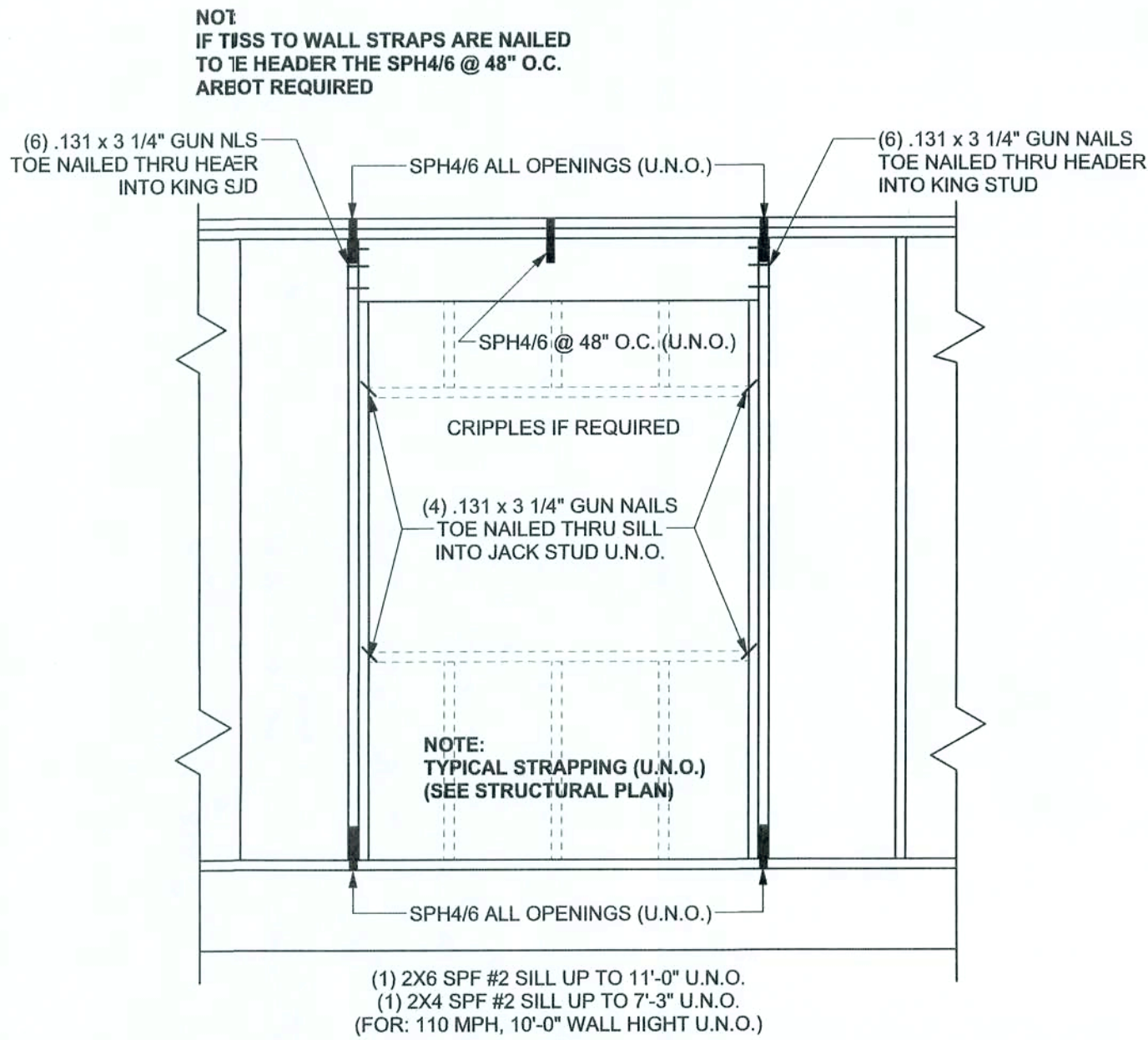


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

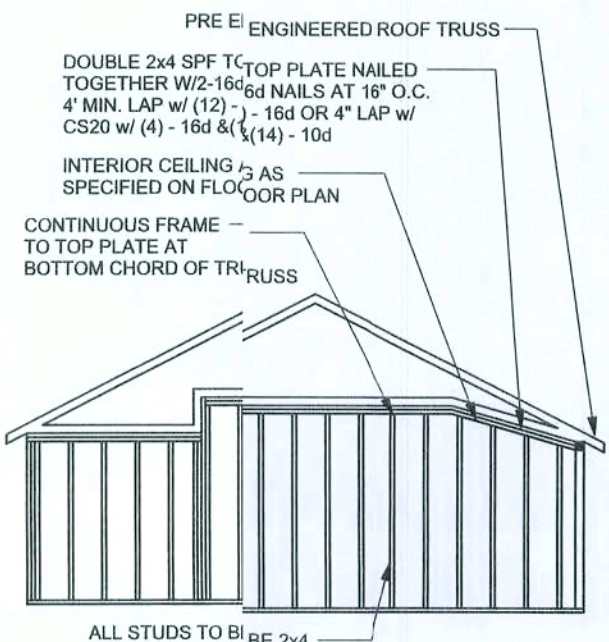
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON-LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



TYPICAL HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	LAMBERSTRAND	1700	1.7
LVL	MICROLAM	1800	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL
SCALE: N.T.S.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 8" x 6" W4 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWW OR REINFORCEMENT. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI, ALL LAP SPLICES 4" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2,400psi, E = 1,800,000psi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS @ 12" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY CBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block.
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

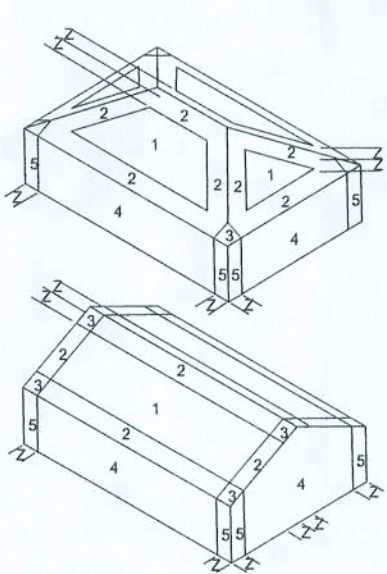
DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 50 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1.) BASIC WIND SPEED = 110 MPH
- 2.) WIND EXPOSURE = B
- 3.) WIND IMPORTANCE FACTOR = 1.0
- 4.) BUILDING CATEGORY = II
- 5.) ROOF ANGLE = 10-45 DEGREES
- 6.) MEAN ROOF HEIGHT = <30 FT
- 7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)	10	100
1	19.9 - 21.8	18.1	-18.1
2	19.9 - 25.5	18.1	-21.8
2 O/H	140.8	-	-40.6
3	19.9 - 25.5	18.1	-21.8
3 O/H	-68.3	-	-42.4
4	21.8 - 23.6	18.5	-20.4
5	21.8 - 29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	-29.1
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS



WINDLOAD ENGINEER: Mark Disway, PE No. 53915, POB 88, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stud dimensions supstrate scaled dimensions. Refer all cautions to Mark Disway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, as the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISWAY
P.E. 53915
SAL

Wayne & Kathy
Pelley Addition

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PRINTED DATE:
May 07, 2008

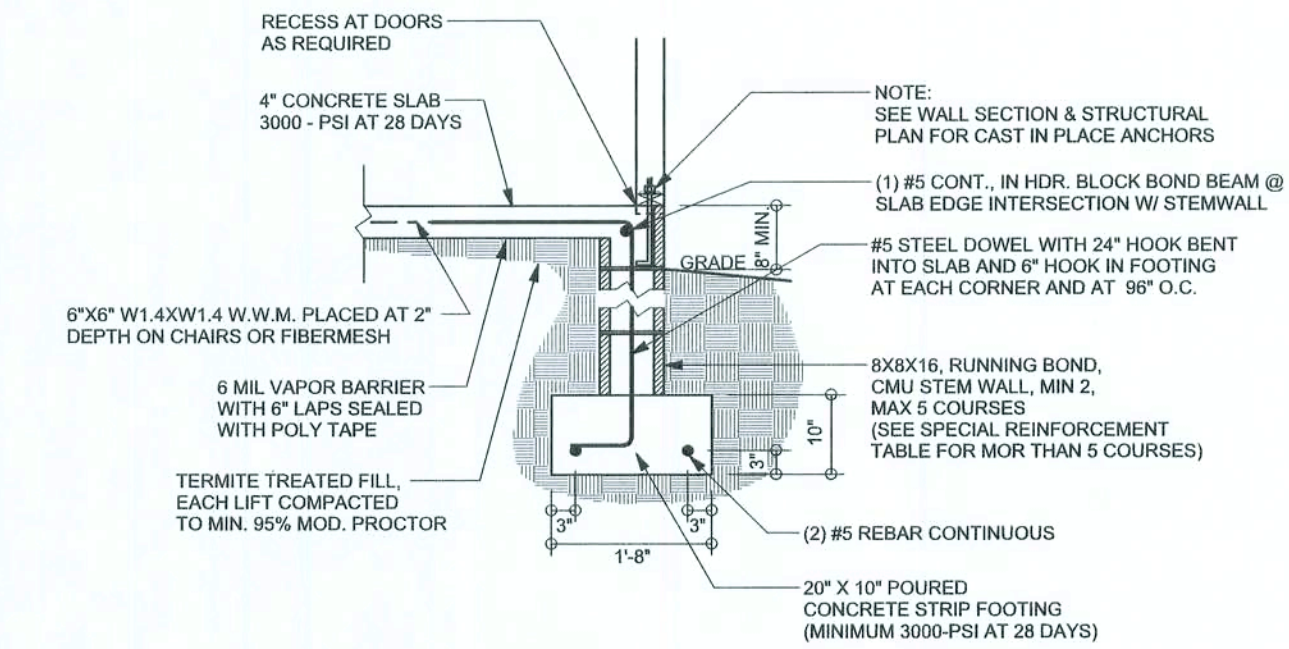
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FINALS DATE:
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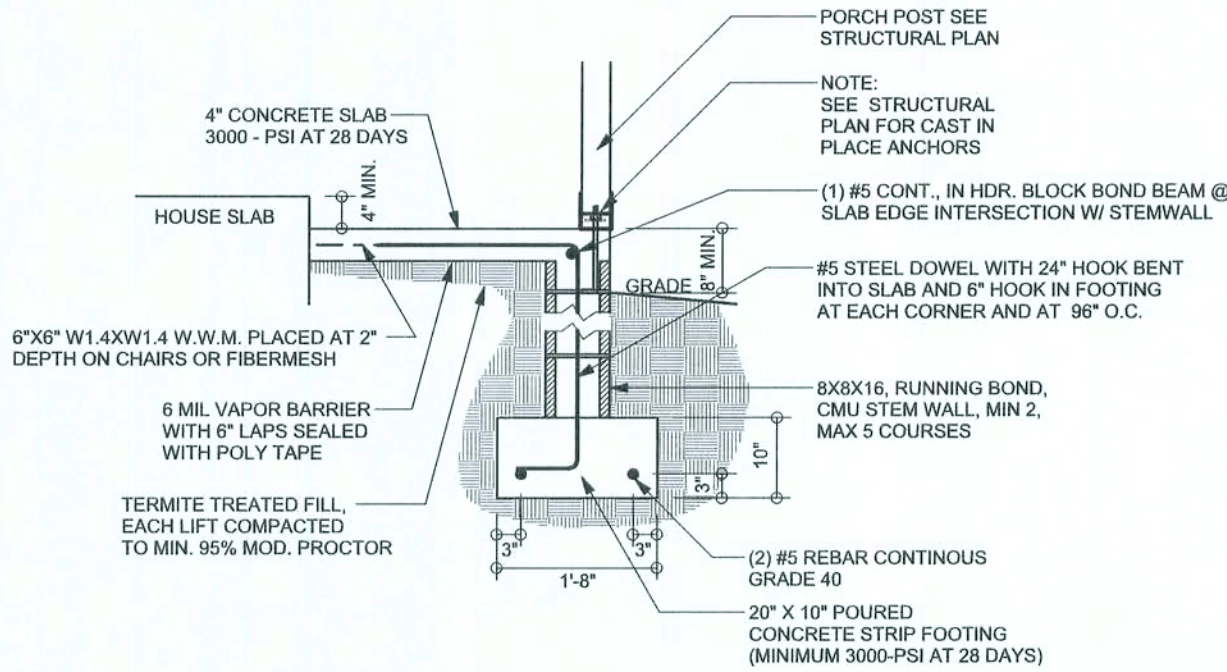
JOB NUMBER:
805022
DRAWING NUMBER

S-1

OF 2 SHEETS



F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

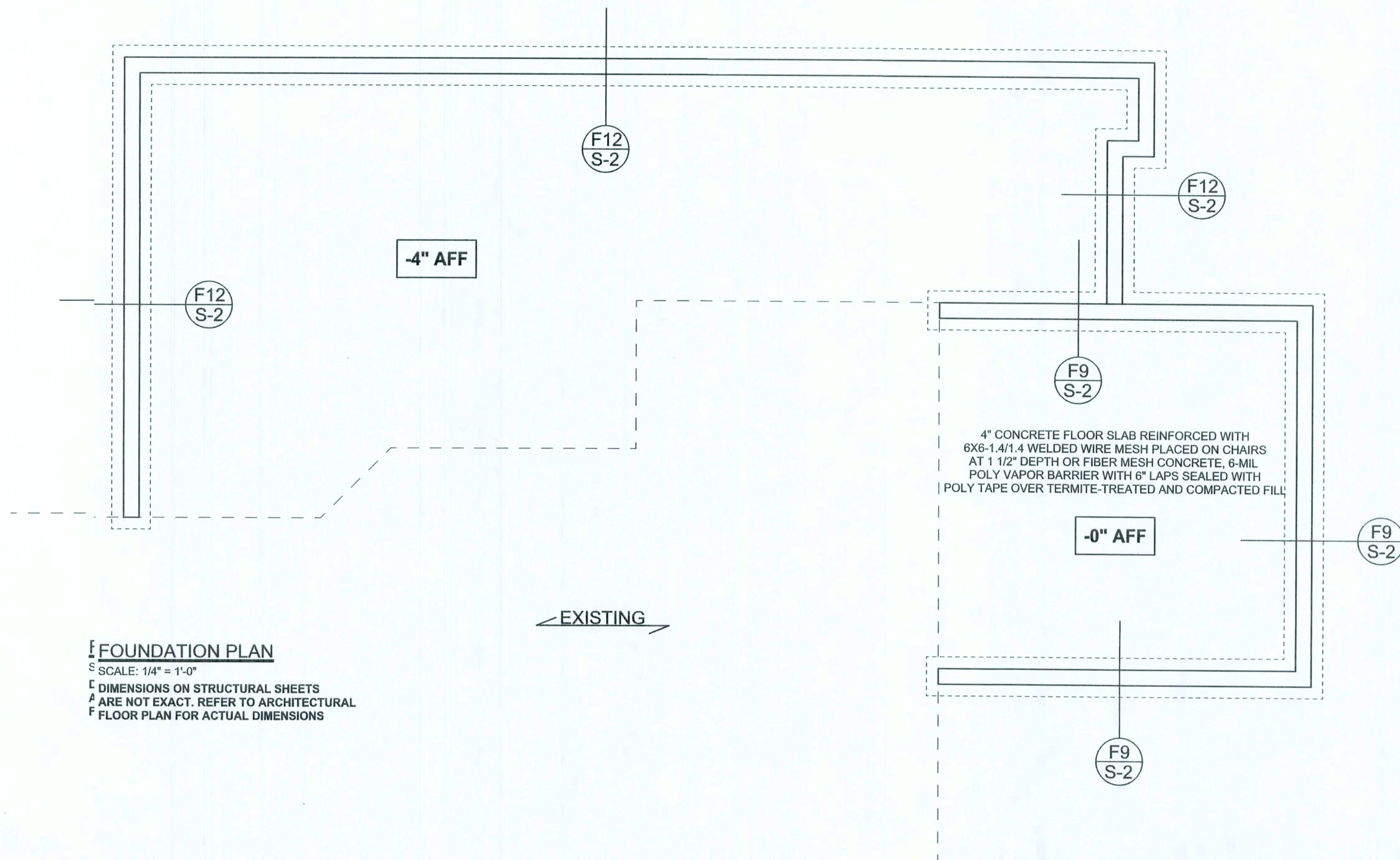


F12 S-2 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 18"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN

S SCALE: 1/4" = 1'-0"
D DIMENSIONS ON STRUCTURAL SHEETS
A ARE NOT EXACT. REFER TO ARCHITECTURAL
F FLOOR PLAN FOR ACTUAL DIMENSIONS

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSH-03, BCSH-61, BCSH-62, & BCSH-63. BCSH-61, BCSH-62, & BCSH-63 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

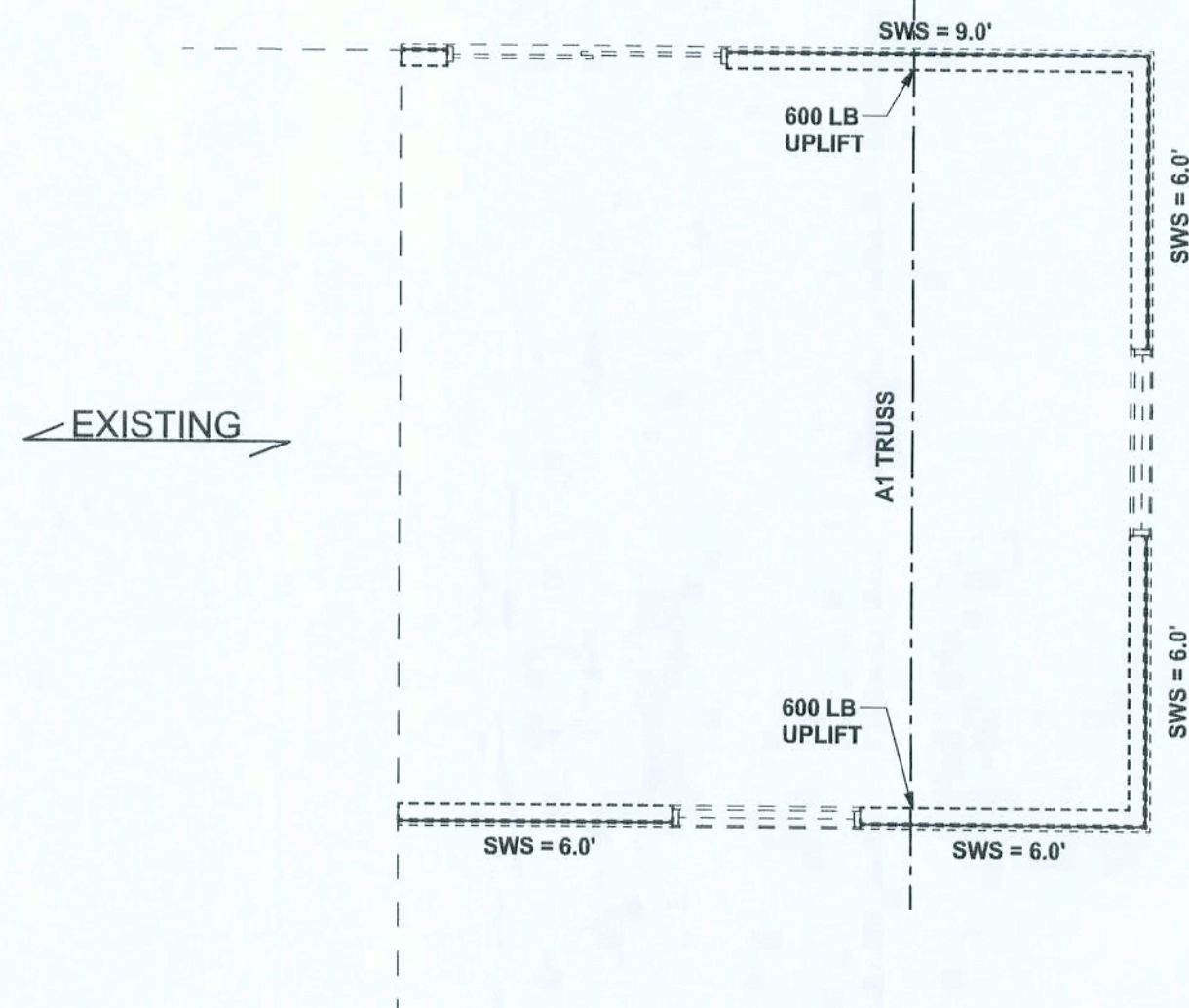
WALL LEGEND

S SWS = 0.0'	1ST FLOOR EXTERIOR WALL
S SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

HEADER LEGEND

(2) 2X12X0', 1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
NUMBER OF KING STUDS (FULL LENGTH)	
NUMBER OF JACK STUDS (UNDER HEADER)	
SPAN OF HEADER	
SIZE OF HEADER MATERIAL	
NUMBER OF PLIES IN HEADER	

USE H2.5A (5351b) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	10.0'	12.0'
LONGITUDINAL	8.0'	21.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. UNIVERSAL FOREST PRODUCTS JOB #Pelley

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Discoway, P.E. No. 53915, POB 86, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. If resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R912.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
P.E. 53915

08 MAY 08
SIA

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Pelley addition

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PRINTED DATE:
May 07, 2008

STRUCTURAL BY:

FINALS DATE:
7 May 08

JOB NUMBER:
805022

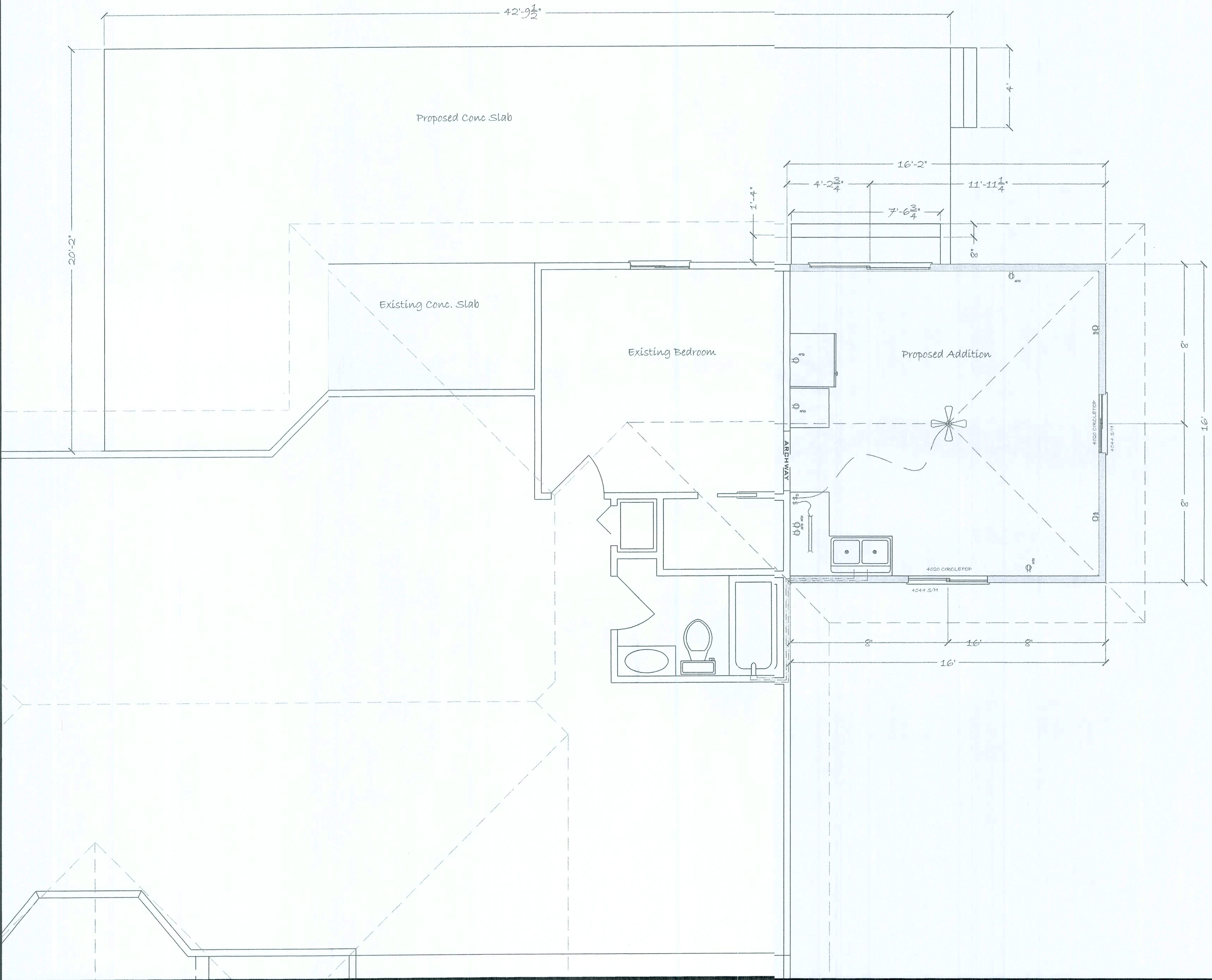
DRAWING NUMBER

S2

OF 2 SHEETS

FLOOR/ELECTRICAL PLAN

SCALE $\frac{3}{8} = 1' - 0''$

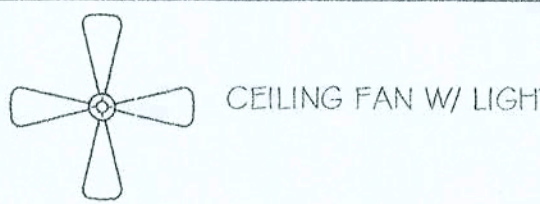


NOTE:

ALL ELECTRICAL
IS BASIC IN NATURE
AND EXACT DETAILS TO BE
PROVIDED BY OWNER

ELECTRICAL SYMBOLS

	PANEL BOX
	THREE-WAY SWITCH
	SINGLE-POLE SWITCH
	DIMMER SWITCH
	DUPLEX RECEPTACLE OUTLET
	ARC FAULT CIRCUIT INTERCEPTOR
	WATER PROOF OUTLET
	220 VOLT OUTLET
	CEILING OUTLET FIXTURE
	RECESSED CEILING OUTLET FIXTURE
	FLORESCENT LIGHT FIXTURE
	SMOKE DETECTOR
	EXHAUST FAN
	CEILING OUTLET FIXTURE W/ FULL CORD
	HOSE B/B
	TELEVISION HOOK-UP
	TELEPHONE HOOK-UP
	COMPUTER HOOK-UP
	F.O.D. LIGHT



NOTES

1. ACTUAL SLOPE AND/OR VARIATIONS IN GRADE CONDITIONS TO BE DETERMINED ON SITE PRIOR TO CONSTRUCTION
2. CHECK ALL DIMENSIONS FOR ACCURACY PRIOR TO CONSTRUCTION
3. EXACT DESCRIPTION AND/OR SPECIFICATIONS NOT NOTED ON PLANS TO BE PROVIDED BY OWNER AND/OR CONTRACTOR (DOORS, WINDOWS, CABINETS, ELECTRICAL, HVAC, FINISHES, ECT.)
4. ALL ELECTRICAL, PLUMBING & HVAC TO BE INSTALLED BY CURRENTLY LICENSED FLORIDA CONTRACTORS IN ACCORDANCE WITH STATE AND LOCAL CODES
5. ALL CONSTRUCTION MUST CONFORM TO CURRENT STATE AND LOCAL CODES WHERE APPLICABLE:
 - FLORIDA BUILDING CODE 2007 EDITION
 - FLORIDA FIRE CODE 2007 EDITION
 - NATIONAL ELECTRIC CODE (NEC)

North Florida COMMUNITY COLLEGE
DRAFTING DEPARTMENT
325 NW Turner Davis Drive
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CERTIFICATION

PROJECT NAME

Chris Pelley

TITLE

Addition

DWG. BY

Chris Pelley

CHK. BY

Lauri Newton

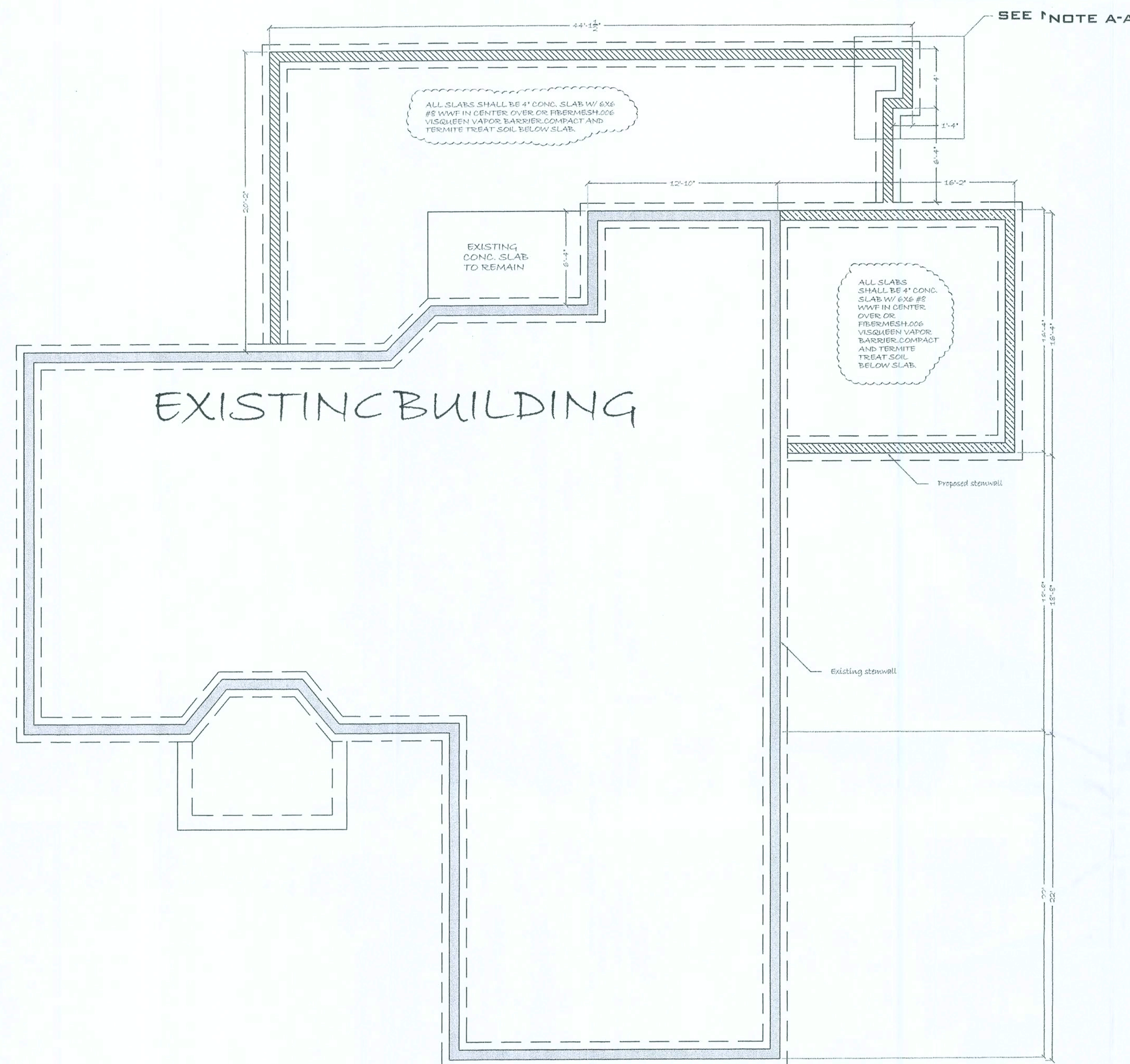
SCALE

3/8" = 1'-0"

DATE

PROJECT No.

PG - 2

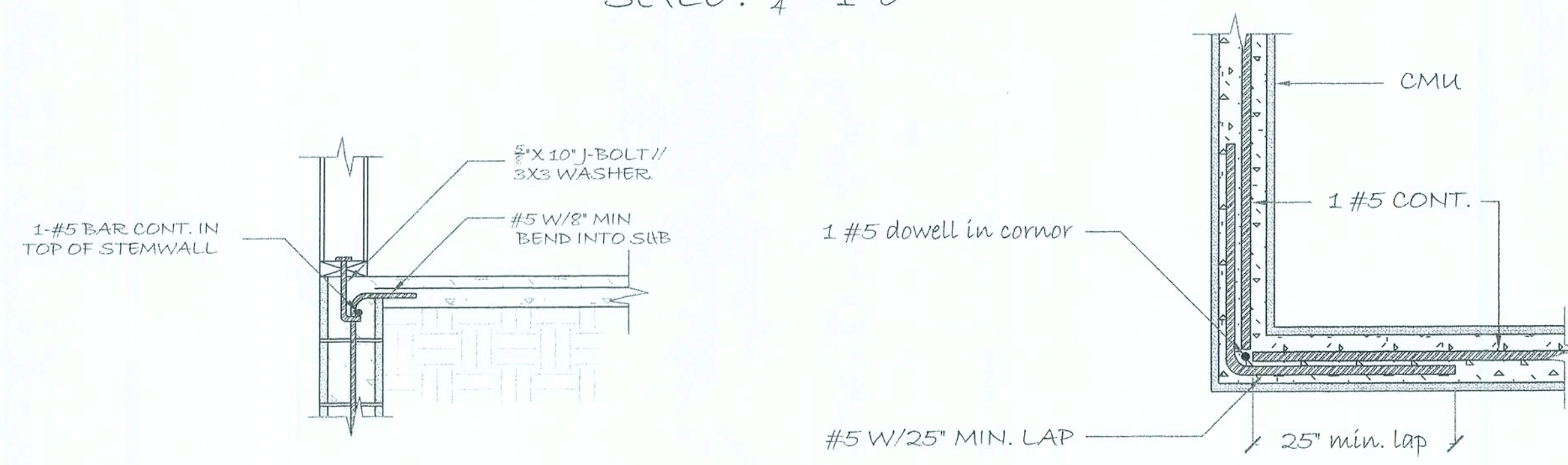


FOUNDATION PLAN

SCALE: $\frac{3}{16}" = 1'-0"$

CONTINUITY REINFORCEMENT AT CORNERS

SCALE: $\frac{3}{4}" = 1'-0"$



STEMWALL ELEVATION VIEW

STEM WELL PLAN VIEW

ARCH GRADE SHINGLES (OWNER SELECTED)

15 LB. UNDERLAYMENT

$\frac{1}{2}"$ CDX ROOF DECKING FASTEN W/8D COMMON 6" EDGES, 8" FIELD

ENGINEERED TRUSSES @ 24" O.C. (TYP)

2X6 FASICA P.T. W/1X4 DRIP STRIP P.T. 5 METAL EVE DRIP

2X4 LOOKOUT @ 24" O.C.

$\frac{1}{2}"$ CDX SOFFIT

2X4 CONT NAILER

R-13 BATT INSULATION

Brick Veneer (OWNER SELECTED)

$\frac{7}{16}"$ OSB SHEATHING (SPlice OSB IN CENTER FASTEN W/8d COMMON 4" O.C. EDGES, 8" O.C. FIELD)

2X4 STUD @ 16" O.C.

$\frac{5}{8}"$ X10" J-BOLT @ 48" O.C. ALSO LOCATED @ 6" MAX. EA. SIDE OF ALL WINDOW AND DOOR OPENINGS @ ALL SOLE PLATE SPLICES. AND @ ALL CORNERS

INSTALL SIMPSON SP4 TIES @ CORNERS OPENINGS, 5 @ 32" O.C. STUD TO SOLE PLATE

1-#5 REBAR CONT. FILL TOP BLOCK COURSE W/CONCRETE

20X10 CONC. FOOTING CONT. W/3-#5 STL. RODS CONT.

ATTACH GABLE END TRUSS W/SIMPSON LSTA 12 STRAP FROM 2X4X8" BRACERS OVER BOTTOM CHORD TO STUDS @ 48" O.C. PROVIDE SIMPSON LTP4 SHEAR CONNECTOR, BOTTOM CHORD TO TOP PLATE, CORNERS, 5 48" O.C.

SIMPSON SP4 TIE @ 32" O.C.

2X4 DOUBLE TOP PLATE.

$\frac{1}{2}"$ GYPSUM

NOTE: INSTALL SIMPSON LSTA21 STRAP W/ 16-10d NAILS EA. SIDE OF ALL DOORS AND WINDOW HEADERS (TOP PLATE TO HEADER TO STUD/TRIMMER CONNECTION)

2X4 P.T. SOLE PLATE

4" CONC. SLAB W/6X6 #8 WWF IN CENTER OVER .006 VISQUEEN VAPOR BARRIER COMPACT AND TERMITE TREAT SOIL BELOW SLAB.

#5 VERTICAL STL. ROD W/STANDARD AND 8" MIN. BEND INTO SLAB. IN FULLY GROUTED CELL 5 @ 72" O.C.

WALL SECTION

SCALE: $\frac{3}{4}" = 1'-0"$

NOTES

1-ELEVATIONS INDICATE LEVEL SITE CONDITIONS TO BE DETERMINED ON SITE PRIOR TO CONSTRUCTION

2-CHECK ALL DIMENSIONS FOR ACCURACY PRIOR TO CONSTRUCTION

3. ALL CONCRETE SHALL BE 3000 PSI. ALL REBAR SHALL BE GRADE 40.

4. OVERLAP ALL REBAR SPLICES 25" MINIMUM.

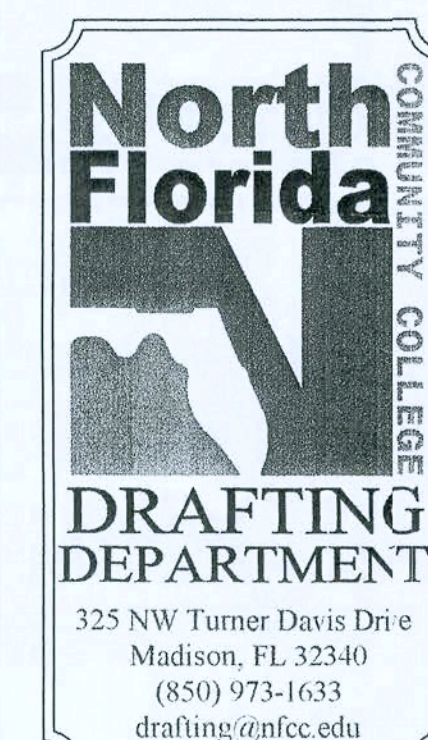
5-ALL CONSTRUCTION MUST CONFORM TO CURRENT STATE AND LOCAL CODES WHERE APPLICABLE:

-FLORIDA BUILDING CODE 2007 EDITION

-FLORIDA FIRE CODE 2007 EDITION

-NATIONAL ELECTRIC CODE (NEC)

A-A STAIRS TO BE INCORPORATED INTO PROPOSED STEMWALL



CERTIFICATION

PROJECT NAME

WYNNE/LATHY
Chris Pelley

TITLE

Foundation Plan

DWG. BY

Chris Pelley

CHK. BY

Lauri Newton

SCALE

Varies

DATE

PROJECT No.

PAGE No.

PG-3