DATE 01/04/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000025366

APPLICANT CHARESE NORTON PHONE 386.752.3331

ADDRESS 3367 S US HWY 441, STE 101 LAKE CITY FL 32025

OWNER PAUL TROIANO PHONE 386.961.9328

ADDRESS 269 SW BRIARBROOK PLACE LAKE CITY FL 32025

CONTRACTOR JAMES H. NORTON PHONE 386.752.3331

LOCATION OF PROPERTY 47-S TO C-242,TR TO MOCKINGBIRD WAY,TL TO BRIARBROOK PL, TR
IT'S THE 6TH LOT ON THE R.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 87000.00

HEATED FLOOR AREA 1740.00 TOTAL AREA 2770.00 HEIGHT 20.60 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 25-4S-16-03121-066 SUBDIVISION PICCADILLY PARK

LOT 14 BLOCK D PHASE SOUTH UNIT TOTAL ACRES 0.50

		RB0031780	Charese S. Norton	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X32'MITERED	06-01107N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD.

Check # or Cash 22690

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by Pool _____ date/app. by

Reconnection _____ date/app. by Pump pole _____ date/app. by Utility Pole _____ date/app. by

M/H Pole _____ date/app. by Travel Trailer _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 435.00 CERTIFICATION FEE $ 13.85 SURCHARGE FEE $ 13.85

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 537.70

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY, AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE. PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Called 12-28-06 LH

Columbia County Building Permit Application

For Office Use Only Application # 0612-74 Date Received 12-27-06 By LH Permit # 28971 25366

Application Approved by - Zoning Official BLK Date 29.12.06 Plans Examiner OK JTH Date 12-29-06

Flood Zone X Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low DEN.

Comments SITE PLAN PAGE 1 OF PLANS

NOC EH Deed or PA Site Plan NA State Road Info Parent Parcel # Development Permit

Name Authorized Person Signing Permit Charese Norton Phone 386-752-3331 Fax 386-752-6427

Address 3367 S US HWY 441, Ste 101, Lake City, FL 32025

Owners Name Paul Troiano Phone 386-961-9328

911 Address 269 SW Briarbrook Pl, Lake City, FL 32024

Contractors Name James H. NORTON Phone 386-752-3331

Address 3367 S. US HWY 441, Ste 101, Lake City, FL 32025

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address Tim Delbene, 192 SW Sagewood Gln, Lake City, FL 32024
Mark Disaway, POB 868, Lake City, FL 32056

Mortgage Lenders Name & Address NA

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number R03121-066 25-4S-16 Estimated Cost of Construction 150000.00

Subdivision Name Piccadilly Park South Lot 14 Block D Unit – Phase –

Driving Directions HWY 47S to CR 242 turn right, go to SW mockingbird way turn left, go to SW Briarbrook PL turn right, 6th lot on Right.

Type of Construction SFD, new home Const. Number of Existing Dwellings on Property 0

Total Acreage 1/2 Lot Size _____ Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 59' Side 45'8" Side 46' Rear 50'

Total Building Height 20'6" Number of Stories 1 Heated Floor Area 1740 TOTAL 2776 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James H Norton
Owner Builder or Authorized Person by Notarized Letter

STATE OF FLORIDA
COUNTY OF COLUMBIA

PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

Sworn to (or affirmed) and subscribed before me this 22 day of Dec 2006.

Personally known ___ or Produced Identification ___

James H Norton
Contractor Signature
Contractors License Number RB0031780
Competency Card Number 3553
NOTARY STAMP/SEAL

Patricia T. Peeler
Notary Signature

(Revised Sept. 2006)

± 2690

CERTIFICATE OF

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 25-4S-16-03121-066 **Building permit No.** 000025366

Use Classification SFD/UTILITY **Fire:** 0.00

Permit Holder JAMES H. NORTON **Waste:**

Owner of Building PAUL TROIANO **Total:** 0.00

Location: 269 SW BRIARBROOK PLACE, LAKE CITY, FL

Date: 05/21/2007

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Permit #25316

Cal-Tech Testing, Inc.

• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
6919 Distribution Ave. S., Unit #5, Jacksonville, FL 32257 • Tel(904)262-4046 • Fax(904)262-4047
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 07-019
DATE TESTED: 1/12/07
DATE REPORTED: 1/16/07

PROJECT:	Piccadilly Park Lot # 14, Lake City, FL
CLIENT:	Norton Home Improvement, 3367, S. US Hwy 441, Suite 101, Lake City, FL 32025
GENERAL CONTRACTOR:	Norton Home Improvement
EARTHWORK CONTRACTOR:	Norton Home Improvement
INSPECTOR:	Chad Day

ASTM METHOD	SOIL USE
(D-2922) Nuclear	OTHER

SPECIFICATION REQUIREMENTS: 95%

TEST NO.	TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1	SE Corner of Footer 2' North	12"	107.9	8.0	99.9	1	104.5	95.6%
2	Approx. Center of South Footer	12"	109.1	8.7	100.4	1	104.5	96.0%
3	Approx. Center of Footer in Center of House Foundation	12"	110.0	10.5	99.5	1	104.5	95.3%
4	NE Corner of Footer 5' South	12"	111.3	7.7	103.3	1	104.5	98.9%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1	Tan Fine Sand w/Trace of Silt	104.5	13.2	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

EE

Reviewed By:

Robert W Clark

Date: 1/18/07
Florida Registration No: 52210

Columbia County Property Appraiser

DB Last Updated: 11/20/2006

Parcel: 25-4S-16-03121-066 HX 13

Tax Record | Property Card | Interactive GIS Map | Print

2007 Proposed Values

Search Result: 1 of 1

Owner & Property Info

Owner's Name	TROIANO PAUL A & ANNETTE		
Site Address	BRIARBROOK		
Mailing Address	287 SW BRIARBROOK PL LAKE CITY, FL 32024		
Use Desc. (code)	SINGLE FAM (000100)		
Neighborhood	25416.02	**Tax District**	2
UD Codes	MKTA06	**Market Area**	06
Total Land Area	0.000 ACRES		
Description	LOTS 14 & 15 BLOCK D PICCADILLY PARK SOUTH S/D. ORB 906-1897,		

GIS Aerial

Property & Assessment Values

Mkt Land Value	cnt: (1)	$34,850.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (1)	$195,123.00
XFOB Value	cnt: (6)	$25,502.00
Total Appraised Value		$255,475.00

Just Value	$255,475.00
Class Value	$0.00
Assessed Value	$189,965.00
Exempt Value	(code: HX 13) $189,965.00
Total Taxable Value	$0.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
7/20/2000	906/1897	WD	V	Q		$20,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	2000	Common BRK (19)	2491	4064	$195,123.00
	Note: All S.F. calculations are based on exterior building dimensions.					

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0190	FPLC PF	2000	$1,600.00	1.000	0 x 0 x 0	(.00)
0280	POOL R/CON	2000	$7,020.00	240.000	24 x 10 x 0	(.00)
0282	POOL ENCL	2000	$8,177.00	740.000	37 x 20 x 0	(.00)
0166	CONC,PAVMT	2000	$1,509.00	1006.000	0 x 0 x 0	(.00)
0260	PAVEMENT-A	2000	$3,196.00	3196.000	0 x 0 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000100	SFR (MKT)	2.000 LT - (.000AC)	1.00/1.00/.85/1.00	$17,425.00	$34,850.00

Columbia County Property Appraiser DB Last Updated: 11/20/2006

1 of 1

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

****** THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.******

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number R03121-066 25-4S-16

1. **Description of property: (legal description of the property and street address or 911 address)**
   25-4S-16-03121-066, Picadilly Park South Lot 14, Block D
   269 SW Briarbrook Place, Lake City, FL 32024
2. **General description of improvement:** New Home Construction
3. **Owner Name & Address** Paul Troiano, 269 SW Briarbrook Place, Lake City, FL 32024
   Interest in Property Own
4. **Name & Address of Fee Simple Owner (if other than owner):** NA
5. **Contractor Name** James H. Norton **Phone Number** 386-752-3331
   Address 3367 S US Hwy 441, Suite 101, Lake City, FL 32025
6. **Surety Holders Name** NA **Phone Number**
   Address
   Amount of Bond NA
   Inst:2006030188 Date:12/27/2006 Time:12:04
   DC,P.DeWitt Cason,Columbia County B:1105 P:2375
7. **Lender Name** NA
   Address
8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name Norton Home Improvement Co., Inc **Phone Number** 386-752-3331
   Address 3367 S US Hwy 441, Suite 101, Lake City, FL 32025
9. **In addition to himself/herself the owner designates** James H. Norton of Norton Home Improvement **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee** 386-752-3331
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)**

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Paul Troiano
Signature of Owner

Sworn to (or affirmed) and subscribed before day of 22 Dec, 2006

NOTARY STAMP/SEAL

PATRICIA T. PEELER
Notary Public, State of Florida
My comm. exp. Sep. 5, 2010
Comm. No. DD 579471

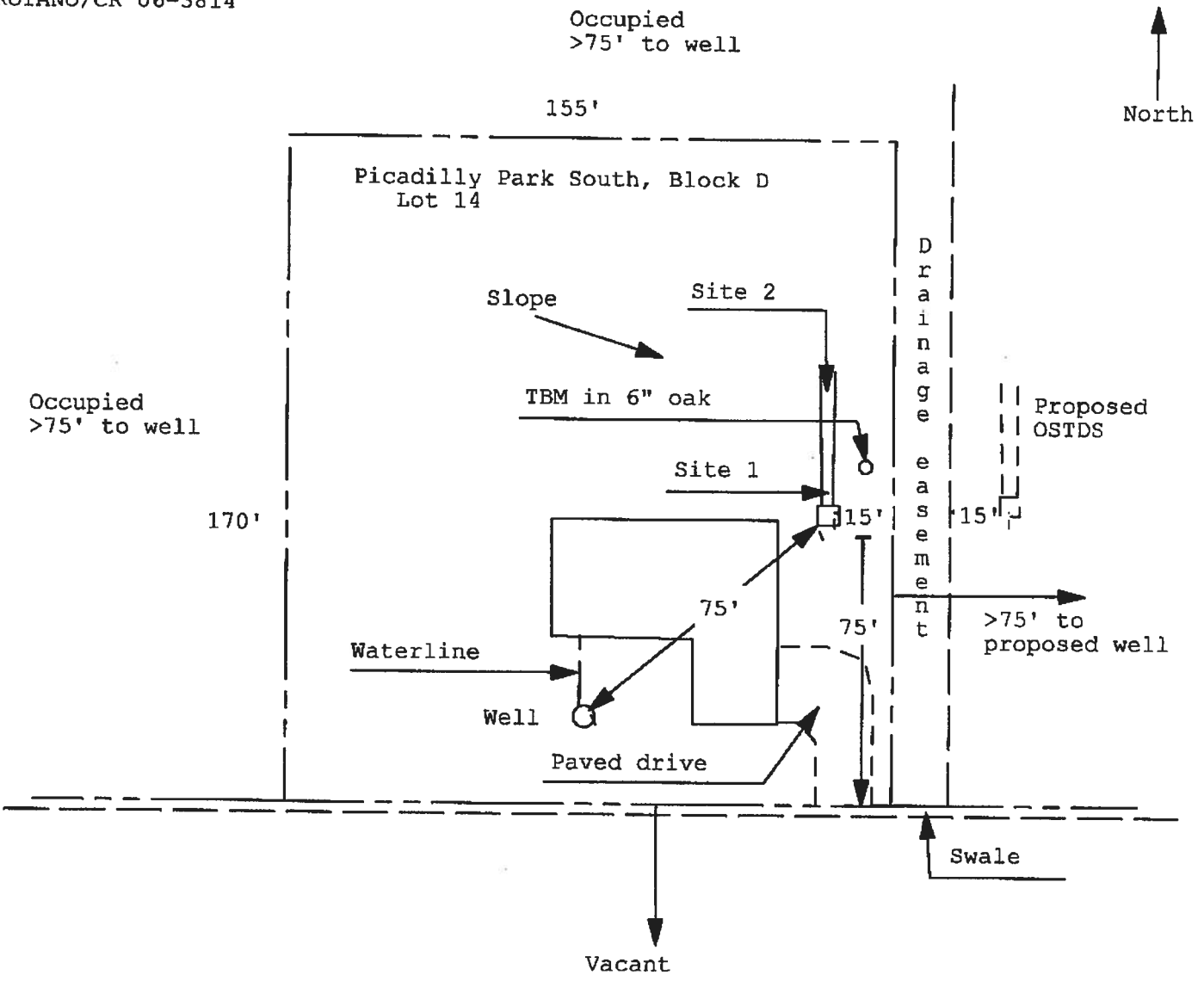
Patricia T. Peeler
Signature of Notary

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-01107N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

TROIANO/CR 06-3814



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd Date 12/14/06

Plan Approved ✓ Not Approved ____ Date 12/20/6

By [signature] APPROVED Columbia CHD CPHU

Notes: ____

LYNCH WELL DRILLING, INC.

173 SW Tustenuggee Ave
Lake City, FL. 32025
Phone 386-752-6677
Fax 386-752-1477

Building Permit #_____ Owner's Name. Paul Troiano

Well Depth_____ Ft Casing Depth _____Ft Water Level _____Ft

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model S20-100 HP 1

System Pressure (PSI) On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20(GPM)

Tank Installation: Bladder /Galvanized Make Challenger

Model PC 244 Size 81 gallon

Tank Draw-down per cycle at system pressure 25.1 gallons

I HEREBY VERTIFY THAT THIS WATER WELL SYSTEM HAS BEEN INSTALLED AS PER THE ABOVE INFORMATION.

Linda Newcomb
Signature

Linda Newcomb
Print Name

2609
License Number

12-13-06
Date

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001287

DATE 01/04/2007 PARCEL ID # 25-4S-16-03121-066

APPLICANT CHARESE NORTON PHONE 386.752.3331

ADDRESS 3367 S US HWY 441, STE 101 LAKE CITY FL 32025

OWNER PAUL TROIANO PHONE 386.961.9328

ADDRESS 269 SW BRIARBROOK PLACE LAKE CITY FL 32025

CONTRACTOR JAMES H. NORTON PHONE 386.752.3331

LOCATION OF PROPERTY 47-S TO C-242,TR TO MOCKINGBIRD WAY,TL TO BRIARBROOK PL, TR AND IT'S THE 6TH LOT ON THE R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT PICCADILLY PARK 14 D SOUTH

SIGNATURE Charese L. Norton

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

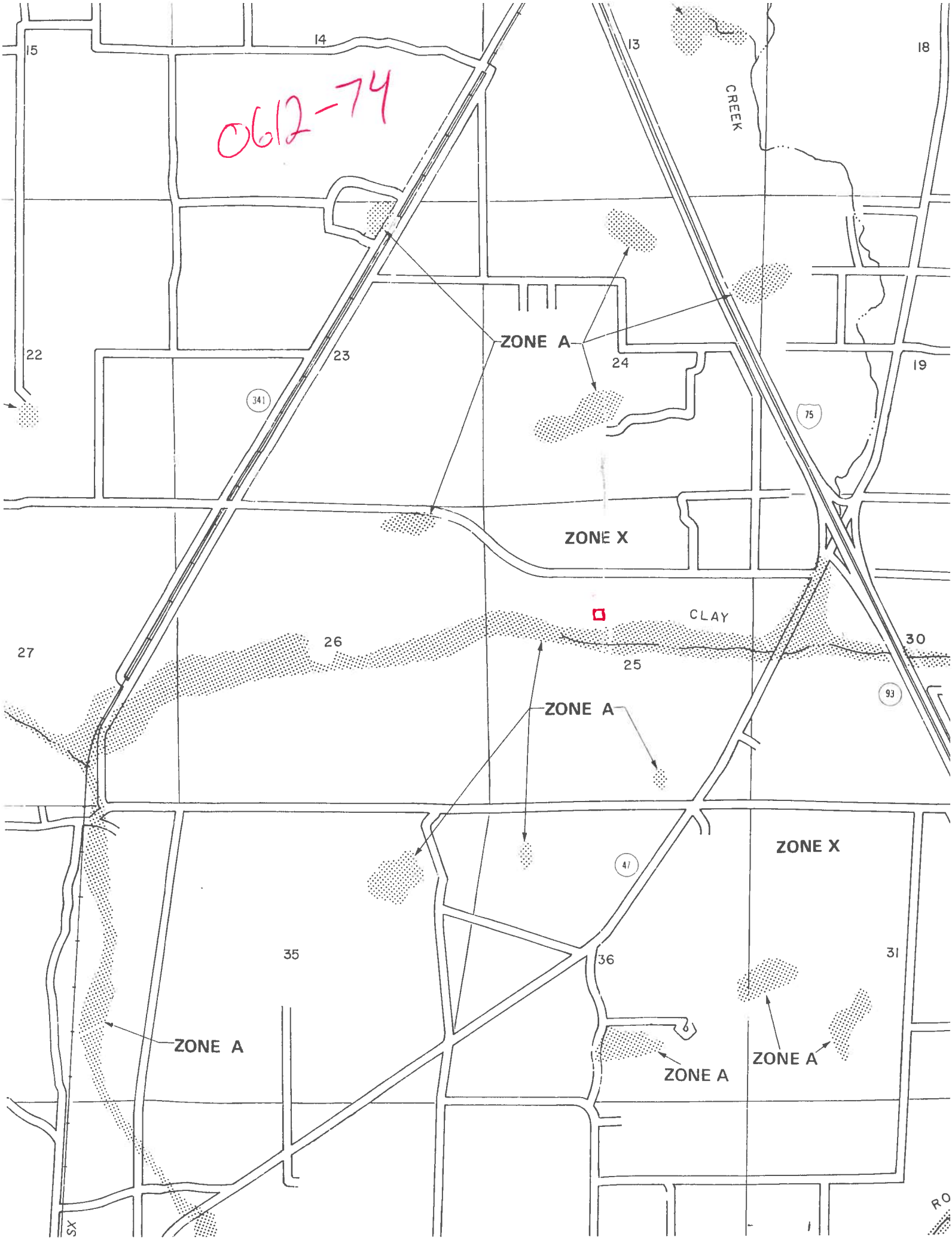
[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA


0612-74
15
14
13
18
CREEK
22
23
24
19
341
75
ZONE A
ZONE X
CLAY
27
26
25
30
93
ZONE A
ZONE X
41
35
36
31
ZONE A
ZONE A
ZONE A
SX
RO

** LAMAR BOOZER **		PROJECT:	CUSTOM
900 EAST PUTNAM STREET		CLIENT:	NORTON BUILDERS
LAKE CITY, FL 32055		DATE:	12 17 06
RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS		DESIGNER:	LAMAR BOOZER

CLIENT INFORMATION:

NAME: NORTON BUILDERS
ADDRESS:
CITY, STATE: LAKE CITY FL

TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS	AREA QUAN	SEN. LOSS	LAT. + GAIN	SEN. = GAIN	TOTAL GAIN
3-C WINDOW DBL PANE CLR GLS METL FR	133	4,342	0	3,862	3,862
9-I FRENCH DOOR DBL CLR GLS METL FR	42	1,425	0	689	689
12-D WALL R-11 +1/2"ASPHLT BRD(R-1.3)	1,497	5,388	0	2,946	2,946
13-C PART R-11 + 1/2" GYPSUM(R-0.5)	112	227	0	161	161
11-C DOOR METAL POLYSTYRENE CORE	42	888	0	486	486
16-G CEILING R-30 INSULATION	1,740	2,824	0	2,824	2,824
22-A SLAB ON GRADE NO EDGE INSUL	182	6,633	0	0	0
SUBTOTALS FOR STRUCTURE:	3,748	21,727	0	10,968	10,968
PEOPLE	19	0	4,370	5,700	10,070
APPLIANCES	0	0	0	1,500	1,500
DUCTWORK	0	2,018	0	2,203	2,203
INFILTRATION W.CFM: 376.5 S.CFM: 167.3	0	18,637	5,576	3,865	9,441
VENTILATION W.CFM: 0.0 S.CFM: 0.0	0	0	0	0	0
SENSIBLE GAIN TOTAL				24,236	
TEMP. SWING MULTIPLIER				X 1.00	
BUILDING LOAD TOTALS		42,381	9,946	24,236	34,182

SUPPLY CFM AT 20 DEG DT:	1,102	CFM PER SQUARE FOOT:	0.579
SQUARE FT. OF ROOM AREA:	1,740	SQUARE FOOT PER TON:	667.720

TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 42.381 MBH
TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.849 TONS

CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

25366

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
Laboratories

P.O. Box 1625 • Lake City, FL 32056-1625 • Tel(386)755-3633 • Fax(386)752-5456
6919 Distribution Ave. S., Unit #5, Jacksonville, FL 32257 • Tel(904)262-4046 • Fax(904)262-4047
2230 Greensboro Hwy • Quincy, FL 32351 • Tel(850)442-3495 • Fax(850)442-4008

REPORT OF IN-PLACE DENSITY TEST

JOB NO.: 07-019
DATE TESTED: 1/12/07
DATE REPORTED: 1/16/07

PROJECT:	Piccadilly Park Lot # 14, Lake City, FL
CLIENT:	Norton Home Improvement, 3367, S. US Hwy 441, Suite 101, Lake City, FL 32025
GENERAL CONTRACTOR:	Norton Home Improvement
EARTHWORK CONTRACTOR:	Norton Home Improvement
INSPECTOR:	Chad Day

ASTM METHOD	SOIL USE
(D-2922) Nuclear	OTHER

SPECIFICATION REQUIREMENTS: 95%

TEST NO.		TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft³)	MOISTURE PERCENT	DRY DENSITY (lb/ft³)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
1		SE Corner of Footer 2' North	12"	107.9	8.0	99.9	1	104.5	95.6%
2		Approx. Center of South Footer	12"	109.1	8.7	100.4	1	104.5	96.0%
3		Approx. Center of Footer in Center of House Foundation	12"	110.0	10.5	99.5	1	104.5	95.3%
4		NE Corner of Footer 5' South	12"	111.3	7.7	103.3	1	104.5	98.9%

REMARKS: The Above Tests Meet Specification Requirements.

PROCTORS

PROCTOR NO.		SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft³)	OPT. MOIST.	TYPE
1		Tan Fine Sand w/Trace of Silt	104.5	13.2	MODIFIED (ASTM D-1557)

Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer, CEO, DBE

Linda M. Creamer
President - CEO

EE

Reviewed By:

Robert W Clark

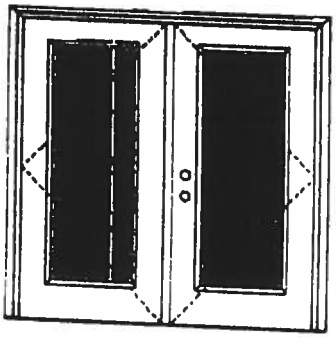
Date: 1/18/07
Florida Registration No: 52210

XX
Glazed Inswing Unit

COP-WL-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
Maximum unit size = 6'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0002-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


106, 160 Series*


120 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 6-panel; 6-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson EntrySystems

March 29, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

PREMDOOR Collection Premium Quality Doors

Exclusively from Masonite

Masonite International Corporation

XX
Glazed Inswing Unit

COP-WI-JH4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:
3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Johnson Entry Systems

March 28, 2002

PREMDOOR Collection Premium Quality Doors

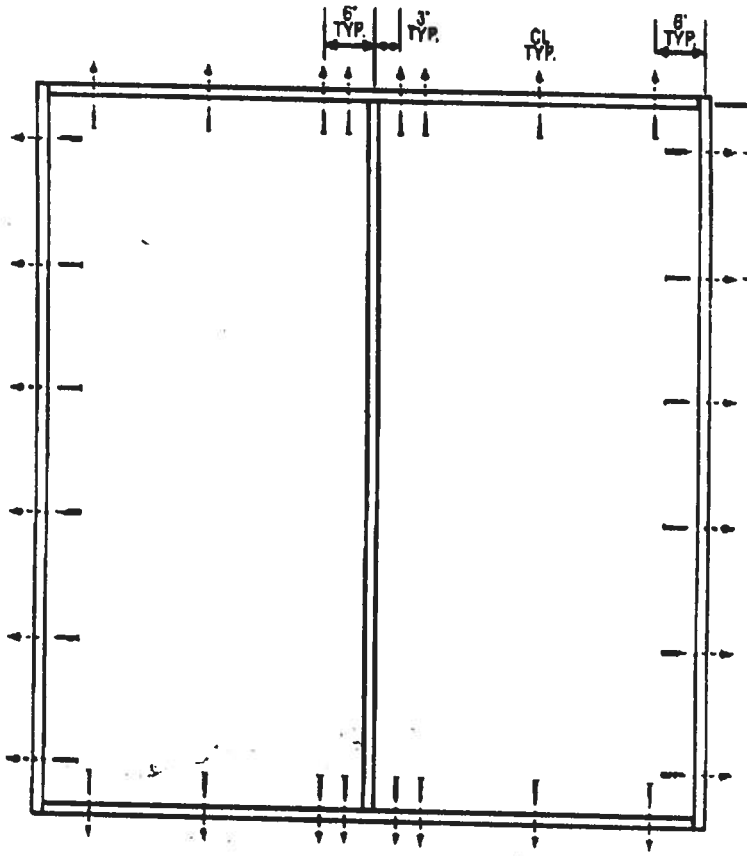
Exclusively from Masonite

Masonite International Corporation

XX
Unit

MID-WL-MA0002-02

DOUBLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 8 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

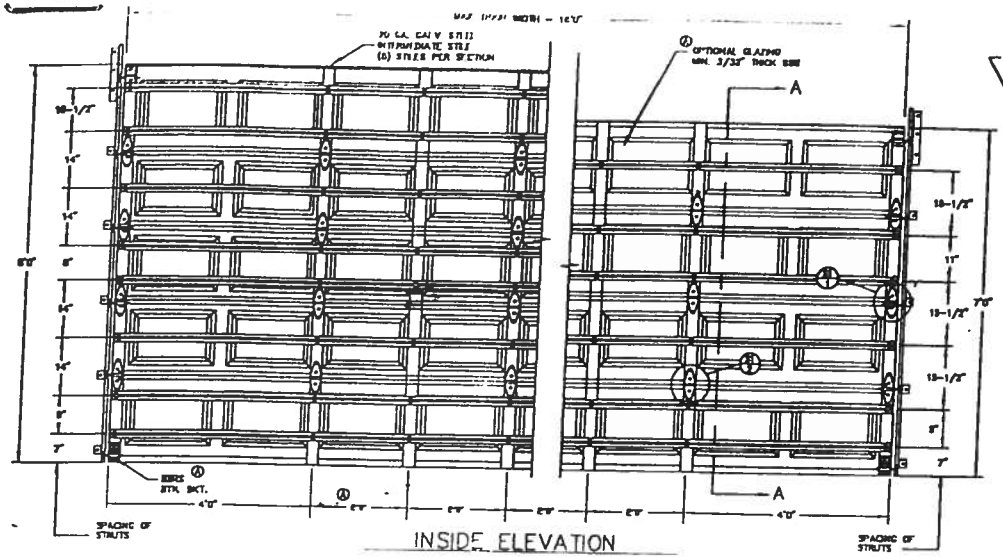
Latching Hardware:

- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylinderical and deadlock hardware be installed.

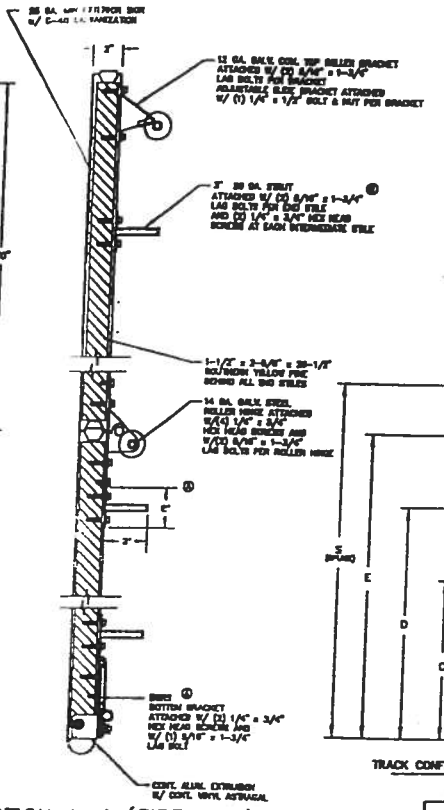
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and a minimum of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

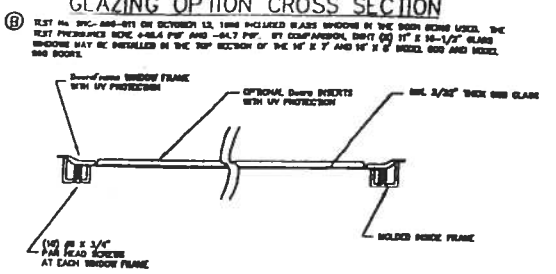
16x7



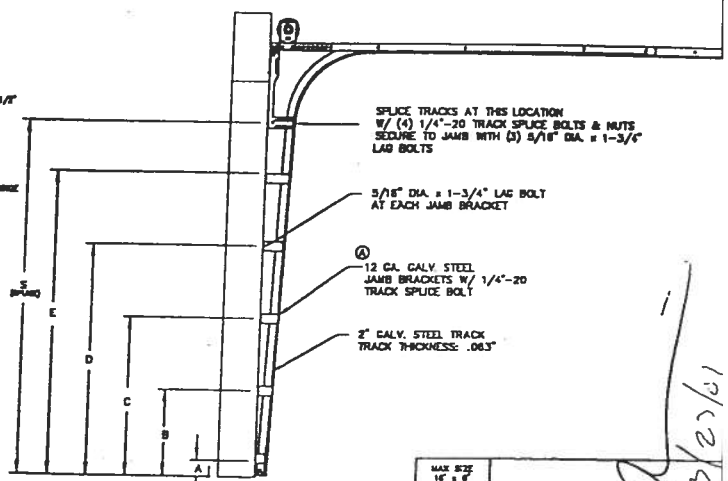
INSIDE ELEVATION



SECTION A-A (SIDE VIEW)



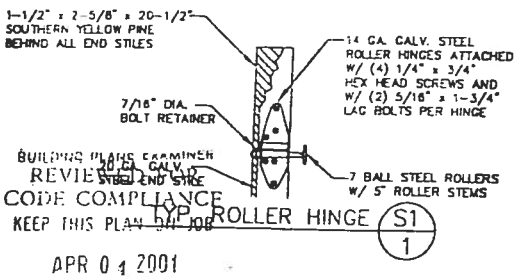
GLAZING OPTION CROSS SECTION



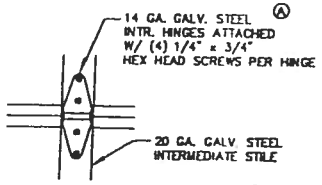
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"



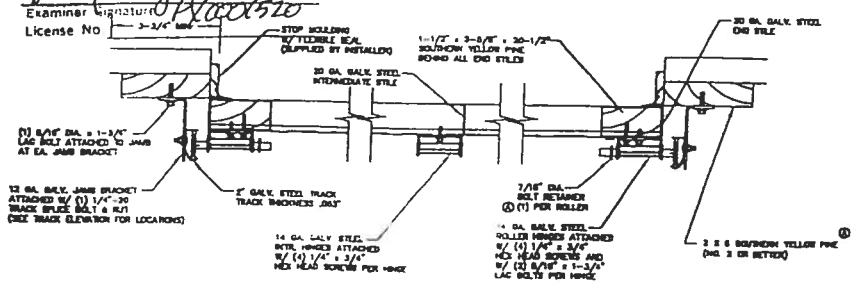
TYP. ROLLER HINGE (S1/1)



TYP. HINGE CONNECTION (S1/2)

BUILDING PLANS EXAMINER
REVIEWED FOR
CODE COMPLIANCE
KEEP THIS PLAN ON JOB
APR 04 2001

Building & Zoning Inspection Div Jax., FL
Examiner Signature
License No



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 100 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" x 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 3 OOD PR CONCRETE
HILTI KWIK BOLT 3 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 3-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-RO BLOCK
HILTI SLEEVE ANCHOR 3/8" X 3-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 3-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 12" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
**PREPARATION OF WOOD JAMBS BY OTHERS

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.018") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS
   A. BASIC WIND SPEED OF 100 MPH
   B. DOOR CAN BE INSTALLED WITH 8 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

MAX SIZE 16' x 8'

DESIGN LOADS: +24.3 PSF, -28.3 PSF

TEST LOADS: +36.5 PSF, -38.5 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY
	ADDED GLAZING CROSS SECTION DETAIL		
	ADDED CHANGES REFER TO DRAWINGS DATED 02/23/01		
	ADDED DIMENSIONS ON STRUT		
	CHANGED TO 6065 BOTTOM BRACKET		
	ADDED CENTER SILL DIMENSIONS		
A	REVISED CRITERIA USED TO CALCULATE WIND LOAD	03/26/01	RJ
	REVISED TRACK DETAIL AND CHECKED CHART FOR 7' & 8' TALL		
	ADDED WOOD JAMB ATTACHMENT TO STRUCTURE		
	ADDED 2 X 6 WOOD JAMB INFORMATION		
	ADDED DESIGN LOADS		
	ADDED GLAZING COMPRESSOR		
B	ADDED STRUT ASSESSMENT	05/01/01	RJ
	INCLUDED FASHION-MLE WIND SPEED RATING		
	CHANGED TEXT IN NOTE 6-B		

Amarr

MODELS #950 & #600
Short Panel & Flush

SIZE B | DRAWN BY MC | DATE 02/23/01 | CHECKED BY MC | DATE 02/23/01 | DRAWING NUMBER FLA-570-001 | SHEET 1 OF 1

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

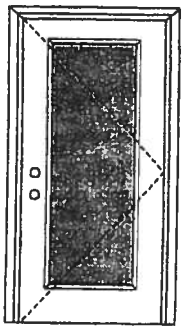
2300 35th STREET P.O. BOX 2149 TUSCALOOSA

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7-national, state or local building codes specify the edition required.

Warnock Hersey

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etisamko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

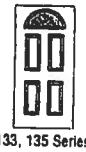
MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:

1/4 GLASS:


*Oval Series


133, 135 Series


136 Series


680 Series


822 Series

1/2 GLASS:

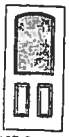

105 Series*


106, 160 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass kit may also be used in the following door styles: 5-panel; 5-panel with scroll; Eyebrow 5-panel; Eyebrow 5-panel with scroll.

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Warnock Hersey

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WH website (www.etlsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Johnson™ EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 3500 Twin
TYPE: Mulled PVC Single Hung Window

Title	Summary of Results
AAMA Rating	H-R15 96 x 78
Operating Force	19 lb max.
Air Infiltration	0.10 cfm/ft^2
Water Resistance Test Pressure	5.25 psf
Uniform Load Deflection Test Pressure	15.0 psf
Uniform Load Structural Test Pressure	22.5 psf
Forced Entry Resistance	Grade 10

Reference should be made to ATI Report No. 01-45879.01 for complete test specimen description and data.

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

Report No: 01-45879.01
Test Date: 06/03/03
And: 06/04/03
Report Date: 06/24/03
Expiration Date: 06/03/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on a Series/Model 3500 Twin, mulled PVC single hung window at their test facility in Elizabethville, Pennsylvania. The sample tested successfully met the performance requirements for an H-R15 96 x 78 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 3500 Twin

Type: Mulled PVC Single Hung Window

Overall Size: 8' 0-1/4" wide by 6' 6-1/8" high

Active Sash Size (2): 3' 10" wide by 3' 2-1/2" high

Fixed Daylight Opening Size (2): 3' 8" wide by 2' 11-1/4" high

Screen Size: 3' 9" wide by 3' 1-3/4" high

Finish: All PVC was white.

Glazing Details: The window utilized 7/8" thick sealed insulating glass constructed from two sheets of 3/32" thick clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed onto single-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone 717 764 7700
fax 717 764.4129
www.architest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
3/16" round foam filled vinyl bulb seal	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	1 Row	Interior vertical sill leg, interior meeting rail and stiles
1/4" round foam filled vinyl bulb seal with single leaf	1 Row	Bottom rail
0.310" high by 0.187" backed polypile with center fin	1 Row	Stiles

Frame Construction: The frame was constructed of extruded vinyl with mitered and welded corners. End caps were utilized on the ends of the meeting rails and secured with two #6 x 5/8" screws per cap. The fixed meeting rails were then secured to the frame utilizing two #6 x 5/8" screws. The windows were mulled together utilizing interior and exterior snap-in caps. Silicone was utilized at the head and sill mullion points.

Sash Construction: The sash were constructed of extruded vinyl with mitered and welded corners.

Screen Construction: The screen frames were constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Metal sweep locks with keepers	4	7" from stiles on interior meeting rails with keepers adjacent
Constant force balance assembly	4	One per jamb
Tilt latch	4	Each end of active meeting rails
Metal tilt pin	4	Each end of bottom rails
Tension springs	4	5" from top rail ends of screens

Test Specimen Description: (Continued)

Drainage:

Description	Quantity	Location
1/8" wide by 1" long weephole	4	2-1/4" from sill ends and mullion on sill face
3/16" wide by 1/2" long weephole	8	Two per corner through sill interior walls
1/16" wide by 1/2" long weephole	8	Two 2-1/2" from bottom rail ends
3/16" wide by 1/2" long weephole	4	1-1/4" from jambs in bottom rail glazing channels

Reinforcement: Sash rails contained a roll-formed steel "I" reinforcement (drawing #GV1-451-020). The fixed meeting rail contained a roll-formed steel reinforcement (drawing #RF-1045-020).

Installation: The windows were installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured utilizing #8 x 1-5/8" drywall screws located in the corners and 9" on center around the nail fin perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	19 lbs	30 lbs max.
2.1.2	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.10 cfm/ft^2	0.3 cfm/ft^2 max.

Note #1: *The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Deflections reported were taken on the mullion) (Loads were held for 52 seconds)		
	@ 15.0 psf (positive)	1.27"	See Note #2
	@ 15.0 psf (negative)	1.18"	See Note #2

Note #2: *The Uniform Load Deflection test is not an AAMA/NWWDA 101/I.S.2-97 requirement for this product designation. The data is recorded in this report for information only.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Permanent Sets reported were taken on the mullion) (Loads were held for 10 seconds)		
	@ 22.5 psf (positive)	0.17"	0.29" max.
	@ 22.5 psf (negative)	0.18"	0.29" max.
2.2.1.6.2	Deglazing Test (ASTM E 987-88) In operating direction at 70 lbs		
	Right sash, meeting rail	0.13"/25%	0.50"/100%
	Right sash, bottom rail	0.13"/25%	0.50"/100%
	Left sash, meeting rail	0.13"/25%	0.50"/100%
	Left sash, bottom rail	0.13"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Right sash, right stile	0.06"/13%	0.50"/100%
	Right sash, left stile	0.06"/13%	0.50"/100%
	Left sash, right stile	0.03"/6%	0.50"/100%
	Left sash, left stile	0.03"/6%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Steven M. Urich, P.E.
Senior Project Engineer

MAH:baw
01-45879.01

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.

SERIES/MODEL: 8500/1250
PRODUCT TYPE: PVC Single Hung Window

	Summary of Results		
Title	**Test Specimen #1**	**Test Specimen #2**	**Test Specimen #3**
Rating	H-R25 48 x 78	H-R35* 36 x 72	H-R25* 40 x 83
Operating Force	21 lbf max.	N/A	N/A
Air Infiltration	0.15 cfm/ft^2	N/A	N/A
Water Resistance Test Pressure	6.0 psf	N/A	N/A
Uniform Load Deflection Test Pressure	±25.0 psf	+35.0 psf/-40.0 psf	±25.0 psf
Uniform Load Structural Test Pressure	±37.5 psf	+52.5 psf/60.0 psf	±37.5 psf
Forced Entry Resistance	Grade 10	N/A	N/A

Reference should be made to ATI Report No. 56448.02-122-47 for complete test specimen description and data.

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI WINDOWS AND DOORS, INC.
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No.:	56448.02-122-47
Test Date:	03/17/05
And:	03/18/05
Report Date:	03/29/05
Expiration Date:	03/18/09

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Windows and Doors, Inc. to witness testing on three Series/Model 8500/1250, PVC single hung windows at MI Windows and Doors, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: H-R25 48 x 78; Test Specimen #2: H-R35* 36 x 72; Test Specimen #3: H-R25* 40 x 83. Test specimen description and results are reported herein.

***General Note**: An asterisk (*) next to the performance grade indicates that the size tested for optional performance was smaller than the Gateway test size for the product type and class.*

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 8500/1250

Product Type: PVC Single Hung Windows

Test Specimen #1: H-R25 48 x 78

Overall Size: 4' 0" wide by 6' 6" high

Interior Sash Size: 3' 9-1/2" wide by 3' 1-3/4" high

Daylight Opening Size: 3' 6-7/8" wide by 2' 10-3/4" high

Screen Size: 3' 8" wide by 3' 1" high

130 Derry Court
York, PA 17402-9405
phone: 717-764-7700
fax: 717-764-4129
www.archtest.com

Test Specimen Description: (Continued)

Test Specimen #2: H-R35* 36 x 72

Overall Size: 3' 0" wide by 6' 0" high

Interior Sash Size: 2' 9-5/8" wide by 2' 10-3/4" high

Daylight Opening Size: 2' 6-3/4" wide by 2' 7-3/4" high

Screen Size: 2' 8" wide by 2' 9-1/2" high

Test Specimen #3: H-R25* 40 x 83 (Oriel)

Overall Size: 3' 4-1/4" wide by 6' 11-1/4" high

Interior Sash Size: 3' 1-1/2" wide by 2' 4-3/4" high

Daylight Opening Size: 2' 10-3/4" wide by 4' 0-7/8" high

Screen Size: 3' 0-1/8" wide by 2' 4" high

The following descriptions apply to all specimens.

Finish: All PVC was white.

Glazing Details: The test specimens utilized 7/8" thick, sealed insulating glass fabricated from two sheets of 3/32" thick, clear annealed glass and a metal reinforced butyl spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Interior vertical sill leg, active meeting rail, and stiles
0.290" high by 0.187" backed polypile with center fin	1 Row	Stiles
1/8" round vinyl foam filled polypile with center fin	1 Row	Fixed meeting rail
5/16" round vinyl foam filled bulb seal	1 Row	Bottom rail

Test Specimen Description: (Continued)

Frame Construction: The frame was constructed of extruded vinyl with mitered and welded corners. End caps were utilized on the ends of the fixed meeting rail and secured with three #6 x 5/8" flat head screws through the end cap into the fixed meeting rail screw boss. The end caps were then secured to the jamb with three #6 x 5/8" flat head screws through the end caps into the jambs. The sill utilized a snap-in sill insert.

Sash Construction: The sashes were constructed of extruded vinyl with mitered and welded corners.

Screen Construction: The screen was constructed of roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible wrap around vinyl spline.

Hardware:

Description	Quantity	Location
Metal sweep locks	2	7" from active sash ends
Tilt latch	2	Active meeting rail ends
Tilt pins	2	Bottom rail
Constant force balance assembly	2	One per jamb

Drainage:

Description	Quantity	Location
1" long by 1/8" high weephole	2	3" from sill ends, on sill face
1/2" long 3/16" high weephole	2	2-1/2" from jambs in sill track
1/2" long by 3/16" high weephole	2	Bottom rail under glazing
1/2" long by 1/16" high weephole	4	Two 2-1/2" from bottom rail ends

Reinforcement: The fixed meeting rail utilized a roll-formed steel reinforcement (Drawing #RF-104). The active meeting rail and the bottom rail utilized a roll-formed steel reinforcement (Drawing #GVL-451).

Installation: The windows were installed into a #2 Spruce-Pine-Fir wood buck. The nail fin was back bedded in silicone and secured utilizing #6 x 1-5/8" drywall screws located 3" from corners and 8" on center. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-R25 48 x 78			
2.2.1.6.1	Operating Force	21 lbf	30 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.15 cfm/ft^2	0.30 cfm/ft^2 max.

***Note #1**: The tested specimen meets (or exceeds) the performance levels specified in ANSI/AAMA/NWWDA 101/I.S.2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547 (with and without screen) 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting rail) (Loads were held for 52 seconds)		
	15.0 psf (positive)	0.31"	See Note #2
	15.0 psf (negative)	0.32"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	22.5 psf (positive)	0.01"	0.17" max.
	22.5 psf (negative)	0.03"	0.17" max.

***Note #2**: The Uniform Load Deflection test is not a requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: H-R25 48 x 78 (Continued)			
2.2.1.6.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Meeting rail	0.13"/25%	0.50"/100%
	Bottom rail	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Right stile	0.06"/13%	0.50"/100%
	Left stile	0.06"/13%	0.50"/100%
2.1.7	Welded Corner Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547 (with and without screen) 6.0 psf	No leakage	No leakage

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
	Test Specimen #1: H-R25 48 x 78 (Continued)		
	Optional Performance: (Continued)		
4.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	25.0 psf (positive)	0.62"	See Note #2
	25.0 psf (negative)	0.49"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	37.5 psf (positive)	0.09"	0.17" max.
	37.5 psf (negative)	0.07"	0.17" max.
	Test Specimen #2: H-R35* 36 x 72		
	Optional Performance		
4.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	35.0 psf (positive)	0.19"	See Note #2
	40.0 psf (negative)	0.21"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	52.5 psf (positive)	0.03"	0.17" max.
	60.0 psf (negative)	0.05"	0.17" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #3: H-R25* 40 x 83 (Oriel)			
Optional Performance			
4.4.1	Uniform Load Deflection per ASTM E 330		
	(Deflections reported were taken on the meeting rail)		
	(Loads were held for 52 seconds)		
	25.0 psf (positive)	0.33"	See Note #2
	25.0 psf (negative)	0.22"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330		
	(Permanent sets reported were taken on the meeting rail)		
	(Loads were held for 10 seconds)		
	37.5 psf (positive)	0.02"	0.15" max.
	37.5 psf (negative)	0.02"	0.15" max.

Note: A lead check swab test was performed on all polymeric profiles. The test result was negative for the presence of lead (Pb).

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced, except in full, without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess vlm

Digitally Signed for: Mark A. Hess by Vicki L. McElwain

Mark A. Hess
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P.E.
Senior Project Engineer

MAH:vlm

Revision Log

Rev. #	Date	Page(s)	Revision(s)
0	03/29/05	N/A	Original report issue

Project Information for:	L219140		
Builder:	NORTON HOMES	Date:	12/19/2006
Lot:	N/A	Start Number:	1004
Subdivision:	269 SW BRIARBROOK PLACE	SEI Ref:	L219140
County or City:	COLUMBIA COUNTY		
Truss Page Count:	12		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORTON, JAMES H. RB 0031780
Address: 3367 S US HWY 441, SUITE 101
LAKE CITY, FL 32025 Designer: 126

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	T01	1219061004	12/19/2006				
2	T01G	1219061005	12/19/2006				
3	T02	1219061006	12/19/2006				
4	T02G	1219061007	12/19/2006				
5	T03	1219061008	12/19/2006				
6	T03G	1219061009	12/19/2006				
7	T04	1219061010	12/19/2006				
8	T04G	1219061011	12/19/2006				
9	T05	1219061012	12/19/2006				
10	T06	1219061013	12/19/2006				
11	T07	1219061014	12/19/2006				
12	T08	1219061015	12/19/2006				

DEC 19 2006

Dwg.#1219061003

Log On

DBPR Home | Online Services Home | Help | Site Map

4:14:09 PM 12/7/2006

Public Services

Search for a Licensee
Apply for a License
View Application Status
Apply to Retake Exam
Find Exam Information
File a Complaint
AB&T Delinquent Invoice & Activity List Search

User Services

Renew a License
Change License Status
Maintain Account
Change My Address
View Messages
Change My PIN
View Continuing Ed

Term Glossary
Online Help

Licensee Details

Licensee Information

Name:	**NORTON, JAMES H (Primary Name)**
	NORTON HOME IMPROVEMENT COMPANY INC (DBA Name)
Main Address:	**3367 S US HWY 441, SUITE 101**
	LAKE CITY Florida 32025
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**RT 28 BOX 388A**
	LAKE CITY FL 32025
County:	**COLUMBIA**

License Information

License Type:	**Registered Building Contractor**
Rank:	**Reg Building**
License Number:	**RB0031780**
Status:	**Current,Active**
Licensure Date:	**02/16/1978**
Expires:	**08/31/2007**

Special Qualifications	**Qualification Effective**
Bldg Code Core Course Credit	
Qualified Business License Required	**02/20/2004**

View Related License Information

View License Complaint

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

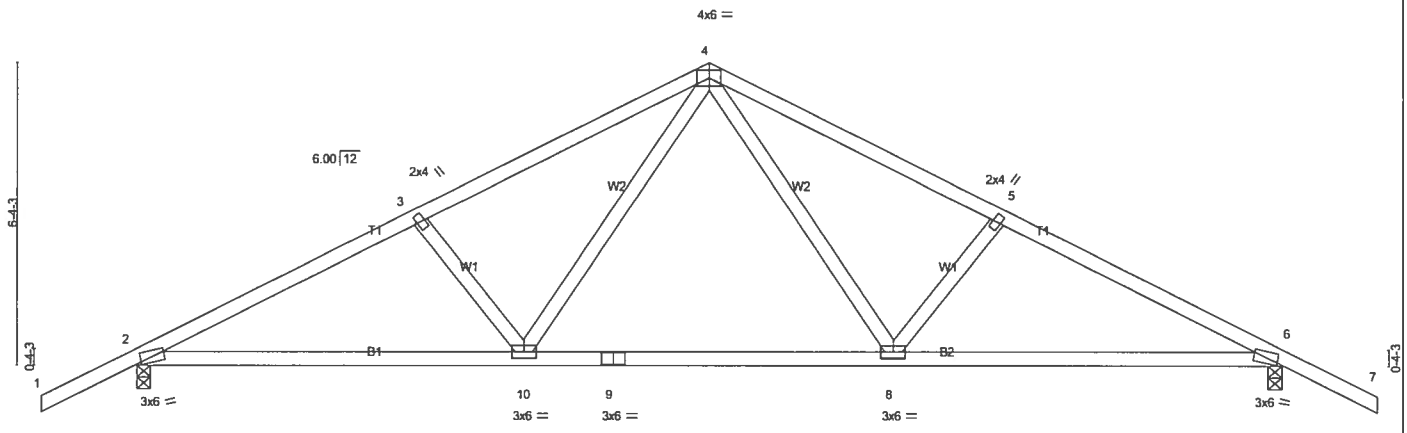
Dwg.#1219061004

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T01	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:48:56 2006 Page 1

-2-0-0	5-11-8	12-0-0	18-0-8	24-0-0	26-0-0
2-0-0	5-11-8	6-0-8	6-0-8	5-11-8	2-0-0

Scale = 1:46.3



8-1-9	15-10-7	24-0-0
8-1-9	7-8-15	8-1-9

Plate Offsets (X,Y): [2:0-1-5,0-0-7], [6:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.23	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	-0.21	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 114 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-7 oc bracing.

REACTIONS (lb/size) 2=1112/0-3-8, 6=1112/0-3-8
Max Horz 2=122(load case 5)
Max Uplift2=-746(load case 5), 6=-746(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1694/1372, 3-4=-1501/1361, 4-5=-1501/1361, 5-6=-1694/1372, 6-7=0/47
BOT CHORD 2-10=-1082/1449, 9-10=-634/974, 8-9=-634/974, 6-8=-1082/1449
WEBS 3-10=-305/265, 4-10=-615/579, 4-8=-615/579, 5-8=-305/265

JOINT STRESS INDEX
2 = 0.89, 3 = 0.34, 4 = 0.56, 5 = 0.34, 6 = 0.89, 8 = 0.44, 9 = 0.33 and 10 = 0.44

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 746 lb uplift at joint 2 and 746 lb uplift at joint 6.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061005

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T01G	GABLE	1	1	Job Reference (optional)

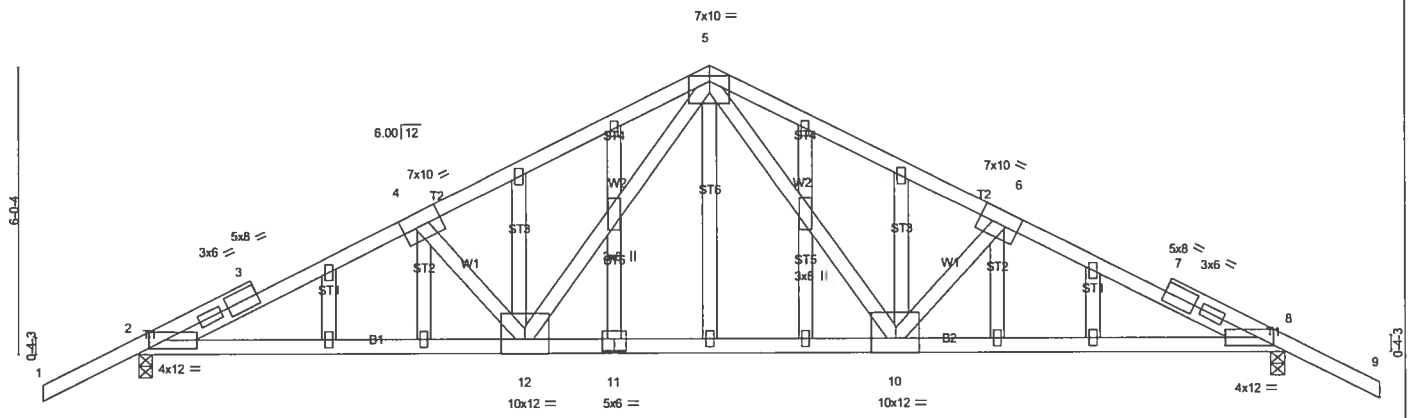
Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:48:58 2006 Page 1

-2-0-0 | 5-11-8 | 12-0-0 | 18-0-8 | 24-0-0 | 26-0-0

2-0-0 | 5-11-8 | 6-0-8 | 6-0-8 | 5-11-8 | 2-0-0

Scale = 1:46.3

WndRoofZone~2



8-1-9 | 15-10-7 | 24-0-0

8-1-9 | 7-8-15 | 8-1-9

Plate Offsets (X,Y): [2:0-3-12,0-2-0], [8:0-3-12,0-2-0], [11:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.38	8-10	>755	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.34	8-10	>828	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 160 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-7-6 oc bracing.

REACTIONS (lb/size) 2=1952/0-3-8, 8=1952/0-3-8
Max Horz 2=-118(load case 6)
Max Uplift2=-1315(load case 5), 8=-1315(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-39/100, 2-3=-3225/3023, 3-4=-3125/2987, 4-5=-2771/2746, 5-6=-2771/2746, 6-7=-3125/2987, 7-8=-3225/3023, 8-9=-39/100
BOT CHORD 2-12=-2575/2828, 11-12=-1485/1773, 10-11=-1485/1773, 8-10=-2575/2828
WEBS 4-12=-764/716, 5-12=-1109/1004, 5-10=-1109/1004, 6-10=-764/716

JOINT STRESS INDEX
2 = 0.83, 3 = 0.00, 3 = 0.73, 3 = 0.79, 4 = 0.40, 5 = 0.83, 6 = 0.40, 7 = 0.00, 7 = 0.79, 7 = 0.73, 8 = 0.83, 10 = 0.26, 11 = 0.43, 12 = 0.26, 13 = 0.34, 14 = 0.60, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.60, 23 = 0.34, 24 = 0.34, 25 = 0.34 and 26 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1315 lb uplift at joint 2 and 1315 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061006

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T02	COMMON	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:48:59 2006 Page 1

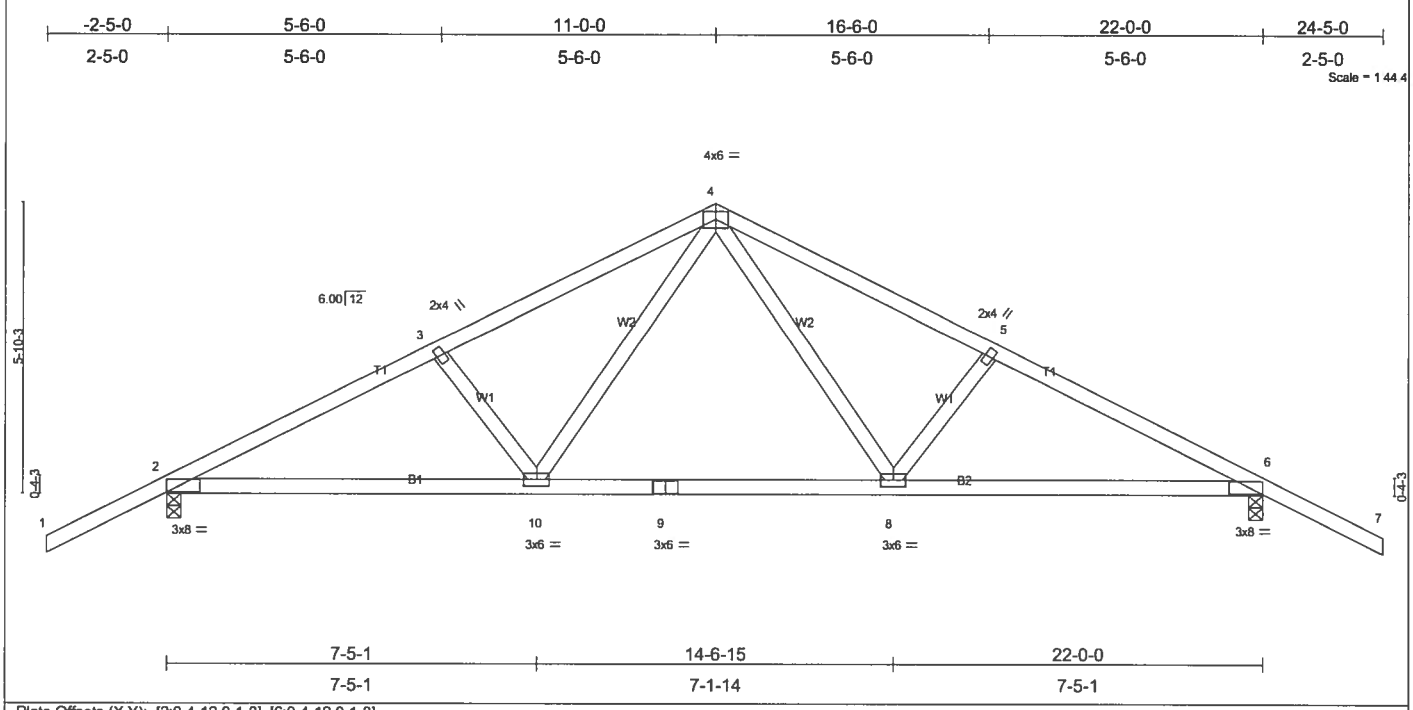


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.24	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.38	8-10	>684	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 106 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-8-13 oc bracing.

REACTIONS (lb/size) 2=1229/0-3-8, 6=1229/0-3-8
Max Horz 2=-123(load case 6)
Max Uplift2=-515(load case 5), 6=-515(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/57, 2-3=-1937/604, 3-4=-1770/591, 4-5=-1770/591, 5-6=-1937/604, 6-7=0/57
BOT CHORD 2-10=-502/1658, 9-10=-256/1137, 8-9=-256/1137, 6-8=-421/1658
WEBS 3-10=-243/210, 4-10=-224/733, 4-8=-224/733, 5-8=-243/210

JOINT STRESS INDEX
2 = 0.74, 3 = 0.34, 4 = 0.59, 5 = 0.34, 6 = 0.74, 8 = 0.55, 9 = 0.84 and 10 = 0.55

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 515 lb uplift at joint 2 and 515 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061007

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T02G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:01 2006 Page 1

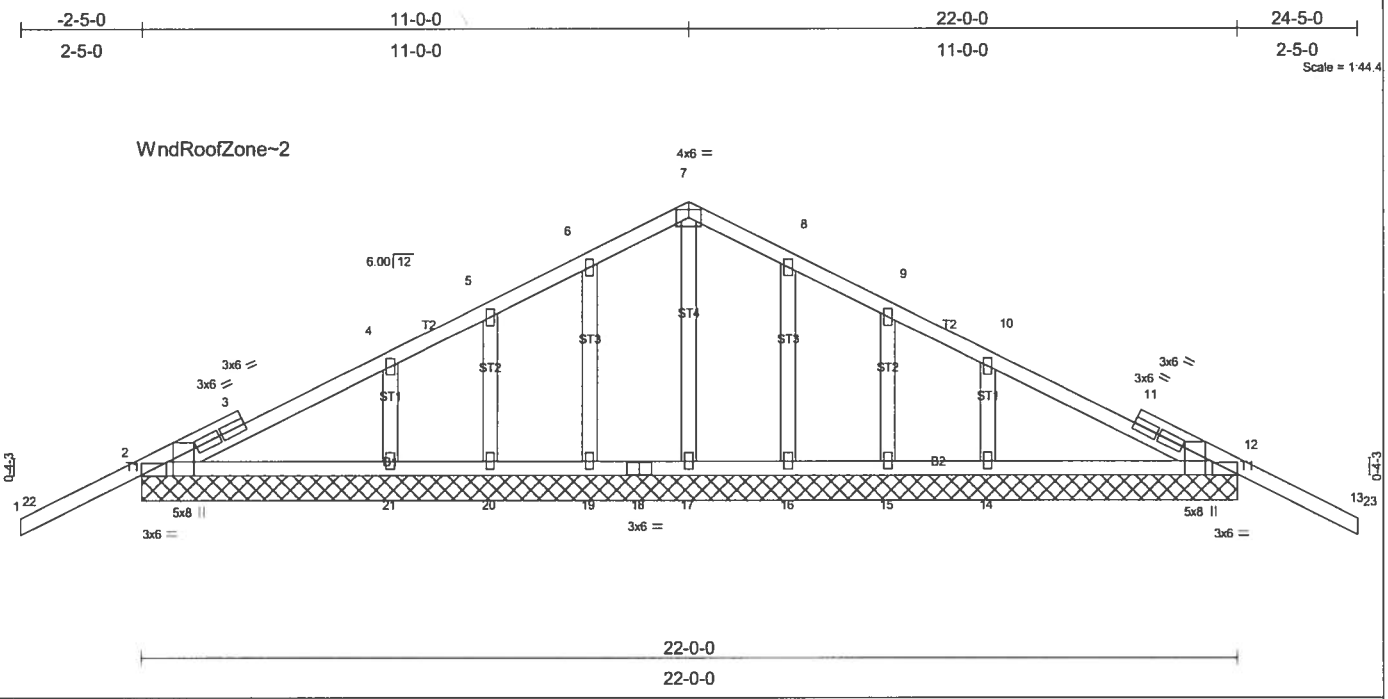


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-8,0-1-8], [11:0-2-8,0-1-8], [12:0-3-8,Edge], [12:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.04	13	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.06	13	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 116 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=568/22-0-0, 12=568/22-0-0, 17=383/22-0-0, 19=330/22-0-0, 20=147/22-0-0, 21=598/22-0-0, 16=330/22-0-0, 15=147/22-0-0, 14=598/22-0-0
Max Horz 2=-119(load case 6)
Max Uplift2=-313(load case 5), 12=-331(load case 6), 17=-25(load case 5), 19=-136(load case 5), 20=-106(load case 5), 21=-205(load case 5), 16=-135(load case 6), 15=-104(load case 6), 14=-210(load case 6)
Max Grav 2=574(load case 9), 12=574(load case 10), 17=383(load case 1), 19=333(load case 9), 20=147(load case 1), 21=600(load case 9), 16=333(load case 10), 15=147(load case 1), 14=600(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-22=0/10, 2-22=-11/109, 2-3=-93/103, 3-4=-112/249, 4-5=-21/151, 5-6=0/194, 6-7=0/186, 7-8=0/186, 8-9=0/194, 9-10=0/151, 10-11=-64/249, 11-12=-45/103, 12-23=-11/109, 13-23=0/10
BOT CHORD 2-21=-120/181, 20-21=-120/181, 19-20=-120/181, 18-19=-120/181, 17-18=-120/181, 16-17=-120/181, 15-16=-120/181, 14-15=-120/181, 12-14=-120/181
WEBS 7-17=-329/37, 6-19=-260/150, 5-20=-133/115, 4-21=-449/232, 8-16=-260/149, 9-15=-133/115, 10-14=-449/236

JOINT STRESS INDEX
2 = 0.79, 2 = 0.18, 3 = 0.00, 3 = 0.46, 3 = 0.47, 4 = 0.34, 5 = 0.34, 6 = 0.34, 7 = 0.25, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.00, 11 = 0.47, 11 = 0.46, 12 = 0.79, 12 = 0.18, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.15, 19 = 0.34, 20 = 0.34 and 21 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 313 lb uplift at joint 2, 331 lb uplift at joint 12, 25 lb uplift at joint 17, 136 lb uplift at joint 19, 106 lb uplift at joint 20, 205 lb uplift at joint 21, 135 lb uplift at joint 16, 104 lb uplift at joint 15 and 210 lb uplift at joint 14.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-22=-54, 7-22=-114(F=-60), 7-23=-114(F=-60), 13-23=-54, 2-12=-30

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

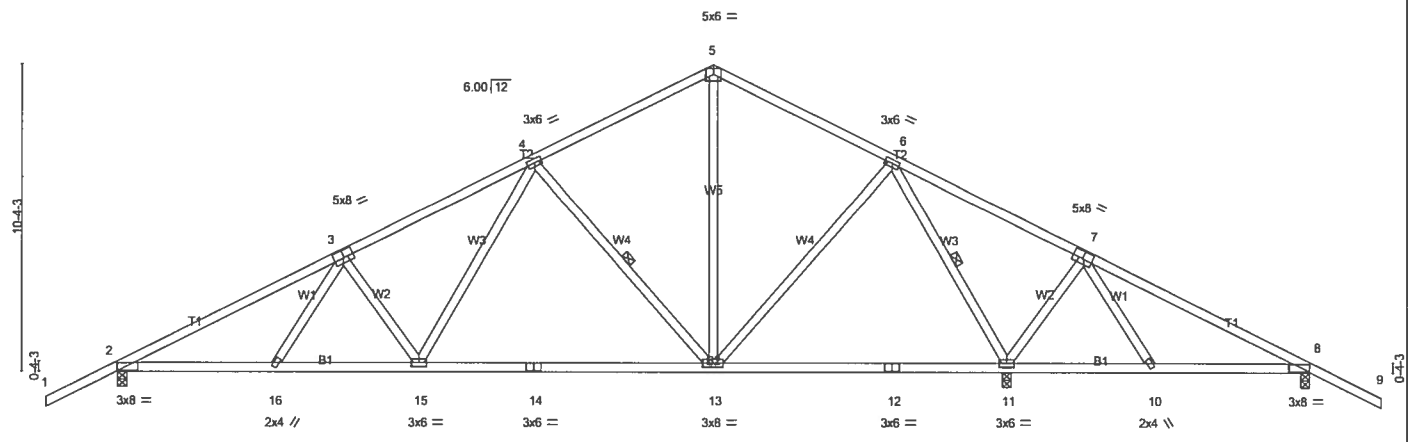
Dwg.#1219061008

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T03	COMMON	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:03 2006 Page 1

-2-5-0	7-7-0	14-0-2	20-0-0	25-11-14	32-5-0	40-0-0	42-5-0
2-5-0	7-7-0	6-5-2	5-11-14	5-11-14	6-5-2	7-7-0	2-5-0

Scale = 1:74.1



5-4-2	10-1-12	20-0-0	29-10-4	34-7-14	40-0-0
5-4-2	4-9-10	9-10-4	9-10-4	4-9-10	5-4-2

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-4-0,0-3-0], [7:0-4-0,0-3-0], [8:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.22	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.37	13-15	>970	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 224 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-13, 6-11

REACTIONS (lb/size) 2=1287/0-3-8, 11=2062/0-3-8, 8=263/0-3-8
Max Horz 2=-186(load case 6)
Max Uplift2=-535(load case 5), 11=-740(load case 5), 8=-357(load case 6)
Max Grav 2=1287(load case 1), 11=2062(load case 1), 8=356(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/57, 2-3=-2020/586, 3-4=-1658/601, 4-5=-796/366, 5-6=-796/365, 6-7=-179/728, 7-8=-38/381, 8-9=0/57
BOT CHORD 2-16=-528/1716, 15-16=-544/1637, 14-15=-280/1092, 13-14=-280/1092, 12-13=0/209, 11-12=0/209, 10-11=-392/192, 8-10=-314/110
WEBS 3-16=0/154, 3-15=-409/286, 4-15=-219/660, 4-13=-698/409, 5-13=-124/335, 6-13=-161/677, 6-11=-1657/558, 7-11=-448/391, 7-10=-220/163

JOINT STRESS INDEX
2 = 0.78, 3 = 0.62, 4 = 0.56, 5 = 0.36, 6 = 0.56, 7 = 0.62, 8 = 0.78, 10 = 0.34, 11 = 0.67, 12 = 0.66, 13 = 0.68, 14 = 0.66, 15 = 0.67 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 535 lb uplift at joint 2, 740 lb uplift at joint 11 and 357 lb uplift at joint 8.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

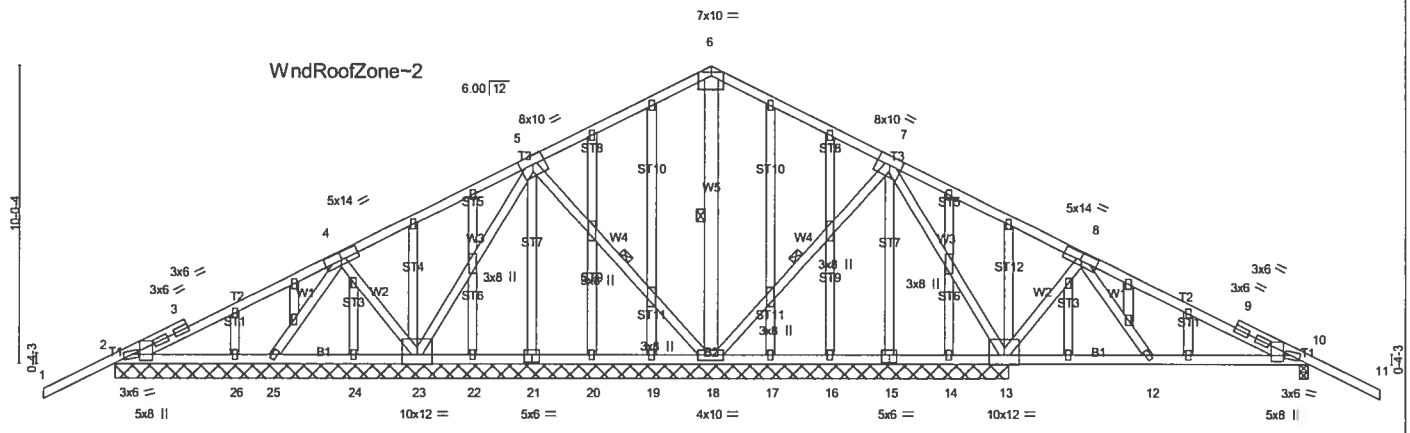
Dwg.#1219061009

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T03G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:05 2006 Page 1

-2-5-0	7-7-0	14-0-2	20-0-0	25-11-14	32-5-0	40-0-0	42-5-0
2-5-0	7-7-0	6-5-2	5-11-14	5-11-14	6-5-2	7-7-0	2-5-0

Scale = 1:74.1



5-4-2	10-1-12	20-0-0	29-10-4	34-7-14	40-0-0
5-4-2	4-9-10	9-10-4	9-10-4	4-9-10	5-4-2

Plate Offsets (X,Y): [2:0-3-13,0-0-3], [2:0-0-14,0-9-14], [3:0-2-9,0-1-8], [4:0-7-0,0-3-0], [8:0-7-0,0-3-0], [9:0-2-9,0-1-8], [10:0-3-13,0-0-3], [10:0-0-14,0-9-14], [15:0-3-0,0-3-0], [21:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	0.08	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.09	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 345 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 "Except"
W5 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 10-12.
WEBS 1 Row at midpt 5-18, 6-18, 7-18

REACTIONS (lb/size) 2=427/30-0-0, 25=711/30-0-0, 23=640/30-0-0, 18=1382/30-0-0, 13=1531/30-0-0, 10=787/0-3-8, 19=40/30-0-0, 20=105/30-0-0, 22=106/30-0-0, 24=96/30-0-0, 26=256/30-0-0, 17=38/30-0-0, 16=109/30-0-0, 14=80/30-0-0
Max Horz 2=182(load case 5)
Max Uplift2=-296(load case 5), 25=-413(load case 5), 23=-479(load case 5), 18=-793(load case 5), 13=-1043(load case 6), 10=-597(load case 6), 26=-85(load case 6)
Max Grav 2=428(load case 9), 25=727(load case 9), 23=670(load case 9), 18=1382(load case 1), 13=1556(load case 10), 10=789(load case 10), 19=40(load case 1), 20=105(load case 10), 22=106(load case 9), 24=96(load case 9), 26=258(load case 9), 17=38(load case 1), 16=109(load case 9), 14=80(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-38/120, 2-3=-434/523, 3-4=-516/840, 4-5=-158/412, 5-6=-65/423, 6-7=-47/423, 7-8=-389/793, 8-9=-97/128, 9-10=-259/174, 10-11=-38/121
BOT CHORD 2-26=-591/556, 25-26=-591/556, 24-25=-81/258, 23-24=-81/258, 22-23=-46/254, 21-22=-46/254, 20-21=-46/254, 19-20=-46/254, 18-19=-46/254, 17-18=-144/351, 16-17=-144/351, 15-16=-144/351, 14-15=-144/351, 13-14=-144/351, 12-13=-63/214, 10-12=-9/118
WEBS 4-25=-937/614, 4-23=-249/300, 5-23=-517/320, 5-18=-374/385, 6-18=-921/491, 7-18=-149/206, 7-13=-889/624, 8-13=-861/760, 8-12=-263/259

JOINT STRESS INDEX
2 = 0.90, 2 = 0.64, 3 = 0.00, 3 = 0.67, 3 = 0.67, 4 = 0.95, 5 = 0.49, 6 = 0.75, 7 = 0.49, 8 = 0.95, 9 = 0.00, 9 = 0.67, 9 = 0.67, 10 = 0.90, 10 = 0.64, 12 = 0.47, 13 = 0.25, 14 = 0.34, 15 = 0.20, 16 = 0.34, 17 = 0.34, 18 = 0.42, 19 = 0.34, 20 = 0.34, 21 = 0.20, 22 = 0.34, 23 = 0.25, 24 = 0.34, 25 = 0.47, 26 = 0.34, 27 = 0.46, 28 = 0.34, 29 = 0.46, 30 = 0.34, 31 = 0.78, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.46, 40 = 0.34, 41 = 0.46, 42 = 0.34, 43 = 0.78, 44 = 0.34, 45 = 0.34, 46 = 0.34, 47 = 0.34, 48 = 0.34, 49 = 0.34 and 50 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 413 lb uplift at joint 25, 479 lb uplift at joint 23, 793 lb uplift at joint 18, 1043 lb uplift at joint 13, 597 lb uplift at joint 10 and 85 lb uplift at joint 26.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-30

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061010

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T04	COMMON	7	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:07 2006 Page 1

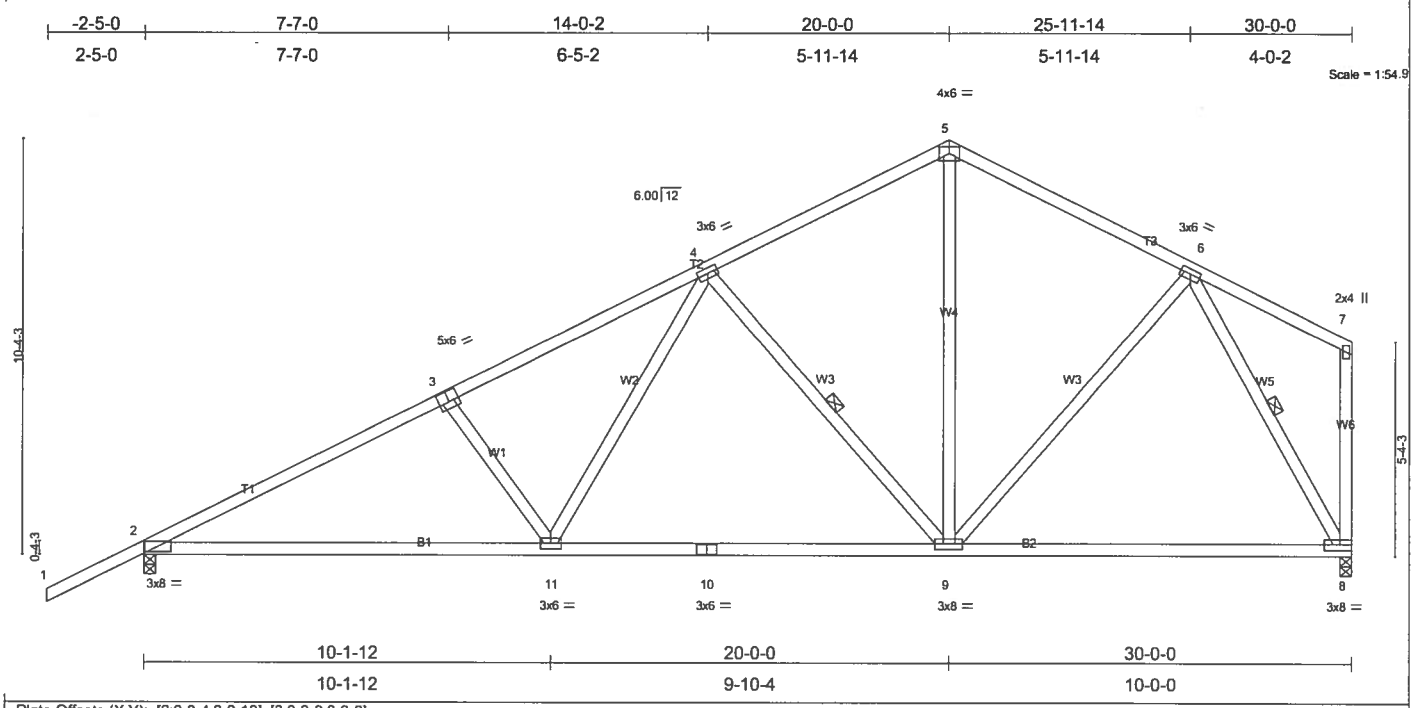


Plate Offsets (X,Y): [2:0-8-4,0-0-10], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.27	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.46	2-11	>776	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-3 oc bracing.
WEBS 1 Row at midpt 4-9, 6-8

REACTIONS (lb/size) 2=1392/0-3-8, 8=1242/0-3-8
Max Horz 2=380(load case 5)
Max Uplift2=-562(load case 5), 8=-386(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/57, 2-3=-2143/665, 3-4=-1915/651, 4-5=-1026/406, 5-6=-1024/423, 6-7=-96/39, 7-8=-119/63
BOT CHORD 2-11=-792/1834, 10-11=-524/1305, 9-10=-524/1305, 8-9=-199/587
WEBS 3-11=-350/297, 4-11=-208/690, 4-9=-708/405, 5-9=-173/515, 6-9=-98/444, 6-8=-1121/421

JOINT STRESS INDEX
2 = 0.77, 3 = 0.61, 4 = 0.47, 5 = 0.52, 6 = 0.42, 7 = 0.76, 8 = 0.54, 9 = 0.57, 10 = 0.61 and 11 = 0.56

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 562 lb uplift at joint 2 and 386 lb uplift at joint 8.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061011

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T04G	GABLE	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:09 2006 Page 1

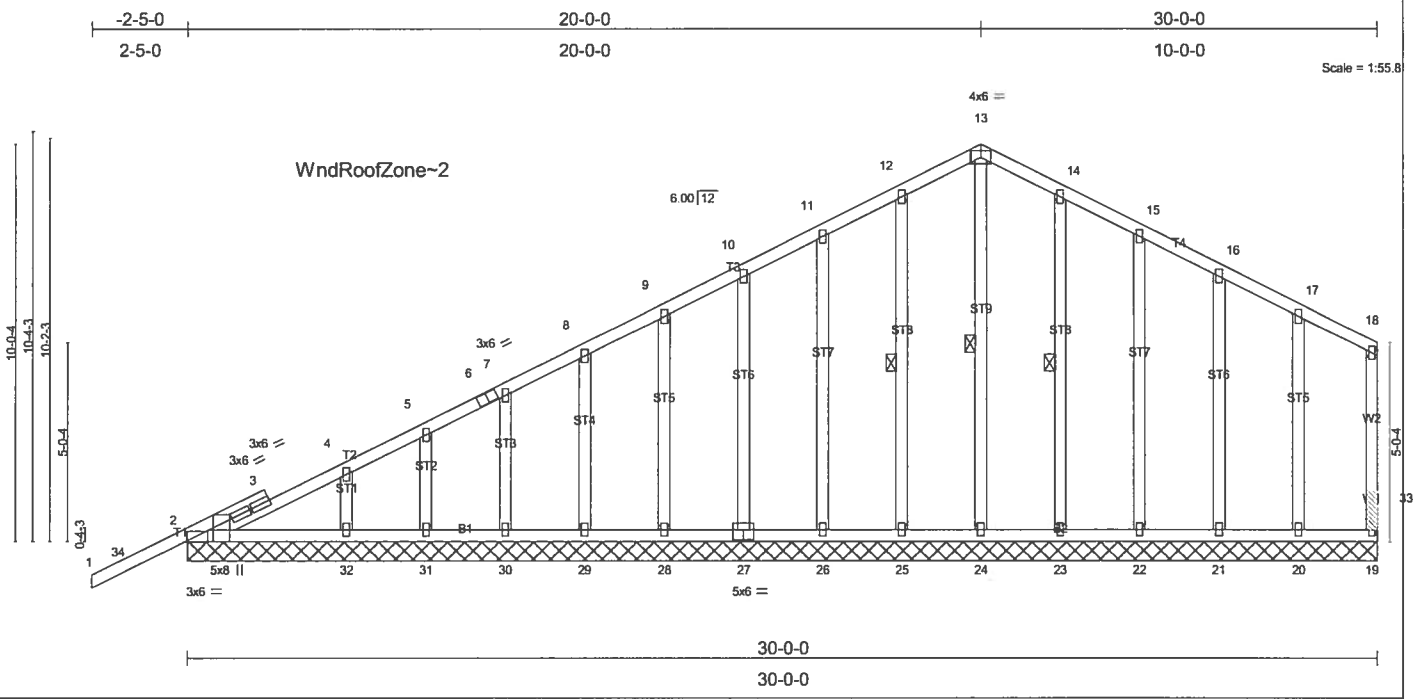


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [3:0-2-8,0-1-8], [27:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.02	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.05	1	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.00	19	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 224 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 13-24, 12-25, 14-23

REACTIONS (lb/size) 2=581/30-0-0, 19=121/30-0-0, 24=283/30-0-0, 25=289/30-0-0, 26=288/30-0-0, 27=288/30-0-0, 28=289/30-0-0, 29=285/30-0-0, 30=301/30-0-0, 31=236/30-0-0, 32=434/30-0-0, 23=289/30-0-0, 22=288/30-0-0, 21=288/30-0-0, 20=292/30-0-0
Max Horz 2=365(load case 5)
Max Uplift2=-246(load case 5), 19=-48(load case 6), 25=-122(load case 5), 26=-136(load case 5), 27=-131(load case 5), 28=-132(load case 5), 29=-133(load case 5), 30=-128(load case 5), 31=-147(load case 5), 32=-124(load case 6), 23=-117(load case 6), 22=-138(load case 6), 21=-130(load case 6), 20=-141(load case 6)
Max Grav 2=581(load case 1), 19=121(load case 1), 24=283(load case 1), 25=292(load case 9), 26=288(load case 1), 27=288(load case 9), 28=289(load case 9), 29=285(load case 9), 30=301(load case 1), 31=236(load case 9), 32=434(load case 1), 23=292(load case 10), 22=288(load case 1), 21=288(load case 1), 20=292(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-34=0/10, 2-34=-14/109, 2-3=-250/27, 3-4=-265/82, 4-5=-216/39, 5-6=-151/56, 6-7=-163/55, 7-8=-111/71, 8-9=-66/99, 9-10=-56/135, 10-11=-56/170, 11-12=-56/209, 12-13=-56/255, 13-14=-57/255, 14-15=-56/207, 15-16=-56/147, 16-17=-56/91, 17-18=-46/49, 19-33=-94/64, 18-33=-94/64
BOT CHORD 2-32=-2/4, 31-32=-2/4, 30-31=-2/4, 29-30=-2/4, 28-29=-2/4, 27-28=-2/4, 26-27=-2/4, 25-26=-2/4, 24-25=-2/4, 23-24=-2/4, 22-23=-2/4, 21-22=-2/4, 20-21=-2/4, 19-20=-2/4
WEBS 13-24=-223/0, 12-25=-232/144, 11-26=-228/169, 10-27=-228/162, 9-28=-228/163, 8-29=-226/163, 7-30=-236/162, 5-31=-193/167, 4-32=-333/156, 14-23=-232/144, 15-22=-228/169, 16-21=-228/161, 17-20=-233/170

JOINT STRESS INDEX
2 = 0.80, 2 = 0.18, 3 = 0.00, 3 = 0.42, 3 = 0.42, 4 = 0.34, 5 = 0.34, 6 = 0.15, 7 = 0.34, 8 = 0.34, 9 = 0.34, 10 = 0.34, 11 = 0.34, 12 = 0.34, 13 = 0.25, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 19 = 0.00, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.20, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.00 and 33 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 2, 48 lb uplift at joint 19, 122 lb uplift at joint 25, 136 lb uplift at joint 26, 131 lb uplift at joint 27, 132 lb uplift at joint 28, 133 lb uplift at joint 29, 128 lb uplift at joint 30, 147 lb uplift at joint 31, 124 lb uplift at joint 32, 117 lb uplift at joint 23, 138 lb uplift at joint 22, 130 lb uplift at joint 21 and 141 lb uplift at joint 20.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-34=-54, 13-34=-114(F=-60), 13-18=-114(F=-60), 2-19=-30

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

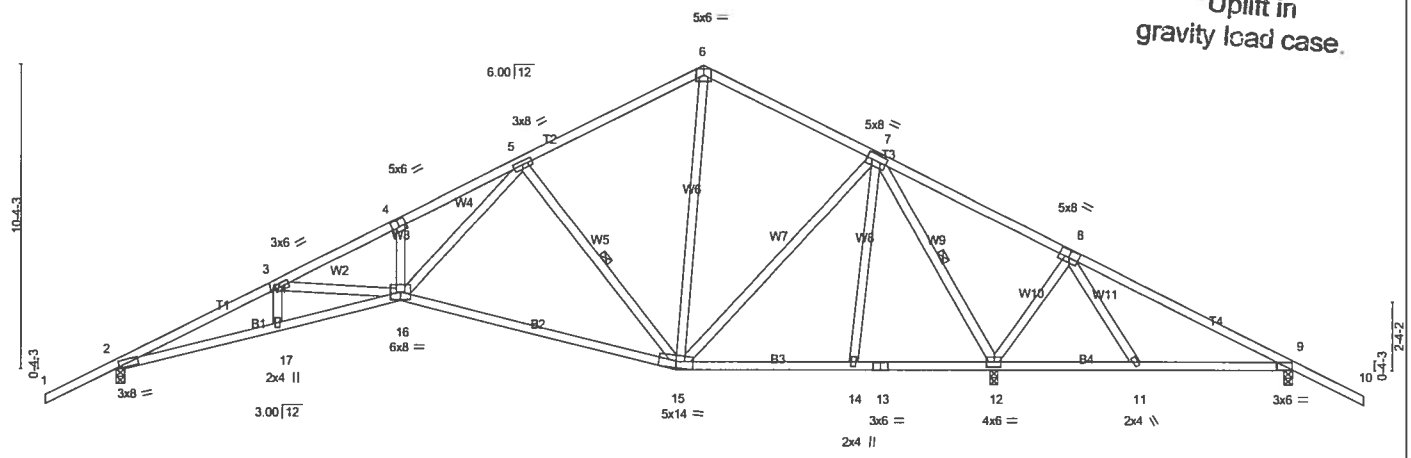
Dwg.#1219061012

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:11 2006 Page 1

-2-5-0	5-5-11	9-8-2	13-10-3	20-0-0	25-10-4	32-5-0	40-0-0	42-5-0
2-5-0	5-5-11	4-2-7	4-2-1	6-1-13	5-10-4	6-6-12	7-7-0	2-5-0

Scale = 1:75.1



5-5-11	9-8-2	19-0-12	25-1-0	29-10-4	34-7-14	40-0-0
5-5-11	4-2-7	9-4-10	6-0-4	4-9-4	4-9-10	5-4-2

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.36	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.59	15-16	>603	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.20	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 231 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-10 oc bracing.
WEBS 1 Row at midpt 5-15, 7-12

REACTIONS (lb/size) 2=1140/0-3-8, 12=2647/0-3-8, 9=-174/0-3-8
Max Horz 2=-186(load case 6)
Max Uplift2=-487(load case 5), 12=-930(load case 5), 9=-421(load case 9)
Max Grav 2=1140(load case 1), 12=2647(load case 1), 9=100(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-3116/965, 3-4=-2594/798, 4-5=-2572/886, 5-6=-536/343, 6-7=-525/317, 7-8=-482/1707, 8-9=-349/1316, 9-10=0/57
BOT CHORD 2-17=-910/2786, 16-17=-913/2802, 15-16=-248/1062, 14-15=-352/268, 13-14=-366/264, 12-13=-366/264, 11-12=-1196/463, 9-11=-1095/386
WEBS 3-17=0/91, 3-16=-434/243, 4-16=-189/177, 5-16=-595/1888, 5-15=-975/473, 6-15=-99/106, 7-15=-309/989, 7-14=0/128, 7-12=-2395/748, 8-12=-487/390, 8-11=-206/205

JOINT STRESS INDEX
2 = 0.74, 3 = 0.41, 4 = 0.31, 5 = 0.98, 6 = 0.38, 7 = 0.71, 8 = 0.81, 9 = 0.59, 11 = 0.34, 12 = 0.66, 13 = 0.15, 14 = 0.34, 15 = 0.45, 16 = 0.83 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 487 lb uplift at joint 2, 930 lb uplift at joint 12 and 421 lb uplift at joint 9.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

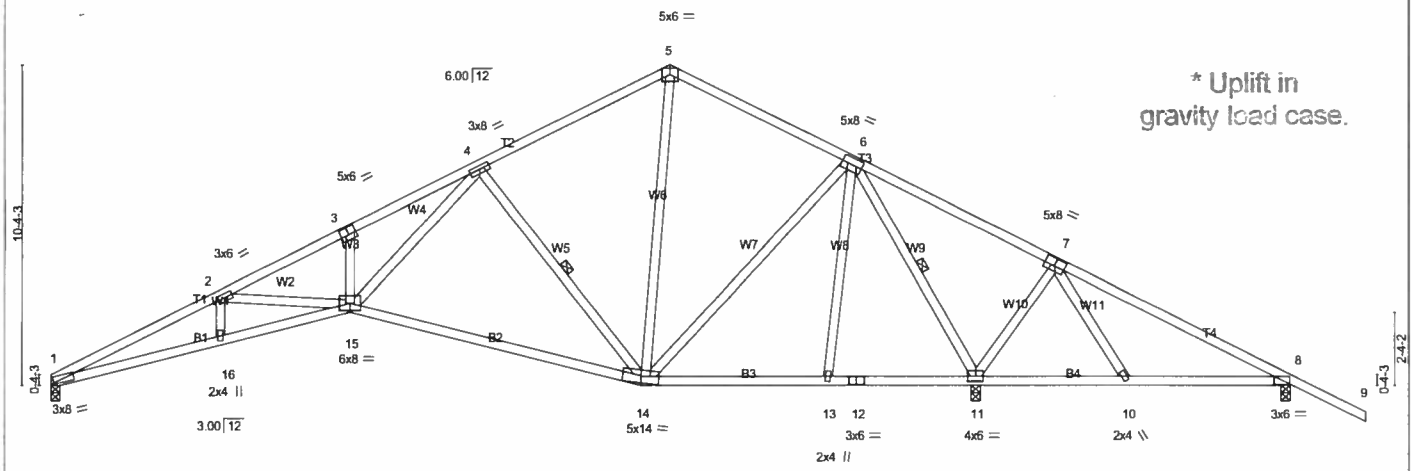
Dwg.#1219061013

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T06	SPECIAL	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:13 2006 Page 1

5-5-11	9-8-2	13-10-3	20-0-0	25-10-4	32-5-0	40-0-0	42-5-0
5-5-11	4-2-7	4-2-1	6-1-13	5-10-4	6-6-12	7-7-0	2-5-0

Scale = 1:71.3



5-5-11	9-8-2	19-0-12	25-1-0	29-10-4	34-7-14	40-0-0
5-5-11	4-2-7	9-4-10	6-0-4	4-9-4	4-9-10	5-4-2

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.36	14-15	>979	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.60	14-15	>592	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(TL)	0.21	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 227 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-11 oc bracing.
WEBS 1 Row at midpt 4-14, 6-11

REACTIONS (lb/size) 1=989/0-3-8, 11=2676/0-3-8, 8=-191/0-3-8
Max Horz 1=-215(load case 6)
Max Uplift1=-328(load case 5), 11=-967(load case 5), 8=-438(load case 9)
Max Grav 1=989(load case 1), 11=2676(load case 1), 8=95(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3216/1110, 2-3=-2621/833, 3-4=-2596/916, 4-5=-531/341, 5-6=-520/315, 6-7=-529/1744, 7-8=-396/1353, 8-9=0/57
BOT CHORD 1-16=-1059/2903, 15-16=-1054/2913, 14-15=-251/1065, 13-14=-369/290, 12-13=-384/286, 11-12=-384/286, 10-11=-1228/505, 8-10=-1128/428
WEBS 2-16=0/104, 2-15=-521/354, 3-15=-180/167, 4-15=-631/1917, 4-14=-987/489, 5-14=-98/102, 6-14=-334/1008, 6-13=0/128, 6-11=-2427/789, 7-11=-488/390, 7-10=-206/206

JOINT STRESS INDEX
1 = 0.74, 2 = 0.41, 3 = 0.31, 4 = 1.00, 5 = 0.38, 6 = 0.72, 7 = 0.82, 8 = 0.59, 10 = 0.34, 11 = 0.67, 12 = 0.15, 13 = 0.34, 14 = 0.46, 15 = 0.83 and 16 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 1, 967 lb uplift at joint 11 and 438 lb uplift at joint 8.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061014

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:14 2006 Page 1

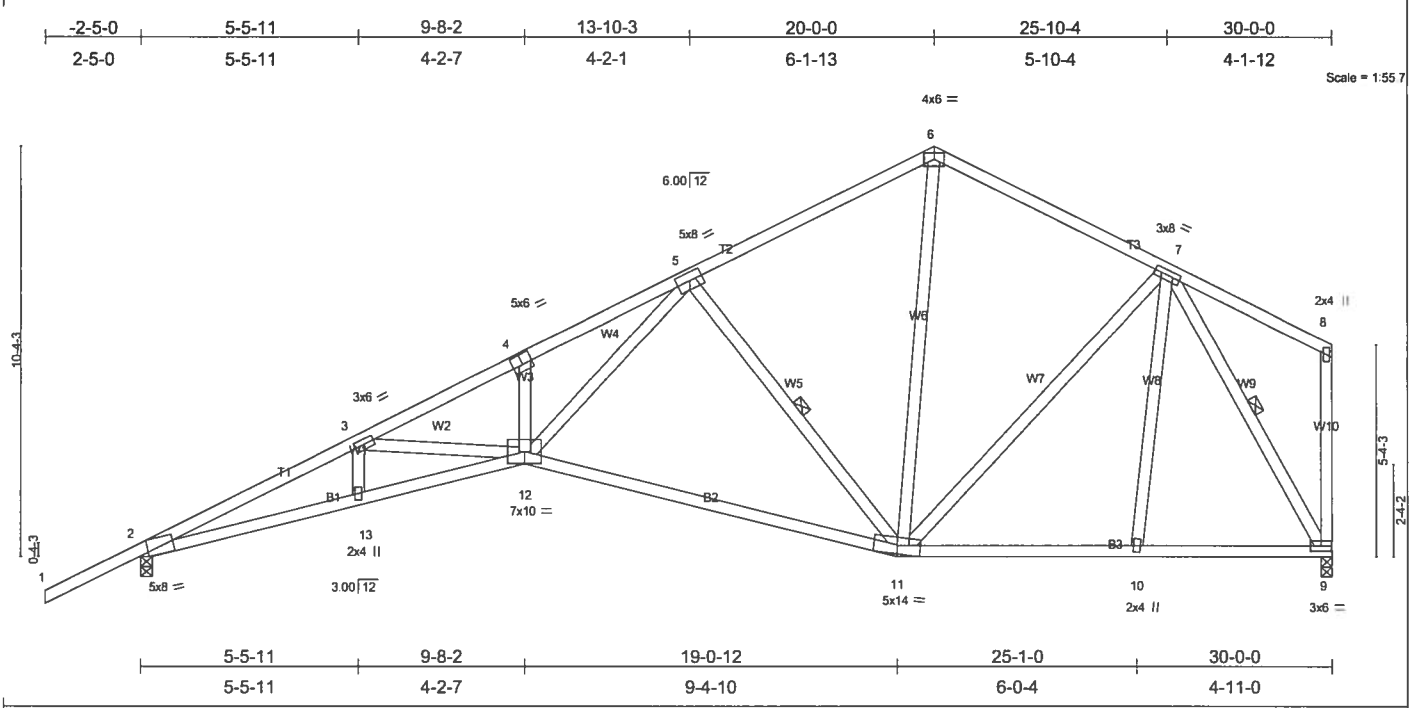


Plate Offsets (X,Y): [2:0-2-6,Edge], [4:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.43	11-12	>826	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.71	11-12	>501	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(TL)	0.29	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-0-3 oc bracing.
WEBS 1 Row at midpt 5-11, 7-9

REACTIONS (lb/size) 2=1392/0-3-8, 9=1242/0-3-8
Max Horz 2=380(load case 5)
Max Uplift2=-561(load case 5), 9=-387(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/56, 2-3=-4116/1447, 3-4=-3650/1310, 4-5=-3629/1398, 5-6=-1094/435, 6-7=-1031/437, 7-8=-62/53, 8-9=-101/75
BOT CHORD 2-13=-1547/3696, 12-13=-1552/3714, 11-12=-679/1691, 10-11=-192/632, 9-10=-196/617
WEBS 3-13=0/87, 3-12=-377/209, 4-12=-190/179, 5-12=-944/2389, 5-11=-1157/601, 6-11=-183/547, 7-11=-120/366, 7-10=0/137, 7-9=-1223/392

JOINT STRESS INDEX
2 = 0.80, 3 = 0.41, 4 = 0.44, 5 = 0.69, 6 = 0.56, 7 = 0.70, 8 = 0.46, 9 = 0.50, 10 = 0.34, 11 = 0.39, 12 = 0.70 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 561 lb uplift at joint 2 and 387 lb uplift at joint 9.

LOAD CASE(S) Standard

DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#1219061015

Job	Truss	Truss Type	Qty	Ply	NORTON - TROIANO RES.
L219140	T08	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Nov 30 09:49:16 2006 Page 1

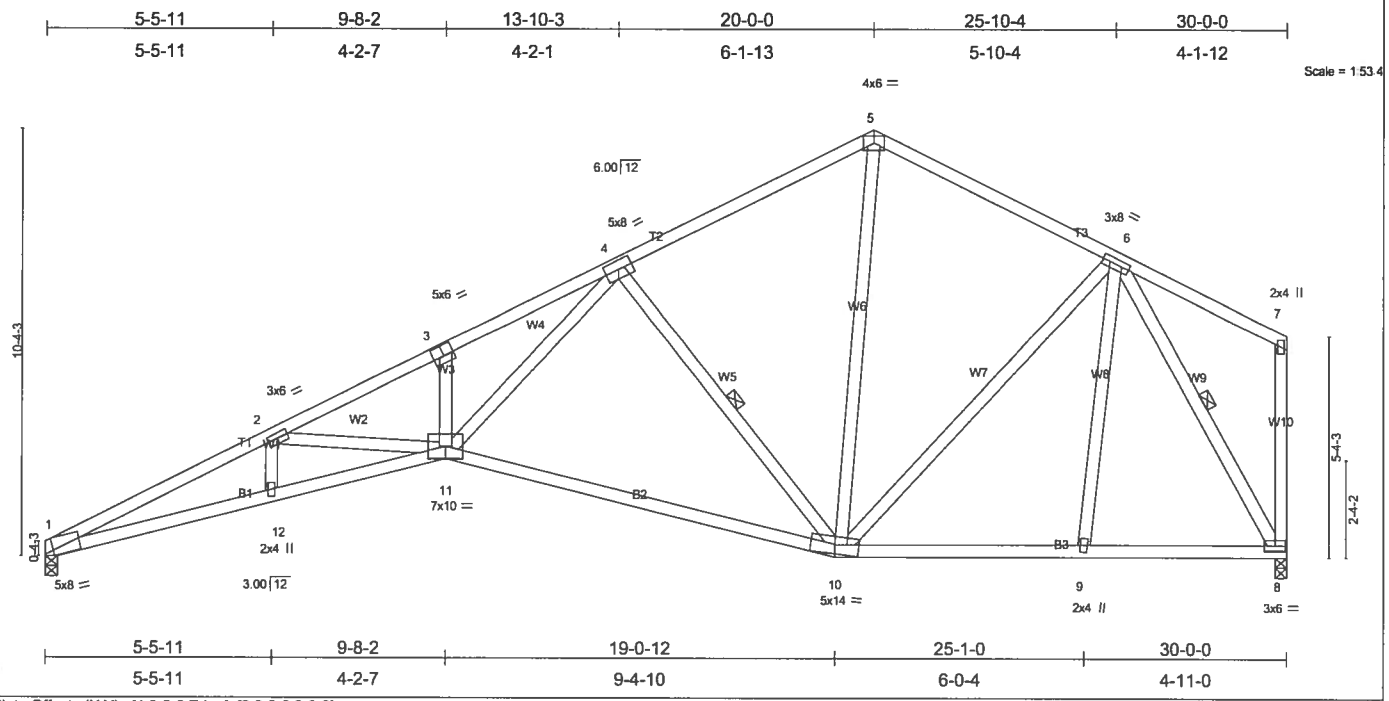


Plate Offsets (X,Y): [1:0-2-6,Edge], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.44	10-11	>808	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.73	10-11	>491	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.30	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 186 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-10, 6-8

REACTIONS (lb/size) 1=1248/0-3-8, 8=1248/0-3-8
Max Horz 1=306(load case 5)
Max Uplift1=-410(load case 5), 8=-394(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4239/1621, 2-3=-3702/1375, 3-4=-3666/1459, 4-5=-1102/445, 5-6=-1038/446, 6-7=-62/53, 7-8=-101/75
BOT CHORD 1-12=-1723/3834, 11-12=-1719/3845, 10-11=-701/1708, 9-10=-196/635, 8-9=-200/620
WEBS 2-12=0/97, 2-11=-463/319, 3-11=-182/168, 4-11=-995/2428, 4-10=-1174/622, 5-10=-192/554, 6-10=-125/371, 6-9=0/136, 6-8=-1229/401

JOINT STRESS INDEX
1 = 0.81, 2 = 0.41, 3 = 0.44, 4 = 0.70, 5 = 0.56, 6 = 0.70, 7 = 0.46, 8 = 0.50, 9 = 0.34, 10 = 0.40, 11 = 0.73 and 12 = 0.34

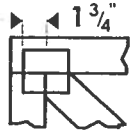
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 410 lb uplift at joint 1 and 394 lb uplift at joint 8.

LOAD CASE(S) Standard

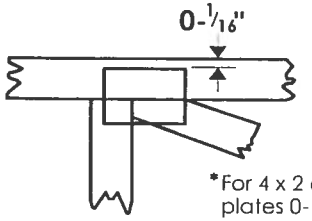
DECEMBER 19, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

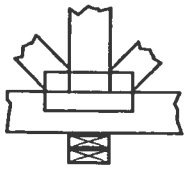
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

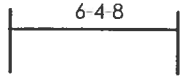


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

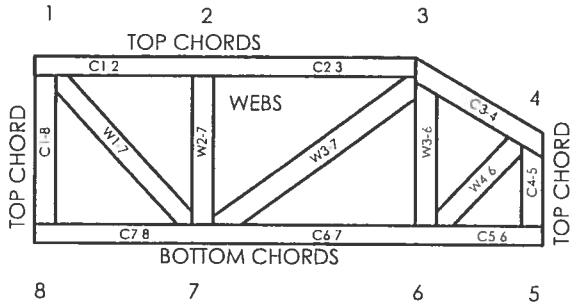
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

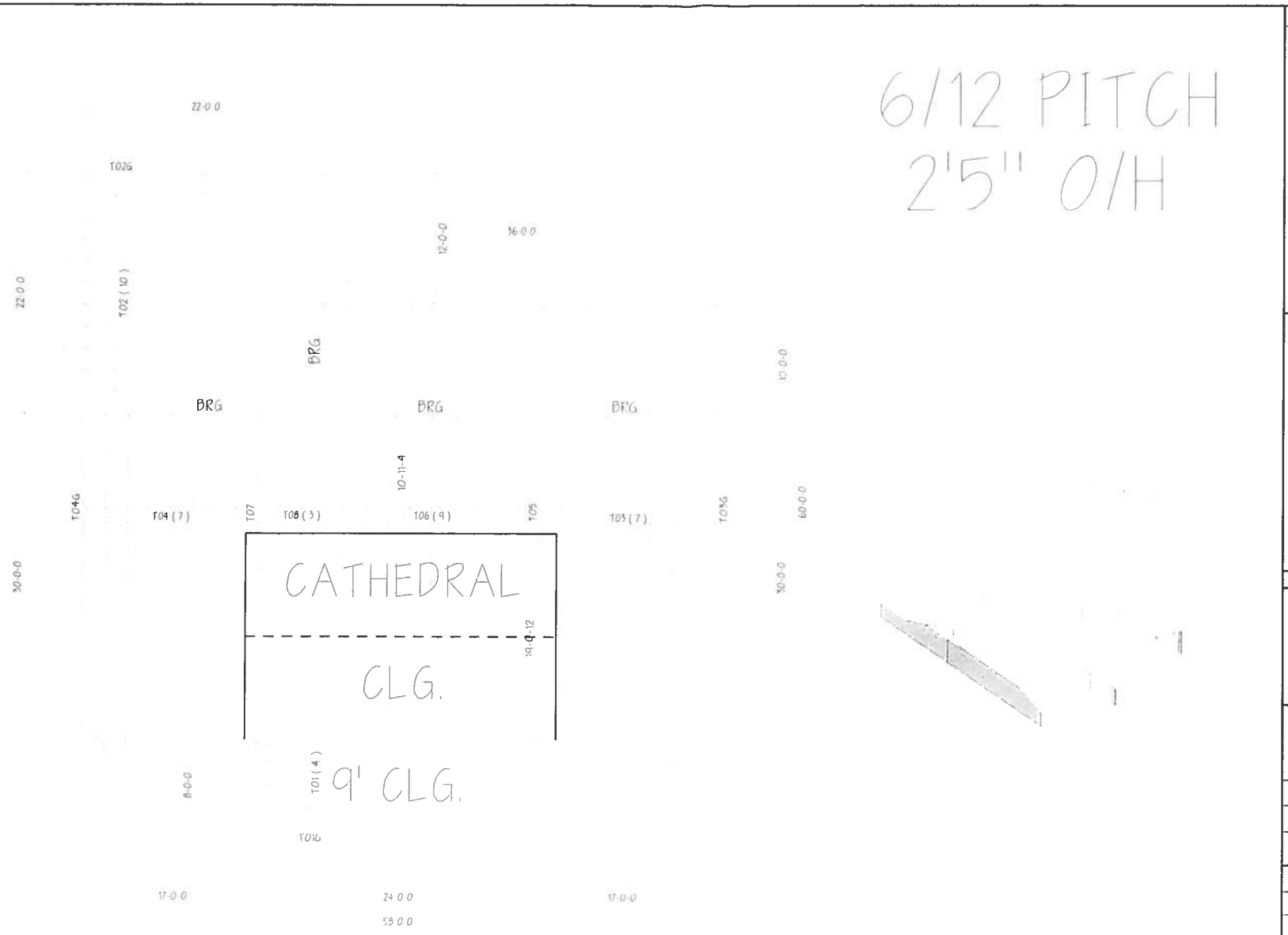
TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.



BEARING HEIGHT SCHEDULE

NOTES

SHOP DRAWING APPROVAL

Builders FirstSource
Bunnell
Jacksonville
Lake City
Sanford

NORTON BLDG.

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Therma-Tru Corporation
108 Mutzfeld Road
Butler, IN 46721

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Landmark" 6'8" W/E Outswing Opaque Insulated Steel Door – Impact Resistant

APPROVAL DOCUMENT: Drawing No. **S-2189**, dated 11/27/01, with revision 1 dated 01/30/02, titled "Landmark Woodedge Opaque Single 6'8 Outswing Door in Wood Frame", sheets 1 through 5, prepared by R. W. Building Consultants, Inc., bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact Resistant

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P.E.**

THERMA-TRU®

"LANDMARK SERIES"

6'8 OUTSWING INSULATED WOOD EDGE STEEL DOOR WITH WOOD FRAMES

GENERAL NOTES

1. THIS PRODUCT IS DESIGNED TO COMPLY WITH THE FLORIDA BUILDING CODE.
2. WOOD BUCKS BY OTHERS, MUST BE ANCHORED PROPERLY TO TRANSFER LOADS TO THE STRUCTURE.
3. PRODUCT ANCHORS SHALL BE AS LISTED AND SPACED AS SHOWN ON DETAILS. ANCHOR EMBEDMENT TO BASE MATERIAL SHALL BE BEYOND WALL DRESSING OR STUCCO.
4. DESIGNED PRESSURE RATING SEE TABLE PAGE 1.
5. THIS PRODUCT MEETS THE WATER REQUIREMENTS FOR "HIGH VELOCITY HURRICANE ZONES".

RESIDENTIAL INSULATED STEEL DOOR
(Common to all frame conditions)

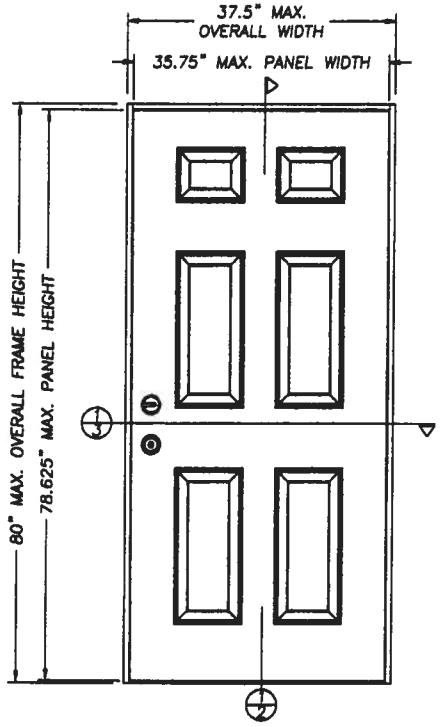
Door Construction:

Door Face Sheets: 25 GA. (0.019") minimum thickness, Galvanized steel A-525 commercial quality – AKDQ per ASTM 620 with yield strength Fy(min.)=23,300 psi

Core design: Polyurethane foam core, with 1.9 lbs. density by BASF.

Door Panel Construction: Flush or embossed type. The vertical edges of the skin, rolled formed to provide a mechanical interlock with finger jointed pine stiles. Wood end rails are butt jointed and pressure fitted with contact cement to the wood stiles at the corners.

Frame Construction: The frame is constructed from finger jointed Ponderosa Pine measuring 4.5625" wide x 1.25" thick. The header is joined to the side jambs with (3) 16ga. 1/2" crown x 2" long staples at each side. The threshold is joined to the side jambs with (2) 16ga. 1/2" crown x 2.5" long staples at each side. The units use an Outswing Bumpface threshold measuring 4.625" long x 1.0" high or a High Dam threshold measuring 5.25" long x 1.75" high.



3-0 x 6-8 OUTSWING
ELEVATION
VIEWED FROM INTERIOR

TABLE OF CONTENTS	
SHEET #	DESCRIPTION
1	TYPICAL ELEVATIONS & GENERAL NOTES
2	VERTICAL CROSS SECTIONS & BILL OF MATERIALS
3	HORIZONTAL CROSS SECTION
4	ANCHORING LOCATIONS & DETAILS
5	UNIT COMPONENTS & DOOR MODELS

DESIGN PRESSURE RATING	
UNIT TYPE	WHERE WATER INFILTRATION REQUIREMENT IS NEEDED
SINGLE (with high dam threshold)	+ 60.0 PSF – 60.0 PSF
SINGLE (with standard threshold)	+ 55.0 PSF – 55.0 PSF

Approved as complying with the Florida Building Code
Date June 6, 2007
NOA# 01-1219.05
Miami Dade Product Control Division
By Manuel Perez

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

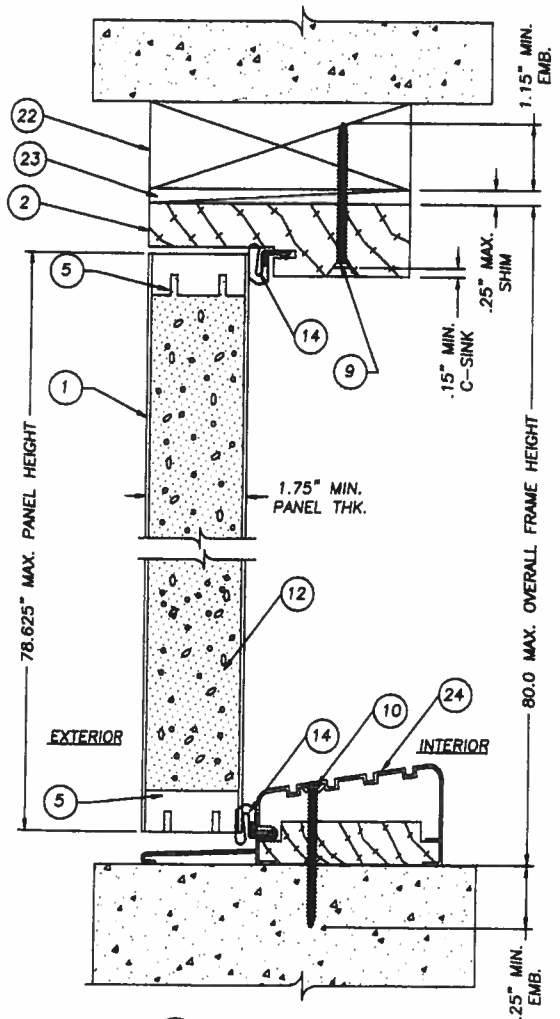
PRODUCT: "LANDMARK WOODEDGE OPAQUE" SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: TYPICAL ELEVATION & GENERAL NOTES

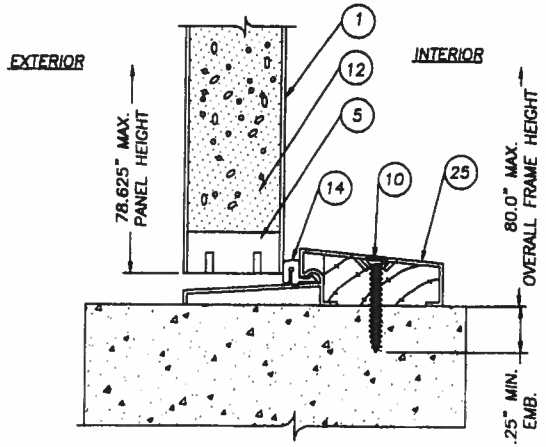
NO.	DATE	REVISIONS	WN	BY
1	1/30/02	GENERAL REVISION		

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 1 OF 5



1/2 VERTICAL CROSS SECTION
OUTSWING UNIT (OPTION 1)
HIGH DAM THRESHOLD
(SEE DESIGN PRESURE TABLE)



1/2 VERTICAL CROSS SECTION
OUTSWING UNIT (OPTION 2)
STANDARD BUMP THRESHOLD
(SEE DESIGN PRESURE TABLE)

Item	DESCRIPTION	Material
1	DOOR SKIN (25 GA. .018" MIN.)	STEEL
2	4 9/16" HEADER	WOOD
3	4 9/16" LATCH JAMB	WOOD
4	4 9/16" HINGE JAMB	WOOD
5	TOP AND BOTTOM RAIL	WOOD
6	RADIUS & SQ. CORNER LEAF 4" X 4" HINGE	STEEL
7	#10 x 3/4" lg. (Hinge to Jamb)	STEEL
8	NOT USED	
9	#8 x 2-1/2" PFH WS	STEEL
10	3/16" TAPCON ANCHOR (ELCO)	STEEL
11	NOT USED	-
12	BASF FOAM CORE	
13	#10 x 3/4" PFH WS (Hinge to Door)	STEEL
14	COMPRESSION WEATHER-STRIP (Therma-Tru)	
15	NOT USED	-
16	#8 x 1/2" LG. TYPE "AB" PANHEAD	STEEL
17	NOT USED	STEEL
18	KWIKSET 700 SERIES PASSAGE	
19	KWIKSET 700 SERIES DEADBOLT	
20	LATCH & DEADBOLT WOODBLOCK 3.0" WIDE x 3.5" HIGH	WOOD
21	#10 x 2" LG. PFH WOOD SCREW (Hinge to Jamb)	STEEL
22	2x WOOD SUB BUCK	WOOD
23	MAX. 1/4" SHIM MATERIAL	WOOD
24	HIGH DAM THRESHOLD (Imperial)	ALUMINUM
25	STANDARD BUMP THRESHOLD	ALUMINUM
26	HINGE STYLE	WOOD
27	LATCH STYLE	WOOD

Approved as complying with the Florida Building Code
Date June 6, 2002
NOA# 01-1219.03
Miami Dade Product Control Division
By

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

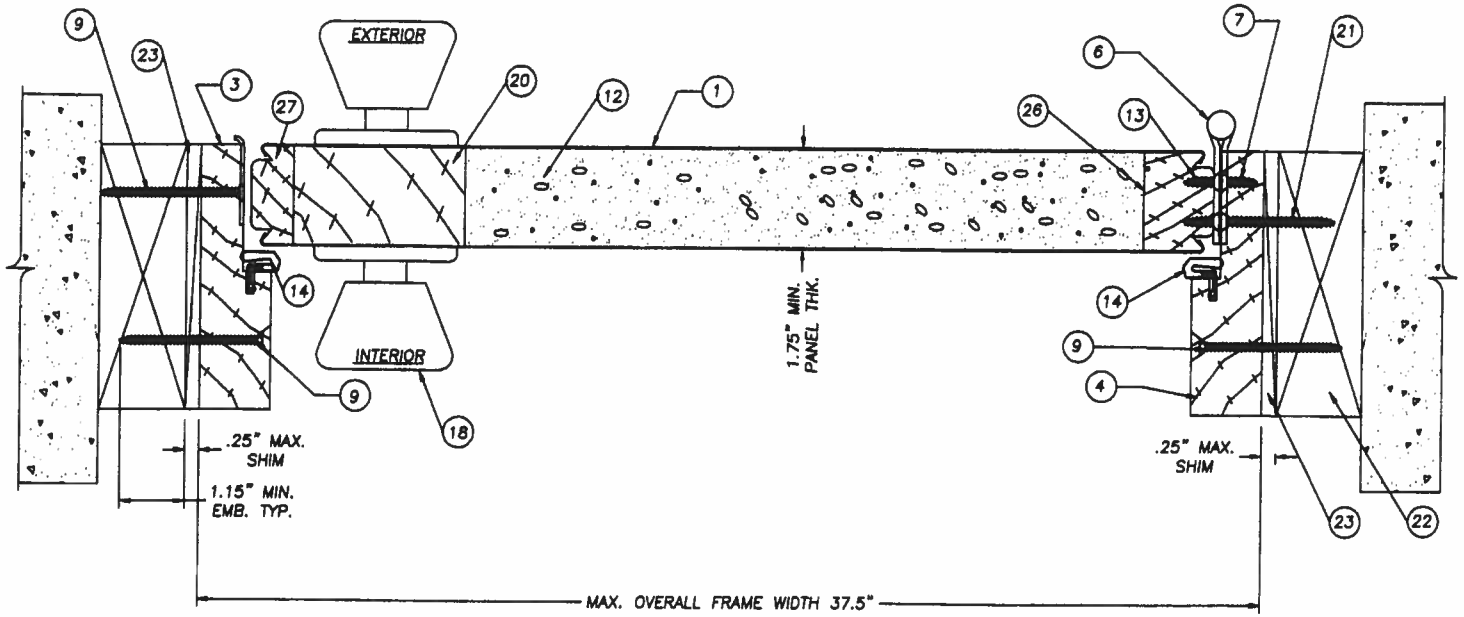
PRODUCT: "LANDMARK WOODEDGE OPAQUE" SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: VERTICAL CROSS SECTION

NO.	DATE	REVISIONS	BY
1	1/30/02	GENERAL REVISION	WLN

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 2 OF 5



1/3 HORIZONTAL CROSS SECTION
HINGE & LATCH JAMB TO BUCK

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

PRODUCT: LANDMARK WOODEDGE OPAQUE SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: HORIZONTAL CROSS SECTION

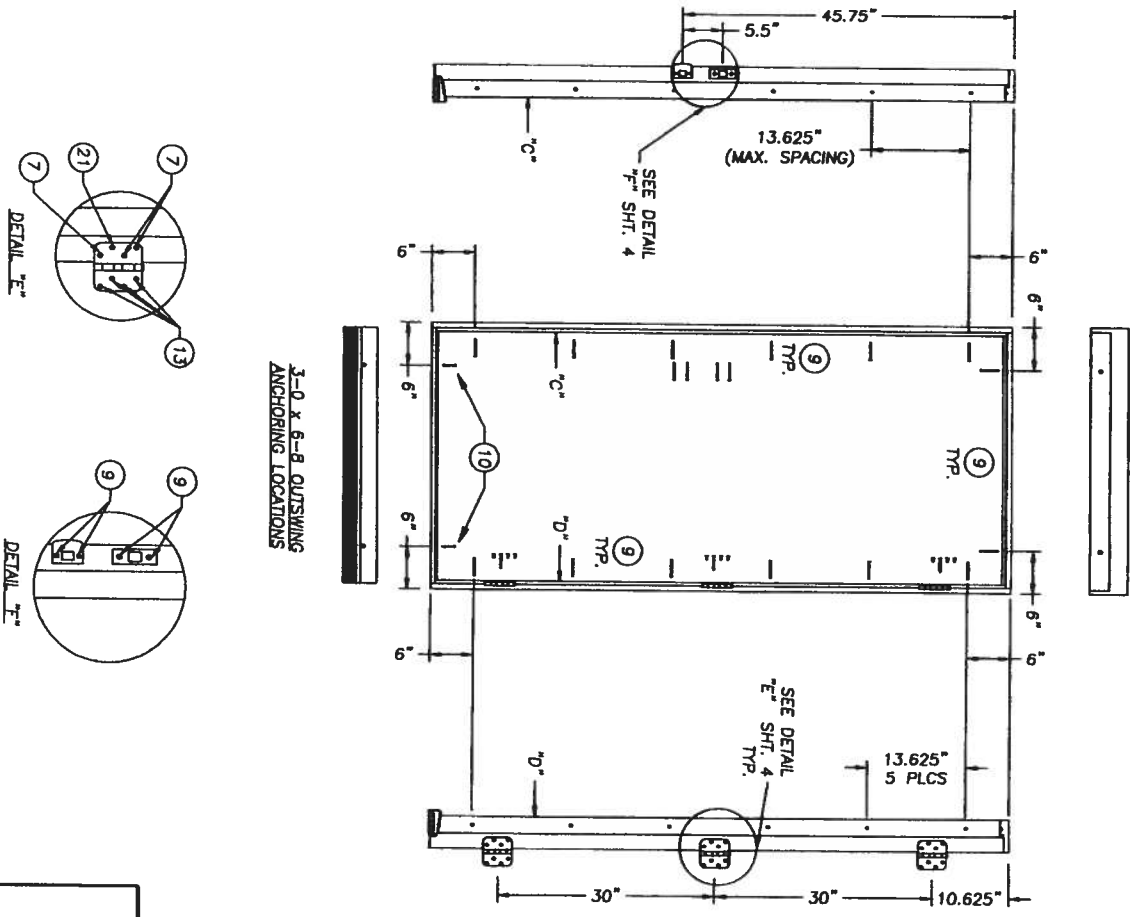
NO.	DATE	GENERAL REVISION	BY
1	1/30/02		WN

REVISIONS

RW BUILDING CONSULTANTS, INC.
813.684.3831

Approved as complying with the Florida Building Code
Date June 6, 2002
NOA# 01-1219.05
Miami Dade Product Control Division
By

DATE: 11/28/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 3 OF 5



Approved as complying with the Florida Building Code
Date: June 19, 2002
NOA# 01-1219.02
Miami Dade Product Control Division
By: [signature]

1	1/30/02	GENERAL REVISION	WN
NO.	DATE	REVISIONS	BY

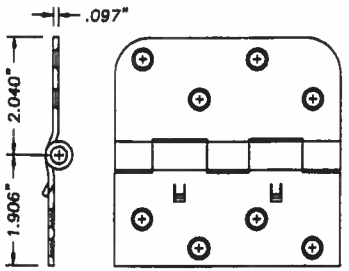
RW BUILDING CONSULTANTS, INC. 813.884.3831

DATE: 11/27/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 4 OF 5

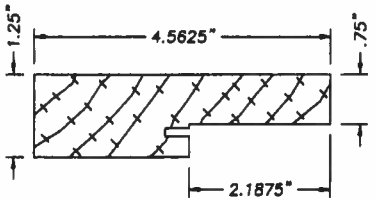
PRODUCT:
"LANDMARK WOODEDGE OPAQUE" SINGLE 6'8 OUTSWING IN WOOD FRAME

PART OR ASSEMBLY:
ANCHORING LOCATIONS & DETAILS

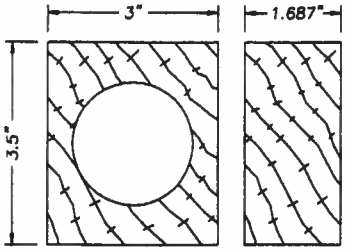
THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827



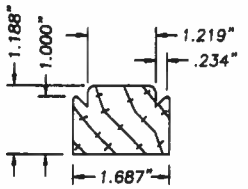
(6) G.P. & SQ. CORNER LEAF
4" x 4" HINGE



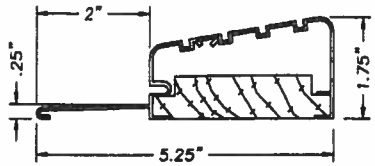
(2)(3)(4) 4 9/16" WOOD FRAME



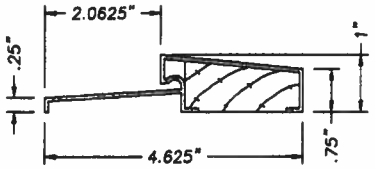
(20) LATCH & DEADBOLT WOOD BLOCK
W/CROSS BORE WOOD BLOCK



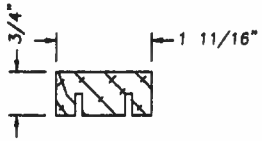
(26) HINGE STILE
MATERIAL: CLEAR SPRUCE



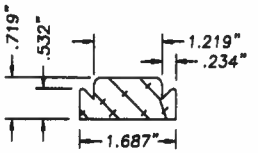
(24) OUTSWING HIGN DAM
ALUMINUM THRESHOLD



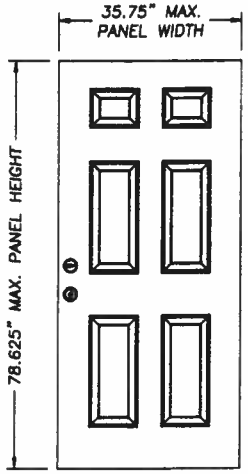
(25) OUTSWING BUMP FACE
ALUMINUM THRESHOLD
.0625" WALL THK.



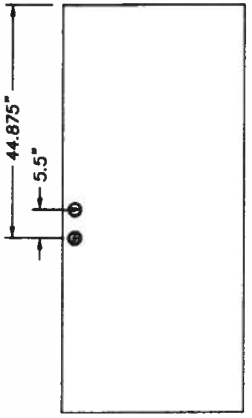
(5) WOOD RAIL
MATERIAL: CLEAR SPRUCE



(27) LATCH STILE
MATERIAL: CLEAR SPRUCE



DOOR MODEL
6 PANEL EMBOSSED



DOOR MODEL
FLUSH

Approved as complying with the
Florida Building Code
Date June 6, 2002
NOA# 01-1219.04
Miami Dade Product Control
Division
By

THERMA-TRU CORPORATION
1687 WOODLANDS DRIVE
MAUMEE, OHIO
PH. (800) 537-8827

PRODUCT: LANDMARK WOODEDGE OPAQUE SINGLE 6'8 OUTSWING DOOR IN WOOD FRAME

PART OR ASSEMBLY: UNIT COMPONENTS & DOOR MODELS

NO.	DATE	REVISIONS	BY
1	1/30/02	GENERAL REVISION	WN

RW BUILDING CONSULTANTS, INC.
813.684.3831

DATE: 11/28/01
SCALE: NTS
DWG. BY: WLN
CHK. BY: RW
DRAWING NO.: S-2189
SHEET 5 OF 5