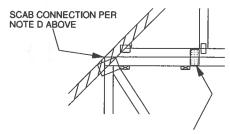
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft. E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH

MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72' O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REO. REGARDLESS OF SPAN)

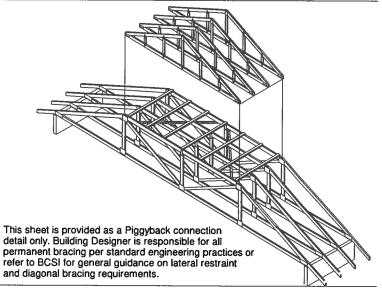


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

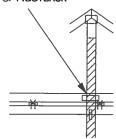
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH NAILON PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS

VEHTICAL WEBS OF PIGGYBACK AND BASE TRUSS
MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
AS SHOWN IN DETAIL.
ATTACH 2 x x 4'-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS

BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.

4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.

No 39380

STATE OF THE STATE OF

Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-10

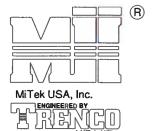
MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

EXPOSURE B or C ASCE 7-10

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.



A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0.131* X.3.5") TOE-NAILED.
B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24* O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0.131* X.3.5") NAILS EACH.
D - 2 X __X 4*-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0.131* X.3") NAILS @ 4* O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH
DIRECTIONS AND:

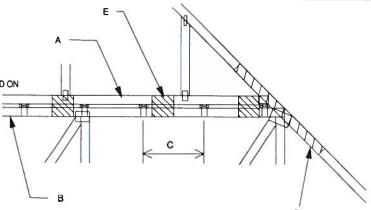
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:

1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

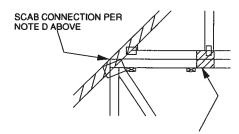
FOR WIND SPEED IN THE RANGE 126 MPH - 160 MPH ADD 9" x 9" x 1/2" PLYWCOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH

3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

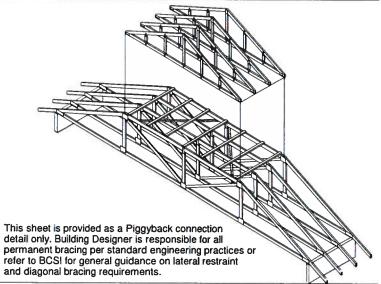


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

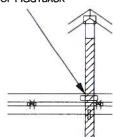
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



 $7^{\rm o}$ x $7^{\rm o}$ x $1/2^{\rm o}$ PLYWOOD (or $7/16^{\rm o}$ OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.

AS SHOWN IN DETAIL.

ATTACH 2 x 4'-0" SCAB TO EACH FACE OF
TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS
SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)

VEHICLE WEBS OF PIGGTBACK AND BASE THUSS.)
(MINIMUM 2X4)
THIS CONNECTION IS ONLY VALID FOR A MAXIMUM
CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW
BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
GREATER THAN 4000 LBS.

GREATER THAN 4000 LBS.
FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parte East Bivd. Tampa FL 33610

January 19, 2018

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

Page 1 of 1



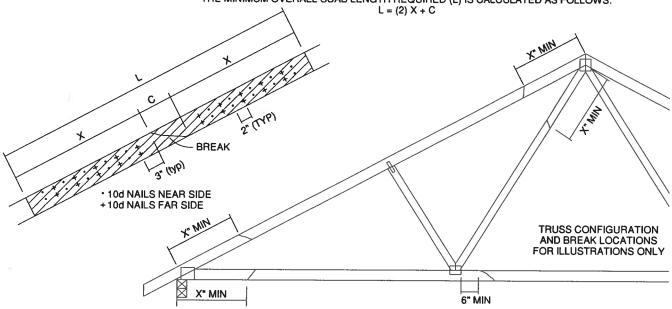
MiTek USA, Inc. ENGINEERED STATES

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *			MAXIMUM FORCE (Ibs) 15% LOAD DURATION							
		X INCHES	SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	3111	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:



THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES
 NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS
 SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED
 REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

 3. THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID
- UNUSUAL SPLITTING OF THE WOOD.

 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.

 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x_ORIENTATION ONLY. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



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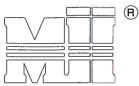
January 19, 2018

LATERAL TOE-NAIL DETAIL

MII-TOENAIL_SP

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

NOTES:

- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.

 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

ILLUSTRATION PURPOSES ONLY

NEAR SIDE

NEAR SIDE

VIEWS SHOWN ARE FOR

SIDE VIEW (2x3) 2 NAILS

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail) SP DIAM. SPF-S DF HF .131 69.9 88.0 80.6 68.4 59.7 .135 93.5 85.6 74.2 72.6 63.4 .162 108.8 99.6 86.4 84.5 73.8 .128 74.2 57.6 50.3 67.9 58.9 75.9 60.3 .131 69.5 59.0 51.1 81.4 64.6 .148 74.5 63.2 52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL. APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

EXAMPLE:

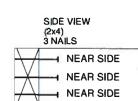
3.5" LONG

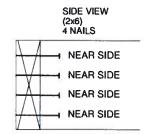
3.25" LONG

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:

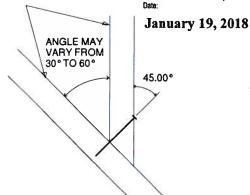
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

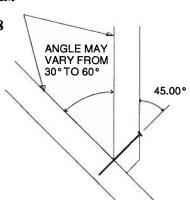


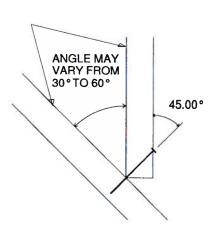




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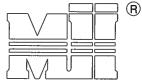


TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1



MiTek USA, Inc.

GABLE END, COMMON TRUSS OR GIRDER TRUSS

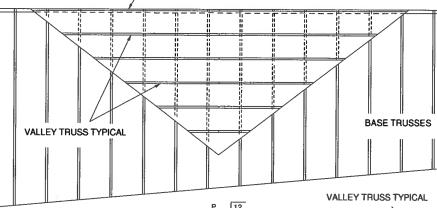
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
- 1. NAIL SIZE 100 (0.131" X 3")
 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
 DO NOT USE DRYWALL OR DECKING TYPE SCREW
 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
 SECURE PER DETAIL A
 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
 INDIVIDUAL DESIGN DRAWINGS.

 BASE TRUSS SHALL BE DESIGNED WITH A RUBLING

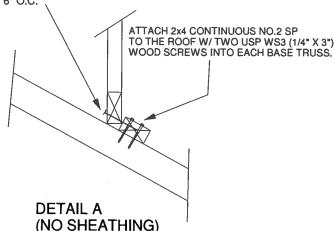
 BASE TRUSS SHALL BE DESIGNED WITH A RUBLING.

- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 6. NAILING DONE PER NDS 01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



GABLE END, COMMON TRUSS OR GIRDER TRUSS Р 12 SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C**

WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES



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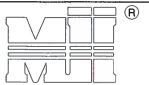
January 19, 2018

TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2

MiTek USA, Inc.

Page 1 of 1

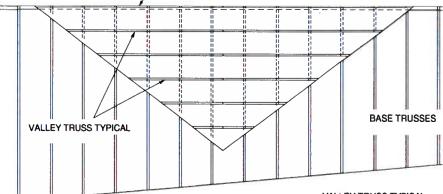


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GABLE END, COMMON TRUSS OR GIRDER TRUSS

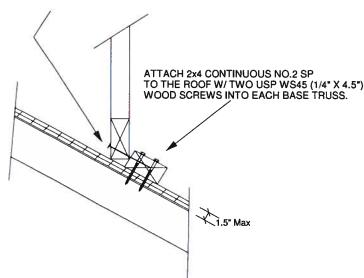
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
 2. WOOD SCREW = 4.5" WS45 USP OR EQUILIVANT
 3. INSTALL SHEATHING TO TOP CHORD OF BASE TRUSSES.
 4. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE TO BASE TRUSSES AS PER DETAIL A
 5. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
 6. NAILING DONE PER NDS-01
- 6. NAILING DONE PER NDS-01
- 7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.



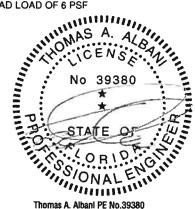
GABLE END, COMMON TRUSS VALLEY TRUSS TYPICAL 12 OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

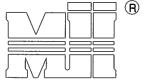
ON THE TRUSSES



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Page 1 of 1

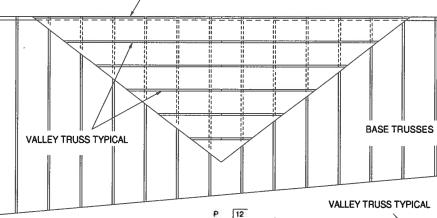


MiTek USA, Inc.

GABLE END, COMMON TRUSS OR GIRDER TRUSS

GENERAL SPECIFICATIONS

- NAIL SIZE 16d (0.131" X 3.5")
 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
 BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 4. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
- 5. NAILING DONE PER NDS 01
- 6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.
- 7. ALL LUMBER SPECIES TO BE SP.



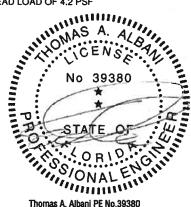
GABLE END, COMMON TRUSS OR GIRDER TRUSS SEE DETAIL A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d NAILS 6" O.C. ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/TWO 16d NAILS INTO EACH BASE TRUSS.

DETAIL A (MAXIMUM 1" SHEATHING) N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24° O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES

ON THE TRUSSES



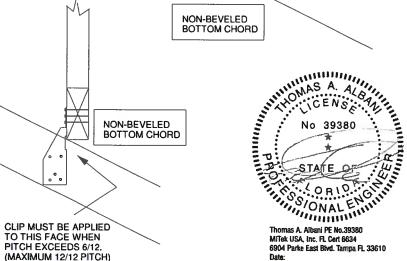
Thomas A. Albani PE No.39380 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

TRUSSED VALLEY SET DETAIL **MII-VALLEY AUGUST 1, 2016** (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT R Page 1 of 1 MiTek USA, Inc. TO EXCEED 48" O.C. SPACING MiTek USA, Inc. FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH **USP RT7 OR EQUIVALENT** WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET **CATEGORY II BUILDING** NON-BEVELED **EXPOSURE B or C** BOTTOM CHORD WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY)

SUPPORTING TRUSSES DIRECTLY UNDER VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.



January 19, 2018

Standard Gable End Detail

MII-GE146-001

MiTek USA, Inc.

Page 1 of 2

(2) - 10d NAILS

Trusses @ 24" o.c.

2x6 DIAGONAL BRACE SPACED 48" O.C.

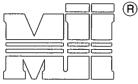
ATTACHED TO VERTICAL WITH (4) -16d

HORIZONTAL BRACE

(SEE SECTION A-A)

TO BLOCKING WITH (5) - 10d NAILS.

NAILS AND ATTACHED



MiTek USA, Inc. ENGINEERED B

Typical _x4 L-Brace Nailed To 2x_ Verticals W/10d Nails spaced 6" o.c. Vertical Stud SECTION B-B

TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY. Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA **

3x4 =

24" Max

Diag. Brace

at 1/3 points

End Wall

if needed

Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x_ Verticals w/(4)-10d Nails **SECTION A-A**

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

Roof Sheathing

1'-3'

Max.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SP BLOCK

- 1Ó¢Ł

NAILS'

- Diagonal Bracing Refer to Section A-A

L-Bracing Refer to Section B-B

NOTE

1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.

CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

WALL TO BE PHOVIDED BY PHOJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG.
ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT
BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES:
2x4 No 3/STUD SP OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAGONAL BRACE

4'-0" O.C. MAX

DIAPHRAM AT 4'-0" O.C.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES.

10. NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Minimum Stud Size Species	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS		
and Grade		Maxin	num Stud L	tud Length			
2x4 SP No 3/Stud	12" O.C.	3-11-3	6-8-0	7-2-14	11-9-10		
2x4 SP No 3/Stud	16" O.C.	3-6-14	5-9-5	7-1-13	10-8-11		
2x4 SP No 3/Stud	24" O.C.	3-1-8	4-8-9	6-2-15	9-4-7		

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 146 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE 8 or C ASCE 7-98, ASCE 7-02, ASCE 7-05 **DURATION OF LOAD INCREASE: 1.60**

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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January 19, 2018

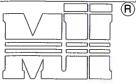
OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

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Page 1 of 1



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ENGINEERED B

TRUSS CRITERIA:

LOADING: 40-10-0-10 **DURATION FACTOR: 1.15** SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL **END BEARING CONDITION**

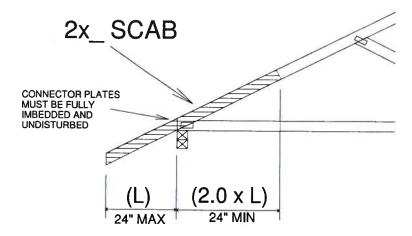
NOTES:

1. ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF

TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



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LATERAL BRACING RECOMMENDATIONS

MII-STRGBCK

(R)

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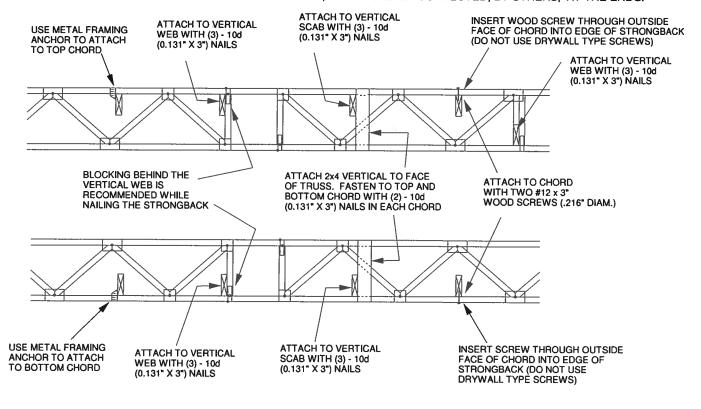
Page 1 of 1

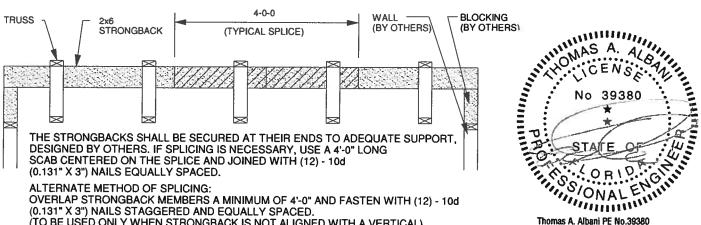


TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS. 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





(0.131" X 3") NAILS STAGGERED AND EQUALLY SPACED. (TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

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