



**AAMA/NWWDA 101/L.S.2-97
TEST REPORT SUMMARY**

Rendered to:

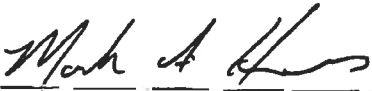
MI HOME PRODUCTS, INC.

**SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft ²
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

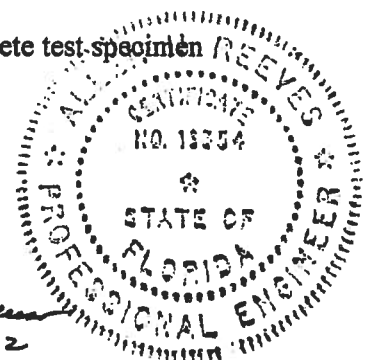
Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.


Mark A. Hess, Technician

MAH:nlb


1 APRIL 2002





Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

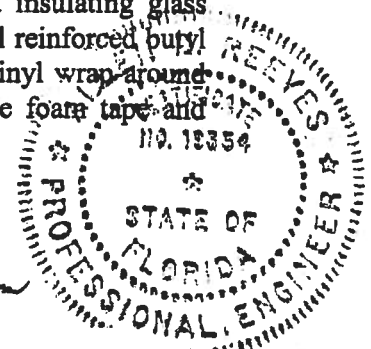
Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002



Architectural Testing

AAMA/NWWDA 101/L.S.2-97 TEST REPORT

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MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
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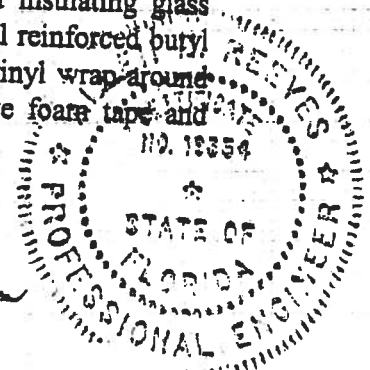
Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

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Allen M. Ream
1 APRIL 2002



Test Specimen Description: (Continued)**Weatherstripping:**

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

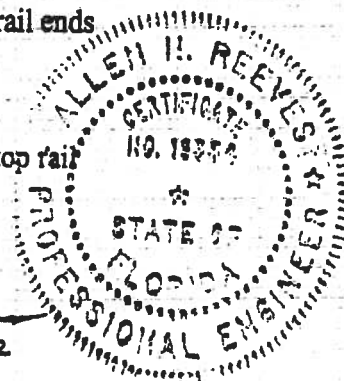
Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

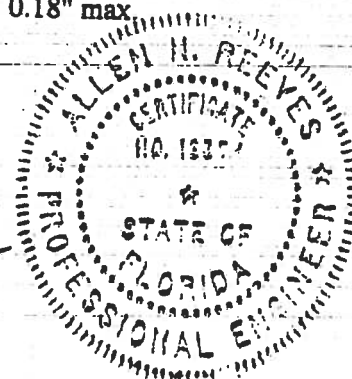
The results are tabulated as follows:

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft ²	0.3 cfm/ft ² max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0.42"* 0.43"*	0.26" max. 0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0.02" 0.02"	0.18" max. 0.18" max.
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Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

<u>Paragraph</u>	<u>Title of Test - Test Method</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A		
	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

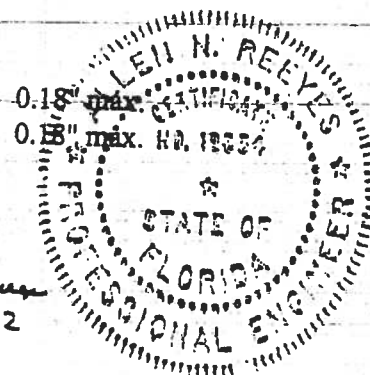
Optional Performance

4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"	0.26" max.
	@ 47.2 psf (negative)	0.46"	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

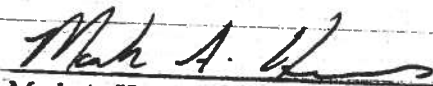
Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)	
@ 67.5 psf (positive)	0.05"
@ 70.8 psf (negative)	0.05"

Allen N. Reeves
1 APRIL 2002



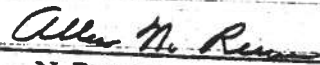
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:



Mark A. Hess
Technician

MAH:mlb
01-41134.01



Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002





FEB - 4 2002

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

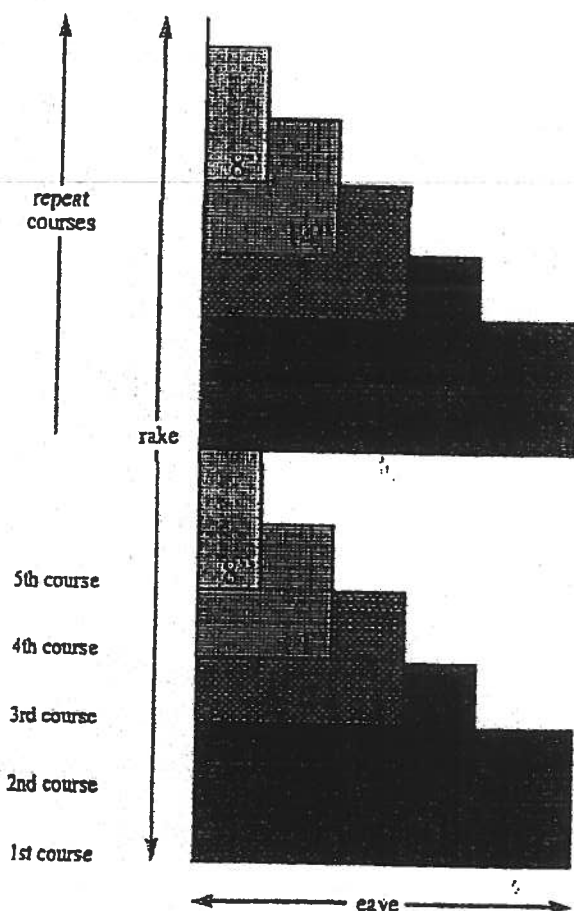
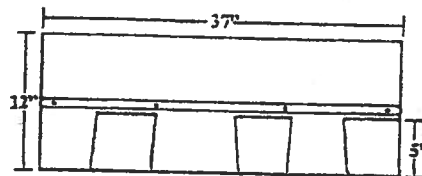
Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

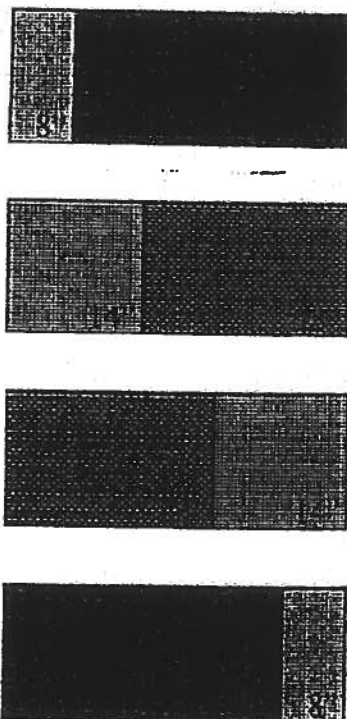


Application Instructions For Heritage® 25 Series Shingles

SPECIFICATIONS (APPROX.)	
Length	37"
Width	12"
Bundles per Sq.	3
Shingles per Sq.	78
Shingles per Bundle	26
Coverage per Sq. (Sq. Ft.)	100
Exposure	5"



The 4 cuts in the first 10 courses:



In the first 10 courses, there are 4 cuts and no waste.

When you reach the other side of the roof, whatever has to be trimmed off can be used in the field of roofing.

For additional application information consult the application instructions printed on the product package.

NOTE: These application instructions apply only to Heritage 25 and Heritage 25 AR shingles.

TAMKO

ROOFING PRODUCTS

Application Instructions for

- Glass-Seal
 - Glass-Seal AR
 - Elite Glass-Seal®
 - Elite Glass-Seal® AR
- ### THREE-TAB ASPHALT SHINGLES

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS. THIS PRODUCT IS COVERED BY A LIMITED WARRANTY, THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER. IN COLD WEATHER (BELOW 40°F), CARE MUST BE TAKEN TO AVOID DAMAGE TO THE EDGES AND CORNERS OF THE SHINGLES.

IMPORTANT: It is not necessary to remove the plastic strip from the back of the shingles.

1. ROOF DECK

These shingles are for application to roof decks capable of receiving and retaining fasteners, and to inclines of not less than 2 in. per foot. For roofs having pitches 2 in. per foot to less than 4 in. per foot, refer to special instructions titled "Low Slope Application". Shingles must be applied properly. TAMKO assumes no responsibility for leaks or defects resulting from improper application, or failure to properly prepare the surface to be roofed over.

NEW ROOF DECK CONSTRUCTION: Roof deck must be smooth, dry and free from warped surfaces. It is recommended that metal drip edges be installed at eaves and rakes.

PLYWOOD: All plywood shall be exterior grade as defined by the American Plywood Association. Plywood shall be a minimum of 3/8 in. thickness and applied in accordance with the recommendations of the American Plywood Association.

SHEATHING BOARDS: Boards shall be well-seasoned tongue-and-groove boards and not over 6 in. nominal width. Boards shall be a 1 in. nominal minimum thickness. Boards shall be properly spaced and nailed.

2. VENTILATION

Inadequate ventilation of attic spaces can cause accumulation of moisture in winter months and a build up of heat in the summer. These conditions can lead to:

1. Vapor Condensation
2. Buckling of shingles due to deck movement.
3. Rotting of wood members.
4. Premature failure of roof.

To insure adequate ventilation and circulation of air, place louvers of sufficient size high in the gable ends and/or install continuous ridge and soffit vents.

FHA minimum property standards require one square foot of net free ventilation area to each 150 square feet of space to be vented, or one square foot per 300 square feet if a vapor barrier is installed on the warm side of the ceiling or if at least one half of the ventilation is provided near the ridge. If the ventilation openings are screened, the total area should be doubled.

IT IS PARTICULARLY IMPORTANT TO PROVIDE ADEQUATE VENTILATION.

3. FASTENING

NAILS: TAMKO recommends the use of nails as the preferred method of application.

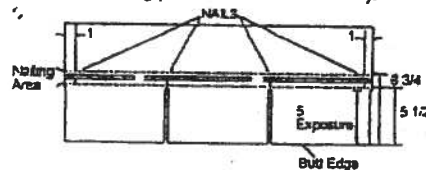
WIND CAUTION: Extreme wind velocities can damage these shingles after application when proper sealing of the shingles does not occur. This can especially be a problem if the shingles are applied in cooler months or in areas on the roof that do not receive direct sunlight. These

conditions may impede the sealing of the adhesive strips on the shingles. The inability to seal down may be compounded by prolonged cold weather conditions and/or blowing dust. In these situations, hand sealing of the shingles is recommended. Shingles must also be fastened according to the fastening instructions described below.

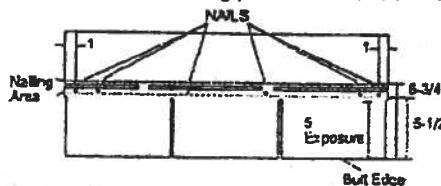
Correct placement of the fasteners is critical to the performance of the shingle. If the fasteners are not placed as shown in the diagram and described below, TAMKO will not be responsible for any shingles blown off or displaced. TAMKO will not be responsible for damage to shingles caused by winds or gusts exceeding gale force. Gale force shall be the standard as defined by the U.S. Weather Bureau.

FASTENING PATTERNS: Fasteners must be placed above or below the factory applied sealant in an area between 5-1/2" and 6-3/4" from the butt edge of the shingle. Fasteners should be located horizontally according to the diagram below. Do not nail into the sealant. TAMKO recommends nailing below the sealant whenever possible for greater wind resistance.

1) Standard Fastening Pattern. (For use on decks with slopes 2 in. per foot to 21 in. per foot.) One fastener 1 in. back from each end and one 12 in. back from each end of the shingle for a total of 4 fasteners. (See standard fastening pattern illustrated below).



2) Mansard or High Wind Fastening Pattern. (For use on decks with slopes greater than 21 in. per foot.) One fastener 1 in. back from each end and one fastener 10-1/2 in. back from each end and one fastener 13-1/2 in. back from each end for a total of 6 fasteners per shingle. (See Mansard fastening pattern illustrated below.)



NAILS: TAMKO recommends the use of nails as the preferred method of application. Standard type roofing nails should be used. Nail shanks should be made of minimum 12-gauge wire, and a minimum head diameter of 3/8 in. Nails should be long enough to penetrate 3/4 in.

(Continued)

Visit Our Web Site at
www.tamko.com

Central District	220 West 4th St., Joplin, MO 64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD 21701	800-368-2055
Southeast District	2300 35th St., Tuscaloosa, AL 35401	800-229-2656
Southwest District	7910 S. Central Exp., Dallas, TX 75216	800-443-1834
Western District	5300 East 43rd Ave., Denver, CO 80216	800-530-8868

07/01

TAMKO

ROOFING PRODUCTS

(CONTINUED from Pg. 2)

• Glass-Seal
• Glass-Seal AR

• Elite Glass-Seal®
• Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

with quick setting asphalt adhesive cement immediately upon installation. Spots of cement must be equivalent in size to a \$.25 piece and applied to shingles with a 5 in. exposure, use 6 fasteners per shingle. See Section 3 for the Mansard Fastening Pattern.

8. RE-ROOFING

Before re-roofing, be certain to inspect the roof decks. All plywood shall meet the requirements listed in Section 1.

Nail down or remove curled or broken shingles from the existing roof. Replace all missing shingles with new ones to provide a smooth base. Shingles that are buckled usually indicate warped decking or protruding nails. Hammer down all protruding nails or remove them and refasten in a new location. Remove all drip edge metal and replace with new.

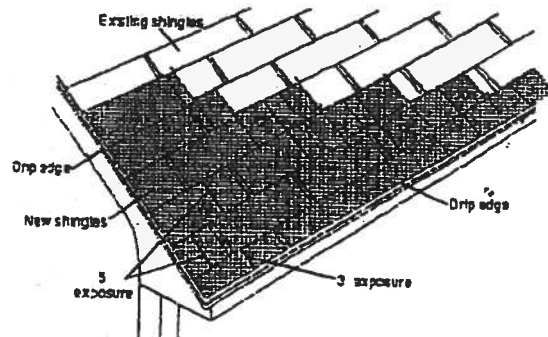
If re-roofing over an existing roof where new flashing is required to protect against ice dams (freeze/thaw cycle of water and/or the backup of water in frozen or clogged gutters), remove the old roofing to a point at least 24 in. beyond the interior wall line and apply TAMKO's Moisture Guard Plus® waterproofing underlayment. Contact TAMKO's Technical Services Department for more information.

The nesting procedure described below is the preferred method for re-roofing over square tab strip shingles with a 5 in. exposure.

Starter Course: Begin by using TAMKO Shingle Starter or by cutting shingles into 5 x 36 inch strips. This is done by removing the 5 in. tabs from the bottom and approximately 2 in. from the top of the shingles so that the remaining portion is the same width as the exposure of the old shingles. Apply the starter piece so that the self-sealing adhesive lies along the eaves and is even with the existing roof. The starter strip should be wide enough to overhang the eaves and carry water into the gutter. Remove 3 in. from the length of the first starter shingle to ensure that the joints from the old roof do not align with the new.

First Course: Cut off approximately 2 in. from the bottom edge of the shingles so that the shingles fit beneath the existing third course and align with the edge of the starter strip. Start the first course with a full 36 in. long shingle and fasten according to the instructions printed in Section 3.

Second and Succeeding Courses: According to the off-set application method you choose to use, remove the appropriate length from the



rake end of the first shingle in each succeeding course. Place the top edge of the new shingle against the butt edge of the old shingles in the courses above. The full width shingle used on the second course will reduce the exposure of the first course to 3 in. The remaining courses will automatically have a 5 in. exposure.

9. VALLEY APPLICATION

Over the shingle underlayment, center a 36 in. wide sheet of TAMKO Nail-Fast® or a minimum 50-lb. roll roofing in the valley. Nail the fast only where necessary to hold it in place and then only nail the outside edges.

IMPORTANT: PRIOR TO INSTALLATION WARM SHINGLES TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES TO FORM VALLEY.

- Apply the first course of shingles along the eaves of one of the intersecting roof planes and across the valley.

Note: For proper flow of water over the trimmed shingle, always start applying the shingles on the roof plane that has the lower slope or less height.

- Extend the end shingle at least 12 in. onto the adjoining roof. Apply succeeding courses in the same manner, extending them across the valley and onto the adjoining roof.
- Do not trim if the shingle length exceeds 12 in. Lengths should vary.
- Press the shingles tightly into the valley.
- Use normal shingle fastening methods.

Note: No fastener should be within 5 in. of the valley centerline, and two fasteners should be placed at the end of each shingle crossing the valley.

- To the adjoining roof plane, apply one row of shingles extending it over previously applied shingles and trim a minimum of 2 in. back from the centerline of the valley.

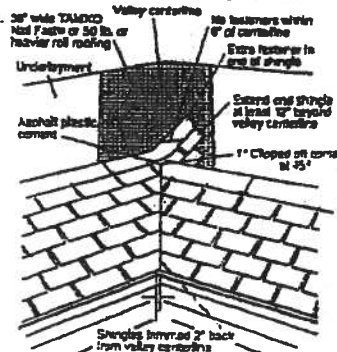
Note: For a neater installation, snap a chalkline over the shingles for guidance.

- Clip the upper corner of each shingle at a 45-degree angle and embed the end of the shingle in a 3 in. wide strip of asphalt plastic cement. This will prevent water from penetrating between the courses by directing it into the valley.

CAUTION:
Adhesive must be applied in smooth, thin, even layers.

Excessive use of adhesive will cause blistering to this product.

TAMKO assumes no responsibility for blistering.



(Continued)

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07/01



(CONTINUED from Pg. 3)

- Glass-Seal
- Glass-Seal AR

- Elite Glass-Seal®
- Elite Glass-Seal® AR

THREE-TAB ASPHALT SHINGLES

FOR ALTERNATE VALLEY APPLICATION METHODS, PLEASE CONTACT TAMKO'S TECHNICAL SERVICES DEPARTMENT.

10. HIP AND RIDGE FASTENING DETAIL

Apply the shingles with a 5 in. exposure beginning at the bottom of the hip or from the end of the ridge opposite the direction of the prevailing winds. Secure each shingle with one fastener 5-1/2 in. back from the exposed end and 1 in. up from the edge. Do not nail directly into the sealant.

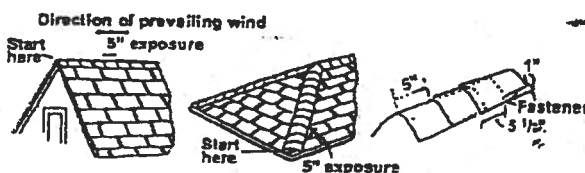
TAMKO recommends the use of TAMKO Hip & Ridge shingle products. Where matching colors are available, it is acceptable to use TAMKO's Glass-Seal or Elite Glass-Seal shingles cut down to 12 in. pieces.

NOTE: AR type shingle products should be used as Hip & Ridge on Glass-Seal AR and Elite Glass-Seal AR shingles.

Fasteners should be 1/4 in. longer than the one used for shingles.

IMPORTANT: PRIOR TO INSTALLATION, CARE NEEDS TO BE TAKEN TO PREVENT DAMAGE WHICH CAN OCCUR WHILE BENDING SHINGLES IN COOL WEATHER.

THESE ARE THE MANUFACTURER'S APPLICATION INSTRUCTIONS FOR THE ROOFING CONDITIONS DESCRIBED. TAMKO ROOFING PRODUCTS, INC. ASSUMES NO RESPONSIBILITY FOR LEAKS OR OTHER ROOFING DEFECTS RESULTING FROM FAILURE TO FOLLOW THE MANUFACTURER'S INSTRUCTIONS.



THIS PRODUCT IS COVERED BY A LIMITED WARRANTY. THE TERMS OF WHICH ARE PRINTED ON THE WRAPPER.

IMPORTANT - READ CAREFULLY BEFORE OPENING BUNDLE

In this paragraph "You" and "Your" refer to the installer of the shingles and the owner of the building on which these shingles will be installed. This is a legally binding agreement between You and TAMKO Roofing Products, Inc. ("TAMKO"). By opening this bundle You agree: (a) to install the shingles strictly in accordance with the instructions printed on this wrapper; or (b) that shingles which are not installed strictly in accordance with the instructions printed on this wrapper are sold "AS IS" and are not covered by the limited warranty that is also printed on this wrapper, or any other warranty, including, but not limited to (except where prohibited by law) implied warranties of MERCHANTABILITY and FITNESS FOR USE.

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800-228-2656
800-443-1834
800-530-8868

07/01

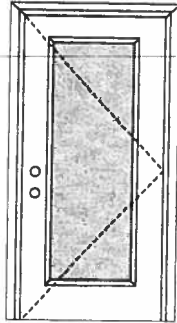
X

Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure

+40.5/-40.5

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:



100 Series



133, 135 Series



136 Series



680 Series

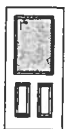


822 Series

1/2 GLASS:



105 Series*



106, 160 Series*



129 Series*



200 Series*



12 R/L, 23 R/L, 24 R/L Series*



107 Series*



108 Series



304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson
EntrySystems

June 17, 2002
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PREMDOR Collection
Premium Quality Doors



Exclusively from

Masonite

Masonite International Corporation

Y
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES: 3/4 GLASS:



404 Series



410 Series

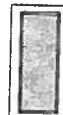


450 Series

FULL GLASS:



109 Series



114, 120, 122
Series



152 Series



149 Series



300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthaz

State of Florida, Professional Engineer
Kurt Balthazor, P.E. - License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

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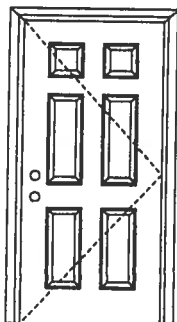
Masonite

Masonite International Corporation

X

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS**APPROVED ARRANGEMENT:****Note:**

Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.itswh.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure**+66.0/-66.0**

Limited water unless special threshold design is used.

Large Missile Impact Resistance

Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

Flush



Arch Top 3-panel



3-panel



6-panel



New England 4-panel



Eyebrow 4-panel



8-panel



9-panel



15-panel



5-panel



5-panel with scroll



Eyebrow 5-panel



Eyebrow 5-panel with scroll

Johnson
EntrySystems

June 17, 2002
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Exclusively from

Masonite
Masonite International Corporation

Y

Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).



State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etssemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

2

Johnson
EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.



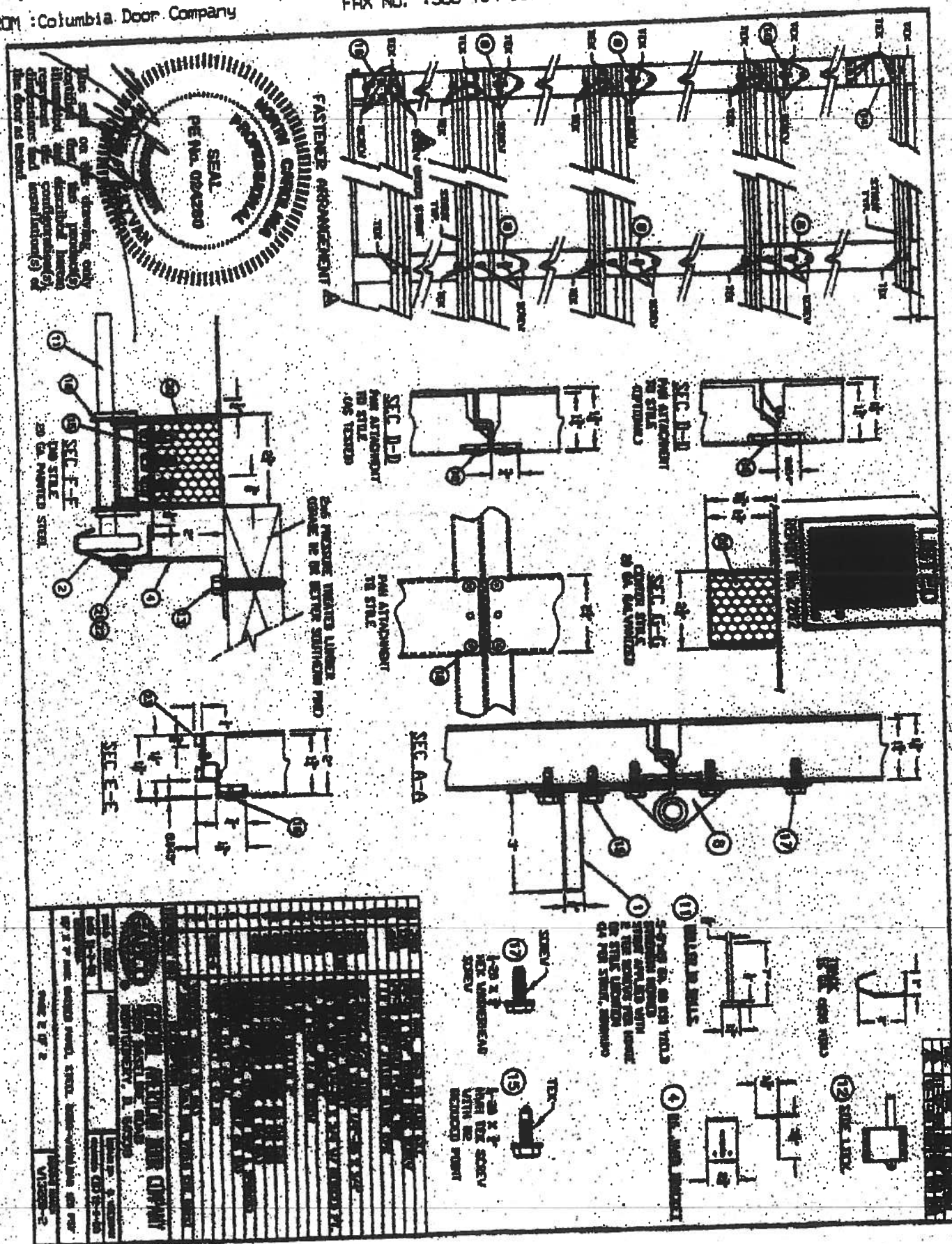
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Masonite International Corporation



FAX NO. : 386-754-9993





FLORIDA BUILDING CODE

- Overview
- User Registration
- User Authentication
- Organization Search
- Organization Authentication

Select the organization type, status, or name to find an organization

Organization Type: Product Manufacturer

Approval Status: (ALL)

Organization Name: General American Door - Product Manufacturer

Search

Cancel

Result List for Organizations

Displaying 1-1 of 1

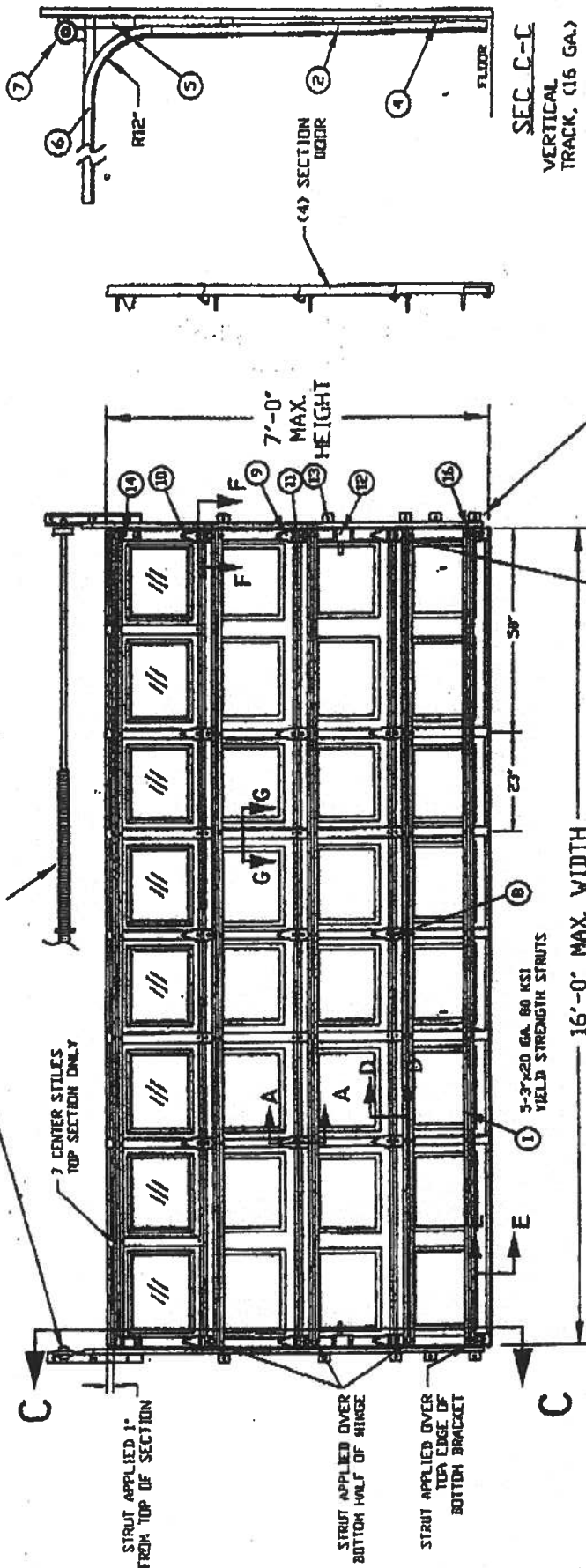
Name	City	Contact	Phone	Type	Expiry	Status
General American Door	Montgomery	James Campbell	6808591000	Product Manufacturer	01/01/2099	Approved
Org Codes PDM	System ID: 3305	Site Link: www.gadco.com				

Displaying 1-1 of 1

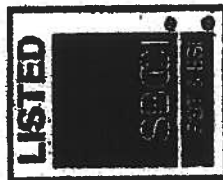
NOTES:

1. TESTED TO POSITIVE AND NEGATIVE 20 PSF DESIGN AND POSITIVE AND NEGATIVE 30 PSF TEST PRESSURES PER ASTM E-330
2. MAXIMUM SECTION HEIGHT: 21'
3. SECTION HEIGHTS OF 21.0' AND 19.5' ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS REAR HEIGHTS.
4. VARIOUS MAY BE INSTALLED IN THE TOP SECTION, (AS TESTED WITH 1/8" RIB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.
5. MINIMUM LENGTH OF ROLLER STICH IS 3/4" (7' AS TESTED)
6. THE STRUT PLACEMENT ON DOOR MUST BE CONSISTENT WITH THE DOOR DRAWING.
7. STRUTS SECURED AT ALL LOCATIONS WITH TEK SCREWS.
8. QUANTITY OF SIDE LOADS CAN BE Q1, OR Q2 AS TESTED.
9. DROP IN TYPE OF INSULATION IS OPTIONAL.

NUT PART OF WIND LOAD SYSTEM
EXTENSION SPRING COUNTERBALANCE
TORSION SPRING COUNTERBALANCE



INSIDE ELEVATION



The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configuration(s), dimensions and installation(s) of the door as tested.

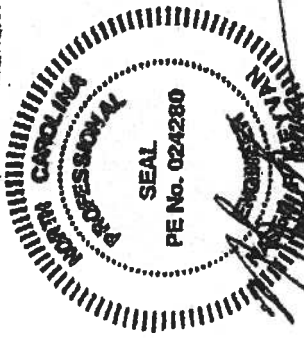
TEST REPORTS ON FILE VIDEO 10/19/00 0002933

DESIGN LOAD +20.0 PSF & -20.0 PSF
TEST LOAD +30.0 PSF & -30.0 PSF

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

GABCO DOORS			
SERIES 7400, EXTERIOR STEEL -017 MIN GAS TESTED			
SERIES 7825, EXTERIOR STEEL -019 MIN A			
SERIES 7524, EXTERIOR STEEL -024 MIN A			
TESTED WITH WINDINGS			
MAXIMUM DOOR WIDTH	MAXIMUM DOOR HEIGHT	TYPICAL CTR. STILE SPACING	VERTICAL TRACK
16'	7'	23"	5 2 IN.

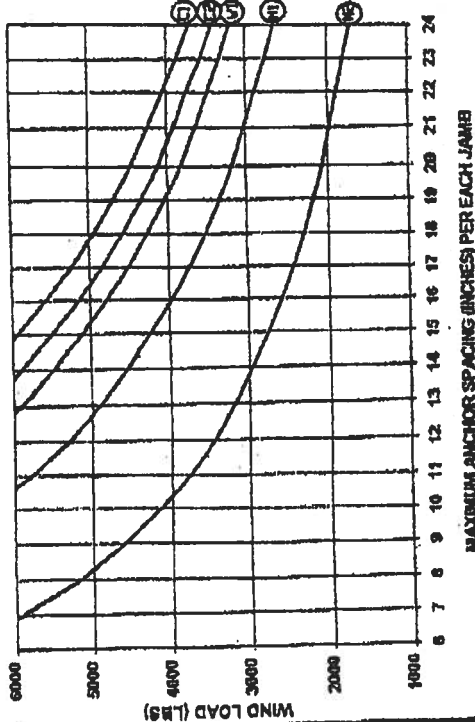
DATE	REVISION	BY	DATE
10-10-00	(A)	JL	10-10-00
10-10-00	(B)	JL	10-10-00
10-10-00	(C)	JL	10-10-00
10-10-00	(D)	JL	10-10-00
10-10-00	(E)	JL	10-10-00
10-10-00	(F)	JL	10-10-00
10-10-00	(G)	JL	10-10-00
10-10-00	(H)	JL	10-10-00
10-10-00	(I)	JL	10-10-00
10-10-00	(J)	JL	10-10-00
10-10-00	(K)	JL	10-10-00
10-10-00	(L)	JL	10-10-00
10-10-00	(M)	JL	10-10-00
10-10-00	(N)	JL	10-10-00
10-10-00	(O)	JL	10-10-00
10-10-00	(P)	JL	10-10-00
10-10-00	(Q)	JL	10-10-00
10-10-00	(R)	JL	10-10-00
10-10-00	(S)	JL	10-10-00
10-10-00	(T)	JL	10-10-00
10-10-00	(U)	JL	10-10-00
10-10-00	(V)	JL	10-10-00
10-10-00	(W)	JL	10-10-00
10-10-00	(X)	JL	10-10-00
10-10-00	(Y)	JL	10-10-00
10-10-00	(Z)	JL	10-10-00



The seal on this drawing only certifies that the product(s) illustrated and described herein represent the configurations, dimensions and installation(s) of the door as tested.

23	2445	UNITED BULL	CHICK 16 ALUM EXC
24	2446	UNITED BULL	CHICK 16 ALUM EXC
25	2447	UNITED BULL	CHICK 16 ALUM EXC
26	2448	UNITED BULL	CHICK 16 ALUM EXC
27	2449	UNITED BULL	CHICK 16 ALUM EXC
28	2450	UNITED BULL	CHICK 16 ALUM EXC
29	2451	UNITED BULL	CHICK 16 ALUM EXC
30	2452	UNITED BULL	CHICK 16 ALUM EXC
31	2453	UNITED BULL	CHICK 16 ALUM EXC
32	2454	UNITED BULL	CHICK 16 ALUM EXC
33	2455	UNITED BULL	CHICK 16 ALUM EXC
34	2456	UNITED BULL	CHICK 16 ALUM EXC
35	2457	UNITED BULL	CHICK 16 ALUM EXC
36	2458	UNITED BULL	CHICK 16 ALUM EXC
37	2459	UNITED BULL	CHICK 16 ALUM EXC
38	2460	UNITED BULL	CHICK 16 ALUM EXC
39	2461	UNITED BULL	CHICK 16 ALUM EXC
40	2462	UNITED BULL	CHICK 16 ALUM EXC
41	2463	UNITED BULL	CHICK 16 ALUM EXC
42	2464	UNITED BULL	CHICK 16 ALUM EXC
43	2465	UNITED BULL	CHICK 16 ALUM EXC
44	2466	UNITED BULL	CHICK 16 ALUM EXC
45	2467	UNITED BULL	CHICK 16 ALUM EXC
46	2468	UNITED BULL	CHICK 16 ALUM EXC
47	2469	UNITED BULL	CHICK 16 ALUM EXC
48	2470	UNITED BULL	CHICK 16 ALUM EXC
49	2471	UNITED BULL	CHICK 16 ALUM EXC
50	2472	UNITED BULL	CHICK 16 ALUM EXC
51	2473	UNITED BULL	CHICK 16 ALUM EXC
52	2474	UNITED BULL	CHICK 16 ALUM EXC
53	2475	UNITED BULL	CHICK 16 ALUM EXC
54	2476	UNITED BULL	CHICK 16 ALUM EXC
55	2477	UNITED BULL	CHICK 16 ALUM EXC
56	2478	UNITED BULL	CHICK 16 ALUM EXC
57	2479	UNITED BULL	CHICK 16 ALUM EXC
58	2480	UNITED BULL	CHICK 16 ALUM EXC
59	2481	UNITED BULL	CHICK 16 ALUM EXC
60	2482	UNITED BULL	CHICK 16 ALUM EXC
61	2483	UNITED BULL	CHICK 16 ALUM EXC
62	2484	UNITED BULL	CHICK 16 ALUM EXC
63	2485	UNITED BULL	CHICK 16 ALUM EXC
64	2486	UNITED BULL	CHICK 16 ALUM EXC
65	2487	UNITED BULL	CHICK 16 ALUM EXC
66	2488	UNITED BULL	CHICK 16 ALUM EXC
67	2489	UNITED BULL	CHICK 16 ALUM EXC
68	2490	UNITED BULL	CHICK 16 ALUM EXC
69	2491	UNITED BULL	CHICK 16 ALUM EXC
70	2492	UNITED BULL	CHICK 16 ALUM EXC
71	2493	UNITED BULL	CHICK 16 ALUM EXC
72	2494	UNITED BULL	CHICK 16 ALUM EXC
73	2495	UNITED BULL	CHICK 16 ALUM EXC
74	2496	UNITED BULL	CHICK 16 ALUM EXC
75	2497	UNITED BULL	CHICK 16 ALUM EXC
76	2498	UNITED BULL	CHICK 16 ALUM EXC
77	2499	UNITED BULL	CHICK 16 ALUM EXC
78	2500	UNITED BULL	CHICK 16 ALUM EXC
79	2501	UNITED BULL	CHICK 16 ALUM EXC
80	2502	UNITED BULL	CHICK 16 ALUM EXC
81	2503	UNITED BULL	CHICK 16 ALUM EXC
82	2504	UNITED BULL	CHICK 16 ALUM EXC
83	2505	UNITED BULL	CHICK 16 ALUM EXC
84	2506	UNITED BULL	CHICK 16 ALUM EXC
85	2507	UNITED BULL	CHICK 16 ALUM EXC
86	2508	UNITED BULL	CHICK 16 ALUM EXC
87	2509	UNITED BULL	CHICK 16 ALUM EXC
88	2510	UNITED BULL	CHICK 16 ALUM EXC
89	2511	UNITED BULL	CHICK 16 ALUM EXC
90	2512	UNITED BULL	CHICK 16 ALUM EXC
91	2513	UNITED BULL	CHICK 16 ALUM EXC
92	2514	UNITED BULL	CHICK 16 ALUM EXC
93	2515	UNITED BULL	CHICK 16 ALUM EXC
94	2516	UNITED BULL	CHICK 16 ALUM EXC
95	2517	UNITED BULL	CHICK 16 ALUM EXC
96	2518	UNITED BULL	CHICK 16 ALUM EXC
97	2519	UNITED BULL	CHICK 16 ALUM EXC
98	2520	UNITED BULL	CHICK 16 ALUM EXC
99	2521	UNITED BULL	CHICK 16 ALUM EXC
100	2522	UNITED BULL	CHICK 16 ALUM EXC

WIND LOAD VS ANCHOR SPACING



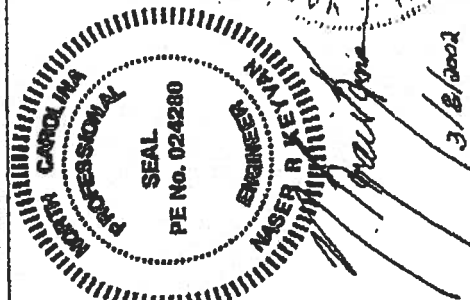
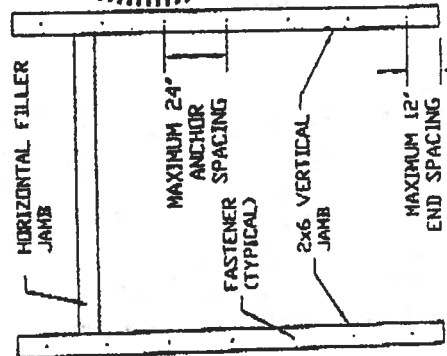
DESIGN (LBS) X GARAGE DOOR AREA (WIDTH-FT X HEIGHT-FT) = WIND LOAD (LBS)

EXAMPLE

30 LBS X 16 FT WIDE X 8 FT HIGH = 3840 LBS

- ① USE 22" SPACING
 ② USE 21" SPACING
 ③ USE 19" SPACING

SEE NOTE 11 FOR ADDITIONAL REQUIRED 2X6 WOOD JAMB ANCHORS



2X6 JAMB TO SUPPORTING STRUCTURE ATTACHMENT

2X6 PRESSURE TREATED GRADE #2 OR BETTER SOUTHERN PINE WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

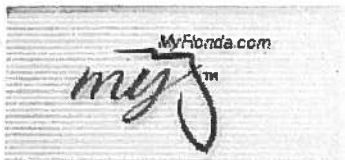
NOTES:

- ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT WITH DUE CONSIDERATION GIVEN TO INSTALLATIONS USING CENTER "HURRICANE" POSTS.
- ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI "STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION" SSTB 10, CURRENT EDITION.
- ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.
- WOOD FRAME BUILDINGS STUDS AT EACH SIDE OF DOOR OPENING SHALL BE PROPERLY DESIGNED, CONNECTED, ANCHORED AND SHALL CONSIST OF A MINIMUM OF THREE (3) LAMINATIONS OF 2X6 PRESSURE TREATED SOUTHERN PINE #2 GRADE OR BETTER WALL STUDS CONTINUOUS FROM FOOTING TO DOUBLE TOP PLATE.
- REINFORCED CMU OR CONCRETE: 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2500 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
- EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.
- ANCHORS FOR CONCRETE AND CONCRETE MASONRY UNITS (CMU) SHALL HAVE A MINIMUM 3" EDGE DISTANCE FROM ALL EDGES OF CONCRETE OR CONCRETE MASONRY UNITS. ANCHORS FOR CONCRETE AND CMU SHALL HAVE A MINIMUM SPACING OF 3-3/4"
- LAG SCREWS SHALL BE CENTERED IN ONE OF THE 1-1/2" DIMENSION FACES OF THE TRIPLE 2X6 WALL STUDS.
- WASHERS ARE REQUIRED ON ALL FASTENERS.
- THE WIND LOAD VS. ANCHOR SPACING CHART IS FOR A MAXIMUM DOOR SIZE OF 18' X 8' AT A MAXIMUM 42 PSF DESIGN WIND LOAD.
- FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, AND AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.



GENERAL AMERICAN DOOR COMPANY
5000 BASELINE ROAD
HUNTINGBURY, IL 60538

SALES MAN	APPROVED BY	DATE
SALES MAN	APPROVED BY	DATE
JAMB TO STRUCTURE ATTACHMENT FOR WIND LOADED GARAGE DOORS		
AL0560		



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Licensee Details

Licensee Information

Name: STEWART, ROBERT SCOTT (Primary Name)
ROB STEWART LC (DBA Name)
Main Address: P O BOX 3001
LAKE CITY Florida 32056
County: COLUMBIA

License Mailing:

LicenseLocation: 507 W DUVAL STREET
LAKE CITY FL 32055
County: COLUMBIA

License Information

License Type: Certified Building Contractor
Rank: Cert Building
License Number: CBC1252898
Status: Current,Active
Licensure Date: 04/19/2005
Expires: 08/31/2006

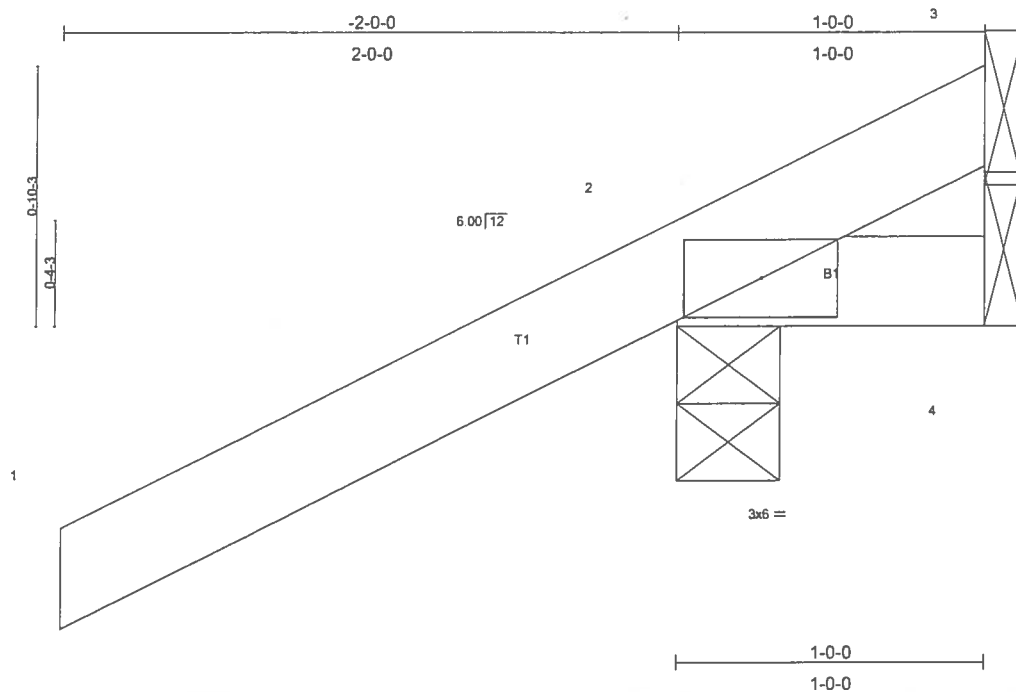
Special Qualifications **Qualification Effective**
Qualified Business **08/11/2005**
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Job L155888	Truss CJ1	Truss Type JACK	Qty 16	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:09 2006 Page 1		



Scale = 1:7.2

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.00	2	>999	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL) -0.00	2	>999	180			
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00	3	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							Weight: 7 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift 2=-287(load case 5), 4=-9(load case 3), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

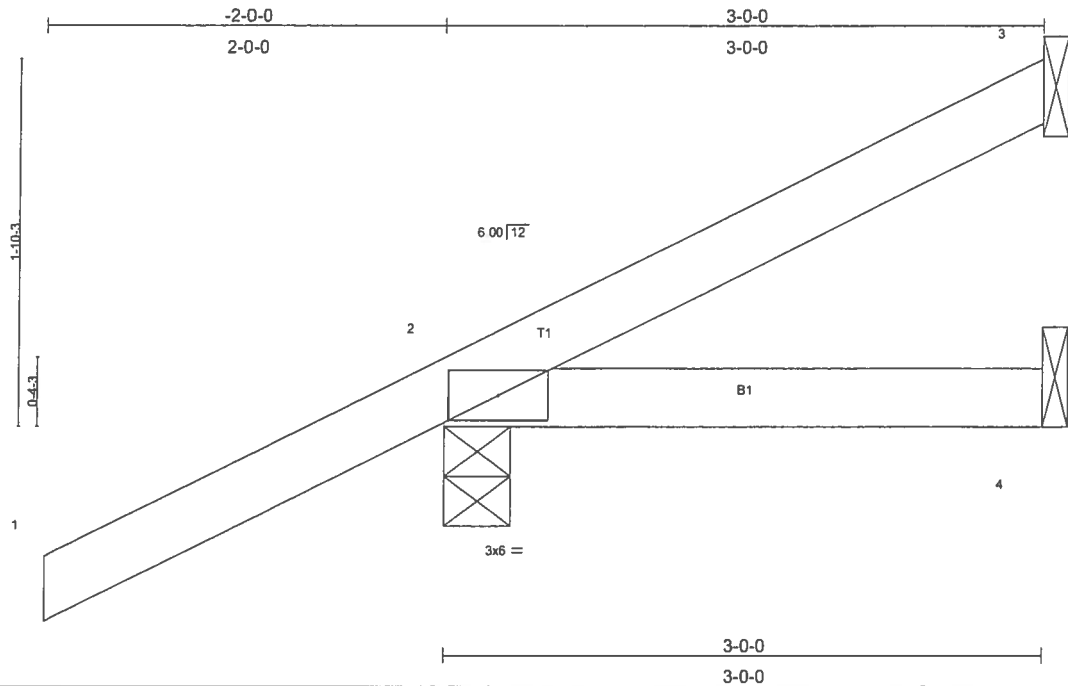
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

LOAD CASE(S) Standard

Job L155888	Truss CJ3	Truss Type JACK	Qty 16	Ply 1	00
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional)
6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Mar 17 14:07:12 2006					Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	V/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL) 0.01 2-4	>999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.08	Vert(TL) 0.01 2-4	>999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3	n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)				
					Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-240(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-58/7
 BOT CHORD 2-4=0/0

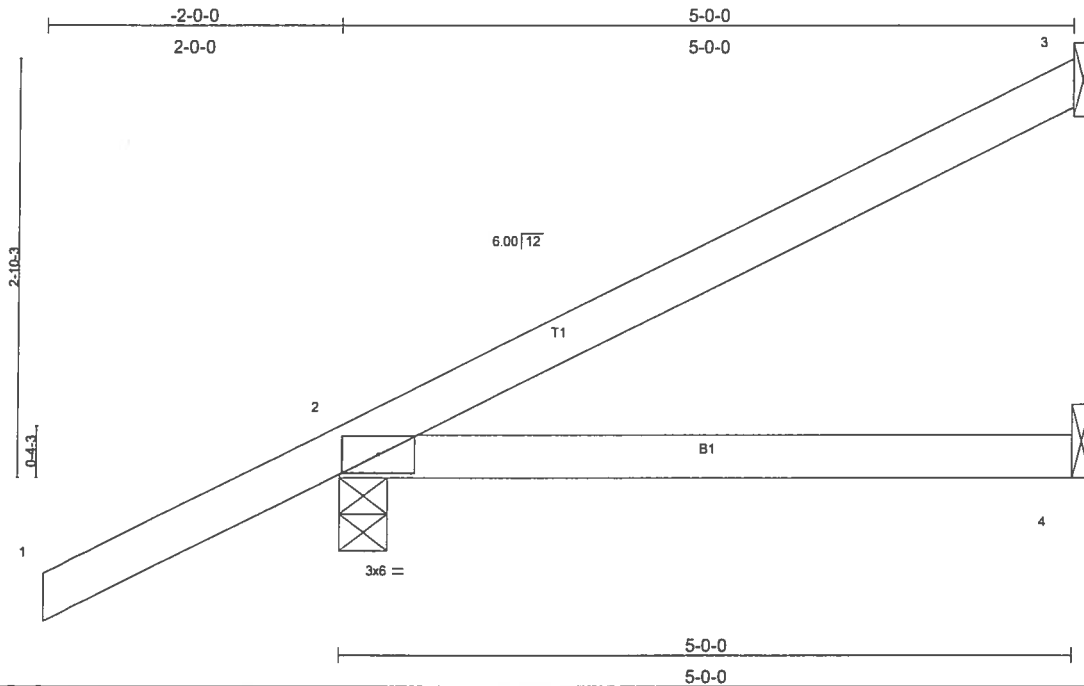
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

LOAD CASE(S) Standard

**MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Job L155888	Truss CJ5	Truss Type JACK	Qty 14	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:14 2006 Page 1			



Scale = 1:15.0

LOADING (psf)	SPACING	2'-0'-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 19 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5'-0\"/>

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift 3=86(load case 5), 2=261(load case 5), 4=46(load case 3)

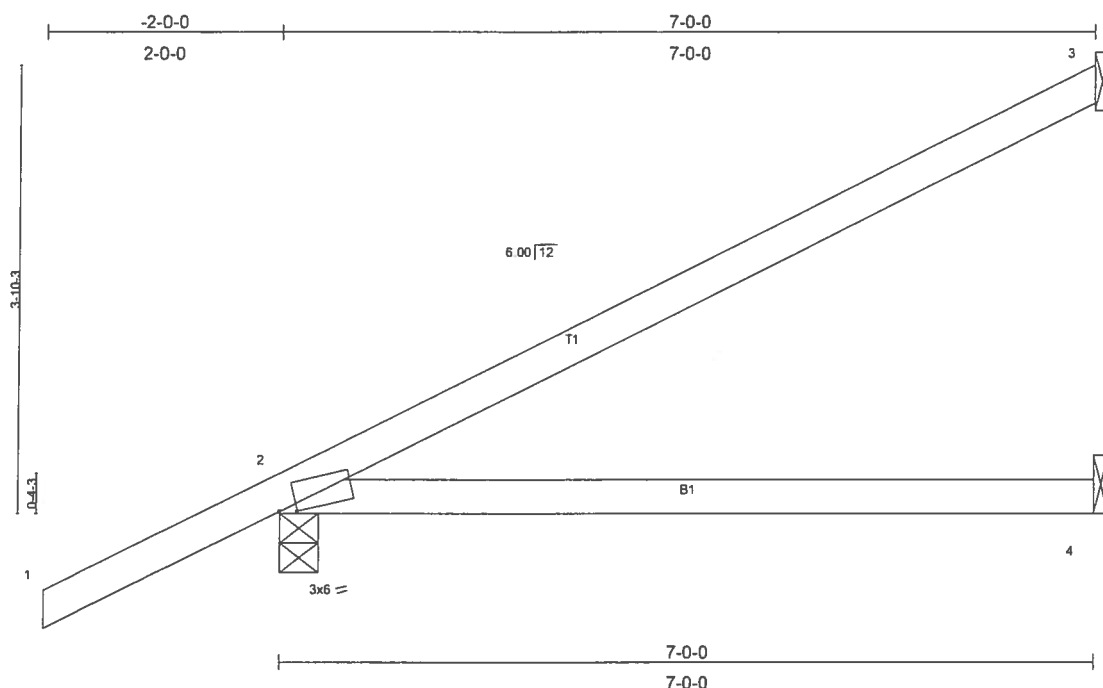
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=87/36
BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	0 0
L155888	EJ7	MONO TRUSS	22	1	
					Job Reference (optional)
Builders FirstSource, Lake City, FL 32055					6,200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:16 2006 Page 1



Scale = 1:18.9
Camber = 1/16 in

Plate Offsets (X,Y): [2:0-1-13,0-0-7]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl l/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.50	Vert(LL) 0.32 2-4 >253 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.45	Vert(TL) 0.27 2-4 >298 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			Weight: 26 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-143(load case 5), 2=-296(load case 5), 4=-67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-130/58
BOT CHORD 2-4=0/0

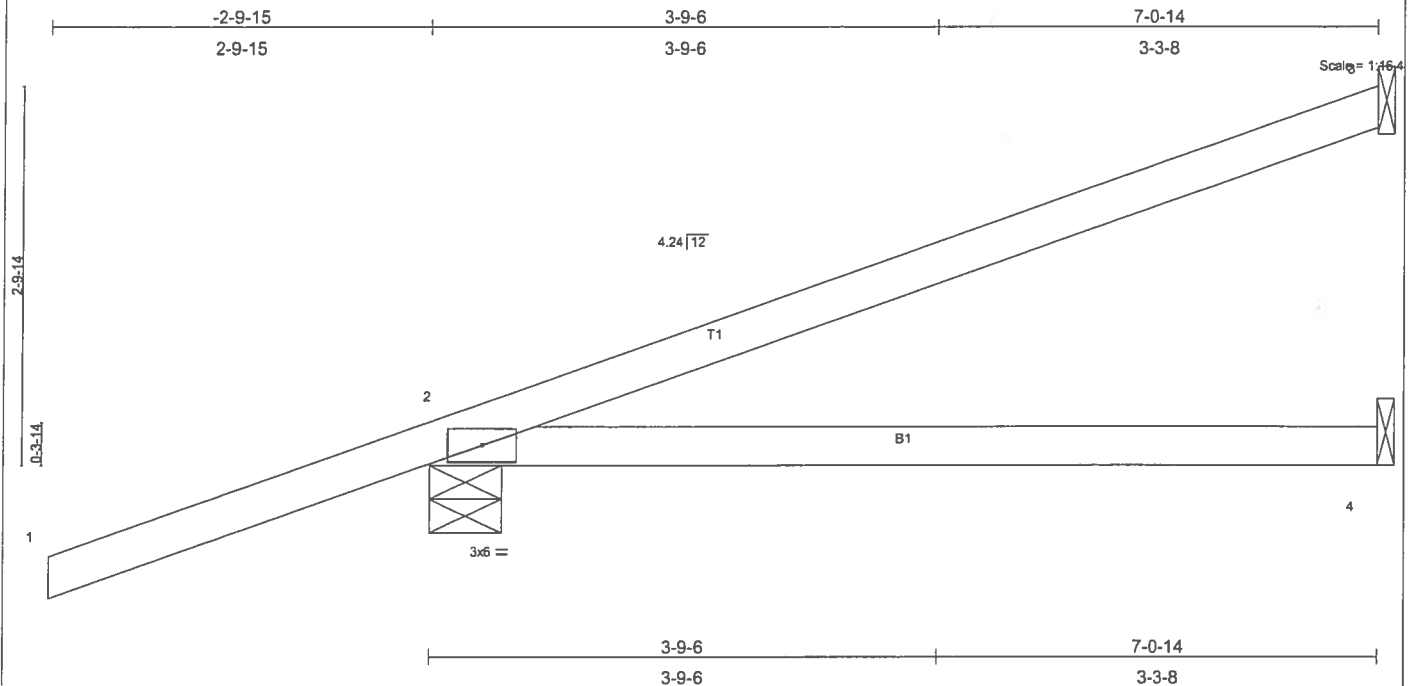
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 3, 296 lb uplift at joint 2 and 67 lb uplift at joint 4.

LOAD CASE(S) Standard

MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L155888	Truss HJ7	Truss Type JACK	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:19 2006 Page 1			



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.55	Vert(LL)	0.10	2-4	>773	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	-0.13	2-4	>645	180		
BCLL 10.0	Rep Stress incr	NO	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purtins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

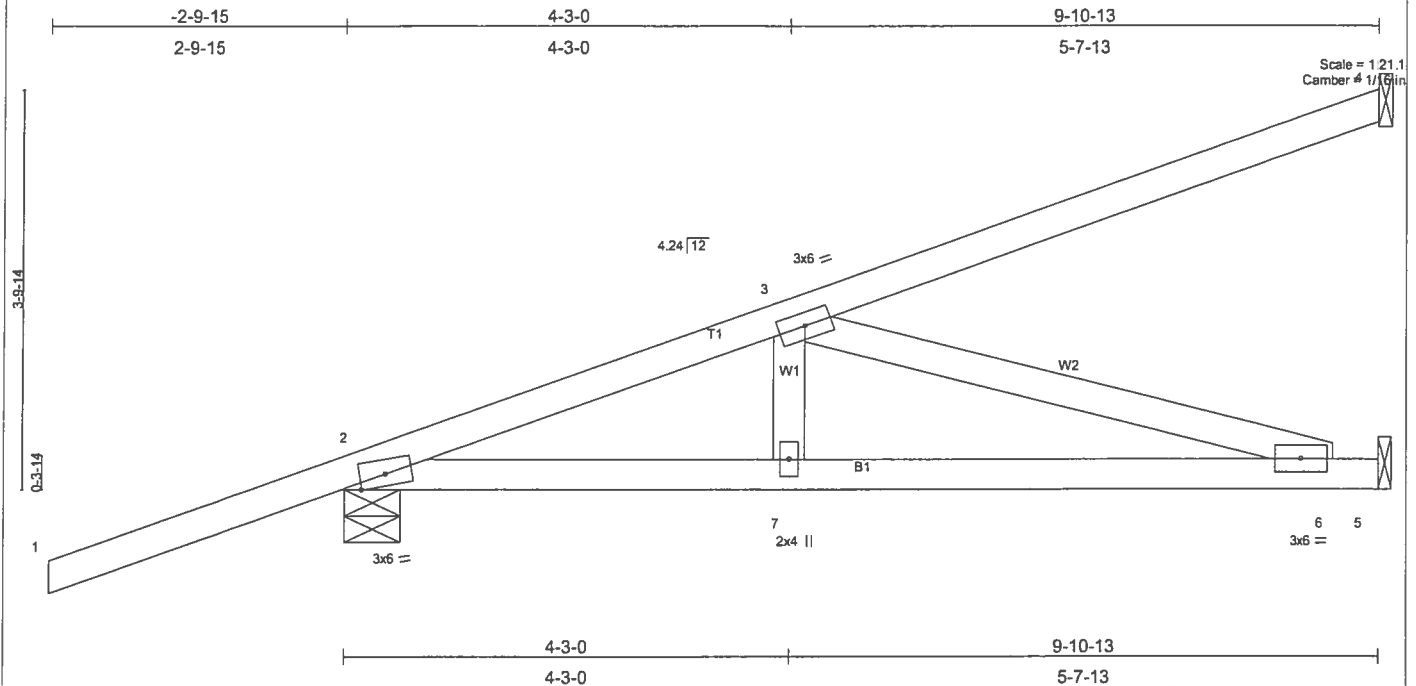
REACTIONS (lb/size) 3=190/Mechanical, 2=381/0-6-7, 4=110/Mechanical
Max Horz 2=168(load case 2)
Max Uplift 3=153(load case 2), 2=338(load case 2), 4=55(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=71/45
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 3, 338 lb uplift at joint 2 and 55 lb uplift at joint 4.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-4=-53(F=-12, B=-12)

Job L155888	Truss HJ9	Truss Type MONO TRUSS	Qty 7	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:22 2006 Page 1			



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.62	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.61	Vert(LL) -0.11 6-7 >999 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.46	Vert(TL) -0.18 6-7 >623 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 5 n/a n/a		
	Code FBC2004/TP12002			Weight: 45 lb	

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-6, 5=372/Mechanical
 Max Horz 2=270(load case 2)
 Max Uplift 4=233(load case 2), 2=404(load case 2), 5=180(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/50, 2-3=877/358, 3-4=-105/66
 BOT CHORD 2-7=530/810, 6-7=530/810, 5-6=0/0
 WEBS 3-7=-94/186, 3-6=-844/553

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Refer to girder(s) for truss to truss connections.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.
- 4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-54
 Trapezoidal Loads (plf)
 Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

Job L155888	Truss T01	Truss Type HIP	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MITek Industries, Inc. Fri Mar 17 14:07:25 2006 Page 1			

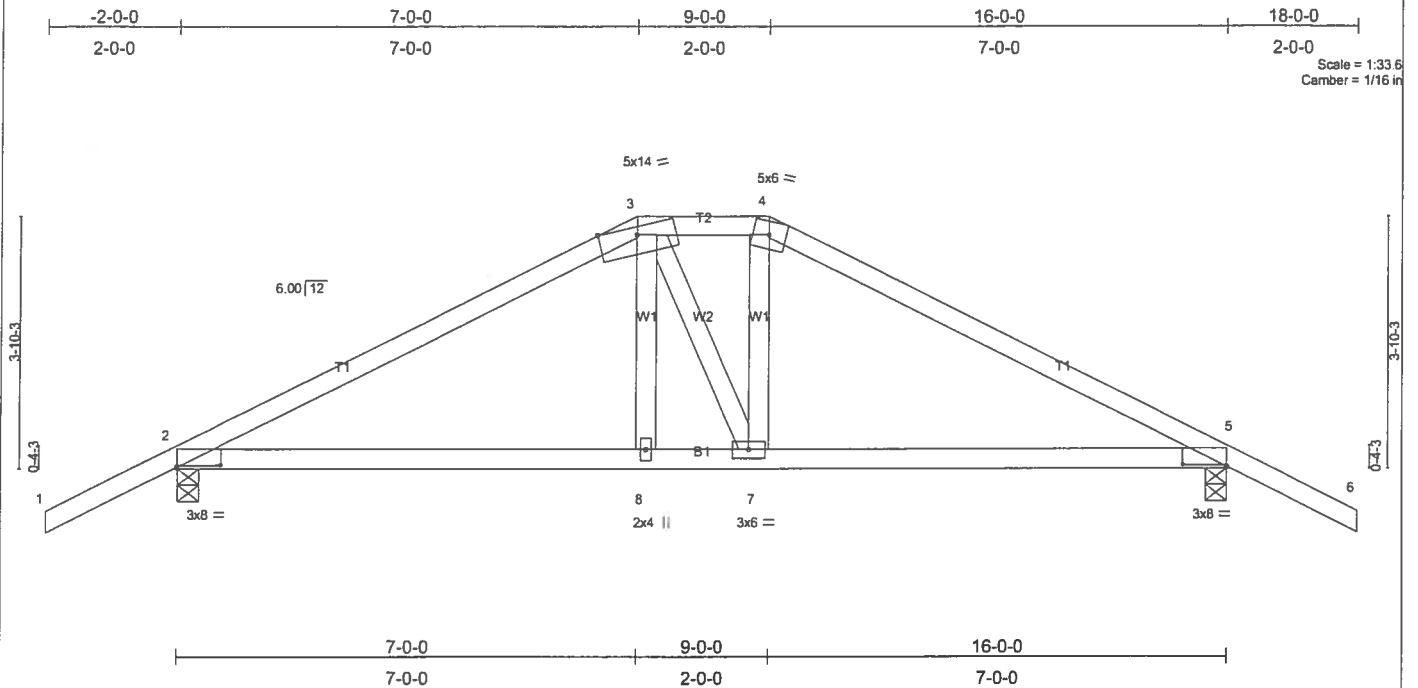


Plate Offsets (X,Y): [2-0-8-0-0-0-6], [5-0-8-0-0-0-6]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	0.13	2-8	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.61	Vert(TL)	-0.20	2-8	>954	180	GRIP
BCLL 10.0	Rep Stress Incr	NO	WB 0.27	Horz(TL)	0.05	5	n/a	n/a	244/190
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 72 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-9-2 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-1-0 oc bracing.

REACTIONS (lb/size) 2=1411/0-4-0, 5=1411/0-4-0
 Max Horz 2=-87(load case 5)
 Max Uplift 2=-864(load case 4), 5=-864(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-2311/1220, 3-4=-2013/1168, 4-5=-2315/1222, 5-6=0/47
 BOT CHORD 2-8=-1015/1979, 7-8=-1029/2008, 5-7=-994/1983
 WEBS 3-8=-375/717, 3-7=-141/161, 4-7=-422/830

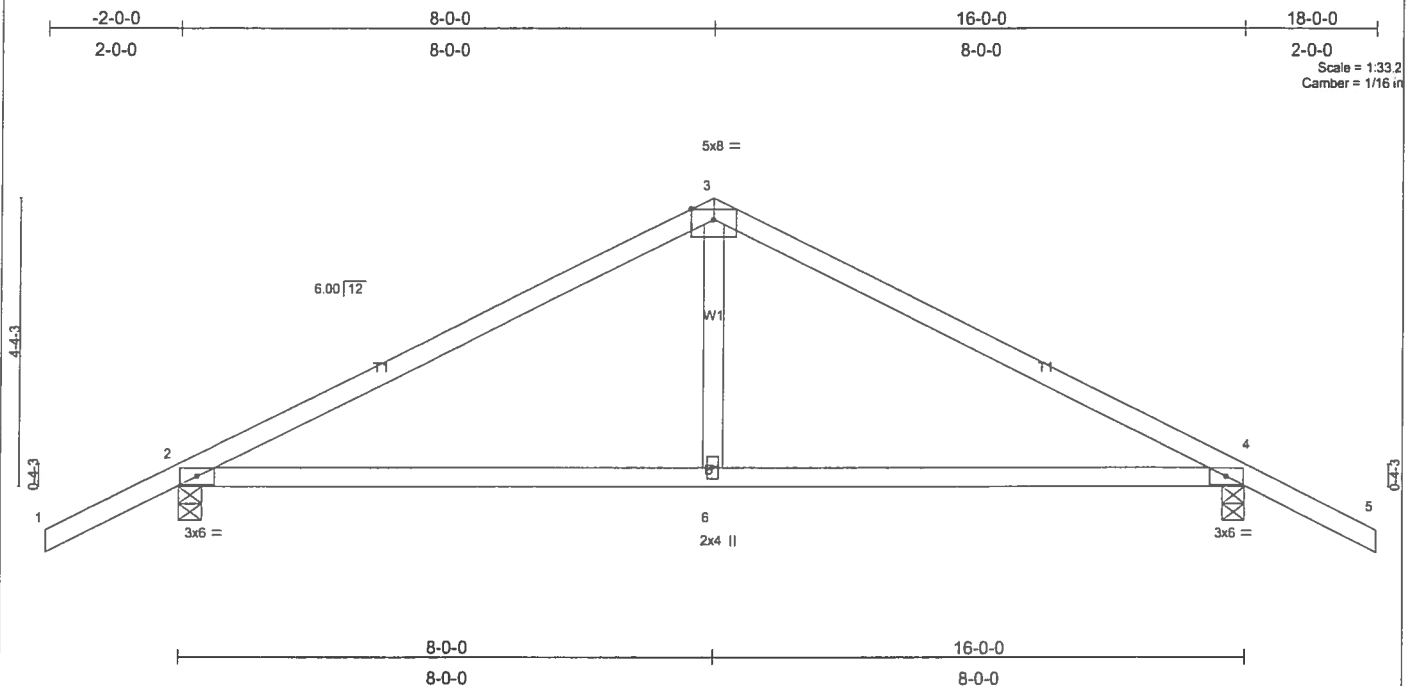
NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 864 lb uplift at joint 2 and 864 lb uplift at joint 5.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-4=-120(F=-66), 4-6=-54, 2-8=-30, 7-8=-67(F=-37), 5-7=-30
 Concentrated Loads (lb)
 Vert: 8=-539(F) 7=-539(F)

Job L155888	Truss T02	Truss Type COMMON	Qty 2	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6/2005 Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:28 2006 Page 1			



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.51	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.55	Vert(LL) 0.24 4-6 >789 240		
BCLL 10.0	Lumber Increase 1.25	WB 0.14	Vert(TL) -0.20 4-6 >937 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.02 4 n/a n/a		
	Code FBC2004/TPI2002				Weight: 63 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing.

REACTIONS (lb/size) 2=775/0-4-0, 4=775/0-4-0
 Max Horz 2=94(load case 5)
 Max Uplift 2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-924/975, 3-4=-924/975, 4-5=0/47
 BOT CHORD 2-6=-683/746, 4-6=-683/746
 WEBS 3-6=-487/293

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

LOAD CASE(S) Standard

Job L155888	Truss T03	Truss Type HIP	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6,200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:30 2006 Page 1			

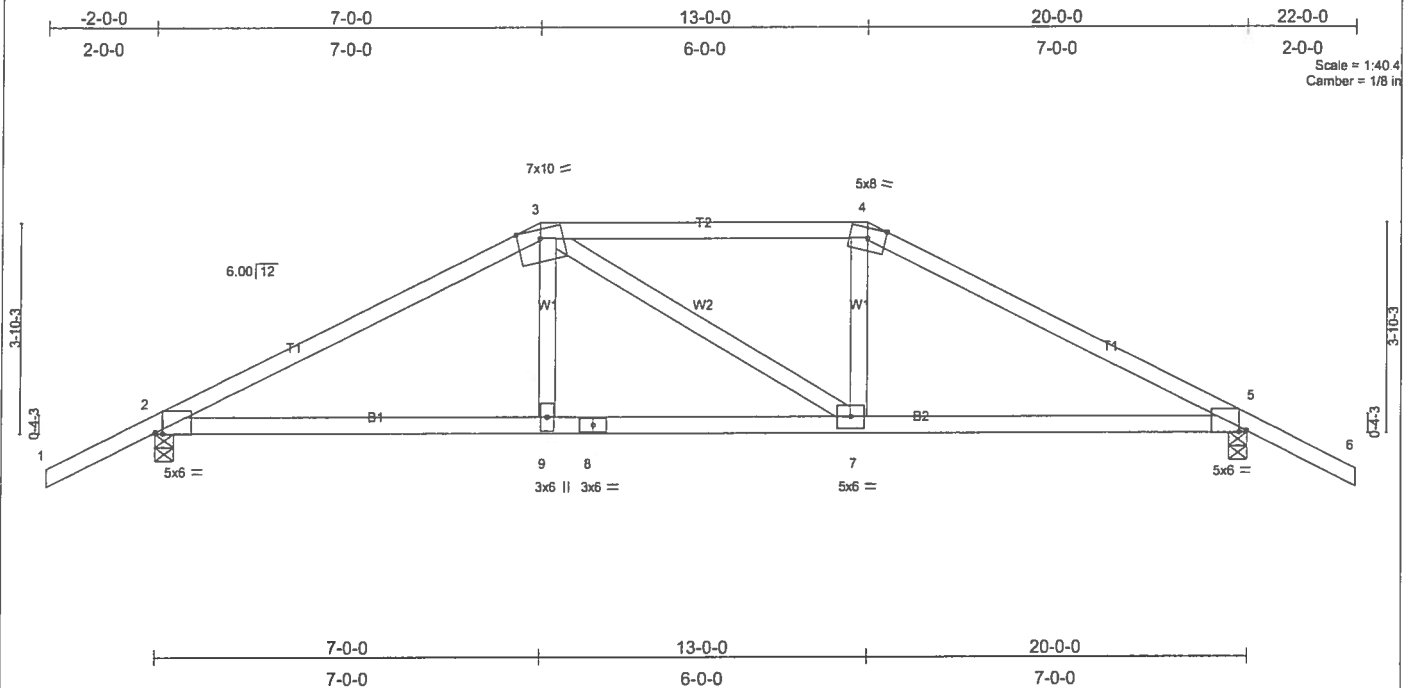


Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-1-11,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCDL 20.0	Plates Increase 1.25	TC 0.60	Vert(LL)	-0.17	7-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.74	Vert(TL)	-0.27	7-9	>859	180		
BCLL 10.0	Rep Stress Incr NO	WB 0.29	Horz(TL)	0.09	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							Weight: 88 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-3 oc bracing.

REACTIONS (lb/size) 2=1784/0-4-0, 5=1784/0-4-0
Max Horz 2=87(load case 4)
Max Uplift 2=813(load case 4), 5=813(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3162/1283, 3-4=-2780/1220, 4-5=-3163/1283, 5-6=0/47
BOT CHORD 2-9=-1082/2743, 8-9=-1091/2778, 7-8=-1091/2778, 5-7=-1041/2745
WEBS 3-9=-219/852, 3-7=-122/126, 4-7=-240/904

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 813 lb uplift at joint 2 and 813 lb uplift at joint 5.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 13-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-120(F=-66), 4-6=-54, 2-9=-30, 7-9=-67(F=-37), 5-7=-30
Concentrated Loads (lb)
Vert: 9=-539(F) 7=-539(F)

MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L155888	Truss T04	Truss Type HIP	Qty 1	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:33 2006 Page 1		

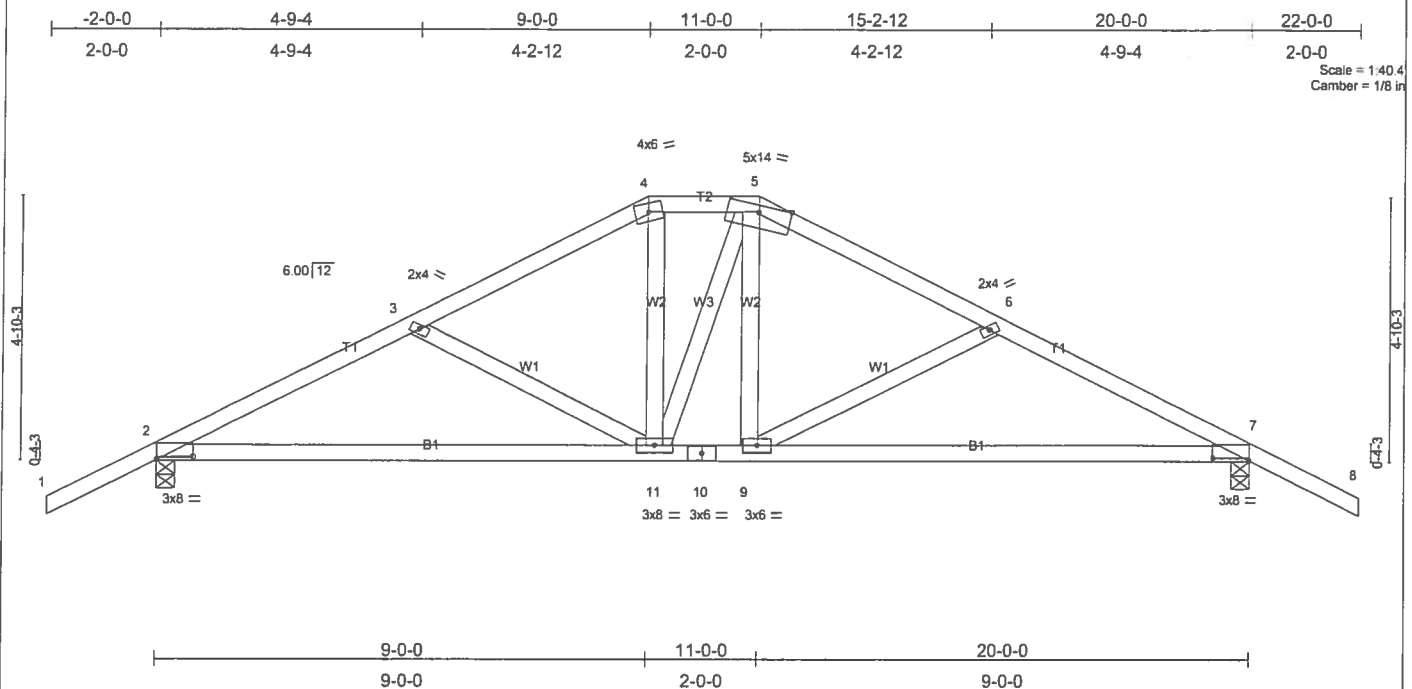


Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]									
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.30	Vert(LL)	-0.15	7-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.48	Vert(TL)	-0.26	7-9	>903	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.13	Horz(TL)	0.04	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)							
								Weight: 103 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-2-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=943/0-4-0, 7=943/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift 2=-390(load case 5), 7=-390(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=1323/560, 3-4=1062/446, 4-5=907/450, 5-6=1061/445, 6-7=1323/560, 7-8=0/47
BOT CHORD 2-11=337/1143, 10-11=140/904, 9-10=140/904, 7-9=337/1143
WEBS 3-11=280/224, 4-11=75/306, 5-11=98/109, 5-9=63/287, 6-9=282/225

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 2 and 390 lb uplift at joint 7.

LOAD CASE(S) Standard

**MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549**

Job L155888	Truss T05	Truss Type COMMON	Qty 3	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MtTek Industries, Inc. Fri Mar 17 14:07:36 2006 Page 1			

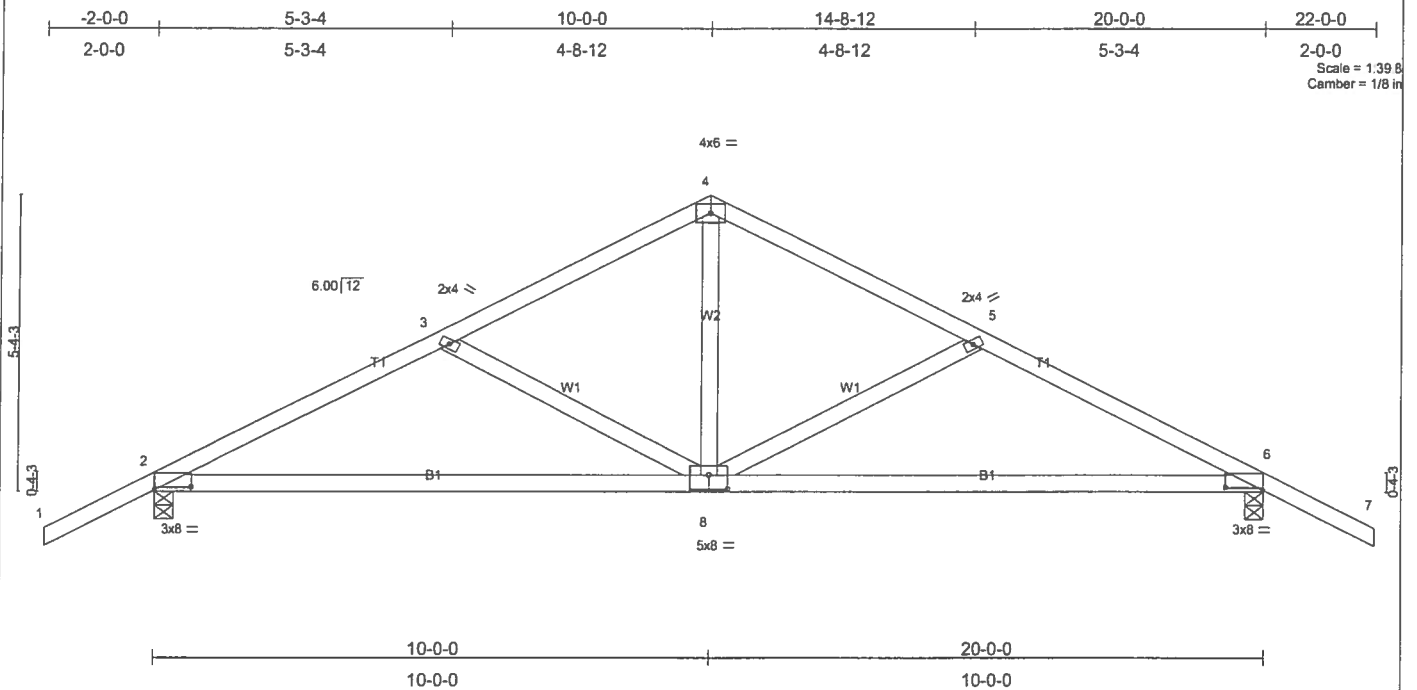


Plate Offsets (X,Y): [2-0-8-0,0-0-10], [6-0-8-0,0-0-10], [8-0-4-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.17	2-8	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.57	Vert(TL)	-0.30	2-8	>797	180		
BCCL 10.0	Rep Stress Incr	YES	WB 0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 93 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=943/0-4-0, 6=943/0-4-0
Max Horz 2=-108(load case 6)
Max Uplift 2=-396(load case 5), 6=-396(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1302/559, 3-4=-1018/435, 4-5=-1018/435, 5-6=-1302/559, 6-7=0/47
BOT CHORD 2-8=-341/1121, 6-8=-332/1121
WEBS 3-8=-322/249, 4-8=-158/604, 5-8=-322/249

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 2 and 396 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L155888	Truss T06	Truss Type COMMON	Qty 3	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Mar 17 14:07:38 2006 Page 1			

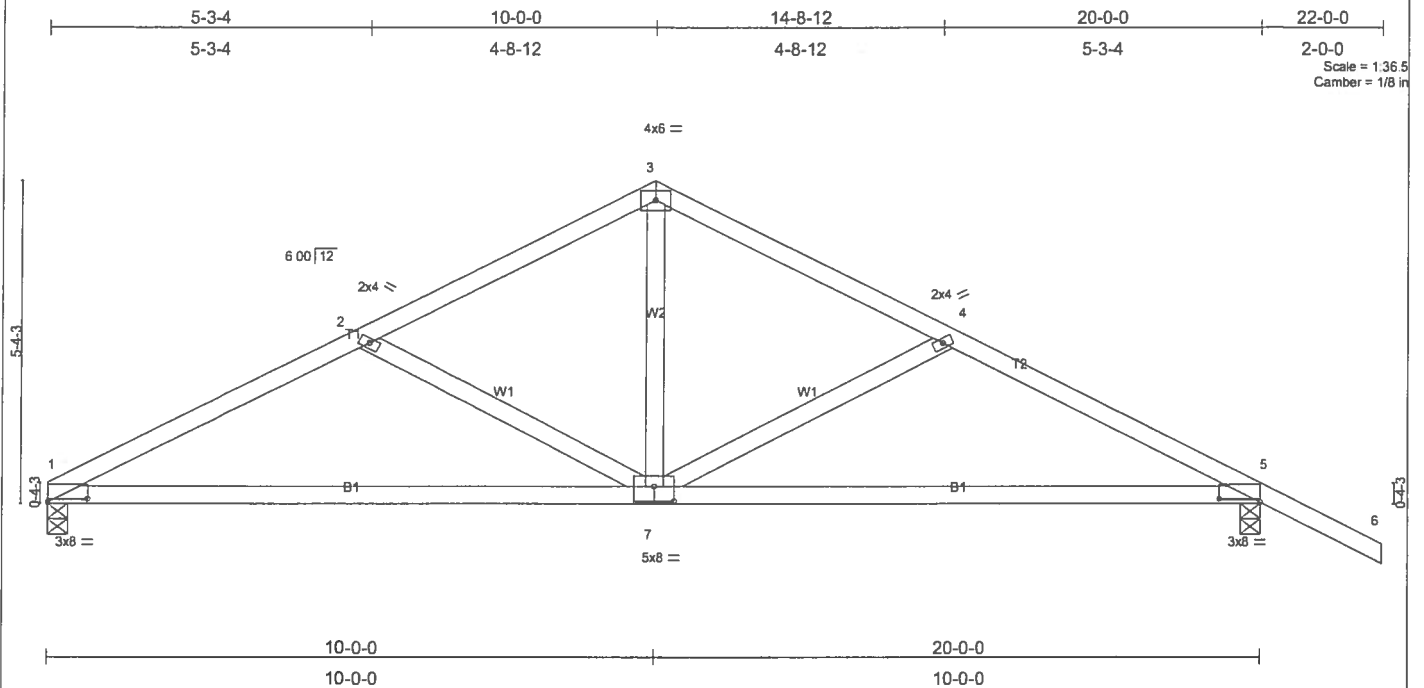


Plate Offsets (X,Y): [1:0-8-0,0-0-10], [5:0-8-0,0-0-10], [7:0-4-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	V/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.34	Vert(LL)	-0.20	1-7	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL)	-0.34	1-7	>685	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.20	Horz(TL)	0.04	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
										Weight: 89 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-6 oc bracing.

REACTIONS

(lb/size) 1=820/0-4-0, 5=949/0-4-0
Max Horz 1=-133(load case 6)
Max Uplift 1=-266(load case 5), 5=-398(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1336/614, 2-3=-1036/464, 3-4=-1034/461, 4-5=-1318/586, 5-6=0/47
BOT CHORD 1-7=-395/1160, 5-7=-355/1135
WEBS 2-7=-349/294, 3-7=-191/624, 4-7=-322/249

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 1 and 398 lb uplift at joint 5.

LOAD CASE(S) Standard

Job L155888	Truss T07	Truss Type HIP	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MITek Industries, Inc. Fri Mar 17 14:07:41 2006 Page 1			

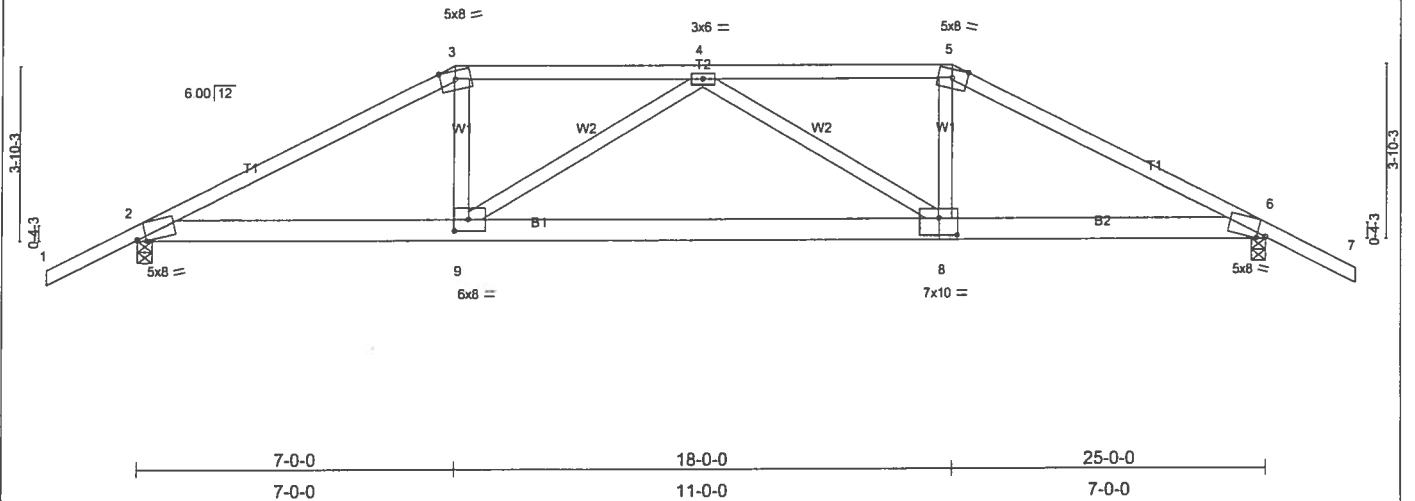
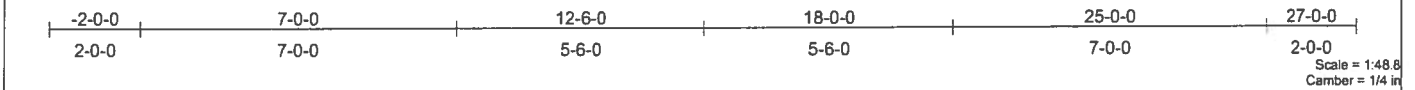


Plate Offsets (X,Y): [2-0-2-7, Edge], [6-0-2-7, Edge], [8-0-5-0, 0-4-8], [9-0-3-8, 0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.63	Vert(LL)	-0.35	8-9	>837	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(TL)	-0.59	8-9	>505		
BCLL 10.0	Rep Stress Incr NO	WB 0.52	Horz(TL)	0.09	6	n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)						
							Weight: 132 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-12 oc bracing.

REACTIONS

(lb/size) 2=2250/0-4-0, 6=2250/0-4-0
Max Horz 2=89(load case 4)
Max Uplift 2=-992(load case 4), 6=-992(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-4319/1710, 3-4=-3841/1587, 4-5=-3841/1587, 5-6=-4319/1710, 6-7=0/51
BOT CHORD 2-8=-1487/3781, 8-9=-1921/4363, 6-8=-1447/3781
WEBS 3-9=-469/1449, 4-9=-727/585, 4-8=-727/585, 5-8=-469/1449

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 992 lb uplift at joint 2 and 992 lb uplift at joint 6.
- Girder carries hip end with 7-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 18-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-120(F=-66), 5-7=-54, 2-9=-30, 8-9=-67(F=-37), 6-8=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 9=-539(F)

Job L155888	Truss T08	Truss Type HIP	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 Mittek Industries, Inc. Fri Mar 17 14:07:44 2006 Page 1			

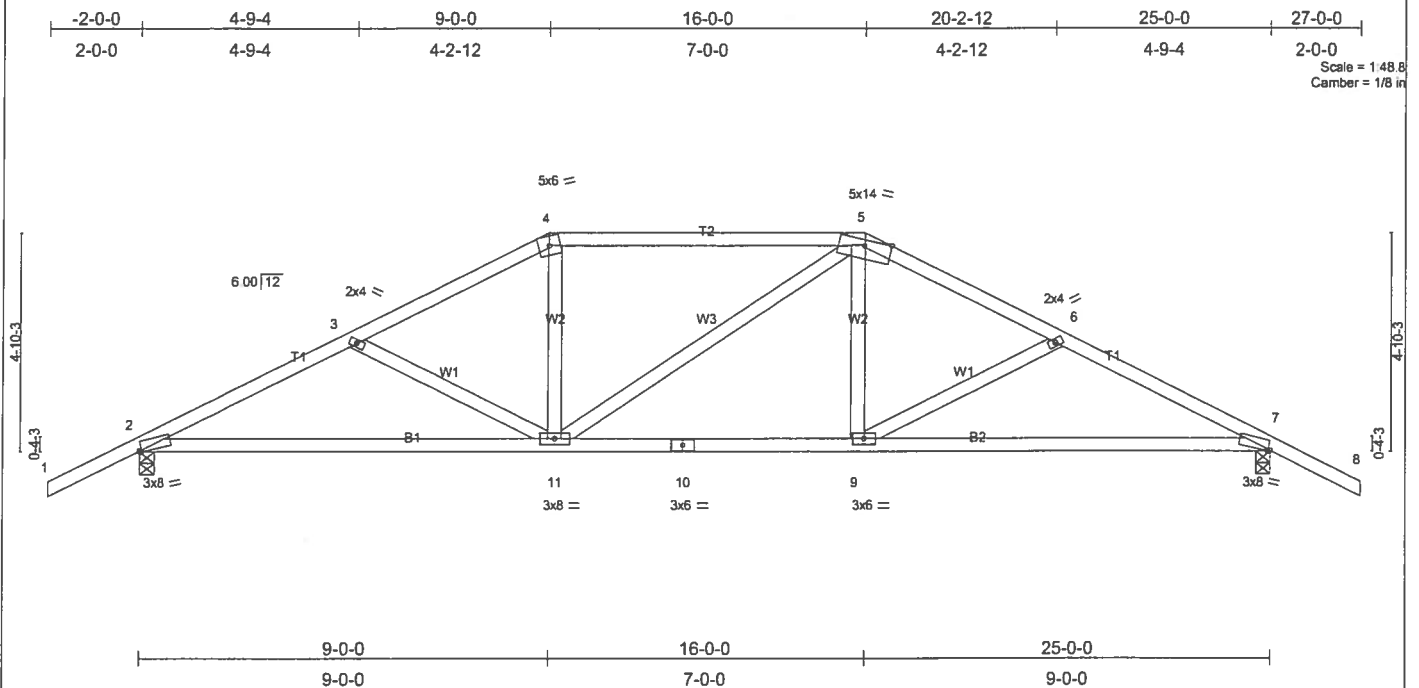


Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCCL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.18	7-9	>999	240
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.31	7-9	>970	180
BCLL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(TL)	0.06	7	n/a	n/a
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)						
Weight: 123 lb										

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-5-9 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-11-5 oc bracing.

REACTIONS (lb/size) 2=1153/0-4-0, 7=1153/0-4-0
 Max Horz 2=101(load case 5)
 Max Uplift 2=440(load case 5), 7=440(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1757/729, 3-4=-1531/635, 4-5=-1341/627, 5-6=-1530/636, 6-7=-1757/729, 7-8=0/47
 BOT CHORD 2-11=-484/1522, 10-11=-318/1341, 9-10=-318/1341, 7-9=-484/1523
 WEBS 3-11=-217/189, 4-11=-35/336, 5-11=-115/116, 5-9=-34/336, 6-9=-218/188

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 2 and 440 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L155888	Truss T09	Truss Type HIP	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:47 2006 Page 1			

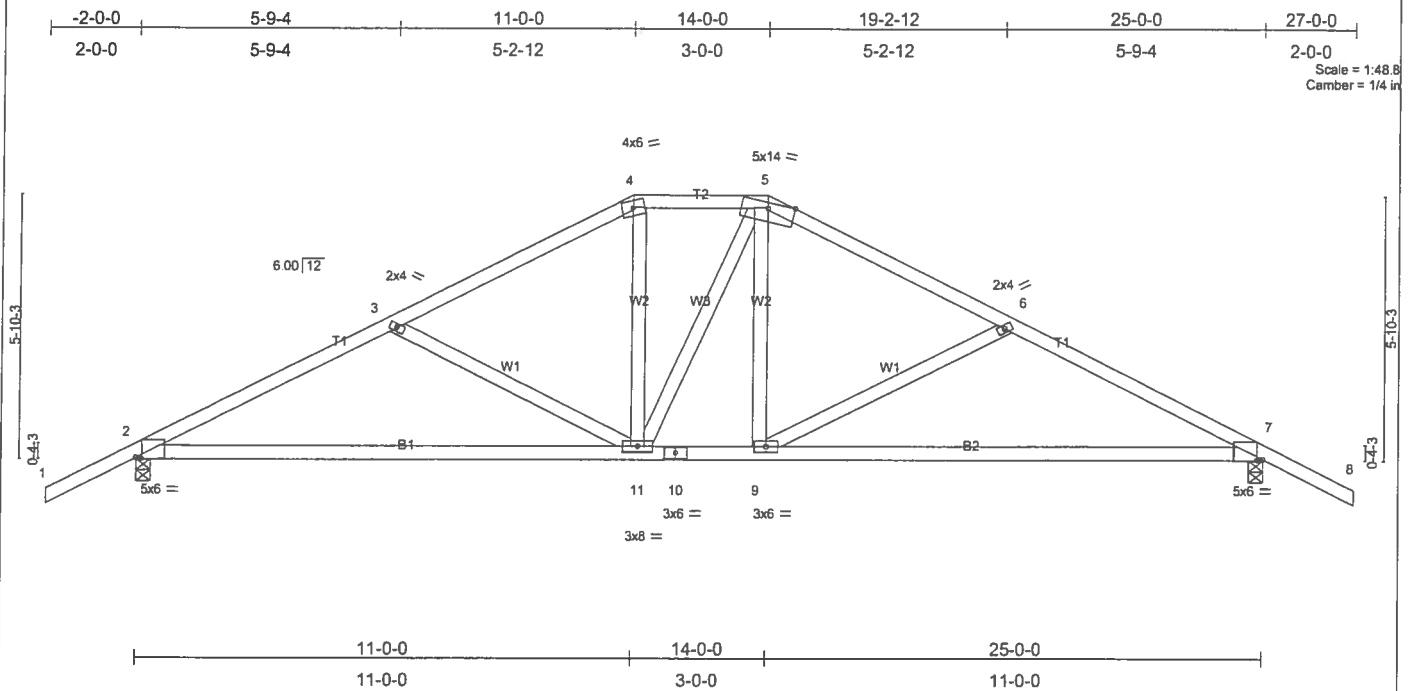


Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.38	Vert(LL)	-0.34	7-9	>866	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.67	Vert(TL)	-0.59	7-9	>502	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.25	Horz(TL)	0.06	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
Weight: 127 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-10-6 oc bracing.

REACTIONS

(lb/size) 2=1153/0-4-0, 7=1153/0-4-0
 Max Horz 2=115(load case 5)
 Max Uplift 2=-454(load case 5), 7=-454(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

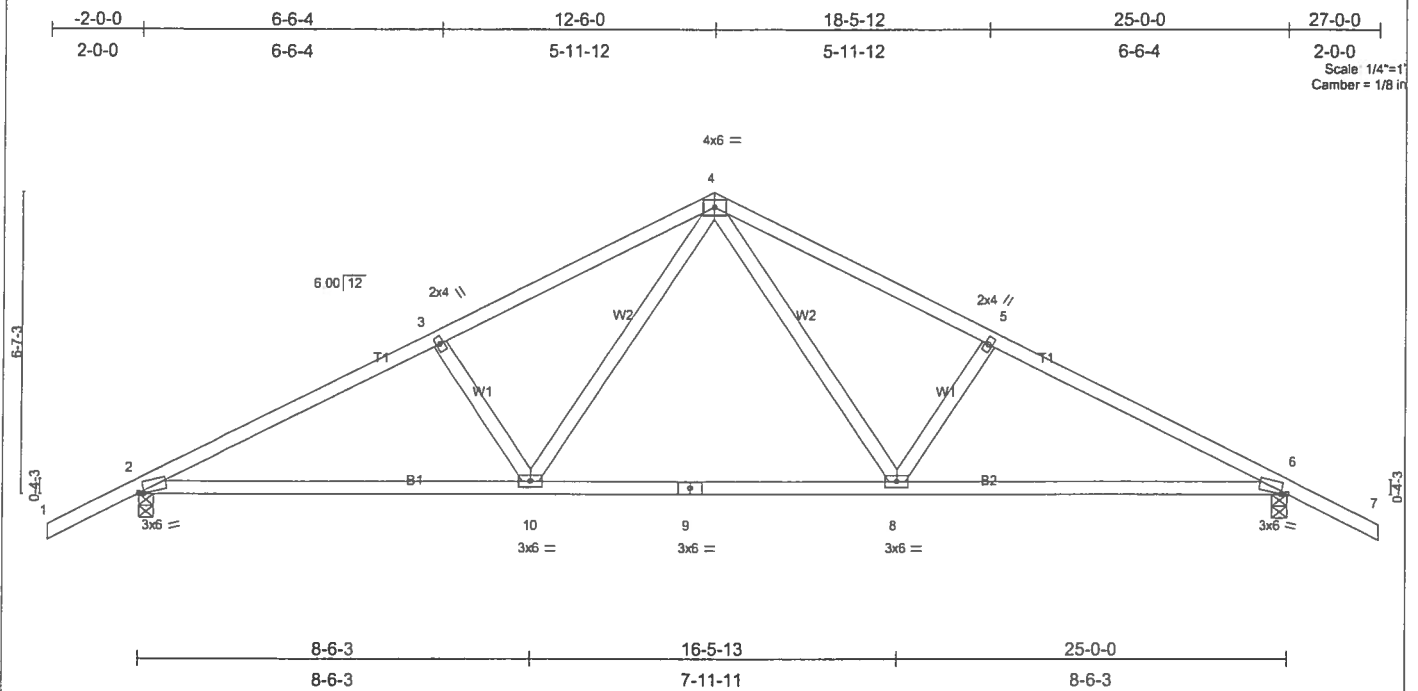
TOP CHORD 1-2=0/47, 2-3=-1709/749, 3-4=-1374/594, 4-5=-1172/591, 5-6=-1373/594, 6-7=-1709/749, 7-8=0/47
 BOT CHORD 2-11=-496/1487, 10-11=-231/1171, 9-10=-231/1171, 7-9=-496/1487
 WEBS 3-11=-367/300, 4-11=-92/382, 5-11=-127/132, 5-9=-92/375, 6-9=-368/301

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2 and 454 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L155888	Truss T10	Truss Type COMMON	Qty 1	Ply 1	00
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6200 s Jul 13 2005 Miltek Industries, Inc. Fri Mar 17 14:07:50 2006 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) -0.15 6-8 >999 240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(TL) -0.26 6-8 >999 180		
BCLL 10.0	Rep Stress Incr YES	WB 0.22	Horz(TL) 0.05 6 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)		Weight: 118 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-5-8 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 9-2-3 oc bracing.

REACTIONS (lb/size) 2=1153/0-4-0, 6=1153/0-4-0
Max Horz 2=126(load case 5)
Max Uplift 2=-463(load case 5), 6=-463(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=1750/730, 3-4=1566/725, 4-5=1566/725, 5-6=1750/730, 6-7=0/47
BOT CHORD 2-10=472/1492, 9-10=200/1011, 8-9=200/1011, 6-8=472/1492
WEBS 3-10=316/285, 4-10=223/620, 4-8=223/620, 5-8=316/285

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 463 lb uplift at joint 2 and 463 lb uplift at joint 6.

LOAD CASE(S) Standard

Job L155888	Truss T11	Truss Type COMMON	Qty 5	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:52 2006 Page 1			

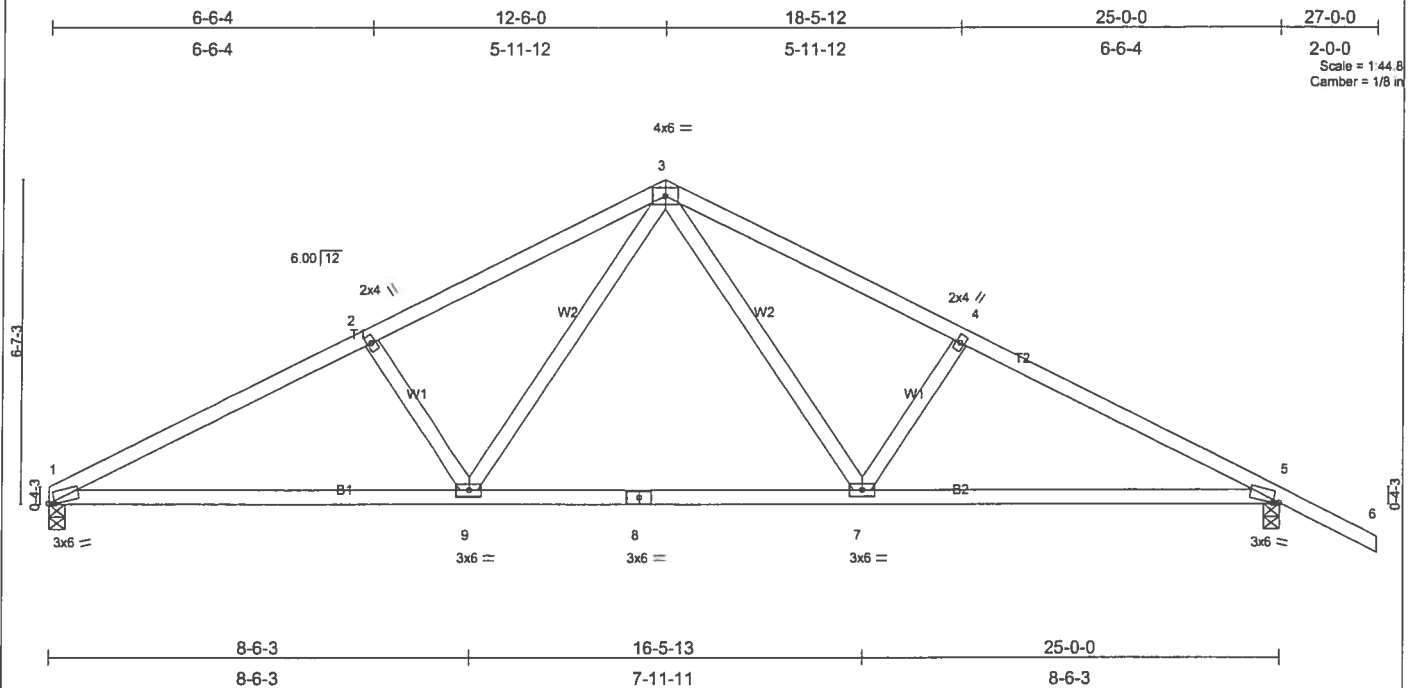


Plate Offsets (X,Y): [1:0-1-8,0-0-7], [2:0-0-0,0-0-0], [3:0-0-0,0-0-0], [4:0-0-0,0-0-0], [5:0-1-8,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	L/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	-0.18	1-9	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.52	Vert(TL)	-0.30	1-9	>978	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.27	Horz(TL)	0.05	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							Weight: 115 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-10 oc bracing.

REACTIONS (lb/size) 1=1031/0-4-0, 5=1158/0-4-0

Max Horz 1=-150(load case 6)

Max Uplift 1=-335(load case 5), 5=-465(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1765/781, 2-3=-1595/775, 3-4=-1575/742, 4-5=-1760/747, 5-6=0/47

BOT CHORD 1-9=-527/1525, 8-9=-216/1020, 7-8=-216/1020, 5-7=-487/1501

WEBS 2-9=-331/310, 3-9=-270/648, 3-7=-219/619, 4-7=-316/287

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C

Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 335 lb uplift at joint 1 and 465 lb uplift at joint 5.

LOAD CASE(S) Standard

Job L155888	Truss T12	Truss Type COMMON	Qty 2	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:55 2006 Page 1			

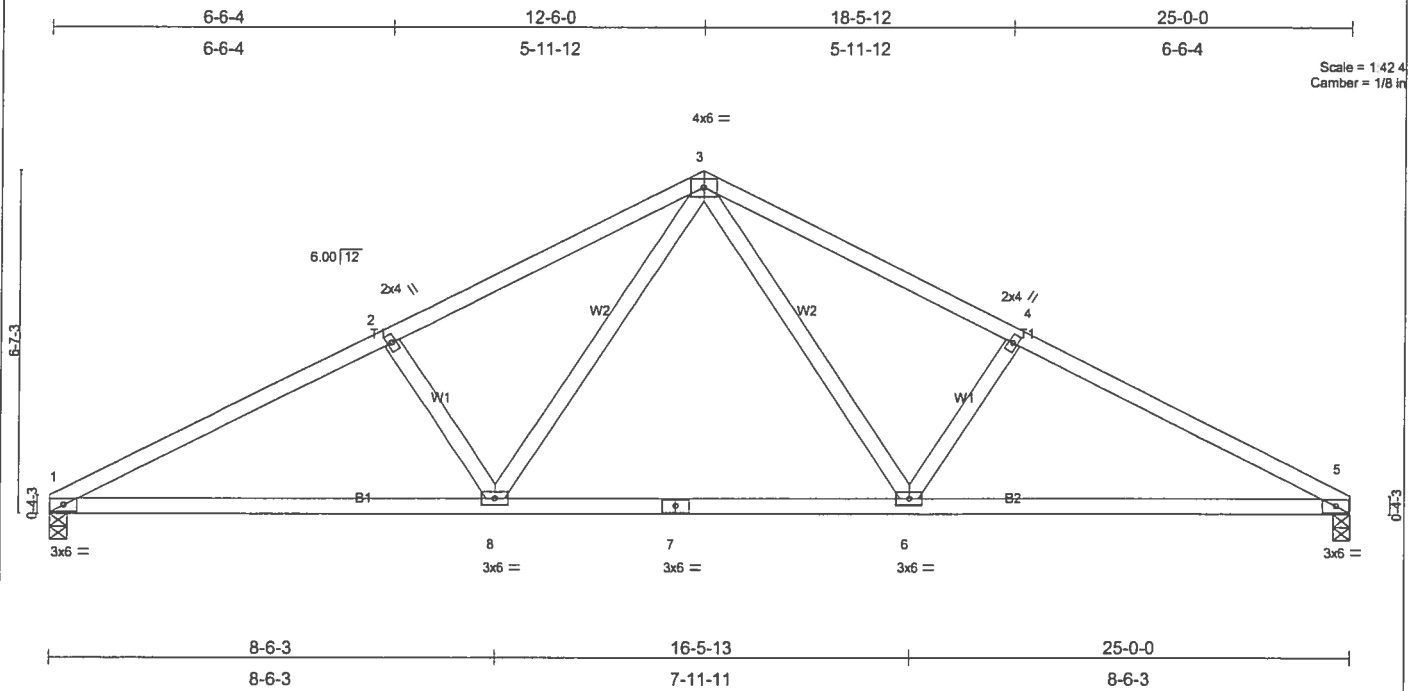


Plate Offsets (X,Y): [1:0-0-0,0-0-0], [2:0-0-0,0-0-0], [3:0-0-0,0-0-0], [4:0-0-0,0-0-0], [5:0-0-0,0-0-0]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.51	Vert(LL) -0.18 5-6 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.26	Vert(TL) -0.31 5-6 >967 180		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) 0.06 5 n/a n/a		
Weight: 111 lb					

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-9-1 oc bracing.

REACTIONS (lb/size) 1=1036/0-4-0, 5=1036/0-4-0
 Max Horz 1=-91(load case 3)
 Max Uplift 1=-337(load case 5), 5=-337(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-1776/799, 2-3=-1605/792, 3-4=-1605/792, 4-5=-1776/799
 BOT CHORD 1-8=-620/1535, 7-8=-311/1031, 6-7=-311/1031, 5-6=-620/1535
 WEBS 2-8=-331/312, 3-8=-266/647, 3-6=-266/647, 4-6=-331/312

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 337 lb uplift at joint 1 and 337 lb uplift at joint 5.

LOAD CASE(S) Standard

Job L155888	Truss T13	Truss Type SPECIAL	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:07:58 2006 Page 1			

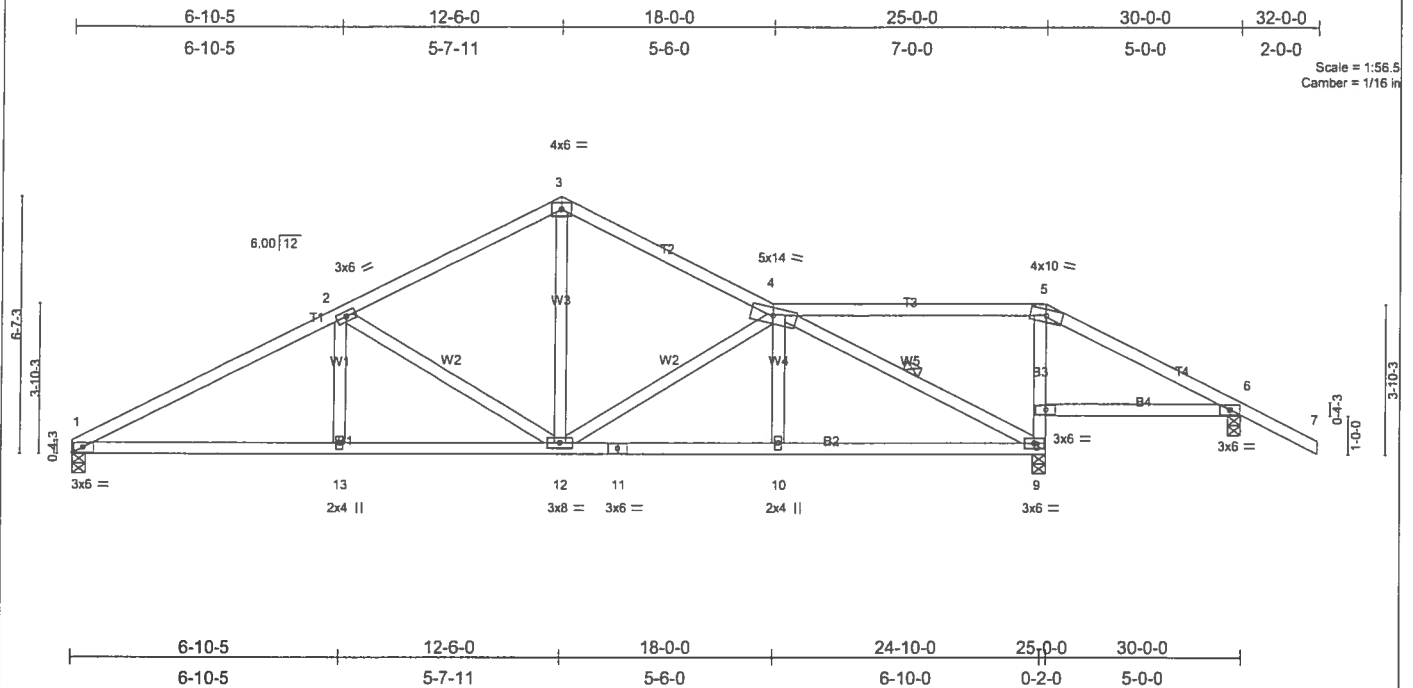


Plate Offsets (X,Y): [1:0-2,12:0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.49	Vert(LL)	-0.12	1-13	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.92	Vert(TL)	-0.20	1-13	>999	180		
BCLL 10.0	Rep Stress Incr	NO	WB 0.50	Horz(TL)	0.06	9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 150 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-8 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 3-11-15 oc bracing.
 WEBS 1 Row at midpt 4-9

REACTIONS

(lb/size) 1=998/0-4-0, 6=419/0-4-0, 9=1703/0-4-0
 Max Horz 1=-122(load case 5)
 Max Uplift 1=-325(load case 4), 6=-337(load case 5), 9=-607(load case 5)
 Max Grav 1=998(load case 1), 6=450(load case 9), 9=1703(load case 1)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1738/543, 2-3=-1160/432, 3-4=-1153/418, 4-5=-15/179, 5-6=-94/324, 6-7=0/47
 BOT CHORD 1-13=473/1479, 12-13=473/1479, 11-12=-325/1317, 10-11=-325/1317, 9-10=-323/1325, 8-9=-829/395, 5-8=-615/298, 6-8=-153/160
 WEBS 2-13=0/225, 2-12=-613/322, 3-12=-206/686, 4-12=-453/271, 4-10=0/198, 4-9=-1694/510

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS gable end zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 325 lb uplift at joint 1, 337 lb uplift at joint 6 and 607 lb uplift at joint 9.
- Girder carries tie-in span(s): 5-5-14 from 25-0-0 to 30-0-0; 4-1-14 from 25-0-0 to 30-0-0
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-4=-54, 4-5=-54, 5-6=-120(F=-66), 6-7=-54, 1-9=-30, 6-8=-68(F=-38)

Job L155888	Truss T14	Truss Type SPECIAL	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Mar 17 14:08:01 2006 Page 1			

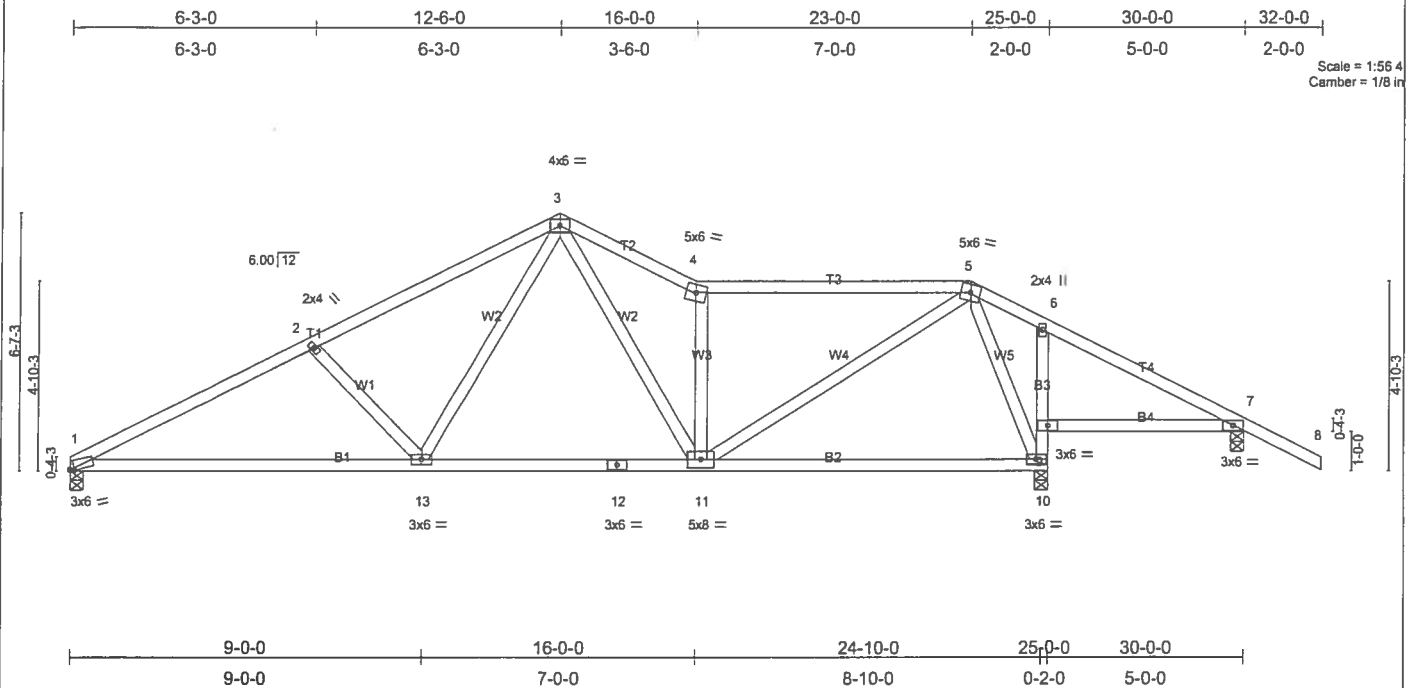


Plate Offsets (X,Y): [1:0-1-5,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	Vert(LL) -0.21	1-13	>999	240		MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.53	Vert(TL) -0.35	1-13	>857	180			
BCLL 10.0	Rep Stress incr YES	WB 0.53	Horz(TL) 0.04	10	n/a	n/a			
BCDL 5.0	Code FBC2004/TP12002	(Matrix)							Weight: 154 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-14 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=1027/0-4-0, 7=302/0-4-0, 10=1280/0-4-0

Max Horiz 1=-104(load case 6)
 Max Uplift 1=-328(load case 5), 7=-281(load case 6), 10=-462(load case 6)
 Max Grav 1=1027(load case 1), 7=327(load case 10), 10=1280(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

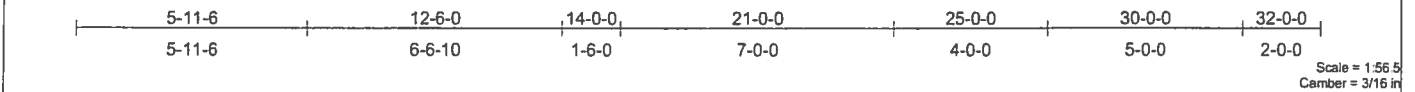
TOP CHORD 1-2=-1760/795, 2-3=-1534/730, 3-4=-1537/794, 4-5=-1307/647, 5-6=-56/83, 6-7=-96/77, 7-8=0/47
 BOT CHORD 1-13=-596/1527, 12-13=-256/1013, 11-12=-256/1013, 10-11=-54/376, 9-10=-214/228, 8-9=-120/122, 7-9=-14/142
 WEBS 2-13=-346/332, 3-13=-237/589, 3-11=-310/706, 4-11=-945/548, 5-11=-439/1116, 5-10=-1010/472

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 1, 281 lb uplift at joint 7 and 462 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L155888	Truss T15	Truss Type SPECIAL	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:04 2006 Page 1			



LOADING (psf)	SPACING 2-0-0	CSI	DEFL In (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.40	Vert(LL) -0.27 10-11 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.90	Vert(TL) -0.46 10-11 >640 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.70	Horz(TL) 0.04 10 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 158 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purtins.
 BOT CHORD Rigid ceiling directly applied or 3-1-0 oc bracing.

REACTIONS (lb/size) 1=1044/0-4-0, 7=386/0-4-0, 10=1179/0-4-0
 Max Horz 1=-104(load case 6)
 Max Uplift 1=-328(load case 5), 7=-276(load case 6), 10=-469(load case 6)
 Max Grav 1=1044(load case 1), 7=404(load case 10), 10=1179(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-1824/799, 2-3=-1651/778, 3-4=-1389/697, 4-5=-1211/608, 5-6=-219/119, 6-7=-258/27, 7-8=0/47
 BOT CHORD 1-13=-605/1585, 12-13=-254/1069, 11-12=-254/1069, 10-11=-96/640, 9-10=-296/317, 6-9=-206/213, 7-9=0/173
 WEBS 2-13=-321/318, 3-13=-285/574, 3-11=-268/756, 4-11=-824/437, 5-11=-267/735, 5-10=-874/437

- NOTES**
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf, BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 3) Provide adequate drainage to prevent water ponding.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 1, 276 lb uplift at joint 7 and 469 lb uplift at joint 10.

LOAD CASE(S) Standard

Job L155888	Truss T16	Truss Type SPECIAL	Qty 1	Ply 1	0 0
Builders FirstSource, Lake City, FL 32055					Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:06 2006 Page 1

6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:06 2006 Page 1

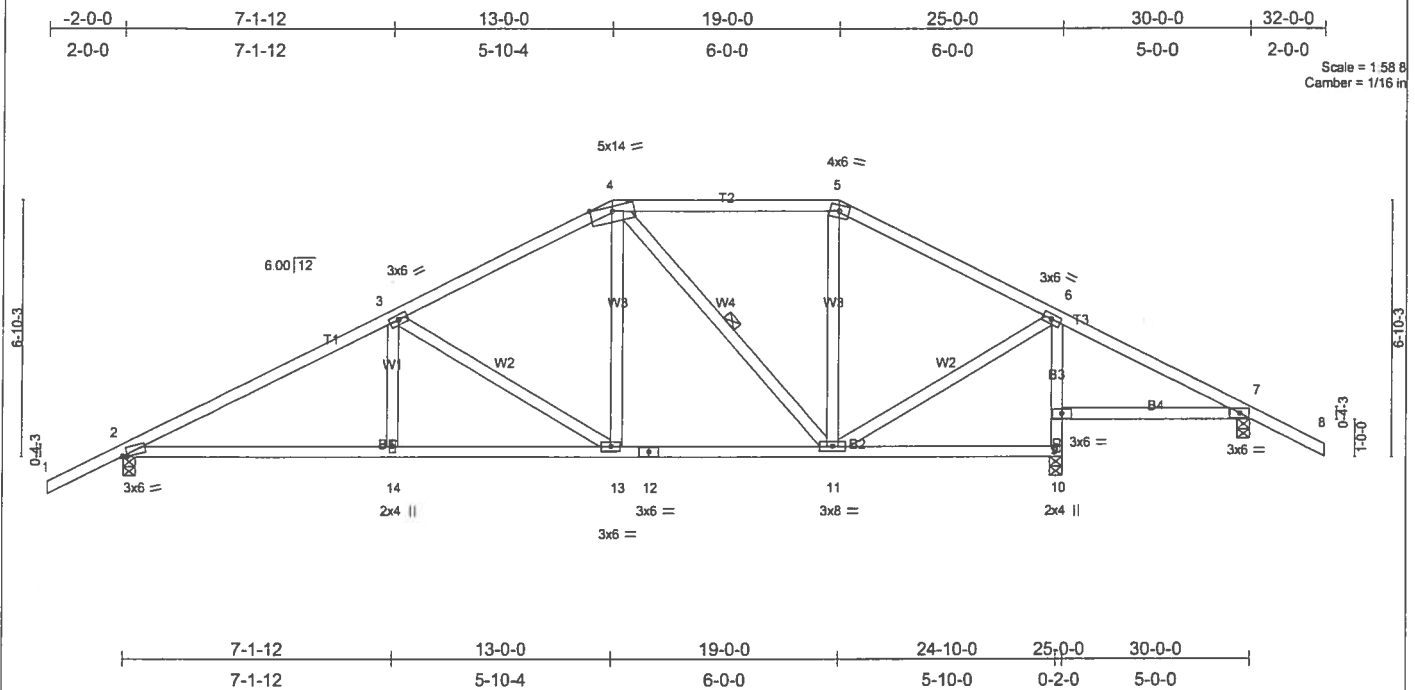


Plate Offsets (X,Y): [2-0-1-8,0-0-7]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL				PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.32	in (loc)	l/defl	L/d	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(LL)	-0.11 2-14	>999	240		
BCLL	10.0	Rep Stress Incr	YES	WB	0.51	Vert(TL)	-0.18 2-14	>999	180		
BCDL	5.0	Code FBC2004/TP12002		(Matrix)		Horz(TL)	0.04 10	n/a	n/a		
									Weight: 161 lb		

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD	2 X 4 SYP No.2 *Except*	BOT CHORD	Rigid ceiling directly applied or 4-10-1 oc bracing.
	B3 2 X 4 SYP No.3	WEBS	1 Row at midpt 4-11
WEBS	2 X 4 SYP No.3		

REACTIONS (lb/size) 2=1145/0-4-0, 10=1296/0-4-0, 7=285/0-4-0
 Max Horz 2=161(load case 5)
 Max Uplift2=461(load case 5), 10=408(load case 5), 7=285(load case 6)
 Max Grav 2=1145(load case 1), 10=1296(load case 1), 7=305(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=1743/700, 3-4=1153/566, 4-5=711/463, 5-6=865/449, 6-7=46/119, 7-8=0/47
BOT CHORD 2-14=488/1476, 13-14=488/1476, 12-13=209/974, 11-12=209/974, 10-11=36/105, 9-10=1211/644, 6-9=1128/547, 7-9=47/119
WEBS 3-14=0/222, 3-13=598/330, 4-13=137/471, 4-11=442/167, 5-11=34/112, 6-11=244/866

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2, 408 lb uplift at joint 10 and 285 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L155888	Truss T17	Truss Type HIP	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Mar 17 14:08:09 2006 Page 1			

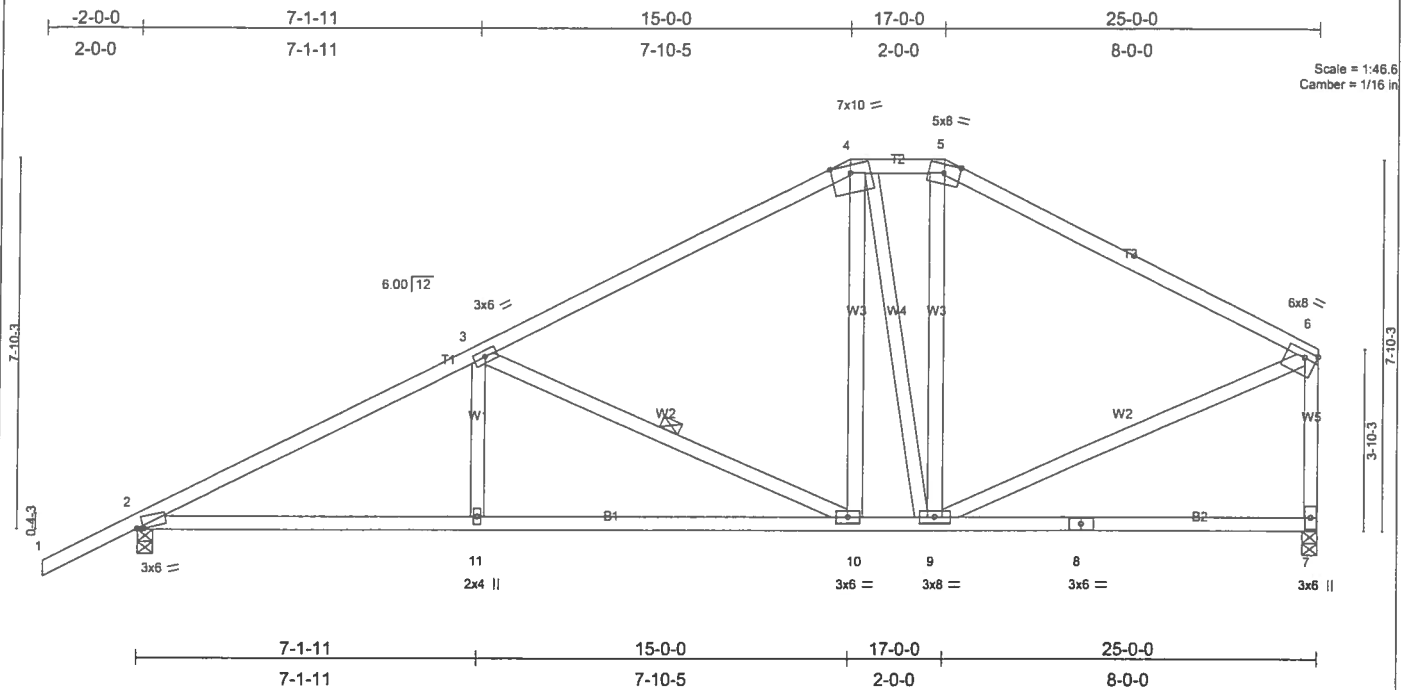


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase 1.25	TC 0.89	Vert(LL)	-0.14 10-11	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.48	Vert(TL)	-0.23 10-11	>999	180		
BCCL 10.0	Rep Stress Incr YES	WB 0.42	Horz(TL)	0.04 7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)						
Weight: 150 lb								

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-14 oc bracing.
WEBS 1 Row at midpt 3-10

REACTIONS

(lb/size) 2=1159/0-4-0, 7=1032/0-4-0
Max Horz 2=284(load case 5)
Max Uplift 2=467(load case 5), 7=323(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=1803/703, 3-4=1039/494, 4-5=784/497, 5-6=969/457, 6-7=907/467
BOT CHORD 2-11=707/1534, 10-11=707/1534, 9-10=328/849, 8-9=64/109, 7-8=64/109
WEBS 3-11=0/253, 3-10=763/419, 4-10=160/446, 4-9=408/186, 5-9=73/321, 6-9=255/737

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 467 lb uplift at joint 2 and 323 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L155888	Truss T18	Truss Type COMMON	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:11 2006 Page 1			

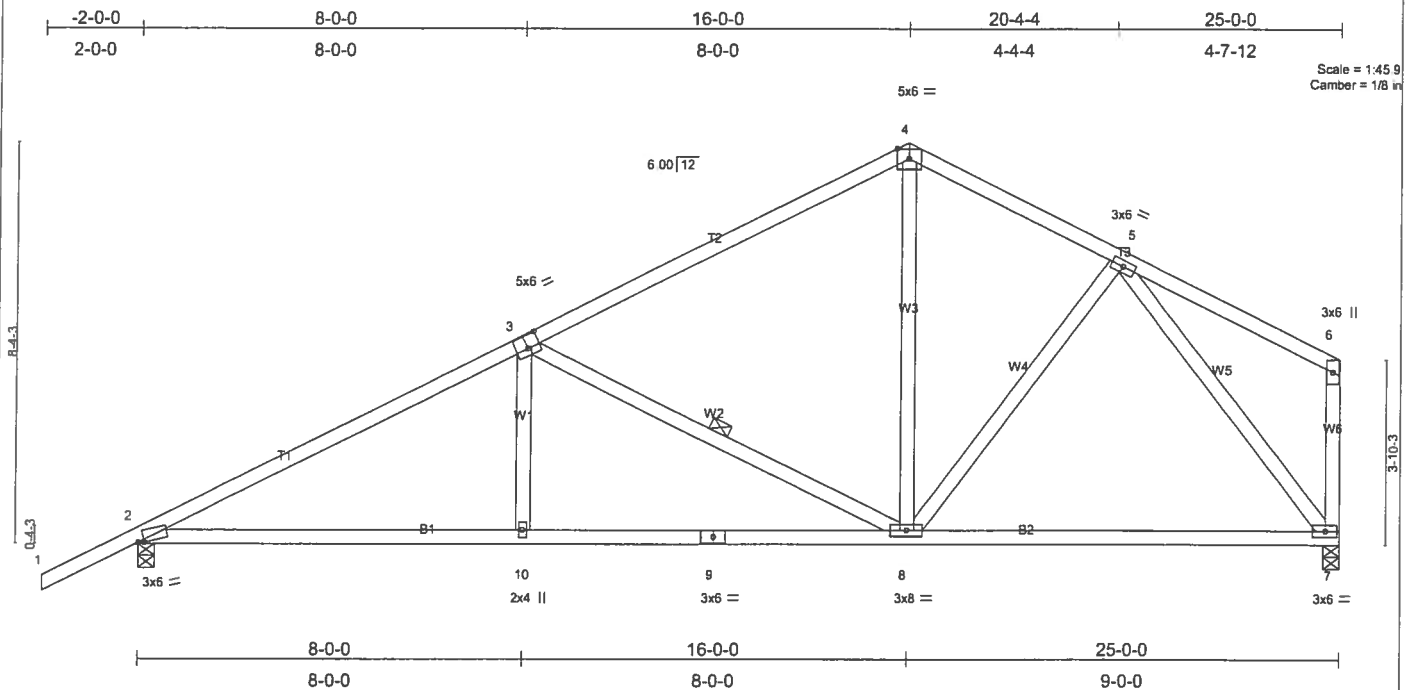


Plate Offsets (X,Y): [2:0-1-9:0-0-7], [3:0-3-0,0-3-4]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	Vdefl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.59	Vert(LL)	-0.15	2-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.50	Vert(TL)	-0.25	2-10	>999	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.85	Horz(TL)	0.05	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 138 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-1 oc bracing.
WEBS 1 Row at midpt 3-8

REACTIONS

(lb/size) 2=1159/0-4-0, 7=1032/0-4-0
Max Horz 2=291(load case 5)
Max Uplift 2=471(load case 5), 7=312(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=1744/691, 3-4=970/476, 4-5=905/498, 5-6=140/83, 6-7=168/126
BOT CHORD 2-10=685/1474, 9-10=685/1473, 8-9=685/1473, 7-8=279/628
WEBS 3-10=0/258, 3-8=788/449, 4-6=141/417, 5-8=55/309, 5-7=921/443

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 471 lb uplift at joint 2 and 312 lb uplift at joint 7.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	00
L155888	T19	HIP	1	1	
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:14 2006 Page 1

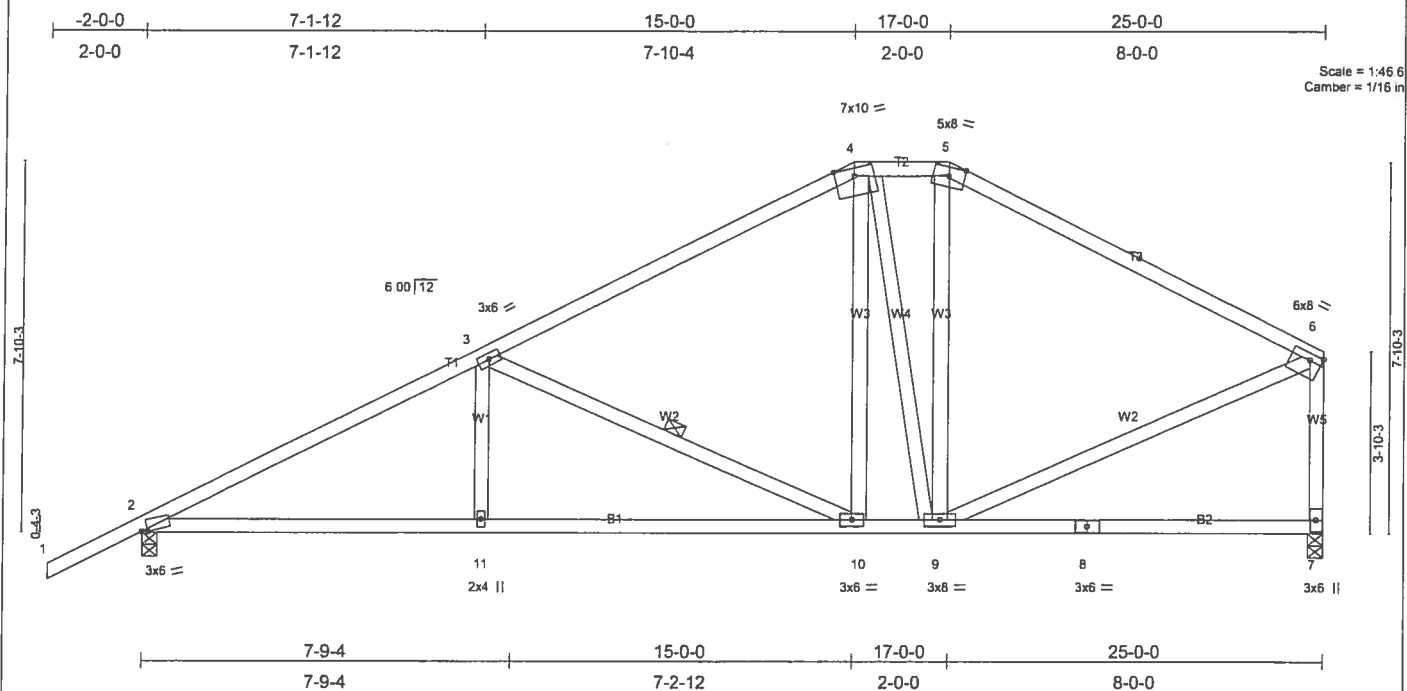


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:Edge,0-1-12]													
LOADING (psf)		SPACING 2-0-0		CSI		DEFL				PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.89	in (loc)	l/defl	L/d		MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.14 10-11	>999	240				
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Vert(TL)	-0.23 10-11	>999	180				
BCDL	5.0	Code FBC2004/TP12002		(Matrix)		Horz(TL)	0.04 7	n/a	n/a				
										Weight: 150 lb			

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 4-4-10 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 7-5-15 oc bracing.
WEBS	1 Row at midpt 3-10

REACTIONS (lb/size) 2=1159/0-4-0, 7=1032/0-4-0
Max Horiz 2=284(load case 5)
Max Uplift2=467(load case 5), 7=323(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/47, 2-3=-1802/702, 3-4=-1039/494, 4-5=-784/497, 5-6=-969/457, 6-7=-907/467
BOT CHORD	2-11=0/753, 11-10=770/1534, 9-10=-328/849, 8-9=-64/109, 7-8=-64/109
WEBS	3-11=0/253, 3-10=-762/419, 4-10=-161/446, 4-9=-408/186, 5-9=-73/321, 6-9=-255/737

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20ft$; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 467 lb uplift at joint 2 and 323 lb uplift at joint 7.

LOAD CASE(S) Standard

MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L155888	Truss T20	Truss Type HIP	Qty 1	Ply 1	00	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:17 2006 Page 1			

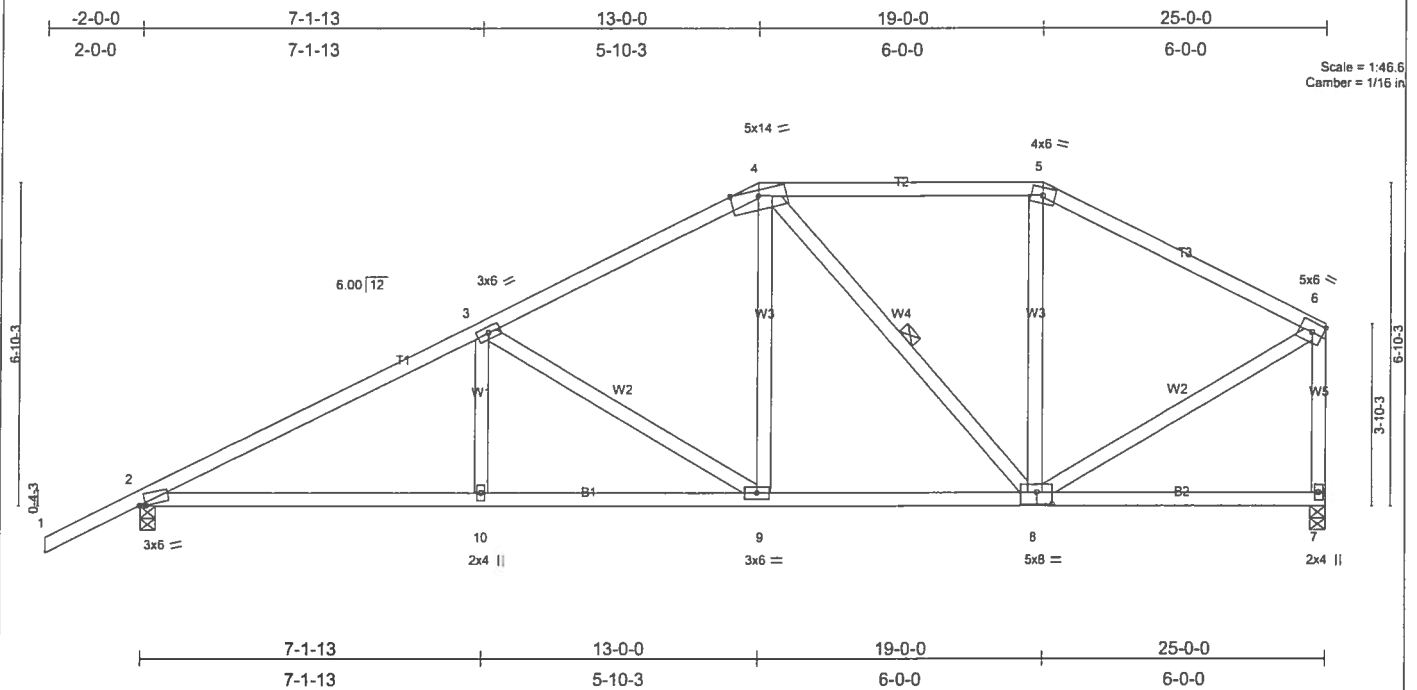


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [8:0-3-12,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.50	Vert(LL)	-0.11	2-10	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.42	Vert(TL)	-0.18	2-10	>999	180		
BCLL 10.0	Rep Stress Incr	YES	WB 0.51	Horz(TL)	0.04	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TP/2002		(Matrix)							
Weight: 142 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-8-2 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS

(lb/size) 2=1159/0-4-0, 7=1032/0-4-0
Max Horz 2=270(load case 5)
Max Uplift 2=-459(load case 5), 7=-289(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1773/678, 3-4=-1183/544, 4-5=-748/435, 5-6=-902/412, 6-7=-944/452
BOT CHORD 2-10=-680/1502, 9-10=-680/1502, 8-9=-400/1001, 7-8=-33/52
WEBS 3-10=0/222, 3-9=-598/330, 4-9=-140/473, 4-8=-433/170, 5-8=-33/121, 6-8=-297/813

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02: 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 459 lb uplift at joint 2 and 289 lb uplift at joint 7.

LOAD CASE(S) Standard

Job L155888	Truss T21	Truss Type HIP	Qty 1	Ply 1	00
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:20 2006 Page 1		

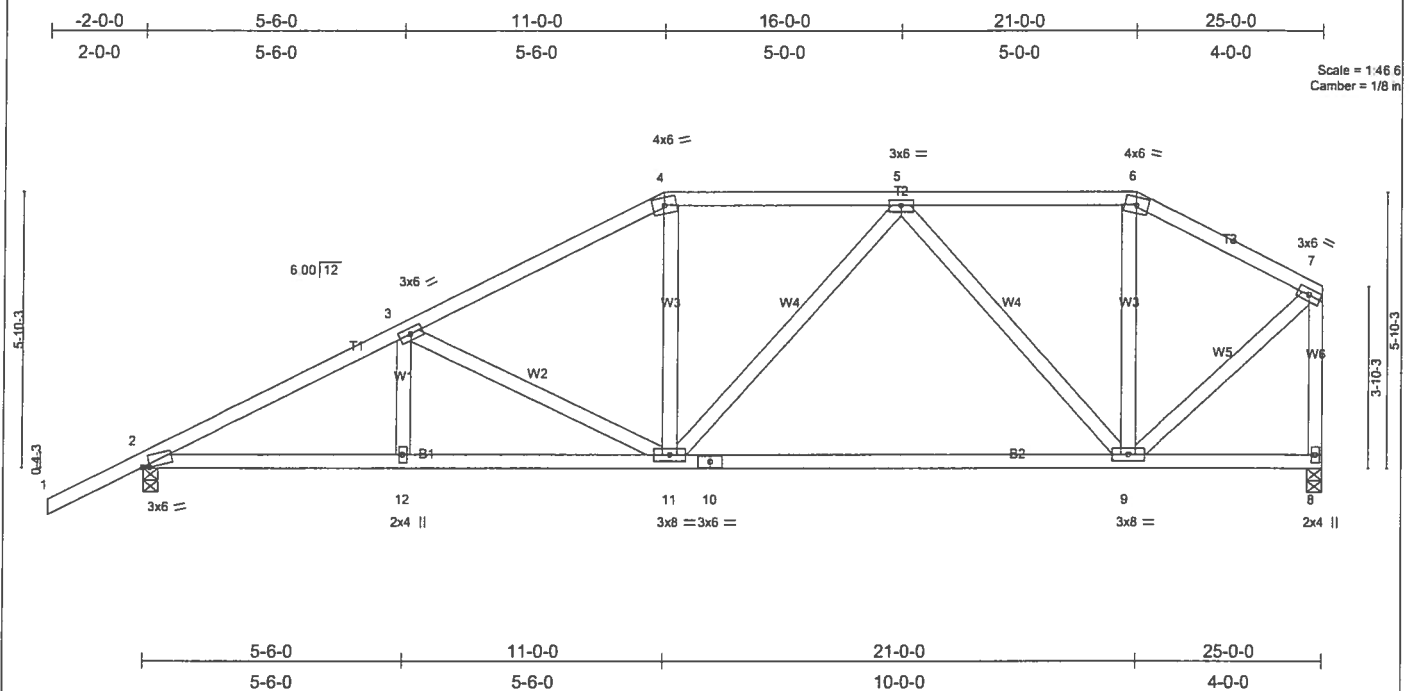


Plate Offsets (X,Y): [2:0-1-13:0-0-7]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES	
TCLL	20.0	Plates Increase	1.25	TC	0.30	in	(loc)	l/defl	L/d
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(LL)	-0.21	9-11	>999
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Vert(TL)	-0.36	9-11	>833
BCDL	5.0	Code FBC2004/TP12002		(Matrix)		Horz(TL)	0.04	8	n/a
								GRIP	
								MT20	
								244/190	
								Weight: 142 lb	

LUMBER	
TOP CHORD 2 X 4 SYP No.2	
BOT CHORD 2 X 4 SYP No.2	
WEBS 2 X 4 SYP No.3	
	BRACING
	TOP CHORD Structural wood sheathing directly applied or 4-6-1 oc purlins, except end verticals.
	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1159/0-4-0, 8=1032/0-4-0
Max Horz 2=256(load case 5)
Max Uplift 2=-448(load case 5), 8=-270(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1840/691, 3-4=-1379/575, 4-5=-1182/576, 5-6=-673/345, 6-7=-788/334, 7-8=-1021/428
BOT CHORD 2-12=-712/1575, 11-12=-712/1575, 10-11=-439/1042, 9-10=-439/1042, 8-9=-15/7
WEBS 3-12=0/137, 3-11=-448/263, 4-11=-28/988, 5-11=-80/225, 5-9=-587/294, 6-9=0/136, 7-9=-313/893

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); $h=20ft$; $TCLE=4.2psf$; $BCDL=3.0psf$; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber $DOL=1.60$ plate grip $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 448 lb uplift at joint 2 and 270 lb uplift at joint 8.

LOAD CASE(S) Standard

MARCH 17, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job L155888	Truss T23	Truss Type MONO HIP	Qty 1	Ply 1	0 0	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Mar 17 14:08:24 2006 Page 1			

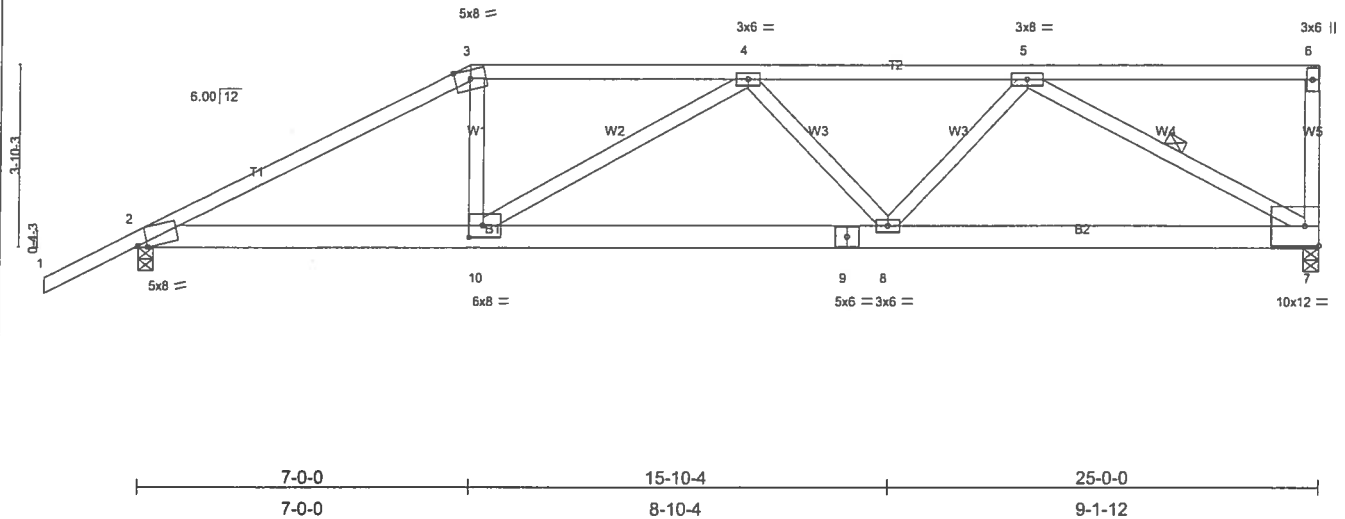
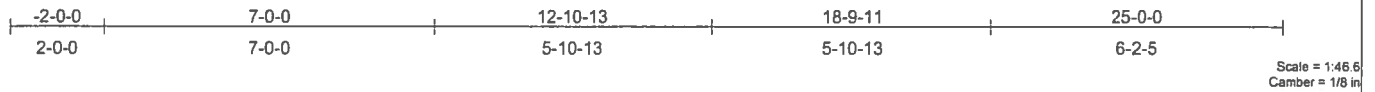


Plate Offsets (X,Y): [2-0-2-7, Edge], [10-0-3-8, 0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.67	Vert(LL)	-0.24	8-10	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.40	8-10	>748	180	244/190
BCLL 10.0	Rep Stress Incr	NO	WB 0.91	Horz(TL)	0.08	7	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 142 lb

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3 *Except*
 W5 2 X 4 SYP No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-8-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-2-0 oc bracing.
 WEBS 1 Row at midpt 5-7

REACTIONS (lb/size) 7=2350/0-4-0, 2=2203/0-4-0
 Max Horz 2=228(load case 4)
 Max Uplift 7=969(load case 3), 2=886(load case 4)

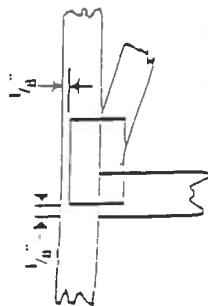
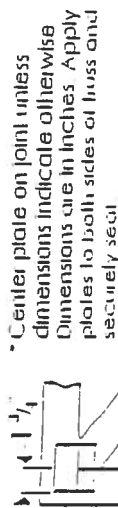
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/51, 2-3=4154/1574, 3-4=3693/1464, 4-5=3986/1525, 5-6=173/70, 6-7=357/261
 BOT CHORD 2-10=1449/3637, 9-10=1813/4288, 8-9=1813/4288, 7-8=1366/3148
 WEBS 3-10=383/1299, 4-10=696/466, 4-8=467/444, 5-8=245/1294, 5-7=3430/1495

NOTES
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
 2) Provide adequate drainage to prevent water ponding.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 969 lb uplift at joint 7 and 886 lb uplift at joint 2.
 4) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 240 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-6=-120(F=-66), 2-10=-30, 7-10=-67(F=-37)
 Concentrated Loads (lb)
 Vert: 10=539(F)

Symbols

PLATE LOCATION AND ORIENTATION



* This symbol indicates the required direction of slots in connector plates



PLATE SIZE

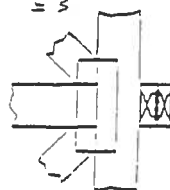
$L \times B$
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



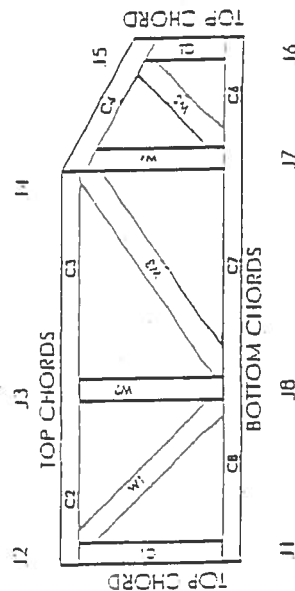
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DIII HR	960022-W, 970036-11
IIR	561



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (1.6' from adjacent joint).
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size and in all respects equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft spacing, or less. If no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.