| | | | |
|---|---|---|---|
| **Project Information for:** | L169926 | | |
| Builder: | PRUDENTIAL BLRDS | Date: | 6/16/2006 |
| Lot: | Lot 45 PRICE CREEK LANDING | Start Number: | **1197** |
| Subdivision: | N/A | | |
| County or City: | COLUMBIA COUNTY | | |
| Truss Page Count: | 28 | | |

**Truss Design Load Information (UNO)** Design Program: MiTek 5.2 / 6.2

| **Gravity** | | **Wind** | | **Building Code:** | **FBC2004** |
|---|---|---|---|---|---|
| Roof (psf): | **42** | Wind Standard: | **ASCE 7-02** | | |
| Floor (psf): | 55 | Wind Speed (mph): | **110** | | |

Note: See individual truss drawings for special loading conditions

**Building Designer, responsible for Structural Engineering: (See attached)**

FITZHUGH, JUSTIN M. CRC1328401
Address: P.O. BOX 3333
LAKE CITY, FL 32056 Designer: **39**

**Truss Design Engineer:** Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

| # | Truss ID | Dwg. # | Seal Date | # | Truss ID | Dwg. # | Seal Date |
|---|---|---|---|---|---|---|---|
| 1 | CJ1 | 0616061197 | 6/16/2006 | | | | |
| 2 | CJ3 | 0616061198 | 6/16/2006 | | | | |
| 3 | CJ5 | 0616061199 | 6/16/2006 | | | | |
| 4 | EJ4 | 0616061200 | 6/16/2006 | | | | |
| 5 | EJ7 | 0616061201 | 6/16/2006 | | | | |
| 6 | EJ7A | 0616061202 | 6/16/2006 | | | | |
| 7 | HJ5 | 0616061203 | 6/16/2006 | | | | |
| 8 | HJ9 | 0616061204 | 6/16/2006 | | | | |
| 9 | T01G | 0616061205 | 6/16/2006 | | | | |
| 10 | T02 | 0616061206 | 6/16/2006 | | | | |
| 11 | T03G | 0616061207 | 6/16/2006 | | | | |
| 12 | T04 | 0616061208 | 6/16/2006 | | | | |
| 13 | T04G | 0616061209 | 6/16/2006 | | | | |
| 14 | T05 | 0616061210 | 6/16/2006 | | | | |
| 15 | T06 | 0616061211 | 6/16/2006 | | | | |
| 16 | T07 | 0616061212 | 6/16/2006 | | | | |
| 17 | T08 | 0616061213 | 6/16/2006 | | | | |
| 18 | T09 | 0616061214 | 6/16/2006 | | | | |
| 19 | T10 | 0616061215 | 6/16/2006 | | | | |
| 20 | T11 | 0616061216 | 6/16/2006 | | | | |
| 21 | T12 | 0616061217 | 6/16/2006 | | | | |
| 22 | T13 | 0616061218 | 6/16/2006 | | | | |
| 23 | T14 | 0616061219 | 6/16/2006 | | | | |
| 24 | T15 | 0616061220 | 6/16/2006 | | | | |
| 25 | T16 | 0616061221 | 6/16/2006 | | | | |
| 26 | T17 | 0616061222 | 6/16/2006 | | | | |
| 27 | T18 | 0616061223 | 6/16/2006 | | | | |
| 28 | T19 | 0616061224 | 6/16/2006 | | | | |

JUN 16 2006

Dwg.#0616061196




## Licensee Details

### Licensee Information

| | |
|---|---|
| Name: | **FITZHUGH, JUSTIN M (Primary Name)** |
| | **PRUDENTIAL BUILDERS INC (DBA Name)** |
| Main Address: | **P.O. BOX 3333** |
| | **LAKE CITY Florida 32056** |
| County: | **COLUMBIA** |
| License Mailing: | |
| LicenseLocation: | |

### License Information

| | |
|---|---|
| License Type: | **Certified Residential Contractor** |
| Rank: | **Cert Residental** |
| License Number: | **CRC1328401** |
| Status: | **Current,Active** |
| Licensure Date: | **12/23/2005** |
| Expires: | **08/31/2008** |

| | |
|---|---|
| **Special Qualifications** | **Qualification Effective** |
| **Qualified Business License Required** | **02/06/2006** |

View Related License Information
View License Complaint

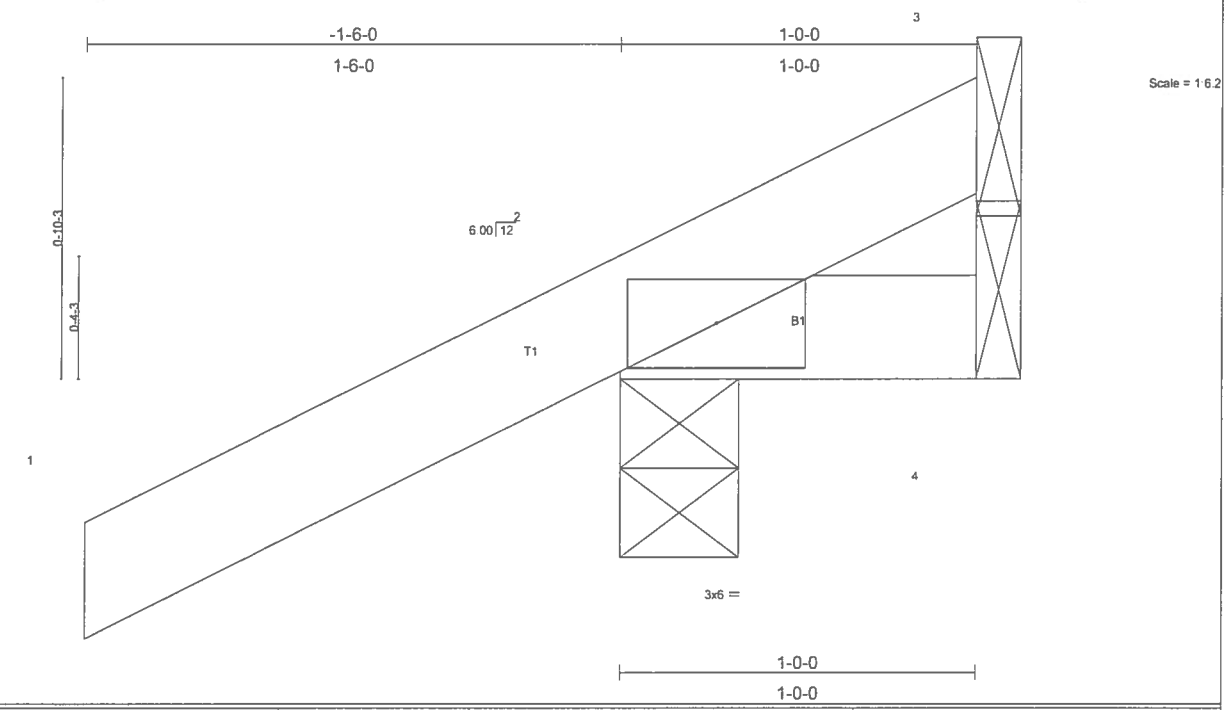


JUNE 16, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0616061197

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | CJ1 | JACK | 8 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:53 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.15 | Vert(LL) | -0.00 | 2 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.01 | Vert(TL) | -0.00 | 2 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | 0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 6 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=189/0-4-0, 4=14/Mechanical, 3=-41/Mechanical
Max Horz 2=70(load case 5)
Max Uplift2=-193(load case 5), 4=-9(load case 3), 3=-41(load case 1)
Max Grav 2=189(load case 1), 4=14(load case 1), 3=61(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/35
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 2, 9 lb uplift at joint 4 and 41 lb uplift at joint 3.

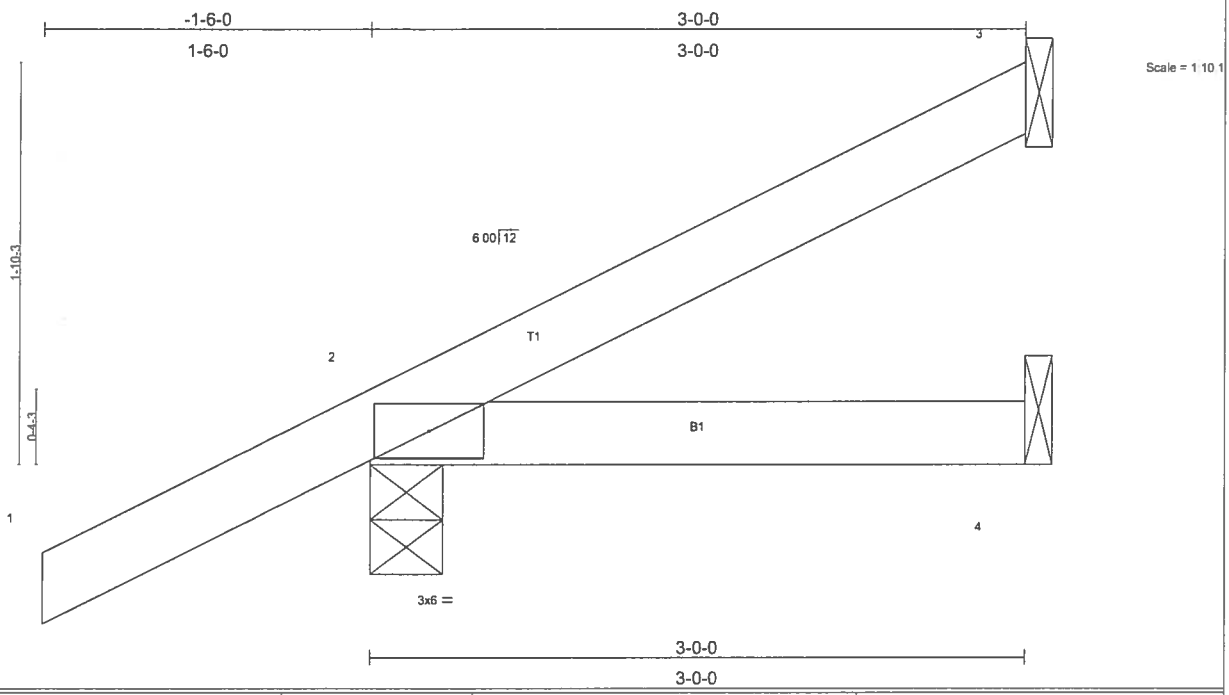
**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061198

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | CJ3 | JACK | 8 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:54 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.17 | Vert(LL) | 0.01 | 2-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.08 | Vert(TL) | 0.01 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 12 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=48/Mechanical, 2=233/0-4-0, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-37(load case 5), 2=-187(load case 5), 4=-26(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-49/16
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.11

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3, 187 lb uplift at joint 2 and 26 lb uplift at joint 4.

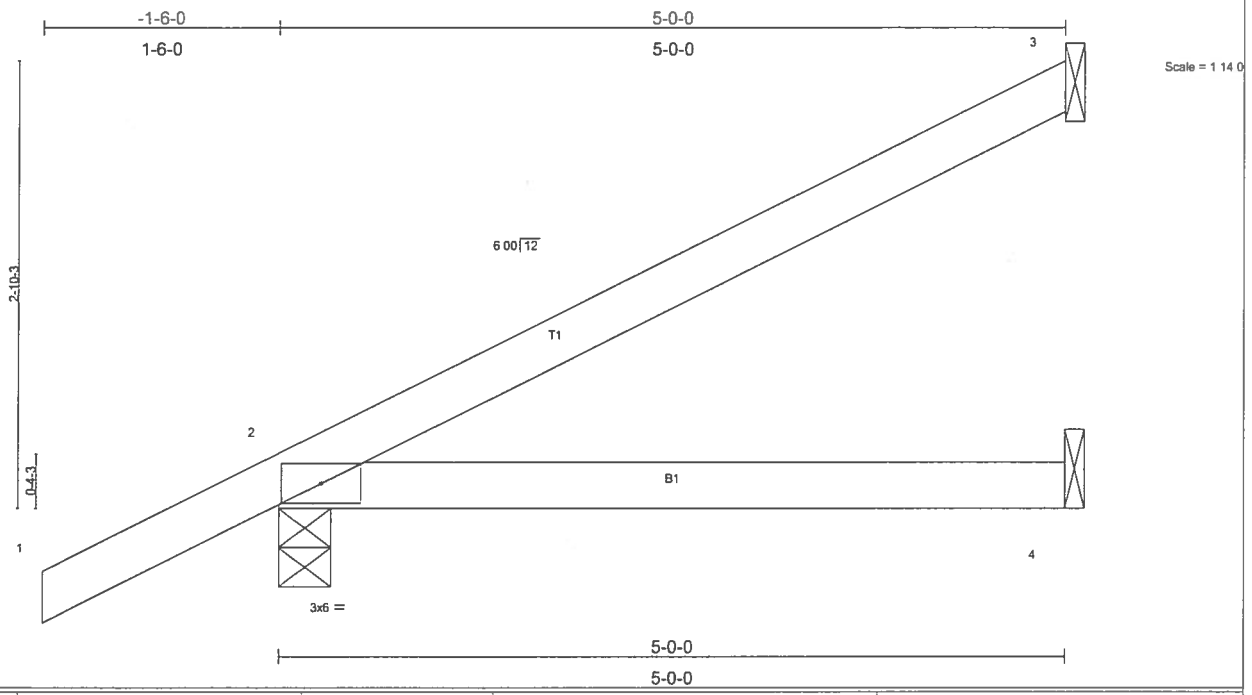
**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061199

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | CJ5 | JACK | 4 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:55 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.24 | Vert(LL) | -0.03 | 2-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.16 | Vert(TL) | -0.05 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 18 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=113/Mechanical, 2=306/0-4-0, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-159(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-96/41
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.13

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 159 lb uplift at joint 2.

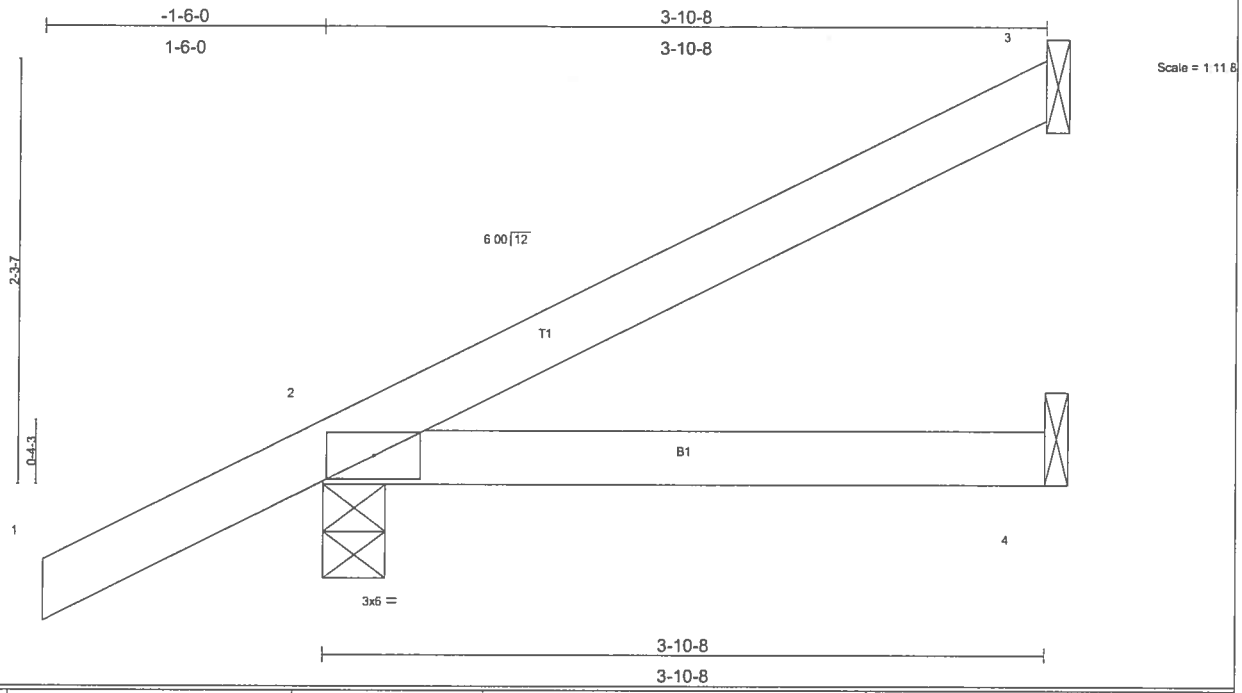
**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061200

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | EJ4 | JACK | 7 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:56 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.17 | Vert(LL) | 0.03 | 2-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.14 | Vert(TL) | 0.02 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 15 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-10-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=78/Mechanical, 2=264/0-4-0, 4=55/Mechanical
Max Horz 2=136(load case 5)
Max Uplift3=-67(load case 5), 2=-199(load case 5), 4=-35(load case 3)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-68/27
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.11

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 3, 199 lb uplift at joint 2 and 35 lb uplift at joint 4.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061201

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | EJ7 | MONO TRUSS | 23 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:57 2006 Page 1

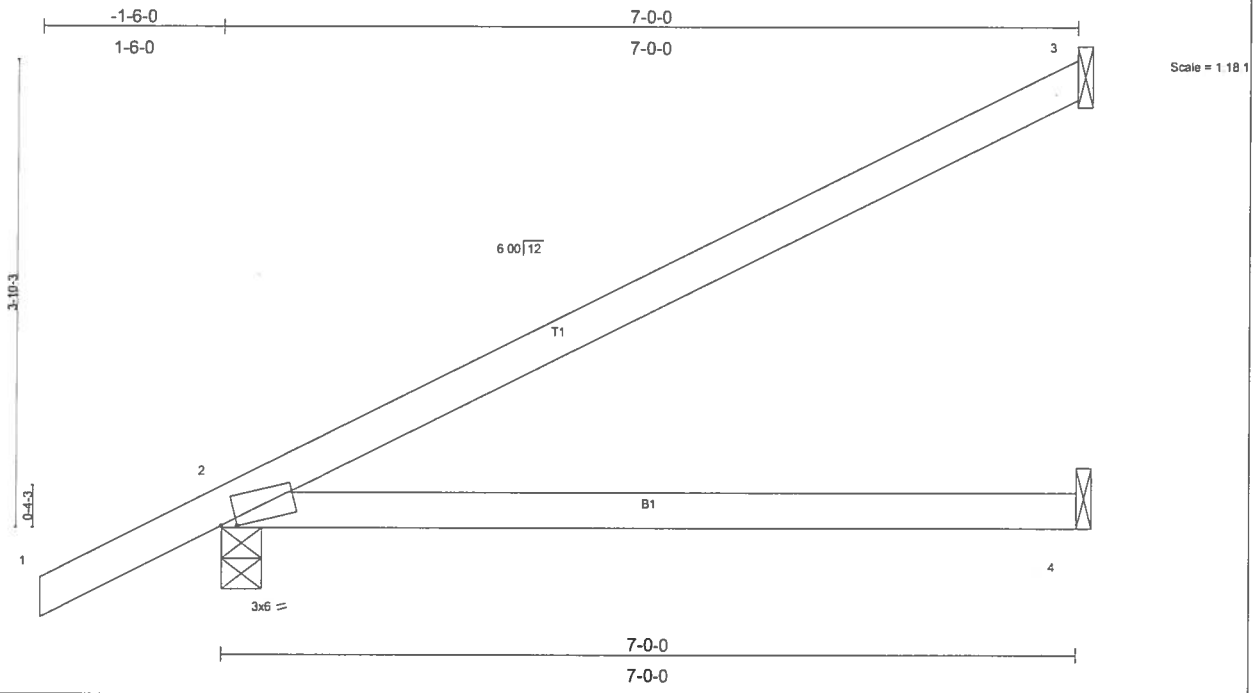



Plate Offsets (X,Y) [2:0-1-9,0-0-7]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.46 | Vert(LL) | -0.13 | 2-4 | >614 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.37 | Vert(TL) | -0.22 | 2-4 | >370 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 25 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=165/Mechanical, 2=385/0-4-0, 4=108/Mechanical
Max Horz 2=208(load case 5)
Max Uplift3=-138(load case 5), 2=-173(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-122/59
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.91

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 138 lb uplift at joint 3 and 173 lb uplift at joint 2.

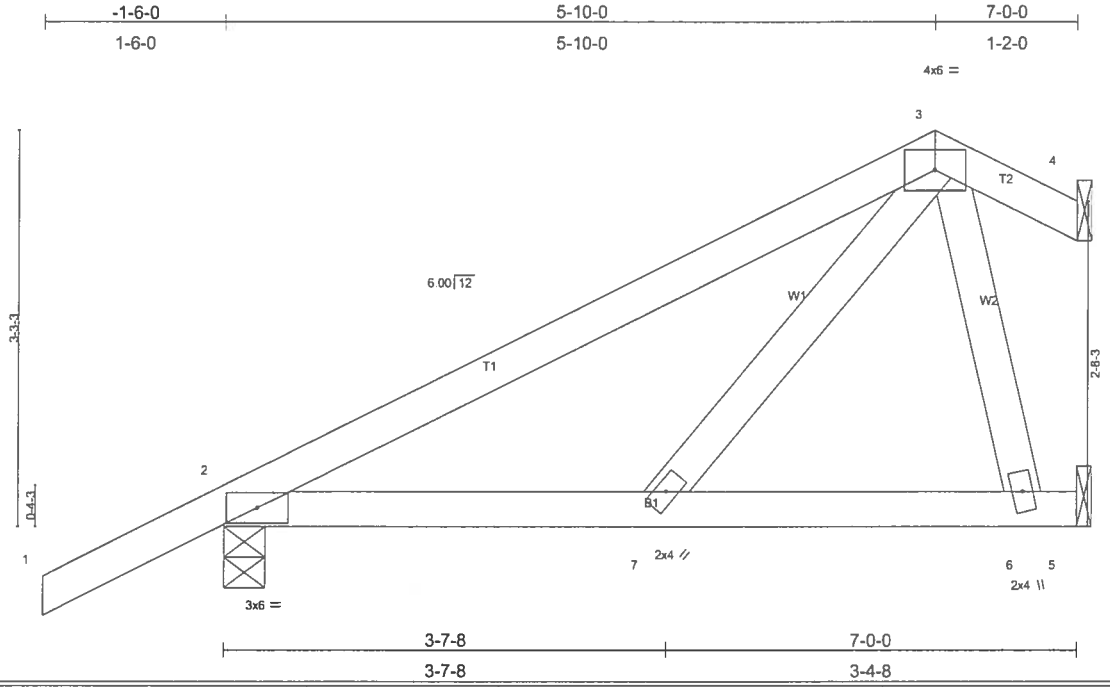
**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061202

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | EJ7A | COMMON | 3 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:57 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.22 | Vert(LL) | 0.02 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.23 | Vert(TL) | -0.03 | 6-7 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.09 | Horz(TL) | 0.01 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 34 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 4=-67/Mechanical, 2=385/0-4-0, 5=340/Mechanical
Max Horz 2=165(load case 5)
Max Uplift4=-89(load case 9), 2=-191(load case 5), 5=-178(load case 5)
Max Grav 4=76(load case 5), 2=385(load case 1), 5=340(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-345/91, 3-4=-45/56
BOT CHORD 2-7=-129/241, 6-7=-86/88, 5-6=0/0
WEBS 3-7=-68/247, 3-6=-355/348

**JOINT STRESS INDEX**
2 = 0.47, 3 = 0.65, 6 = 0.19 and 7 = 0.16

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 4, 191 lb uplift at joint 2 and 178 lb uplift at joint 5.

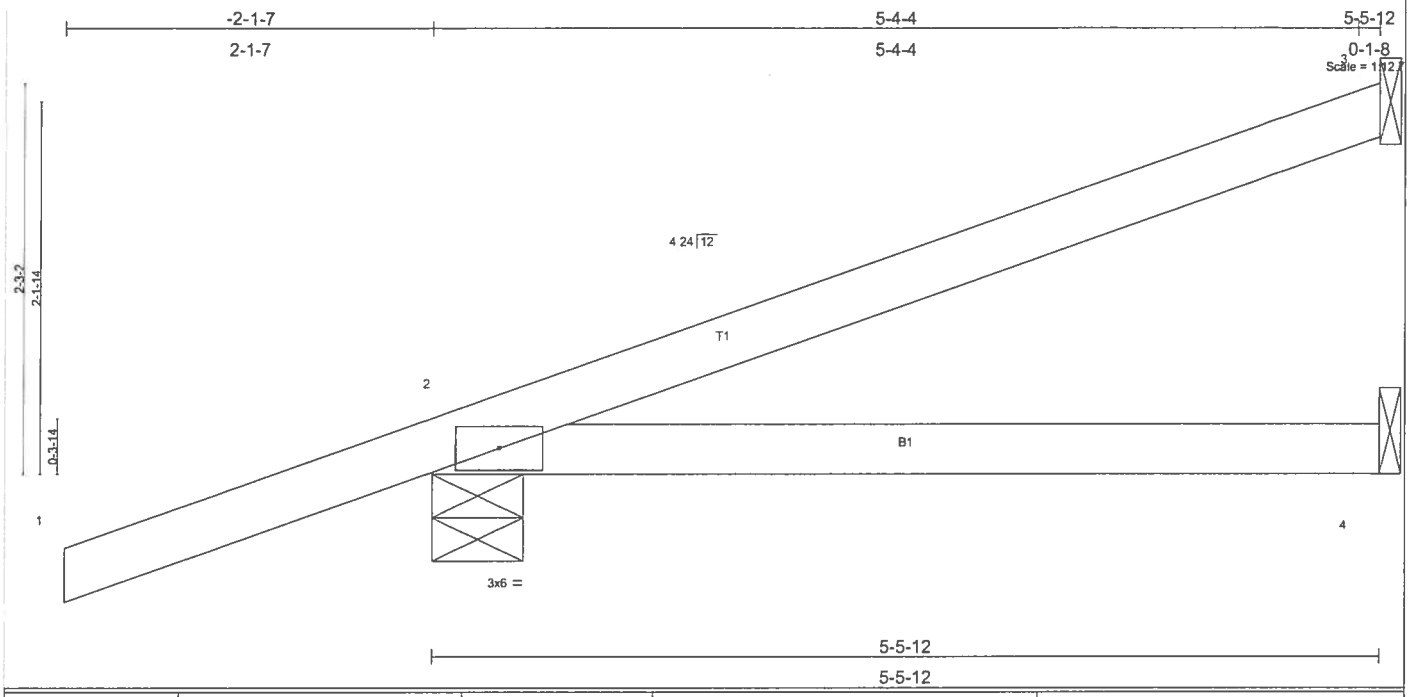
**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061203

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | HJ5 | ROOF TRUSS | 2 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:16:58 2006 Page 1




| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.33 | Vert(LL) | 0.05 | 2-4 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.15 | Vert(TL) | -0.04 | 2-4 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.00 | Horz(TL) | -0.00 | 3 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 20 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-5-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 3=100/Mechanical, 2=264/0-6-7, 4=71/Mechanical
Max Horz 2=108(load case 2)
Max Uplift3=-71(load case 2), 2=-252(load case 2), 4=-50(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-36/23
BOT CHORD 2-4=0/0

**JOINT STRESS INDEX**
2 = 0.10

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 3, 252 lb uplift at joint 2 and 50 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-3=-74(F=-10, B=-10), 2=0(F=15, B=15)-to-4=-41(F=-6, B=-6)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061204

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | HJ9 | ROOF TRUSS | 2 | 1 | Job Reference (optional) |

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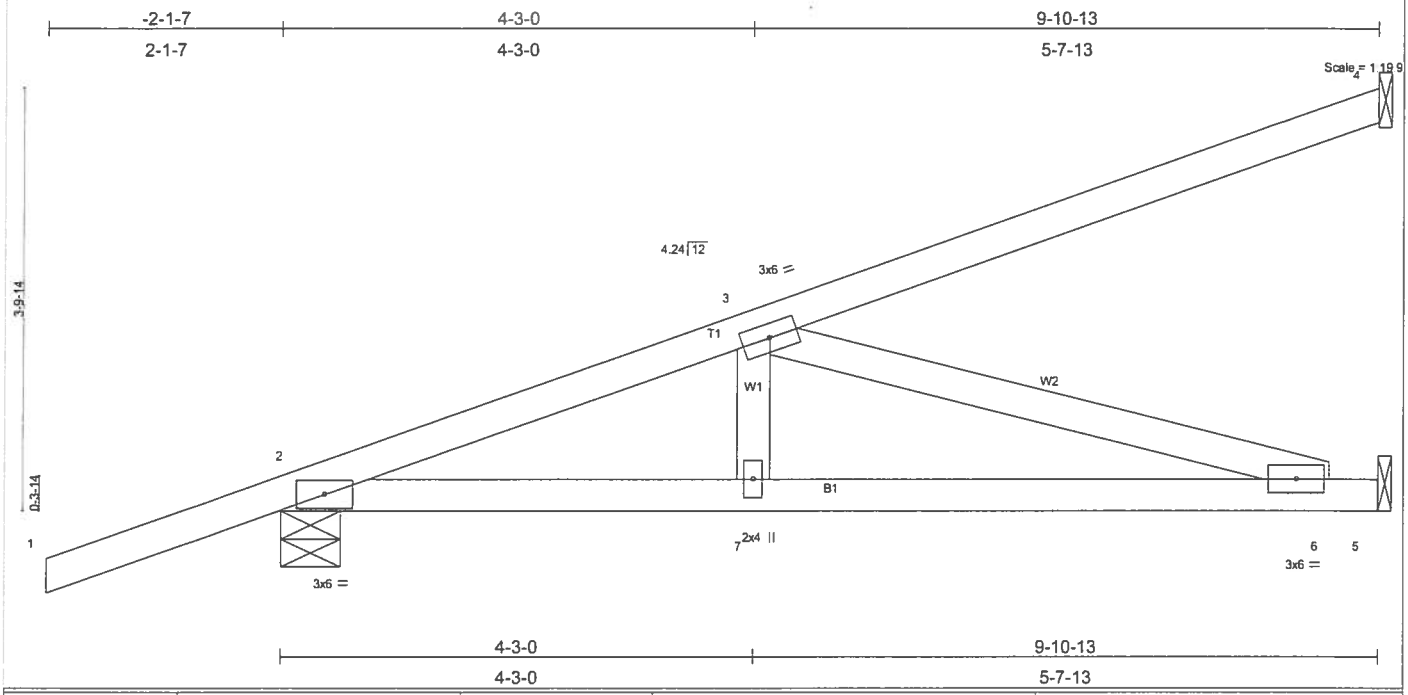



| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.61 | Vert(LL) | -0.11 | 6-7 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.62 | Vert(TL) | -0.18 | 6-7 | >627 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.49 | Horz(TL) | 0.01 | 5 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 43 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-11 oc bracing.

**REACTIONS** (lb/size) 4=268/Mechanical, 2=488/0-6-7, 5=385/Mechanical
Max Horz 2=253(load case 2)
Max Uplift4=-230(load case 2), 2=-275(load case 4), 5=-76(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/38, 2-3=-929/177, 3-4=-104/65
BOT CHORD 2-7=-367/864, 6-7=-367/864, 5-6=0/0
WEBS 3-7=0/206, 3-6=-900/383

**JOINT STRESS INDEX**
2 = 0.63, 3 = 0.24, 6 = 0.25 and 7 = 0.15

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 275 lb uplift at joint 2 and 76 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

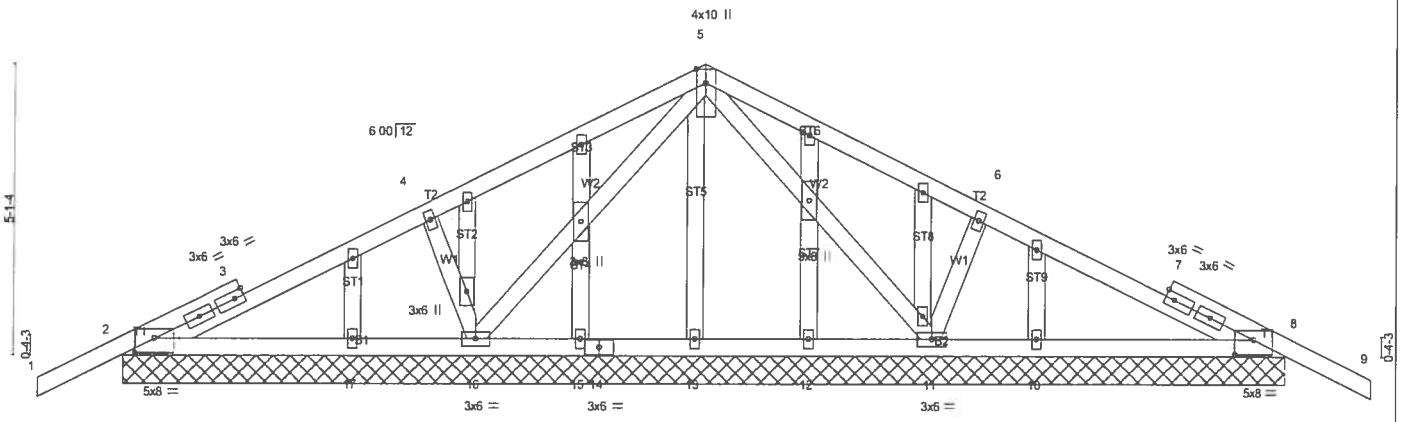
Dwg.#0616061205

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T01G | COMMON | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:17:01 2006 Page 1

-1-6-0 | 5-4-4 | 10-2-0 | 14-11-12 | 20-4-0 | 21-10-0
1-6-0 | 5-4-4 | 4-9-12 | 4-9-12 | 5-4-4 | 1-6-0

Scale = 1 38 6




6-2-0 | 14-2-0 | 20-4-0
6-2-0 | 8-0-0 | 6-2-0

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [8:0-4-0,0-3-1]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.26 | Vert(LL) | 0.01 | 9 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.12 | Vert(TL) | 0.02 | 9 | n/r | 90 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.22 | Horz(TL) | 0.00 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 126 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 2=271/20-4-0, 8=275/20-4-0, 16=542/20-4-0, 11=540/20-4-0, 17=137/20-4-0, 15=67/20-4-0, 13=55/20-4-0, 12=74/20-4-0, 10=143/20-4-0
Max Horz 2=-95(load case 6)
Max Uplift2=-155(load case 5), 8=-168(load case 6), 16=-303(load case 5), 11=-289(load case 6)
Max Grav 2=277(load case 9), 8=281(load case 10), 16=542(load case 1), 11=540(load case 1), 17=137(load case 1), 15=67(load case 1), 13=55(load case 10), 12=74(load case 1), 10=143(load case 1)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-73/55, 3-4=-66/143, 4-5=-33/248, 5-6=-15/252, 6-7=-38/147, 7-8=-45/59, 8-9=0/41
BOT CHORD 2-17=-57/151, 16-17=-57/151, 15-16=0/143, 14-15=0/143, 13-14=0/143, 12-13=0/143, 11-12=0/143, 10-11=-60/139, 8-10=-60/139
WEBS 4-16=-316/265, 5-16=-298/141, 5-11=-304/141, 6-11=-318/265

**JOINT STRESS INDEX**
2 = 0.75, 3 = 0.00, 3 = 0.24, 3 = 0.24, 4 = 0.34, 5 = 0.93, 6 = 0.34, 7 = 0.00, 7 = 0.24, 7 = 0.24, 8 = 0.75, 10 = 0.34, 11 = 0.49, 12 = 0.34, 13 = 0.34, 14 = 0.15, 15 = 0.34, 16 = 0.49, 17 = 0.34, 18 = 0.34, 19 = 0.16, 20 = 0.34, 21 = 0.46, 22 = 0.34, 23 = 0.34, 24 = 0.46, 25 = 0.34, 26 = 0.34 and 27 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 155 lb uplift at joint 2, 168 lb uplift at joint 8, 303 lb uplift at joint 16 and 289 lb uplift at joint 11.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061206

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T02 | COMMON | 10 | 1 | Job Reference (optional) |

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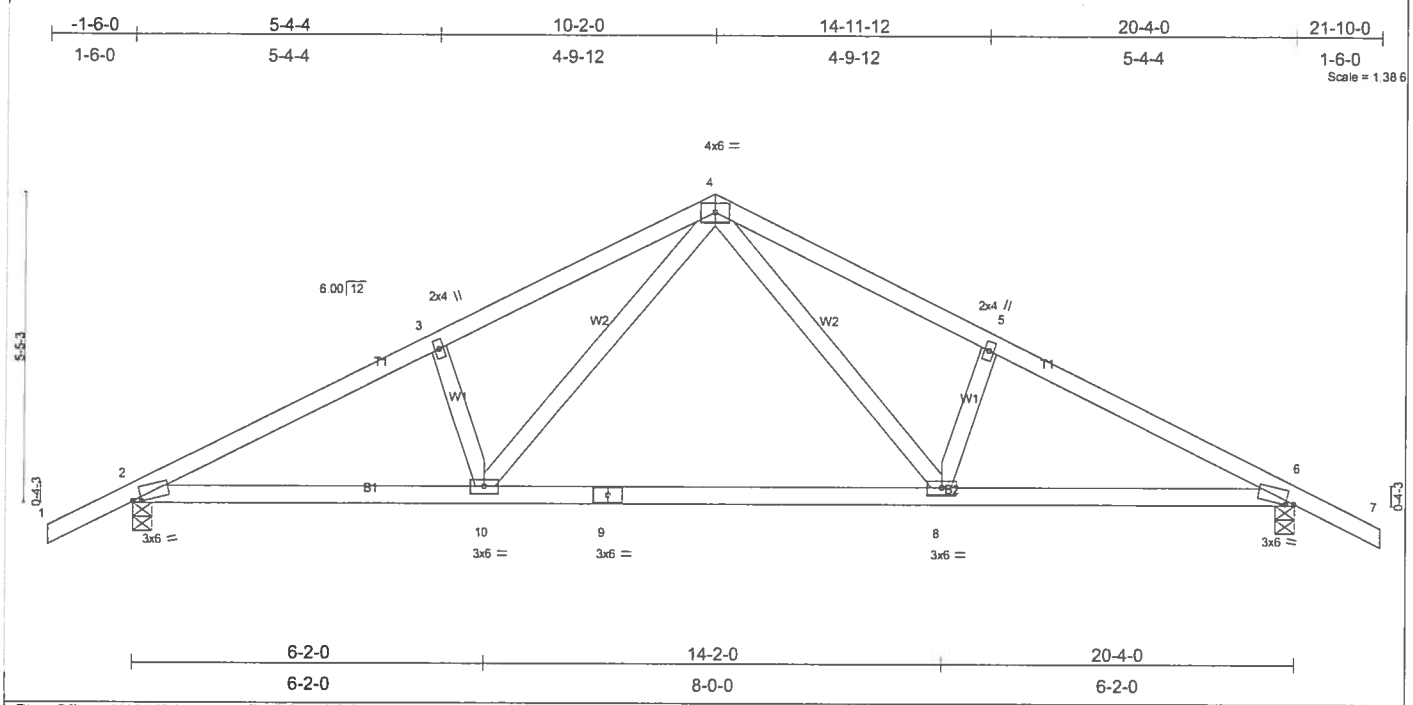


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.26 | Vert(LL) | -0.32 | 8-10 | >759 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.98 | Vert(TL) | -0.52 | 8-10 | >466 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.27 | Horz(TL) | 0.05 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 96 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-5-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-2 oc bracing.

**REACTIONS** (lb/size) 2=1130/0-4-0, 6=1130/0-4-0
Max Horz 2=-100(load case 6)
Max Uplift2=-445(load case 5), 6=-445(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1933/797, 3-4=-1836/851, 4-5=-1836/851, 5-6=-1933/797, 6-7=0/35
BOT CHORD 2-10=-566/1652, 9-10=-288/1080, 8-9=-288/1080, 6-8=-566/1652
WEBS 3-10=-212/222, 4-10=-342/853, 4-8=-342/853, 5-8=-212/222

**JOINT STRESS INDEX**
2 = 0.81, 3 = 0.34, 4 = 0.67, 5 = 0.34, 6 = 0.81, 8 = 0.59, 9 = 0.76 and 10 = 0.59

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 445 lb uplift at joint 2 and 445 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061207

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T03G | COMMON | 1 | 1 | Job Reference (optional) |

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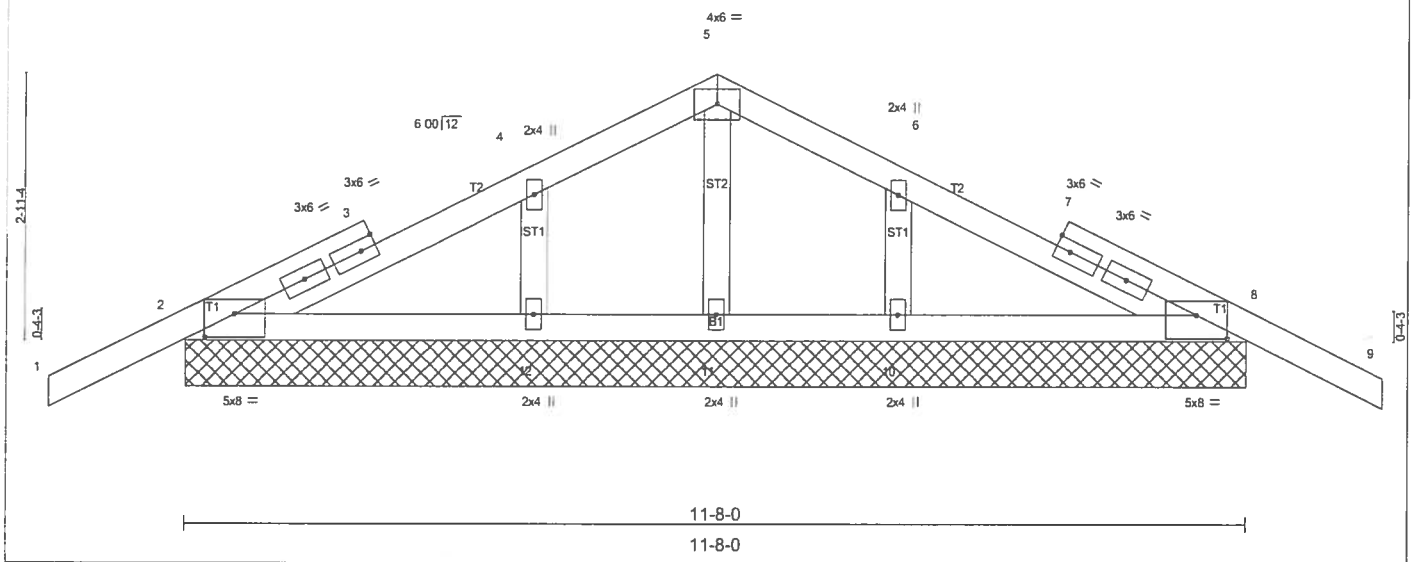
-1-6-0 | 5-10-0 | 11-8-0 | 13-2-0
1-6-0 | 5-10-0 | 5-10-0 | 1-6-0
Scale = 1:24.3




11-8-0
11-8-0

Plate Offsets (X,Y): [2:0-4-0,0-3-1], [8:0-4-0,0-3-1]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.18 | Vert(LL) | -0.00 | 9 | n/r | 120 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.07 | Vert(TL) | -0.00 | 9 | n/r | 90 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.05 | Horz(TL) | 0.00 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 54 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 2=251/11-8-0, 8=251/11-8-0, 11=147/11-8-0, 12=320/11-8-0, 10=320/11-8-0
Max Horz 2=65(load case 5)
Max Uplift2=-157(load case 5), 8=-167(load case 6), 11=-12(load case 5), 12=-127(load case 5), 10=-130(load case 6)
Max Grav 2=255(load case 9), 8=255(load case 10), 11=147(load case 1), 12=323(load case 9), 10=323(load case 10)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-43/52, 3-4=-38/108, 4-5=0/81, 5-6=0/81, 6-7=-14/108, 7-8=-18/52, 8-9=0/41
BOT CHORD 2-12=-48/115, 11-12=-48/115, 10-11=-48/115, 8-10=-48/115
WEBS 5-11=-124/21, 4-12=-213/190, 6-10=-213/190

**JOINT STRESS INDEX**
2 = 0.62, 3 = 0.00, 3 = 0.22, 3 = 0.22, 4 = 0.10, 5 = 0.06, 6 = 0.10, 7 = 0.00, 7 = 0.22, 7 = 0.22, 8 = 0.62, 10 = 0.11, 11 = 0.05 and 12 = 0.11

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 157 lb uplift at joint 2, 167 lb uplift at joint 8, 12 lb uplift at joint 11, 127 lb uplift at joint 12 and 130 lb uplift at joint 10.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-64(F=-10), 5-9=-64(F=-10), 2-8=-30

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061208

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T04 | COMMON | 3 | 1 | Job Reference (optional) |

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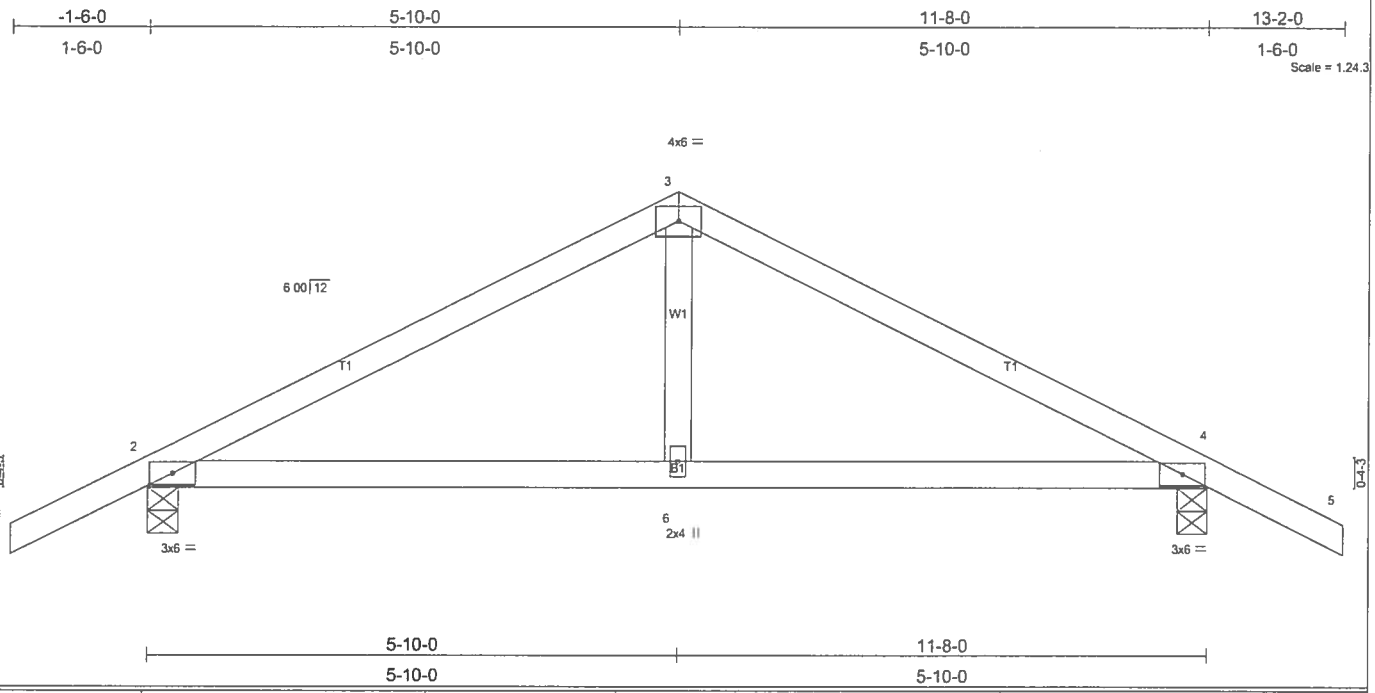


| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.23 | Vert(LL) | -0.04 | 4-6 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.27 | Vert(TL) | -0.06 | 4-6 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.07 | Horz(TL) | 0.01 | 4 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 46 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=566/0-4-0, 4=566/0-4-0
Max Horz 2=69(load case 5)
Max Uplift2=-254(load case 5), 4=-254(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-657/289, 3-4=-657/289, 4-5=0/35
BOT CHORD 2-6=-106/527, 4-6=-106/527
WEBS 3-6=0/208

**JOINT STRESS INDEX**
2 = 0.36, 3 = 0.71, 4 = 0.36 and 6 = 0.15

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 2 and 254 lb uplift at joint 4.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061209

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T04G | COMMON | 1 | 2 | Job Reference (optional) |

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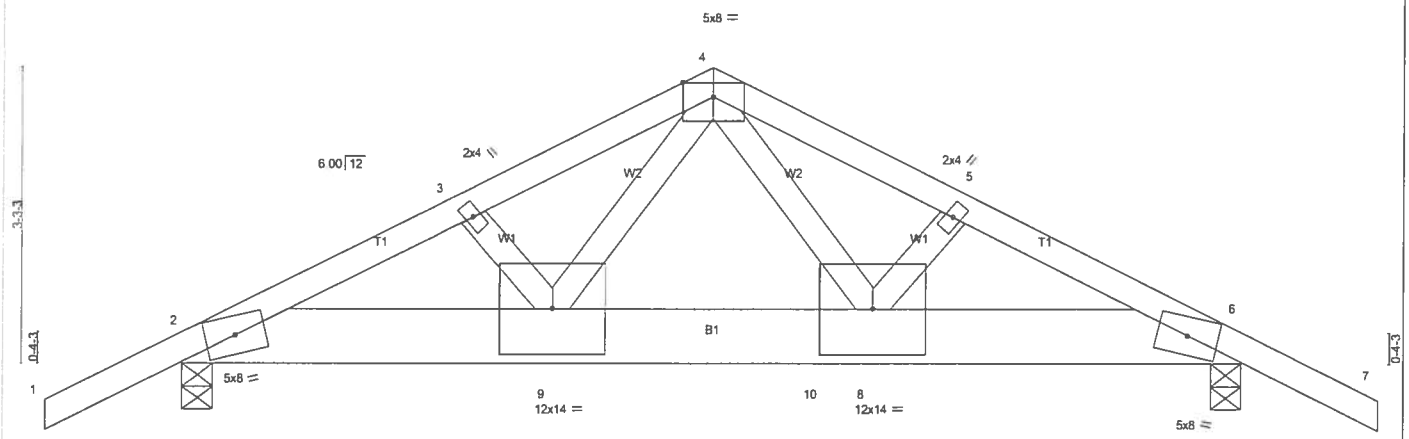
| -1-6-0 | 3-2-4 | 5-10-0 | 8-5-12 | 11-8-0 | 13-2-0 |
|---|---|---|---|---|---|
| 1-6-0 | 3-2-4 | 2-7-12 | 2-7-12 | 3-2-4 | 1-6-0 |

Scale = 1:24.3




| 4-0-13 | 7-0-0 | 7-7-3 | 11-8-0 |
|---|---|---|---|
| 4-0-13 | 2-11-3 | 0-7-3 | 4-0-13 |

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.32 | Vert(LL) | -0.08 | 8-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.36 | Vert(TL) | -0.13 | 8-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.93 | Horz(TL) | 0.02 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 147 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS** (lb/size) 2=2614/0-4-0, 6=5361/0-4-0
Max Horz 2=73(load case 4)
Max Uplift2=-1028(load case 4), 6=-2065(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-5346/1937, 3-4=-5269/1939, 4-5=-8187/3039, 5-6=-8260/3041, 6-7=0/42
BOT CHORD 2-9=-1710/4747, 9-10=-1493/4230, 8-10=-1493/4230, 6-8=-2671/7377
WEBS 3-9=-63/93, 4-9=-309/911, 4-8=-2163/5820, 5-8=-86/102

**JOINT STRESS INDEX**
2 = 0.86, 3 = 0.04, 4 = 0.85, 5 = 0.04, 6 = 0.86, 8 = 0.50 and 9 = 0.50

**NOTES**
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1028 lb uplift at joint 2 and 2065 lb uplift at joint 6.
7) Girder carries tie-in span(s): 38-0-0 from 7-0-0 to 11-8-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3473 lb down and 1311 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 6-10=-779(F=-749)
Concentrated Loads (lb)
Vert: 10=-3473(F)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061210

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T05 | HIP | 1 | 1 | Job Reference (optional) |

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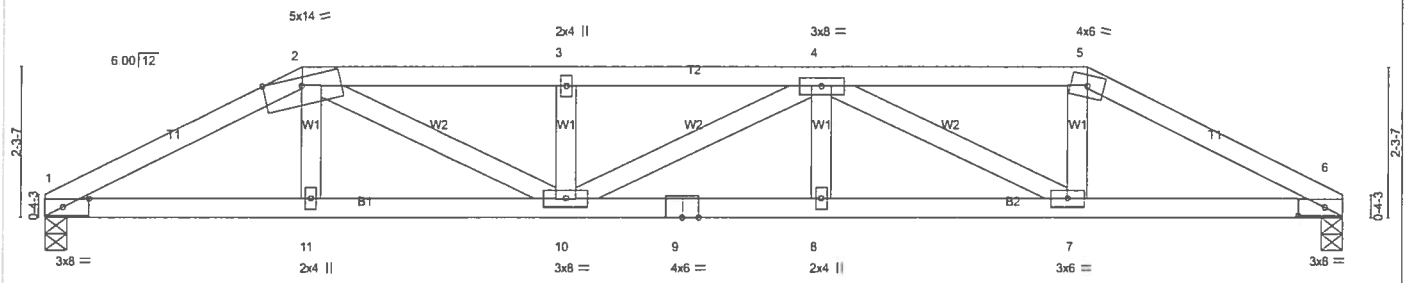
| 3-10-8 | 7-10-12 | 11-9-4 | 15-9-8 | 19-8-0 |
|---|---|---|---|---|
| 3-10-8 | 4-0-4 | 3-10-8 | 4-0-4 | 3-10-8 |

Scale = 1:33.4




| 3-10-8 | 7-10-12 | 11-9-4 | 15-9-8 | 19-8-0 |
|---|---|---|---|---|
| 3-10-8 | 4-0-4 | 3-10-8 | 4-0-4 | 3-10-8 |

Plate Offsets (X,Y): [1:0-4-12,0-1-8], [6:0-4-12,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.24 | Vert(LL) | -0.16 | 8-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.60 | Vert(TL) | -0.25 | 8-10 | >919 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.35 | Horz(TL) | 0.07 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 88 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-3 oc bracing.

**REACTIONS** (lb/size) 1=1132/0-4-0, 6=1132/0-4-0
Max Horz 1=-29(load case 2)
Max Uplift1=-400(load case 3), 6=-400(load case 2)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2159/818, 2-3=-2801/1122, 3-4=-2801/1122, 4-5=-1932/764, 5-6=-2158/817
BOT CHORD 1-11=-721/1879, 10-11=-724/1900, 9-10=-1085/2801, 8-9=-1085/2801, 7-8=-1085/2801, 6-7=-691/1878
WEBS 2-11=-48/282, 2-10=-454/1061, 3-10=-297/228, 4-10=-26/24, 4-8=0/161, 4-7=-1032/445, 5-7=-238/743

**JOINT STRESS INDEX**
1 = 0.76, 2 = 0.52, 3 = 0.34, 4 = 0.57, 5 = 0.49, 6 = 0.76, 7 = 0.49, 8 = 0.34, 9 = 0.88, 10 = 1.00 and 11 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 400 lb uplift at joint 1 and 400 lb uplift at joint 6.
6) Girder carries hip end with 3-10-8 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 66 lb up at 15-9-8, and 129 lb down and 66 lb up at 3-10-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-5=-75(F=-21), 5-6=-54, 1-11=-30, 7-11=-42(F=-12), 6-7=-30
Concentrated Loads (lb)
Vert: 11=-129(F) 7=-129(F)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

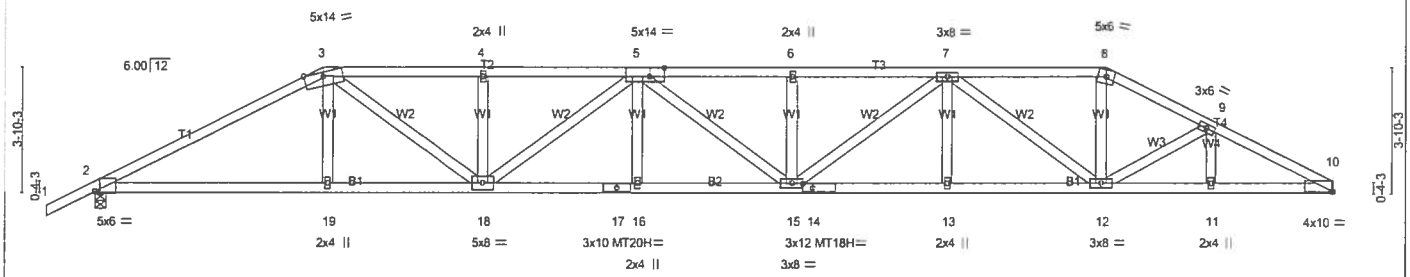
Dwg.#0616061211

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T06 | HIP | 1 | 2 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:17:08 2006 Page 1

| 1-6-0 | 3-9-4 | 7-0-0 | 11-10-10 | 16-7-9 | 21-4-7 | 26-1-6 | 31-0-0 | 34-2-12 | 38-0-0 |
|---|---|---|---|---|---|---|---|---|---|
| 1-6-0 | 3-9-4 | 3-2-12 | 4-10-10 | 4-8-14 | 4-8-14 | 4-8-14 | 4-10-10 | 3-2-12 | 3-9-4 |

Scale = 1:67.6




| 3-9-4 | 7-0-0 | 11-10-10 | 16-7-9 | 21-4-7 | 26-1-6 | 31-0-0 | 34-2-12 | 38-0-0 |
|---|---|---|---|---|---|---|---|---|
| 3-9-4 | 3-2-12 | 4-10-10 | 4-8-14 | 4-8-14 | 4-8-14 | 4-10-10 | 3-2-12 | 3-9-4 |

Plate Offsets (X,Y): [2:0-1-10,Edge], [5:0-5-4,0-3-4], [10:0-0-8,0-0-2], [14:0-3-9,0-1-8]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.40 | Vert(LL) | -0.51 | 15-16 | >897 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.93 | Vert(TL) | -0.81 | 15-16 | >560 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.47 | Horz(TL) | 0.22 | 10 | n/a | n/a | MT18H | 244/190 |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 393 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-2-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-4 oc bracing.

**REACTIONS** (lb/size) 10=3473/Mechanical, 2=3355/0-4-0
Max Horz 2=-270(load case 5)
Max Uplift10=-1425(load case 2), 2=-1393(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-6617/2845, 3-4=-8148/3638, 4-5=-8147/3638, 5-6=-9226/4123, 6-7=-9226/4123, 7-8=-5724/2589, 8-9=-6341/2770, 9-10=-6871/2855
BOT CHORD 2-19=-2281/5806, 18-19=-2290/5839, 17-18=-3844/9262, 16-17=-3844/9262, 15-16=-3844/9262, 14-15=-3285/8043, 13-14=-3285/8043, 12-13=-3285/8043, 11-12=-2425/6024, 10-11=-2425/6024
WEBS 3-19=-234/823, 3-18=-1382/2949, 4-18=-574/480, 5-18=-1418/649, 5-16=0/307, 5-15=-89/30, 6-15=-553/446, 7-15=-669/1520, 7-13=0/301, 7-12=-2949/1328, 8-12=-855/2280, 9-12=-456/358, 9-11=0/188

**JOINT STRESS INDEX**
2 = 0.81, 3 = 0.78, 4 = 0.34, 5 = 0.98, 6 = 0.34, 7 = 0.73, 8 = 0.58, 9 = 0.41, 10 = 0.80, 11 = 0.34, 12 = 0.71, 13 = 0.34, 14 = 0.79, 15 = 0.73, 16 = 0.34, 17 = 0.94, 18 = 0.69 and 19 = 0.34

**NOTES**
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Refer to girder(s) for truss to truss connections.
8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1425 lb uplift at joint 10 and 1393 lb uplift at joint 2.
10) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-8=-117(F=-63), 8-10=-117(F=-63), 2-19=-30, 10-19=-65(F=-35)
Concentrated Loads (lb)
Vert: 19=-539(F)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061212

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T07 | HIP | 1 | 1 | Job Reference (optional) |

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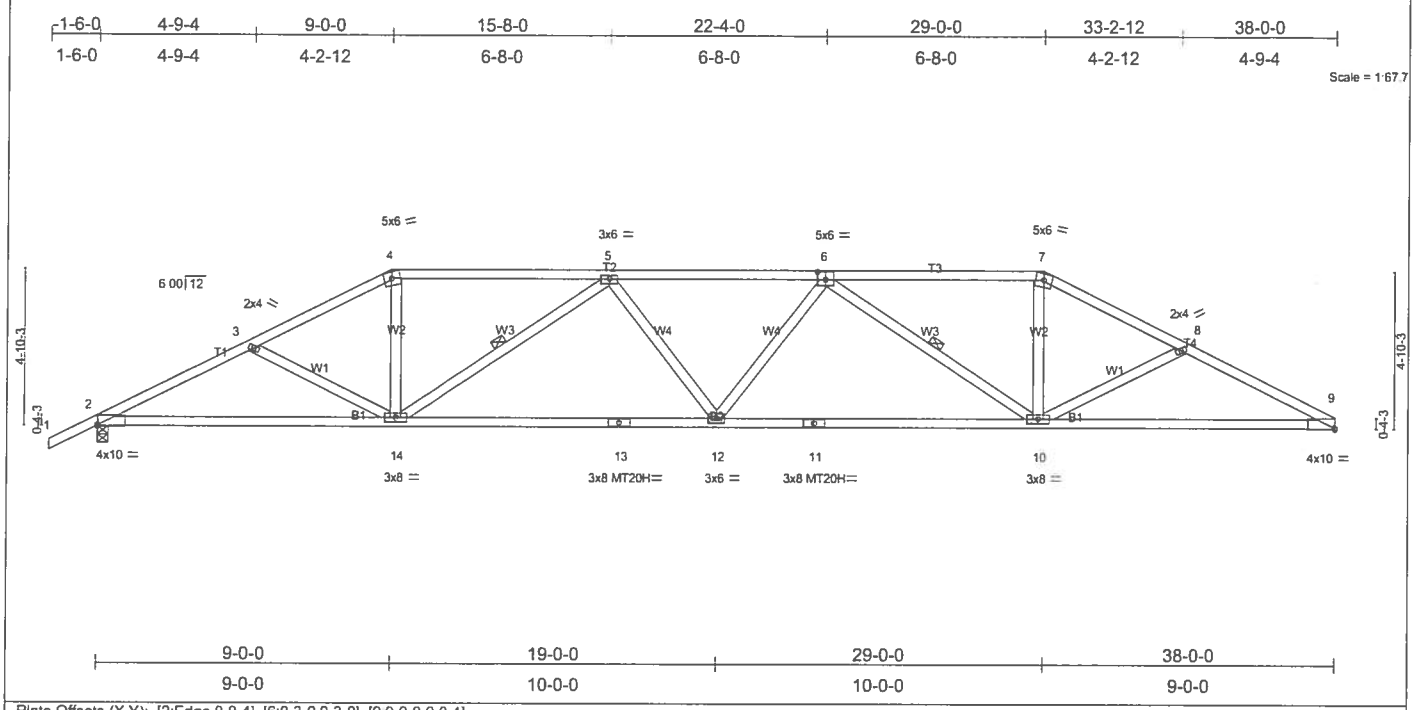


Plate Offsets (X,Y): [2:Edge,0-0-4], [6:0-3-0,0-3-0], [9.0-0-0,0-0-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | -0.40 | 12-14 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.87 | Vert(TL) | -0.65 | 12-14 | >694 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.32 | Horz(TL) | 0.18 | 9 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 185 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-2-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-12 oc bracing.
WEBS 1 Row at midpt 5-14, 6-10

**REACTIONS** (lb/size) 9=1584/Mechanical, 2=1678/0-4-0
Max Horz 2=110(load case 5)
Max Uplift9=-440(load case 6), 2=-536(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2968/1227, 3-4=-2759/1128, 4-5=-2456/1073, 5-6=-3307/1391, 6-7=-2485/1096, 7-8=-2792/1156, 8-9=-3036/1283
BOT CHORD 2-14=-1010/2593, 13-14=-1177/3194, 12-13=-1177/3194, 11-12=-1185/3204, 10-11=-1185/3204, 9-10=-1074/2672
WEBS 3-14=-190/200, 4-14=-266/900, 5-14=-980/436, 5-12=0/238, 6-12=0/225, 6-10=-962/429, 7-10=-285/920, 8-10=-246/247

**JOINT STRESS INDEX**
2 = 0.74, 3 = 0.34, 4 = 0.61, 5 = 0.40, 6 = 0.57, 7 = 0.61, 8 = 0.34, 9 = 0.74, 10 = 0.57, 11 = 0.89, 12 = 0.40, 13 = 0.89 and 14 = 0.57

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 440 lb uplift at joint 9 and 536 lb uplift at joint 2.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061213

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T08 | HIP | 1 | 1 | Job Reference (optional) |

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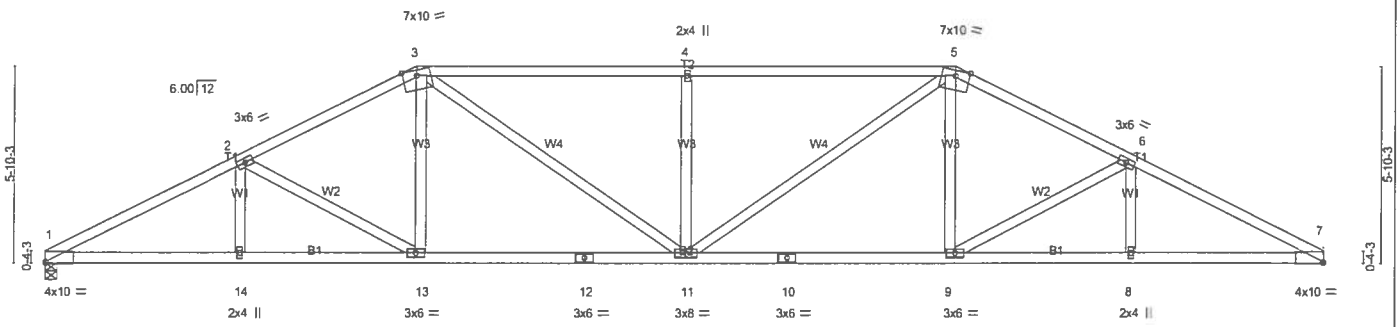
| 5-9-4 | 11-0-0 | 19-0-0 | 27-0-0 | 32-2-12 | 38-0-0 |
|---|---|---|---|---|---|
| 5-9-4 | 5-2-12 | 8-0-0 | 8-0-0 | 5-2-12 | 5-9-4 |

Scale = 1:65.5




| 5-9-4 | 11-0-0 | 19-0-0 | 27-0-0 | 32-2-12 | 38-0-0 |
|---|---|---|---|---|---|
| 5-9-4 | 5-2-12 | 8-0-0 | 8-0-0 | 5-2-12 | 5-9-4 |

Plate Offsets (X,Y): [1:0-0-0,0-0-4], [7:0-0-0,0-0-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.48 | Vert(LL) | -0.30 | 9-11 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.67 | Vert(TL) | -0.48 | 9-11 | >938 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.47 | Horz(TL) | 0.15 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 193 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-3-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

**REACTIONS** (lb/size) 1=1586/0-4-0, 7=1586/Mechanical
Max Horz 1=-81(load case 3)
Max Uplift1=-459(load case 5), 7=-460(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3049/1260, 2-3=-2583/1125, 3-4=-2781/1267, 4-5=-2781/1267, 5-6=-2597/1131, 6-7=-3085/1275
BOT CHORD 1-14=-1036/2646, 13-14=-1036/2646, 12-13=-787/2272, 11-12=-787/2272, 10-11=-792/2284, 9-10=-792/2284, 8-9=-1053/2688, 7-8=-1053/2688
WEBS 2-14=0/173, 2-13=-443/286, 3-13=-96/414, 3-11=-294/739, 4-11=-459/326, 5-11=-291/726, 5-9=-103/429, 6-9=-476/301, 6-8=0/175

**JOINT STRESS INDEX**
1 = 0.75, 2 = 0.41, 3 = 0.67, 4 = 0.34, 5 = 0.67, 6 = 0.41, 7 = 0.75, 8 = 0.34, 9 = 0.35, 10 = 0.91, 11 = 0.71, 12 = 0.91, 13 = 0.35 and 14 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B, enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 459 lb uplift at joint 1 and 460 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061214

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T09 | HIP | 1 | 1 | Job Reference (optional) |

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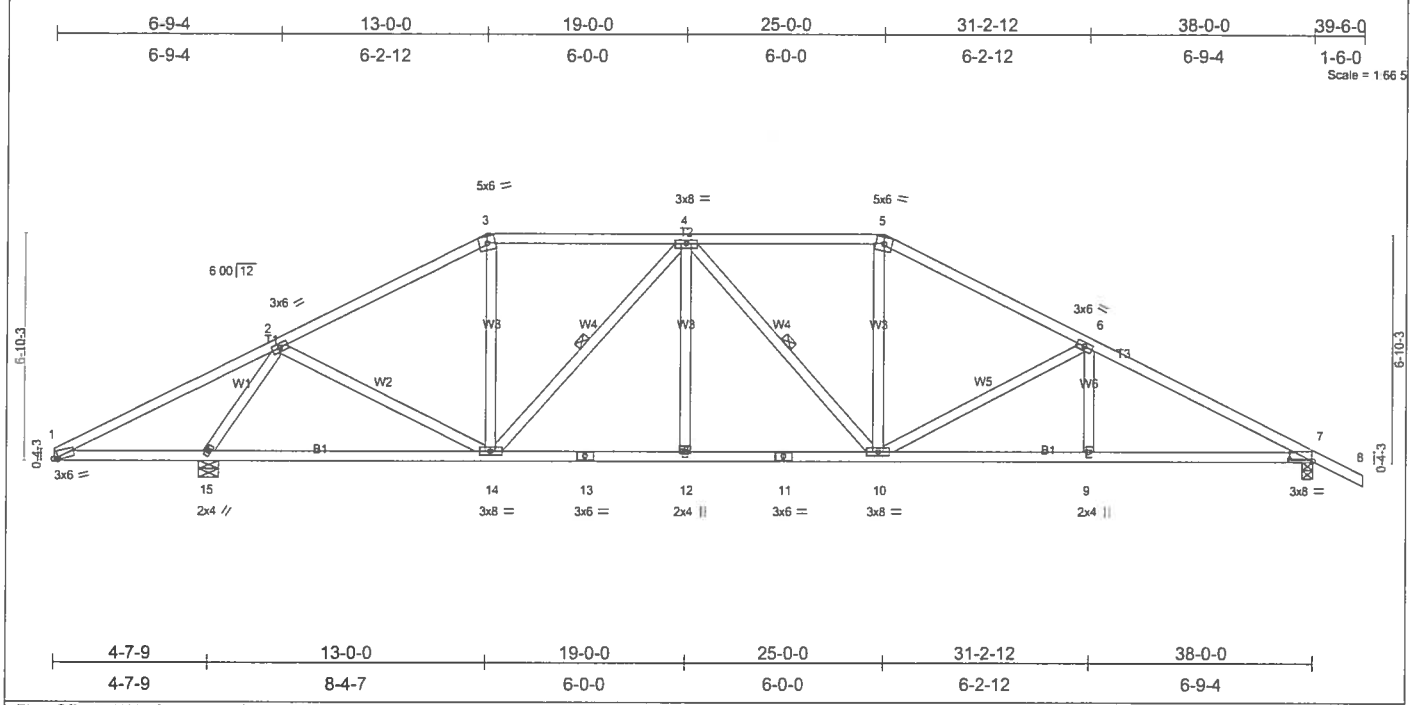


Plate Offsets (X,Y): [1:0-1-8,0-0-7], [7:0-8-8,0-0-10]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.35 | Vert(LL) | -0.14 | 10-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.59 | Vert(TL) | -0.23 | 10-12 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.58 | Horz(TL) | 0.09 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 204 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14, 4-10

**REACTIONS** (lb/size) 15=1803/0-7-3, 7=1465/0-4-0
Max Horz 15=-138(load case 6)
Max Uplift15=-663(load case 5), 7=-522(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-404/455, 2-3=-1532/646, 3-4=-1306/642, 4-5=-1669/822, 5-6=-1932/842, 6-7=-2534/1002, 7-8=0/35
BOT CHORD 1-15=-328/428, 14-15=-211/669, 13-14=-426/1699, 12-13=-426/1699, 11-12=-426/1699, 10-11=-426/1699, 9-10=-734/2183, 7-9=-734/2183
WEBS 2-15=-1822/1027, 2-14=-197/714, 3-14=-47/383, 4-14=-646/264, 4-12=0/157, 4-10=-193/139, 5-10=-121/501, 6-10=-599/348, 6-9=0/217

**JOINT STRESS INDEX**
1 = 0.86, 2 = 0.61, 3 = 0.50, 4 = 0.57, 5 = 0.55, 6 = 0.41, 7 = 0.80, 9 = 0.34, 10 = 0.57, 11 = 0.67, 12 = 0.34, 13 = 0.61, 14 = 0.67 and 15 = 0.90

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft: TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 663 lb uplift at joint 15 and 522 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061215

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T10 | HIP | 1 | 1 | Job Reference (optional) |

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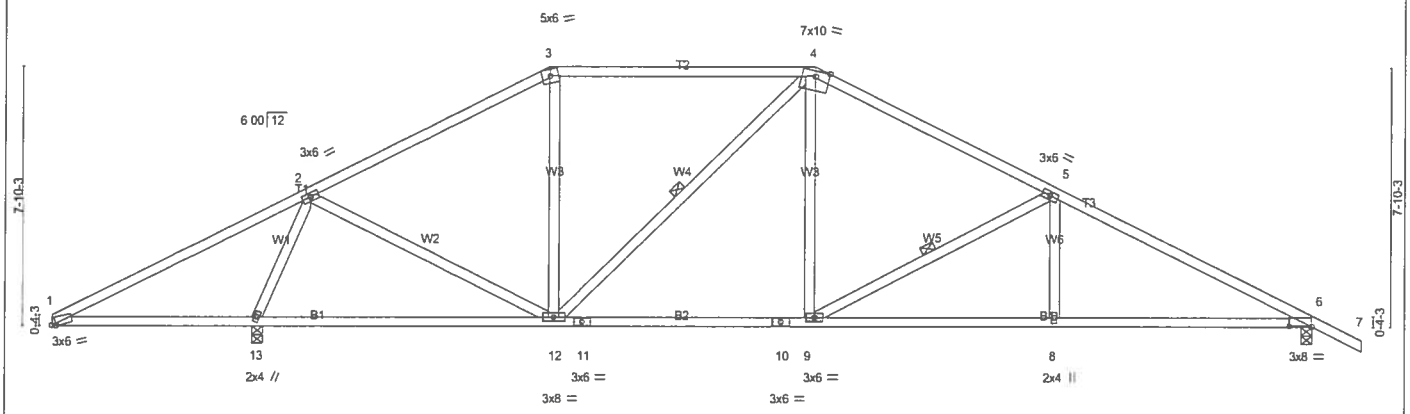
| 7-9-4 | 15-0-0 | 23-0-0 | 30-2-12 | 38-0-0 | 39-6-0 |
|---|---|---|---|---|---|
| 7-9-4 | 7-2-12 | 8-0-0 | 7-2-12 | 7-9-4 | 1-6-0 |

Scale = 1:66.5




| 6-2-0 | 15-0-0 | 23-0-0 | 30-2-12 | 38-0-0 |
|---|---|---|---|---|
| 6-2-0 | 8-10-0 | 8-0-0 | 7-2-12 | 7-9-4 |

Plate Offsets (X,Y): [1:0-1-8,0-0-7], [6:0-8-4,0-0-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.47 | Vert(LL) | -0.17 | 6-8 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.65 | Vert(TL) | -0.28 | 6-8 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.59 | Horz(TL) | 0.07 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 194 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-12, 5-9

**REACTIONS** (lb/size) 13=1892/0-4-0, 6=1376/0-4-0
Max Horz 13=-152(load case 6)
Max Uplift13=-754(load case 5), 6=-514(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-503/597, 2-3=-1254/523, 3-4=-1043/540, 4-5=-1581/696, 5-6=-2293/891, 6-7=0/35
BOT CHORD 1-13=-441/525, 12-13=-103/239, 11-12=-261/1345, 10-11=-261/1345, 9-10=-261/1345, 8-9=-624/1962, 6-8=-624/1962
WEBS 2-13=-1737/1033, 2-12=-314/908, 3-12=-9/206, 4-12=-472/225, 4-9=-156/547, 5-9=-711/415, 5-8=0/258

**JOINT STRESS INDEX**
1 = 0.91, 2 = 0.61, 3 = 0.75, 4 = 0.88, 5 = 0.41, 6 = 0.77, 8 = 0.34, 9 = 0.36, 10 = 0.53, 11 = 0.62, 12 = 0.85 and 13 = 0.68

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 754 lb uplift at joint 13 and 514 lb uplift at joint 6.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061216

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T11 | HIP | 1 | 1 | Job Reference (optional) |

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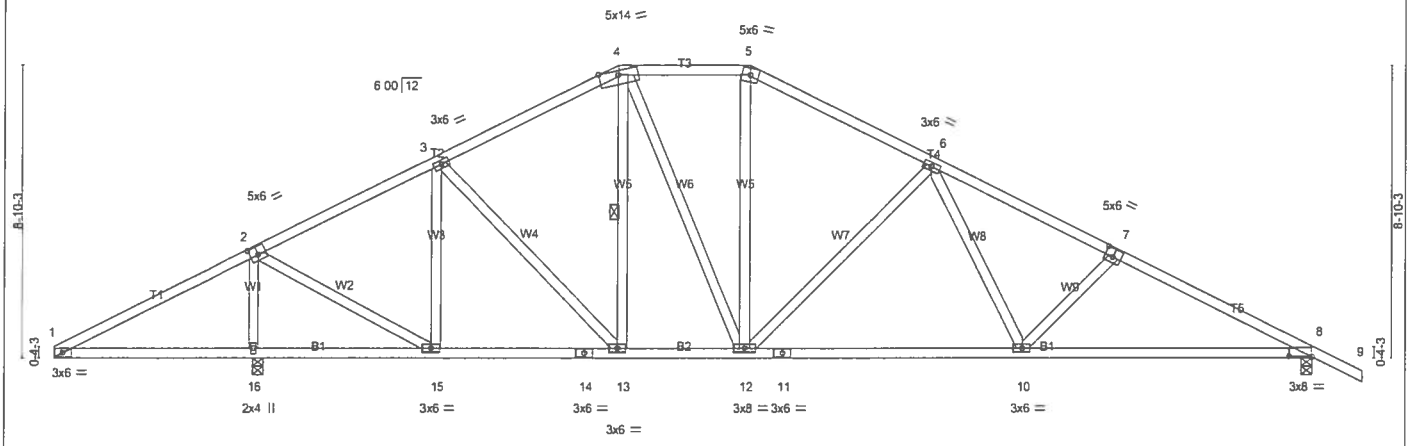
| 6-0-5 | 11-6-3 | 17-0-0 | 21-0-0 | 26-5-13 | 31-11-11 | 38-0-0 | 39-6-0 |
|---|---|---|---|---|---|---|---|
| 6-0-5 | 5-5-13 | 5-5-13 | 4-0-0 | 5-5-13 | 5-5-13 | 6-0-5 | 1-6-0 |

Scale = 1:66.6




| 6-2-0 | 11-6-3 | 17-0-0 | 21-0-0 | 29-2-12 | 38-0-0 |
|---|---|---|---|---|---|
| 6-2-0 | 5-4-3 | 5-5-13 | 4-0-0 | 8-2-12 | 8-9-4 |

Plate Offsets (X,Y): [2:0-3-0,0-3-0], [7:0-3-0,0-3-0], [8:0-8-4,0-0-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.36 | Vert(LL) | -0.18 | 8-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.62 | Vert(TL) | -0.30 | 8-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.72 | Horz(TL) | 0.06 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 219 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13

**REACTIONS** (lb/size) 16=1888/0-4-0, 8=1380/0-4-0
Max Horz 16=-166(load case 6)
Max Uplift16=-767(load case 5), 8=-525(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-432/529, 2-3=-1030/389, 3-4=-1212/599, 4-5=-1161/655, 5-6=-1361/667, 6-7=-2106/895, 7-8=-2311/948, 8-9=0/35
BOT CHORD 1-16=-396/445, 15-16=-357/478, 14-15=-125/857, 13-14=-125/857, 12-13=-115/1022, 11-12=-441/1593, 10-11=-441/1593, 8-10=-694/2008
WEBS 2-16=-1670/939, 2-15=-608/1393, 3-15=-537/373, 3-13=-102/260, 4-13=-69/89, 4-12=-153/418, 5-12=-111/329, 6-12=-627/403, 6-10=-161/538, 7-10=-277/271

**JOINT STRESS INDEX**
1 = 0.37, 2 = 0.73, 3 = 0.41, 4 = 0.57, 5 = 0.37, 6 = 0.41, 7 = 0.49, 8 = 0.78, 10 = 0.46, 11 = 0.52, 12 = 0.64, 13 = 0.37, 14 = 0.34, 15 = 0.81 and 16 = 0.61

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 767 lb uplift at joint 16 and 525 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061217

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T12 | COMMON | 5 | 1 | Job Reference (optional) |

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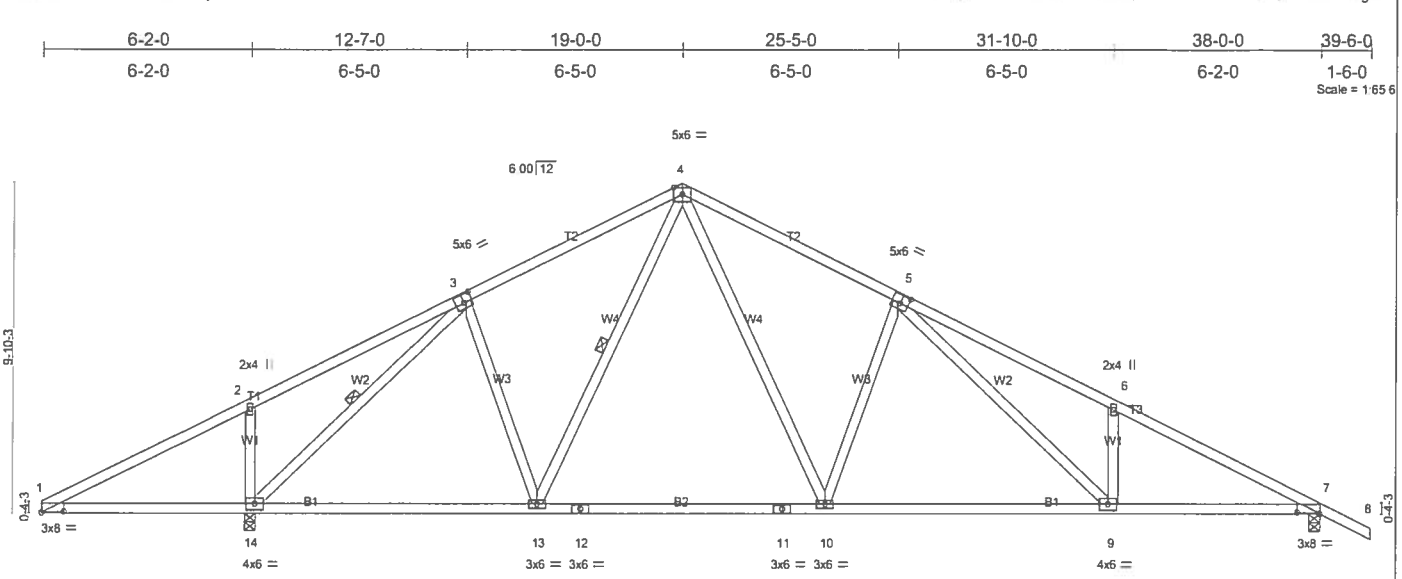


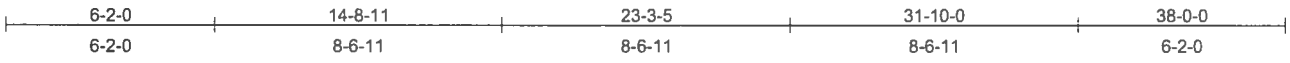


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [7:0-8-0,0-0-6]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.38 | Vert(LL) | -0.20 | 9-10 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.60 | Vert(TL) | -0.33 | 9-10 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.81 | Horz(TL) | 0.07 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 206 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-14, 4-13

**REACTIONS** (lb/size) 14=1896/0-4-0, 7=1372/0-4-0
Max Horz 14=-180(load case 6)
Max Uplift14=-787(load case 5), 7=-532(load case 6)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-446/512, 2-3=-247/499, 3-4=-1166/608, 4-5=-1560/814, 5-6=-2369/1109, 6-7=-2381/935, 7-8=0/35
BOT CHORD 1-14=-379/454, 13-14=-166/893, 12-13=-112/972, 11-12=-112/972, 10-11=-112/972, 9-10=-399/1504, 7-9=-684/2051
WEBS 2-14=-380/396, 3-14=-1782/842, 3-13=-24/324, 4-13=-76/126, 4-10=-414/881, 5-10=-570/447, 5-9=-400/768, 6-9=-303/329

**JOINT STRESS INDEX**
1 = 0.74, 2 = 0.34, 3 = 0.69, 4 = 0.57, 5 = 0.69, 6 = 0.34, 7 = 0.75, 9 = 0.39, 10 = 0.76, 11 = 0.43, 12 = 0.43, 13 = 0.76 and 14 = 0.39

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 787 lb uplift at joint 14 and 532 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061218

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T13 | MONO HIP | 1 | 1 | Job Reference (optional) |

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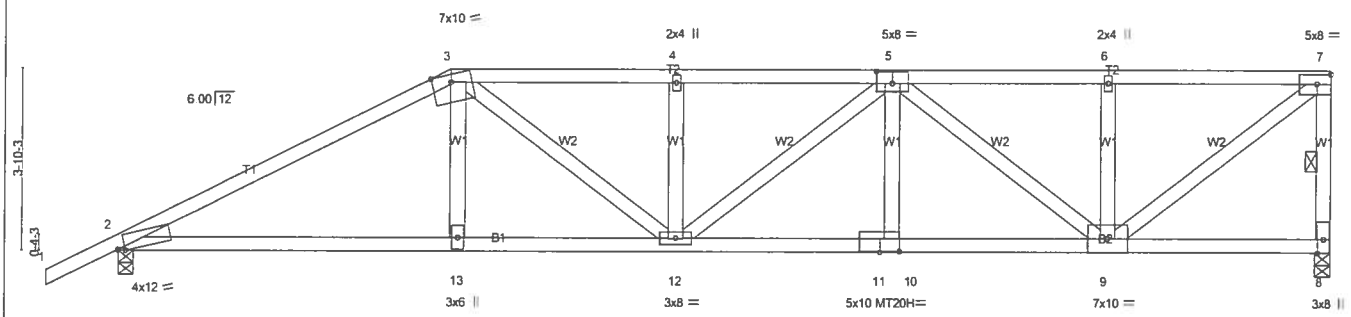
| -1-6-0 | 3-9-4 | 7-0-0 | 11-8-6 | 16-3-0 | 20-9-10 | 25-6-0 |
|---|---|---|---|---|---|---|
| 1-6-0 | 3-9-4 | 3-2-12 | 4-8-6 | 4-6-10 | 4-6-10 | 4-8-6 |

Scale = 1.46 3




| 3-9-4 | 7-0-0 | 11-8-6 | 16-3-0 | 20-9-10 | 25-6-0 |
|---|---|---|---|---|---|
| 3-9-4 | 3-2-12 | 4-8-6 | 4-6-10 | 4-6-10 | 4-8-6 |

Plate Offsets (X,Y): [2:0-1-13,Edge], [5:0-4-0,0-3-0], [11:0-5-0,0-0-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.69 | Vert(LL) | -0.25 | 10-12 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.93 | Vert(TL) | -0.40 | 10-12 | >750 | 180 | MT20H | 187/143 |
| BCLL | 10.0 | Rep Stress Incr | NO | WB | 0.88 | Horz(TL) | 0.13 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 136 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 2-7-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-14 oc bracing.
WEBS 1 Row at midpt 7-8

**REACTIONS** (lb/size) 8=2346/0-4-0, 2=2192/0-4-0
Max Horz 2=210(load case 4)
Max Uplift8=-1055(load case 3), 2=-930(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-4098/1726, 3-4=-4312/1934, 4-5=-4312/1934, 5-6=-2553/1147, 6-7=-2553/1147, 7-8=-2191/1068
BOT CHORD 2-13=-1575/3571, 12-13=-1584/3605, 11-12=-1778/3964, 10-11=-1778/3964, 9-10=-1778/3964, 8-9=-35/78
WEBS 3-13=-235/820, 3-12=-502/885, 4-12=-520/468, 5-12=-212/441, 5-10=0/299, 5-9=-1790/801, 6-9=-544/456, 7-9=-1410/3141

**JOINT STRESS INDEX**
2 = 0.78, 3 = 0.80, 4 = 0.34, 5 = 0.69, 6 = 0.34, 7 = 0.70, 8 = 0.41, 9 = 0.69, 10 = 0.00, 11 = 0.81, 12 = 0.86 and 13 = 0.27

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1055 lb uplift at joint 8 and 930 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)** Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-117(F=-63), 2-13=-30, 8-13=-65(F=-35)
Concentrated Loads (lb)
Vert: 13=-539(F)

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061219

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T14 | SPECIAL | 1 | 1 | Job Reference (optional) |

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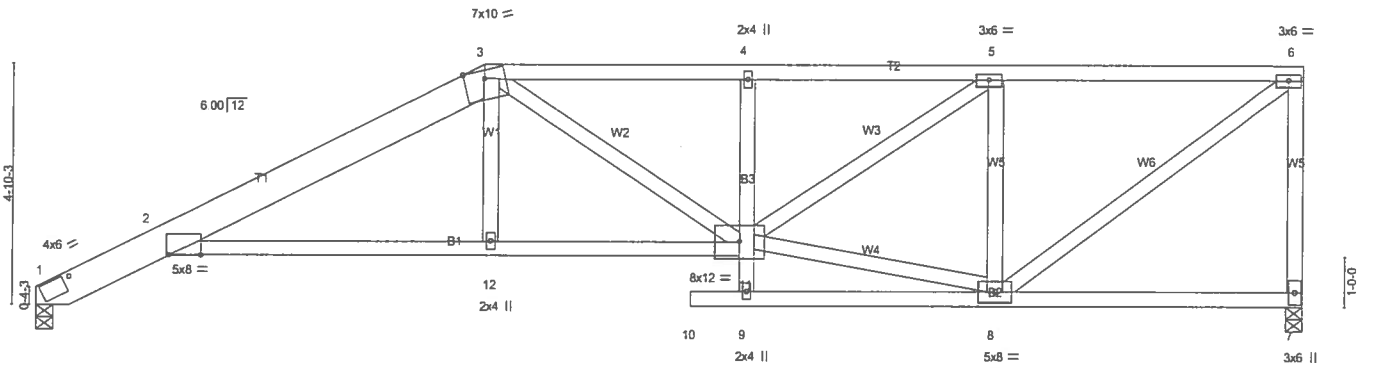
-1-6-0 | 2-0-0 | 9-0-0 | 13-2-0 | 19-4-0 | 25-6-0

1-6-0 | 2-0-0 | 7-0-0 | 4-2-0 | 6-2-0 | 6-2-0

Scale = 1:44.3




2-0-0 | 9-0-0 | 13-2-0 | 14-2-0 | 19-4-0 | 25-6-0

2-0-0 | 7-0-0 | 4-2-0 | 1-0-0 | 5-2-0 | 6-2-0

Plate Offsets (X,Y): [2:0-7-8,0-0-0], [2:1-11-14,0-6-1]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.78 | Vert(LL) | -0.35 | 2-12 | >867 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.76 | Vert(TL) | -0.57 | 2-12 | >532 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.55 | Horz(TL) | 0.24 | 7 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 150 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 4-0-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-9-8 oc bracing: 11-12.
1 Row at midpt 2-12

**REACTIONS** (lb/size) 1=1028/0-4-0, 7=1074/0-4-0
Max Horz 1=211(load case 5)
Max Uplift1=-301(load case 5), 7=-380(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-394/13, 2-3=-2062/800, 3-4=-1913/793, 4-5=-1895/791, 5-6=-1134/465, 6-7=-980/447
BOT CHORD 2-12=-860/1902, 11-12=-861/1913, 9-11=0/112, 4-11=-186/168, 9-10=0/0, 8-9=-67/137, 7-8=-18/46
WEBS 3-12=-5/264, 3-11=-101/110, 8-11=-406/1016, 5-11=-400/932, 5-8=-863/467, 6-8=-561/1365

**JOINT STRESS INDEX**
2 = 0.56, 2 = 0.00, 3 = 0.31, 4 = 0.34, 5 = 0.55, 6 = 0.81, 7 = 0.30, 8 = 0.64, 9 = 0.78, 11 = 0.44 and 12 = 0.34

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 301 lb uplift at joint 1 and 380 lb uplift at joint 7.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

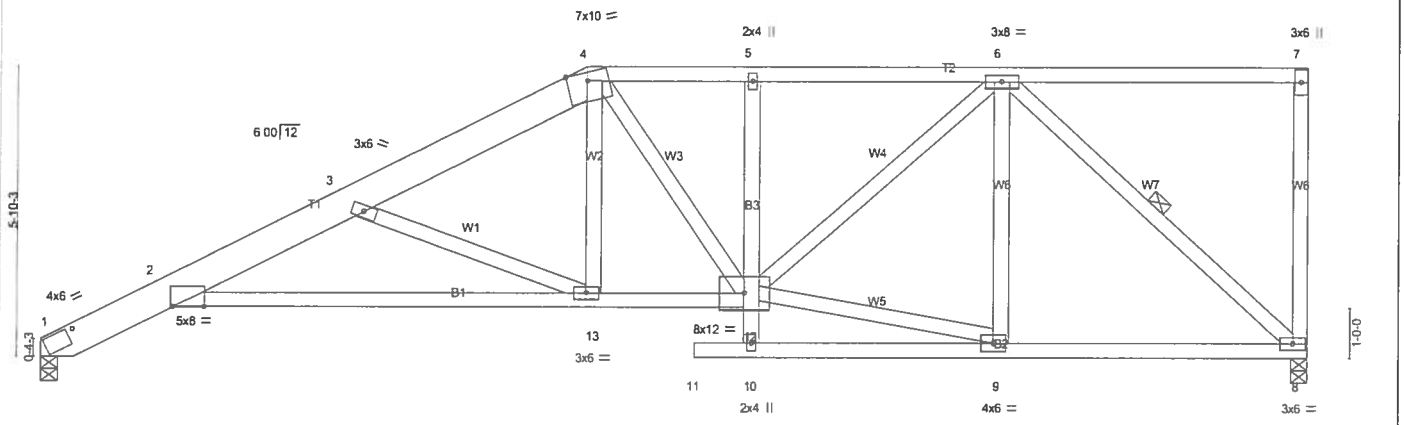
Dwg.#0616061220

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T15 | SPECIAL | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:17:20 2006 Page 1

-1-6-0 | 2-0-0 | 6-6-0 | 11-0-0 | 13-2-0 | 19-4-0 | 25-6-0
1-6-0 | 2-0-0 | 4-6-0 | 4-6-0 | 2-2-0 | 6-2-0 | 6-2-0

Scale = 1:44.4




2-0-0 | 11-0-0 | 13-2-0 | 14-2-0 | 19-4-0 | 25-6-0
2-0-0 | 9-0-0 | 2-2-0 | 1-0-0 | 5-2-0 | 6-2-0

Plate Offsets (X,Y) [2:0-7-8,0-0-0], [2:2-0-3,0-5-15]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.65 | Vert(LL) | -0.33 | 2-13 | >920 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.74 | Vert(TL) | -0.54 | 2-13 | >558 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.50 | Horz(TL) | 0.21 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 168 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-0-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-12 oc bracing. Except
1 Row at midpt 2-13
WEBS 1 Row at midpt 6-8

**REACTIONS** (lb/size) 1=1028/0-4-0, 8=1074/0-4-0
Max Horz 1=257(load case 5)
Max Uplift1=-309(load case 5), 8=-372(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-394/0, 2-3=-2572/1121, 3-4=-1798/723, 4-5=-1483/654, 5-6=-1478/654, 6-7=-33/13, 7-8=-153/107
BOT CHORD 2-13=-1299/2522, 12-13=-702/1551, 10-12=0/110, 5-12=-167/136, 10-11=0/0, 9-10=-37/145, 8-9=-397/932
WEBS 3-13=-1068/649, 4-13=-220/663, 4-12=-119/96, 9-12=-367/802, 6-12=-347/736, 6-9=0/119, 6-8=-1223/523

**JOINT STRESS INDEX**
2 = 0.58, 2 = 0.00, 3 = 0.41, 4 = 0.34, 5 = 0.34, 6 = 0.72, 7 = 0.30, 8 = 0.47, 9 = 0.37, 10 = 0.71, 12 = 0.37 and 13 = 0.43

**NOTES**
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 1 and 372 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061221

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T16 | SPECIAL | 1 | 1 | Job Reference (optional) |

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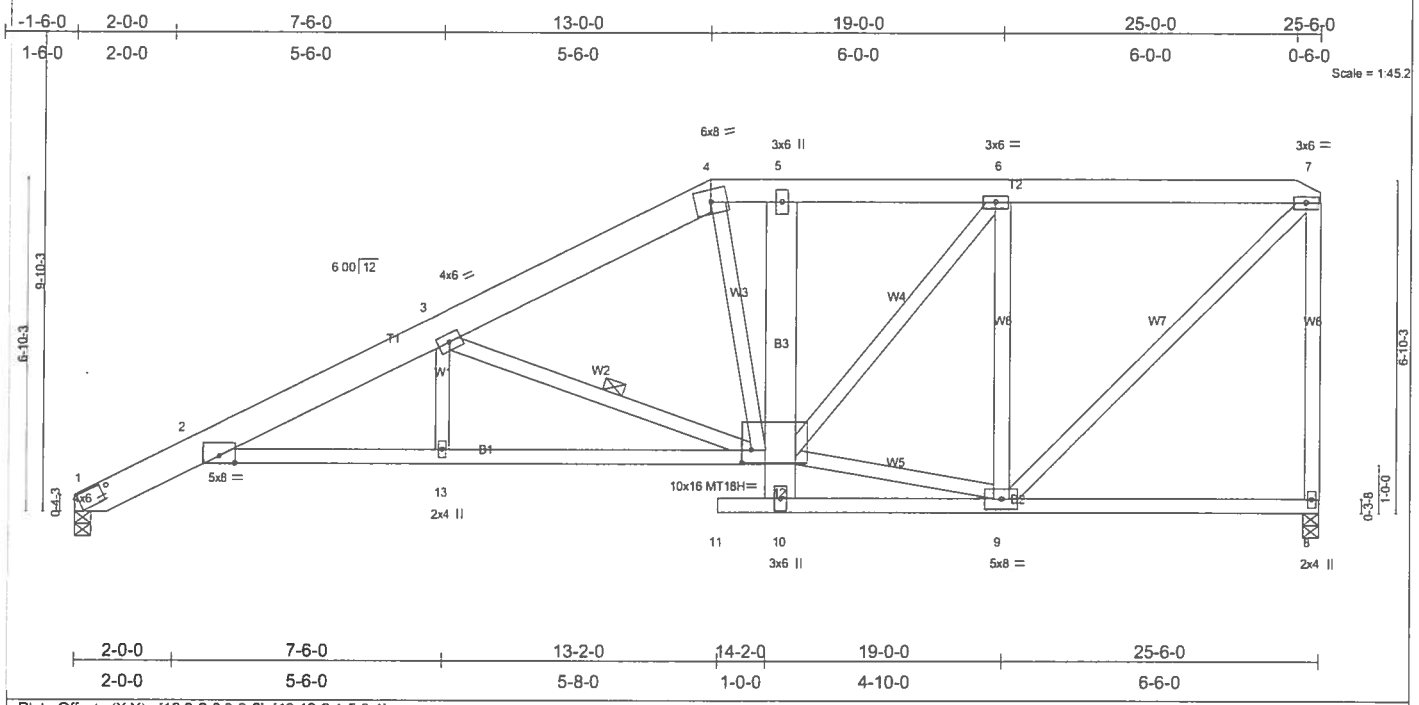


Plate Offsets (X,Y): [12:0-2-6,0-3-2], [12:12-2-1,5-3-1]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.63 | Vert(LL) | -0.27 | 2-13 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.83 | Vert(TL) | -0.44 | 2-13 | >687 | 180 | MT18H | 244/190 |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.74 | Horz(TL) | 0.22 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 197 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except* T2 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E *Except* B1 2 X 4 SYP No.2, B2 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
1 Row at midpt 12-13
WEBS 1 Row at midpt 3-12
JOINTS 1 Brace at Jt(s): 12, 13

**REACTIONS** (lb/size) 1=1030/0-4-0, 8=1077/0-4-0
Max Horz 1=300(load case 5)
Max Uplift1=-313(load case 5), 8=-361(load case 4)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-395/0, 2-3=-2437/978, 3-4=-1397/556, 4-5=-1261/562, 5-6=-1201/543, 6-7=-841/371
BOT CHORD 2-13=-1178/2305, 12-13=-1178/2305, 10-12=0/93, 5-12=-37/67, 10-11=0/0, 9-10=-133/170, 8-9=0/0
WEBS 3-13=0/243, 3-12=-1212/673, 9-12=-244/688, 6-12=-270/565, 6-9=-811/474, 7-9=-530/1201, 4-12=-55/335, 7-8=-998/479

**JOINT STRESS INDEX**
2 = 0.68, 3 = 0.30, 4 = 0.24, 5 = 0.15, 6 = 0.40, 7 = 0.77, 8 = 0.36, 9 = 0.57, 10 = 0.15, 12 = 0.52, 12 = 0.00 and 13 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 1 and 361 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061222

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T17 | SPECIAL | 1 | 1 | Job Reference (optional) |

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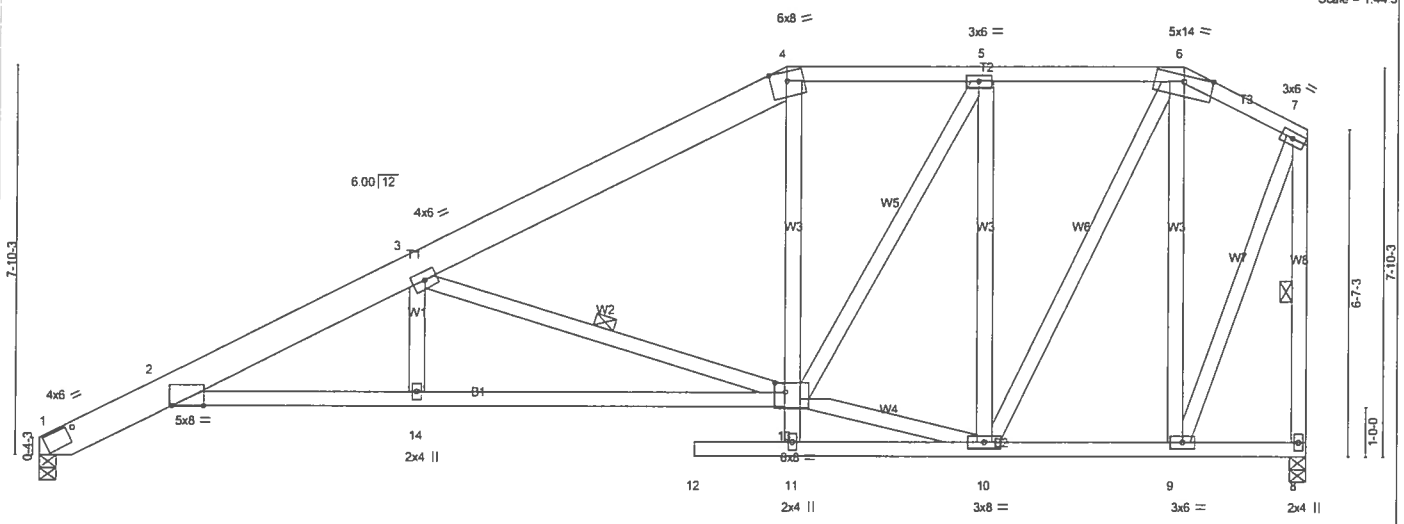
-1-6-0 | 2-0-0 | 7-7-0 | 13-2-0 | 15-0-0 | 23-0-0 | 25-6-0
1-6-0 | 2-0-0 | 5-7-0 | 5-7-0 | 1-10-0 | 8-0-0 | 2-6-0

Scale = 1:44.3




2-0-0 | 7-7-0 | 13-2-0 | 15-0-0 | 23-0-0 | 25-6-0
2-0-0 | 5-7-0 | 5-7-0 | 1-10-0 | 8-0-0 | 2-6-0

Plate Offsets (X,Y): [2:0-7-8,0-0-0], [2:1-11-13,0-6-0], [13:0-2-8,0-2-4]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.63 | Vert(LL) | -0.29 | 13-14 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.87 | Vert(TL) | -0.47 | 13-14 | >637 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.79 | Horz(TL) | 0.22 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 199 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
1 Row at midpt 13-14
WEBS 1 Row at midpt 3-13, 7-8
JOINTS 1 Brace at Jt(s): 14

**REACTIONS** (lb/size) 1=1037/0-4-0, 8=1086/0-4-0
Max Horz 1=311(load case 5)
Max Uplift1=-322(load case 5), 8=-320(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-398/0, 2-3=-2485/1000, 3-4=-1307/524, 4-5=-1066/538, 5-6=-701/365, 6-7=-396/190, 7-8=-1048/472
BOT CHORD 2-14=-1198/2349, 13-14=-1198/2349, 11-12=0/0, 10-11=-5/36, 9-10=-134/319, 8-9=-4/8
WEBS 3-14=0/285, 3-13=-1364/763, 11-13=0/129, 4-13=-3/246, 6-9=-735/377, 7-9=-375/890, 5-10=-781/417, 5-13=-335/695, 10-13=-308/685, 6-10=-363/823

**JOINT STRESS INDEX**
2 = 0.74, 2 = 0.00, 3 = 0.31, 4 = 0.20, 5 = 0.57, 6 = 0.41, 7 = 0.75, 8 = 0.41, 9 = 0.82, 10 = 0.94, 11 = 0.34, 13 = 0.60 and 14 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 322 lb uplift at joint 1 and 320 lb uplift at joint 8.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061223

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T18 | SPECIAL | 1 | 1 | Job Reference (optional) |

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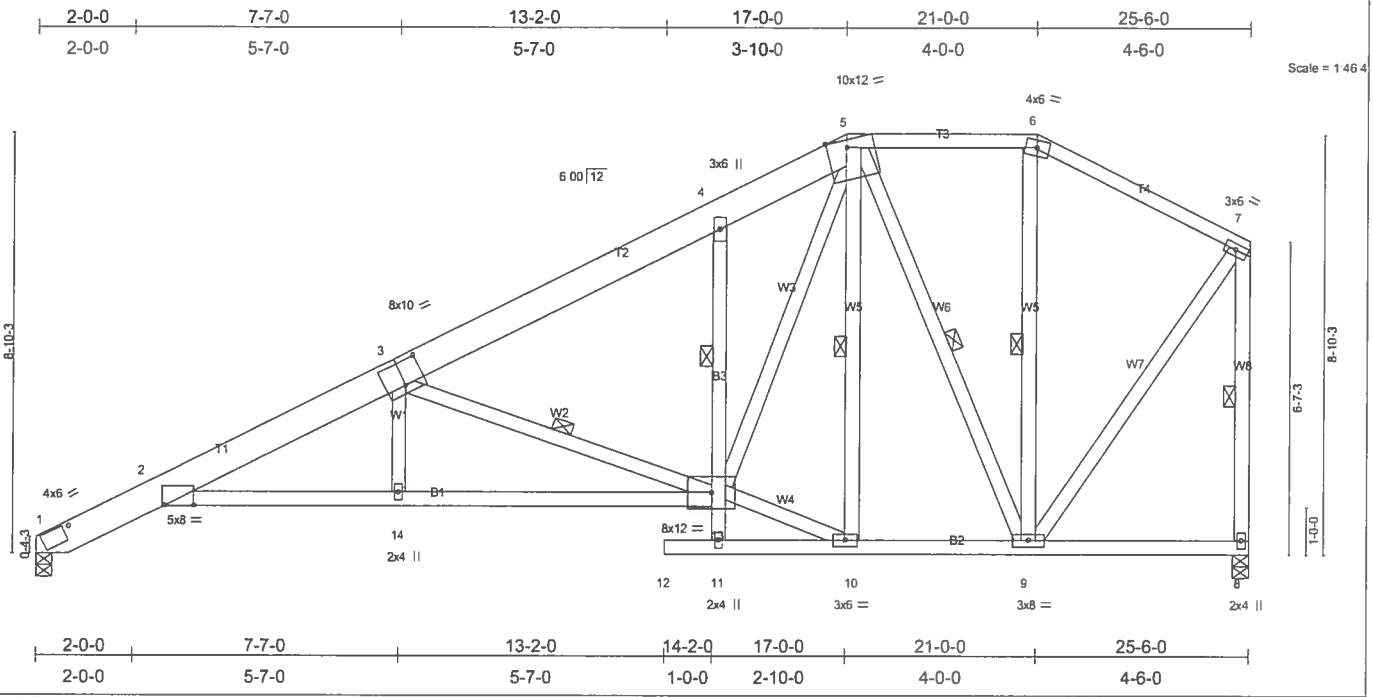



Plate Offsets (X,Y): [2:1-11-11,0-6-0], [2:0-7-8,0-0-0], [3:0-5-0,0-6-0], [5:0-5-3,Edge]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.62 | Vert(LL) | -0.27 | 2-14 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.74 | Vert(TL) | -0.44 | 2-14 | >686 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.59 | Horz(TL) | 0.22 | 8 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 204 lb | |

**LUMBER**
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T3 2 X 4 SYP No.2, T4 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-6-1 oc bracing. Except:
1 Row at midpt 4-13
WEBS 1 Row at midpt 3-13, 5-10, 5-9, 6-9, 7-8

**REACTIONS** (lb/size) 8=1074/0-4-0, 1=1028/0-4-0
Max Horz 1=325(load case 5)
Max Uplift8=-344(load case 5), 1=-329(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-394/0, 2-3=-2446/1017, 3-4=-1377/571, 4-5=-1269/685, 5-6=-482/315, 6-7=-585/296, 7-8=-1006/493
BOT CHORD 2-14=-1212/2317, 13-14=-1212/2316, 11-13=0/77, 4-13=-166/238, 11-12=0/0, 10-11=-59/0, 9-10=-325/727, 8-9=-9/15
WEBS 3-14=0/243, 3-13=-1274/727, 10-13=-284/794, 5-13=-598/1137, 5-10=-168/111, 5-9=-630/289, 6-9=-51/97, 7-9=-342/805

**JOINT STRESS INDEX**
2 = 0.66, 2 = 0.00, 3 = 0.23, 4 = 0.16, 5 = 0.29, 6 = 0.44, 7 = 0.67, 8 = 0.54, 9 = 0.83, 10 = 0.45, 11 = 0.34, 13 = 0.83 and 14 = 0.34

**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 344 lb uplift at joint 8 and 329 lb uplift at joint 1.

**LOAD CASE(S)** Standard

**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

Dwg.#0616061224

| Job | Truss | Truss Type | Qty | Ply | 0 0 |
|---|---|---|---|---|---|
| L169926 | T19 | COMMON | 1 | 1 | Job Reference (optional) |

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Jun 16 10:17:25 2006 Page 1

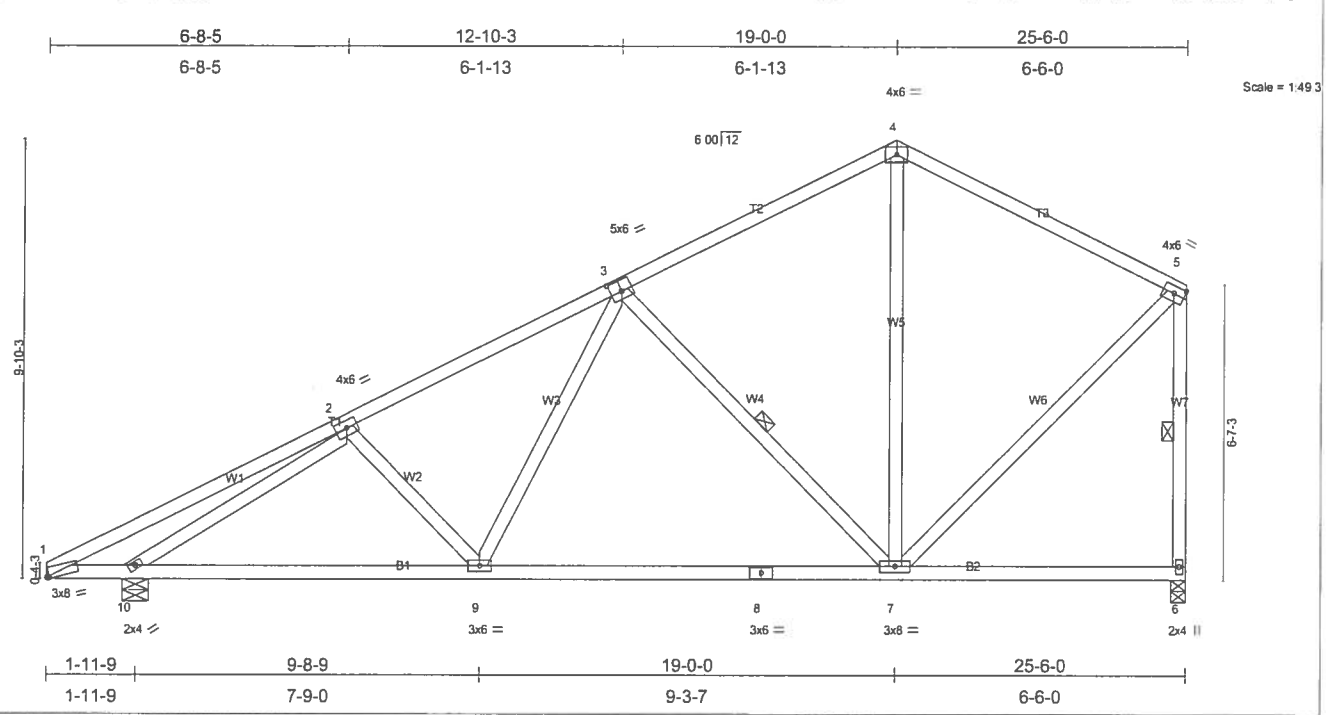



Plate Offsets (X,Y): [1:0-0-10,Edge], [3:0-3-0,0-3-0]

| LOADING (psf) | | SPACING | 2-0-0 | CSI | | DEFL | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| TCLL | 20.0 | Plates Increase | 1.25 | TC | 0.45 | Vert(LL) | -0.16 | 7-9 | >999 | 240 | MT20 | 244/190 |
| TCDL | 7.0 | Lumber Increase | 1.25 | BC | 0.45 | Vert(TL) | -0.28 | 7-9 | >999 | 180 | | |
| BCLL | 10.0 | Rep Stress Incr | YES | WB | 0.85 | Horz(TL) | 0.03 | 6 | n/a | n/a | | |
| BCDL | 5.0 | Code FBC2004/TPI2002 | | (Matrix) | | | | | | | Weight: 155 lb | |

**LUMBER**
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

**BRACING**
TOP CHORD Structural wood sheathing directly applied or 5-5-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-4-9 oc bracing.
WEBS 1 Row at midpt 3-7, 5-6

**REACTIONS** (lb/size) 10=1143/0-7-3, 6=987/0-4-0
Max Horz 10=338(load case 5)
Max Uplift10=-365(load case 5), 6=-350(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-105/72, 2-3=-1278/555, 3-4=-653/356, 4-5=-651/349, 5-6=-911/482
BOT CHORD 1-10=0/130, 9-10=-724/1159, 8-9=-502/907, 7-8=-502/907, 6-7=-18/28
WEBS 2-10=-1396/643, 2-9=-127/245, 3-9=-132/394, 3-7=-578/393, 4-7=-23/232, 5-7=-298/700

**JOINT STRESS INDEX**
1 = 0.94, 2 = 0.52, 3 = 0.52, 4 = 0.83, 5 = 0.76, 6 = 0.68, 7 = 0.69, 8 = 0.51, 9 = 0.46 and 10 = 0.75

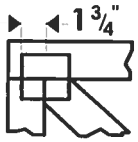
**NOTES**
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 365 lb uplift at joint 10 and 350 lb uplift at joint 6.

**LOAD CASE(S)** Standard

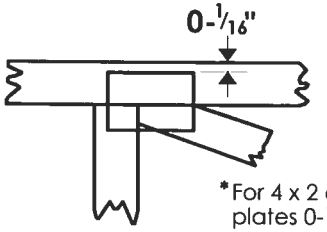
**JUNE 16, 2006 TRUSS DESIGN ENGINEER:**
**THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987**
**STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196**
**16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549**

# Symbols

## PLATE LOCATION AND ORIENTATION




*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.




*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

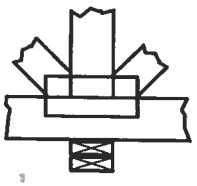
## PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

## BEARING

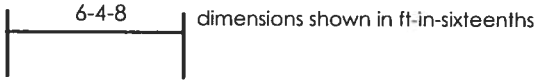


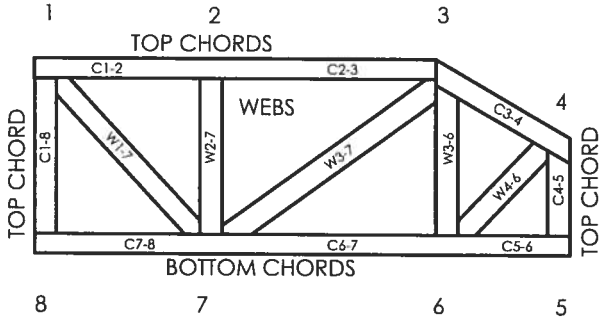
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

**Industry Standards:**

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System




**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

## CONNECTOR PLATE CODE APPROVALS

| | |
|---|---|
| BOCA | 96-31, 95-43, 96-20-1, 96-67, 84-32 |
| ICBO | 4922, 5243, 5363, 3907 |
| SBCCI | 9667, 9730, 9604B, 9511, 9432A |

MiTek®

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**MiTek Engineering Reference Sheet: MII-7473**

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.