

PERMIT
000027292

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

ck 927064

For Office Use Only Application # 0808-06 Date Received 8/4 By JW Permit # 1664/27292
Zoning Official BLK Date 14.08.08 Flood Zone 103 ft. plat FEMA Map # N/A Zoning RSF-2
Land Use RES. Low Density Elevation N/A MFE 103 ft. plat River N/A Plans Examiner (W) Date 8/13/08
Comments Elevation Confirmation Letter Required at slab
☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☒ State Road Info ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☒ Letter of Authorization from Contractor on File
☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 08-0522 Patrick Wilson Fax (904)-332-6367
Name Authorized Person Signing Permit Theodore C. Brock/ Phone (904)-296-1490
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490
911 Address 361 SW TIMBER RIDGE DRIVE LAKE CITY, FL 32024
Contractors Name Theodore C. Brock Phone (407)-227-3504
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771
Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789
Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-112 Estimated Cost of Construction 94,000.00
Subdivision Name Timberlands Lot 12 Block _____ Unit _____ Phase _____
Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge, 12th Lot on Right.
Number of Existing Dwellings on Property 0

Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size _____
Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height = 15.0'
Actual Distance of Structure from Property Lines - Front 58.0' Side 40.0' Side 39.0' Rear 84.0'
Number of Stories 2 Heated Floor Area 2729 Total Floor Area 3181 Roof Pitch 12/6

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

\$ 931.82

Emailed Patrick 8/14/08
M. M. L. L. L.

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001664

DATE 08/26/2008 PARCEL ID # 10-4S-16-02856-112
APPLICANT PATRICK WILSON PHONE 904.296.1490
ADDRESS 6800 SOUTHPPOINT PKWY # 300 JACKSONVILLE FL 32216
OWNER MARONDA HOMES INC. OF FLORIDA PHONE 904.296.1490
ADDRESS 361 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE C. BROCK PHONE 407.227.3504
LOCATION OF PROPERTY 90-W TO SR.247-S,TL TO C-252-B,TR TO TIMBER RIDGE,TL,12TH
LOT ON R. _____

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBELANDS 12

SIGNATURE _____

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT

Inst: 200812016087 Date: 8/28/2008 Time: 4:17 PM
 DC, P. DeWitt Cason, Columbia County Page 1 of 1 B: 1157 P: 1365

Tax Parcel Identification Number 10-4S-10-02856-112

County Clerk's Office Stamp or Seal

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this **NOTICE OF COMMENCEMENT**.

1. Description of property (legal description): 12/1 Timberlands
- a) Street (job) Address: 301 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
 - a) Name and address: Maronda Homes Inc of FL 6800 Southpoint Pkwy #300 Jax FL 32216
 - b) Name and address of fee simple titleholder (if other than owner): _____
 - c) Interest in property: _____
4. Contractor Information
 - a) Name and address: Maronda Homes Inc of FL 6800 Southpoint Pkwy #300 Jax FL 32216
 - b) Telephone No.: (904) 296-1490 Fax No. (Opt.): (904) 332-0375
5. Surety Information
 - a) Name and address: _____
 - b) Amount of Bond: _____
 - c) Telephone No.: _____ Fax No. (Opt.): _____
6. Lender
 - a) Name and address: _____
 - b) Phone No.: _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
 - a) Name and address: Southern Title Holding Co LLC 3945 Baymeadows Rd Jax FL 32217
 - b) Telephone No.: (904) 734-2205 Fax No. (Opt.): _____
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
 - a) Name and address: _____
 - b) Telephone No.: _____ Fax No. (Opt.): _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

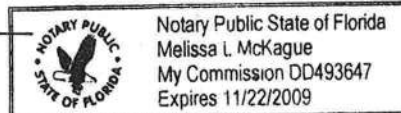
STATE OF FLORIDA
 COUNTY OF COLUMBIA

10. [Signature]
 Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
 Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 10 day of June, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
 fact) for Maronda Homes Inc of Florida (name of party on behalf of whom instrument was executed).

Personally Known ☒ OR Produced Identification _____ Type _____

Notary Signature Melissa L McKague Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
 Signature of Natural Person Signing (in line #10 above.)

FILE (NOV)0007-57-405

This Instrument Prepared by and Return to :

Amy Wesp
SOUTHERN TITLE HOLDING
COMPANY, LLC.
3943 BAY MEADOWS ROAD
JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions
contained in a title insurance commitment issued by it.

Property Appraisers Parcel L.D. (Folio) Number(s):

R02856-000

Grantee(s) LD.#(s):

File No: JX0812085

Inst: 200812010775 Date: 6/6/2008 Time: 1:05 PM

Doc Stamp-Deed: 6293.00

DC, P. DeWitt Cason, Columbia County Page 1 of 1 B: 1151 P: 2385

WARRANTY DEED
(CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of ~~\$10,000~~ Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9, PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Wherever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation.)

In Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereunto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed, sealed and delivered in our presence:

ATTEST:

Secretary

RML HOLDINGS INC.

Witness Signature:

Worth D. Morris

Printed Name:

WORTH D. MORRIS

Witness Signature:

Jody M. Goble

Printed Name:

Jody M. Goble

BY:

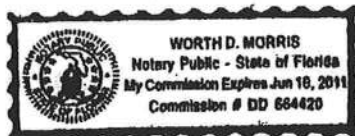
Robert R. Lardizabal

ROBERT R. LARDIZABAL, PRESIDENT

STATE OF FLORIDA
COUNTY OF DUVAL

The foregoing instrument was acknowledged before me this 28th day of May, 2008, by ROBERT R. LARDIZABAL as PRESIDENT of RML HOLDINGS INC., A FLORIDA CORPORATION, on behalf of the corporation. He/she is personally known to me or who has produced driver license(s) as identification.

My Commission Expires:



Printed Name:

WORTH D. MORRIS

Notary Public

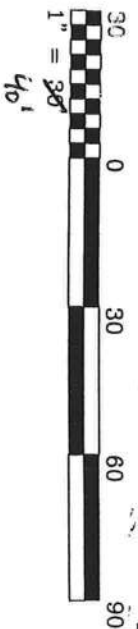
Serial Number



IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



REV. TABLE:	
RADIUS	TANGENT
560.00'	44.60'
LENGTH	DELTA
89.02'	9705.28.7"
CHORD	CHORD BEARING
89.03'	S 89°14'00" W



- = FOUND/1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- = FOUND 3/4" IRON PIPE
- = FOUND 4" x 4" CONC. MON. NO IDENTIFICATION
- = SET 4" x 4" CONC. MON. P.S.M. 5582
- × = SET NAIL & DISK P.S.M. 5582
- × = FOUND NAIL & DISK
- ⊗ = FOUND 6" x 6" S.R.D. R/W MON.
- ⊖ = CATY RISER
- ⊙ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE
- ASPH = ASPHALT CONCRETE
- CAV = CALCULATED FROM MEASURED
- CAB = CABLE TELEVISION
- CE = CHIMNEY
- CH = CHIMNEY BRICK
- CM = CONCRETE MONUMENT
- CONC = CONCRETE
- ELEV = ELEVATION
- ELV = ELEVATION
- FND = FOUND
- FENCE = FENCE
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MFF = MINIMUM FINISHED FLOOR
- MH = MANHOLE
- OLU = OVERHEAD UTILITIES
- PB = PLAT BOOK
- PUE = PUBLIC UTILITIES EASEMENT
- TRANS = TRANSFORMER
- TP = TYPICAL
- WM = WATER METER
- WV = WATER VALVE

ASPH	ASPHALT PAVEMENT	MEASURED
C	CALCULATED FROM	
CATV	CABLE TELEVISION	
C/B	CONCRETE BLOCK	
CLF	CHAIN LINK FENCE	
CM	CONCRETE MONUMENT	
CONC	CONCRETE	
ELEC	ELECTRIC	
ELEV	ELEVATION	
FND	FOUND	
FNC	FENCE	
LB	LICENSED SURVEYOR BUSINESS	
(M)	FIELD MEASURED	
WFF	MINIMUM FINISHED FLOOR	
MH	MANHOLE	
O.D.	OVERHEAD UTILITIES	
PB	PLAT BOOK	
P.U.E.	PUBLIC UTILITIES EASEMENT	
TRANS	TRANSFORMER	
TYP	TYPICAL	
WM	WATER METER	
WV	WATER VALVE	

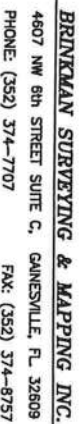
**THIS IS NOT A BOUNDARY SURVEY
CERTIFICATE OF SURVEYOR:**

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURETOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURETOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

JAMES E. BRINKMAN, PSY - FLA CERT# 5582

DATE: 6/17/03



PHONE: (352) 374-7707 FAX: (352) 374-8757

FAX: (352) 374-8757

SCALE: 1" = 30'	"THE BENCHMARK IN QUALITY SERVICE"
DATE: 6/12/08	

DRAWN BY: ZL

CHECKED BY: J.B

FIELD WORK COMPLETED ON ***** FBI BROOK 00 PAGE 0000

1

PREPARED FOR: MARONDA

DRAWING NUMBER

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

12/1 TM

Project Name: **NANTUCKET GAINESVILLE**
Address: 3101 SW Timber Ridge Dr
City, State: Lake City, FL 32615
Owner: **ELECTRIC**
Climate Zone: **North**

Builder: **MARONDA HOMES**
Permitting Office: Columbia
Permit Number: 27292
Jurisdiction Number: 221000

1. New construction or existing New ☐
2. Single family or multi-family Single family ☐
3. Number of units, if multi-family 1 ☐
4. Number of Bedrooms 4 ☐
5. Is this a worst case? Yes ☐
6. Conditioned floor area (ft²) 2724 ft² ☐
7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default)
 - a. U-factor: Description Area
(or Single or Double DEFAULT) 7a (Single Default) 237.0 ft² ☐
 - b. SHGC:
(or Clear or Tint DEFAULT) 7b. (Clear) 237.0 ft² ☐
8. Floor types
 - a. Slab-On-Grade Edge Insulation R=0.0, 193.0(p) ft ☐
 - b. N/A ☐
 - c. N/A ☐
9. Wall types
 - a. Concrete, Int Insul, Exterior R=4.1, 1269.0 ft² ☐
 - b. Frame, Wood, Exterior R=13.0, 942.0 ft² ☐
 - c. Frame, Wood, Adjacent R=13.0, 302.0 ft² ☐
 - d. N/A ☐
 - e. N/A ☐
10. Ceiling types
 - a. Under Attic R=19.0, 2150.0 ft² ☐
 - b. N/A ☐
 - c. N/A ☐
11. Ducts
 - a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft ☐
 - b. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 125.0 ft ☐

12. Cooling systems
 - a. Central Unit Cap: 33.8 kBtu/hr ☐
SEER: 13.00 ☐
 - b. Central Unit Cap: 22.6 kBtu/hr ☐
SEER: 13.00 ☐
 - c. N/A ☐
13. Heating systems
 - a. Electric Heat Pump Cap: 33.8 kBtu/hr ☐
HSPF: 7.80 ☐
 - b. Electric Heat Pump Cap: 22.6 kBtu/hr ☐
HSPF: 8.20 ☐
 - c. N/A ☐
14. Hot water systems
 - a. Electric Resistance Cap: 50.0 gallons ☐
EF: 0.90 ☐
 - b. N/A ☐
 - c. Conservation credits
(HR-Heat recovery, Solar
DHP-Dedicated heat pump) ☐
15. HVAC credits PT, ☐
(CF-Ceiling fan, CV-Cross ventilation,
HF-Whole house fan,
PT-Programmable Thermostat,
MZ-C-Multizone cooling,
MZ-H-Multizone heating)

Glass/Floor Area: 0.09

Total as-built points: 32762

Total base points: 35996

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Wayne CampbellDATE: 06/17/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McKagueDATE: 06/17/08

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt Area X SPM X SOF = Points						
.18	2724.0	18.59	9115.0	1.Single, Clear	NE	1.0	6.0	32.0	33.55	0.97	1043.0
				2.Single, Clear	NE	1.0	6.0	32.0	33.55	0.97	1043.0
				3.Single, Clear	NE	1.0	8.0	40.0	33.55	0.99	1330.0
				4.Single, Clear	SW	4.0	6.0	5.0	45.75	0.58	133.0
				5.Single, Clear	NW	1.0	6.0	16.0	29.42	0.97	457.0
				6.Single, Clear	SW	1.0	6.0	32.0	45.75	0.96	1405.0
				7.Single, Clear	SE	1.0	6.0	16.0	48.65	0.96	747.0
				8.Single, Clear	SE	1.0	6.0	16.0	48.65	0.96	747.0
				9.Single, Clear	NW	1.0	6.0	16.0	29.42	0.97	457.0
				10.Single, Clear	NW	1.0	6.0	32.0	29.42	0.97	915.0
				As-Built Total: 237.0 8277.0							
WALL TYPES Area X BSPM = Points				Type	R-Value Area X SPM = Points						
Adjacent	302.0	0.70	211.4	1. Concrete, Int Insul, Exterior		4.1	1269.0	1.13	1440.3		
Exterior	2211.0	1.70	3758.7	2. Frame, Wood, Exterior		13.0	942.0	1.50	1413.0		
				3. Frame, Wood, Adjacent		13.0	302.0	0.60	181.2		
Base Total: 2513.0 3970.1				As-Built Total: 2513.0 3034.5							
DOOR TYPES Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	18.0	2.40	43.2	1.Exterior Wood			20.0	6.10	122.0		
Exterior	20.0	6.10	122.0	2.Adjacent Wood			18.0	2.40	43.2		
Base Total: 38.0 165.2				As-Built Total: 38.0 165.2							
CEILING TYPES Area X BSPM = Points				Type	R-Value Area X SPM X SCM = Points						
Under Attic	2065.0	1.73	3572.4	1. Under Attic		19.0	2150.0	2.34 X 1.00	5031.0		
Base Total: 2065.0 3572.4				As-Built Total: 2150.0 5031.0							
FLOOR TYPES Area X BSPM = Points				Type	R-Value Area X SPM = Points						
Slab	193.0(p)	-37.0	-7141.0	1. Slab-On-Grade Edge Insulation		0.0	193.0(p)	-41.20	-7951.6		
Raised	0.0	0.00	0.0								
Base Total: -7141.0				As-Built Total: 193.0 -7951.6							
INFILTRATION Area X BSPM = Points				Area X SPM = Points							
	2724.0	10.21	27812.0	2724.0 10.21 27812.0							

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 37493.8				Summer As-Built Points: 36368.2						
Total Summer Points	X Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
37493.8	0.3250	12185.5		(sys 1: Central Unit 33800btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)						
				36368	0.60	(1.08 x 1.147 x 0.86)	0.260	0.950		5770.5
				(sys 2: Central Unit 22600btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS)						
				36368	0.40	(1.08 x 1.147 x 0.86)	0.260	0.950		3858.4
37493.8	0.3250	12185.5		36368.2	1.00	1.072	0.260	0.950		9628.8

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area				Type/SC Overhang Ornt Len Hgt Area X WPM X WOF = Points							
.18	2724.0	20.17	9890.0	1.Single, Clear	NE	1.0	6.0	32.0	32.04	1.00	1026.0
				2.Single, Clear	NE	1.0	6.0	32.0	32.04	1.00	1026.0
				3.Single, Clear	NE	1.0	8.0	40.0	32.04	1.00	1280.0
				4.Single, Clear	SW	4.0	6.0	5.0	24.09	1.38	166.0
				5.Single, Clear	NW	1.0	6.0	16.0	32.93	1.00	526.0
				6.Single, Clear	SW	1.0	6.0	32.0	24.09	1.02	787.0
				7.Single, Clear	SE	1.0	6.0	16.0	21.82	1.04	362.0
				8.Single, Clear	SE	1.0	6.0	16.0	21.82	1.04	362.0
				9.Single, Clear	NW	1.0	6.0	16.0	32.93	1.00	526.0
				10.Single, Clear	NW	1.0	6.0	32.0	32.93	1.00	1053.0
				As-Built Total: 237.0 7114.0							
WALL TYPES Area X BWPM = Points				Type R-Value Area X WPM = Points							
Adjacent	302.0	3.60	1087.2	1. Concrete, Int Insul, Exterior		4.1	1269.0	6.42	8147.0		
Exterior	2211.0	3.70	8180.7	2. Frame, Wood, Exterior		13.0	942.0	3.40	3202.8		
				3. Frame, Wood, Adjacent		13.0	302.0	3.30	996.6		
Base Total: 2513.0 9267.9				As-Built Total: 2513.0 12346.4							
DOOR TYPES Area X BWPM = Points				Type Area X WPM = Points							
Adjacent	18.0	11.50	207.0	1.Exterior Wood			20.0	12.30	246.0		
Exterior	20.0	12.30	246.0	2.Adjacent Wood			18.0	11.50	207.0		
Base Total: 38.0 453.0				As-Built Total: 38.0 453.0							
CEILING TYPES Area X BWPM = Points				Type R-Value Area X WPM X WCM = Points							
Under Attic	2065.0	2.05	4233.3	1. Under Attic		19.0	2150.0	2.70 X 1.00	5805.0		
Base Total: 2065.0 4233.3				As-Built Total: 2150.0 5805.0							
FLOOR TYPES Area X BWPM = Points				Type R-Value Area X WPM = Points							
Slab	193.0(p)	8.9	1717.7	1. Slab-On-Grade Edge Insulation		0.0	193.0(p)	18.80	3628.4		
Raised	0.0	0.00	0.0								
Base Total: 1717.7				As-Built Total: 193.0 3628.4							
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
	2724.0	-0.59	-1607.2				2724.0	-0.59	-1607.2		

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE			AS-BUILT					
Winter Base Points: 23954.7			Winter As-Built Points: 27739.6					
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 33800 btuh ,EFF(7.8) Ducts:Unc(S),Con(R),Int(AH),R6.0 27739.6 0.599 (1.060 x 1.169 x 0.88)0.437 0.950 7558.7 (sys 2: Electric Heat Pump 22600 btuh ,EFF(8.2) Ducts:Unc(S),Con(R),Int(AH),R6.0 27739.6 0.401 (1.060 x 1.169 x 0.88)0.416 0.950 4807.5					
23954.7	0.5540	13270.9	27739.6	1.00	1.095	0.428	0.950	12358.8

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
WATER HEATING										
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X Credit	= Total Multiplier
4		2635.00	10540.0	50.0	0.90	4		1.00	2693.56	1.00 10774.2
				As-Built Total:						10774.2

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points = Total Points	Cooling Points	+	Heating Points	+	Hot Water Points = Total Points
12185		13271		10540 35996	9629		12359		10774 32762

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.4

The higher the score, the more efficient the home.

ELECTRIC, , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 33.8 kBtu/hr
3. Number of units, if multi-family	1		SEER: 13.00
4. Number of Bedrooms	4	b. Central Unit	Cap: 22.6 kBtu/hr
5. Is this a worst case?	Yes		SEER: 13.00
6. Conditioned floor area (ft ²)	2724 ft ²	c. N/A	
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT)	7a(Sngle Default) 237.0 ft ²	a. Electric Heat Pump	Cap: 33.8 kBtu/hr
b. SHGC:			HSPF: 7.80
(or Clear or Tint DEFAULT)	7b. (Clear) 237.0 ft ²	b. Electric Heat Pump	Cap: 22.6 kBtu/hr
8. Floor types			HSPF: 8.20
a. Slab-On-Grade Edge Insulation	R=0.0, 193.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Concrete, Int Insul, Exterior	R=4.1, 1269.0 ft ²		EF: 0.90
b. Frame, Wood, Exterior	R=13.0, 942.0 ft ²	b. N/A	
c. Frame, Wood, Adjacent	R=13.0, 302.0 ft ²		
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=19.0, 2150.0 ft ²	15. HVAC credits	PT,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		HF-Whole house fan,	
11. Ducts		PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft		MZ-C-Multizone cooling,	
b. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 125.0 ft		MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

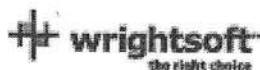
Builder Signature: Melissa McRae Date: 06/17/08

Address of New Home: 301 SW Timber Ridge City/FL Zip: Lake City, FL 32015



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5.2)



Project Summary
Entire House
Maronda Homes

Job:
Date: Aug 04, 2006
By: G.Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-0064

Project Information

For: NANTUCKET 1 ST FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	31534 Btuh
Ducts	6771 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	38305 Btuh

Sensible Cooling Equipment Load Sizing

Structure	17951 Btuh
Ducts	8403 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	25563 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

Latent Cooling Equipment Load Sizing

Structure	3114 Btuh
Ducts	1909 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	5024 Btuh
Equipment total load	30587 Btuh
Req. total capacity at 0.78 SHR	2.7 ton

	Heating	Cooling
Area (ft²)	1747	1747
Volume (ft³)	16336	16336
Air changes/hour	0.38	0.20
Equiv. AVF (cfm)	103	54

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H336AKA

Efficiency	8.2 HSPF
Heating input	0 Btuh @ 47°F
Heating output	0 °F
Temperature rise	1240 cfm
Actual air flow	0.032 cfm/Btuh
Air flow factor	0.60 in H2O
Static pressure	
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H336AKA
Coil	FSM4X3600A
Efficiency	13 SEER
Sensible cooling	26676 Btuh
Latent cooling	7524 Btuh
Total cooling	34200 Btuh
Actual air flow	1240 cfm
Air flow factor	0.047 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.84

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Page 1

Project Information

For: NANTUCKET 1 ST FLOOR

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

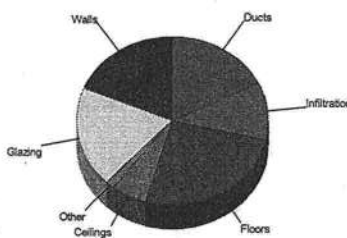
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

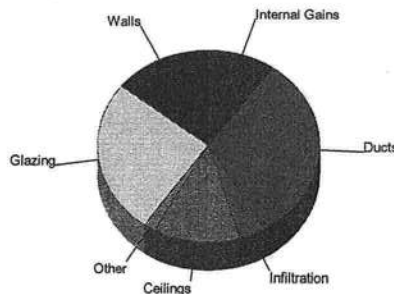
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	7209	18.8
Glazing	47.0	7377	19.3
Doors	14.4	548	1.4
Ceilings	1.8	2313	6.0
Floors	24.0	9898	25.8
Infiltration	2.3	4188	10.9
Ducts		6771	17.7
Piping		0	0.0
Humidification		0	0.0
Ventilation		0	0.0
Adjustments		0	0.0
Total		38305	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	3781	14.3
Glazing	43.7	6854	26.0
Doors	11.4	432	1.6
Ceilings	2.6	3292	12.5
Floors	0.0	0	0.0
Infiltration	0.6	1013	3.8
Ducts		8403	31.9
Ventilation		0	0.0
Internal gains		2580	9.8
Blower		0	0.0
Adjustments		0	0.0
Total		26354	100.0



Overall U-value = 0.210 Btuh/ft²-°F

Data entries checked.



Project Summary
Entire House
Maronda Homes

Job:
Date: Aug 04, 2006
By: G.Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-0064

Project Information

For: NANTUCKET 2ND FLOOR

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	11605 Btuh
Ducts	2841 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	14445 Btuh

Sensible Cooling Equipment Load Sizing

Structure	12977 Btuh
Ducts	3601 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	16080 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	869	869
Volume (ft³)	6952	6952
Air changes/hour	0.61	0.32
Equiv. AVF (cfm)	71	37

Latent Cooling Equipment Load Sizing

Structure	1304 Btuh
Ducts	793 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	2097 Btuh
Equipment total load	18177 Btuh
Req. total capacity at 0.76 SHR	1.8 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H324AKA

Efficiency	8.2 HSPF
Heating input	0 Btuh @ 47°F
Heating output	0 °F
Temperature rise	0 °F
Actual air flow	724 cfm
Air flow factor	0.050 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H324AKA
Coil	FSM4X2400A
Efficiency	13 SEER
Sensible cooling	17176 Btuh
Latent cooling	5424 Btuh
Total cooling	22600 Btuh
Actual air flow	724 cfm
Air flow factor	0.044 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.89

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Page 1

Project Information

For: NANTUCKET 2ND FLOOR

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

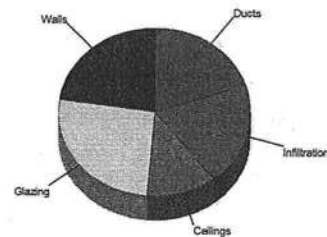
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

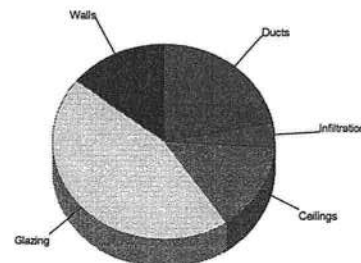
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	3.6	3273	22.7
Glazing	47.0	3759	26.0
Doors	0.0	0	0.0
Ceilings	1.8	1711	11.8
Floors	0.0	0	0.0
Infiltration	2.9	2861	19.8
Ducts		2841	19.7
Piping		0	0.0
Humidification		0	0.0
Ventilation		0	0.0
Adjustments		0	0.0
Total		14445	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.6	2366	14.3
Glazing	93.6	7485	45.2
Doors	0.0	0	0.0
Ceilings	2.6	2435	14.7
Floors	0.0	0	0.0
Infiltration	0.7	690	4.2
Ducts		3601	21.7
Ventilation		0	0.0
Internal gains		0	0.0
Blower		0	0.0
Adjustments		0	0.0
Total		16578	100.0



Overall U-value = 0.122 Btuh/ft²-°F

Data entries checked.

Duct System Summary
Entire House
Maronda Homes

Job:
Date: Aug 04, 2006
By: G.Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-0064

Project Information

For: NANTUCKET 1 ST FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.48 / 0.12 in H2O	0.48 / 0.12 in H2O
Lowest friction rate	1.935 in/100ft	1.935 in/100ft
Actual air flow	1240 cfm	1240 cfm
Total effective length (TEL)	31 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h 5637	200	200	1.935	7	x00	VIFx	25.0	0.0	st1
FAMILY	c 4616	200	200	4.839	7	x00	VIFx	10.0	0.0	st1
MASTER BATH	c 2432	100	100	3.226	5	x00	VIFx	15.0	0.0	st1
MASTER TIOLET	h 572	30	30	4.839	4	x00	VIFx	10.0	0.0	ST4
NOOK	h 4337	145	145	3.226	6	x00	VIFx	15.0	0.0	st1
KITCHEN	c 3688	145	145	4.839	6	x00	VIFx	10.0	0.0	st1
HALL BATH	c 829	30	30	9.677	4	x00	VIFx	5.0	0.0	st1
DINING/LIVING-A	h 3267	190	190	4.839	7	x00	VIFx	10.0	0.0	ST2
DINING/LIVING	h 3267	200	200	3.226	7	x00	VIFx	15.0	0.0	ST3

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	820	820	1.935	669	0	20 x 9	VinIFlx	
ST2	Peak AVF	190	190	4.839	586	7	0 x 0	VinIFlx	
ST3	Peak AVF	200	200	3.226	586	7	0 x 0	VinIFlx	
ST4	Peak AVF	30	30	4.839	314	4	0 x 0	VinIFlx	

Bold/Italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1240	1240	6.0	1.935	568	18	x0 0		VIFx	

Duct System Summary

Entire House

Maronda Homes

Job:
Date: Aug 04, 2006
By: G.Carmack

4005 Maronda Way, Sanford, FL 32771 Phone: 407-321-0064

Project Information

For: NANTUCKET 2ND FLOOR

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.43 / 0.17 in H2O	0.43 / 0.17 in H2O
Lowest friction rate	2.857 in/100ft	2.857 in/100ft
Actual air flow	724 cfm	724 cfm
Total effective length (TEL)	21 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
BDR #3	c 398.3	140	140	4.286	6	x0	VIFx	10.0	0.0	ST3
BDR #2	c 387.1	200	200	2.857	7	x0	VIFx	15.0	0.0	st4
UPSTAIRS BATH	h 673	24	24	4.286	4	x0	VIFx	10.0	0.0	ST2
BDR #4	h 223.9	180	180	8.571	7	x0	VIFx	5.0	0.0	st1
BDR #5	c 383.0	180	180	4.286	7	x0	VIFx	10.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	360	360	4.286	624	10	0 x 0	VinIFlx	
ST2	Peak AVF	24	24	4.286	547	4	0 x 0	VinIFlx	
ST3	Peak AVF	140	140	4.286	550	6	0 x 0	VinIFlx	
st4	Peak AVF	200	200	2.857	699	7	0 x 0	VinIFlx	

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	724	724	6.0	2.857	677	16	x0 0		VIFx	

Bold/italic values have been manually overridden

**COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST
FOR THE FLORIDA RESIDENTIAL BUILDING CODE 2004 with 2005 & 2006
Supplements and One (1) and Two (2) Family Dwellings**

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current FLORIDA BUILDING CODES and the Current FLORIDA RESIDENTIAL CODE. ALL PLANS OR DRAWING SHALL PROVIDED CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE- AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the Residential Code (Florida Wind speed map) SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

GENERAL REQUIREMENTS:

- ✓ Two (2) complete sets of plans containing the following:
- ✓ All drawings must be clear, concise and drawn to scale, details that are not used shall be marked void
- ✓ Condition space (Sq. Ft.) and total (Sq. Ft.) under roof shall be shown on the plans.
- ✓ Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents per FBC 106.1.

Site Plan information including:

- ✓ Dimensions of lot or parcel of land
- ✓ Dimensions of all building set backs
- ✓ Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.
- ✓ Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required:

- ✓ Plans or specifications must meet state compliance with FRC Chapter 3
- ✓ The following information must be shown as per section FRC
- ✓ Basic wind speed (3-second gust), miles per hour
- ✓ Wind importance factor and nature of occupancy
- ✓ Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated
- ✓ The applicable internal pressure coefficient, Components and Cladding The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifiably designed by the registered design professional.

Elevations Drawing including:

- ✓ All side views of the structure
- ✓ Roof pitch
- ✓ Overhang dimensions and detail with attic ventilation
- ✓ Location, size and height above roof of chimneys
- ✓ Location and size of skylights with Florida Product Approval
- ✓ Number of stories
- ✓ e) Building height from the established grade to the roofs highest peak

Floor Plan including:

- ✓ Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies and raised floor surfaces located more than 30 inches above the floor or grade
- ✓ All exterior and interior shear walls indicated
- ✓ Shear wall opening shown (Windows, Doors and Garage doors)
- ✓ Emergency escape and rescue opening in each bedroom (net clear opening shown)
- ✓ Safety glazing of glass where needed
- ✓ Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FRC)
- ✓ Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FRC 311)
- ✓ Plans must show and identify accessibility of bathroom (see FRC 322)

All materials placed within opening or onto/into exterior shear walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

Foundation Plans Per FRC 403:

- ✓ a) Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.
- b) All posts and/or column footing including size and reinforcing
- ✓ c) Any special support required by soil analysis such as piling.
- ✓ d) Assumed load-bearing value of soil _____ (psf)
- e) Location of horizontal and vertical steel, for foundation or walls (include # size and type)

CONCRETE SLAB ON GRADE Per FRC R506

- ✓ Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
- Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports

PROTECTION AGAINST TERMITES Per FRC 320:

- ✓ Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. Protection shall be provided by registered termiticides

Masonry Walls and Stem walls (load bearing & shear Walls) FRC Section R606

- ✓ Show all materials making up walls, wall height, and Block size, mortar type
- ✓ Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

- ✓ Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer
- ✓ Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers
- Girder type, size and spacing to load bearing walls, stem wall and/or piers
- ✓ Attachment of joist to girder
- ✓ Wind load requirements where applicable
- ✓ Show required under-floor crawl space
- ✓ Show required amount of ventilation opening for under-floor spaces
- ✓ Show required covering of ventilation opening.
- ✓ Show the required access opening to access to under-floor spaces
- ✓ Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing
- ✓ Show Draft stopping, Fire caulking and Fire blocking
- ✓ Show fireproofing requirements for garages attached to living spaces, per FRC section R309
- ✓ Provide live and dead load rating of floor framing systems (psf).

WOOD WALL FRAMING CONSTRUCTION FRC CHAPTER 6

- ✓ Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls.
- ✓ Fastener schedule for structural members per table R602.3 (1) are to be shown.
- ✓ Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing
- ✓ Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems.
- ✓ Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FRC Table R502.5 (1)
- ✓ Indicate where pressure treated wood will be placed.
- ✓ Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas
- ✓ A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail

ROOF SYSTEMS:

- ✓ Truss design drawing shall meet section FRC R802.10 Wood trusses. Include a layout and truss details and be signed and sealed by Fl. Pro. Eng.
- ✓ Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters
- ✓ Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details
- ✓ Provide dead load rating of trusses

Conventional Roof Framing Layout Per FRC 802:

- ✓ Rafter and ridge beams sizes, span, species and spacing
- ✓ Connectors to wall assemblies' include assemblies' resistance to uplift rating.
- ✓ Valley framing and support details
- ✓ Provide dead load rating of rafter system.

ROOF SHEATHING FRC Table R602,3(2) FRC 803

- ✓ Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

- ✓ Include all materials which will make up the roof assemblies covering; with Florida Product Approval numbers for each component of the roof assemblies covering.

FCB Chapter 13 Florida Energy Efficiency Code for Building Construction

- ✓ Residential construction shall comply with this code by using the following compliance methods in the FBC Subchapter 13-6, Residential buildings compliance methods. Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area
- ✓ Show the insulation R value for the following areas of the structure: Attic space, Exterior wall cavity and Crawl space (if applicable)

HVAC information shown

- ✓ Manual J sizing equipment or equivalent computation
- ✓ Exhaust fans locations in bathrooms

Plumbing Fixture layout shown

- ✓ All fixtures waste water lines shall be shown on the foundation plan

Electrical layout shown including:

- ✓ Switches, outlets/receptacles, lighting and all required GFCI outlets identified
- ✓ Ceiling fans
- ✓ Smoke detectors
- ✓ Service panel, sub-panel, location(s) and total ampere ratings

- ✓ On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.
- ✓ Appliances and HVAC equipment and disconnects
- ✓ Arc Fault Circuits (AFCI) in bedrooms
- ✓ Notarized Disclosure Statement for Owner Builders
- Notice of Commencement Recorded (in the Columbia County Clerk Office) Notice Of Commencement is required to be filed with the building department Before Any Inspections Will Be Done.

Private Potable Water

- ✓ Size of pump motor
- ✓ Size of pressure tank
- ✓ Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- ✓ Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.
- ✓ Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- ✓ Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- ✓ City Approval: If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- ✓ Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.** A development permit will also be required. The permit cost is \$50.00.
- ✓ Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
- 911 Address: If the project is located in an area where the 911 address has been issued, then the proper Paper work from the 911 Addressing Departments must be submitted. (386) 758-1125

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. NOTIFICATION WILL BE GIVEN WHEN THE APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT.

PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:** _____

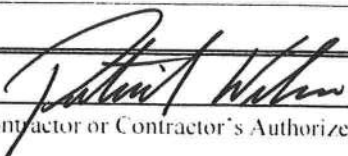
As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.fdot.com/procure/procure.asp

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging			
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS			
1. Single hung			
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			
6. Awning			
7. Pass-through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11. Dual Action			
12. Other			
C. PANEL WALL			
1. Siding			
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles			
2. Underlayments			
3. Roofing Fasteners			
4. Non-structural Metal Rf			
5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS			
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS			
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

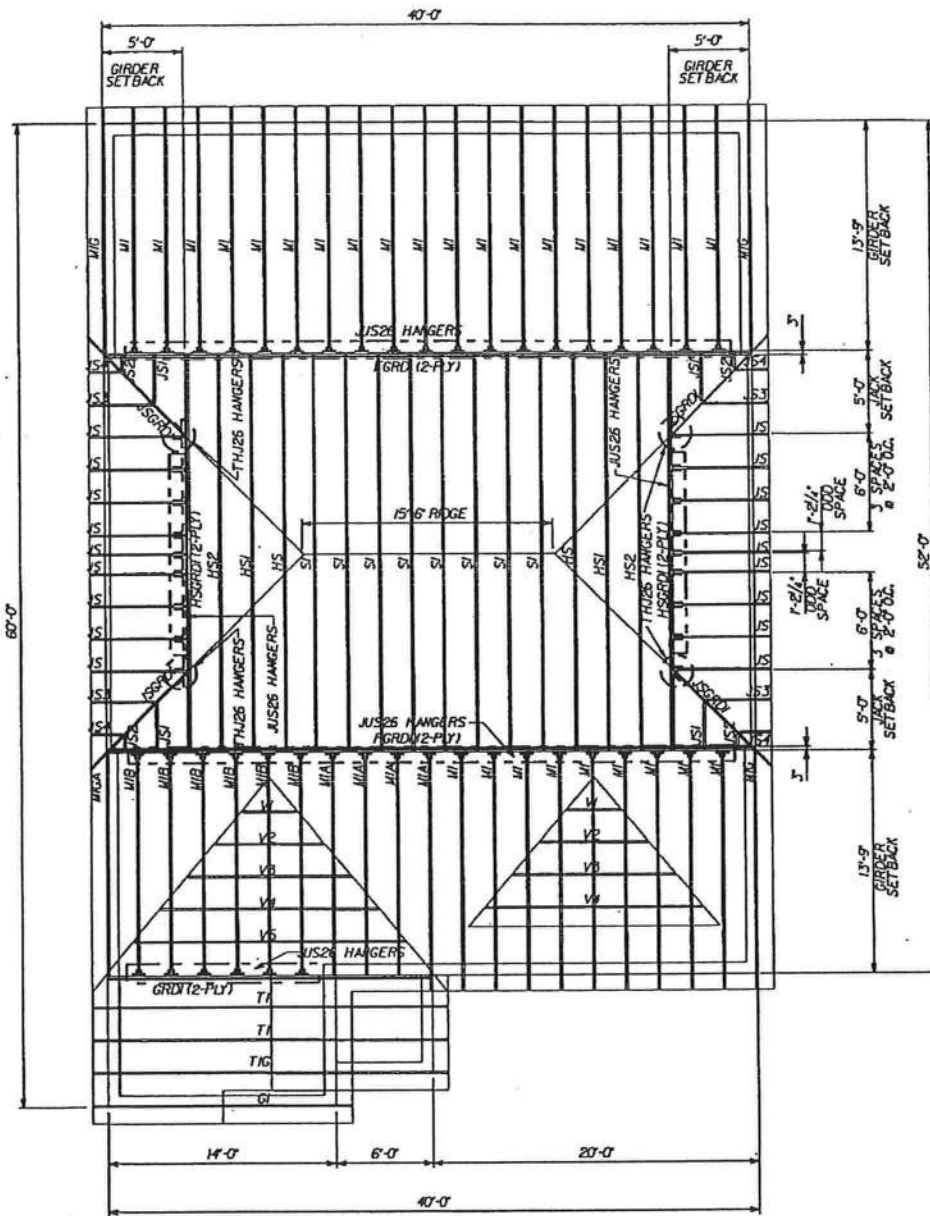
I understand these products may have to be removed if approval cannot be demonstrated during inspection


Contractor or Contractor's Authorized Agent Signature

PATRICK Wilson 8/4/08
Print Name Date

Location

Permit # (FOR STAFF USE ONLY)



HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP6F
- 5 MPA1 & MPA1F
- 6 SKH26 L/R
- 7 SKHH26 L/R
- 8 SUS26
- 9 SUS28
- 10 THD26
- 11 THD28
- 12 THD28-2
- 13 THDH28-3
- 14 THD48
- 15 THJ26**
- 16 LTW12

[Signature]

JUN 05 2008

HARDWARE MANUFACTURED
BY USP

* HARDWARE MANUFACTURED
BY SIMPSON

** HARDWARE MANUFACTURED
BY CLEVELAND

NANTUCKET ELEVATION "M" - FL

GARAGE: RIGHT

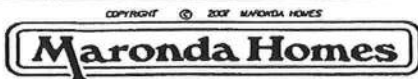
DESIGNER: CP
CHECKER: MIKE

DRAWN BY: EJ

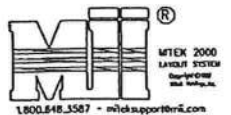
SCALE: 1/8" = 1'-0"
DATE: 11/01/2007

LOADING-FBC2004/TPI2002

TC LIVE	16.00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LIVE	10.00	PLATE DOL	1.25
BC DEAD	10.00	WIND	1.25
TOTAL	43.00	SPACING	2'-0"



14071 321-0064 4005 MARONDA WAY SAFFORD, FLORIDA



GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

Warning! The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

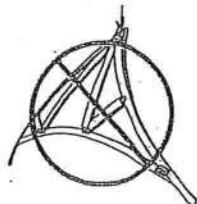
El resultado de un manejo, levantamiento, instalación, restricción y arriostre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

Warning! Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

*Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

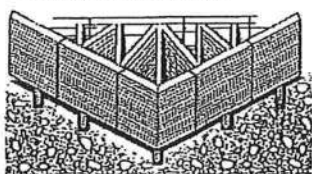
HANDLING — MANEJO

Warning! Avoid lateral bending. — Evite la flexión lateral.



Warning! The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



Checkmark! If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

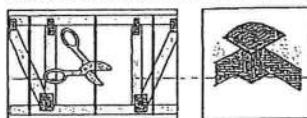
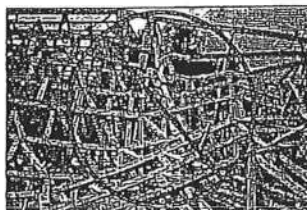
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

NOTAS GENERALES

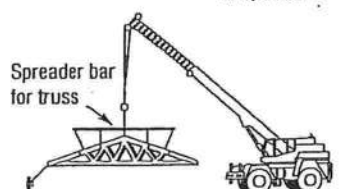
Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporal. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



Warning! Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Checkmark! Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



Warning! Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



Warning! Do not store on uneven ground.

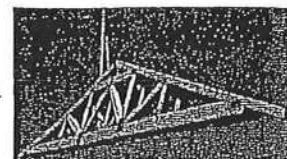
No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Warning! Don't overload the crane.
¡Advertencia! No sobrecargue la grúa!

Warning! Never use banding alone to lift a bundle.
Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete.
No levante un grupo de empaques individuales.



Checkmark! A single lift point may be used for bundles with trusses up to 45'.
Two lift points may be used for bundles with trusses up to 60'.
Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes más de 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

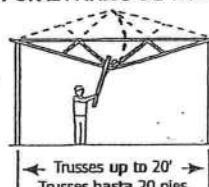
Warning! Do not over load supporting structure with truss bundle.
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

Checkmark! Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

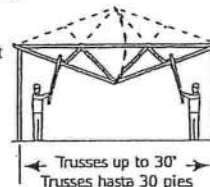
Checkmark! Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



Checkmark! Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.

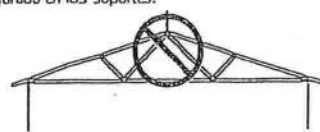


HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

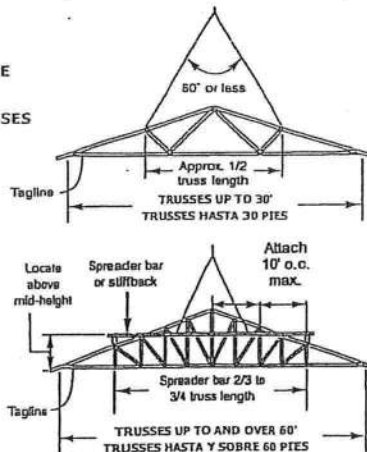
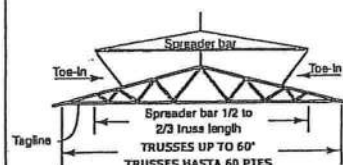
Checkmark! Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalada y el truss está asegurado en los soportes.

Warning! Using a single pick-point at the peak can damage the truss.
¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

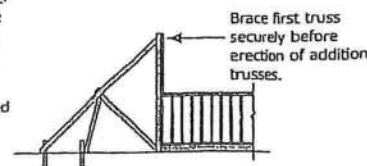
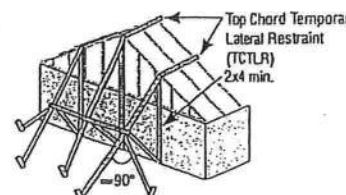
Warning! Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI B2 - Restricción/Arriostre Temporal y Instalación de los Trusses para más información.

Checkmark! Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).

Warning! Do not walk on unbraced trusses.
No camine en trusses sueltos.



STEPS TO SETTING TRUSSES
AS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.
 - 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.
- Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.
- Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
- RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLTR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un Ingeniero para trusses de mas de 60 pies.

- See BCSI-B2 for TCLTR options.
Vea el BCSI-B2 para las opciones de TCLTR.

Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

DIAGONAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT
LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!

10'-15' max. Same spacing as bottom chord Lateral Restraint

Some chord and web members not shown for clarity.

BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.

Bottom chords

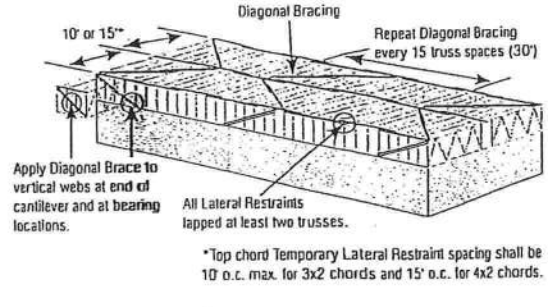
Diagonal Braces every 10 truss spaces (20' max.)

Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

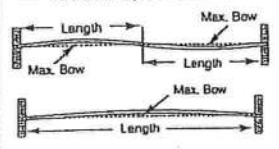
Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.

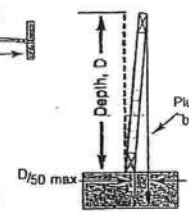


INSTALLING — INSTALACION

- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	28.2'
2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

- Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

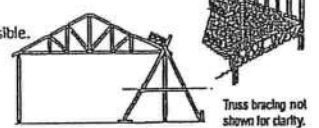
- Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Material	Height
Gypsum Board	12"
Plywood or OSB	18"
Asphalt Shingles	2 bundles
Concrete Block	8"
Clay Tile	3-4 tiles high



- Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.
- Never stack materials near a peak.
Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
- Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

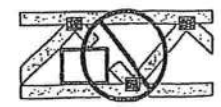


ALTERATIONS — ALTERACIONES

- Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.

Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
- No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



- Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WITCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.

WITCA
WOOD INSTITUTE FOR TRUSS CONSTRUCTION
6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com

TPI
TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinstr.org

81WARN11x17 20061115

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

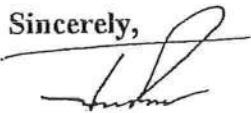
From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANS/PTI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to verify that the data shown are in agreement with the local building codes, local climate records for which the truss is intended, and the design loadings applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over bolt holes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x3 plate is 6" wide x 3" long. Slots (holes) run parallel to the plate length specified. Double ends on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber. For additional information refer to ANS/PTI 1-1995.

PRECAUTIONARY NOTES

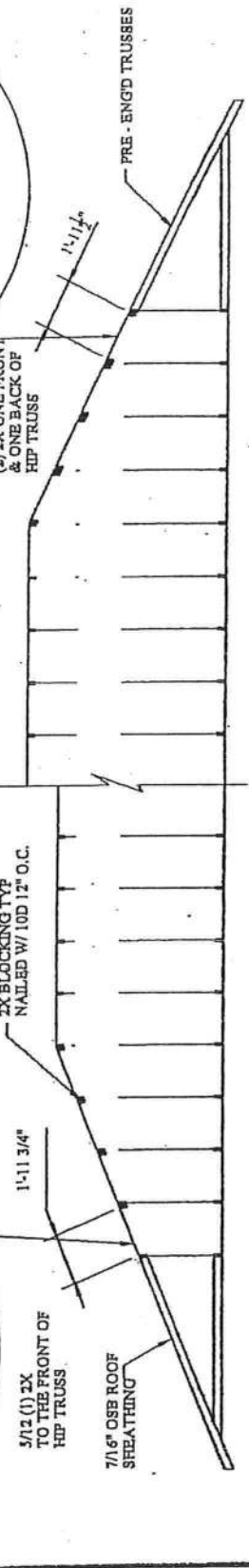
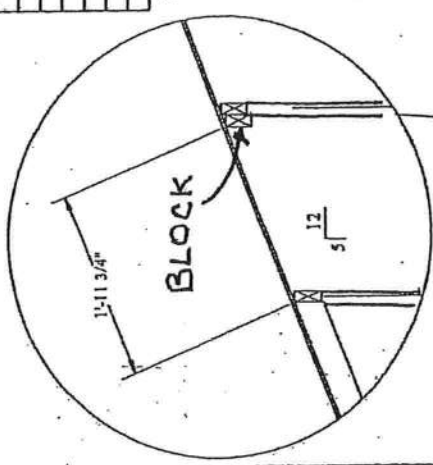
All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during hauling and unloading, delivery and installation to avoid damage. Temporary and permanent bracing and shoring shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Continuation of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

HIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083
SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C.

Roof Slope	24/16	32/16	40/20	48/24
< 1:12	OK	OK	OK	OK
< 2:12	OK	OK	OK	OK
< 3:12	OK	OK	OK	OK
< 4:12	OK	OK	OK	OK
< 5:12	OK	OK	OK	OK
< 6:12	OK	OK	OK	OK
< 7:12	OK	OK	OK	OK
< 8:12	OK	OK	OK	OK
< 9:12	OK	OK	OK	OK
< 10:12	OK	OK	OK	OK
< 11:12	OK	OK	OK	OK
< 12:12	OK	OK	OK	OK
< 13:12	OK	OK	OK	OK
< 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRES 2X LUMBER BLOCKING BETWEEN TRUSSES AT UN-SUPPORTED PANEL EDGES IN HIP ROOF AREAS



NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion PL Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI). For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 383 Dromedary Drive, Madison, Wisconsin 53719).
Campan Engineering Co., P.A., 818 Seaside Rd., Eden, NC 27332

Eng Job: WO: HIPDETAIL
DWG: TT: HIP
DSGNR: TLY Chk: 11/2/2006
TC Live 16.0 psf Lbr DF: 1.25
TC Dead 7.0 psf Plt DF: 1.25
BC Live 10.0 psf O.C.: 2-0-0
BC Dead 10.0 psf TPI-02/FBC-04
TOTAL 43.0 psf Code: FLA
v4.7.32-0

NOV 02 2006



Trenco
818 Soundside Rd
Edenton, NC 27932

Re: NANTUCKET
NANTUCKET_FL_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4510444 thru E4510471

My license renewal date for the state of Florida is February 28, 2009.

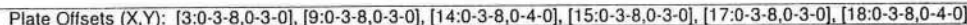


November 12, 2007

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.
Engineering services provided by Truss Engineering Company.

7 000 s Aug 7 2007 MiTek Industries, Inc. Fri Apr 25 13:35:52 2008 Page 1

LUMBER

BRACING

REACTIONS (lb/size) 20=14718/0-7-14, 12=14536/0-7-14
Max Uplift 20=-5076(LC 8), 12=-5059(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

A circular professional engineer seal for the State of Florida. The outer ring contains the text "TOMAS PONCE" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside this ring is a smaller circle with a dotted border. Within the dotted circle, the word "LICENSE" is at the top, "No. 50088" is in the center, and "STATE OF FLORIDA" is at the bottom, also separated by two stars. To the left of the seal, the text "67", "19", and "9/5/22" is visible.

NOTES

- 4-25-08

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FLORIDA_125
NANTUCKET	FGRD1	SPECIAL	2	2	Job Reference (optional)

Maronda Homes Inc., Sanford, Florida

7.000 s Aug 7 2007 MiTek Industries, Inc. Fri Apr 25 13:35:52 2008 Page 2

NOTES

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 48 lb down and 5 lb up at 1-1-5, 75 lb down at 3-1-5, 1696 lb down and 1001 lb up at 5-1-0, 997 lb down and 176 lb up at 7-0-4, 997 lb down and 221 lb up at 9-0-4, 997 lb down and 184 lb up at 11-0-4, 997 lb down and 191 lb up at 12-10-12, 997 lb down and 191 lb up at 14-10-12, 997 lb down and 191 lb up at 16-10-12, 997 lb down and 191 lb up at 18-10-12, 997 lb down and 191 lb up at 20-10-12, 997 lb down and 191 lb up at 22-10-12, 997 lb down and 191 lb up at 24-10-12, 997 lb down and 191 lb up at 26-10-12, 997 lb down and 184 lb up at 28-10-12, 997 lb down and 221 lb up at 30-10-12, 997 lb down and 176 lb up at 32-10-12, 1696 lb down and 1001 lb up at 34-10-0, and 62 lb down at 36-9-11, and 48 lb down at 38-9-11 on top chord, and 98 lb down and 53 lb up at 0-2-12, 487 lb down and 261 lb up at 1-10-12, 527 lb down and 261 lb up at 3-10-12, 527 lb down and 261 lb up at 5-10-12, 487 lb down and 261 lb up at 7-10-12, 487 lb down and 261 lb up at 9-10-12, 487 lb down and 261 lb up at 11-10-12, 527 lb down and 261 lb up at 13-10-12, 527 lb down and 261 lb up at 15-10-12, 487 lb down and 261 lb up at 17-10-12, 527 lb down and 261 lb up at 19-10-12, 487 lb down and 261 lb up at 21-10-12, 527 lb down and 261 lb up at 23-10-12, 527 lb down and 261 lb up at 25-10-12, 487 lb down and 261 lb up at 27-10-12, 527 lb down and 261 lb up at 29-10-12, 487 lb down and 261 lb up at 31-10-12, 527 lb down and 261 lb up at 33-10-12, 527 lb down and 261 lb up at 35-10-12, and 487 lb down and 261 lb up at 37-10-12, and 42 lb down and 53 lb up at 39-8-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

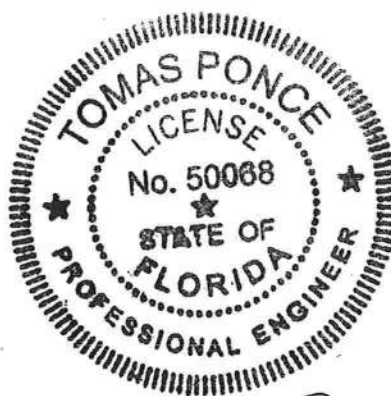
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-11=-23, 20-38=-40, 19-38=-20, 19-41=-40, 18-41=-20, 18-44=-40, 17-44=-20, 17-47=-40, 47-50=-20, 15-50=-40, 15-53=-20, 14-53=-40, 14-56=-20, 13-56=-40, 13-59=-20, 12-59=-40

Concentrated Loads (lb)

Vert: 20=-98(F) 2=-1696(B) 18=-487(F) 5=-997(B) 16=-527(F) 7=-997(B) 14=-527(F) 10=-1696(B) 21=5(B) 22=-75(B) 23=-997(B) 24=-997(B) 25=-997(B) 26=-997(B) 27=-997(B) 28=-997(B) 29=-997(B) 30=-997(B) 31=-997(B) 32=-997(B) 33=-997(B) 34=-997(B) 37=-487(F) 39=-527(F) 40=-527(F) 42=-487(F) 43=-487(F) 45=-527(F) 46=-527(F) 48=-487(F) 49=-487(F) 51=-527(F) 52=-527(F) 54=-487(F) 55=-487(F) 57=-527(F) 58=-527(F) 60=-487(F)



[Signature]
4-25-08

Job	Truss	Truss Type	Qty	By	NANTUCKET_FL_125	E4510447
NANTUCKET	G1	GABLE	1	1	Job Reference (optional)	
Meranda Homes Inc., Sanford, Florida						
7.010 a Oct 16 2007 MITek Industries, Inc. Sat Nov 10 09:35:48 2007 Page 1						

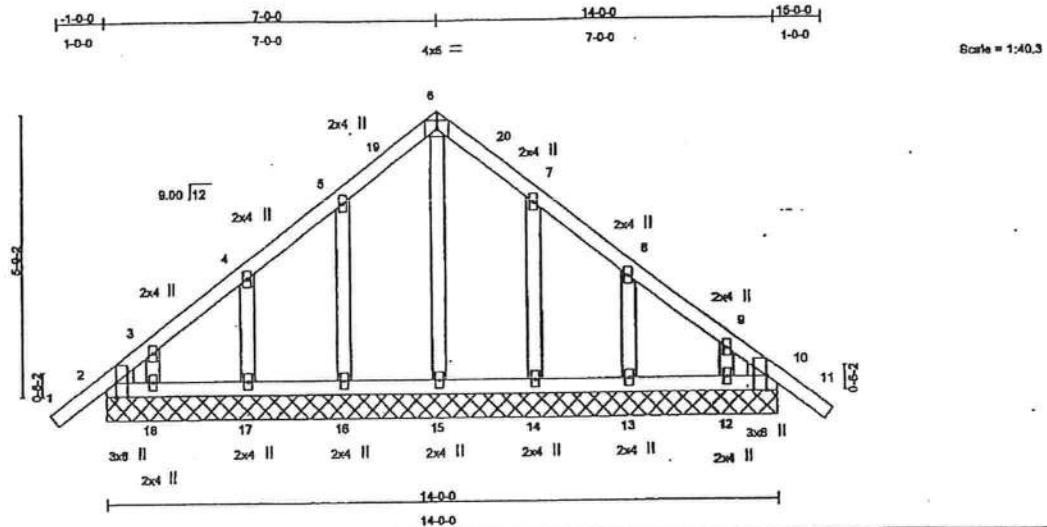


Plate Offsets (X,Y): [2-0-3-8 Edge] [10-0-3-8 Edge]												
LOADING (psf)		SPACING 2-0-0		CSI		DEFL In (loc)		I/def	L/d	PLATES	GRIP	
TCLL	18.0	Plates Increase 1.25		TC	0.11	Vert(LL)	-0.00	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase 1.25		BC	0.07	Vert(TL)	-0.00	11	n/r	120		
BCLL	10.0	Rep Stress Incr NO		WB	0.07	Horz(TL)	0.00	10	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 80 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3
WEDGE

Left: 2 X 4 SYP No.2, Right: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

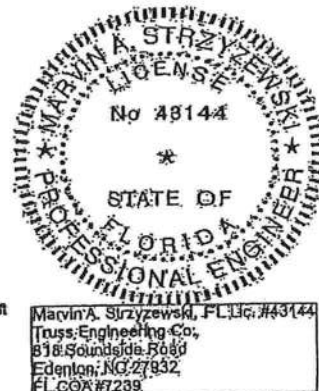
REACTIONS (lb/size) 2=117/14-0-0, 10=117/14-0-0, 15=241/14-0-0, 16=208/14-0-0, 17=178/14-0-0, 18=110/14-0-0, 14=208/14-0-0, 13=178/14-0-0, 12=110/14-0-0
Max Horz 2=248(LC 4)
Max Uplift 2=133(LC 4), 10=75(LC 5), 16=127(LC 6), 17=163(LC 8), 18=81(LC 5), 14=125(LC 7), 13=184(LC 7), 12=69(LC 4)
Max Grav 2=117(LC 1), 10=117(LC 1), 15=241(LC 1), 16=212(LC 10), 17=176(LC 1), 18=112(LC 10), 14=212(LC 11), 13=176(LC 1), 12=112(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=235/166, 3-4=159/141, 4-5=82/133, 5-19=47/183, 6-19=19/189, 6-20=19/189, 7-20=47/193, 7-8=48/99, 8-9=81/53, 9-10=150/81, 10-11=0/25
BOT CHORD 2-18=40/183, 17-18=40/183, 16-17=40/183, 15-16=40/183, 14-15=40/183, 13-14=40/183, 12-13=40/183, 10-12=40/183
WEBS 6-15=135/0, 5-16=94/152, 4-17=97/184, 3-18=81/132, 7-14=94/150, 8-13=97/184, 9-12=61/128

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 2, 75 lb uplift at joint 10, 127 lb uplift at joint 16, 183 lb uplift at joint 17, 81 lb uplift at joint 18, 125 lb uplift at joint 14, 164 lb uplift at joint 13 and 69 lb uplift at joint 12.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-81 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERED BY
TRENCO
A MITek Affiliate

818 Soundside Road
Edenton, NC 27832

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510448
NANTUCKET	GRD1	COMMON	1	2	Job Reference (optional)	
Maronde Homes Inc., Sanford, Florida						
7.010 s Oct 16 2007 MITek Industries, Inc. Sat Nov 10 09:35:47 2007 Page 1						

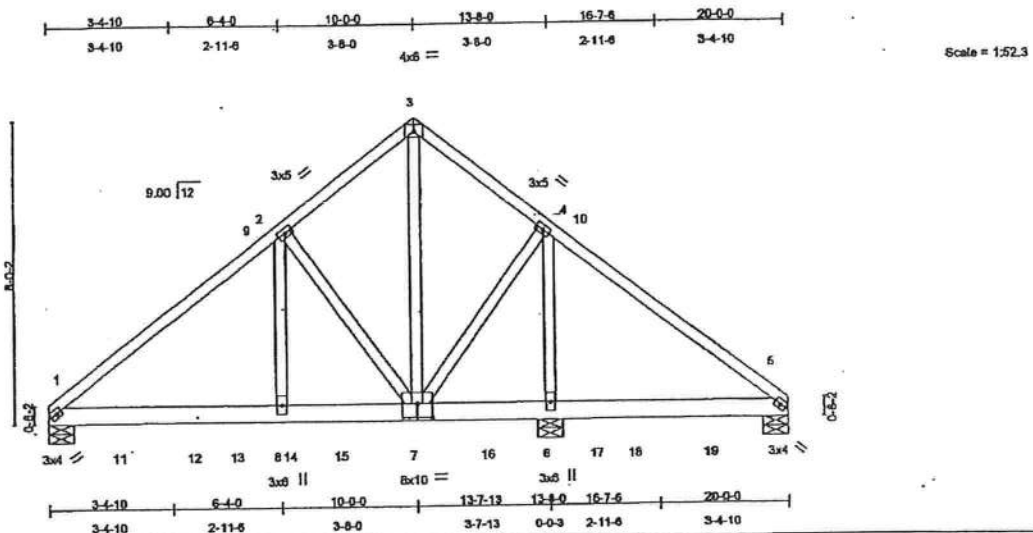


Plate Offsets (X,Y): 0-0-5-0-0-4-8						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	Vdefl
TCLL 16.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.06	1-8 >099
TCOL 7.0	Lumber Increase	1.25	BC 0.68	Vert(TL)	-0.12	1-8 >099
BCCL 10.0	Rep Stress Incr	NO	WB 0.31	Horz(TL)	0.01	6 n/a
BCOL 10.0	Code FBC2004/TP12002		(Metric)			
						Weight: 251 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
3-7 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-0 oc bracing.

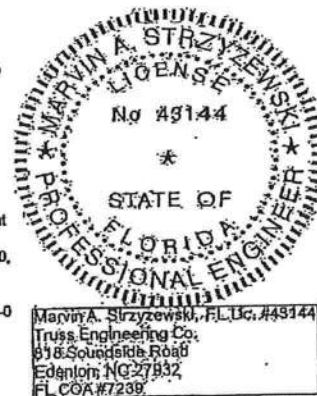
REACTIONS (lb/size) 1=2145/0-8-8, 5=1091/0-8-8, 8=3951/0-8-8
Max Horz 1=341(LC 3)
Max Uplift 1=535(LC 5), 5=237(LC 6), 8=938(LC 6)
Max Grav 1=2145(LC 1), 5=1103(LC 10), 8=3951(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-9=2233/534, 2-9=2077/536, 2-3=910/344, 3-4=912/340, 4-10=45/272, 5-10=76/263
BOT CHORD 1-11=494/1700, 11-12=494/1700, 12-13=494/1700, 13-14=494/1700, 8-14=494/1700, 8-15=494/1700,
7-15=494/1700, 7-16=160/183, 6-16=160/183, 6-17=160/183, 17-18=160/183, 18-19=160/183, 5-19=160/183
WEBS 2-8=356/1939, 2-7=1712/592, 3-7=338/925, 4-7=346/1334, 4-8=1957/595

NOTES

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all piers, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.80 plate grip DOL=1.80.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 535 lb uplift at joint 1, 237 lb uplift at joint 5 and 938 lb uplift at joint 6.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 58 lb down and 22 lb up at 0-4-0, 542 lb down and 116 lb up at 1-11-4, 542 lb down and 116 lb up at 3-11-4, 542 lb down and 116 lb up at 5-11-4, 542 lb down and 116 lb up at 7-11-4, 542 lb down and 116 lb up at 9-11-4, 542 lb down and 116 lb up at 11-11-4, 531 lb down and 107 lb up at 13-11-4, 531 lb down and 107 lb up at 15-11-4, and 531 lb down and 107 lb up at 17-11-4, and 551 lb down and 107 lb up at 19-8-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
Continued on page 2



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE ME-T473 BEFORE USE.
Design valid for use only with Mittek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/APA Quality Criteria, DSR-89 and ICC-ES Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mittek Affiliate
818 Soundside Road
Edenboro, NC 27832

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510448
NANTUCKET	GRD1	COMMON	1	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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LOAD CASE(S) Standard

1) Regular; Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-46, 3-5=-46, 1-13=-40, 8-13=-80, 6-8=-40, 6-17=-80, 5-17=-40

Concentrated Loads (lb)

Vert: 1=-38(F) 5=-551(F) 7=-542(F) 6=-531(F) 11=-542(F) 12=-542(F) 14=-542(F) 15=-542(F) 16=-542(F) 18=-531(F) 19=-531(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED METEK REFERENCE PAGE M1-7473 BEFORE USE.
 Design void for use only with Metek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of components is responsibility of building designer - not truss designer. Bracing shown
 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and IBC311 Building Component
 Safety Information available from Truss Plate Institute, 583 O'Connell Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Metek Alliance

818 Soundside Road
 Eden, NC 27532

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510448
NANTUCKET	HS	SPECIAL	2	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida						
7.010 s Oct 16 2007 Mitek Industries, Inc. Sat Nov 10 09:35:48 2007 Page 1						

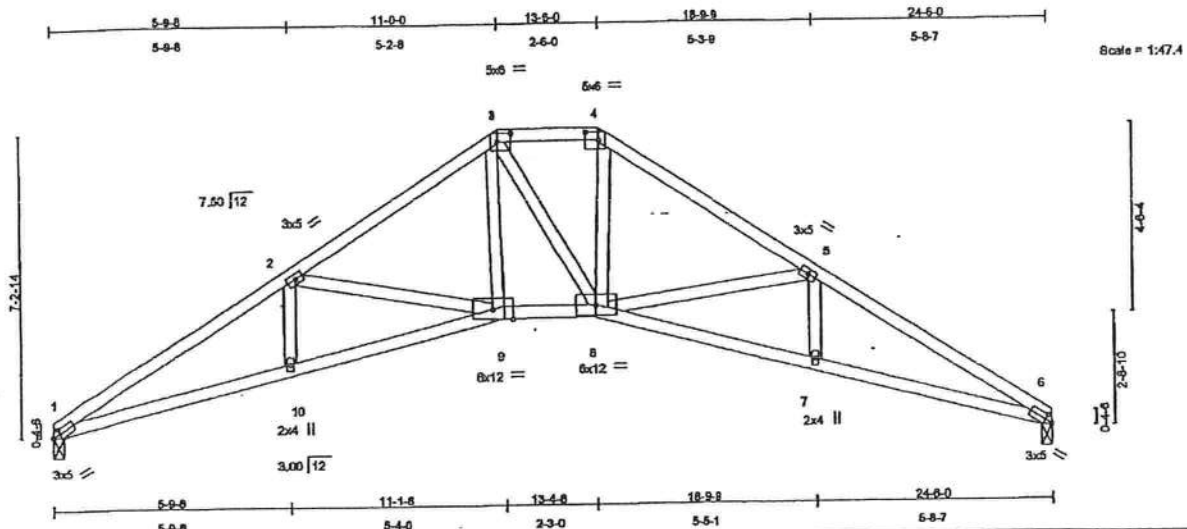


Plate Offsets (X,Y): [1-0-2-2-0-1-8], [3-0-4-0-0-2-4], [4-0-4-0-0-2-4], [6-0-2-2-0-1-8]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Vdefl
TCCL 18.0	Plates Increase	1.25	TC 0.27	Vert(LL)	-0.15	9-10 >899
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL)	-0.30	9-10 >860
BCCL 10.0	Rep Stress Incr	YES	WB 0.31	Horz(TL)	0.23	6 n/a
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)			
						Weight: 120 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-8-12 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-1-8 oc bracing.

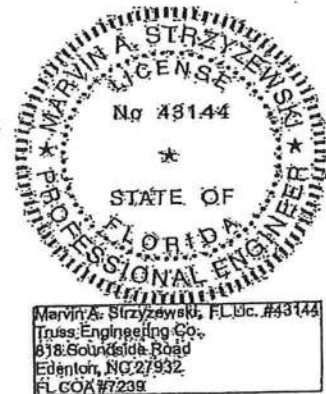
REACTIONS (lb/size) 1=1043/0-3-0, 8=1043/0-3-0
 Max Horz 1=250(LC 4)
 Max Uplift 1=184(LC 6), 8=203(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-2537/756, 2-3=-1824/559, 3-4=-1518/540, 4-5=-1834/560, 5-6=-2542/758
 BOT CHORD 1-10=-567/2182, 9-10=-566/2173, 8-9=-211/1507, 7-8=-570/2179, 6-7=-573/2186
 WEBS 2-10=0/254, 2-9=-623/340, 3-8=-174/175, 5-8=-611/339, 5-7=0/249, 4-8=-140/753, 3-9=-149/763

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 184 lb uplift at joint 1 and 203 lb uplift at joint 8.

LOAD CASE(S) Standard



November 12, 2007

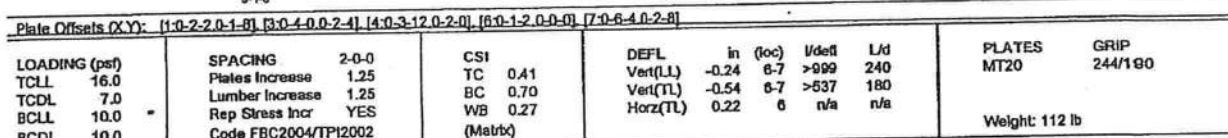
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M1-1478 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D38-89 and BCS11 Building Component Safety Information - available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITEK AFFILIATE
 818 Soundside Road
 Edenton, NC 27932

4.9 9.0 15.6 18.8 24.6

4.9 9.0 15.6 18.8 24.6

Scale = 1:43.5



BRACING	Structural wood sheathing directly applied or 3-6-10 oc purfins.
TOP CHORD	
BOT CHORD	Rigid ceiling directly applied or 7-8-6 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=2405/841, 2-3=2125/656, 3-4=1789/623, 4-5=2125/855, 5-8=2414/848
BOT CHORD	1-8=655/2100, 7-8=355/1785, 6-7=661/2109
WEBS	2-8=2432/723, 4-8=189/189, 5-7=302/729, 4-7=107/845, 3-8=108/840

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.80 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSVTP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 220 lb uplift at joint 1 and 171 lb uplift at joint 6.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-4478 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Mitek designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AISC/TPI Quality Criteria, D58-87 and BCSII Building Component Safety Information available from Trust Plate Institute, 583 O'Connell Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
A MTRK Affiliat

518 Soundside Road
Edenboro, NC 27932

Memoda Homes Inc. Sanford, Florida

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LOADING (psi)	
TCLL	15.0
TCDL	7.0
BCLL	10.0
BCDL	10.0

SPACING 2-0-0
Plates Increase 1.25
Lumber Increase 1.25
Rep Stress Incr YES
Code FBC2004/TPI2002

CSI	
TC	0.40
BC	0.71
WB	0.31
(Matrix)	

DEFL	0.0
Vert(LL)	-0.1
Vert(TL)	-0.3
Horz(TL)	0.2

pc)	l/defl	U/d
-8	>999	240
-8	>847	180
5	n/a	n/a

PLATES	GRIP
MT20	244/190
Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	Structural wood sheathing directly applied or 3-7-7 oc purlins.
TOP CHORD	
BOT CHORD	Rigid ceiling directly applied or 7-11-5 oc bracing.

REACTIONS (lb/size) 1=1043/0-3-0, 5=1043/0-3-0
Max Horz 1=180(LC 4)
Max Uplift 1=194(LC 5), 5=194(LC 4)

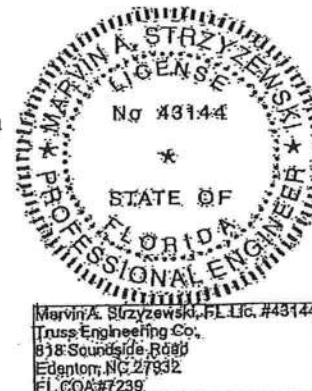
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=2430/773, 2-3=2037/1749, 3-4=2037/1749, 4-5=2430/773
BOT CHORD	1-8=572/2094, 7-8=625/2340, 6-7=625/2340, 5-6=564/2094
WEBS	3-8=469/320, 3-7=Q/161, 3-6=469/321, 4-6=133/955, 2-8=131/955

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$, Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.80 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSVTP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 194 lb uplift at joint 1 and 194 lb uplift at joint 5.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES MU-9473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and ICCSI Building Component Safety Information, available from Truss Plate Institute, 583 O'Donoghue Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A-1000 A-1000

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NAHTUCKET_PL_123	E4510452
NANTUCKET	HSGRO1	SPECIAL	2	2	Job Reference (optional)	
Maronde Homes Inc., Sanford, Florida						7.010 a Oct 18 2007 MITek Industries, Inc. Sat Nov 10 09:35:56 2007 Page 1

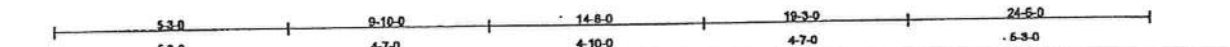
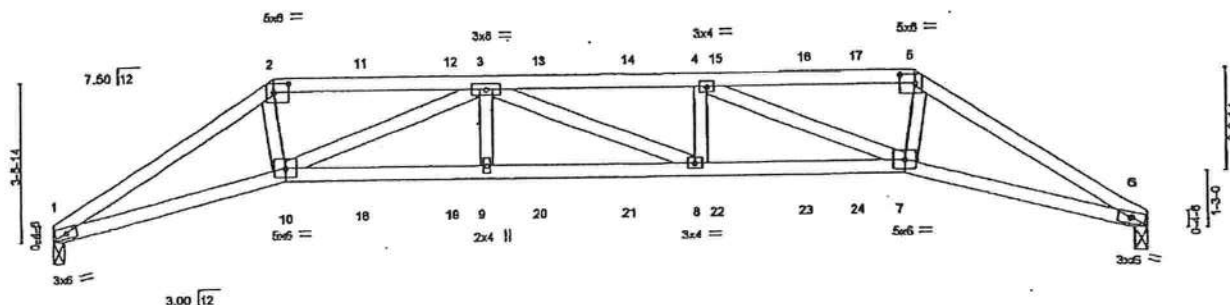
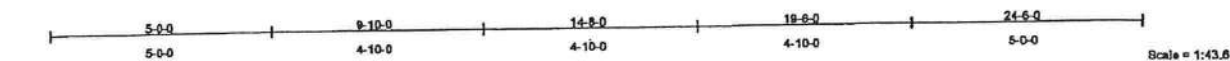


Plate Offsets (X,Y): [2-0-4-0-0-2-4], [5-0-4-0-0-2-4]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.37	Ver(LL)	0.28	8-8	>600	MT20	244/160
TCCL 7.0	Lumber Increase	1.25	BC 0.63	Ver(TL)	-0.45	8-8	>650		
BCCL 10.0	Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.25	6	n/a		
BCCL 10.0	Code FBC2004/TPI2002		(Matrb)						Weight: 218 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

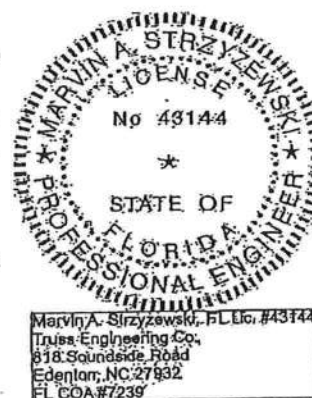
BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-13 oc bracing.

REACTIONS (lb/ft) 1=1742/0-3-0, 6=1788/0-3-0
Max Horz 1=115(LC 4)
Max Uplift 1=1008(LC 7), 6=1069(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=4596/2829, 2-11=4072/2547, 11-12=4071/2547, 3-12=4071/2548, 3-13=5765/3596, 13-14=5765/3596,
4-14=5765/3596, 4-15=4186/2680, 15-16=4197/2680, 16-17=4197/2681, 5-17=4197/2681, 5-6=4740/2988
BOT CHORD 1-10=2420/3950, 10-18=3435/5696, 18-19=3435/5696, 9-19=3435/5696, 9-20=3435/5696, 20-21=3435/5696,
8-21=3435/5696, 8-22=3512/5765, 22-23=3512/5765, 23-24=3512/5765, 7-24=3512/5765, 6-7=2509/4075
WEBS 3-9=0/323, 3-8=119/91, 4-8=0/317, 4-7=1762/1125, 5-7=851/2104, 2-10=946/2081, 3-10=1823/1180

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf, BCCL=6.0psf; Category II; Exp B; enclosed; MVFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1008 lb uplift at joint 1 and 1069 lb uplift at joint 6.

Continued on page 2



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-14TS BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D38-89 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Alliance

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510452
NANTUCKET	HSGRD1	SPECIAL	2	2	Job Reference (optional)	

Maronda Homes Inc., Sanford, Florida

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NOTES

- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 198 lb down and 378 lb up at 5-0-0, 57 lb down and 152 lb up at 7-0-12, 57 lb down and 152 lb up at 9-0-12, 57 lb down and 152 lb up at 11-0-12, 57 lb down and 152 lb up at 13-0-12, 57 lb down and 152 lb up at 15-0-12, 57 lb down and 152 lb up at 17-0-12, and 57 lb down and 152 lb up at 18-3-0, and 188 lb down and 378 lb up at 19-6-0 on top chord, and 161 lb down at 5-3-0, 56 lb down at 7-0-12, 56 lb down at 9-0-12, 56 lb down at 11-0-12, 56 lb down at 13-0-12, 56 lb down at 15-0-12, 56 lb down at 17-0-12, and 56 lb down at 18-3-0, and 161 lb down at 19-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert 1-2=-46, 2-5=-46, 5-6=-46, 1-10=-40, 7-10=-40, 6-7=-40

Concentrated Loads (lb)

Vert 2=-166(F) 5=-166(F) 10=-161(F) 7=-161(F) 11=-57(F) 12=-57(F) 13=-57(F) 14=-57(F) 15=-57(F) 16=-57(F) 17=-57(F) 18=-56(F) 19=-56(F) 20=-56(F) 21=-56(F) 22=-56(F) 23=-56(F) 24=-56(F)

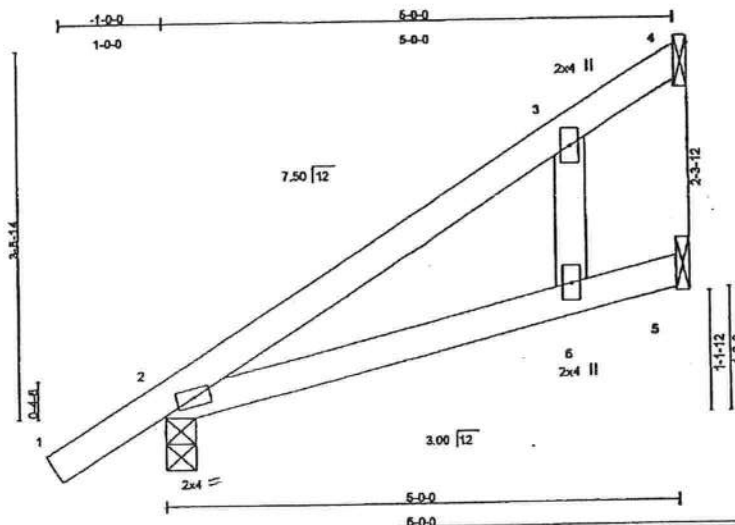


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7478 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AISC/IT11 Quality Criteria, D58-89 and IBCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

816 Boudside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NAHUTCKET_FL_125	E4910453
NAHUTCKET	JS	MONO SCISSOR	18	1	Job Reference (optional)	
Marenda Homes Inc., Sanford, Florida						
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/def	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.20	Ver(LL)	0.05	2-8	>889	MT20	244/180
TCDL 7.0	Lumber Increase	1.25	BC 0.23	Ver(TL)	-0.07	2-8	>786		
BCLL 10.0	Rep Stress Incr	YES	WB 0.05	Horz(TL)	-0.00	4	n/a		
BCDL 10.0	Code FBC2004/TP1002		(Matrix)						
									Weight: 21 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 4=83/Mechanical, 2=289/0-3-8, 5=108/Mechanical
Max Horz2=238(LC 6)
Max Uplift4=27(LC 6), 2=138(LC 6), 5=114(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/25, 2-3=148/25, 3-4=22/47
BOT CHORD 2-8=20/13, 5-6=34/25
WEBS 3-6=18/184

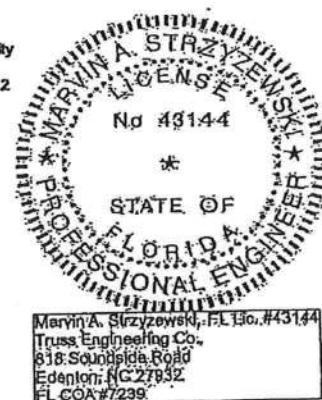
NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 1'-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 4, 138 lb uplift at joint 2 and 114 lb uplift at joint 5.

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 5'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5'-0-0 oc bracing.



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, D58-87 and ICSI Building Component Safety Information, available from Truss Plate Institute, 553 O'Connell Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27832

Mamoda Homes Inc., Sanford, Florida

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PLATES	GRIP
MT20	244/190

Weight: 10 lb

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

TOP CHORD	Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

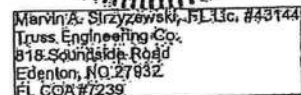
Max Horiz 1=114(LC 6)
Max Uplift 1=16(LC 6), 2=111(LC 6)

TOP CHORD $1-2=81/28$
BOT CHORD $1-3=11/11$

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(s) zone; Lumber $DOL=1.60$ plate grip $DOL=1.80$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 1 and 111 lb uplift at joint 2.

LOAD CASE(S) Standard



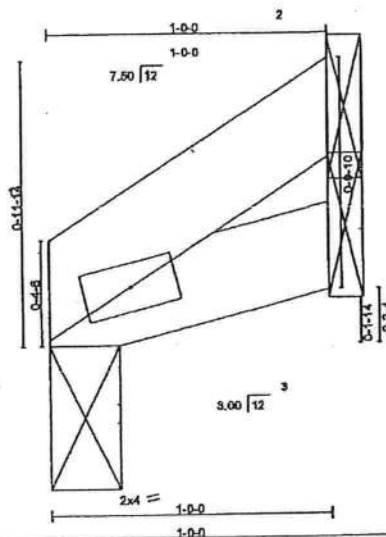
November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE M01-7473 BEFORE USE.
Design valid for use only with Mitel connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not Iuss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control storage, delivery, erection and bracing, consult ANS1/ITI Quality Criteria, D58-87 and BC311 Building Component Safety Information available from Iuss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510455
NANTUCKET	JS2	MONO SCISSOR	4	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						
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Scale = 1/8"

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Udefl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.02	Vert(LL)	-0.00	1	>999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	1	>999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	2	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 4 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=41/0-3-0, 3=18/Mechanical, 2=22/Mechanical
Max Horz 1=43(LC 6)
Max Uplift 1=3(LC 6), 2=40(LC 6)

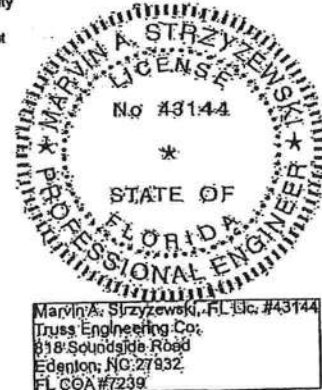
FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=31/11
BOT CHORD 1-3=4/4

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TP11 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3 lb uplift at joint 1 and 40 lb uplift at joint 2.

LOAD CASE(S) Standard



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November 12, 2007

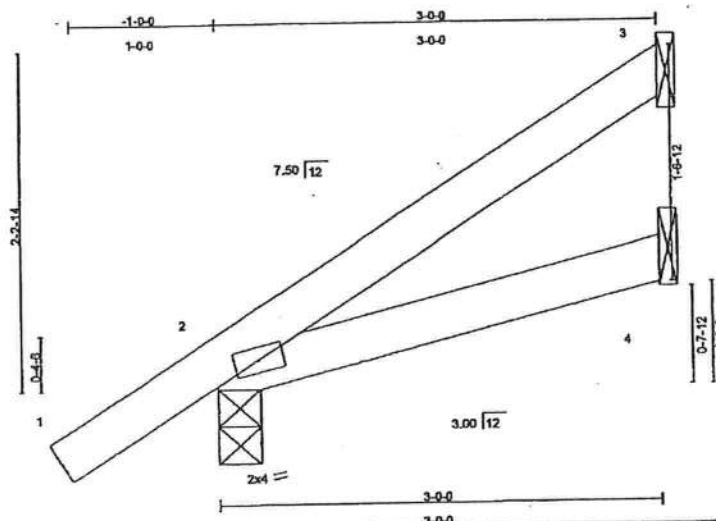


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE ML-7473 BEFORE USE.
Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSI-81 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Connell Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	54510456
NANTUCKET	J83	MONO SCI650R	4	1	Job Reference (optional)	
Meranda Homes Inc., Sanford, Florida						
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Scale = 1:13.1

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Udefl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.12	Veri(LL)	-0.00	2-4	>999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.07	Veri(TL)	-0.01	2-4	>999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						
								Weight: 12 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

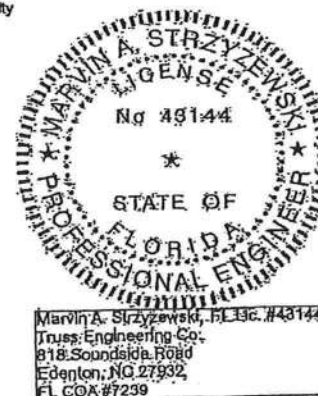
REACTIONS (lb/size) 3=52/Mechanical, 2=189/0-3-8, 4=68/Mechanical
Max Horz 2=164(LC 6)
Max Uplift 3=85(LC 6), 2=133(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=66/22
BOT CHORD 2-4=11/11

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP11 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 3 and 133 lb uplift at joint 2.

LOAD CASE(S) Standard



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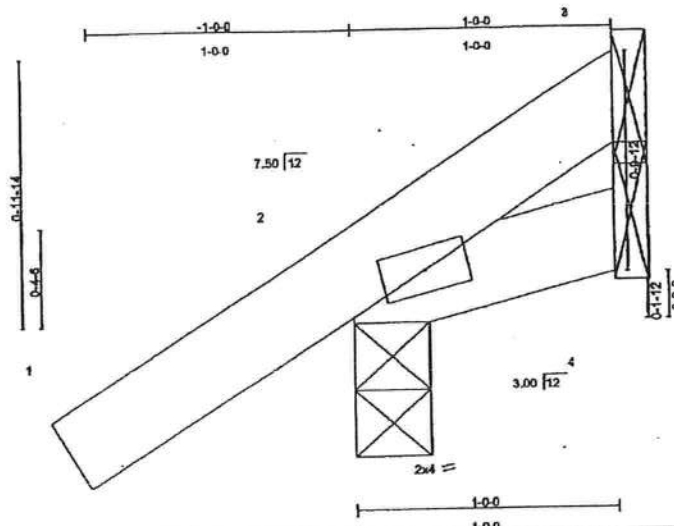
November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M3-1413 BEFORE USE.
Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, D58-87 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Connell Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
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Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510457
NANTUCKET	J84	MONO SCISSOR	4	1	Job Reference (optional)	
Marade Homes Inc., Sanford, Florida						
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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	V/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.10	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 5 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=121/0-3-8, 4=19/Mechanical, 3=7/Mechanical
Max Horz2=60(LC 6)
Max Uplift2=147(LC 6), 3=7(LC 1)
Max Grav2=121(LC 1), 4=19(LC 1), 3=21(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/23, 2-3=31/15
BOT CHORD 2-4=44

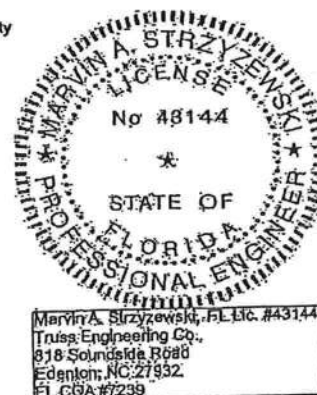
NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 2 and 7 lb uplift at joint 3.

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.



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November 12, 2007



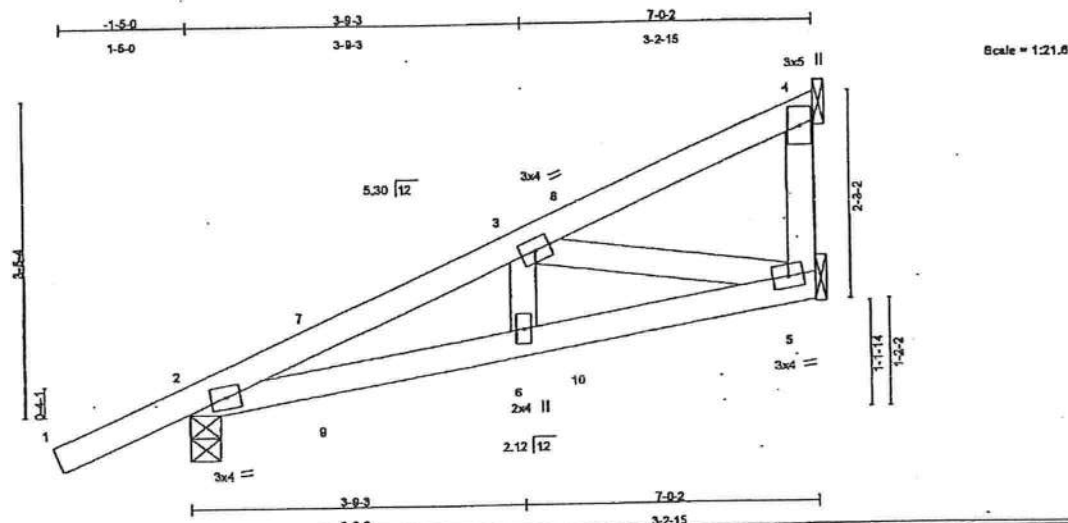
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES M17-173 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and ECSI Building Component Safety Information available from Truss Plate Institute, 383 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A M E T A M E

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510450
NANTUCKET	JSGRD1	MONO SCISSOR	4	1	Job Reference (optional)	
Maronde Homes Inc., Sanford, Florida						7.910 s Oct 16 2007 MTEK Industries, Inc. Sat Nov 10 09:35:52 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.18	Vert(LL)	-0.01	6	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.18	Vert(TL)	-0.02	5-6	>999		
BCLL 10.0	Lumber Increase 1.25	WB 0.11	Horz(TL)	0.01	5	n/a		
BCDL 10.0	Rep Stress Incr NO	(Meibq)						
	Code FBC2004/TP12002						Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (b/size) 2=304/0-4-3, 5=233/Mechanical, 4=68/Mechanical
Max Horz 2=234(LC 5)
Max Uplift 2=199(LC 5), 5=104(LC 5), 4=100(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-7=580/242, 3-7=548/251, 3-8=40/38, 4-8=46/22
BOT CHORD 2-9=326/499, 6-9=320/511, 6-10=330/487, 5-10=328/508
WEBS 4-5=0/0, 3-6=0/165, 3-5=499/325

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 2, 104 lb uplift at joint 6 and 100 lb uplift at joint 4.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 62 lb down and 44 lb up at 0-0-5, 11 lb down and 24 lb up at 1-6-12, 19 lb down and 93 lb up at 4-4-12, and 6 lb down and 67 lb up at 4-4-12, and 44 lb down at 1-6-13 on top chord, and 21 lb up at 1-6-12, 21 lb up at 1-6-13, and 16 lb down at 4-4-12, and 16 lb down at 4-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

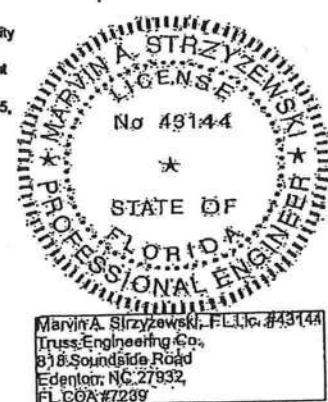
LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (psf)

Vert 1-4=46, 2-5=40

Concentrated Loads (lb)

Vert 2=52(F) 7=24(F) 8=25(F=18, B=5) 9=42(F=21, B=21) 10=32(F=18, B=16)



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November 12, 2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MTEK REFERENCE PAGE M17-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27832

Mamoda Homes Inc. Sanford, Florida



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2 *Except*
3-6 2 X 4 SYP No.3

BRACING TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except and verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WERS	1 Row at midpt 4-5

REACTIONS (lb/size) 5=567/Mechanical, 2=639/0-8
Max Horiz 2=385(LC 6)
Max Up/In 5=249(LC 6), 2=110(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=891/0, 3-4=115/47, 4-5=123/165
BOT CHORD 2-6=286/517, 5-6=286/517
WEBS 3-6=0/310, 3-5=595/328

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 5 and 110 lb uplift at joint 2.

The seal is circular with a double-lined border. Between the lines, the text "PROFESSIONAL ENGINEER" is written along the bottom arc, and "STATE OF FLORIDA" is written along the top arc. In the center of the seal, the text "MARVIN A. STRZYZEWSKI" is at the top, "E.L.G.C.#" is in the middle, "No. 43144" is below that, and "FLORIDA" is at the bottom. There are small stars on either side of the "E.L.G.C.#" text.

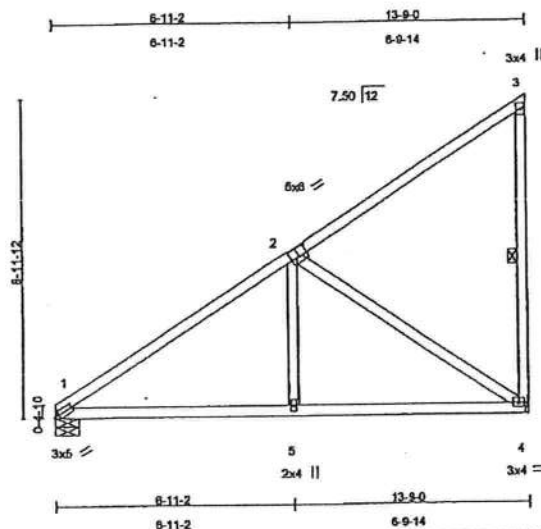
Marvin A. Strzyzewski, E.L.G.C. #43144
 Trues Engineering Co.
 818 Soundside Road
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 FL CEA #7239

November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MS-1473 BEFORE USE.
Design valid for use only with Mitek cone clasp. The design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not Ius designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/PTI Quality Criteria, D58-89 and ECST Building Component Safety Information available from Ius Pole Institute, 583 O'Conor Drive, Madison, WI 53717.

ENGINEERING BY
TRENCO
A McTAK Airmat
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510460
NANTUCKET	M1A	MONO TRUSS	4	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida						
7.010 s Oct 16 2007 MITek Industries, Inc. Sat Nov 10 08:35:53 2007 Page 1						



Scale = 1/25.9

Plate Officials (X.Yr: [1-0-1-8-0-1-8] [2-0-3-0-0-3-0])

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	Wdefl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.29	Vert(LL)	-0.07	1-5	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.48	Vert(TL)	-0.14	1-5	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.58	Horz(TL)	0.01	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2 *Except*
2-5 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-4

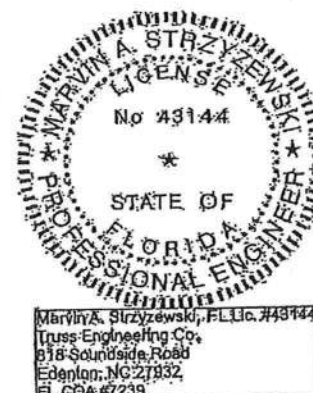
REACTIONS (lb/strze) 4=571/Mechanical, 1=571/0-8-8
Max Horz 1=355(LC 6)
Max Uplift 1=254(LC 6), 1=25(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=682/0, 2-3=115/47, 3-4=122/163
BOT CHORD 1-5=303/525, 4-5=303/525
WEBS 2-5=0/316, 2-4=605/349

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 4 and 25 lb uplift at joint 1.

LOAD CASE(S) Standard



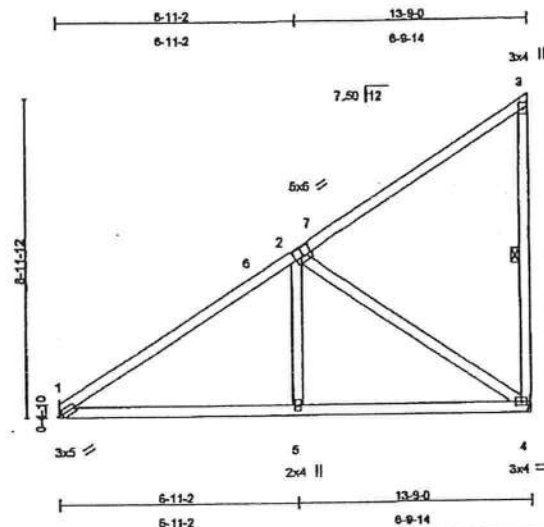
Marvin A. Strzyzewski, F.L.E. #43144
Truss Engineering Co.
818 Soundside Road
Edenport, NC 27832
FL CEA #7239

November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MS-7473 BEFORE USE.
Design valid for use only with MITek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/APA Quality Criteria, D58-89 and ICC311 Building Component Safety Information, available from Truss Plate Institute, 583 O'Connor Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A L.L. & A. LLC
818 Soundside Road
Edenport, NC 27832

Job	Truss	Truss Type	Qty	Pty	NANTUCKET_FL_125	E4510461
NANTUCKET	M18	MONO TRUSS	6	1	Job Reference (optional)	
Maroda Homes Inc., Sanford, Florida						
7.010 s Oct 16 2007 MITek Industries, Inc. Sat Nov 10 09:25:53 2007 Page 1						



Scale = 1:55.9

Plate Offsets (X,Y): [1-0-1-9-0-1-8] [2-0-3-0-0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(oc)	I/defl	L/d	PLATES	GRIP
TCLL 18.0	Plates Increase	1.25	TC 0.35	Ver(LL)	0.10	1-5	>999	240	MT20	244/190
TCCL 7.0	Lumber Increase	1.25	BC 0.49	Ver(TL)	-0.17	1-5	>976	180		
BCCL 10.0	Rep Stress Incr	YES	WB 0.61	Horz(TL)	0.01	4	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Mat/rx)							
									Weight: 75 lb	

LUMBER
 TOP CHORD 2X 4 SYP No.2
 BOT CHORD 2X 4 SYP No.2
 WEBS 2X 4 SYP No.2 "Except"
 2-5 2X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 9-4-10 oc bracing.
 WEBS 1 Row at midpt 3-4

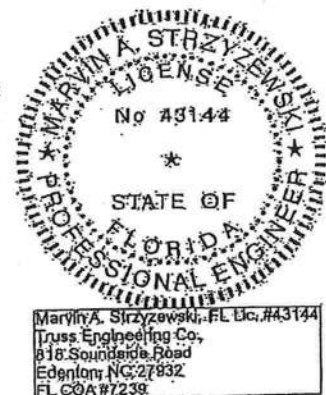
REACTIONS (lb/size) 4=582/Mechanical, 1=582/Mechanical
 Max Horz 1=424(LC 6)
 Max Uplift 4=371(LC 7), 1=104(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-6=703/10, 2-6=566/27, 2-7=127/0, 3-7=111/47, 3-4=120/212
 BOT CHORD 1-5=392/548, 4-5=392/548
 WEBS 2-5=0/324, 2-4=833/452

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.80 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 4 and 104 lb uplift at joint 1.

LOAD CASE(S) Standard

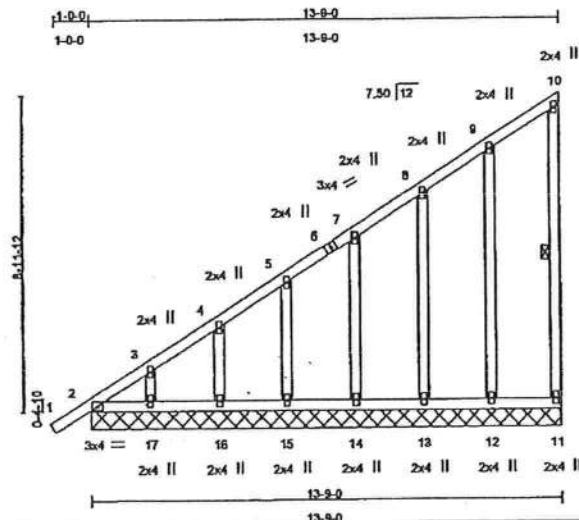


November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M1-1478 BEFORE USE.
 Design valid for use only with MITek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/ITPI Quality Criteria, D58-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 383 D'Oroffo Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A MITIK AFFILIATE
 818 Soundside Road
 Edenton, NC 27832

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510462
NANTUCKET	MIG	GABLE	3	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7,010 s Oct 16 2007 METEK Industries, Inc. Set Nov 10 09:35:54 2.007 Page 1



Scale = 1/56.0

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	Udefl	L/d	PLATES	GRIP
TCLL 15.0	Plates Increase	1.25	TC 0.10	Vert(UL)	0.00	1	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.04	Vert(TL)	-0.00	1	n/r	120		
BCLL 10.0	Rep Stress Incr	NO	WB 0.08	Horz(TL)	-0.00	11	n/e	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Matlrb)							
									Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3 "Except"
8-12 2 X 4 SYP No.2

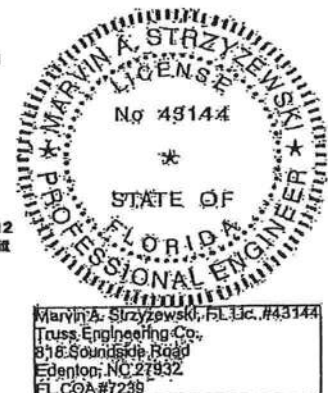
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 10-11

REACTIONS (lb/size) 11=88/13-9-0, 2=127/13-9-0, 12=283/13-9-0, 13=248/13-9-0, 14=258/13-9-0, 15=211/13-9-0, 16=170/13-9-0, 17=158/13-9-0
Max Horz 2=561 (LC 6)
Max Uplift 11=53 (LC 6), 2=35 (LC 4), 12=123 (LC 6), 13=128 (LC 6), 14=125 (LC 6), 15=122 (LC 6), 16=135 (LC 6), 17=100 (LC 6)
Max Grav 11=88 (LC 1), 2=171 (LC 6), 12=283 (LC 1), 13=248 (LC 1), 14=258 (LC 1), 15=211 (LC 1), 16=170 (LC 1), 17=158 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/24, 2-3=568/56, 3-4=482/44, 4-5=391/37, 5-6=304/25, 6-7=296/30, 7-8=216/23, 8-9=128/23, 9-10=38/18, 10-11=36/59
BOT CHORD 2-17=1/1, 16-17=1/1, 15-16=1/1, 14-15=1/1, 13-14=1/1, 12-13=1/1, 11-12=1/1
WEBS 9-12=94/153, 8-13=92/149, 7-14=92/149, 5-15=92/147, 4-16=94/155, 3-17=84/140

- NOTES**
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 2-0-0 oc.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 11, 35 lb uplift at joint 2, 123 lb uplift at joint 12, 128 lb uplift at joint 13, 125 lb uplift at joint 14, 122 lb uplift at joint 15, 135 lb uplift at joint 16 and 100 lb uplift at joint 17.

LOAD CASE(S) Standard

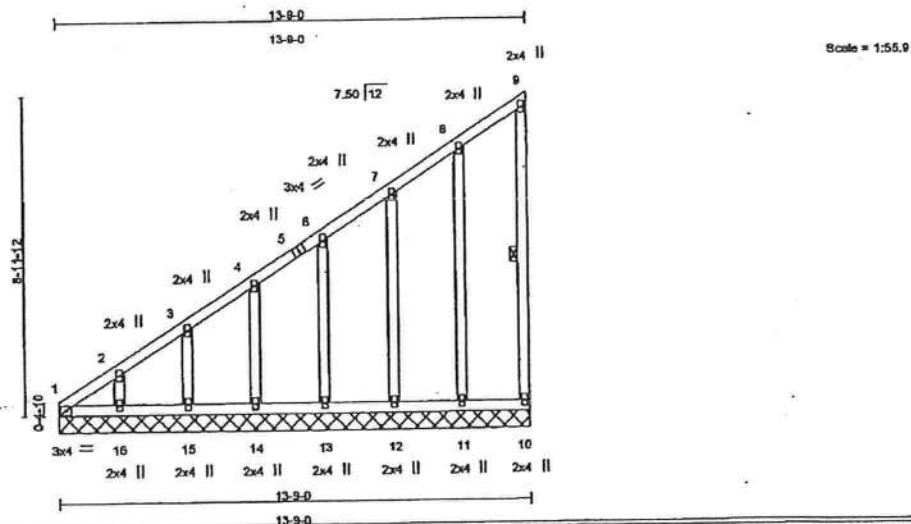


November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED METEK REFERENCE PAGE M3-T473 BEFORE USE.
Design valid for use only with Metek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-81 and IBCS1 Building Component Safety Information available from Truss Plate Institute, 383 O'Neale Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Metek Alliance
818 Soundside Road
Edenport, NC 27932

Job	Truss	Truss Type	Qty	Qty	NANTUCKET_FL_125	E4510403
NANTUCKET	MIGA	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						7.010 s Oct 16 2007 MITek Industries, Inc. Set Nov 10 09:35:54 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Wdth	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.03	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.04	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	NO	WB 0.08	Horz(TL)	-0.00	10	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Metric)						
								Weight: 93 lb	

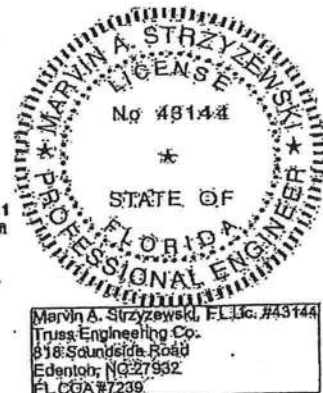
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 8-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.2	WEBS	1 Row at midpt 9-10
OTHERS	2 X 4 SYP No.3 *Except*		
	8-11 2 X 4 SYP No.2		

REACTIONS (lb/size) 10=98/13-0-0, 11=58/13-0-0, 11=283/13-0-0, 12=248/13-0-0, 13=258/13-0-0, 14=212/13-0-0, 15=165/13-0-0, 16=181/13-0-0
 Max Horz 1=510(LC 6)
 Max Uplift 10=53(LC 6), 11=10(LC 4), 11=123(LC 6), 12=128(LC 6), 13=125(LC 6), 14=125(LC 6), 15=123(LC 6), 16=151(LC 6)
 Max Grav 10=98(LC 1), 11=293(LC 6), 11=283(LC 1), 12=248(LC 1), 13=258(LC 1), 14=212(LC 1), 15=165(LC 1), 16=181(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=576/53, 2-3=480/44, 3-4=391/37, 4-5=304/25, 5-6=296/30, 6-7=216/23, 7-8=128/23, 8-9=38/18, 9-10=38/59
 BOT CHORD 1-10=-1/1, 11-16=-1/1, 14-15=-1/1, 13-14=-1/1, 12-13=-1/1, 11-12=-1/1, 10-11=-1/1
 WEBS 8-11=94/153, 7-12=82/149, 6-13=82/149, 4-14=82/148, 3-15=82/150, 2-16=84/163

- NOTES**
- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Ex B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) Gable studs spaced at 2-0-0 oc.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 10, 10 lb uplift at joint 11, 123 lb uplift at joint 11, 128 lb uplift at joint 12, 125 lb uplift at joint 13, 125 lb uplift at joint 14, 123 lb uplift at joint 15 and 151 lb uplift at joint 16.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, F.L.E. #48144
 Truss Engineering Co.
 818 Saunders Road
 Edenton, NC 27932
 FL CGA #7239

November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-1473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 O'Connell Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate
 818 Saunders Road
 Edenton, NC 27932

Marmoda Homes Inc. Sanford, Florida

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LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	Structural wood sheathing directly applied or 3-8-2 cc purtins.
TOP CHORD	
BOT CHORD	Rigid ceiling directly applied or 8-3-15 cc bracing.

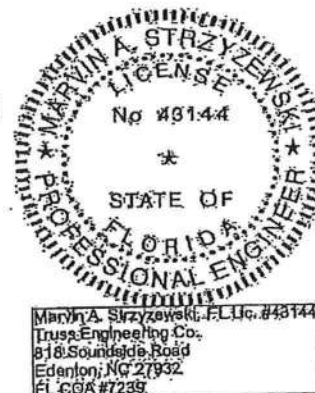
REACTIONS (lb/size) 1=1043/0-3-0, 5=1043/0-3-0
Max Horz 1=277(LC 4)
Max Uplift 1=190(LC 8), 5=190(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=2490/721, 2-3=1714/503, 3-4=1714/503, 4-5=2490/721
BOT CHORD 1-8=529/2144, 7-8=525/2135, 6-7=525/2134, 5-6=529/2144
WEBS 2-6=0/272, 2-7=687/371, 3-7=306/1479, 4-7=687/371, 4-6=0/272

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=26ft$; $TCDL=4.2psf$; $BCDL=8.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior (1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 1 and 190 lb uplift at joint 5.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED HYTEX REFERENCE PAGE HX-4473 BEFORE USE.
Design valid for use only with HYTEX connections. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not HYTEX designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/ASME Quality Criteria, D38-89 and ASCE Building Component Safety Information, available from Texas Institute of Steel, 583 O'Donoghue Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A MITek Affiliate

818 Soundside Road
Eden, NC 27932

Marmora Homes Inc., Sanford, Florida



LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
4-8 2 X 4 SYP No.2

WEDGE
Left: 2 X 6 SYP No.2, Right: 2 X 6 SYP No.2

REACTIONS (lb/size) 2=652/0-8-6, 8=977/0-8-6, 6=282/0-8-6
Max Horz 2=349(LC 5)
Max Uplift 2=307(LC 6), 8=268(LC 6), 6=219(LC 7)
Max Grav 2=652(LC 1), 8=977(LC 1), 6=294(LC 11)

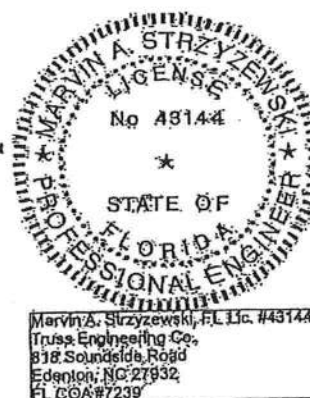
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/27, 2-11=548/248, 3-11=473/250, 3-4=261/297, 4-5=292/297, 5-12=0/106, 6-12=89/96, 6-7=0/27
 BOT CHORD 2-13=207/441, 10-13=207/441, 8-9=207/441, 8-9=35/178, 8-14=35/178, 6-14=35/178, 6-10=35/178
 WEBS 3-10=215/315, 3-9=447/719, 4-9=235/168, 5-9=131/311, 5-8=655/383

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3.6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2, 268 lb uplift at joint 8 and 219 lb uplift at joint 6.

LOAD CASE(S) Standard

BRACING	Structural wood sheathing directly applied or 6-0-0 oc purlins.
TOP CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
BOT CHORD	



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGES MII-7493 BEFORE USE.
Design valid for use only with Mittek connectors. The design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ASH/TPI Quality Criteria, DSR-BF and ICSTI Building Component Safety Information available from Truss Plate Institute, 583 O'Donofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
ALL THE ABOVE

818 Soundside Road
Edenton, NC 27932

Maranda Homes Inc., Sanford, Florida

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LOADING (psf)	
TCLL	16.0
TCDL	7.0
BCLL	10.0
BCDL	10.0

SPACING 2-0-0
Plates Increase 1.25
Lumber Increase 1.25
Rep Stress Incr YES
Code FBC2004/TPI2002

CSI	
TC	0.33
BC	0.29
WB	0.26
(Matrix)	

DEFL	in	(loc)	i/defl	Ld
Vert(LL)	-0.04	2-12	>999	240
Vert(TL)	-0.09	2-12	>999	180
Horz(TL)	0.01	10	n/a	n/a

PLATES GRIP
MT20 244/190

Weight: 145 lb

Left: 2 X 6 SYP No.2, Right 2 X 4 SYP No.2

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

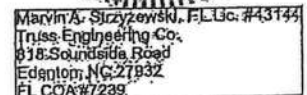
Max Uplift=313(LC 6), 10=273(LC 6), 8=133(LC 7), 8=125(LC 7)
Max Grav=640(LC 1), 10=818(LC 1), 6=163(LC 11), 9=60(LC 7), 8=178(LC 1)

TOP CHORD 1-2=0/27, 2-3=846/266, 3-4=323/297, 4-5=322/298, 5-6=94/96, 6-7=0/25
BOT CHORD 2-12=213/441, 11-12=213/441, 10-11=38/168, 9-10=38/168, 8-9=38/168, 6-8=38/168
WEBS 3-12=0/232, 3-11=371/301, 4-11=198/170, 5-11=117/328, 5-10=650/387

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable studs spaced at 2'-0" oc.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 1'-0" wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 273 lb uplift at joint 10, 133 lb uplift at joint 6 and 125 lb uplift at joint 8.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES MD-4493 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not Trus Joist design. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI Quality Criteria, D58-89 and BCSII Building Component Safety Information, available from Trus Joist Institute, 583 D'Onofrio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MTEK Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4610467
NANTUCKET	V1	VALLEY	2	1	Job Reference (optional)	
Maranda Homes Inc., Sanford, Florida						

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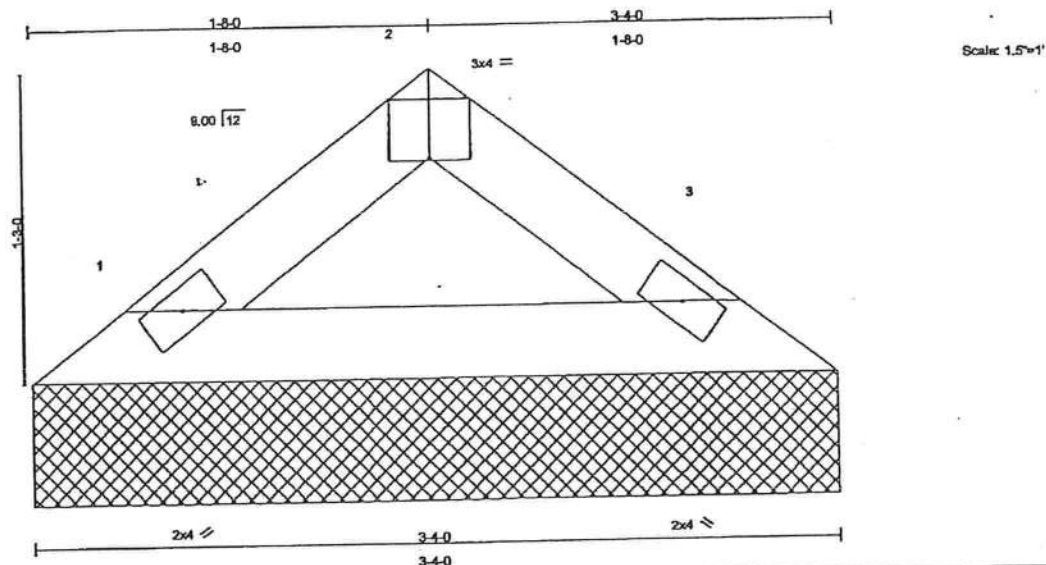


Plate Offsets (X,Y,Z) 12-0-2-0, Edge									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Wdefl	L/d	PLATES	GRIP
TCLL 10.0	Plates Increase	1.25	TC 0.03	Vert(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)						Weight: 10 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

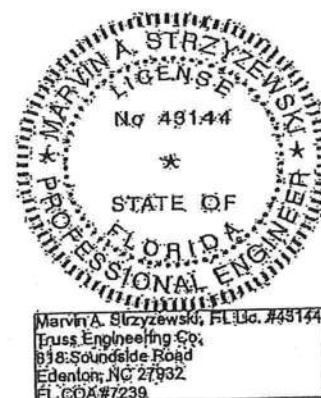
REACTIONS (lb/size) 1=106/3-4-0, 3=106/3-4-0
Max Horz 1=41(LC 4)
Max Uplift 1=48(LC 6), 3=48(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=61/61, 2-3=61/61
BOT CHORD 1-3=20/38

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 48 lb uplift at joint 3.

LOAD CASE(S) Standard

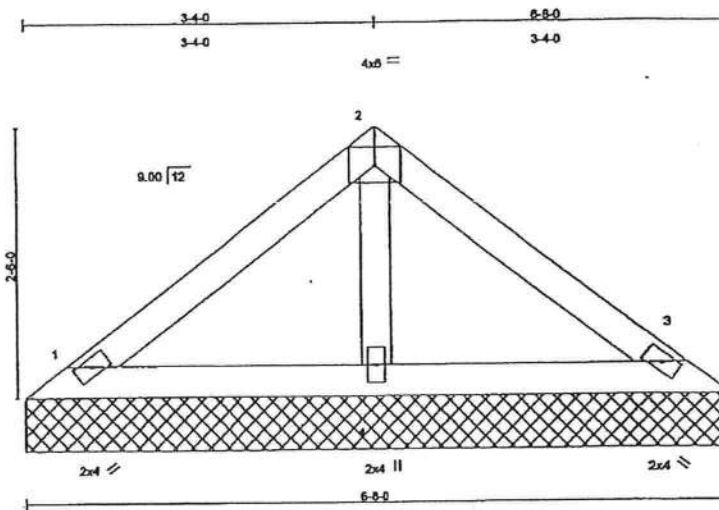


November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGE MS-1473 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-87 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MITek Alliance
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510468
NANTUCKET	V2	GABLE	2	1	Job Reference (optional)	
Maronda Homes Inc., Sanford, Florida						
7,010 s Oct 16 2007 MITek Industries, Inc. Sat Nov 10 09:35:57 2007 Page 1						



Scale = 1:18.3

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.14	Veri(LL)	n/a	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Veri(TL)	n/a	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.02	Horz(TL)	0.00	3	n/a		
BCDL 10.0	Code FBC2004/TPI2002		(Metric)						
								Weight: 24 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=126/6-8-0, 3=126/6-8-0, 4=245/6-8-0
 Max Horz 1=97(LC 5)
 Max Uplift 1=92(LC 5), 3=92(LC 5), 4=43(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-59/63, 2-3=-59/61
 BOT CHORD 1-4=-28/46, 3-4=-26/46
 WEBS 2-4=-100/65

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCCL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) automatic zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 1, 92 lb uplift at joint 3 and 43 lb uplift at joint 4.

LOAD CASE(S) Standard



Marvin A. Strzyzewski, FL Lic. #48144
 Truss Engineering Co.
 838 Soundside Road
 Edgerton, NC 27832
 FL COA #7239

November 12, 2007

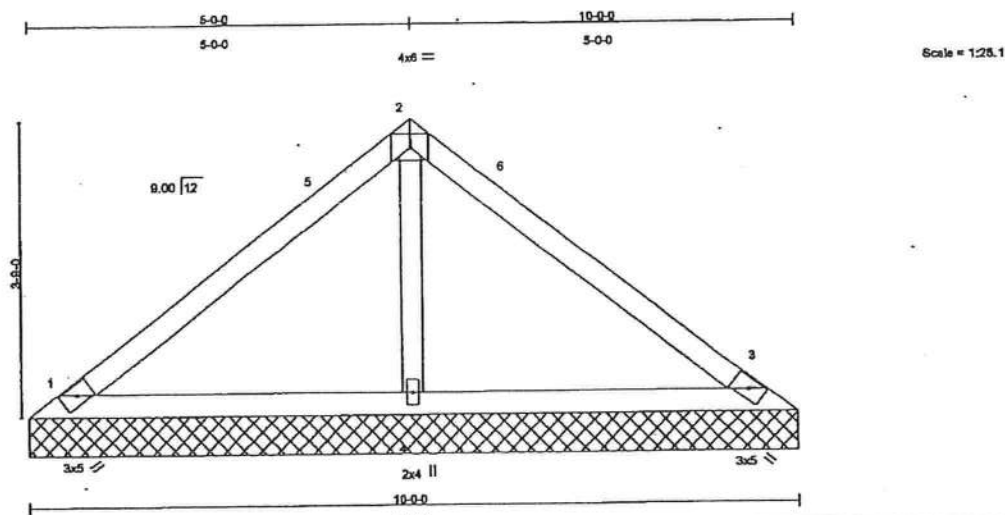
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITek REFERENCE PAGES MD-7473 BEFORE USE.
 Design valid for use only with Mittek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI1 Quality Criteria, D58-89 and ICS11 Building Component Safety Information, available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mittek Affiliate

818 Soundside Road
 Edenon, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET_FL_125	E4510469
NANTUCKET	V3	GABLE	2	1	Job Reference (optional)	
Miranda Homes Inc., Sanford, Florida						

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In	(loc)	Vdefl	Ltd	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.22	Vert(TL)	n/a	-	n/a	999		
BCLL 10.0	Rep Stress Incr	YES	WB 0.04	Horz(TL)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrbx)							
									Weight: 37 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 OTHERS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 8-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

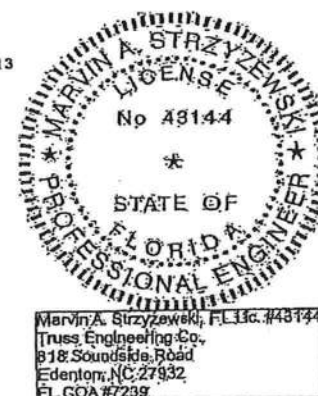
REACTIONS (lb/size) 1=182/10-0-0, 3=182/10-0-0, 4=420/10-0-0
 Max Horz 1=163(LC 4)
 Max Uplift 1=82(LC 6), 3=101(LC 7), 4=134(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-6=96/88, 2-5=41/101, 2-6=12/96, 3-6=96/83
 BOT CHORD 1-4=43/72, 3-4=43/72
 WEBS 2-4=183/162

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 92 lb uplift at joint 1, 101 lb uplift at joint 3 and 134 lb uplift at joint 4.

LOAD CASE(S) Standard



November 12, 2007



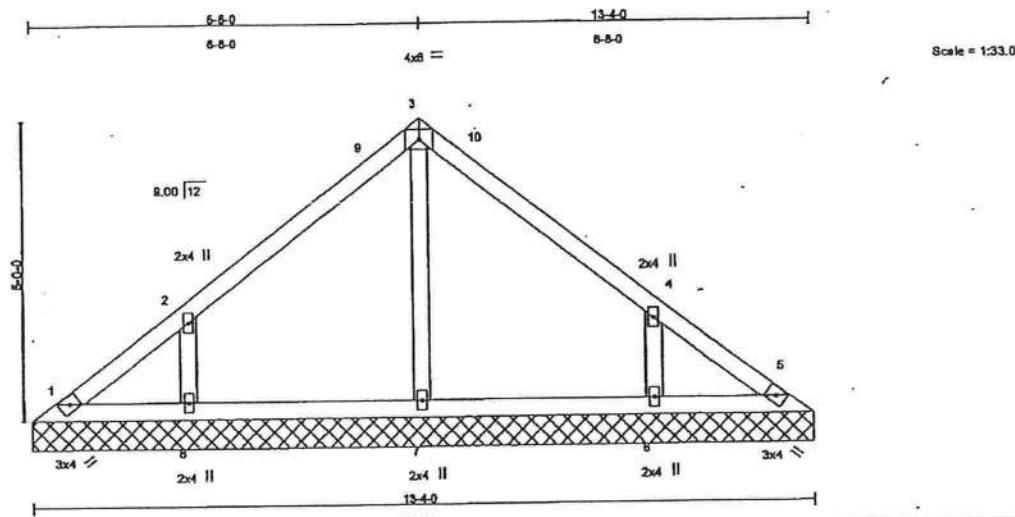
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17-7473 BEFORE USE.
 Design valid for use only with Mitek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, BSR-89 and IBCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	City	Qty	1	NANTUCKET_FL_125	E4510470
NANTUCKET	V4	GABLE	2			Job Reference (optional)	
Maronde Homes Inc., Sanford, Florida							

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Vdef	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.15	Ver(LL)	n/a	-	n/a	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Ver(TL)	n/a	-	n/a		
BCLL 10.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00	5	n/a		
BCDL 10.0	Code FBC2004/TP12002		(Matrix)					Weight: 54 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

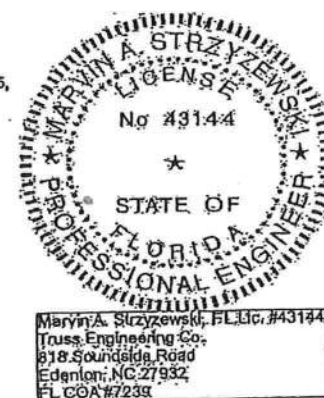
REACTIONS (lb/size) 1=82/13-4-0, 5=62/13-4-0, 7=312/13-4-0, 8=298/13-4-0, 6=298/13-4-0
Max Horz 1=209(LC 4)
Max Uplift 1=68(LC 4), 5=28(LC 5), 8=258(LC 6), 6=258(LC 7)
Max Grav 1=98(LC 5), 5=82(LC 1), 7=312(LC 1), 8=309(LC 10), 6=309(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=179/143, 2-9=95/133, 3-9=28/144, 3-10=8/144, 4-10=95/133, 4-5=112/75
BOT CHORD 1-8=52/104, 7-8=52/104, 6-7=52/104, 5-6=52/104
WEBS 3-7=143/53, 2-8=171/296, 4-6=171/295

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Gable requires continuous bottom chord bearing.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 68 lb uplift at joint 1, 28 lb uplift at joint 5, 258 lb uplift at joint 8 and 258 lb uplift at joint 6.

LOAD CASE(S) Standard



November 12, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AHS/TP1 Quality Criteria, D18-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A LUTAK ALLIATE
818 Soundside Road
Edenton, NC 27932

Memphis Homes Inc., Sanford, Florida

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Truss designed for wind loads in the plane of the truss only.
For studs exposed to wind (normal to the face), see
Standard Industry Gable End Details as applicable, or
consult qualified building designer as per ANSI/TPI 1-2002.

Weight 87 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension

WEBS 5-13=130/3, 4-15=98/146, 3-16=91/175, 2-17=99/170, 6-12=96/146, 7-11=91/176, 8-10=98/170

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 125mph (3-second gust); $h=25ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Corner(3) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 1, 20 lb uplift at joint 9, 98 lb uplift at joint 15, 148 lb uplift at joint 16, 158 lb uplift at joint 17, 97 lb uplift at joint 12, 148 lb uplift at joint 11 and 158 lb uplift at joint 10.
- 6) Non Standard bearing condition. Review required.

The seal is circular with a dashed outer border. Inside the border, the text "MARVIN A. STRZYZEWSKI" is curved along the top, "LICENSE" is curved along the bottom, and "STATE OF FLORIDA" is in the center. The number "No. 48144" is printed in the middle. There are two small stars on either side of the state name. The words "PROFESSIONAL ENGINEER" are written vertically along the right side of the seal.

Marvin A. Strzyzewski, F.L.E.C., #43144
 Trues Engineering Co.,
 818 Soundside Road
 Edenton, NC 27932
 E. CO. #228

November 12, 2007

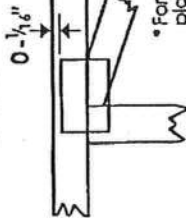
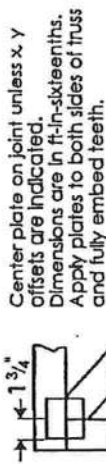
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED HITEK REFERENCE PAGE HD-7473 BEFORE USE.
Design valid for use only with Hitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not Hitek design. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/PTI Quality Criteria, D58-89 and ICCSI Building Component Safety Information available from Truss Plate Institute, 583 O'Connor Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A-11111 A-11111

818 Soundside Road
Edenboro, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

* This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

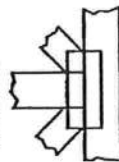
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



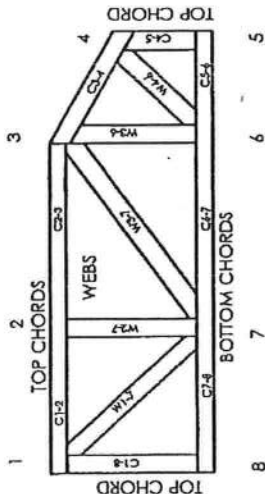
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI 1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR-5243, 96048
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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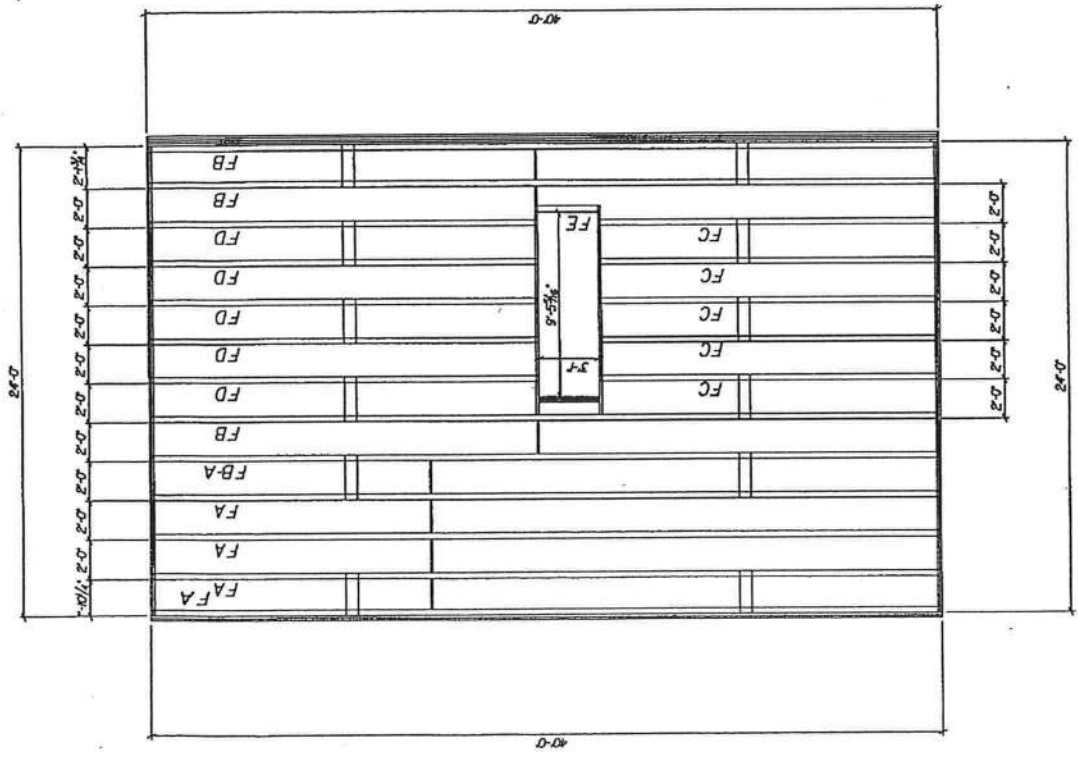


Mitek Engineering Reference Sheet: MIT-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



PERMIT FT2	HARDWARE LEGEND HARDWARE MANUFACTURED BY USP ☐ THD28-2 ☒ THD48	NANTUCKET - FL ALL ELEVATIONS GARAGE : RIGHT	DESIGNER: J. Pitcock CHECKER: K. Cooper LOADING-FBC2004/TPI2002 <table border="1"> <tr> <td>TC LIVE</td> <td>40.00</td> <td>SNOW</td> <td>LOAD</td> <td>N/A</td> </tr> <tr> <td>TC DEAD</td> <td>10.00</td> <td>LUMBER</td> <td>DOL</td> <td>1.00</td> </tr> <tr> <td>BC LIVE</td> <td>0.00</td> <td>PLATE</td> <td>DOL</td> <td>1.00</td> </tr> <tr> <td>BC DEAD</td> <td>5.00</td> <td>WIND</td> <td></td> <td>N/A</td> </tr> <tr> <td>TOTAL</td> <td>55.00</td> <td>SPACING</td> <td></td> <td>2'-0"</td> </tr> </table>	TC LIVE	40.00	SNOW	LOAD	N/A	TC DEAD	10.00	LUMBER	DOL	1.00	BC LIVE	0.00	PLATE	DOL	1.00	BC DEAD	5.00	WIND		N/A	TOTAL	55.00	SPACING		2'-0"	SCALE: 1/8" = 1'-0" DATE: 09/28/07 MTC 2000 LUMBER STATUS 1.800.848.3887 - mtc@supportlan.com
	TC LIVE	40.00	SNOW	LOAD	N/A																								
TC DEAD	10.00	LUMBER	DOL	1.00																									
BC LIVE	0.00	PLATE	DOL	1.00																									
BC DEAD	5.00	WIND		N/A																									
TOTAL	55.00	SPACING		2'-0"																									

RE: ELEV_M - NANTUCKET FL

Trenco

818 Soundside Rd
 Edenton, NC 27932

Site Information:

Project Customer: MARONDA HOMES Project Name: NANTUCKET
 Lot/Block: SANFORD Subdivision: SANFORD
 Address: SANFORD
 City: SANFORD State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
 Address:
 City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002 Design Program: MiTek 20/20 7.0
 Wind Code: N/A Wind Speed: N/A mph Floor Load: 55.0 psf
 Roof Load: N/A psf

This package includes 6 individual, dated Truss Design Drawings and 0 Additional Drawings.
 With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
 This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date
1	E4528540	FA	11/20/07
2	E4528541	FB	11/20/07
3	E4528542	FB-A	11/20/07
4	E4528543	FC	11/20/07
5	E4528544	FD	11/20/07
6	E4528545	FE	11/20/07

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Truss Design Engineer's Name: Lassiter, Frank
 My license renewal date for the state of is February 28, 2009.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Frank R. Lassiter, FL Lic #66084
 Truss Engineering Company
 818 Soundside Rd.
 Edenton, NC 27932
 FL Cert. #7239

November 20, 2007

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528540
ELEV_M	FA	FLOOR	4	1	ELEV M Job Reference (optional)	
Maronda Homes, Pittsburgh, PA						
7.020 s Nov 9 2007 MITEK Industries, Inc. Tue Nov 20 10:05:06 2007 Page 1						

0-1-8
1-1-2-0-1
1-8-0-1-1-8
1-0-4-1-8-0-1-0-12
0-1-8
Scale = 1.716

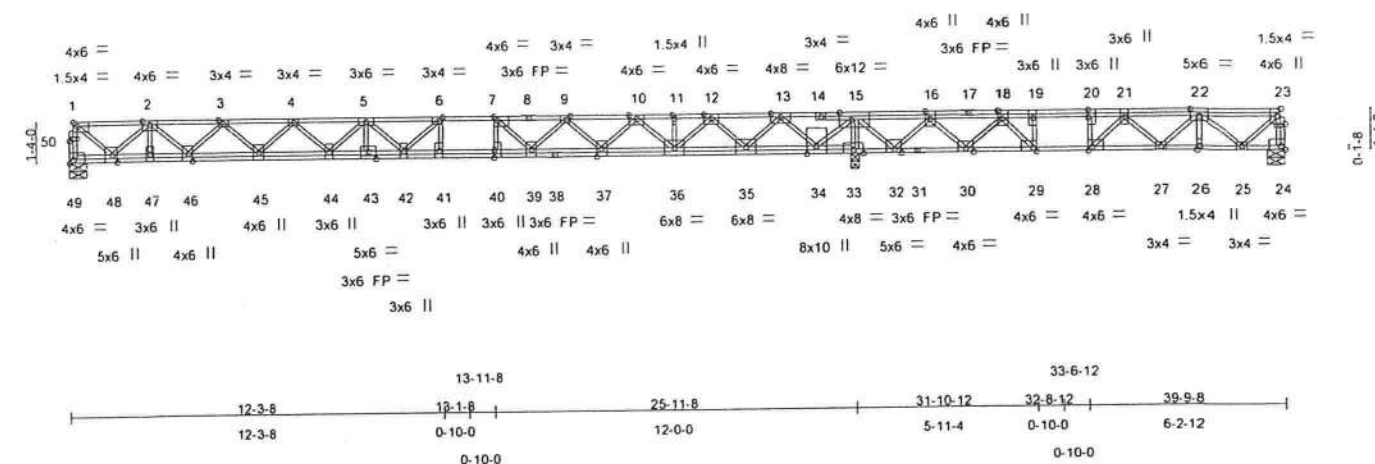


Plate Offsets (X,Y): [1:Edge,0-1-8], [3:0-1-12,Edge], [6:0-1-8,Edge], [7:0-1-8,Edge], [9:0-1-12,Edge], [15:0-4-12,Edge], [16:0-3-0,Edge], [18:0-3-0,Edge], [20:0-3-0,0-0], [22:0-3-0,Edge], [23:0-1-8,0-0-8], [23:0-3-0,Edge], [24:Edge,0-1-8], [28:0-1-8,Edge], [29:0-1-8,Edge], [32:0-2-12,Edge], [34:0-3-0,Edge], [37:0-3-0,Edge], [40:0-3-0,0-0], [43:0-3-0,Edge], [46:0-3-0,Edge], [48:0-3-0,Edge], [49:Edge,0-1-8], [50:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.82	Vert(LL)	-0.52	41-42	>595	360	MT20
TCDL 10.0	Lumber Increase	1.00	BC 0.90	Vert(TL)	-0.79	41-42	>389	240	244/190
BCLL 0.0	Rep Stress Incr	NO	WB 0.96	Horz(TL)	0.06	33	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 265 lb

LUMBER
TOP CHORD 4 X 2 SYP No.1D
BOT CHORD 4 X 2 SYP No.2 *Except*
38-49 4 X 2 SYP No.1D, 33-43 4 X 2 SYP No.1D
WEBS 4 X 2 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-3 oc bracing.

REACTIONS (lb/size) 49=1612/0-6-12, 24=776/0-6-12, 33=2808/0-3-8
Max Uplift24=-44(LC 2)
Max Grav49=1635(LC 2), 24=1341(LC 3), 33=2808(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 49-50=-1625/0, 1-50=-1623/0, 23-24=-1336/48, 1-2=-1266/0, 2-3=-3293/0, 3-4=-4659/0, 4-5=-5507/0, 5-6=-5830/0, 6-7=-5668/0, 7-8=-5045/0, 8-9=-5045/0, 9-10=-3898/0, 10-11=-2182/0, 11-12=-2182/0, 12-13=0/889, 13-14=0/3424, 14-15=0/3424, 15-16=0/3716, 16-17=-413/2762, 17-18=-413/2762, 18-19=-1330/1598, 19-20=-1330/1598, 20-21=-1330/1598, 21-22=-1298/670, 22-23=-561/211
BOT CHORD 48-49=0/89, 47-48=0/2437, 46-47=0/2437, 45-46=0/4093, 44-45=0/5198, 43-44=0/5819, 42-43=0/5819, 41-42=0/5668, 40-41=0/5668, 39-40=0/5668, 38-39=0/4581, 37-38=0/4581, 36-37=0/3172, 35-36=-244/1101, 34-35=-1826/0, 33-34=-5264/0, 32-33=-4939/0, 31-32=-3227/0, 30-31=-3227/0, 29-30=-2273/912, 28-29=-1598/1330, 27-28=-986/1456, 26-27=-435/1073, 25-26=-435/1073, 24-25=0/0
WEBS 6-41=-537/97, 7-40=-57/569, 15-33=-2967/0, 19-29=-830/0, 20-28=0/573, 1-48=0/1609, 2-48=-1602/0, 2-47=0/18, 2-46=0/1171, 3-46=-1120/0, 3-45=0/794, 4-45=-755/0, 4-44=0/433, 5-44=-427/0, 5-43=-50/40, 5-42=-199/245, 6-42=-322/673, 15-34=0/2404, 13-34=-2240/0, 13-35=0/1778, 12-35=-1731/0, 12-36=0/1511, 11-36=-74/0, 10-36=-1388/0, 10-37=0/1051, 9-37=-986/0, 9-39=0/784, 7-39=-1178/0, 15-32=0/1794, 16-32=-1387/0, 16-30=0/1055, 18-30=-1098/0, 18-29=0/1405, 23-25=-282/750, 22-25=-700/307, 22-26=-39/0, 22-27=-321/307, 21-27=-222/443, 21-28=-1021/0

NOTES

- Unbalanced floor live loads have been considered for this design.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 24.
- Load case(s) 3 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.
- Required 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 426 lb down at 0-2-4, and 736 lb down at 39-8-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

This document was originally issued by Lassiter, Frank on November 20, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.
November 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and 8CS11 Building Component Safety Information - available from Truss Plate Institute, 583 D'Oncofrio Drive, Madison, WI 53719.

TRENCO
ENGINEERING BY
A Mitek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528540
ELEV_M	FA	FLOOR	4	1	ELEV M Job Reference (optional)	

Maronda Homes, Pittsburgh, PA

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LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 24-49=-10, 1-23=-100

Concentrated Loads (lb)

Vert: 1=-426(F) 23=-426(F)

3) 2nd unbalanced Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 24-49=-10, 1-15=-20, 15-23=-100

Concentrated Loads (lb)

Vert: 1=-116(F) 23=-736(F)

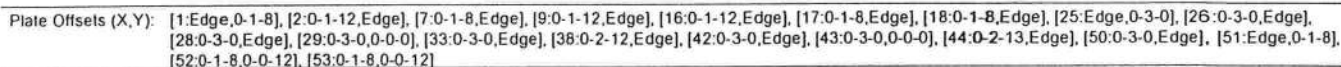


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

7.020 5 Nov 9 2007 MiTek Industries, Inc. Tue Nov 20 10:05:07 2007 Page 1Continued on page 2

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

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Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528541
ELEV_M	FB	FLOOR	3	1	ELEV M Job Reference (optional)	

Maronda Homes, Pittsburgh, PA

7.020 s Nov 9 2007 MiTek Industries, Inc. Tue Nov 20 10:05:08 2007 Page 2

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 25-51=-10, 1-24=-100

Concentrated Loads (lb)

Vert: 1=-426(F) 24=-426(F)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528542
ELEV_M	FB-A	FLOOR	1	1	ELEV M Job Reference (optional)	
Maronda Homes, Pittsburgh, PA						
7.020 s Nov 9 2007 Mitek Industries, Inc. Tue Nov 20 10:05:09 2007 Page 1						

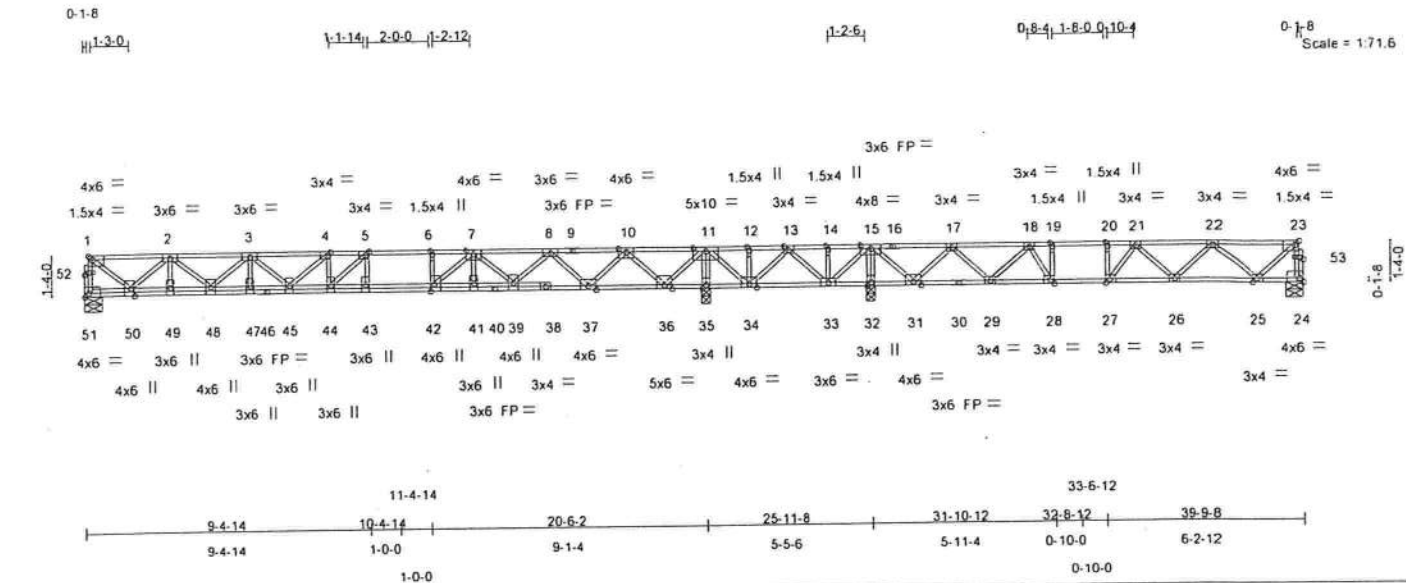


Plate Offsets (X,Y): [1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,Edge], [13:0-1-12,Edge], [23:0-1-8,Edge], [24:Edge,0-1-8], [25:0-1-8,Edge], [27:0-1-8,Edge], [28:0-1-8,Edge], [42:0-3-0,Edge], [50:0-3-0,Edge], [51:Edge,0-1-8], [52:0-1-8-0-0-12], [53:0-1-8-0-0-12]						
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl
TCLL 40.0	Plates Increase	1.00	TC 0.83	Vert(LL)	-0.25 43-44	>971 360
TCDL 10.0	Lumber Increase	1.00	BC 0.86	Vert(TL)	-0.39 43-44	>623 240
BCLL 0.0	Rep Stress Incr	NO	WB 0.70	Horz(TL)	0.03 24	n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)			
				PLATES	GRIP	
				MT20	244/190	
				Weight: 234 lb		

LUMBER		BRACING	
TOP CHORD	4 X 2 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	4 X 2 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
WEBS	4 X 2 SYP No.3		6-0-0 oc bracing: 36-37,35-36,34-35,33-34,32-33,31-32.

REACTIONS (lb/size)		51=1365/0-6-12, 24=1080/0-6-8, 32=725/0-3-8, 35=2019/0-3-8
Max Uplift		32=-75(LC 2)
Max Grav		51=1368(LC 4), 24=1084(LC 4), 32=1220(LC 3), 35=2201(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	51-52=-1358/0, 1-52=-1357/0, 24-53=-1078/0, 23-53=-1077/0, 1-2=-1019/0, 2-3=-2528/0, 3-4=-3341/0, 4-5=-3620/0, 5-6=-3408/0, 6-7=-3408/0, 7-8=-2131/0, 8-9=-541/0, 9-10=-541/0, 10-11=0/1600, 11-12=0/2326, 12-13=0/2326, 13-14=0/1406, 14-15=0/1406, 15-16=-3/261, 16-17=-3/261, 17-18=-995/0, 18-19=-1622/0, 19-20=-1622/0, 20-21=-1622/0, 21-22=-1455/0, 22-23=-652/0
BOT CHORD	50-51=0/74, 49-50=0/1956, 48-49=0/1956, 47-48=0/3104, 46-47=0/3104, 45-46=0/3104, 44-45=0/3620, 43-44=0/3408, 42-43=0/3408, 41-42=0/2781, 40-41=0/2781, 39-40=0/1456, 38-39=0/1456, 37-38=0/1456, 36-37=-427/0, 35-36=-2905/0, 34-35=-2905/0, 33-34=-1838/0, 32-33=-1070/0, 31-32=-1069/0, 30-31=0/571, 29-30=0/571, 28-29=0/1421, 27-28=0/1622, 26-27=0/1655, 25-26=0/1204, 24-25=0/56
WEBS	15-32=-1198/78, 11-35=-2164/0, 19-28=-337/0, 20-27=-127/75, 5-43=-352/37, 6-42=-307/0, 1-50=0/1255, 2-50=-1244/0, 2-49=0/23, 2-48=0/760, 3-48=-765/0, 3-47=0/40, 3-45=0/314, 4-45=-371/0, 4-44=-121/185, 5-44=-274/552, 11-36=0/1740, 10-36=-1632/0, 10-37=0/1268, 8-37=-1278/0, 8-39=0/920, 7-39=-866/0, 7-41=-185/46, 7-42=0/1048, 11-34=0/1205, 12-34=-136/26, 13-34=-996/3, 13-33=-210/787, 14-33=-135/19, 15-33=-598/388, 15-31=0/1086, 17-31=-1003/0, 17-29=0/607, 18-29=-613/0, 18-28=0/523, 23-25=0/810, 22-25=-769/0, 22-26=0/348, 21-26=-279/0, 21-27=-180/189

- NOTES**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) One RT7 USP connectors recommended to connect truss to bearing walls due to uplift at jt(s) 32.
 - 3) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 4) CAUTION, Do not erect truss backwards.
 - 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 426 lb down at 0-2-4, and 426 lb down at 39-7-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

This document was originally issued by Lassiter, Frank on November 20, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above. November 20, 2007

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528542
ELEV_M	FB-A	FLOOR	1	1	ELEV M Job Reference (optional)	

Maronda Homes, Pittsburgh, PA

7.020 s Nov 9 2007 Mitek Industries, Inc Tue Nov 20 10:05:10 2007 Page 2

LOAD CASE(S) Standard

1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 24-51=-10, 1-23=-100

Concentrated Loads (lb)

Vert: 1=-426(F) 23=-426(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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ENGINEERING BY
TRENCO
 A Mitek Affiliate

818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4528543
ELEV_M	FC	FLOOR	5	1	ELEV M Job Reference (optional)	

Maronda Homes, Pittsburgh, PA

7.020 s Nov 9 2007 Mitek Industries, Inc. Tue Nov 20 10:05:10 2007 Page 1

0-1-8

1-3-0

0-11-8 2-0-0 1-0-12

0-11-8

Scale = 1:30.2

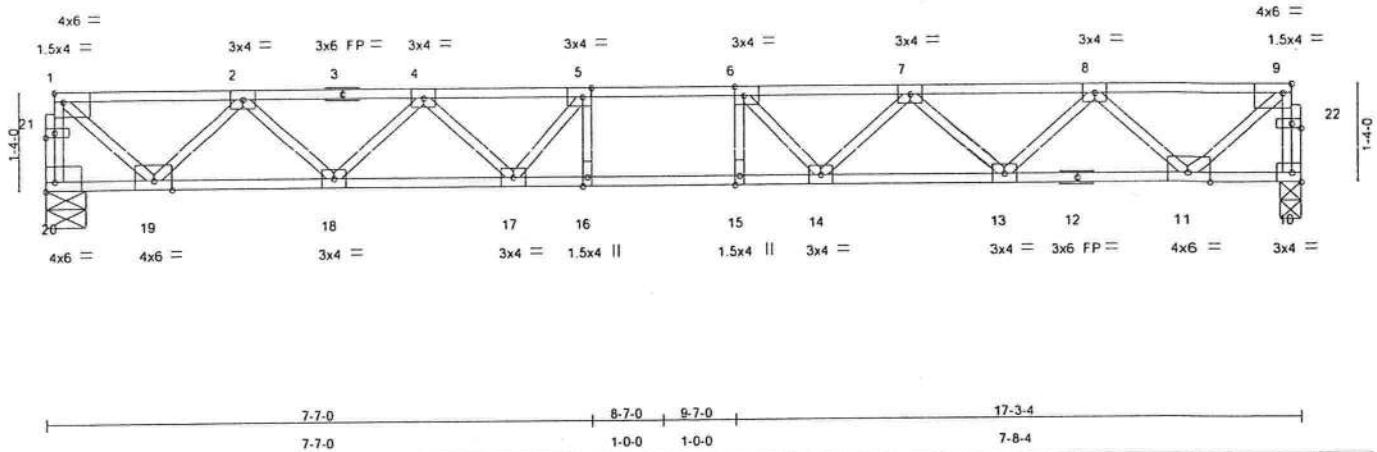


Plate Offsets (X,Y): [1:Edge 0-1-8], [5:0-1-8 Edge], [6:0-1-8 Edge], [9:0-1-8 Edge], [20:Edge 0-1-8], [21:0-1-8,0-0-12], [22:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.49	Vert(LL)	-0.18 15-16	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.82	Vert(TL)	-0.28 15-16	>730	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.49	Horz(TL)	0.05 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
								Weight: 89 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2 *Except*
12-20 4 X 2 SYP No.1D
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 20=1354/0-6-12, 10=931/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 20-21=-1349/0, 1-21=-1348/0, 10-22=-926/0, 9-22=-925/0, 1-2=-959/0, 2-3=-2313/0, 3-4=-2313/0, 4-5=-3075/0, 5-6=-3279/0, 6-7=-3066/0, 7-8=-2302/0, 8-9=-941/0
BOT CHORD 19-20=0/70, 18-19=0/1786, 17-18=0/2813, 16-17=0/3279, 15-16=0/3279, 14-15=0/3279, 13-14=0/2807, 12-13=0/1770, 11-12=0/1770, 10-11=0/48
WEBS 1-19=0/1209, 2-19=-1150/0, 2-18=0/734, 4-18=-695/0, 4-17=0/468, 5-17=-557/36, 9-11=0/1214, 8-11=-1153/0, 8-13=0/740, 7-13=-703/0, 7-14=0/458, 6-14=-549/30, 5-16=-176/216, 6-15=-175/195

NOTES

- Unbalanced floor live loads have been considered for this design.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 426 lb down at 0-2-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 10-20=-10, 1-9=-100
Concentrated Loads (lb)
Vert: 1=-426(F)

This document was originally issued by Lassiter, Frank on November 20,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.
November 20,2007



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	NANTUCKET FL	E4526544
ELEV_M	FD	FLOOR	5	1	ELEV M Job Reference (optional)	
Maronda Homes, Pittsburgh, PA						

7 020 s Nov 9 2007 Mitek Industries, Inc. Tue Nov 20 10:05:11 2007 Page 1

0-1-8

1-3-0

1-2-8 2-0-0 1-2-12

0-1-8

Scale = 1:34.7

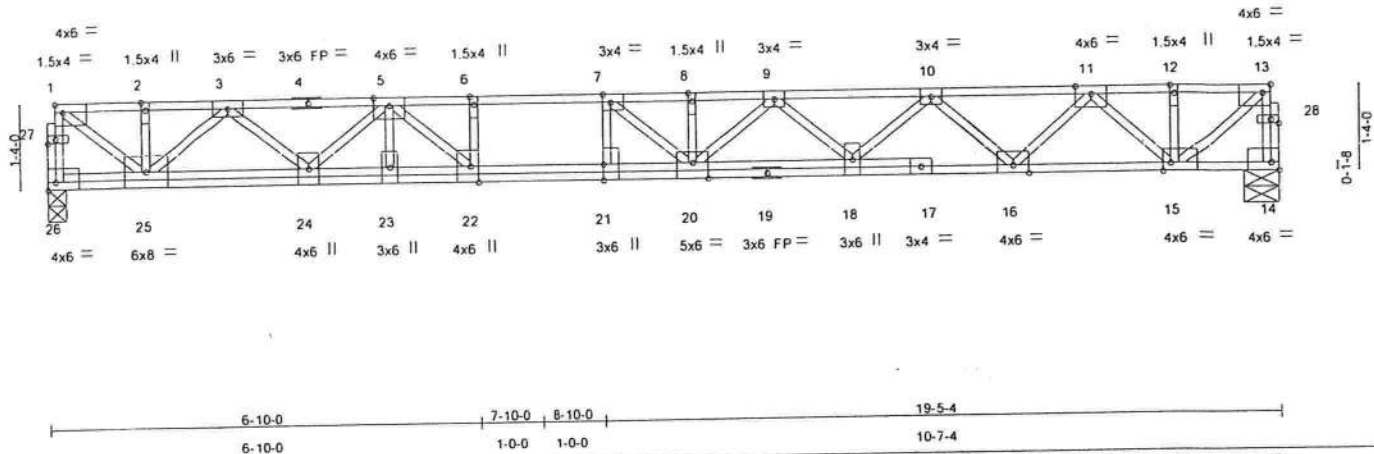


Plate Offsets (X,Y): [1:Edge,0-1-8], [7:0-1-8,Edge], [13:0-1-8,Edge], [14:Edge,0-1-8], [15:0-1-8,Edge], [20:0-3-0,Edge], [21:0-3-0,0-0-0], [22:0-3-0,Edge], [26:Edge,0-1-8], [27:0-1-8,0-0-12], [28:0-1-8,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.80	Vert(LL)	-0.30 20-21	>758	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.77	Vert(TL)	-0.47 20-21	>486	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.61	Horz(TL)	0.05 14	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 123 lb

LUMBER
TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.1D
WEBS 4 X 2 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 26=1050/0-3-8, 14=1474/0-6-12

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 26-27=-1042/0, 1-27=-1040/0, 14-28=-1474/0, 13-28=-1472/0, 1-2=-1228/0, 2-3=-1228/0, 3-4=-2877/0, 4-5=-2877/0, 5-6=-4210/0, 6-7=-4210/0, 7-8=-4444/0, 8-9=-4444/0, 9-10=-3888/0, 10-11=-2731/0, 11-12=-1196/0, 12-13=-1196/0
BOT CHORD 25-26=0/57, 24-25=0/2202, 23-24=0/3528, 22-23=0/3528, 21-22=0/4210, 20-21=0/4210, 19-20=0/4271, 18-19=0/4271, 17-18=0/3435, 16-17=0/3435, 15-16=0/2088, 14-15=0/76
WEBS 6-22=-308/0, 7-21=-396/32, 1-25=0/1524, 2-25=-114/0, 3-25=-1293/0, 3-24=0/916, 5-24=-864/0, 5-23=-236/29, 5-22=0/1138, 13-15=0/1491, 12-15=-124/0, 11-15=-1213/0, 11-16=0/894, 10-16=-980/0, 10-18=0/614, 9-18=-519/0, 9-20=0/296, 8-20=-188/21, 7-20=-297/581

NOTES

- Unbalanced floor live loads have been considered for this design.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 426 lb down at 19-3-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Floor: Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 14-26=-10, 1-13=-100
Concentrated Loads (lb)
Vert: 13=-426(F)

This document was originally issued by Lassiter, Frank on November 20, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above.
November 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Orazio Drive, Madison, WI 53719.

ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Job ELEV_M	Truss FE	Truss Type FLOOR	Qty 1	Ply 1	NANTUCKET FL ELEV M Job Reference (optional)	E4528545
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Maranda Homes, Pittsburgh, PA

7.020 s Nov 9 2007 Mitek Industries, Inc. Tue Nov 20 10:05:11 2007 Page 1

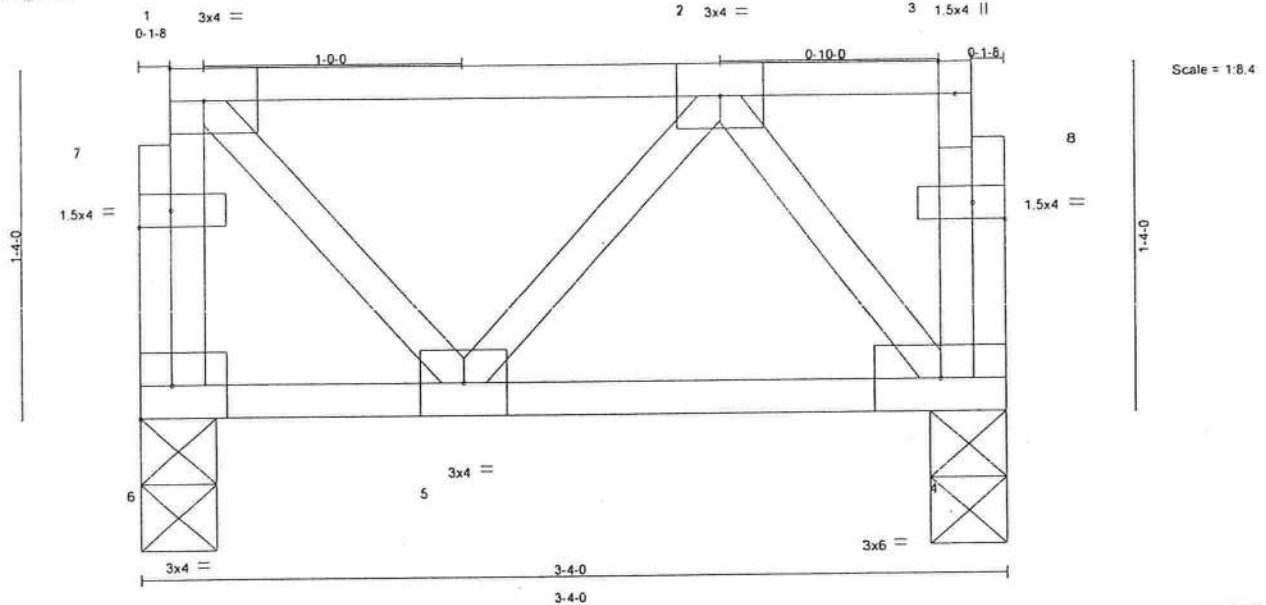


Plate Offsets (X,Y): [7'-0"-1'-8" 0'-0"-12"], [8'-0"-1'-8" 0'-0"-12"]

LOADING (psf)	SPACING	2'-0"-0"	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.22	Vert(LL)	-0.00	5	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.05	Vert(TL)	-0.00	4-5	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.04	Horz(TL)	0.00	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 22 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3'-4"-0" oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0"-0" oc bracing.

REACTIONS (lb/size) 6=163/0-3-8, 4=163/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 6-7=-160/0, 1-7=-160/0, 4-8=-1/0, 3-8=-1/0, 1-2=-75/0, 2-3=-0/0
BOT CHORD 5-6=0/8, 4-5=0/122
WEBS 1-5=0/101, 2-5=-73/0, 2-4=-196/0

NOTES

1) Recommend 2x6 strongbacks, on edge, spaced at 10'-0"-0" oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 20, 2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated above. November 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-7473 BEFORE USE.
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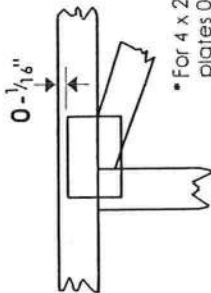
ENGINEERING BY
TRENCO
A Mitek Affiliate

818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



• For 4 x 2 orientation, locate plates 0-¹/₁₆" from outside edge of truss.

• This symbol indicates the required direction of slots in connector plates.



• Plate location details available in **MiTek 20/20** software or upon request.

PLATE SIZE

4 X 4

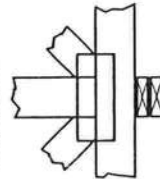
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



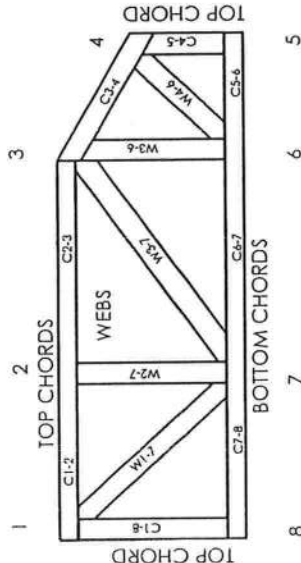
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing.
- BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8
dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

- ESR-1311, ESR-1352, ER-5243, 9604B
- 9730, 95-43, 96-31, 9667A
- NER-487, NER-561
- 95110, 84-32, 96-67, ER-3907, 9432A

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MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 08-0522

----- PART II - SITE PLAN -----

Scale: Each block represents 5 feet and 1 inch = 50 feet.

See
Attached

Notes: _____

Site Plan submitted by: Patrick Wilson Signature

Abert
Title

Plan Approved ☒ Not Approved ☐

Date 8-1-08

By Mr D Zander Columbia County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

CLIENT MARONDA HOMES DATE 17 Sept 08
JOB# 9TMO1201
PROJECT NAME TIMBER LANDS Lot #12 PROJECT NO. 27292
EARTH CONTRACTOR 361 SW Timber Ridge Dr. PERMIT NO. 27292
TESTED BY JWC
COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor
☒ Modified Proctor FIELD CONTACT _____
TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☒ Building Footing ☐ Other

- * Density failed to meet minimum project requirement
- ** Retest indicates minimum density requirement was obtained.
- () Client is aware of unsatisfactory test results.

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-112

Building permit No. 000027292

Use Classification SFD/UTILITY

Fire: 64.20

Permit Holder THEODORE C. BROCK

Waste: 167.50

Owner of Building MARONDA HOMES INC. OF FLORIDA

Total: 231.70

Location: 361 SW TIMBER RIDGE DR, LAKE CITY, FL 32024

Date: 12/09/2008

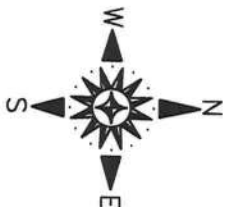
Wayne H. Russ

Building Inspector

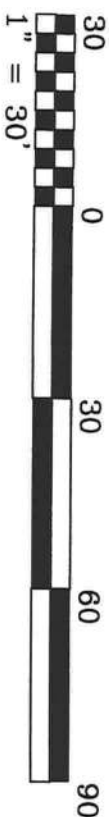
POST IN A CONSPICUOUS PLACE
(Business Places Only)



LEGAL DESCRIPTION:
LOT TWELVE (12) OF 'TIMBERLANDS, PHASE 1' AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR 'TIMBERLANDS' IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

TITLE NOTE:

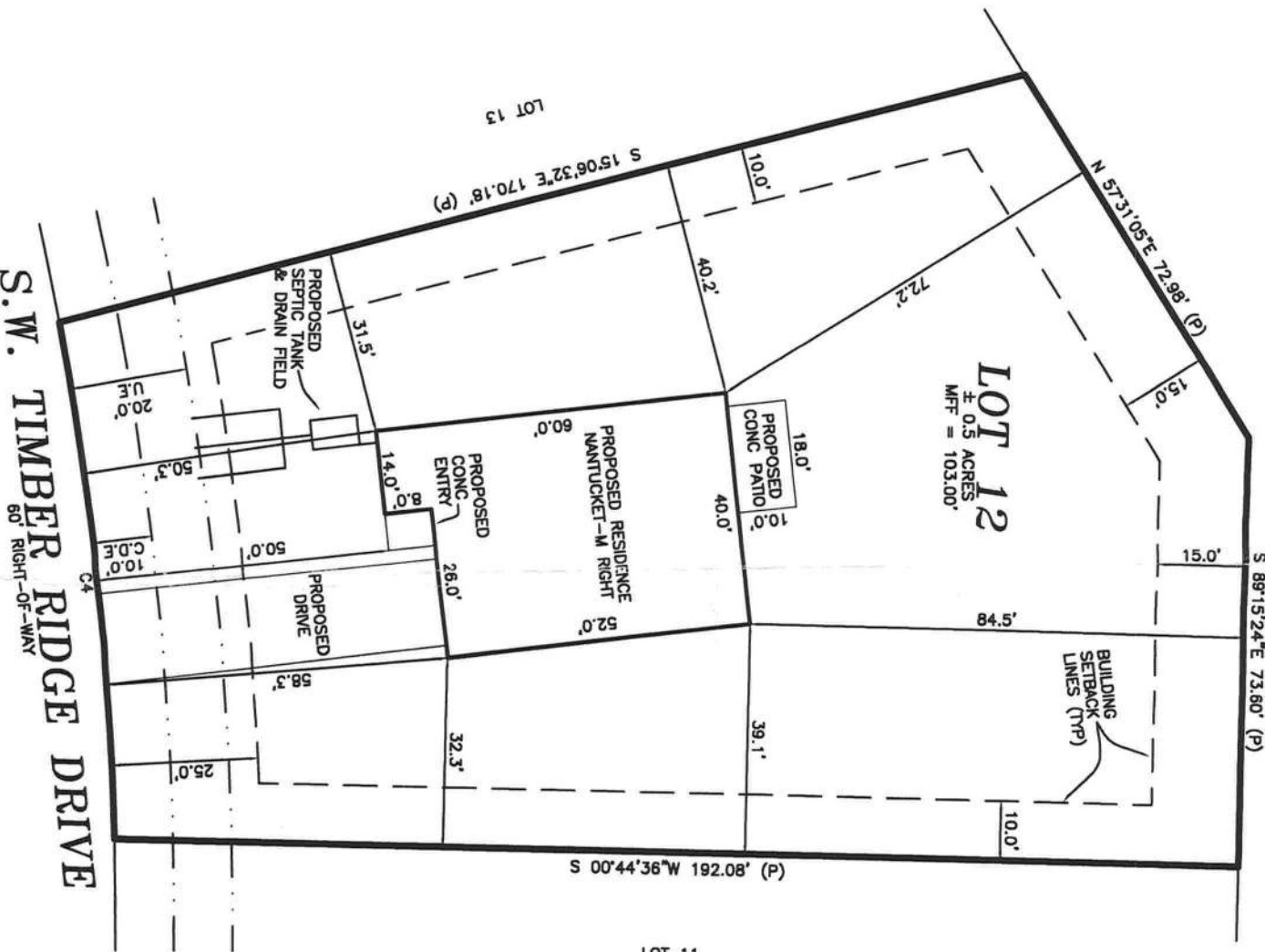
THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-8, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE 'X' WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN DEPICTED HEREON.



S.W. TIMBER RIDGE DRIVE
60' RIGHT-OF-WAY

CURVE TABLE:

CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C4	560.00'	44.60'	89.02'	9°06'28.7"	88.93'	S 83°14'18" W

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
 - = FOUND 1/2" REBAR & CAP L.B. 6894
 - = SET 1/2" REBAR & CAP L.B. 6894
 - = FOUND 3/4" IRON PIPE
 - = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
 - = SET 4" X 4" CONC. MON. P.S.M. 5582
 - ✕ = SET NAIL & DISK P.S.M. 5582
 - ✕ = FOUND NAIL & DISK
 - ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
 - ⊠ = CATV RISER
 - ⊠ = TELEPHONE PEDESTAL
 - ⊠ = WOOD POWER POLE
- ABBREVIATIONS:
- A/C = AIR CONDITIONER
 - ASPH = ASPHALT
 - C = CALCULATED FROM MEASURED
 - CATV = CABLE TELEVISION
 - C/B = CONCRETE BLOCK
 - CLF = CHAIN LINK FENCE
 - CM = CONCRETE MONUMENT
 - CONC = CONCRETE
 - ELEC = ELECTRIC
 - ELEV = ELEVATION
 - FND = FOUND
 - FNC = FENCE
 - LB = LICENSED SURVEYOR BUSINESS
 - (M) = FIELD MEASURED
 - MFT = MINIMUM FINISHED FLOOR
 - MH = MANHOLE
 - O.U. = OVERHEAD UTILITIES
 - P = PLAT
 - P.B. = PLAT BOOK
 - P.U.E. = PUBLIC UTILITIES EASEMENT
 - TRANS = TRANSFORMER
 - TYP = TYPICAL
 - WM = WATER METER
 - WV = WATER VALVE

THIS IS NOT A BOUNDARY SURVEY
CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

JAMES E. BRINKMAN, PSM - FLA. CERT# 5582
DATE: 6/12/08



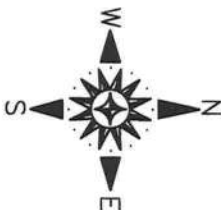
BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'
DATE: 6/12/08
FIELD WORK COMPLETED ON ****
FIELDBOOK **, PAGE **
DRAWN BY: ZL
CHECKED BY: J.B.

PREPARED FOR: MARONDA
DRAWING NUMBER 125-08

LEGAL DESCRIPTION:
LOT TWELVE (12) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

- 1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

TITLE NOTE:

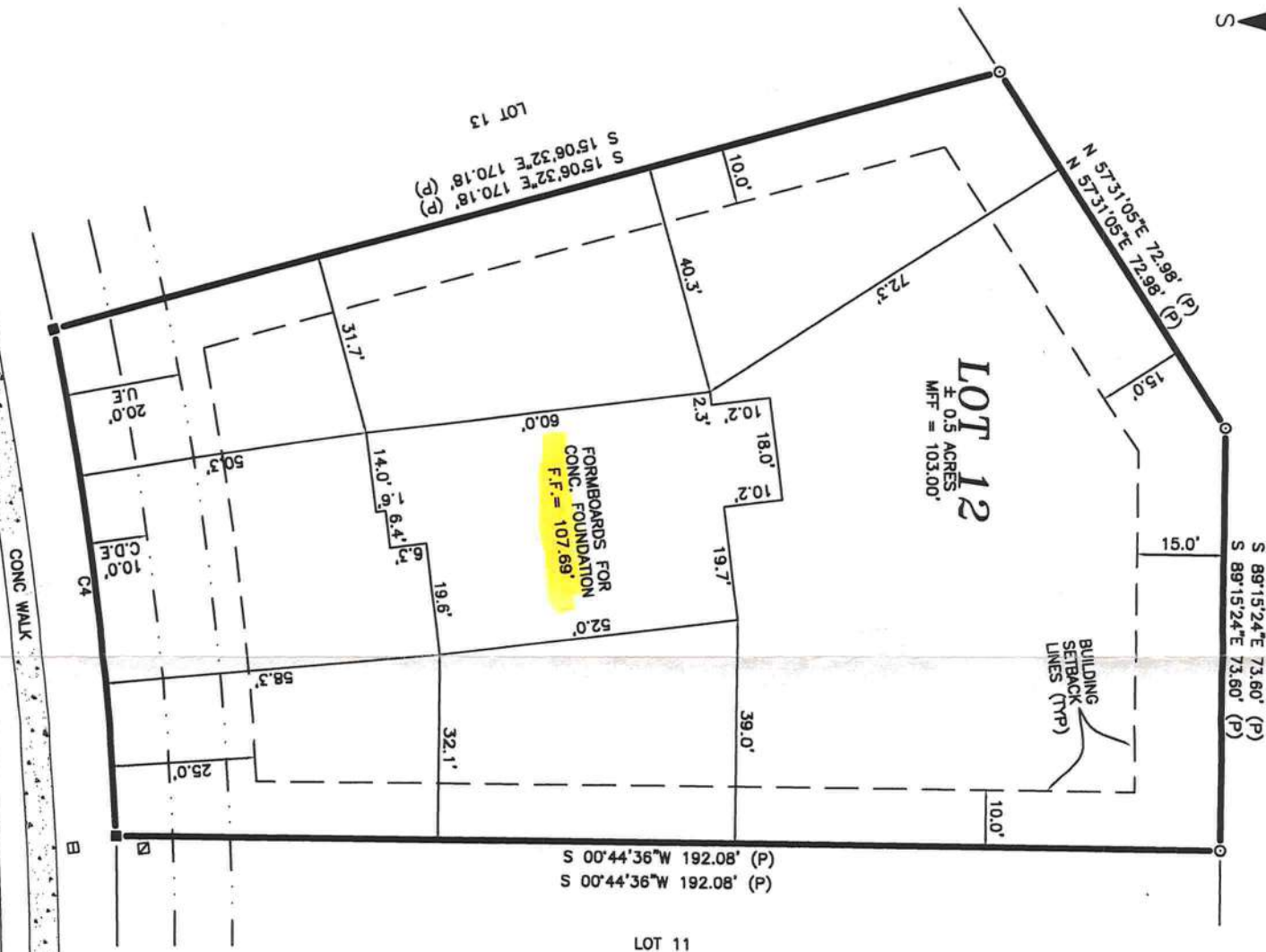
THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-98, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



S.W. TIMBER RIDGE DRIVE
EDGE OF PAVEMENT
60' RIGHT-OF-WAY
±20' ASPHALT ROAD

Permit #27292

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- X = SET NAIL & DISK P.S.M. 5582
- X = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊞ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE

ABBREVIATIONS:

- A/C = AIR CONDITIONER
- ASPH = ASPHALT
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CONC = CONCRETE
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MFT = MINIMUM FINISHED FLOOR
- MH = MANHOLE
- O.U. = OVERHEAD UTILITIES
- P = PLAT
- P.B. = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- TRANS = TRANSFORMER
- TYP = TYPICAL
- WM = WATER METER
- WV = WATER VALVE

CURVE TABLE:

CURVE	RADIUS	TANGENT	LENGTH	DELTA	CHORD	CHORD BEARING
C4	580.00'	44.60'	89.02'	9°06'28.7"	88.93'	S 83°14'18" W

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 63617-6, FLORIDA ADMINISTRATIVE CODE.

BY: JAMES E. BRINKMAN, PSM - FLA. CERT# 5582
DATE: 9/15/08



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'
DATE: 9/15/2008
FIELD WORK COMPLETED ON 9/12/08
FIELDBOOK 98, PAGE 53

PREPARED FOR: MARONDA
DRAWN BY: J.D.M.
CHECKED BY: J.B.
DRAWING NUMBER 125-08

FEEES:

ROAD IMPACT FEE	<u>\$1,046.00</u>	CODE	<u>218</u>	UNIT	<u>1</u>
10100003632400					
EMS IMPACT FEE	<u>\$29.88</u>				
10300003632210					
FIRE PROTECTION IMPACT FEE	<u>\$78.63</u>				
10200003632220					
CORRECTIONS IMPACT FEE	<u>\$407.16</u>				
00100003632200					
SCHOOL IMPACT FEE	<u>\$1,500.00</u>				
00100003632900					

TOTAL FEES CHARGED \$3,063.67 **CHECK NUMBER** 927063